Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 200342 - ROOF DESIGN INFO

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: SOUTHTRUST CONSTRUCTION Project Name: Adams Residence Model: -
Lot/Block: - Subdivision: -
Address: -
City: Alachua State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017
Wind Code: ASCE 7-10 [All HeighII
Roof Load: 37.0 psf

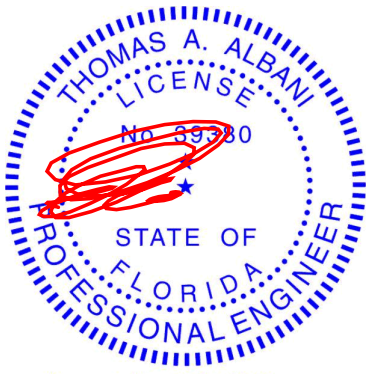
Design Program: OnLine Plus 30.0.071
Wind Speed: 130 mph
Floor Load: N/A psf

This package includes 40 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T20412060	R1	6/8/20	23	T20412082	R22	6/8/20
2	T20412061	R2	6/8/20	24	T20412083	R23	6/8/20
3	T20412062	R3	6/8/20	25	T20412084	R24	6/8/20
4	T20412063	R4	6/8/20	26	T20412085	T1	6/8/20
5	T20412064	R5	6/8/20	27	T20412086	T2	6/8/20
6	T20412065	R6	6/8/20	28	T20412087	V1	6/8/20
7	T20412066	R7	6/8/20	29	T20412088	V2	6/8/20
8	T20412067	R8	6/8/20	30	T20412089	V3	6/8/20
9	T20412068	R9	6/8/20	31	T20412090	V4	6/8/20
10	T20412069	R10	6/8/20	32	T20412091	V5	6/8/20
11	T20412070	R11	6/8/20	33	T20412092	V6	6/8/20
12	T20412071	R12	6/8/20	34	T20412093	V7	6/8/20
13	T20412072	R13	6/8/20	35	T20412094	V8	6/8/20
14	T20412073	R14	6/8/20	36	T20412095	V9	6/8/20
15	T20412074	R15	6/8/20	37	T20412096	V10	6/8/20
16	T20412075	R16	6/8/20	38	T20412097	V11	6/8/20
17	T20412076	R16A	6/8/20	39	T20412098	V12	6/8/20
18	T20412077	R17	6/8/20	40	T20412099	V13	6/8/20
19	T20412078	R18	6/8/20				
20	T20412079	R19	6/8/20				
21	T20412080	R20	6/8/20				
22	T20412081	R21	6/8/20				

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Ridgway Roof Trusses.

Truss Design Engineer's Name: Albani, Thomas
My license renewal date for the state of Florida is February 28, 2021.



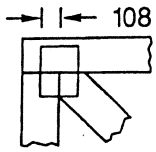
Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
June 8,2020

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Table of Contents

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

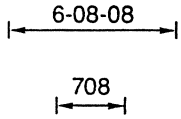
LATERAL BRACING

1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

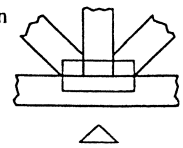
When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Robbins Engineering, Inc.

CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

Wood Truss Council of America
5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

Standard Responsibilities in the Design Process Involving Metal Plate Connected Wood Trusses

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPEAND DEFINITIONS

1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.

1.2 The following definitions shall apply:

1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.

1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.

1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.

1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.

1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.

1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.

1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.

1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.

1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.

1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.

1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;

3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:

3.2.1 All truss orientations and locations;

3.2.2 Information to fully determine all truss profiles;

3.2.3 Adequate support of the Truss and all truss bearing conditions;

3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.

3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;

3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;

3.2.7 Allowable vertical and horizontal deflection criteria;

3.2.8 Proper transfer of design loads affecting the Truss; and

3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.

3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.

3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.

4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.

4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.

4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.

4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.

4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.

5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.

5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.2 For each Truss Design Drawing, set forth as a minimum the following:

6.2.1 Slope or depth, span and spacing;

6.2.2 Location of all joints;

6.2.3 Required bearing widths;

6.2.4 Design loads as applicable:

6.2.4.1 Top chord live load (including snow loads);

6.2.4.2 Top chord dead load;

6.2.4.3 Bottom chord live load;

6.2.4.4 Bottom chord dead load;

6.2.4.5 Concentrated loads and their points of application; and

6.2.4.6 Controlling wind and earthquake loads;

6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;

6.2.6 Each reaction force and direction;

6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;

6.2.8 Lumber size, species, and grade for each member;

6.2.9 Connection Requirements for:

(a) Truss to Truss girder;

(b) Truss ply to ply; and

(c) Field splices;

6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;

6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and

6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

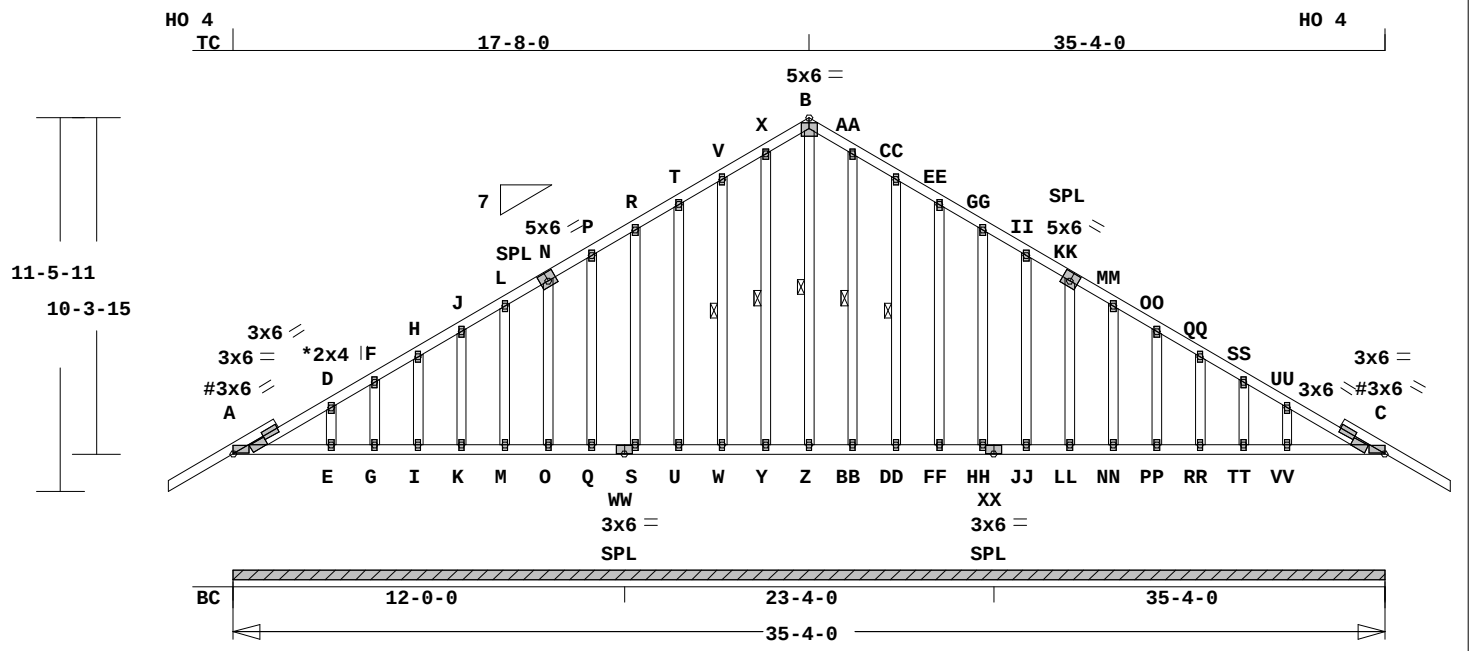
7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R1	1	TR	350400	7	0	0	T20412060

Adams Residence



ALL PLATES ARE MT2020, # = PLATE SELECTED IN PLATE MONITOR

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.170" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 387.7 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.05  2x 4  SP-#2
BC  0.05  2x 4  SP-#2
GW  0.21  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 35- 4- 0
 or   48.0"   0- 0- 0 35- 4- 0
 BC  Cont.    0- 0- 0 35- 4- 0
 or   72.0"   0- 0- 0 35- 4- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  W -V   Y -X   Z -B   BB-AA
  DD-CC
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
        Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   2615   201 U   177 R

Jt   Brg Size   Required
A     424.0"     0"-to- 424"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -D  0.05   129 C  0.00  0.05
D -F  0.04   108 C  0.00  0.04
F -H  0.01    90 C  0.00  0.01
H -J  0.01    85 C  0.00  0.01
J -L  0.01    79 C  0.00  0.01
L -N  0.01    74 C  0.00  0.01
N -P  0.01    68 C  0.00  0.01
P -R  0.01    62 C  0.00  0.01
R -T  0.01    57 T  0.00  0.01
T -V  0.01    81 T  0.00  0.01
V -X  0.01   106 T  0.00  0.01
X -B  0.03   122 T  0.02  0.01
B -AA 0.03   122 T  0.02  0.01
AA-CC 0.01   106 T  0.00  0.01
CC-EE 0.01    81 T  0.00  0.01
EE-GG 0.01    57 T  0.00  0.01
GG-II 0.01    62 C  0.00  0.01
II-KK 0.01    68 C  0.00  0.01
KK-MM 0.01    74 C  0.00  0.01
MM-OO 0.01    79 C  0.00  0.01
OO-QQ 0.01    85 C  0.00  0.01
QQ-SS 0.01    90 C  0.00  0.01
SS-UU 0.04   108 C  0.00  0.04
UU-C  0.05   129 C  0.00  0.05
--------Bottom Chords---------
A -E  0.05     9 T  0.00  0.05
E -G  0.03     0 T  0.00  0.03
G -I  0.01     0 T  0.00  0.01
I -K  0.01     0 T  0.00  0.01
K -M  0.00     0 T
M -O  0.01     0 T  0.00  0.01
O -Q  0.01     0 T  0.00  0.01
Q -WW 0.00     0 T
WW-S  0.00     0 T
S -U  0.01     0 T  0.00  0.01
U -W  0.01     0 T  0.00  0.01
W -Y  0.00     0 T
Y -Z  0.00     0 T
Z -BB 0.00     0 T
BB-DD 0.00     0 T
DD-FF 0.01     0 T  0.00  0.01
FF-HH 0.01     0 T  0.00  0.01
HH-XX 0.00     0 T
XX-JJ 0.00     0 T
JJ-LL 0.01     0 T  0.00  0.01
LL-NN 0.01     0 T  0.00  0.01
NN-PP 0.00     0 T
PP-RR 0.01     0 T  0.00  0.01
RR-TT 0.01     0 T  0.00  0.01
TT-VV 0.03     0 T  0.00  0.03
VV-C  0.05     9 T  0.00  0.05
----------Gable Webs----------
E -D  0.02   129 C
G -F  0.01    56 C
I -H  0.02    73 C
K -J  0.02    72 C
M -L  0.03    71 C
O -N  0.05    72 C
Q -P  0.06    72 C
S -R  0.08    71 C
U -T  0.10    72 C
W -V  0.13    73 C        1 Br
Y -X  0.14    69 C        1 Br
Z -B  0.21    88 C        1 Br
BB-AA 0.14    69 C        1 Br
DD-CC 0.13    73 C        1 Br
FF-EE 0.10    72 C
HH-GG 0.08    71 C
JJ-II 0.06    72 C
LL-KK 0.05    72 C
NN-MM 0.03    71 C
PP-OO 0.02    72 C
RR-QQ 0.02    73 C
TT-SS 0.01    56 C
VV-UU 0.02   129 C

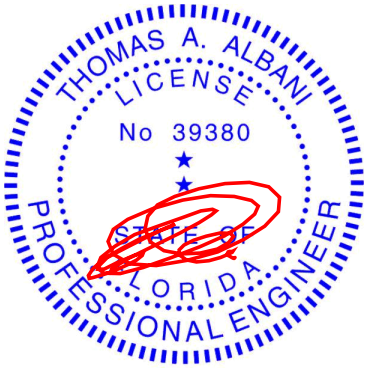
CT Defl   0.00" in ----- L/999
LL Defl   0.00" in ----- L/999
Shear // Grain  in A -D   0.08

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A# MT20  3.0x 6.0 0.2 0.1 0.42
D  MT20  2.0x 4.0 Ctr Ctr 0.00
F  MT20  2.0x 4.0 Ctr Ctr 0.00
H  MT20  2.0x 4.0 Ctr Ctr 0.00
J  MT20  2.0x 4.0 Ctr Ctr 0.00
L  MT20  2.0x 4.0 Ctr Ctr 0.00
N  MT20  5.0x 6.0-0.3 0.5 0.38
P  MT20  2.0x 4.0 Ctr Ctr 0.00
R  MT20  2.0x 4.0 Ctr Ctr 0.00
T  MT20  2.0x 4.0 Ctr Ctr 0.00
V  MT20  2.0x 4.0 Ctr Ctr 0.00
X  MT20  2.0x 4.0 Ctr Ctr 0.00
B  MT20  5.0x 6.0 Ctr-0.2 0.33
AA MT20  2.0x 4.0 Ctr Ctr 0.00
CC MT20  2.0x 4.0 Ctr Ctr 0.00
EE MT20  2.0x 4.0 Ctr Ctr 0.00
GG MT20  2.0x 4.0 Ctr Ctr 0.00
II MT20  2.0x 4.0 Ctr Ctr 0.00
KK MT20  5.0x 6.0 0.3 0.5 0.38
MM MT20  2.0x 4.0 Ctr Ctr 0.00
OO MT20  2.0x 4.0 Ctr Ctr 0.00
QQ MT20  2.0x 4.0 Ctr Ctr 0.00
SS MT20  2.0x 4.0 Ctr Ctr 0.00
UU MT20  2.0x 4.0 Ctr Ctr 0.00
C# MT20  3.0x 6.0-0.3-0.1 0.39
E  MT20  2.0x 4.0 Ctr Ctr 0.00
G  MT20  2.0x 4.0 Ctr Ctr 0.00
I  MT20  2.0x 4.0 Ctr Ctr 0.00
K  MT20  2.0x 4.0 Ctr Ctr 0.00
M  MT20  2.0x 4.0 Ctr Ctr 0.00
O  MT20  2.0x 4.0 Ctr Ctr 0.00
Q  MT20  2.0x 4.0 Ctr Ctr 0.00
WW MT20  3.0x 6.0 Ctr Ctr 0.39
S  MT20  2.0x 4.0 Ctr Ctr 0.00
U  MT20  2.0x 4.0 Ctr Ctr 0.00
W  MT20  2.0x 4.0 Ctr Ctr 0.00
Y  MT20  2.0x 4.0 Ctr Ctr 0.00
Z  MT20  2.0x 4.0 Ctr Ctr 0.00
BB MT20  2.0x 4.0 Ctr Ctr 0.00
DD MT20  2.0x 4.0 Ctr Ctr 0.00
FF MT20  2.0x 4.0 Ctr Ctr 0.00
HH MT20  2.0x 4.0 Ctr Ctr 0.00
XX MT20  3.0x 6.0 Ctr Ctr 0.39
JJ MT20  2.0x 4.0 Ctr Ctr 0.00
LL MT20  2.0x 4.0 Ctr Ctr 0.00
NN MT20  2.0x 4.0 Ctr Ctr 0.00
PP MT20  2.0x 4.0 Ctr Ctr 0.00
RR MT20  2.0x 4.0 Ctr Ctr 0.00
TT MT20  2.0x 4.0 Ctr Ctr 0.00
VV MT20  2.0x 4.0 Ctr Ctr 0.00

# = Plate Monitor used
REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
WARNING  Do Not Cut overframe
  member between outside of
  truss and first tie-plate
  to inside of heel plate.
Design checked for 10 psf non-
  concurrent LL on BC.
Truss designed for wind loads
  in the plane of the truss
  only.  For studs exposed to
  wind (normal to the face),
  see Standard Industry Gable
  End Details as applicable,
  or consult qualified
  Building Designer as per
  ANSI/TPI 1.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :        4.0 psf
  BC Dead Load :        6.0 psf
Max comp. force      129 Lbs
Max tens. force      122 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
```



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

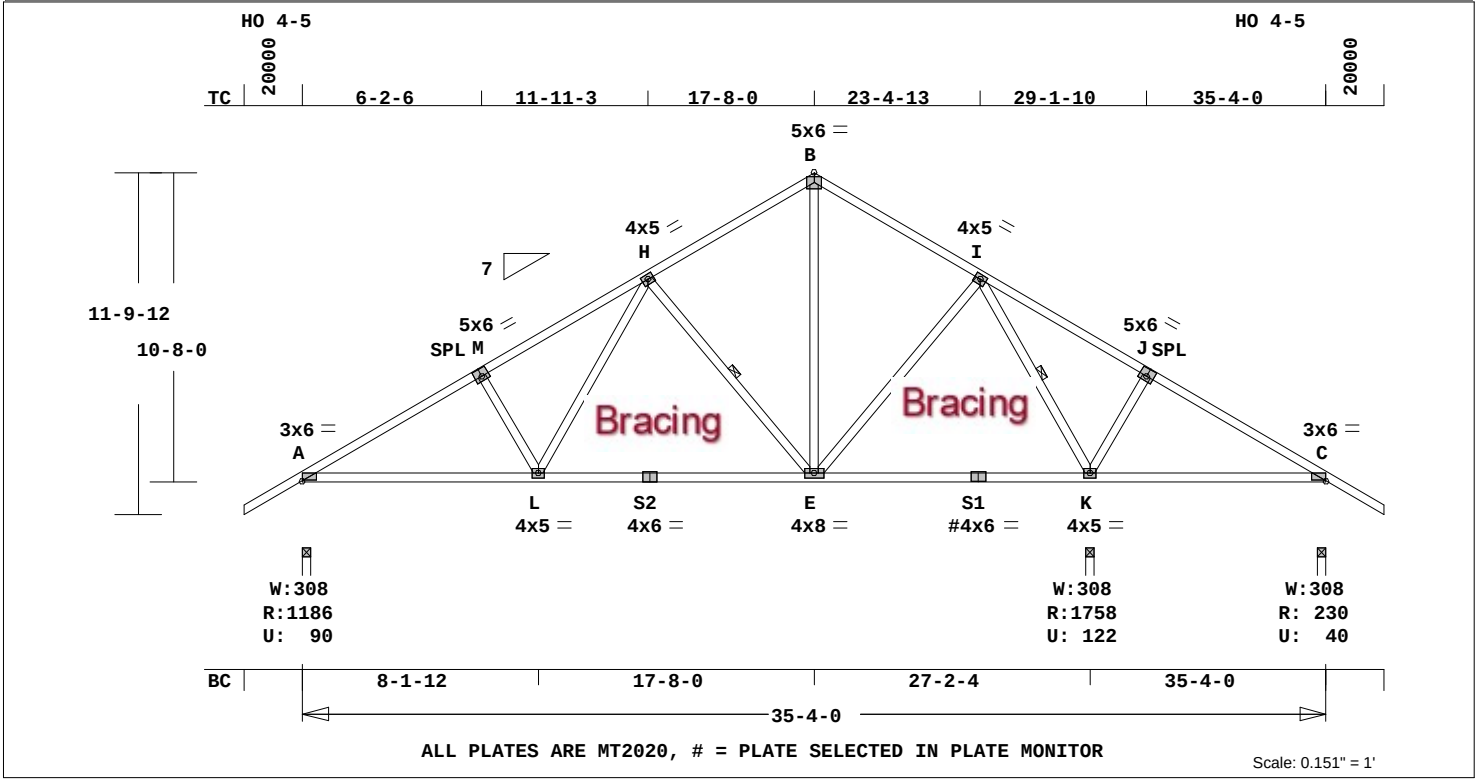
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
200342	*R1*	1	TR	350400	7	0	0	*T20412060*

Adams Residence

creep factor of 2.0 which
is used to calculate
Vert(CT) deflection per
ANSI/TPI 1.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R2	2	TR	350400	7	2- 0- 0	2- 0- 0	T20412061

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 248.9 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.41  2x 4  SP-#2
BC  0.98  2x 4  SP-#2
WB  0.48  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 35- 4- 0
 or   48.0"   0- 0- 0 35- 4- 0
 BC  Cont.    0- 0- 0 35- 4- 0
 or   72.0"   0- 0- 0 35- 4- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  H -E   I -K
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
        Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1186    91 U   184 R
K   1759   122 U
C    231    40 U   184 R

Jt   Brg Size   Required
A       3.5"       1.5"
K       3.5"       1.9"
C       3.5"       1.5"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -M  0.28  1661 C  0.02  0.26
M -H  0.30  1508 C  0.01  0.29
H -B  0.29   764 C  0.00  0.29
B -I  0.27   763 C  0.00  0.27
I -J  0.41   313 T  0.05  0.36
J -C  0.40   162 T  0.04  0.36
--------Bottom Chords---------
A -L  0.74  1435 T  0.29  0.45
L -S2 0.72  1027 T  0.21  0.51
S2-E  0.98  1027 T  0.21  0.77
E -S1 0.82   403 T  0.07  0.75
S1-K  0.63   403 T  0.07  0.56
K -C  0.53   127 C  0.00  0.53
------------Webs-------------
M -L  0.09   271 C
L -H  0.22   582 T
H -E  0.25   588 C        1 Br
E -B  0.17   469 T
E -I  0.16   450 T
I -K  0.48  1361 C        1 Br
K -J  0.10   302 C

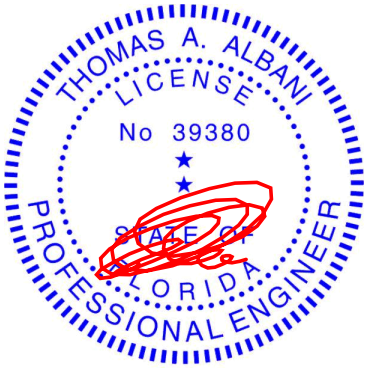
CT Defl  -0.15" in K -C  L/634
LL Defl  -0.29" in L -E  L/999
Shear // Grain  in S2-E   0.23

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 20 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 6.0 0.2 0.1 0.63
M  MT20  5.0x 6.0-0.3 0.5 0.55
H  MT20  4.0x 5.0 Ctr Ctr 0.34
B  MT20  5.0x 6.0 Ctr-0.2 0.34
I  MT20  4.0x 5.0 Ctr Ctr 0.71
J  MT20  5.0x 6.0 0.3 0.5 0.68
C  MT20  3.0x 6.0-0.2 0.1 0.41
L  MT20  4.0x 5.0 Ctr Ctr 0.29
S2 MT20  4.0x 6.0 Ctr 0.2 0.76
E  MT20  4.0x 8.0 Ctr Ctr 0.24
S1#MT20  4.0x 6.0 Ctr 0.3 0.63
K  MT20  4.0x 5.0 Ctr Ctr 0.45

# = Plate Monitor used
REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     1661 Lbs
Max tens. force     1435 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

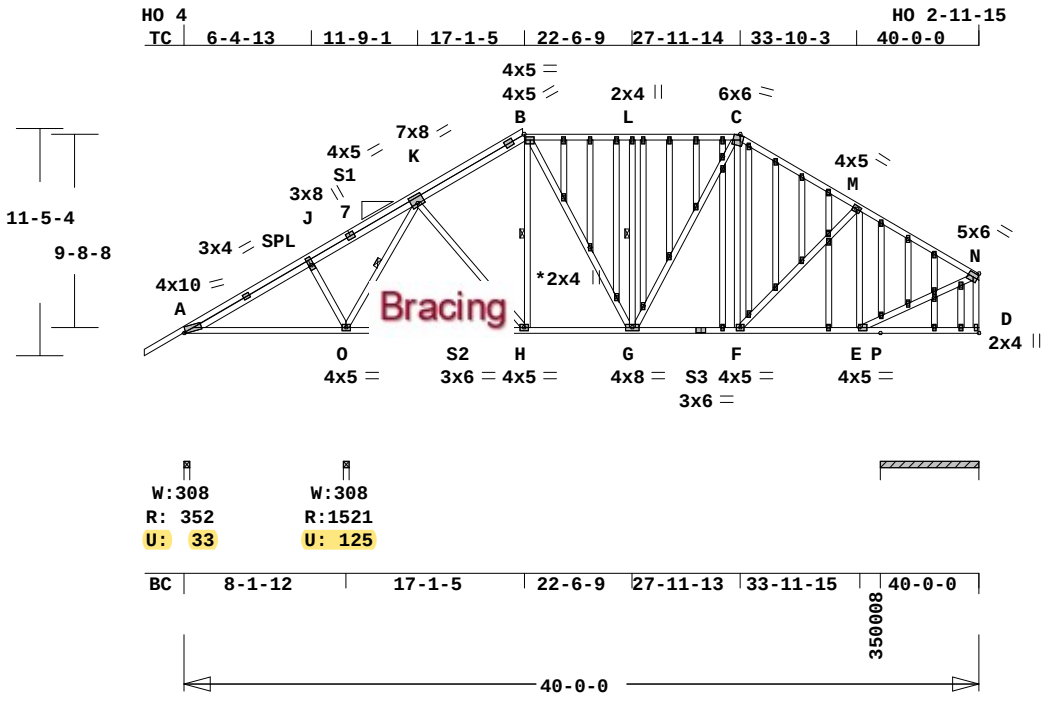


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R3	1	SP	400000	7	0	0	T20412062

Adams Residence



ALL PLATES ARE MT2020

See * For Typical Gable Plate Size and Placement

Scale: 0.103" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 534.6 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.44  2x 4  SP-#2
BC  0.54  2x 4  SP-#2
WB  0.37  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 40- 0- 0
 or   48.0"   0- 0- 0 40- 0- 0
 BC  Cont.    0- 0- 0 40- 0- 0
 or  120.0"   0- 0- 0 40- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  O -K   H -B   G -L
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A    353    33 U   164 R
O   1522   126 U
P   1202    87 U   208 R

Jt   Brg Size   Required
A       3.5"       1.5"
O       3.5"       1.6"
P      59.5"   421"-to- 480"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -J  0.44    50 C  0.00  0.44
J -S1 0.11    29 T  0.00  0.11
J -S1 0.12    29 T  0.00  0.12
S1-K  0.07    63 T  0.01  0.06
S1-K  0.08    63 T  0.01  0.07
K -B  0.09   452 C  0.00  0.09
K -B  0.10   452 C  0.00  0.10
B -B  0.44   751 C  0.00  0.44
B -L  0.28   881 C  0.00  0.28
L -C  0.28   881 C  0.00  0.28
C -M  0.38   962 C  0.00  0.38
M -N  0.40   872 C  0.00  0.40
--------Bottom Chords---------
A -O  0.48   139 T  0.00  0.48
O -S2 0.54   460 T  0.04  0.50
S2-H  0.39   460 T  0.04  0.35
H -G  0.42   724 T  0.08  0.34
G -S3 0.24   821 T  0.16  0.08
S3-F  0.31   821 T  0.16  0.15
F -E  0.33   794 T  0.15  0.18
E -P  0.26   177 T  0.00  0.26
P -D  0.26    27 T  0.00  0.26
------------Webs-------------
J -O  0.10   331 C
O -K  0.37  1155 C        1 Br
K -H  0.17   448 T
H -B  0.09   197 C        1 Br
B -G  0.12   333 T
G -L  0.15   318 C        1 Br
G -C  0.12   152 T
F -C  0.05   185 T
F -M  0.05   116 T
E -M  0.34   456 C
E -N  0.32   849 T
D -N  0.21   942 C WindLd

CT Defl  -0.13" in A -O  L/688
LL Defl  -0.06" in A -O  L/999
Shear // Grain  in A -J   0.22

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x10.0 2.6 1.4 0.47
J  MT20  3.0x 8.0-0.6 1.1 0.26
S1 MT20  4.0x 5.0 Ctr Ctr 0.58
K  MT20  7.0x 8.0-0.9 1.5 0.38
B  MT20  4.0x 5.0 Ctr Ctr 0.58
B  MT20  4.0x 5.0 Ctr Ctr 0.24
L  MT20  2.0x 4.0 Ctr Ctr 0.34
C  MT20  6.0x 6.0-1.5-3.7 0.62
M  MT20  4.0x 5.0 Ctr Ctr 0.29
N  MT20  5.0x 6.0-0.4 0.2 0.99
O  MT20  4.0x 5.0 Ctr Ctr 0.38
S2 MT20  3.0x 6.0 Ctr Ctr 0.68
H  MT20  4.0x 5.0 Ctr Ctr 0.24
G  MT20  4.0x 8.0 Ctr Ctr 0.25
S3 MT20  3.0x 6.0 Ctr Ctr 0.39
F  MT20  4.0x 5.0 Ctr Ctr 0.24
E  MT20  4.0x 5.0 Ctr Ctr 0.41
D  MT20  2.0x 4.0 Ctr Ctr 0.33

 18 Gable studs to be attached
with 2.0x4.0 plates each end.
REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Truss designed for wind loads
  in the plane of the truss
  only.  For studs exposed to
  wind (normal to the face),
  see Standard Industry Gable
  End Details as applicable,
  or consult qualified
  Building Designer as per
  ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     1155 Lbs
Max tens. force      849 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

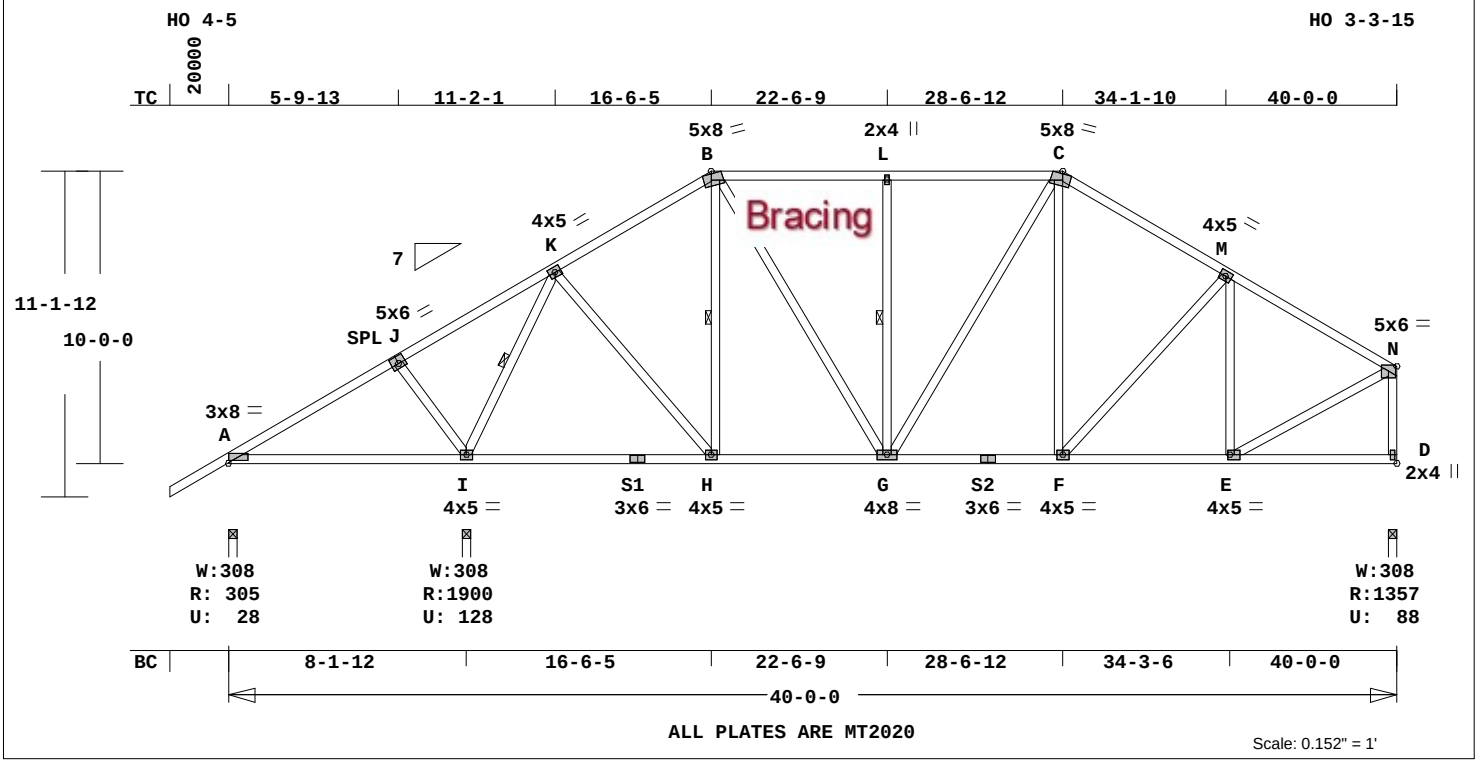
THOMAS A. ALBANI LICENSE No 39380 STATE OF FLORIDA PROFESSIONAL ENGINEER

Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R4	4	SP	400000	7	2- 0- 0	0	T20412063

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 339.2 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.36  2x 4  SP-#2
BC  0.60  2x 4  SP-#2
WB  0.47  2x 4  SP-#3

Brace truss as follows:
      O.C.      From      To
 TC  Cont.    0- 0- 0 40- 0- 0
 or   48.0"   0- 0- 0 40- 0- 0
 BC  Cont.    0- 0- 0 40- 0- 0
 or  120.0"   0- 0- 0 40- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  I -K   H -B   G -L
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
        Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A    306    29 U   162 R
I   1901   129 U
D   1358    89 U   211 R

Jt   Brg Size   Required
A       3.5"       1.5"
I       3.5"       2.0"
D       3.5"       1.6"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -J  0.33    27 T  0.01  0.32
J -K  0.34   183 T  0.02  0.32
K -B  0.25  1030 C  0.01  0.24
B -L  0.36  1138 C  0.01  0.35
L -C  0.36  1138 C  0.01  0.35
C -M  0.33  1241 C  0.01  0.32
M -N  0.35  1189 C  0.01  0.34
--------Bottom Chords---------
A -I  0.46   136 T  0.00  0.46
I -S1 0.60   483 T  0.09  0.51
S1-H  0.54   483 T  0.09  0.45
H -G  0.59   888 T  0.18  0.41
G -S2 0.42  1065 T  0.21  0.21
S2-F  0.46  1065 T  0.21  0.25
F -E  0.44  1040 T  0.21  0.23
E -D  0.22   177 T  0.00  0.22
------------Webs-------------
J -I  0.10   288 C
I -K  0.47  1529 C        1 Br
K -H  0.24   638 T
H -B  0.09   184 C        1 Br
B -G  0.18   481 T
G -L  0.17   353 C        1 Br
G -C  0.14   161 T
F -C  0.07   226 T
F -M  0.10    75 T
E -M  0.31   387 C
E -N  0.45  1190 T
D -N  0.32  1313 C WindLd

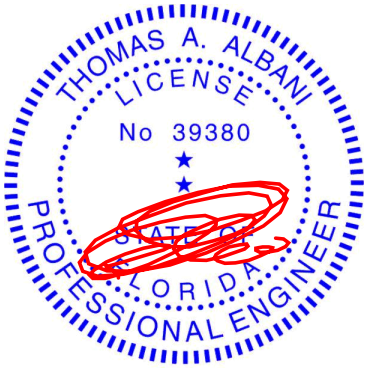
CT Defl  -0.21" in I -H  L/999
LL Defl  -0.07" in A -I  L/999
Shear // Grain  in L -C   0.23

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 8.0 1.2 0.7 0.47
J  MT20  5.0x 6.0-0.3 0.5 0.65
K  MT20  4.0x 5.0 Ctr Ctr 0.78
B  MT20  5.0x 8.0 0.9-3.5 0.50
L  MT20  2.0x 4.0 Ctr Ctr 0.34
C  MT20  5.0x 8.0-0.9-3.5 0.50
M  MT20  4.0x 5.0 Ctr Ctr 0.29
N  MT20  5.0x 6.0 Ctr-0.2 0.77
I  MT20  4.0x 5.0 Ctr Ctr 0.49
S1 MT20  3.0x 6.0 Ctr Ctr 0.49
H  MT20  4.0x 5.0 Ctr Ctr 0.30
G  MT20  4.0x 8.0 Ctr Ctr 0.25
S2 MT20  3.0x 6.0 Ctr Ctr 0.56
F  MT20  4.0x 5.0 Ctr Ctr 0.24
E  MT20  4.0x 5.0 Ctr Ctr 0.54
D  MT20  2.0x 4.0 Ctr Ctr 0.34

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     1529 Lbs
Max tens. force     1190 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

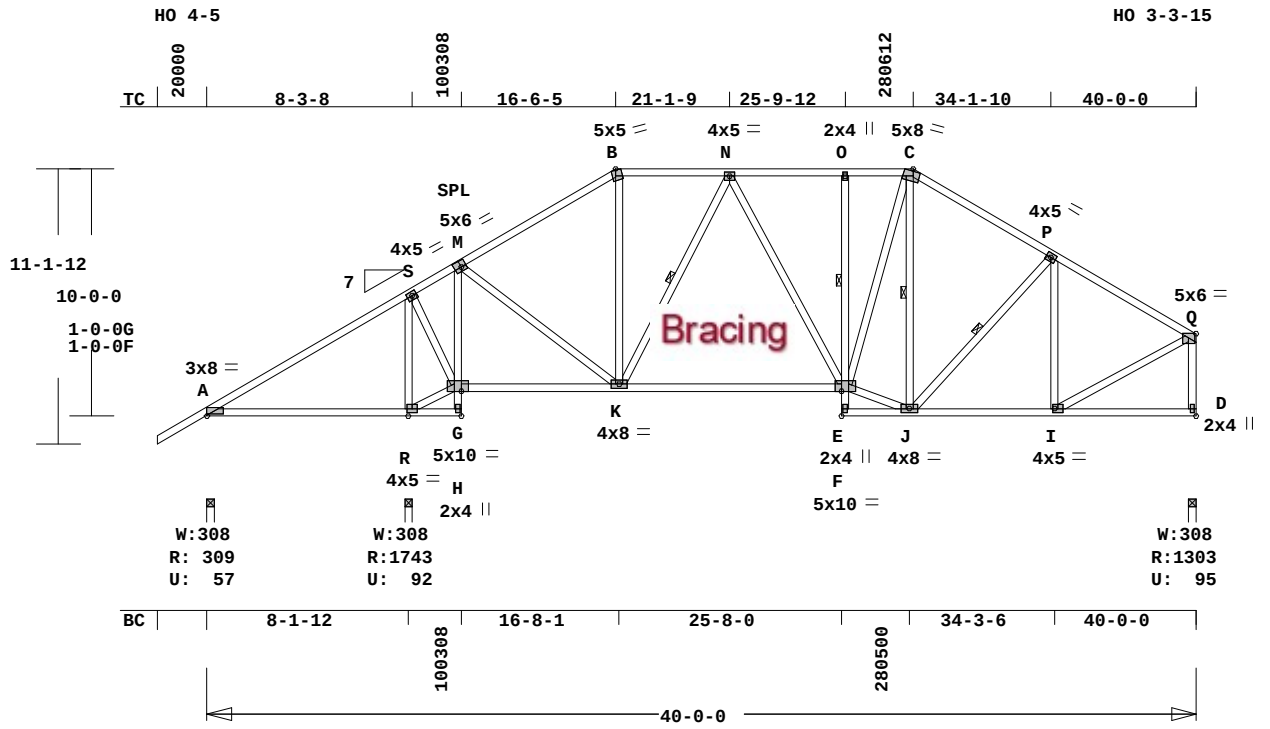


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R5	5	SP	400000	7	2- 0- 0	0	T20412064

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 362.5 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.66	2x 4	SP-#2
BC	0.95	2x 4	SP-#2
CW	0.72	2x 4	SP-#2
--	0.32	2x 4	SP-#3
H -M			
WB	0.68	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40- 0- 0
or 48.0"	0- 0- 0	40- 0- 0
BC Cont.	0- 0- 0	40- 0- 0
or 72.0"	0- 0- 0	40- 0- 0

Continuous Lateral Restraint req'd at mid-point of webs:
K -N J -C J -P F -O
Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	310	57 U	162 R
R	1744	93 U	
D	1303	96 U	211 R

Jt	Brg Size	Required
A	3.5"	1.5"
R	3.5"	1.9"
D	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -S	0.66	137 T	0.04	0.62
S -M	0.55	437 C	0.00	0.55
M -B	0.49	1080 C	0.04	0.45
B -N	0.18	931 C	0.00	0.18
N -O	0.19	1216 C	0.01	0.18
O -C	0.11	1209 C	0.01	0.10
C -P	0.36	1161 C	0.01	0.35
P -Q	0.37	1137 C	0.01	0.36

--------Bottom Chords---------

A -R	0.45	93 C	0.00	0.45
R -H	0.39	49 C	0.00	0.39
G -K	0.56	396 T	0.07	0.49
K -F	0.95	1138 T	0.23	0.72
E -J	0.14	253 C	0.00	0.14
J -I	0.38	997 T	0.20	0.18
I -D	0.22	177 T	0.00	0.22

----------Chord-Webs----------

H -G	0.19	78 C	0.00	0.19
G -M	0.32	935 C	0.26	0.06
E -F	0.72	19 T	0.00	0.72
F -O	0.19	188 C	0.00	0.19

------------Webs------------

R -S	0.68	1447 C	
R -G	0.03	133 C	
S -G	0.35	930 T	
M -K	0.28	736 T	
K -B	0.09	262 T	
K -N	0.22	454 C	1 Br
N -F	0.06	195 T	
F -C	0.28	746 T	
F -J	0.51	1339 T	
J -C	0.17	348 C	1 Br
J -P	0.03	78 C	1 Br
I -P	0.28	351 C	
I -Q	0.43	1141 T	
D -Q	0.31	1261 C	WindLd

CT Defl -0.21" in A -R L/435
LL Defl -0.09" in A -R L/990
Shear // Grain in E -F 0.33

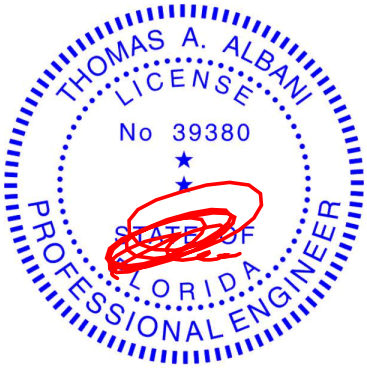
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 8.0	1.2	0.7	0.47
S	MT20	4.0x 5.0	Ctr	Ctr	0.60
M	MT20	5.0x 6.0	-0.7	0.2	0.62
B	MT20	5.0x 5.0	0.8	-3.1	0.63
N	MT20	4.0x 5.0	Ctr	Ctr	0.24
O	MT20	2.0x 4.0	Ctr	Ctr	0.34
C	MT20	5.0x 8.0	-0.9	-3.5	0.68
P	MT20	4.0x 5.0	Ctr	Ctr	0.29
Q	MT20	5.0x 6.0	Ctr	-0.2	0.74
R	MT20	4.0x 5.0	Ctr	Ctr	0.38
H	MT20	2.0x 4.0	Ctr	Ctr	0.58
G	MT20	5.0x10.0	Ctr	0.8	0.61
K	MT20	4.0x 8.0	Ctr	Ctr	0.37
F	MT20	5.0x10.0	Ctr	0.8	0.82
E	MT20	2.0x 4.0	Ctr	Ctr	0.58
J	MT20	4.0x 8.0	Ctr	Ctr	0.71
I	MT20	4.0x 5.0	Ctr	Ctr	0.52
D	MT20	2.0x 4.0	Ctr	Ctr	0.32

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1447 Lbs
Max tens. force 1339 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

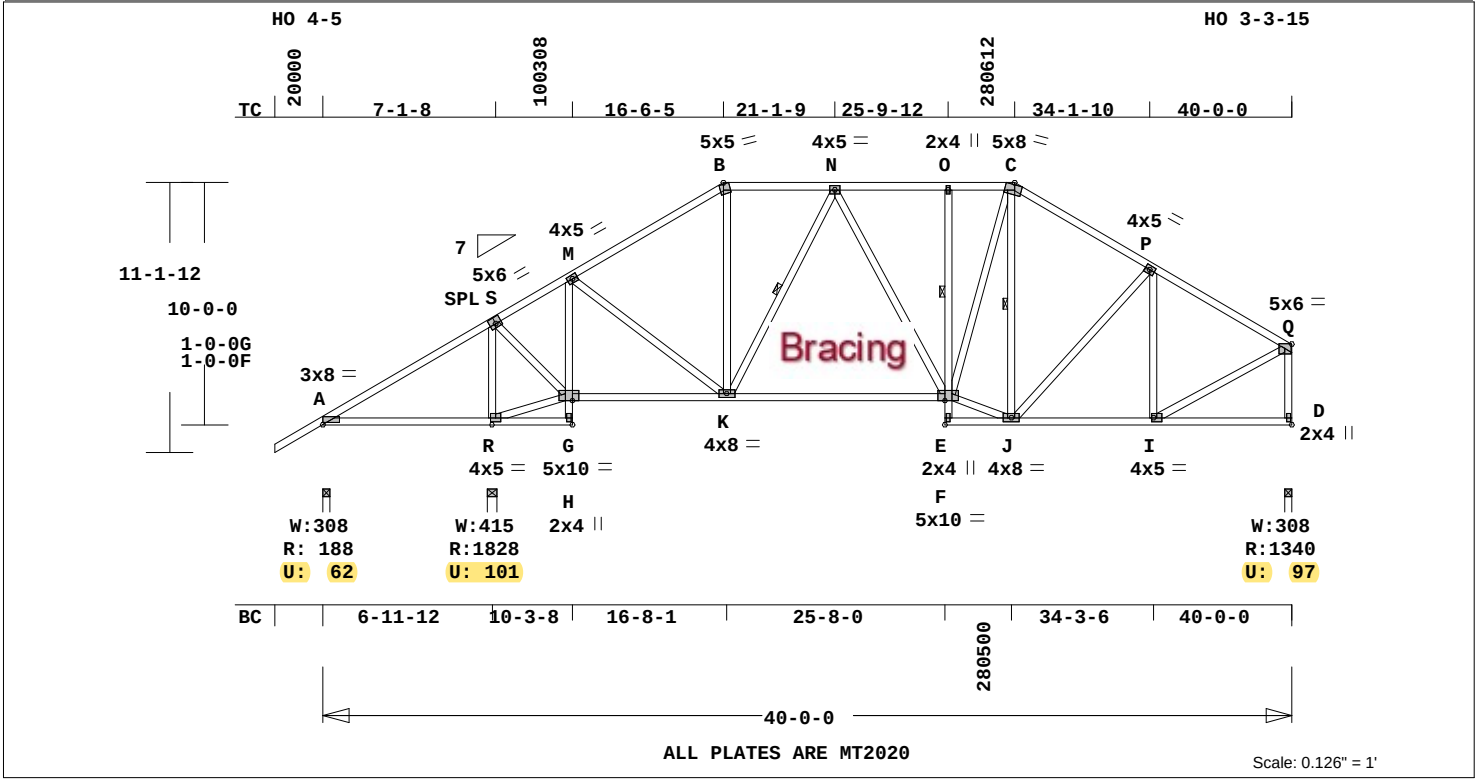


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R6	1	SP	400000	7	2- 0- 0	0	T20412065

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.126" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 363.5 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.51	2x 4	SP-#2
BC	0.96	2x 4	SP-#2
CW	0.71	2x 4	SP-#2
--	0.24	2x 4	SP-#3
H -M			
WB	0.57	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40- 0- 0
or	48.0"	0- 0- 0	40- 0- 0
BC	Cont.	0- 0- 0	40- 0- 0
or	72.0"	0- 0- 0	40- 0- 0

Continuous Lateral Restraint req'd at mid-point of webs: K -N J -C F -O

Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	189	63 U	162 R
R	1828	101 U	
D	1340	97 U	212 R

Jt	Brg Size	Required
A	3.5"	1.5"
R	4.9"	2.0"
D	3.5"	1.6"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -S	0.51	289 T	0.07	0.44
S -M	0.38	663 C	0.00	0.38
M -B	0.49	1192 C	0.04	0.45
B -N	0.18	1028 C	0.00	0.18
N -O	0.19	1275 C	0.01	0.18
O -C	0.11	1268 C	0.01	0.10
C -P	0.36	1211 C	0.01	0.35
P -Q	0.37	1174 C	0.01	0.36

--------Bottom Chords---------

A -R	0.35	241 C	0.00	0.35
R -H	0.27	54 C	0.00	0.27
G -K	0.60	588 T	0.11	0.49
K -F	0.96	1217 T	0.24	0.72
E -J	0.14	252 C	0.00	0.14
J -I	0.39	1029 T	0.21	0.18
I -D	0.22	177 T	0.00	0.22

----------Chord-Webs----------

H -G	0.21	14 T	0.00	0.21
G -M	0.24	861 C	0.22	0.02
E -F	0.71	19 T	0.00	0.71
F -O	0.20	188 C	0.00	0.20

------------Webs------------

R -S	0.57	1566 C	
R -G	0.07	273 C	
S -G	0.43	1129 T	
M -K	0.22	582 T	
K -B	0.11	302 T	
K -N	0.20	413 C	1 Br
N -F	0.04	159 T	
F -C	0.30	801 T	
F -J	0.52	1384 T	
J -C	0.18	376 C	1 Br
J -P	0.10	73 C	
I -P	0.29	369 C	
I -Q	0.44	1177 T	
D -Q	0.32	1297 C	WindLd

CT Defl -0.12" in A -R L/637
LL Defl -0.05" in A -R L/999
Shear // Grain in E -F 0.32

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

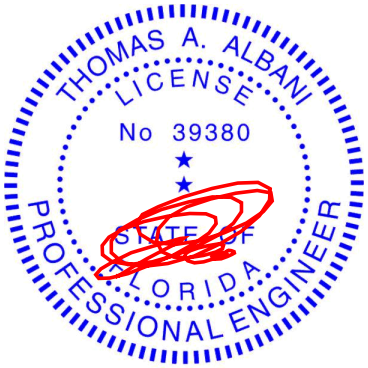
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 8.0	1.2	0.7	0.47
S	MT20	5.0x 6.0-0.3	0.5		0.69
M	MT20	4.0x 5.0 Ctr Ctr			0.54
B	MT20	5.0x 5.0 0.8-3.1			0.66
N	MT20	4.0x 5.0 Ctr Ctr			0.24
O	MT20	2.0x 4.0 Ctr Ctr			0.34
C	MT20	5.0x 8.0-0.9-3.5			0.73
P	MT20	4.0x 5.0 Ctr Ctr			0.29
Q	MT20	5.0x 6.0 Ctr-0.2			0.76
R	MT20	4.0x 5.0 Ctr Ctr			0.41
H	MT20	2.0x 4.0 Ctr Ctr			0.58
G	MT20	5.0x10.0 Ctr 0.8			0.55
K	MT20	4.0x 8.0 Ctr Ctr			0.29
F	MT20	5.0x10.0 Ctr 0.8			0.82
E	MT20	2.0x 4.0 Ctr Ctr			0.58
J	MT20	4.0x 8.0 Ctr Ctr			0.73
I	MT20	4.0x 5.0 Ctr Ctr			0.53
D	MT20	2.0x 4.0 Ctr Ctr			0.33

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1566 Lbs
Max tens. force 1384 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per

ANSI/TPI 1.

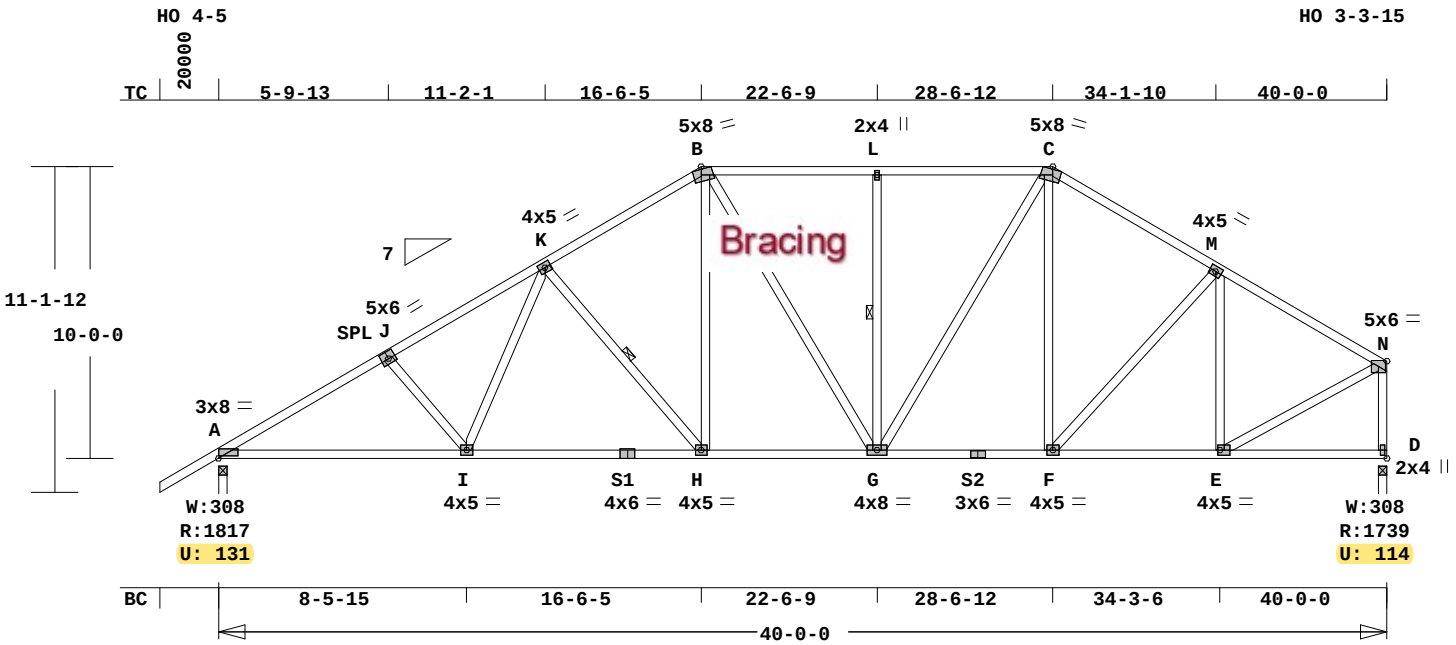


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R7	4	SP	400000	7	2- 0- 0	0	T20412066

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 339.4 LBS

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Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.40  2x 4  SP-#2
BC  0.93  2x 4  SP-#2
WB  0.59  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 40- 0- 0
 or   36.0"   0- 0- 0 40- 0- 0
 BC  Cont.    0- 0- 0 40- 0- 0
 or  120.0"   0- 0- 0 40- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  K -H   G -L
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1818   131 U   162 R
D   1740   115 U   211 R

Jt   Brg Size   Required
A       3.5"       2.1"
D       3.5"       2.1"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -J  0.40  2865 C  0.07  0.33
J -K  0.36  2679 C  0.11  0.25
K -B  0.33  2081 C  0.03  0.30
B -L  0.39  1821 C  0.02  0.37
L -C  0.39  1821 C  0.02  0.37
C -M  0.34  1759 C  0.02  0.32
M -N  0.40  1577 C  0.06  0.34
--------Bottom Chords---------
A -I  0.82  2468 T  0.50  0.32
I -S1 0.88  2144 T  0.44  0.44
S1-H  0.93  2144 T  0.44  0.49
H -G  0.76  1794 T  0.36  0.40
G -S2 0.52  1514 T  0.31  0.21
S2-F  0.56  1514 T  0.31  0.25
F -E  0.51  1375 T  0.28  0.23
E -D  0.22   177 T  0.00  0.22
------------Webs-------------
J -I  0.09   248 C
I -K  0.17   459 T
K -H  0.21   533 C        1 Br
H -B  0.27   713 T
B -G  0.17   105 T
G -L  0.18   370 C        1 Br
G -C  0.22   591 T
F -C  0.04   149 T
F -M  0.07   225 T
E -M  0.45   574 C
E -N  0.59  1573 T
D -N  0.42  1695 C WindLd

CT Defl  -0.43" in I -H  L/999
LL Defl  -0.26" in I -H  L/999
Shear // Grain  in B -L   0.24

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 8.0 1.2 0.7 0.83
J  MT20  5.0x 6.0-0.3 0.5 0.59
K  MT20  4.0x 5.0 Ctr Ctr 0.27
B  MT20  5.0x 8.0 0.9-3.5 0.76
L  MT20  2.0x 4.0 Ctr Ctr 0.34
C  MT20  5.0x 8.0-0.9-3.5 0.54
M  MT20  4.0x 5.0 Ctr Ctr 0.29
N  MT20  5.0x 6.0 Ctr-0.2 0.99
I  MT20  4.0x 5.0 Ctr Ctr 0.25
S1 MT20  4.0x 6.0 Ctr 0.2 0.62
H  MT20  4.0x 5.0 Ctr Ctr 0.38
G  MT20  4.0x 8.0 Ctr Ctr 0.32
S2 MT20  3.0x 6.0 Ctr Ctr 0.73
F  MT20  4.0x 5.0 Ctr Ctr 0.24
E  MT20  4.0x 5.0 Ctr Ctr 0.71
D  MT20  2.0x 4.0 Ctr Ctr 0.44

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     2865 Lbs
Max tens. force     2468 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
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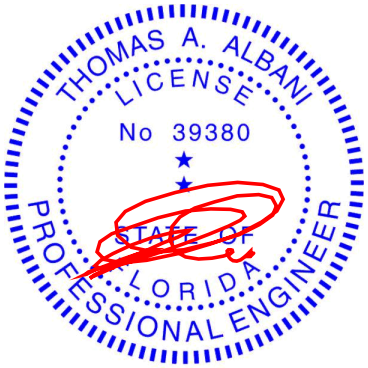
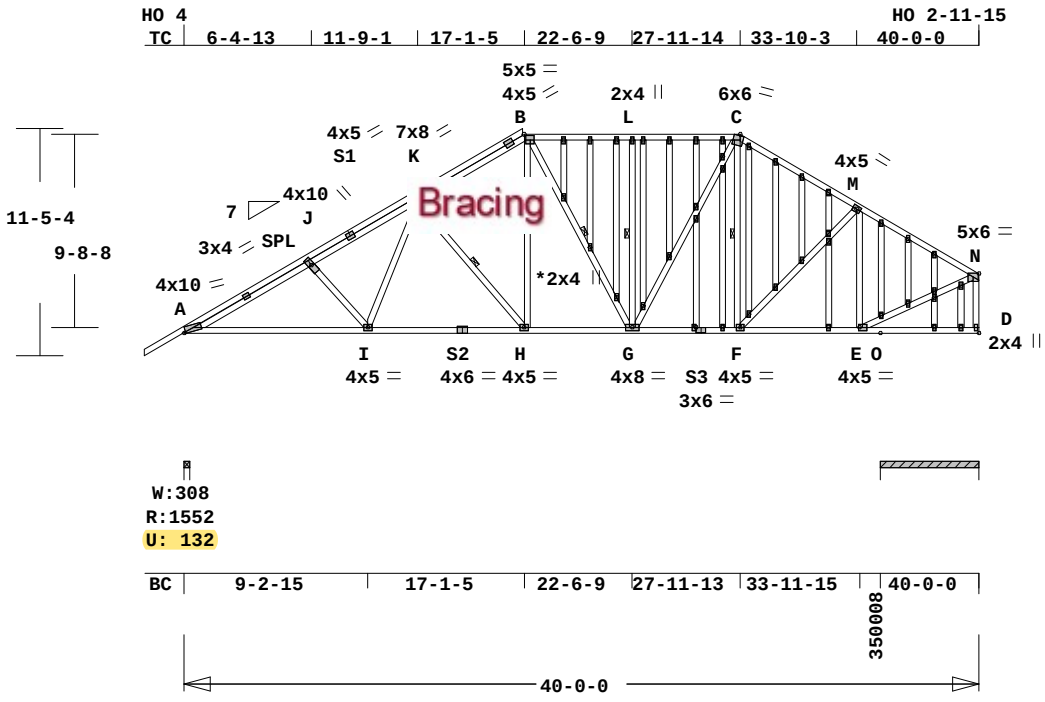


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R8	1	SP	400000	7	0	0	T20412067

Adams Residence



ALL PLATES ARE MT2020

See * For Typical Gable Plate Size and Placement

Scale: 0.103" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 545.8 LBS

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Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.95  2x 4  SP-#2
BC  0.70  2x 4  SP-#2
WB  0.51  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 40- 0- 0
 BC  Cont.    0- 0- 0 40- 0- 0
 or  120.0"   0- 0- 0 40- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  K -H   B -G   G -L   F -C
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1552   132 U   164 R
O   1524   114 U   208 R

Jt   Brg Size   Required
A       3.5"       1.8"
O      59.5"   421"-to- 480"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -J  0.33  2314 C  0.09  0.24
J -S1 0.10  1187 C  0.01  0.09
J -S1 0.11  1187 C  0.01  0.10
S1-K  0.10  1146 C  0.01  0.09
S1-K  0.11  1146 C  0.01  0.10
K -B  0.19   868 C  0.00  0.19
K -B  0.21   868 C  0.00  0.21
B -B  0.95  1472 C  0.02  0.93
B -L  0.31  1393 C  0.01  0.30
L -C  0.31  1393 C  0.01  0.30
C -M  0.39  1348 C  0.01  0.38
M -N  0.39  1098 C  0.00  0.39
--------Bottom Chords---------
A -I  0.70  2002 T  0.24  0.46
I -S2 0.63  1663 T  0.20  0.43
S2-H  0.49  1663 T  0.34  0.15
H -G  0.43  1403 T  0.28  0.15
G -S3 0.33  1157 T  0.23  0.10
S3-F  0.41  1157 T  0.23  0.18
F -E  0.46   963 T  0.19  0.27
E -O  0.40   177 T  0.00  0.40
O -D  0.40    27 T  0.00  0.40
------------Webs-------------
J -I  0.12   310 C
I -K  0.16   477 T
K -H  0.16   432 C        1 Br
H -B  0.17   480 T
B -G  0.06   100 C        1 Br
G -L  0.16   330 C        1 Br
G -C  0.18   492 T
F -C  0.02    91 T        1 Br
F -M  0.10   299 T
E -M  0.51   675 C
E -N  0.40  1065 T
D -N  0.26  1148 C WindLd

CT Defl  -0.30" in A -I  L/999
LL Defl  -0.14" in A -I  L/999
Shear // Grain  in E -O   0.36

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  4.0x10.0 2.6 1.4 0.47
J  MT20  4.0x10.0 Ctr Ctr 0.69
S1 MT20  4.0x 5.0 Ctr Ctr 0.58
K  MT20  7.0x 8.0-0.9 1.5 0.28
B  MT20  4.0x 5.0 Ctr Ctr 0.58
B  MT20  5.0x 5.0 Ctr 0.3 0.24
L  MT20  2.0x 4.0 Ctr Ctr 0.34
C  MT20  6.0x 6.0-1.5-3.7 0.71
M  MT20  4.0x 5.0 Ctr Ctr 0.29
N  MT20  5.0x 6.0 Ctr-0.2 0.67
I  MT20  4.0x 5.0 Ctr Ctr 0.25
S2 MT20  4.0x 6.0 Ctr 0.2 0.55
H  MT20  4.0x 5.0 Ctr Ctr 0.24
G  MT20  4.0x 8.0 Ctr Ctr 0.25
S3 MT20  3.0x 6.0 Ctr Ctr 0.44
F  MT20  4.0x 5.0 Ctr Ctr 0.24
E  MT20  4.0x 5.0 Ctr Ctr 0.51
D  MT20  2.0x 4.0 Ctr Ctr 0.32

 19 Gable studs to be attached
with 2.0x4.0 plates each end.
REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
Design checked for 10 psf non-
  concurrent LL on BC.
Truss designed for wind loads
  in the plane of the truss
  only.  For studs exposed to
  wind (normal to the face),
  see Standard Industry Gable
  End Details as applicable,
  or consult qualified
  Building Designer as per
  ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category    :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     2314 Lbs
Max tens. force     2002 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

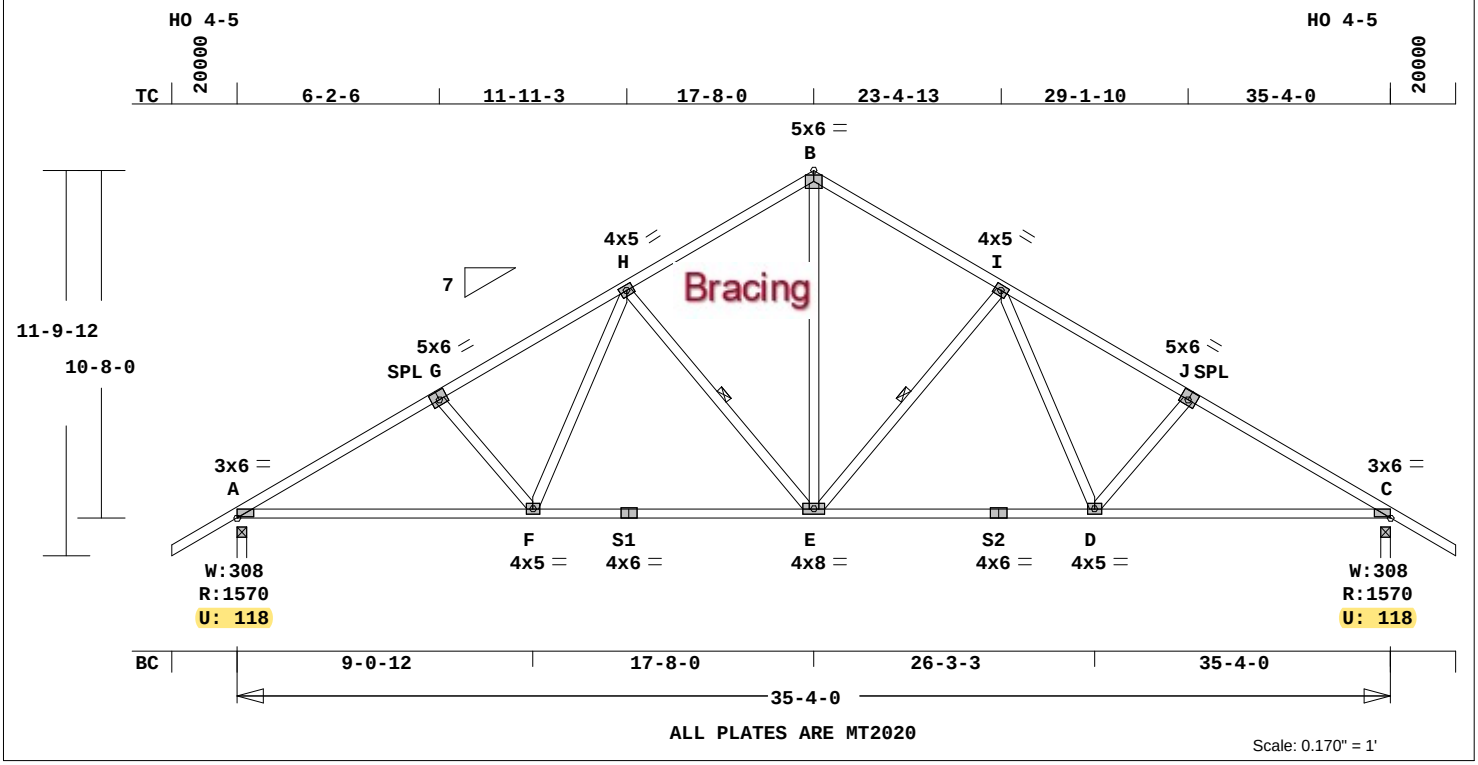
THOMAS A. ALBANI LICENSE No 39380 STATE OF FLORIDA PROFESSIONAL ENGINEER

Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R9	1	TR	350400	7	2- 0- 0	2- 0- 0	T20412068

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 249.5 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.94	2x 4	SP-#2
WB	0.46	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	35- 4- 0
or	42.0"	0- 0- 0	35- 4- 0
BC	Cont.	0- 0- 0	35- 4- 0
or	120.0"	0- 0- 0	35- 4- 0

Continuous Lateral Restraint req'd at mid-point of webs: H -E E -I
Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1571	119 U	183 R
C	1571	119 U	184 R

Jt	Brg Size	Required
A	3.5"	1.9"
C	3.5"	1.9"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.31	2372 C	0.05 0.26
G -H	0.33	2171 C	0.04 0.29
H -B	0.31	1522 C	0.02 0.29
B -I	0.31	1522 C	0.02 0.29
I -J	0.33	2171 C	0.04 0.29
J -C	0.31	2372 C	0.05 0.26
--------Bottom Chords---------			
A -F	0.80	2048 T	0.42 0.38
F -S1	0.74	1687 T	0.34 0.40
S1-E	0.94	1687 T	0.34 0.60
E -S2	0.94	1687 T	0.34 0.60
S2-D	0.74	1687 T	0.34 0.40
D -C	0.80	2048 T	0.42 0.38
------------Webs------------			
G -F	0.12	273 C	
F -H	0.19	504 T	
H -E	0.25	593 C	1 Br
E -B	0.46	1231 T	
E -I	0.25	593 C	1 Br
I -D	0.19	504 T	
D -J	0.12	273 C	

CT Defl -0.44" in F -E L/951
LL Defl -0.29" in F -E L/999
Shear // Grain in F -S1 0.21

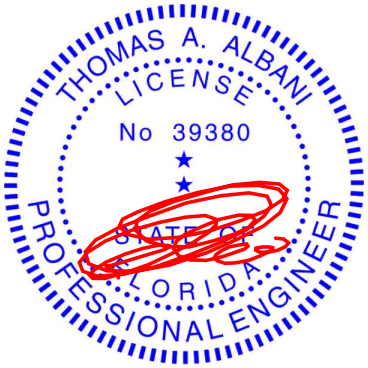
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0	0.2	0.1	0.90
G	MT20	5.0x 6.0-0.3	0.5		0.62
H	MT20	4.0x 5.0 Ctr	Ctr		0.28
B	MT20	5.0x 6.0 Ctr-0.2			0.47
I	MT20	4.0x 5.0 Ctr	Ctr		0.28
J	MT20	5.0x 6.0	0.3	0.5	0.62
C	MT20	3.0x 6.0-0.2	0.1		0.90
F	MT20	4.0x 5.0 Ctr	Ctr		0.27
S1	MT20	4.0x 6.0 Ctr	0.2		0.66
E	MT20	4.0x 8.0 Ctr	Ctr		0.47
S2	MT20	4.0x 6.0 Ctr	0.2		0.66
D	MT20	4.0x 5.0 Ctr	Ctr		0.27

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

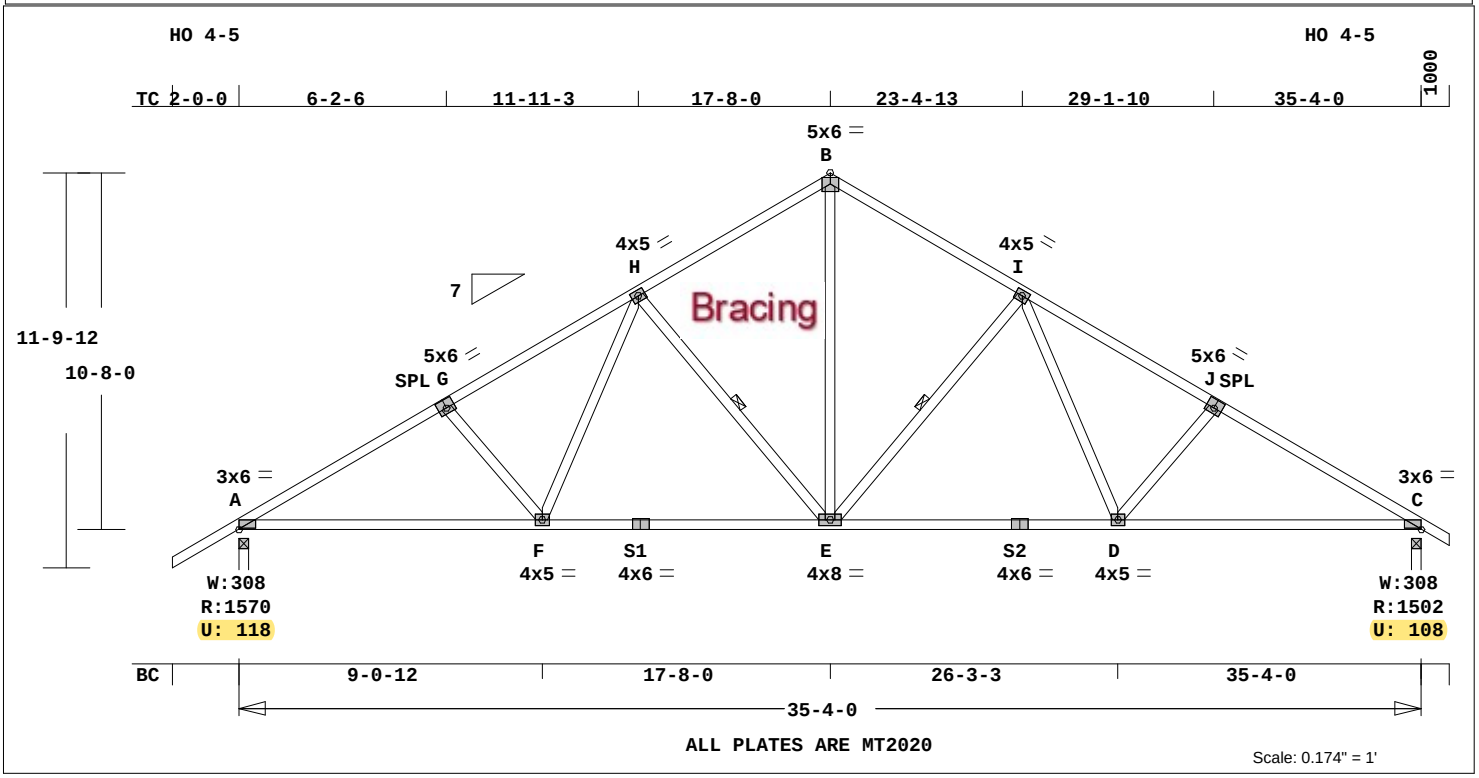
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 2372 Lbs
Max tens. force 2048 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R10	1	TR	350400	7	2- 0- 0	10- 0	T20412069

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 247.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.94	2x 4	SP-#2
WB	0.46	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	35- 4- 0
or	42.0"	0- 0- 0	35- 4- 0
BC	Cont.	0- 0- 0	35- 4- 0
or	120.0"	0- 0- 0	35- 4- 0

Continuous Lateral Restraint req'd at mid-point of webs:
H -E E -I
Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live	
TC	7.0	20.0	
BC	10.0	0.0	
TC+BC	17.0	20.0	
Total	37.0	Spacing	24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1571	119 U	183 R
C	1503	108 U	184 R

Jt	Brg Size	Required
A	3.5"	1.9"
C	3.5"	1.8"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -G	0.31	2372 C	0.05 0.26
G -H	0.33	2171 C	0.04 0.29
H -B	0.31	1523 C	0.02 0.29
B -I	0.30	1523 C	0.02 0.28
I -J	0.32	2173 C	0.04 0.28
J -C	0.31	2374 C	0.05 0.26
Bottom Chords			
A -F	0.80	2048 T	0.42 0.38
F -S1	0.74	1687 T	0.34 0.40
S1-E	0.94	1687 T	0.34 0.60
E -S2	0.94	1688 T	0.34 0.60
S2-D	0.74	1688 T	0.34 0.40
D -C	0.80	2050 T	0.42 0.38
Webs			
G -F	0.12	273 C	
F -H	0.19	504 T	
H -E	0.25	593 C	1 Br
E -B	0.46	1232 T	
E -I	0.25	594 C	1 Br
I -D	0.19	506 T	
D -J	0.12	273 C	

CT Defl -0.44" in F -E L/951
LL Defl -0.29" in F -E L/999
Shear // Grain in F -S1 0.21

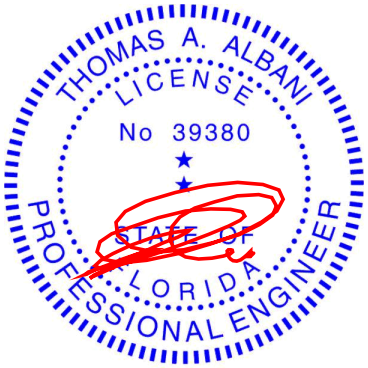
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0	0.2	0.1	0.90
G	MT20	5.0x 6.0-0.3	0.5		0.62
H	MT20	4.0x 5.0 Ctr	Ctr		0.28
B	MT20	5.0x 6.0 Ctr-0.2			0.47
I	MT20	4.0x 5.0 Ctr	Ctr		0.28
J	MT20	5.0x 6.0	0.3	0.5	0.62
C	MT20	3.0x 6.0-0.2	0.1		0.90
F	MT20	4.0x 5.0 Ctr	Ctr		0.27
S1	MT20	4.0x 6.0 Ctr	0.2		0.66
E	MT20	4.0x 8.0 Ctr	Ctr		0.47
S2	MT20	4.0x 6.0 Ctr	0.2		0.66
D	MT20	4.0x 5.0 Ctr	Ctr		0.27

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

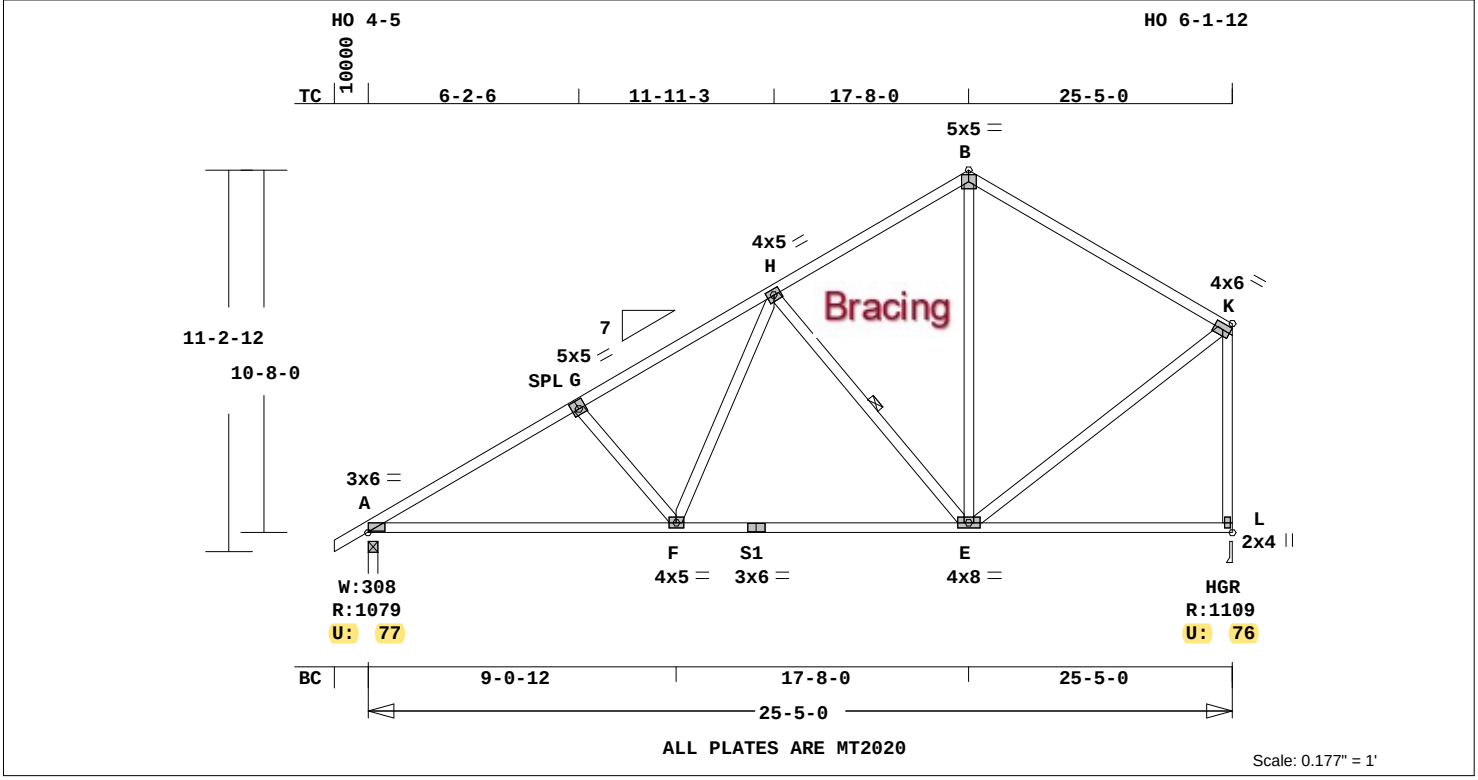
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 2374 Lbs
Max tens. force 2050 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R11	1	TR	250500	7	1- 0- 0	0	T20412070

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 192.2 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.90	2x 4	SP-#2
BC	0.73	2x 4	SP-#2
WB	0.68	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	25- 5- 0
BC	Cont.	0- 0- 0	25- 5- 0
or	120.0"	0- 0- 0	25- 5- 0

Continuous Lateral Restraint req'd at mid-point of webs: H -E
Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1079	77 U	164 R
L	1110	77 U	251 R

Jt	Brg Size	Required
A	3.5"	1.5"
L	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -G	0.31	1548 C	0.01 0.30
G -H	0.31	1343 C	0.01 0.30
H -B	0.37	669 C	0.00 0.37
B -K	0.90	663 C	0.02 0.88
Bottom Chords			
A -F	0.70	1342 T	0.27 0.43
F -S1	0.68	956 T	0.19 0.49
S1-E	0.73	956 T	0.19 0.54
E -L	0.49	186 T	0.00 0.49
Webs			
G -F	0.12	287 C	
F -H	0.20	538 T	
H -E	0.25	588 C	1 Br
E -B	0.10	333 T	
E -K	0.28	739 T	
L -K	0.68	979 C	WindLd

CT Defl -0.28" in F -E L/999
LL Defl -0.20" in F -E L/999
Shear // Grain in B -K 0.25

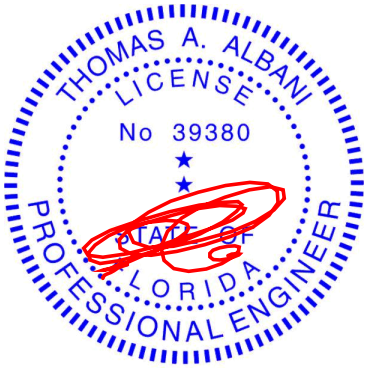
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0	0.2	0.1	0.59
G	MT20	5.0x 5.0-0.3	0.5		0.73
H	MT20	4.0x 5.0	Ctr	Ctr	0.32
B	MT20	5.0x 5.0	Ctr	Ctr	0.84
K	MT20	4.0x 6.0-0.1	0.1		0.92
F	MT20	4.0x 5.0	Ctr	Ctr	0.29
S1	MT20	3.0x 6.0	Ctr	Ctr	0.67
E	MT20	4.0x 8.0	Ctr	Ctr	0.37
L	MT20	2.0x 4.0	Ctr	Ctr	0.30

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

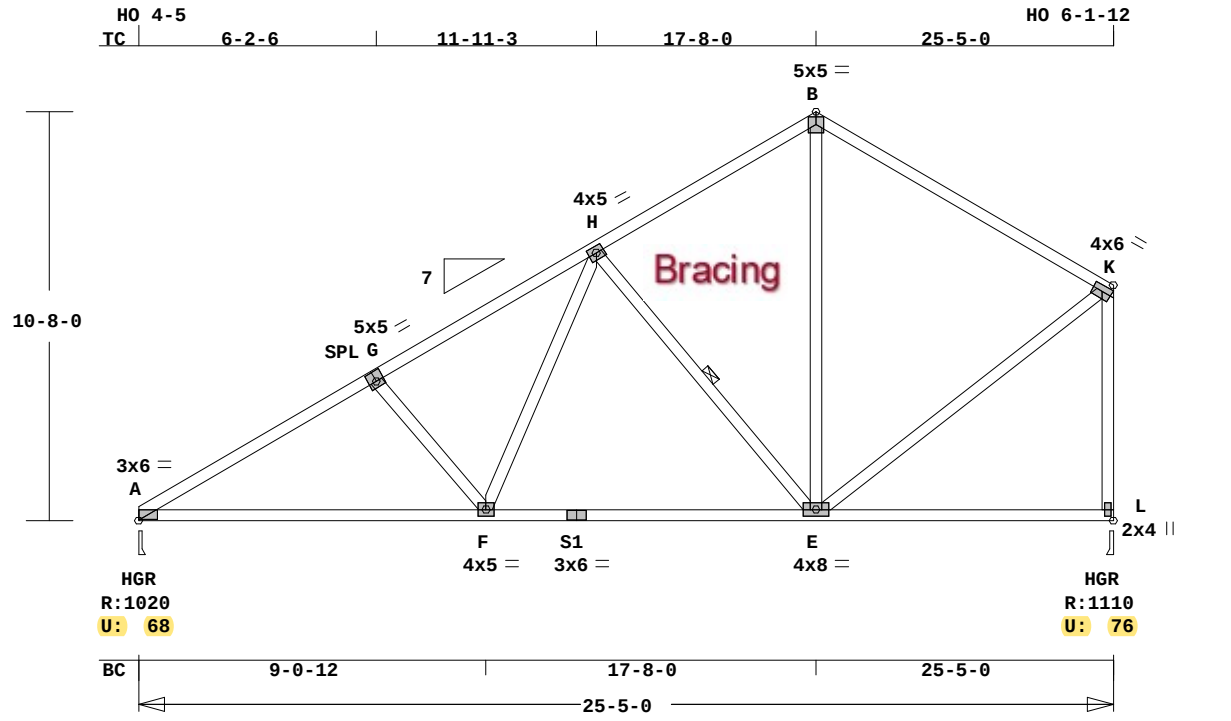
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1548 Lbs
Max tens. force 1342 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R12	10	TR	250500	7	0	0	T20412071

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.200" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 190.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.90	2x 4	SP-#2
BC	0.73	2x 4	SP-#2
WB	0.68	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	25- 5- 0
BC	Cont.	0- 0- 0	25- 5- 0
or	120.0"	0- 0- 0	25- 5- 0

Continuous Lateral Restraint req'd at mid-point of webs: H -E

Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1021	68 U	164 R
L	1110	77 U	251 R

Jt	Brg Size	Required
A	3.5"	1.5"
L	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -G	0.31	1549 C	0.01 0.30
G -H	0.31	1345 C	0.01 0.30
H -B	0.37	669 C	0.00 0.37
B -K	0.90	663 C	0.02 0.88
Bottom Chords			
A -F	0.70	1344 T	0.27 0.43
F -S1	0.68	957 T	0.19 0.49
S1-E	0.73	957 T	0.19 0.54
E -L	0.49	186 T	0.00 0.49
Webs			
G -F	0.12	289 C	
F -H	0.20	540 T	
H -E	0.25	589 C	1 Br
E -B	0.10	333 T	
E -K	0.28	739 T	
L -K	0.68	979 C	WindLd

CT Defl -0.28" in F -E L/999
LL Defl -0.20" in F -E L/999
Shear // Grain in B -K 0.25

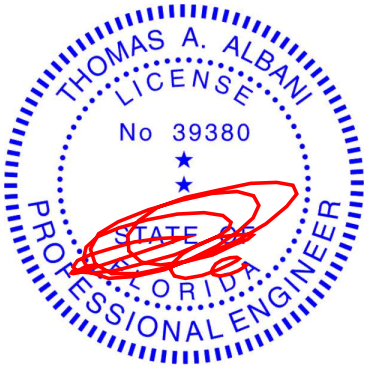
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0	0.2	0.1	0.59
G	MT20	5.0x 5.0-0.3	0.5		0.74
H	MT20	4.0x 5.0 Ctr	Ctr		0.32
B	MT20	5.0x 5.0 Ctr	Ctr		0.84
K	MT20	4.0x 6.0-0.1	0.1		0.92
F	MT20	4.0x 5.0 Ctr	Ctr		0.29
S1	MT20	3.0x 6.0 Ctr	Ctr		0.67
E	MT20	4.0x 8.0 Ctr	Ctr		0.37
L	MT20	2.0x 4.0 Ctr	Ctr		0.30

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1549 Lbs
Max tens. force 1344 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

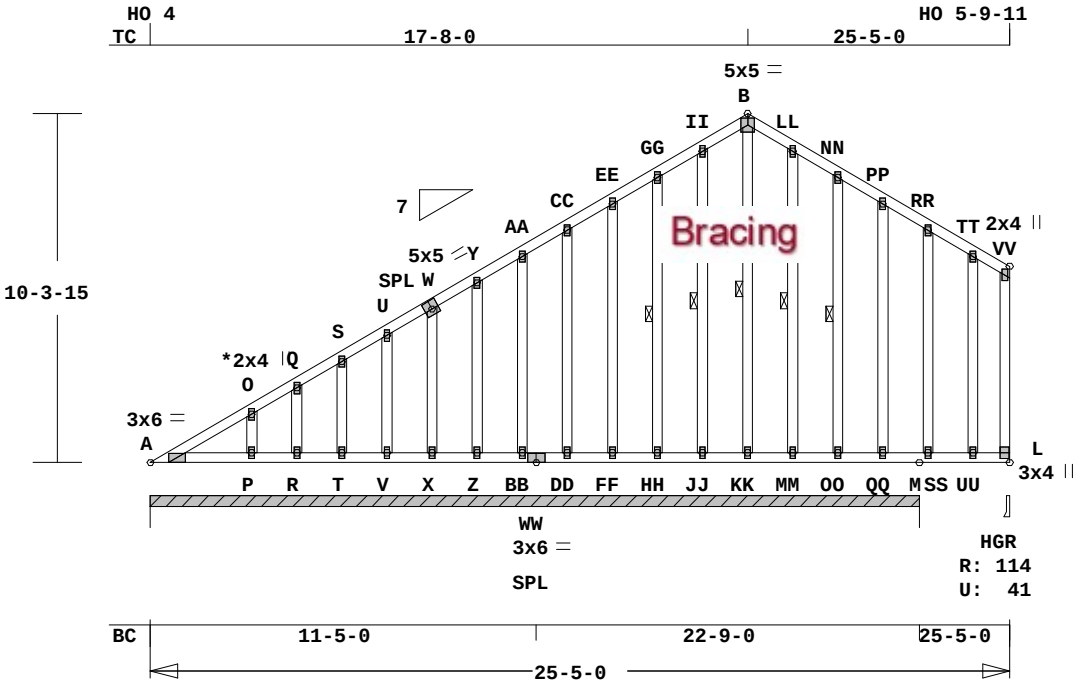


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R13	1	TR	250500	7	0	0	T20412072

Adams Residence



ALL PLATES ARE MT2020

See Joint O For Typical Gable Plate Size and Placement

Scale: 0.176" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 308.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.21	2x 4	SP-#3
GW	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	25- 5- 0
or	48.0"	0- 0- 0	25- 5- 0
BC	Cont.	0- 0- 0	25- 5- 0
or	120.0"	0- 0- 0	25- 5- 0

Continuous Lateral Restraint req'd at mid-point of webs: HH-GG JJ-II KK-B MM-LL OO-NN

Attach CLR with (2)-10d nails at each web.
Refer to BCSI for diagonal restraint requirements.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
M	1734	100 U	159 R
L	115	42 U	240 R

Jt	Brg Size	Required
M	273.0"	-6"-to- 266"
L	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -O	0.03	165 C	0.00 0.03
O -Q	0.02	164 C	0.00 0.02
Q -S	0.02	161 C	0.02 0.00
S -U	0.02	157 C	0.01 0.01
U -W	0.02	152 C	0.01 0.01
W -Y	0.02	146 C	0.01 0.01
Y -AA	0.02	140 C	0.01 0.01
AA-CC	0.02	133 C	0.01 0.01
CC-EE	0.02	126 C	0.01 0.01
EE-GG	0.02	131 T	0.02 0.00
GG-II	0.02	157 T	0.02 0.00
II-B	0.03	170 T	0.02 0.01
B -LL	0.03	170 T	0.02 0.01
LL-NN	0.02	157 T	0.02 0.00
NN-PP	0.02	132 T	0.00 0.02
PP-RR	0.02	108 T	0.00 0.02
RR-TT	0.02	86 T	0.00 0.02
TT-VV	0.07	97 C	0.01 0.06
Bottom Chords			
A -P	0.03	149 T	0.02 0.01
P -R	0.03	154 T	0.02 0.01
R -T	0.02	159 T	0.02 0.00
T -V	0.02	162 T	0.02 0.00
V -X	0.02	164 T	0.02 0.00
X -Z	0.02	166 T	0.02 0.00
Z -BB	0.02	167 T	0.02 0.00
BB-WW	0.02	168 T	0.02 0.00
WW-DD	0.02	168 T	0.02 0.00
DD-FF	0.02	169 T	0.02 0.00
FF-HH	0.02	169 T	0.02 0.00
HH-JJ	0.02	170 T	0.02 0.00
JJ-KK	0.02	170 T	0.02 0.00
KK-MM	0.02	171 T	0.02 0.00
MM-OO	0.02	172 T	0.02 0.00
OO-QQ	0.02	172 T	0.02 0.00
QQ-M	0.05	174 T	0.02 0.03
M -SS	0.05	174 T	0.02 0.03
SS-UU	0.05	177 T	0.02 0.03
UU-L	0.09	179 T	0.02 0.07
Webs			
L -VV	0.21	112 C	0.00 0.21
Gable Webs			
P -O	0.03	94 C	0.00 0.03
R -Q	0.02	67 C	0.00 0.02
T -S	0.01	72 C	0.00 0.01
V -U	0.01	71 C	0.00 0.01
X -W	0.01	71 C	0.00 0.01
Z -Y	0.01	71 C	
BB-AA	0.01	71 C	0.00 0.01
DD-CC	0.01	72 C	0.00 0.01
FF-EE	0.01	71 C	0.01 0.00
HH-GG	0.01	72 C	0.01 0.00
JJ-II	0.02	75 C	0.02 0.00
KK-B	0.10	134 C	0.10 0.00
MM-LL	0.02	72 C	0.02 0.00
OO-NN	0.02	70 C	0.01 0.01
QQ-PP	0.02	78 C	0.00 0.02
SS-RR	0.05	81 C	0.00 0.05
UU-TT	0.03	42 T	0.01 0.02

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in M -SS 0.14

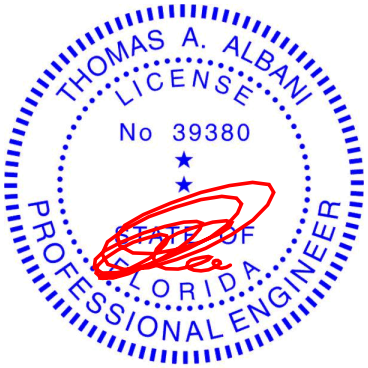
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0 Ctr	Ctr	0.41
O	MT20	2.0x 4.0 Ctr	Ctr	0.00
Q	MT20	2.0x 4.0 Ctr	Ctr	0.00
S	MT20	2.0x 4.0 Ctr	Ctr	0.00
U	MT20	2.0x 4.0 Ctr	Ctr	0.00
W	MT20	5.0x 5.0-0.3	0.5	0.38
Y	MT20	2.0x 4.0 Ctr	Ctr	0.00
AA	MT20	2.0x 4.0 Ctr	Ctr	0.00
CC	MT20	2.0x 4.0 Ctr	Ctr	0.00
EE	MT20	2.0x 4.0 Ctr	Ctr	0.00
GG	MT20	2.0x 4.0 Ctr	Ctr	0.00
II	MT20	2.0x 4.0 Ctr	Ctr	0.00
B	MT20	5.0x 5.0 Ctr	Ctr	0.34
LL	MT20	2.0x 4.0 Ctr	Ctr	0.00
NN	MT20	2.0x 4.0 Ctr	Ctr	0.00
PP	MT20	2.0x 4.0 Ctr	Ctr	0.00
RR	MT20	2.0x 4.0 Ctr	Ctr	0.00
TT	MT20	2.0x 4.0 Ctr	Ctr	0.00
VV	MT20	2.0x 4.0 Ctr	Ctr	0.63
P	MT20	2.0x 4.0 Ctr	Ctr	0.00
R	MT20	2.0x 4.0 Ctr	Ctr	0.00
T	MT20	2.0x 4.0 Ctr	Ctr	0.00
V	MT20	2.0x 4.0 Ctr	Ctr	0.00
X	MT20	2.0x 4.0 Ctr	Ctr	0.00
Z	MT20	2.0x 4.0 Ctr	Ctr	0.00
BB	MT20	2.0x 4.0 Ctr	Ctr	0.00
WW	MT20	3.0x 6.0 Ctr	Ctr	0.39
DD	MT20	2.0x 4.0 Ctr	Ctr	0.00
FF	MT20	2.0x 4.0 Ctr	Ctr	0.00
HH	MT20	2.0x 4.0 Ctr	Ctr	0.00
JJ	MT20	2.0x 4.0 Ctr	Ctr	0.00
KK	MT20	2.0x 4.0 Ctr	Ctr	0.00
MM	MT20	2.0x 4.0 Ctr	Ctr	0.00
OO	MT20	2.0x 4.0 Ctr	Ctr	0.00
QQ	MT20	2.0x 4.0 Ctr	Ctr	0.00
SS	MT20	2.0x 4.0 Ctr	Ctr	0.00
UU	MT20	2.0x 4.0 Ctr	Ctr	0.00
L	MT20	3.0x 4.0 Ctr	Ctr	0.44

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by: RIDGWAY ROOF TRUSS
Analysis Conforms To: FBC2017 TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 165 Lbs
Max tens. force 179 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

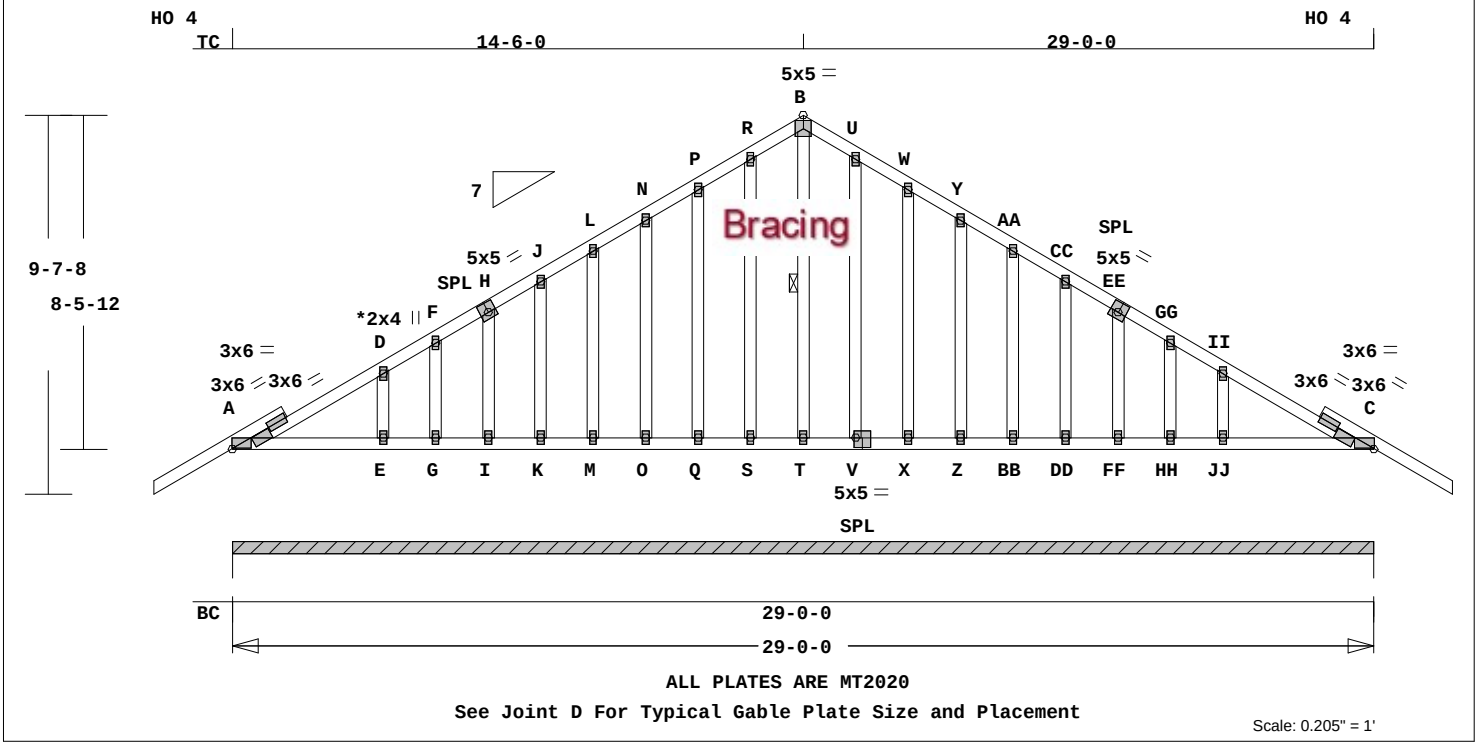


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R14	1	TR	290000	7	0	0	T20412073

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.205" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 279.8 LBS

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Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.09  2x 4  SP-#2
BC  0.09  2x 4  SP-#2
GW  0.11  2x 4  SP-#3

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 29- 0- 0
 or   48.0"   0- 0- 0 29- 0- 0
 BC  Cont.    0- 0- 0 29- 0- 0
 or   72.0"   0- 0- 0 29- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  T -B
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   2146   165 U   139 R

Jt   Brg Size   Required
A     348.0"     0"-to- 348"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -D  0.09   105 C  0.00  0.09
D -F  0.08   119 C  0.00  0.08
F -H  0.01   102 C  0.00  0.01
H -J  0.01    97 C  0.00  0.01
J -L  0.01    92 C  0.00  0.01
L -N  0.01    86 C  0.00  0.01
N -P  0.01    80 C  0.00  0.01
P -R  0.01    89 T  0.00  0.01
R -B  0.01   107 T  0.01  0.00
B -U  0.01   107 T  0.01  0.00
U -W  0.01    89 T  0.00  0.01
W -Y  0.01    80 C  0.00  0.01
Y -AA 0.01    86 C  0.00  0.01
AA-CC 0.01    92 C  0.00  0.01
CC-EE 0.01    97 C  0.00  0.01
EE-GG 0.01   102 C  0.00  0.01
GG-II 0.08   119 C  0.00  0.08
II-C  0.09   105 C  0.00  0.09
--------Bottom Chords---------
A -E  0.09     6 T  0.00  0.09
E -G  0.06     0 T  0.00  0.06
G -I  0.01     0 T  0.00  0.01
I -K  0.01     0 T  0.00  0.01
K -M  0.00     0 T
M -O  0.00     0 T
O -Q  0.00     0 T
Q -S  0.00     0 T
S -T  0.00     0 T
T -V  0.00     0 T
V -X  0.00     0 T
X -Z  0.00     0 T
Z -BB 0.00     0 T
BB-DD 0.00     0 T
DD-FF 0.01     0 T  0.00  0.01
FF-HH 0.01     0 T  0.00  0.01
HH-JJ 0.06     0 T  0.00  0.06
JJ-C  0.09     6 T  0.00  0.09
----------Gable Webs----------
E -D  0.03   172 C
G -F  0.00    38 C
I -H  0.02    74 C
K -J  0.03    72 C
M -L  0.04    71 C
O -N  0.06    72 C
Q -P  0.08    74 C
S -R  0.08    66 C
T -B  0.11    74 C        1 Br
V -U  0.08    66 C
X -W  0.08    74 C
Z -Y  0.06    72 C
BB-AA 0.04    71 C
DD-CC 0.03    72 C
FF-EE 0.02    74 C
HH-GG 0.00    38 C
JJ-II 0.03   172 C

CT Defl  -0.01" in A -E  L/999
LL Defl   0.00" in ----- L/999
Shear // Grain  in A -D   0.11

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 6.0 Ctr Ctr 0.41
D  MT20  2.0x 4.0 Ctr Ctr 0.00
F  MT20  2.0x 4.0 Ctr Ctr 0.00
H  MT20  2.0x 4.0 Ctr Ctr 0.00
J  MT20  2.0x 4.0 Ctr Ctr 0.00
L  MT20  2.0x 4.0 Ctr Ctr 0.00
N  MT20  2.0x 4.0 Ctr Ctr 0.00
P  MT20  2.0x 4.0 Ctr Ctr 0.00
R  MT20  2.0x 4.0 Ctr Ctr 0.00
B  MT20  5.0x 5.0 Ctr Ctr 0.34
U  MT20  2.0x 4.0 Ctr Ctr 0.00
W  MT20  2.0x 4.0 Ctr Ctr 0.00
Y  MT20  2.0x 4.0 Ctr Ctr 0.00
AA MT20  2.0x 4.0 Ctr Ctr 0.00
CC MT20  2.0x 4.0 Ctr Ctr 0.00
EE MT20  5.0x 5.0 0.3 0.5 0.38
GG MT20  2.0x 4.0 Ctr Ctr 0.00
II MT20  2.0x 4.0 Ctr Ctr 0.00
C  MT20  3.0x 6.0 Ctr Ctr 0.41
E  MT20  2.0x 4.0 Ctr Ctr 0.00
G  MT20  2.0x 4.0 Ctr Ctr 0.00
I  MT20  2.0x 4.0 Ctr Ctr 0.00
K  MT20  2.0x 4.0 Ctr Ctr 0.00
M  MT20  2.0x 4.0 Ctr Ctr 0.00
O  MT20  2.0x 4.0 Ctr Ctr 0.00
Q  MT20  2.0x 4.0 Ctr Ctr 0.00
S  MT20  2.0x 4.0 Ctr Ctr 0.00
T  MT20  2.0x 4.0 Ctr Ctr 0.00
V  MT20  5.0x 5.0 Ctr-0.5 0.51
X  MT20  2.0x 4.0 Ctr Ctr 0.00
Z  MT20  2.0x 4.0 Ctr Ctr 0.00
BB MT20  2.0x 4.0 Ctr Ctr 0.00
DD MT20  2.0x 4.0 Ctr Ctr 0.00
FF MT20  2.0x 4.0 Ctr Ctr 0.00
HH MT20  2.0x 4.0 Ctr Ctr 0.00
JJ MT20  2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
WARNING  Do Not Cut overframe
  member between outside of
  truss and first tie-plate
  to inside of heel plate.
Design checked for 10 psf non-
  concurrent LL on BC.
Truss designed for wind loads
  in the plane of the truss
  only.  For studs exposed to
  wind (normal to the face),
  see Standard Industry Gable
  End Details as applicable,
  or consult qualified
  Building Designer as per
  ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force      172 Lbs
Max tens. force      107 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
ANSI/TPI 1.
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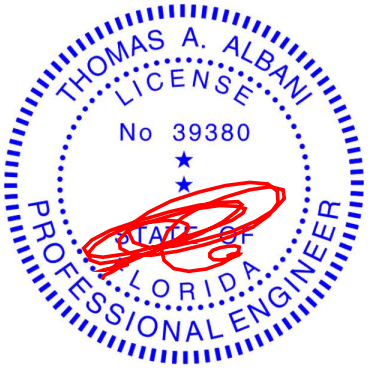
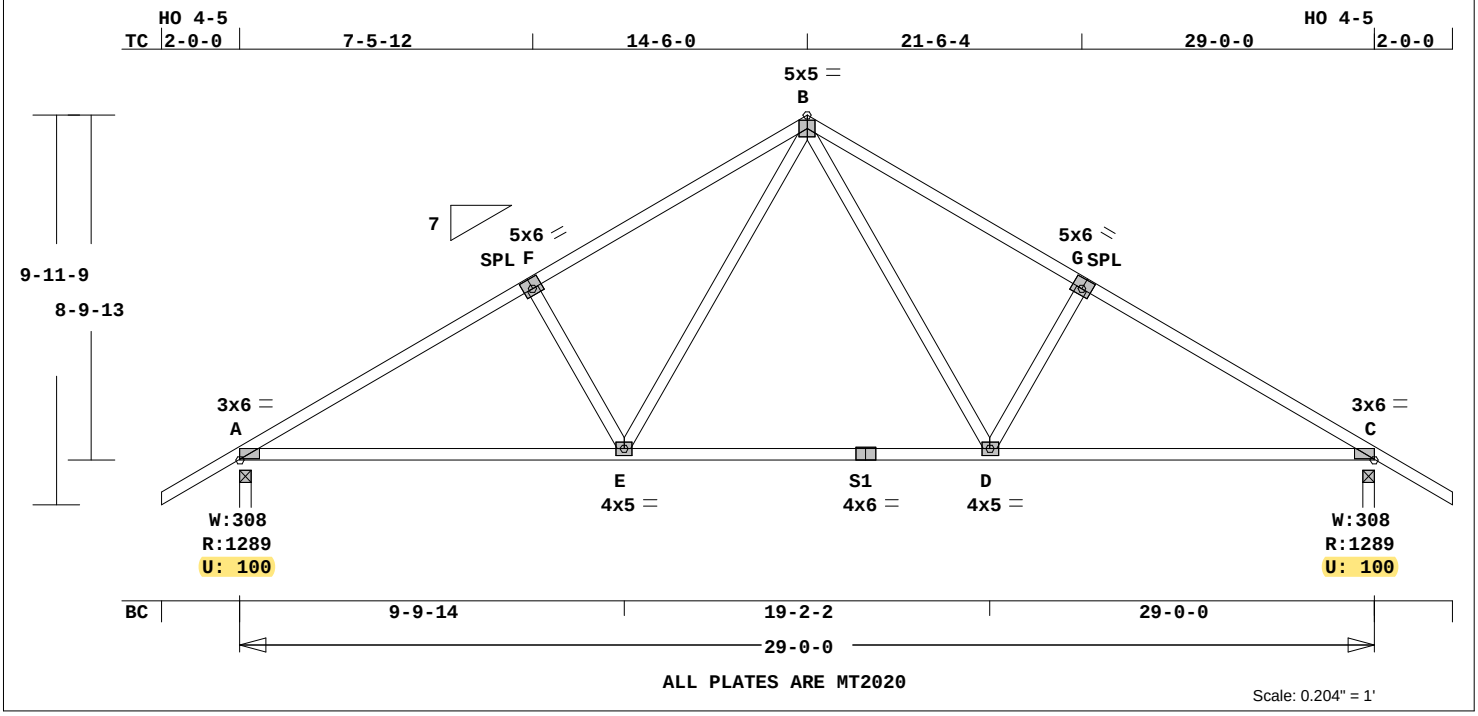


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R15	1	TR	290000	7	2- 0- 0	2- 0- 0	T20412074

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 181.1 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.49	2x 4	SP-#2
BC	0.95	2x 4	SP-#2
WB	0.27	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	29- 0- 0
or	42.0"	0- 0- 0	29- 0- 0
BC	Cont.	0- 0- 0	29- 0- 0
or	120.0"	0- 0- 0	29- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1289	101 U	145 R
C	1289	101 U	145 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.49	1787 C	0.02 0.47
F -B	0.49	1605 C	0.02 0.47
B -G	0.49	1605 C	0.02 0.47
G -C	0.49	1787 C	0.02 0.47
--------Bottom Chords---------			
A -E	0.89	1550 T	0.31 0.58
E -S1	0.95	1025 T	0.21 0.74
S1-D	0.95	1025 T	0.21 0.74
D -C	0.89	1550 T	0.31 0.58
----------Webs----------			
F -E	0.17	355 C	
E -B	0.27	718 T	
B -D	0.27	719 T	
D -G	0.17	355 C	

CT Defl -0.52" in E -D L/660
LL Defl -0.40" in E -D L/846
Shear // Grain in E -S1 0.25

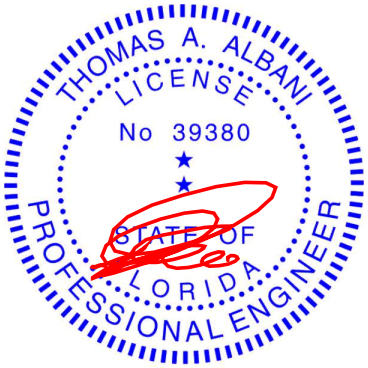
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 6.0	0.2	0.1	0.68
F	MT20	5.0x 6.0-0.3	0.5		0.91
B	MT20	5.0x 5.0 Ctr	Ctr		0.65
G	MT20	5.0x 6.0	0.3	0.5	0.91
C	MT20	3.0x 6.0-0.2	0.1		0.68
E	MT20	4.0x 5.0 Ctr	Ctr		0.36
S1	MT20	4.0x 6.0 Ctr	0.2		0.67
D	MT20	4.0x 5.0 Ctr	Ctr		0.36

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

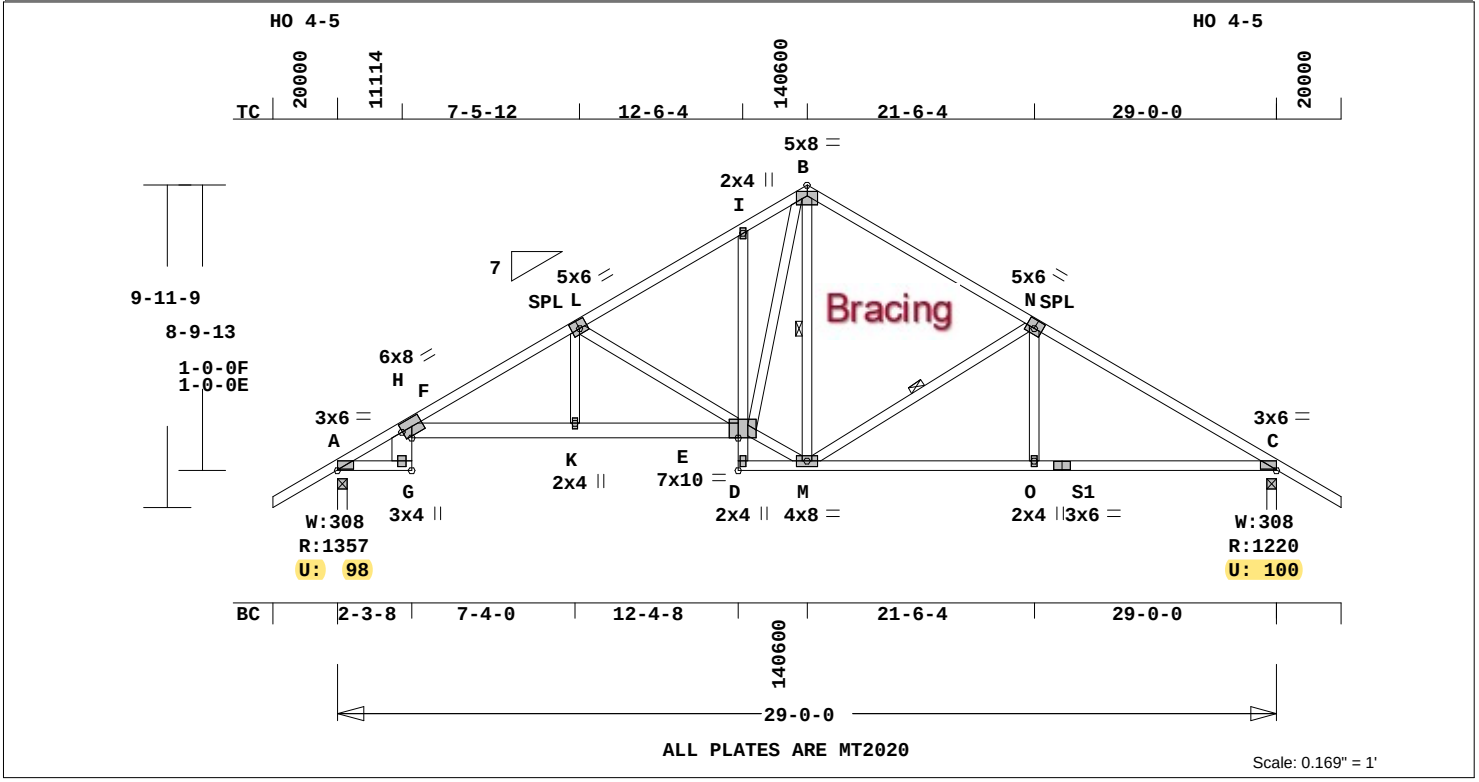
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1787 Lbs
Max tens. force 1550 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R16	1	BOXTRAY	290000	7	2- 0- 0	2- 0- 0	T20412075

Adams Residence



Scale: 0.169" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 226.8 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.71  2x 4  SP-#2
BC  0.76  2x 6  M26-MSR2400
--  0.50  2x 4  SP-#2
    A -G   D -S1  S1-C
--  0.35  2x 8  SP-2400f-2.0E
    G -H
CW  0.60  2x 4  SP-#3
WB  0.75  2x 4  SP-#3
M26-MSR2400 is SP-M26
 or SP-2400f-2.0E

Brace truss as follows:
      O.C.     From      To
 TC  Cont.    0- 0- 0 29- 0- 0
 or   30.0"   0- 0- 0 29- 0- 0
 BC  Cont.    0- 0- 0 29- 0- 0
 or   72.0"   0- 0- 0 29- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  M -B   M -N
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1357    98 U   144 R
C   1221   101 U   144 R

Jt   Brg Size   Required
A       3.5"       1.6"
C       3.5"       1.5"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -H  0.50  1784 C  0.02  0.48
H -L  0.71  2472 C  0.08  0.63
L -I  0.40  1473 C  0.01  0.39
I -B  0.09  1454 C  0.01  0.08
B -N  0.49  1136 C  0.01  0.48
N -C  0.51  1662 C  0.02  0.49
--------Bottom Chords---------
A -G  0.50  1436 T  0.29  0.21
G -H  0.35    65 C  0.00  0.35
H -K  0.76  2171 T  0.10  0.66
K -E  0.26  2171 T  0.10  0.16
D -M  0.20   115 T  0.00  0.20
M -O  0.47  1442 T  0.17  0.30
O -S1 0.50  1442 T  0.29  0.21
S1-C  0.47  1442 T  0.29  0.18
----------Chord-Webs----------
D -E  0.60    23 C  0.00  0.60
E -I  0.03   163 C  0.02  0.01
------------Webs-------------
K -L  0.27   714 T
L -E  0.75  1076 C
E -B  0.43  1150 T
E -M  0.37   979 T
M -B  0.03   102 T        1 Br
M -N  0.20   556 C        1 Br
O -N  0.09   303 T

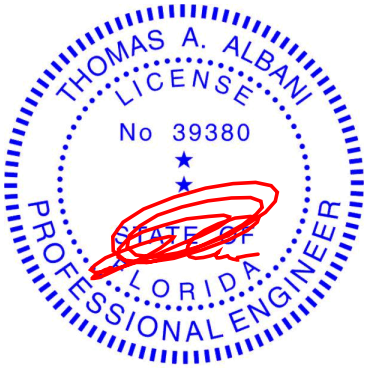
CT Defl  -0.44" in F -K  L/768
LL Defl  -0.27" in F -K  L/999
Shear // Grain  in G -H   0.83

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 6.0 0.2 0.1 0.69
H  MT20  6.0x 8.0 Ctr Ctr 0.69
L  MT20  5.0x 6.0-0.3 0.5 0.80
I  MT20  2.0x 4.0 Ctr Ctr 0.33
B  MT20  5.0x 8.0 Ctr-0.8 0.93
N  MT20  5.0x 6.0 0.3 0.5 0.91
C  MT20  3.0x 6.0-0.2 0.1 0.63
G  MT20  3.0x 4.0 Ctr Ctr 0.80
K  MT20  2.0x 4.0 Ctr Ctr 0.89
E  MT20  7.0x10.0 Ctr 0.8 0.44
D  MT20  2.0x 4.0 Ctr Ctr 0.58
M  MT20  4.0x 8.0 Ctr Ctr 0.48
O  MT20  2.0x 4.0 Ctr Ctr 0.34
S1 MT20  3.0x 6.0 Ctr Ctr 0.55

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :        4.0 psf
  BC Dead Load :        6.0 psf
Max comp. force     2472 Lbs
Max tens. force     2171 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

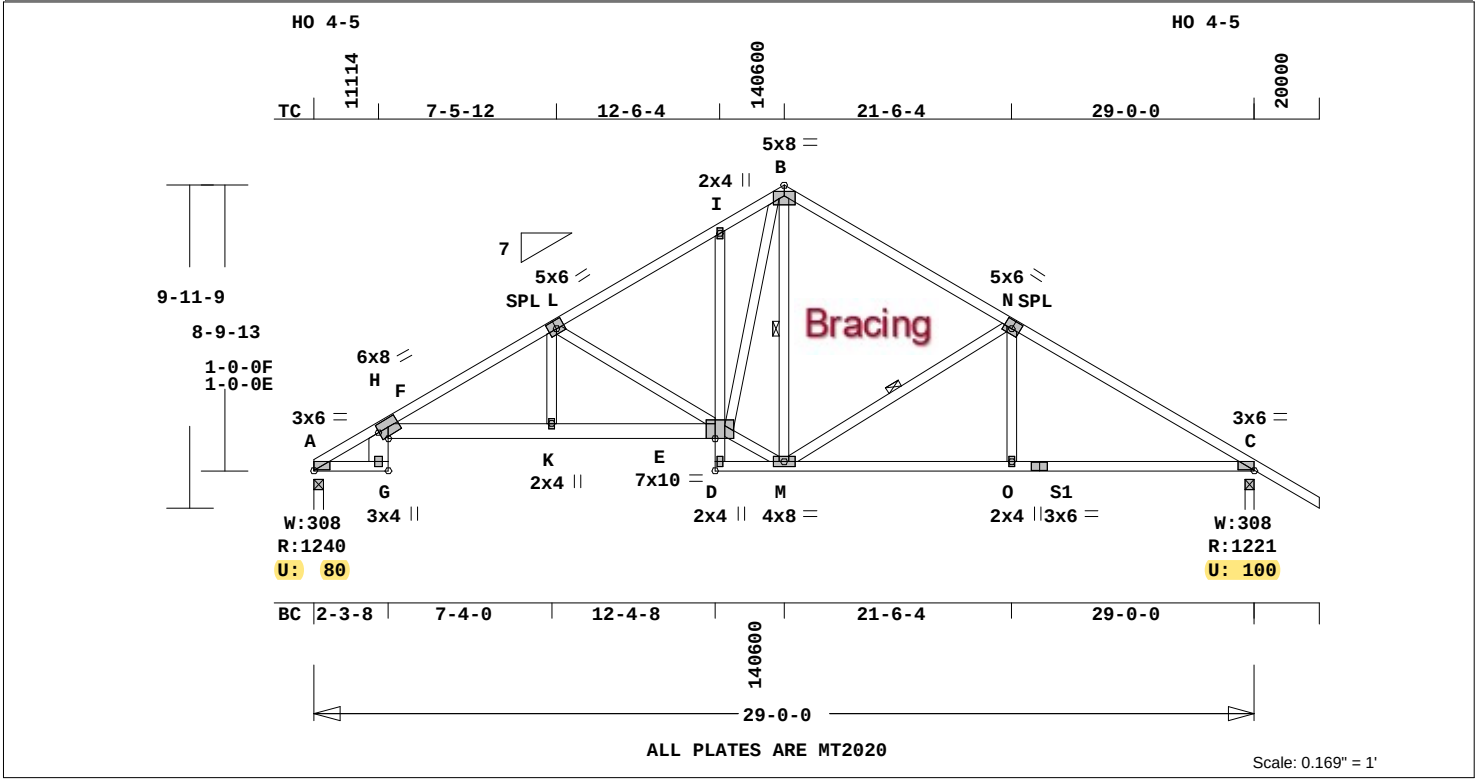


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R16A	5	BOXTRAY	290000	7	0	2- 0- 0	T20412076

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 222.4 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.72  2x 4  SP-#2
BC  0.77  2x 6  M26-MSR2400
--  0.50  2x 4  SP-#2
    A -G   D -S1  S1-C
--  0.35  2x 8  SP-2400f-2.0E
    G -H
CW  0.60  2x 4  SP-#3
WB  0.75  2x 4  SP-#3
M26-MSR2400 is SP-M26
 or SP-2400f-2.0E

Brace truss as follows:
      O.C.      From      To
 TC  Cont.    0- 0- 0 29- 0- 0
 or   30.0"   0- 0- 0 29- 0- 0
 BC  Cont.    0- 0- 0 29- 0- 0
 or   72.0"   0- 0- 0 29- 0- 0
Continuous Lateral Restraint
  req'd at mid-point of webs:
  M -B   M -N
Attach CLR with (2)-10d nails
  at each web.
  Refer to BCSI for diagonal
  restraint requirements.

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
         Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1241    80 U   144 R
C   1221   101 U   144 R

Jt   Brg Size   Required
A       3.5"       1.5"
C       3.5"       1.5"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 BC LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -H  0.50  1800 C  0.02  0.48
H -L  0.72  2477 C  0.08  0.64
L -I  0.40  1475 C  0.01  0.39
I -B  0.09  1456 C  0.01  0.08
B -N  0.49  1137 C  0.01  0.48
N -C  0.51  1663 C  0.02  0.49
--------Bottom Chords---------
A -G  0.49  1452 T  0.29  0.20
G -H  0.35    59 C  0.00  0.35
H -K  0.77  2176 T  0.10  0.67
K -E  0.26  2176 T  0.10  0.16
D -M  0.20   115 T  0.00  0.20
M -O  0.47  1443 T  0.17  0.30
O -S1 0.50  1443 T  0.29  0.21
S1-C  0.47  1443 T  0.29  0.18
----------Chord-Webs----------
D -E  0.60    23 C  0.00  0.60
E -I  0.03   163 C  0.02  0.01
------------Webs-------------
K -L  0.27   718 T
L -E  0.75  1080 C
E -B  0.43  1152 T
E -M  0.37   980 T
M -B  0.03   102 T        1 Br
M -N  0.20   556 C        1 Br
O -N  0.09   303 T

CT Defl  -0.45" in F -K  L/764
LL Defl  -0.27" in F -K  L/999
Shear // Grain  in G -H   0.84

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 6.0 0.2 0.1 0.69
H  MT20  6.0x 8.0 Ctr Ctr 0.69
L  MT20  5.0x 6.0-0.3 0.5 0.80
I  MT20  2.0x 4.0 Ctr Ctr 0.33
B  MT20  5.0x 8.0 Ctr-0.8 0.94
N  MT20  5.0x 6.0 0.3 0.5 0.91
C  MT20  3.0x 6.0-0.2 0.1 0.63
G  MT20  3.0x 4.0 Ctr Ctr 0.80
K  MT20  2.0x 4.0 Ctr Ctr 0.90
E  MT20  7.0x10.0 Ctr 0.8 0.44
D  MT20  2.0x 4.0 Ctr Ctr 0.58
M  MT20  4.0x 8.0 Ctr Ctr 0.48
O  MT20  2.0x 4.0 Ctr Ctr 0.34
S1 MT20  3.0x 6.0 Ctr Ctr 0.55

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
OH Loading
  Soffit psf 2.0
This truss has been designed
  for 20.0  psf LL on the B.C.
  in areas where a rectangle
   3- 6- 0 tall by
   2- 0- 0 wide
  will fit between the B.C.
  and any other member.
Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     2477 Lbs
Max tens. force     2176 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
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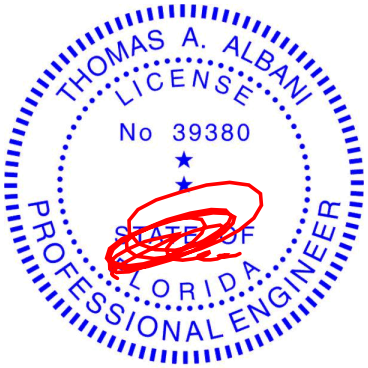
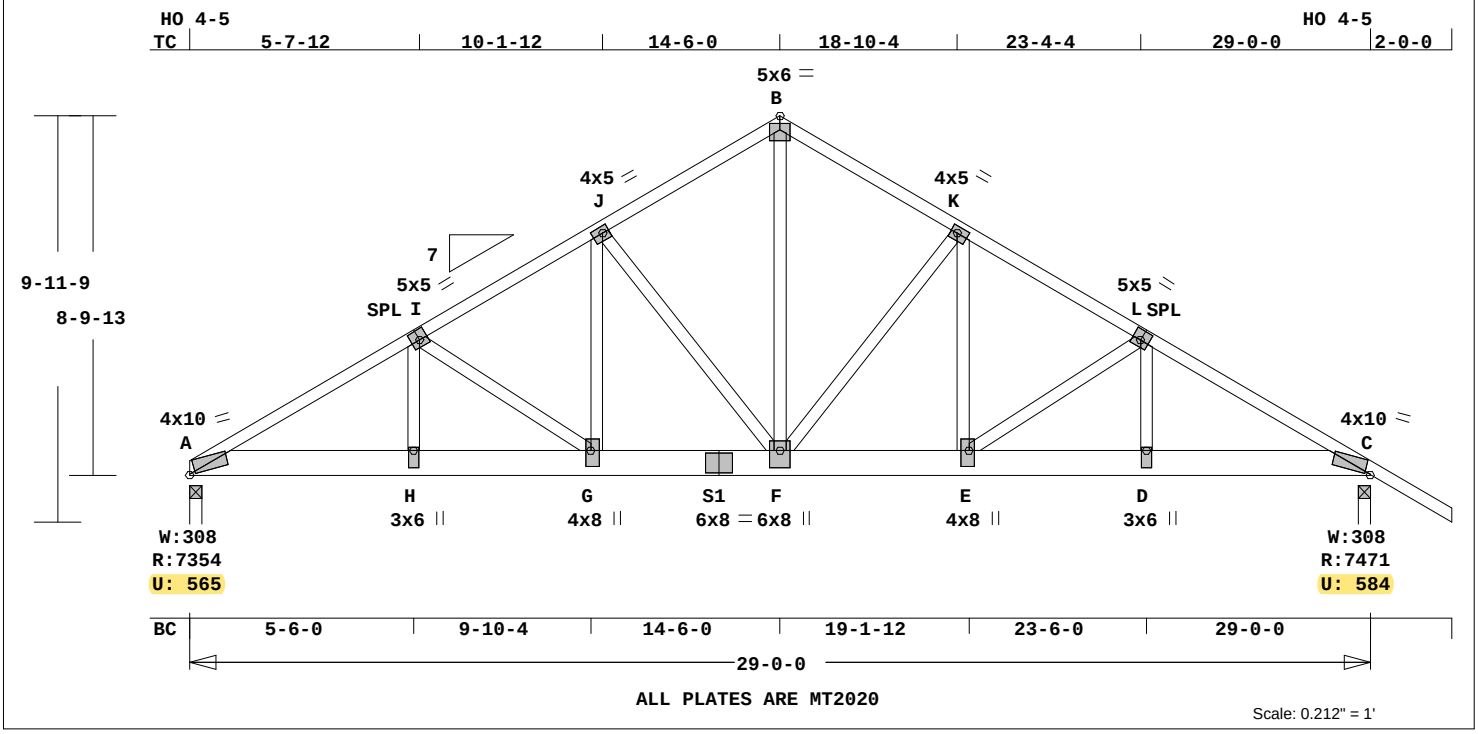


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R17	1*2P	TR	290000	7	0	2- 0- 0	T20412077

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 267.9 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20
****
* 2-Ply Truss *
****

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.86	2x 4	SP-#2
BC	0.40	2x 8	SP-2400f-2.0E
WB	0.80	2x 4	SP-#2
--	0.67	2x 4	SP-#3

H -I I -G G -J J -F
F -K E -K E -L D -L

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	29- 0- 0
or	30.0"	0- 0- 0	29- 0- 0
BC	Cont.	0- 0- 0	29- 0- 0
or	120.0"	0- 0- 0	29- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.00	1.00	1.00	1.00
BC	1.00	1.00	1.00	1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	7354	566 U	143 R
C	7471	584 U	143 R

Jt	Brg Size	Required
A	3.5"	3.0"
C	3.5"	3.1"

LC# 1 Girder Loading
Dur Fctrs - Lbr 1.25 Plt 1.25

plf - Dead	Live*	From	To
TC V	14	40	0.0' 29.0'
BC V	219	234	0.0' 29.0'

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -I	0.86	11922 C	0.53 0.33
I -J	0.53	9596 C	0.36 0.17
J -B	0.36	7382 C	0.25 0.11
B -K	0.36	7382 C	0.25 0.11
K -L	0.53	9595 C	0.36 0.17
L -C	0.86	11917 C	0.53 0.33
--------Bottom Chords---------			
A -H	0.40	10314 T	0.19 0.21
H -G	0.30	10314 T	0.19 0.11
G -S1	0.21	8284 T	0.15 0.06
S1-F	0.25	8284 T	0.15 0.10
F -E	0.25	8283 T	0.15 0.10
E -D	0.30	10310 T	0.19 0.11
D -C	0.40	10310 T	0.19 0.21
------------Webs------------			
H -I	0.43	2288 T	
I -G	0.34	2464 C	
G -J	0.63	3315 T	
J -F	0.67	3064 C	
F -B	0.80	7145 T	
F -K	0.67	3063 C	
E -K	0.63	3314 T	
E -L	0.34	2460 C	
D -L	0.43	2283 T	

CT Defl -0.38" in G -F L/888
LL Defl -0.21" in G -F L/999
Shear // Grain in A -H 0.37

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	4.0x10.0	Ctr	Ctr	0.98
I	MT20	5.0x 5.0	-0.3	0.5	0.92
J	MT20	4.0x 5.0	-0.4	-0.3	0.85
B	MT20	5.0x 6.0	Ctr	-0.7	0.99
K	MT20	4.0x 5.0	0.4	-0.3	0.85
L	MT20	5.0x 5.0	0.3	0.5	0.92
C	MT20	4.0x10.0	Ctr	Ctr	0.98
H	MT20	3.0x 6.0	Ctr	-2.1	0.84
G	MT20	4.0x 8.0	0.5	-0.6	0.88
S1	MT20	6.0x 8.0	Ctr	Ctr	0.79
F	MT20	6.0x 8.0	Ctr	-1.1	0.81
E	MT20	4.0x 8.0	-0.5	-0.6	0.88
D	MT20	3.0x 6.0	Ctr	-2.1	0.83

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

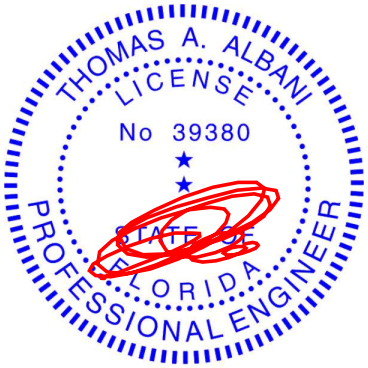
REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2017
TPI 2014
Girder Common
Loading BC
Span 25- 5- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten plies together in staggered pattern.

	Connector	Rows	Spacing
TC	10d Gun Nails	1	9.5 in
BC	10d Gun Nails	3	12.0 in
WB	10d Gun Nails	1	8.0 in

Web Connection Exception -- Space connectors for the following webs-
J -F 1 rows @ 4.0" o.c.
F -K 1 rows @ 4.0" o.c.
10d gun nails (0.131"x3") must be installed as noted above, as each layer is applied.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 11922 Lbs
Max tens. force 10314 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

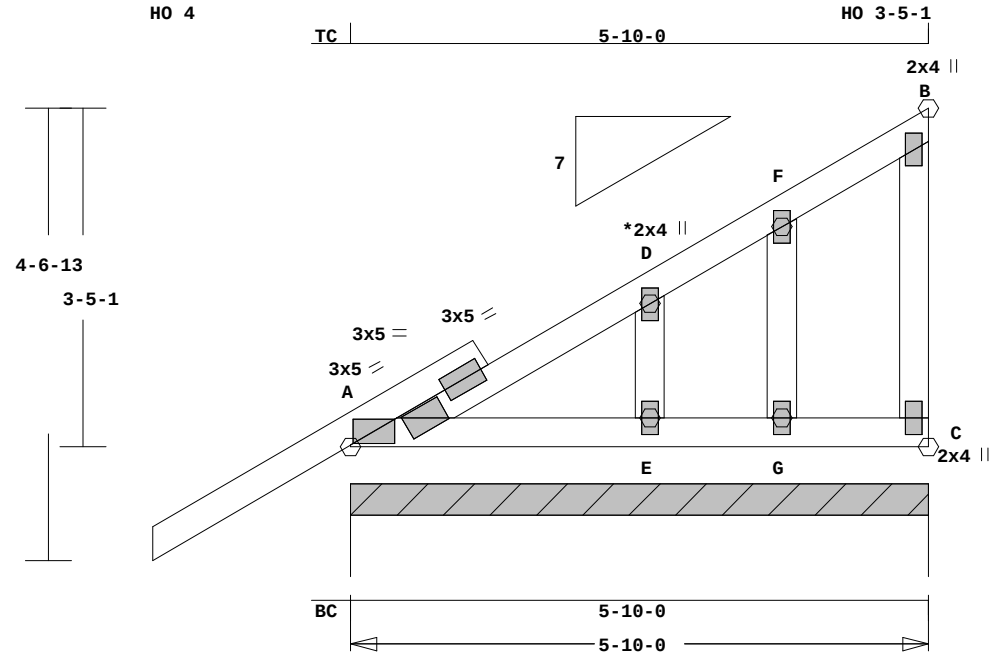


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R18	1	MONO	51000	7	0	0	T20412078

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.515" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 41.6 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.06	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
WB	0.04	2x 4	SP-#3
GW	0.08	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5-10- 0
or	48.0"	0- 0- 0	5-10- 0
BC	Cont.	0- 0- 0	5-10- 0
or	70.0"	0- 0- 0	5-10- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	432	33 U	89 R

Jt	Brg Size	Required
A	70.0"	0"-to- 70"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.06	56 C	0.00 0.06
D -F	0.02	44 C	0.00 0.02
F -B	0.01	25 C	0.00 0.01
--------Bottom Chords---------			
A -E	0.05	4 T	0.00 0.05
E -G	0.02	0 T	0.00 0.02
G -C	0.01	0 T	0.00 0.01
------------Webs------------			
C -B	0.04	42 C	0.00 0.04
----------Gable Webs----------			
E -D	0.08	110 C	0.00 0.08
G -F	0.01	66 C	0.00 0.01

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -D 0.09

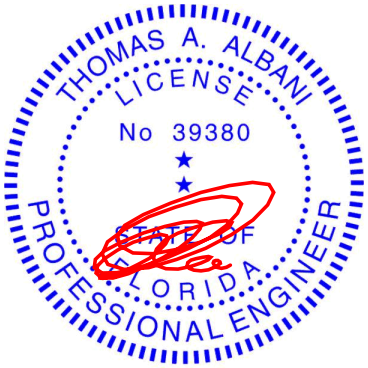
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	2.0x 4.0	Ctr	Ctr	0.18
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	2.0x 4.0	Ctr	Ctr	0.22

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 110 Lbs
Max tens. force 44 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

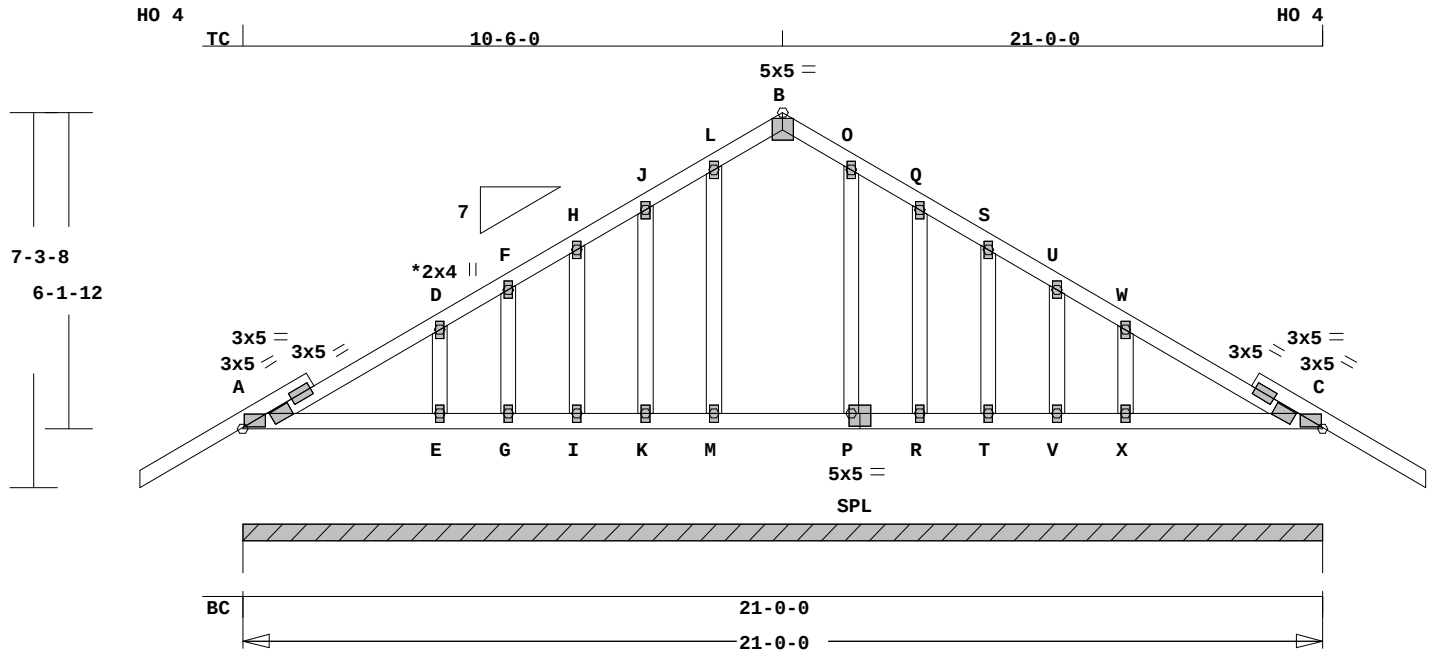


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R19	1	TR	210000	7	0	0	T20412079

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.268" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 160.1 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.09	2x 4	SP-#2
BC	0.08	2x 4	SP-#2
GW	0.03	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	21- 0- 0
or	48.0"	0- 0- 0	21- 0- 0
BC	Cont.	0- 0- 0	21- 0- 0
or	72.0"	0- 0- 0	21- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25				
Plate Duration Factor 1.25				
	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1554	120 U	98 R

Jt	Brg Size	Required
A	252.0"	0"-to- 252"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.09	91 C	0.00 0.09
D -F	0.08	105 C	0.00 0.08
F -H	0.01	88 C	0.00 0.01
H -J	0.01	83 C	0.00 0.01
J -L	0.01	85 C	0.00 0.01
L -B	0.01	82 C	0.00 0.01
B -O	0.01	82 C	0.00 0.01
O -Q	0.01	85 C	0.00 0.01
Q -S	0.01	83 C	0.00 0.01
S -U	0.01	88 C	0.00 0.01
U -W	0.08	105 C	0.00 0.08
W -C	0.09	91 C	0.00 0.09
Bottom Chords			
A -E	0.08	6 T	0.00 0.08
E -G	0.06	0 T	0.00 0.06
G -I	0.01	0 T	0.00 0.01
I -K	0.01	0 T	0.00 0.01
K -M	0.03	0 T	0.00 0.03
M -P	0.03	0 T	0.00 0.03
P -R	0.03	0 T	0.00 0.03
R -T	0.01	0 T	0.00 0.01
T -V	0.01	0 T	0.00 0.01
V -X	0.06	0 T	0.00 0.06
X -C	0.08	6 T	0.00 0.08
Gable Webs			
E -D	0.03	173 C	
G -F	0.00	38 C	
I -H	0.02	74 C	
K -J	0.03	76 C	
M -L	0.03	61 C	
P -O	0.03	61 C	
R -Q	0.03	76 C	
T -S	0.02	74 C	
V -U	0.00	38 C	
X -W	0.03	173 C	

CT Defl -0.01" in A -E L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -D 0.11

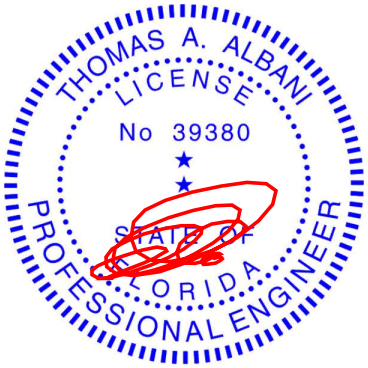
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	2.0x 4.0	Ctr	Ctr	0.00
J	MT20	2.0x 4.0	Ctr	Ctr	0.00
L	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	5.0x 5.0	Ctr-1.9		0.33
O	MT20	2.0x 4.0	Ctr	Ctr	0.00
Q	MT20	2.0x 4.0	Ctr	Ctr	0.00
S	MT20	2.0x 4.0	Ctr	Ctr	0.00
U	MT20	2.0x 4.0	Ctr	Ctr	0.00
W	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 5.0	Ctr	Ctr	0.45
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
I	MT20	2.0x 4.0	Ctr	Ctr	0.00
K	MT20	2.0x 4.0	Ctr	Ctr	0.00
M	MT20	2.0x 4.0	Ctr	Ctr	0.00
P	MT20	5.0x 5.0	Ctr-0.5		0.51
R	MT20	2.0x 4.0	Ctr	Ctr	0.00
T	MT20	2.0x 4.0	Ctr	Ctr	0.00
V	MT20	2.0x 4.0	Ctr	Ctr	0.00
X	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 173 Lbs
Max tens. force 63 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

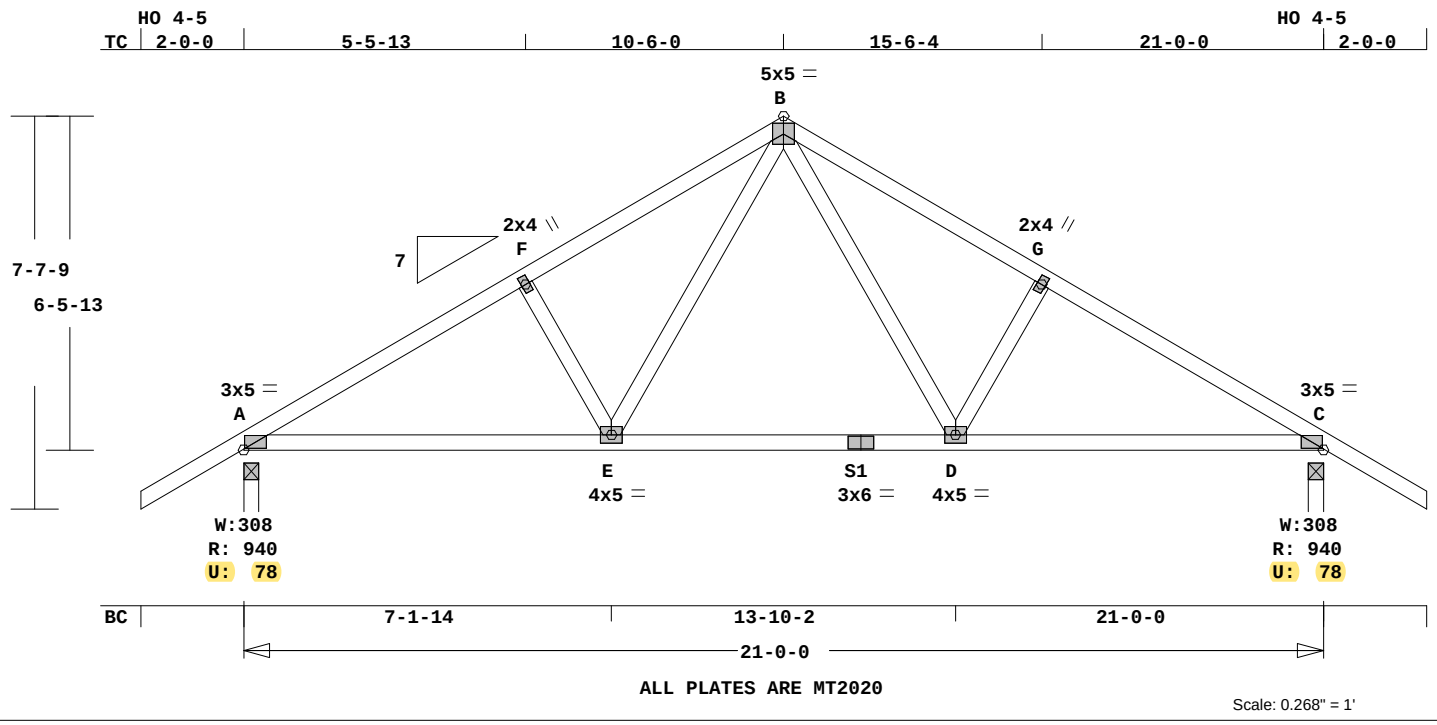


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R20	1	TR	210000	7	2- 0- 0	2- 0- 0	T20412080

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.268" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 133.2 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.23	2x 4	SP-#2
BC	0.45	2x 4	SP-#2
WB	0.18	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	21- 0- 0
or	48.0"	0- 0- 0	21- 0- 0
BC	Cont.	0- 0- 0	21- 0- 0
or	120.0"	0- 0- 0	21- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	940	78 U	104 R
C	940	78 U	104 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -F	0.23	1213 C	0.01 0.22
F -B	0.23	1085 C	0.01 0.22
B -G	0.23	1085 C	0.01 0.22
G -C	0.23	1213 C	0.01 0.22
Bottom Chords			
A -E	0.45	1051 T	0.21 0.24
E -S1	0.44	698 T	0.14 0.30
S1-D	0.44	698 T	0.14 0.30
D -C	0.45	1051 T	0.21 0.24
Webs			
F -E	0.07	250 C	
E -B	0.18	474 T	
B -D	0.18	474 T	
D -G	0.07	250 C	

CT Defl -0.14" in E -D L/999
LL Defl -0.10" in E -D L/999
Shear // Grain in A -F 0.16

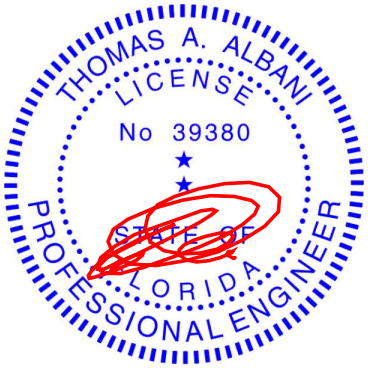
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.56
F	MT20	2.0x 4.0	Ctr	Ctr	0.30
B	MT20	5.0x 5.0	Ctr	Ctr	0.43
G	MT20	2.0x 4.0	Ctr	Ctr	0.30
C	MT20	3.0x 5.0	Ctr	Ctr	0.56
E	MT20	4.0x 5.0	Ctr	Ctr	0.23
S1	MT20	3.0x 6.0	Ctr	Ctr	0.39
D	MT20	4.0x 5.0	Ctr	Ctr	0.23

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

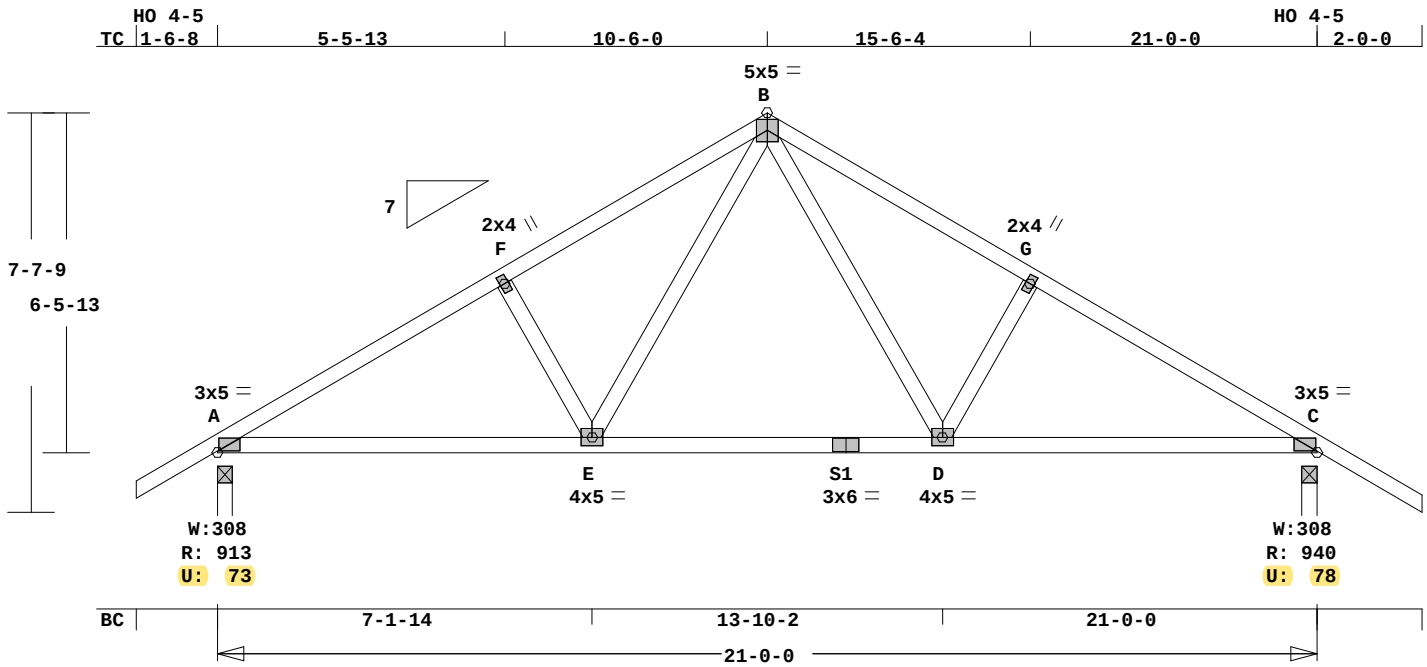
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1213 Lbs
Max tens. force 1051 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R21	1	TR	210000	7	1- 6- 8	2- 0- 0	T20412081

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 132.2 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

CSI	-Size-	----Lumber----
TC 0.23	2x 4	SP-#2
BC 0.45	2x 4	SP-#2
WB 0.18	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	21- 0- 0
or	48.0"	0- 0- 0	21- 0- 0
BC	Cont.	0- 0- 0	21- 0- 0
or	120.0"	0- 0- 0	21- 0- 0

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	913	74 U	104 R
C	940	78 U	104 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -F	0.23	1214 C	0.01 0.22
F -B	0.23	1086 C	0.01 0.22
B -G	0.23	1085 C	0.01 0.22
G -C	0.23	1214 C	0.01 0.22
--------Bottom Chords---------			
A -E	0.45	1052 T	0.21 0.24
E -S1	0.44	699 T	0.14 0.30
S1-D	0.44	699 T	0.14 0.30
D -C	0.45	1052 T	0.21 0.24
------------Webs------------			
F -E	0.07	250 C	
E -B	0.18	475 T	
B -D	0.18	474 T	
D -G	0.07	250 C	

CT Defl -0.14" in E -D L/999
LL Defl -0.10" in E -D L/999
Shear // Grain in A -F 0.16

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

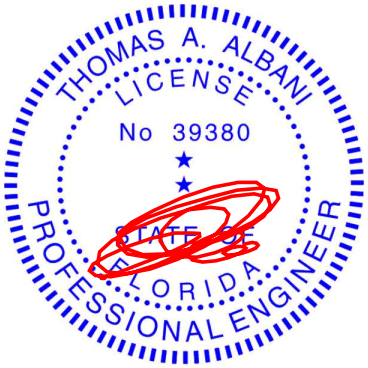
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.56
F	MT20	2.0x 4.0	Ctr	Ctr	0.30
B	MT20	5.0x 5.0	Ctr	Ctr	0.43
G	MT20	2.0x 4.0	Ctr	Ctr	0.30
C	MT20	3.0x 5.0	Ctr	Ctr	0.56
E	MT20	4.0x 5.0	Ctr	Ctr	0.23
S1	MT20	3.0x 6.0	Ctr	Ctr	0.39
D	MT20	4.0x 5.0	Ctr	Ctr	0.23

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.

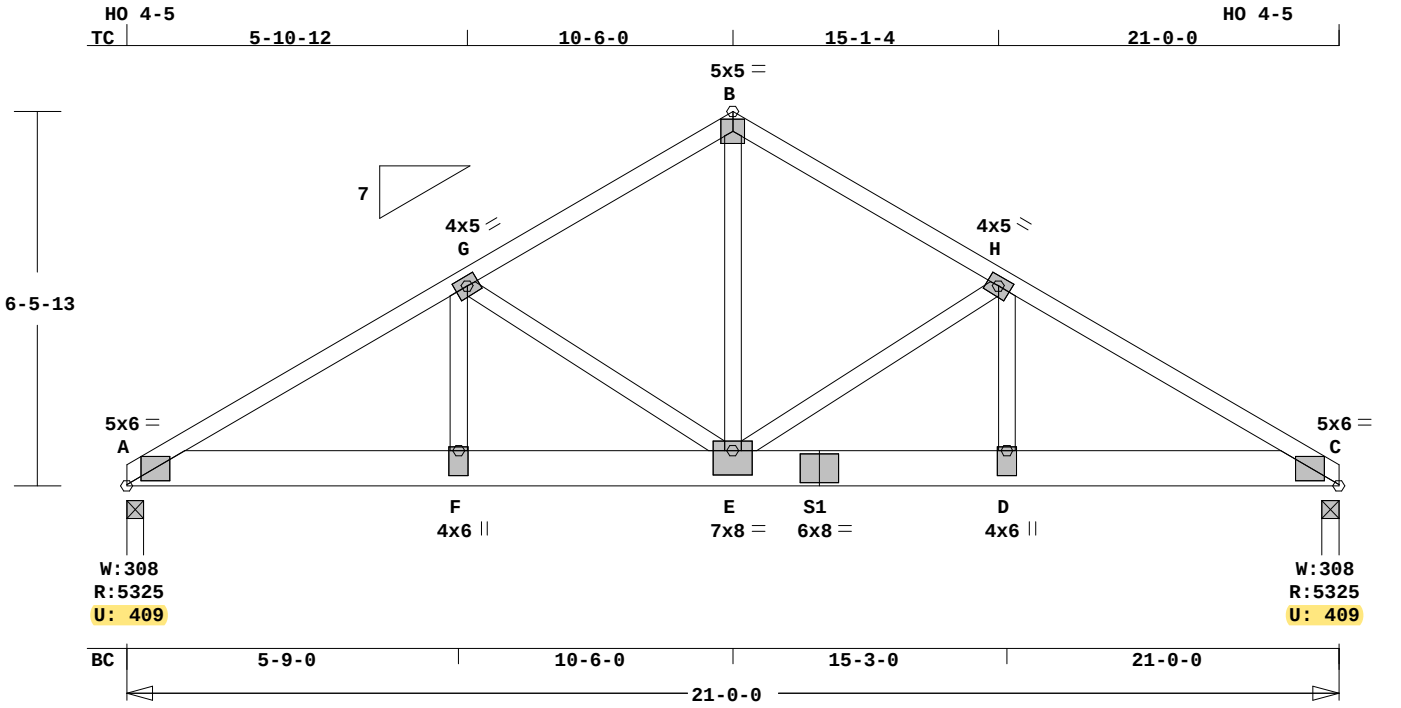
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 1214 Lbs
Max tens. force 1052 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R22	1*2P	TR	210000	7	0	0	T20412082

Adams Residence



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 167.2 LBS

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Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20
***************
* 2-Ply Truss *
***************

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.53  2x 4  SP-#2
BC  0.34  2x 8  SP-2400f-2.0E
WB  0.98  2x 4  SP-#3

Brace truss as follows:
      O.C.      From      To
 TC  Cont.    0- 0- 0 21- 0- 0
 or   42.0"   0- 0- 0 21- 0- 0
 BC  Cont.    0- 0- 0 21- 0- 0
 or  120.0"   0- 0- 0 21- 0- 0

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
        Fb    Fc    Ft   Emin
TC     1.00  1.00  1.00  1.00
BC     1.00  1.00  1.00  1.00

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   5326   410 U   103 R
C   5326   410 U   103 R

Jt   Brg Size   Required
A       3.5"       2.2"
C       3.5"       2.2"

LC#  1  Girder Loading
Dur Fctrs - Lbr 1.25  Plt 1.25
plf - Dead  Live* From    To
TC V    14    40   0.0'  21.0'
BC V   219   234   0.0'  21.0'

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -G  0.53  7867 C  0.24  0.29
G -B  0.21  5400 C  0.14  0.07
B -H  0.21  5400 C  0.14  0.07
H -C  0.53  7867 C  0.24  0.29
--------Bottom Chords---------
A -F  0.34  6809 T  0.13  0.21
F -E  0.21  6809 T  0.13  0.08
E -S1 0.21  6809 T  0.13  0.08
S1-D  0.20  6809 T  0.13  0.07
D -C  0.34  6809 T  0.13  0.21
------------Webs-------------
F -G  0.46  2432 T
G -E  0.39  2594 C
E -B  0.98  5169 T
E -H  0.39  2594 C
D -H  0.46  2432 T

CT Defl  -0.20" in E -D  L/999
LL Defl  -0.11" in E -D  L/999
Shear // Grain  in A -F   0.40

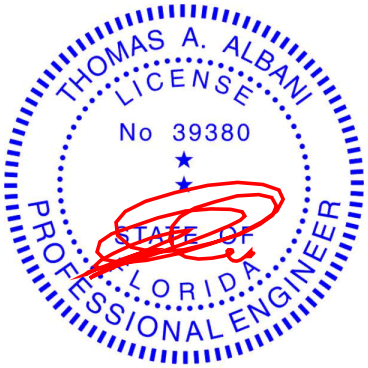
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  5.0x 6.0 Ctr-0.2 0.97
G  MT20  4.0x 5.0 Ctr Ctr 0.76
B  MT20  5.0x 5.0 Ctr Ctr 0.97
H  MT20  4.0x 5.0 Ctr Ctr 0.76
C  MT20  5.0x 6.0 Ctr-0.2 0.97
F  MT20  4.0x 6.0 Ctr-2.2 0.89
E  MT20  7.0x 8.0 Ctr-1.5 0.97
S1 MT20  6.0x 8.0 Ctr Ctr 0.65
D  MT20  4.0x 6.0 Ctr-2.2 0.89

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014
Girder    Common
  Loading BC
  Span    25- 5- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten plies together in
  staggered pattern.
     Connector    Rows Spacing
TC 10d Gun Nails    1  12.0 in
BC 10d Gun Nails    3  12.0 in
WB 10d Gun Nails    1   8.0 in
  10d gun nails (0.131"x3")
  must be installed as noted
  above, as each layer is
  applied.

Design checked for 10 psf non-
  concurrent LL on BC.
Use properly rated hangers for
  loads framing into girder
  truss.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force     7867 Lbs
Max tens. force     6809 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

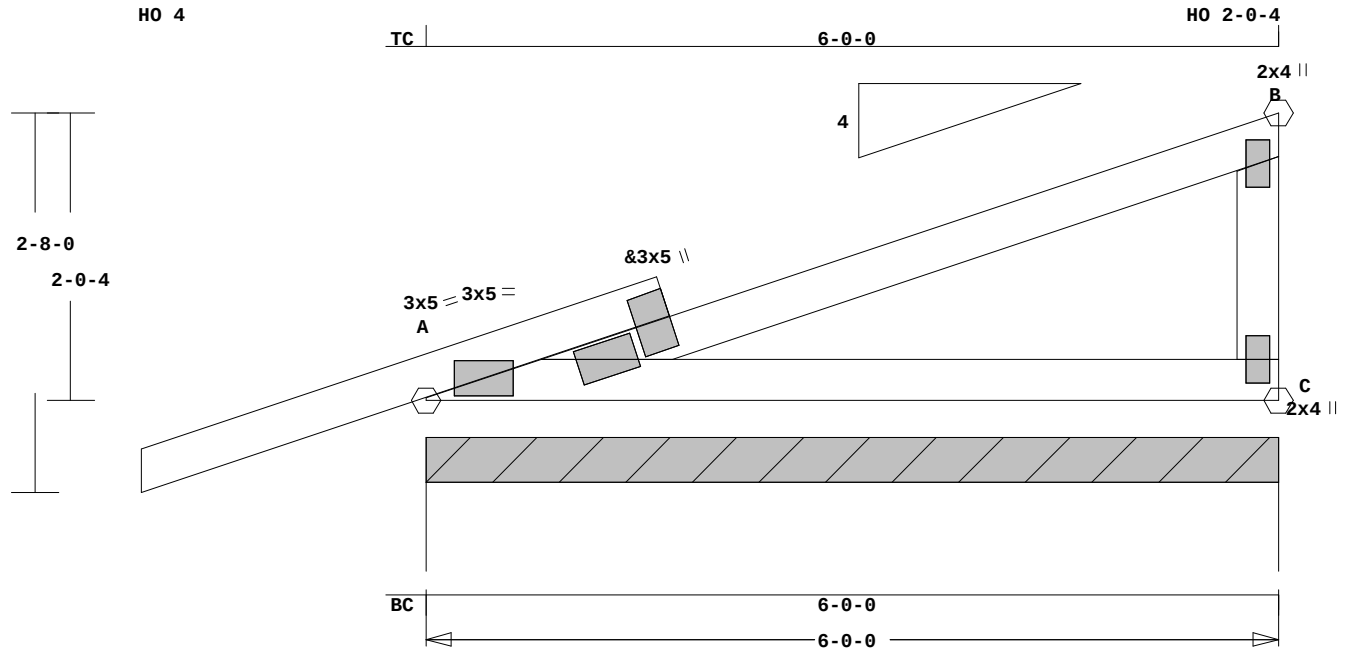


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R23	2	MONO	60000	4	0	0	T20412083

Adams Residence



ALL PLATES ARE MT2020, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.740" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 31.7 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.29	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 0- 0
or	48.0"	0- 0- 0	6- 0- 0
BC	Cont.	0- 0- 0	6- 0- 0
or	72.0"	0- 0- 0	6- 0- 0

psf-Ld	Dead	Live	
TC	7.0	20.0	
BC	10.0	0.0	
TC+BC	17.0	20.0	
Total	37.0	Spacing	24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	444	34 U	48 R

Jt	Brg Size	Required
A	72.0"	0"-to- 72"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.33	18 C	0.00 0.33
Bottom Chords			
A -C	0.29	11 C	0.00 0.29
Webs			
C -B	0.02	132 C	WindLd

CT Defl -0.08" in A -C L/892
LL Defl -0.04" in A -C L/999
Shear // Grain in A -B 0.15

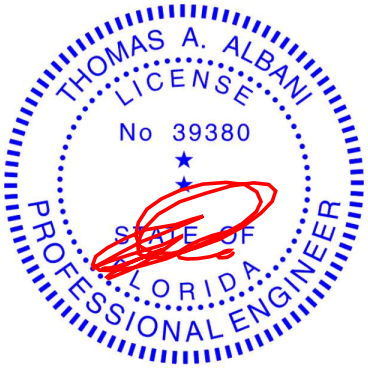
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.52
B	MT20	2.0x 4.0	Ctr	Ctr	0.13
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 132 Lbs
Max tens. force 35 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

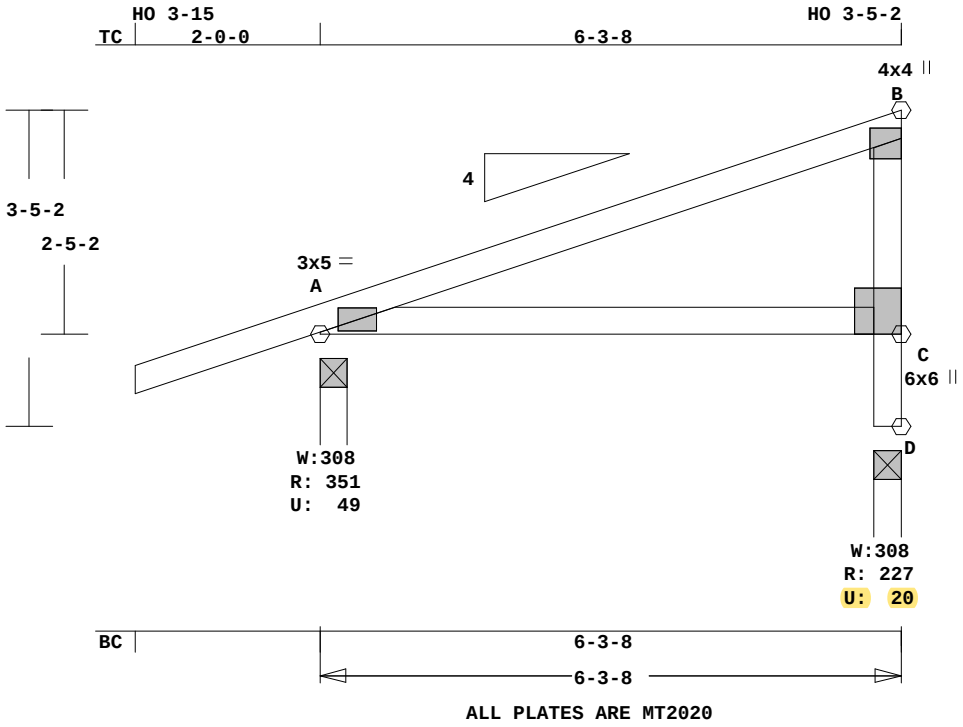


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	R24	14	MONO	60308	4	2- 0- 0	0	T20412084

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.481" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 33.9 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.30	2x 4	SP-#2
BC	0.25	2x 4	SP-#2
WB	0.73	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 3- 8
or	48.0"	0- 0- 0	6- 3- 8
BC	Cont.	0- 0- 0	6- 3- 8
or	75.5"	0- 0- 0	6- 3- 8

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	352	50 U	97 R
D	227	21 U	36 R

Jt	Brg Size	Required
A	3.5"	1.5"
D	3.5"	3.5"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.30	120 C	0.00 0.30
Bottom Chords			
A -C	0.25	122 T	0.02 0.23
Webs			
C -B	0.73	138 C	0.00 0.73
D -C	0.14	227 C	0.00 0.14

CT Defl -0.06" in A -C L/999
LL Defl -0.03" in A -C L/999
Shear // Grain in A -B 0.21

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

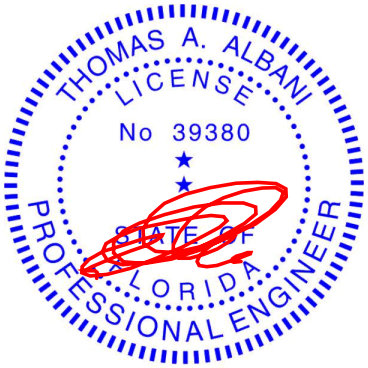
Jt Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0 Ctr	Ctr	0.52
B	MT20	4.0x 4.0-0.2 Ctr		0.81
C	MT20	6.0x 6.0-1.215.0		0.96

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle
3- 6- 0 tall by
2- 0- 0 wide
will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior

TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 227 Lbs
Max tens. force 122 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.
Verticals that are extended below a chord forming a "Leg Down" condition are not designed to resist lateral forces betweeen the truss and the support. Additional design considerations (by others) are required to transfer these forces to the appropriate lateral force resisting elements.

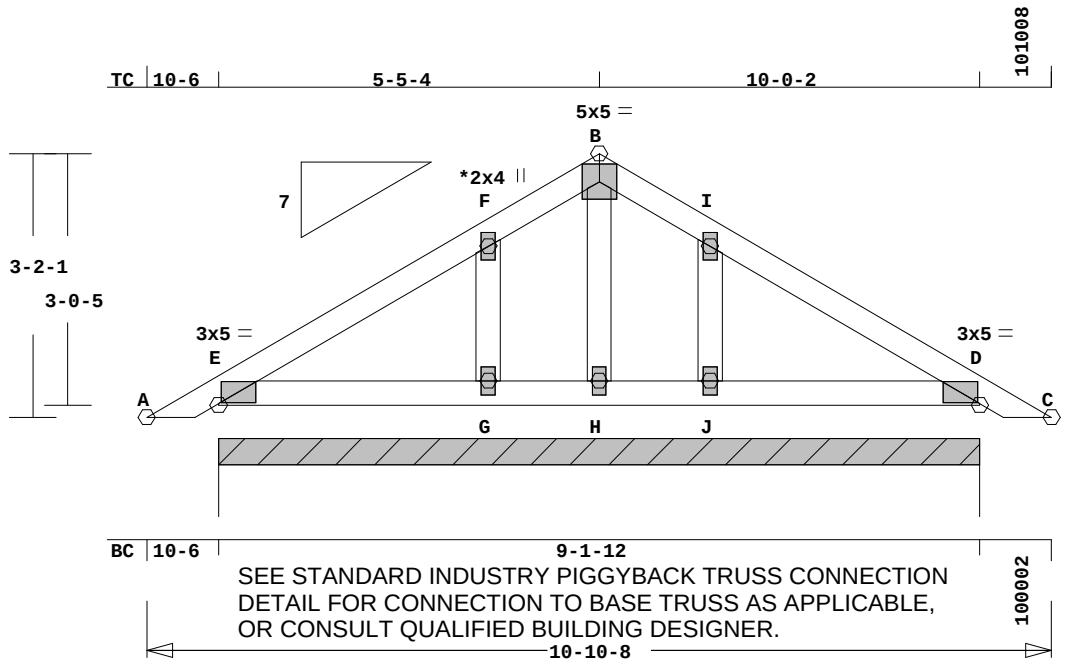


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	T1	2	PB	101008	7	0	0	T20412085

Adams Residence



ALL PLATES ARE MT2020

See Joint F For Typical Gable Plate Size and Placement

Scale: 0.433" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 52.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
GW	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10-10- 8
or	48.0"	0- 0- 0	10-10- 8
BC	Cont.	0- 0- 0	10-10- 8
or	109.8"	0- 0- 0	10-10- 8

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	679	57 U	44 R

Jt	Brg Size	Required
E	109.8"	0"-to- 110"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
E -F	0.07	74 C	0.00 0.07
F -B	0.06	90 C	0.00 0.06
B -I	0.06	90 C	0.00 0.06
I -D	0.07	74 C	0.00 0.07
Bottom Chords			
E -G	0.05	1 T	0.00 0.05
G -H	0.04	0 T	0.00 0.04
H -J	0.04	0 T	0.00 0.04
J -D	0.05	1 T	0.00 0.05
Gable Webs			
G -F	0.02	145 C	
H -B	0.01	51 T	
J -I	0.02	145 C	

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in E -F 0.09

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

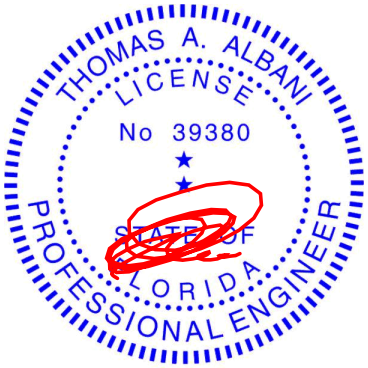
Jt	Type	Plt Size	X	Y	JSI
E	MT20	3.0x 5.0	Ctr	Ctr	0.45
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
I	MT20	2.0x 4.0	Ctr	Ctr	0.00
D	MT20	3.0x 5.0	Ctr	Ctr	0.45
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	2.0x 4.0	Ctr	Ctr	0.00
J	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 145 Lbs
Max tens. force 65 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.
See Standard Industry Piggy-Back Truss Connection Detail for Connection to base truss as applicable, or consult qualified Building Designer.

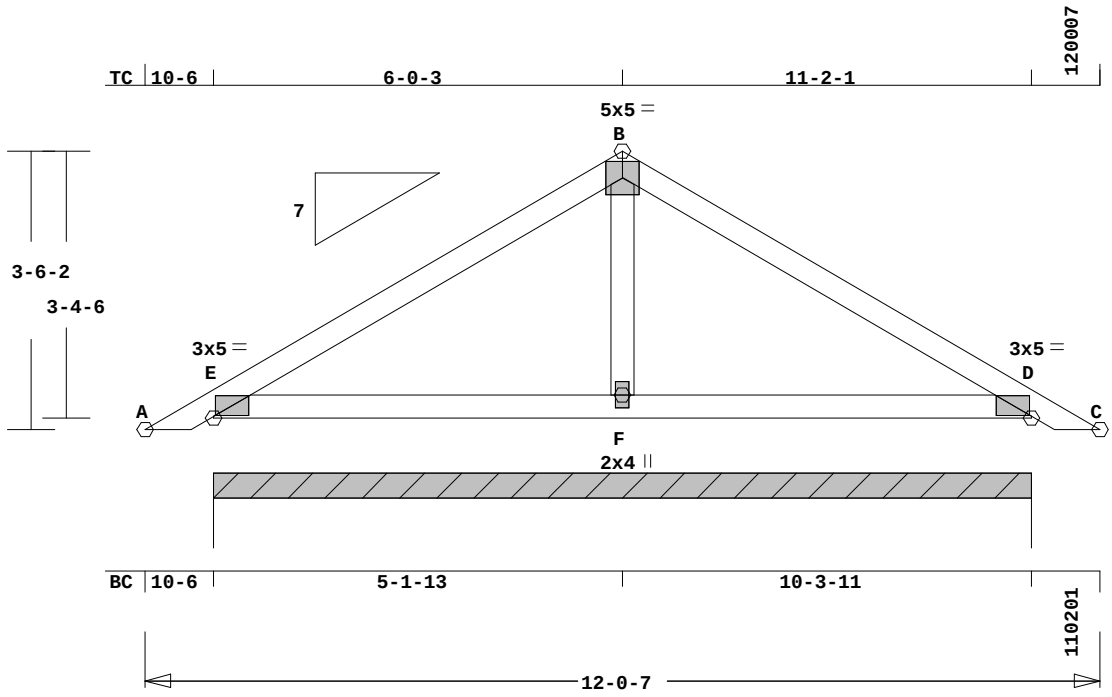


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	*T2*	14	PB	120007	7	0	0	*T20412086*

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.413" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 51.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 7
or	48.0"	0- 0- 0	12- 0- 7
BC	Cont.	0- 0- 0	12- 0- 7
or	120.0"	0- 0- 0	12- 0- 7

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	765	64 U	50 R

Jt	Brg Size	Required
E	123.7"	0"-to- 124"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
E -B	0.20	303 C	0.00 0.20
B -D	0.20	303 C	0.00 0.20
--------Bottom Chords---------			
E -F	0.17	2 T	0.00 0.17
F -D	0.17	2 T	0.00 0.17
------------Webs------------			
F -B	0.02	93 T	

CT Defl -0.03" in F -D L/999
LL Defl -0.01" in F -D L/999
Shear // Grain in E -B 0.14

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
E	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
D	MT20	3.0x 5.0	Ctr	Ctr	0.45
F	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 303 Lbs
Max tens. force 93 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.
See Standard Industry Piggy-Back Truss Connection Detail for Connection to base truss as applicable, or consult qualified Building Designer.

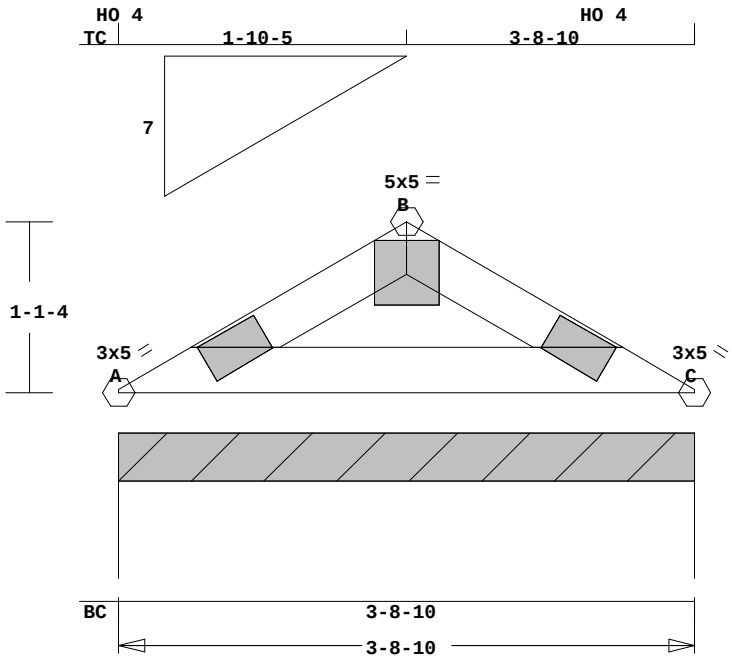
THOMAS A. ALBANI LICENSE No 39380 STATE OF FLORIDA PROFESSIONAL ENGINEER

Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V1	3	VL	30810	7	0	0	T20412087

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.806" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 13.1 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 8-10
or	44.7"	0- 0- 0	3- 8-10
BC	Cont.	0- 0- 0	3- 8-10
or	44.7"	0- 0- 0	3- 8-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	201	15 U	12 R

Jt	Brg Size	Required
A	44.6"	0"-to- 45"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	55 C	
B -C	0.00	55 C	
--------Bottom Chords---------			
A -C	0.01	0 T	0.00 0.01

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -C 0.04

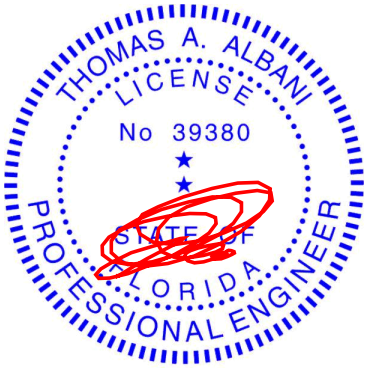
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	5.0x 5.0	Ctr	-1.9	0.33
C	MT20	3.0x 5.0	Ctr	Ctr	0.45

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

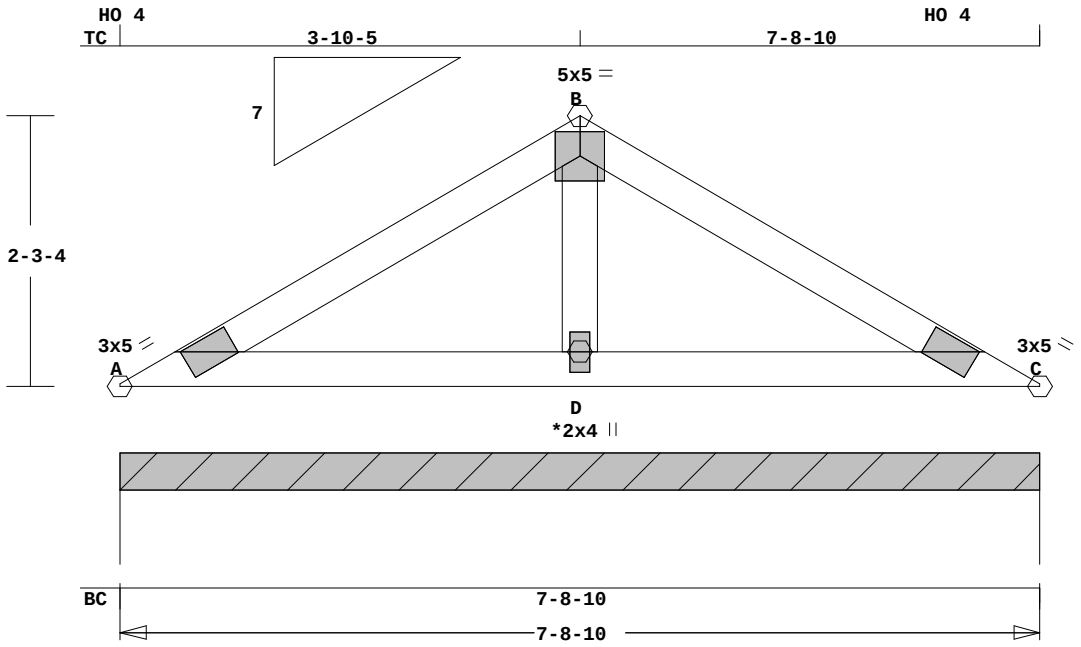
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 55 Lbs
Max tens. force 17 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V2	3	VL	70810	7	0	0	T20412088

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.620" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 32.6 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.06	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
GW	0.02	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 8-10
or	48.0"	0- 0- 0	7- 8-10
BC	Cont.	0- 0- 0	7- 8-10
or	92.7"	0- 0- 0	7- 8-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0

Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	497	38 U	33 R

Jt	Brg Size	Required
A	92.6"	0"-to- 93"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------				
A -B	0.06	51 C	0.00	0.06
B -C	0.06	51 C	0.00	0.06
--------Bottom Chords---------				
A -D	0.05	0 T	0.00	0.05
D -C	0.05	0 T	0.00	0.05
----------Gable Webs----------				
D -B	0.02	106 C		

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

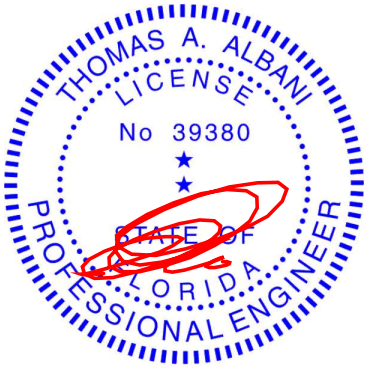
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
C	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10

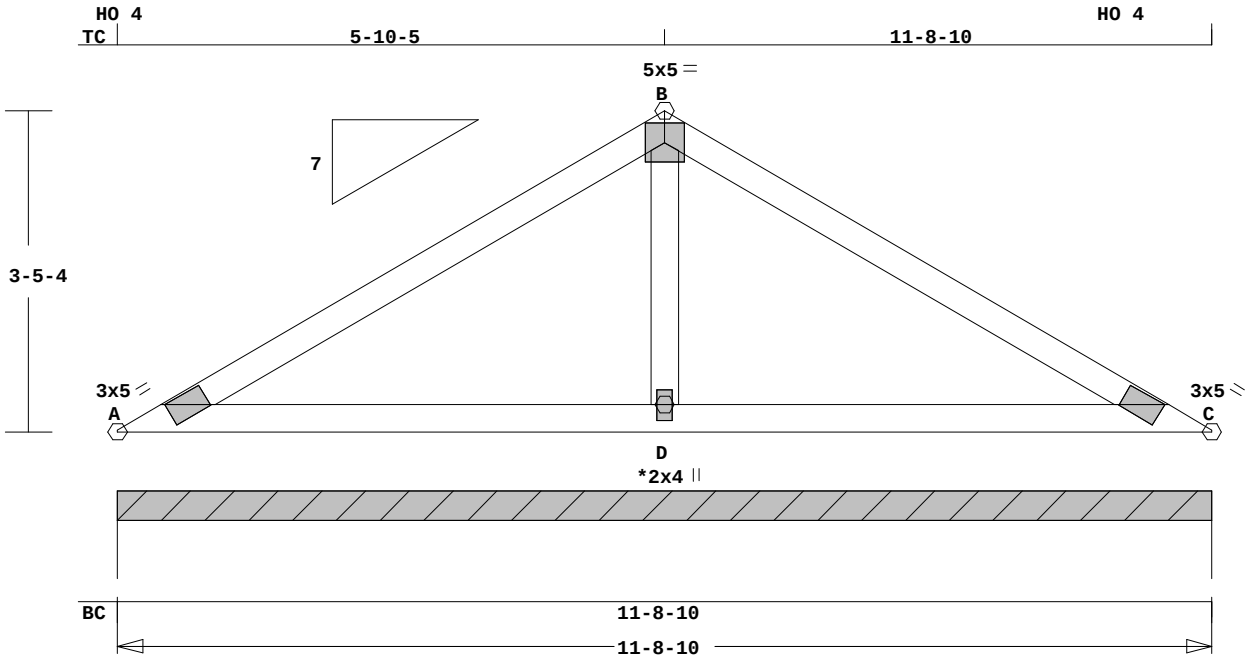
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 106 Lbs
Max tens. force 31 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V3	2	VL	110810	7	0	0	T20412089

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.486" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 51.1 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.15	2x 4	SP-#2
GW	0.04	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 8-10
or	48.0"	0- 0- 0	11- 8-10
BC	Cont.	0- 0- 0	11- 8-10
or	120.0"	0- 0- 0	11- 8-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	793	61 U	53 R

Jt	Brg Size	Required
A	140.6"	0"-to- 141"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.20	92 C	0.00 0.20
B -C	0.20	92 C	0.00 0.20
--------Bottom Chords---------			
A -D	0.15	0 T	0.00 0.15
D -C	0.15	0 T	0.00 0.15
----------Gable Webs----------			
D -B	0.04	170 C	

CT Defl -0.02" in D -C L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

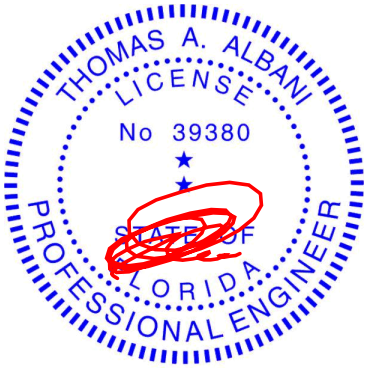
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
C	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10

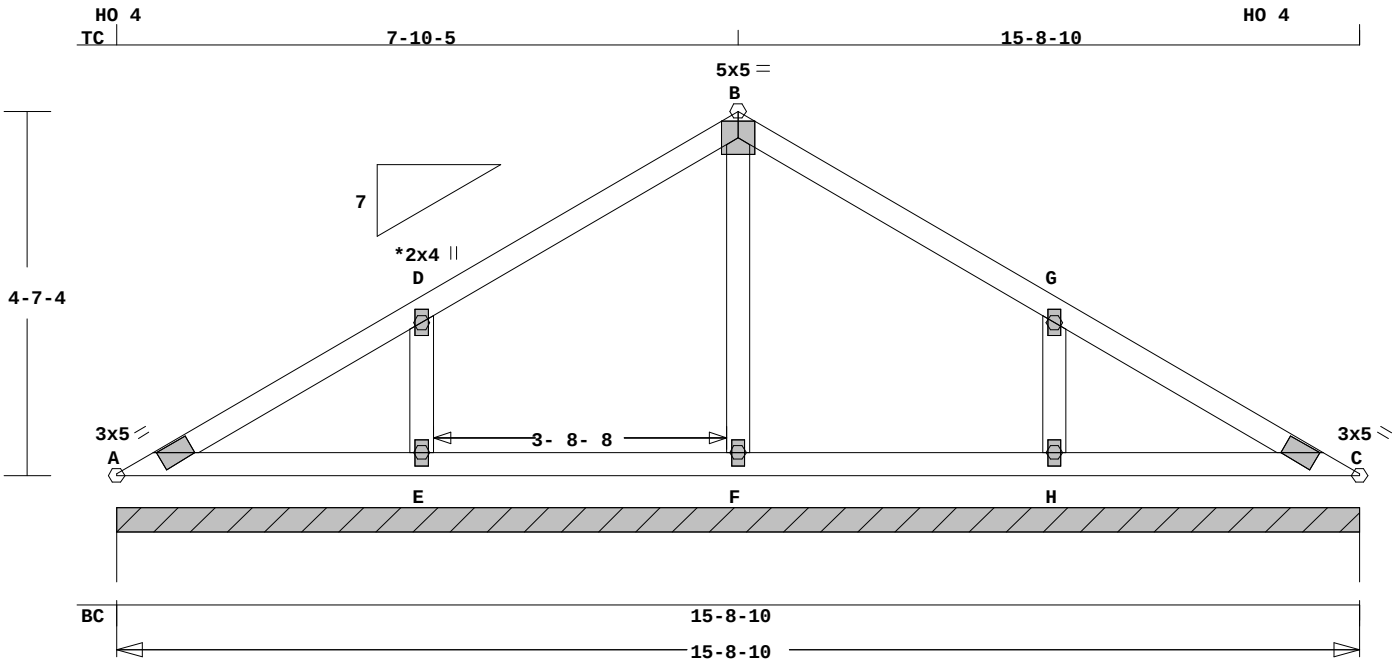
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 170 Lbs
Max tens. force 49 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V4	2	VL	150810	7	0	0	T20412090

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.412" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 76.1 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	Size	Lumber
TC	0.12	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
GW	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	15- 8-10
or	48.0"	0- 0- 0	15- 8-10
BC	Cont.	0- 0- 0	15- 8-10
or	120.0"	0- 0- 0	15- 8-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1089	84 U	73 R

Jt	Brg Size	Required
A	188.6"	0"-to- 189"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.12	66 C	0.00 0.12
D -B	0.12	62 C	0.00 0.12
B -G	0.12	62 C	0.00 0.12
G -C	0.12	66 C	0.00 0.12
Bottom Chords			
A -E	0.07	0 T	0.00 0.07
E -F	0.09	0 T	0.00 0.09
F -H	0.09	0 T	0.00 0.09
H -C	0.07	0 T	0.00 0.07
Gable Webs			
E -D	0.04	207 C	
F -B	0.06	148 C	
H -G	0.04	207 C	

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in D -B 0.12

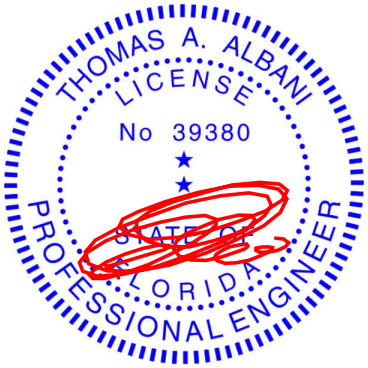
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 5.0	Ctr	Ctr	0.45
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 207 Lbs
Max tens. force 100 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

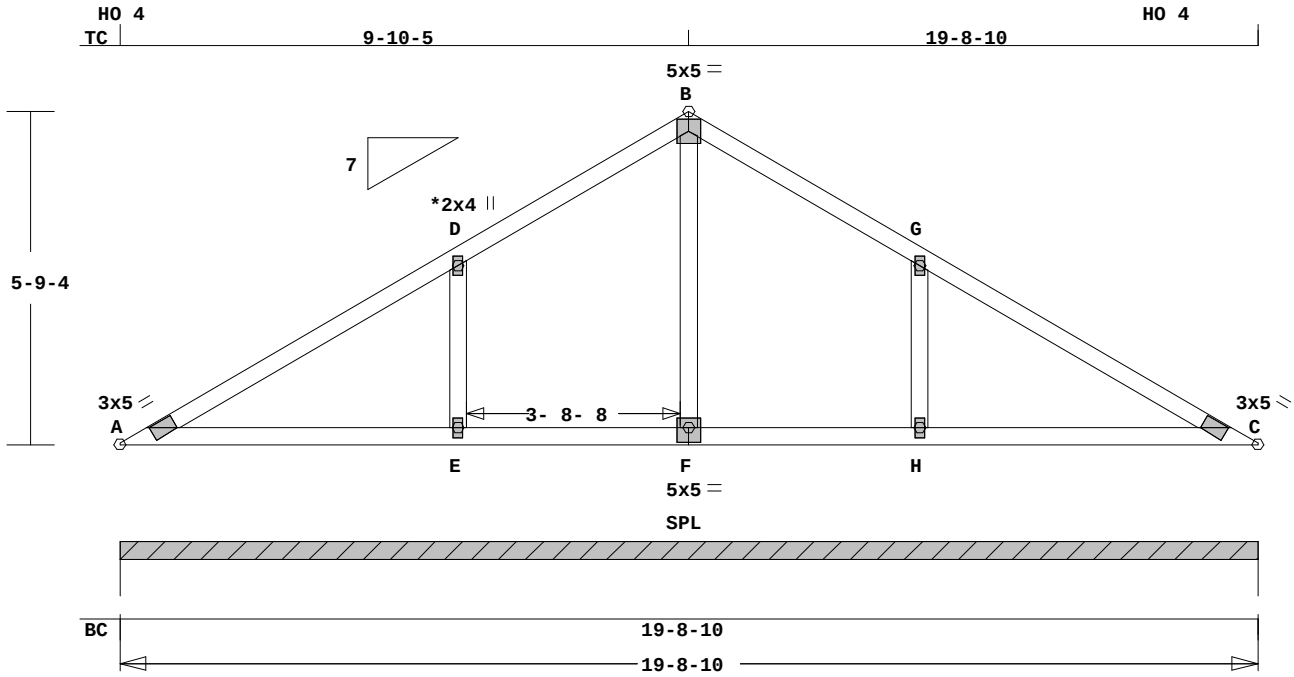


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V5	1	VL	190810	7	0	0	T20412091

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.300" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 99.0 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.19	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
GW	0.09	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	19- 8-10
or	48.0"	0- 0- 0	19- 8-10
BC	Cont.	0- 0- 0	19- 8-10
or	120.0"	0- 0- 0	19- 8-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1385	107 U	93 R

Jt	Brg Size	Required
A	236.6"	0"-to- 237"

Plus 15 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.19	66 C	0.00 0.19
D -B	0.19	58 C	0.00 0.19
B -G	0.19	58 C	0.00 0.19
G -C	0.19	66 C	0.00 0.19
--------Bottom Chords---------			
A -E	0.12	0 T	0.00 0.12
E -F	0.12	0 T	0.00 0.12
F -H	0.12	0 T	0.00 0.12
H -C	0.12	0 T	0.00 0.12
----------Gable Webs----------			
E -D	0.06	260 C	
F -B	0.09	134 C	
H -G	0.06	260 C	

CT Defl -0.01" in A -E L/999
LL Defl -0.01" in A -E L/999
Shear // Grain in A -D 0.15

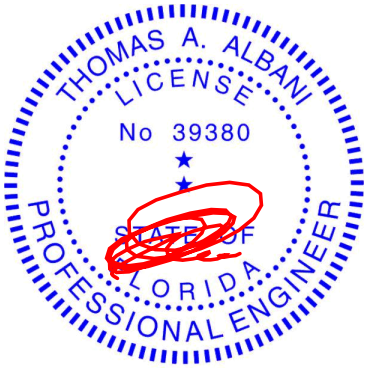
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	5.0x 5.0	Ctr	Ctr	0.34
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	3.0x 5.0	Ctr	Ctr	0.45
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
F	MT20	5.0x 5.0	Ctr	-0.5	0.39
H	MT20	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 260 Lbs
Max tens. force 126 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

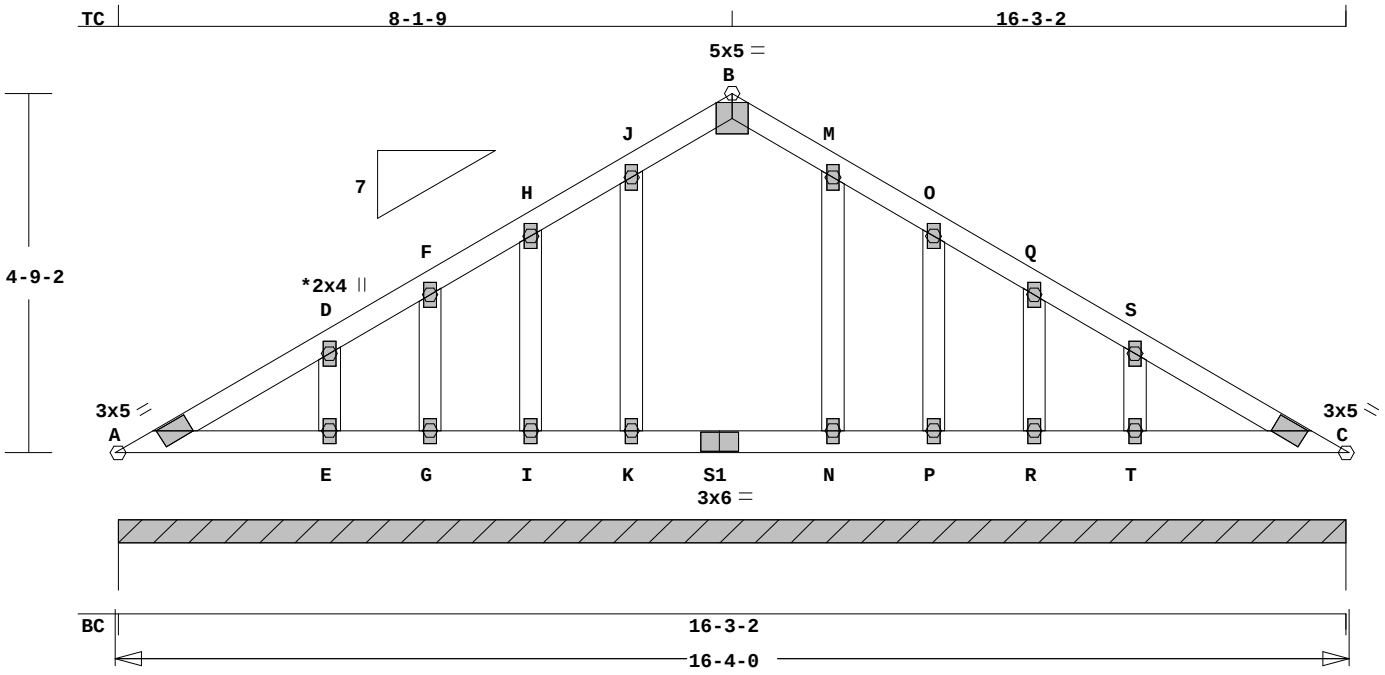


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V6	1	VL	160400	7	0	0	T20412092

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.393" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 98.8 LBS

```
Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON
     CSI -Size- ----Lumber----
TC  0.03  2x 4  SP-#2
BC  0.03  2x 4  SP-#2
GW  0.02  2x 4  SP-#3

Brace truss as follows:
      O.C.      From       To
 TC  Cont.    0- 0- 0 16- 4- 0
 or   48.0"   0- 0- 0 16- 4- 0
 BC  Cont.    0- 0- 0 16- 4- 0
 or   72.0"   0- 0- 0 16- 4- 0

psf-Ld  Dead  Live
TC       7.0  20.0
BC      10.0   0.0
TC+BC   17.0  20.0
Total   37.0   Spacing  24.0"
Lumber Duration Factor  1.25
Plate  Duration Factor  1.25
        Fb    Fc    Ft   Emin
TC     1.15  1.10  1.10  1.10
BC     1.10  1.10  1.10  1.10

Total Load Reactions (Lbs)
Jt  Down  Uplift  Horiz-
A   1129    87 U    76 R

Jt   Brg Size   Required
A     195.1"     0"-to- 196"

Plus 15 Wind Load Case(s)
Plus  1 LL Load Case(s)
Plus  1 DL Load Case(s)

Membr  CSI  P Lbs  Axl-CSI-Bnd
----------Top Chords----------
A -D  0.03    73 C  0.00  0.03
D -F  0.03    71 C  0.00  0.03
F -H  0.01    62 C  0.00  0.01
H -J  0.02    58 C  0.00  0.02
J -B  0.02    64 C  0.00  0.02
B -M  0.02    64 C  0.00  0.02
M -O  0.02    58 C  0.00  0.02
O -Q  0.01    62 C  0.00  0.01
Q -S  0.03    71 C  0.00  0.03
S -C  0.03    73 C  0.00  0.03
--------Bottom Chords---------
A -E  0.03     1 C  0.00  0.03
E -G  0.02     0 T  0.00  0.02
G -I  0.01     0 T  0.00  0.01
I -K  0.01     0 T  0.00  0.01
K -S1 0.01     0 T  0.00  0.01
S1-N  0.01     0 T  0.00  0.01
N -P  0.01     0 T  0.00  0.01
P -R  0.00     0 T
R -T  0.02     0 T  0.00  0.02
T -C  0.03     1 C  0.00  0.03
----------Gable Webs----------
E -D  0.02   110 C
G -F  0.01    61 C
I -H  0.01    70 C
K -J  0.02    80 C
N -M  0.02    80 C
P -O  0.01    70 C
R -Q  0.01    61 C
T -S  0.02   110 C

CT Defl   0.00" in ----- L/999
LL Defl   0.00" in ----- L/999
Shear // Grain  in A -D   0.07

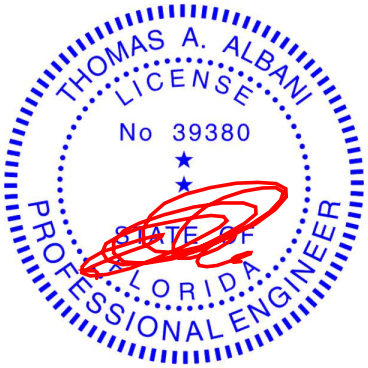
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area
Jt Type  Plt Size  X   Y   JSI
A  MT20  3.0x 5.0 Ctr Ctr 0.45
D  MT20  2.0x 4.0 Ctr Ctr 0.00
F  MT20  2.0x 4.0 Ctr Ctr 0.00
H  MT20  2.0x 4.0 Ctr Ctr 0.00
J  MT20  2.0x 4.0 Ctr Ctr 0.00
B  MT20  5.0x 5.0 Ctr-1.9 0.33
M  MT20  2.0x 4.0 Ctr Ctr 0.00
O  MT20  2.0x 4.0 Ctr Ctr 0.00
Q  MT20  2.0x 4.0 Ctr Ctr 0.00
S  MT20  2.0x 4.0 Ctr Ctr 0.00
C  MT20  3.0x 5.0 Ctr Ctr 0.45
E  MT20  2.0x 4.0 Ctr Ctr 0.00
G  MT20  2.0x 4.0 Ctr Ctr 0.00
I  MT20  2.0x 4.0 Ctr Ctr 0.00
K  MT20  2.0x 4.0 Ctr Ctr 0.00
S1 MT20  3.0x 6.0 Ctr Ctr 0.39
N  MT20  2.0x 4.0 Ctr Ctr 0.00
P  MT20  2.0x 4.0 Ctr Ctr 0.00
R  MT20  2.0x 4.0 Ctr Ctr 0.00
T  MT20  2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:
MiTek USA, Inc.
 6904 Parke East Blvd.
 Tampa, FL  33610

REFER TO ONLINE PLUS GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
   RIDGWAY ROOF TRUSS
Analysis Conforms To:
  FBC2017
  TPI 2014

Design checked for 10 psf non-
  concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main
 Wind-Force Resistance System
 - Directional.
  Wind Speed:          130 mph
  Risk Category     :   II
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       4.0 psf
  BC Dead Load :       6.0 psf
Max comp. force      110 Lbs
Max tens. force       48 Lbs
Connector Plate Fabrication
  Tolerance = 20%
This truss is designed for a
  creep factor of 2.0  which
  is used to calculate
  Vert(CT) deflection per
  ANSI/TPI 1.
```

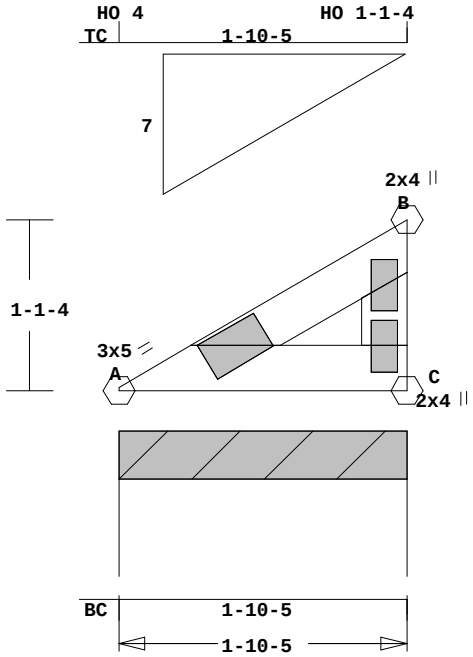


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V7	1	VLM	11005	7	0	0	T20412093

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.806" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 7.5 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.00	2x 4	SP-#2
WB	0.00	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	1-10- 5
or	22.3"	0- 0- 0	1-10- 5
BC	Cont.	0- 0- 0	1-10- 5
or	22.3"	0- 0- 0	1-10- 5

psf-Ld	Dead	Live	
TC	7.0	20.0	
BC	10.0	0.0	
TC+BC	17.0	20.0	
Total	37.0	Spacing	24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	100	8 U	21 R

Jt	Brg Size	Required
A	22.3"	0"-to- 22"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -B	0.00	9	C

--------Bottom Chords---------

A -C	0.00	0	T

------------Webs------------

C -B	0.00	28	C WindLd

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.02

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

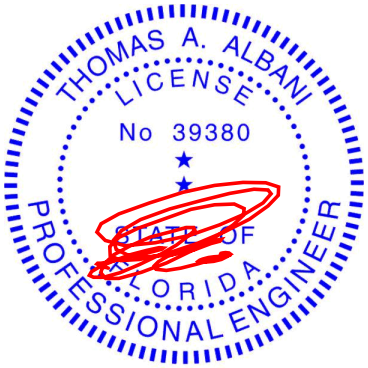
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	2.0x 4.0	Ctr	Ctr	0.12
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 28 Lbs
Max tens. force 11 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

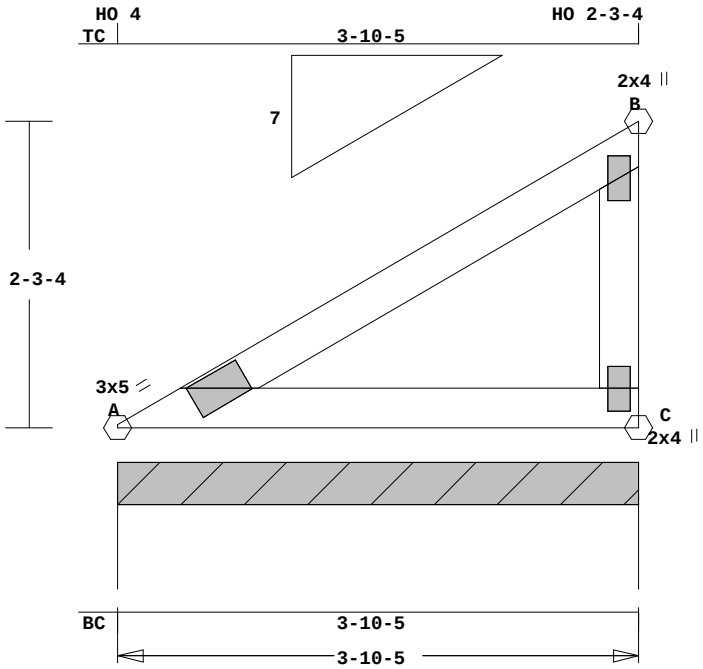


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V8	1	VLM	31005	7	0	0	T20412094

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.703" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 17.8 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.04	2x 4	SP-#2
WB	0.01	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3-10- 5
or	46.3"	0- 0- 0	3-10- 5
BC	Cont.	0- 0- 0	3-10- 5
or	46.3"	0- 0- 0	3-10- 5

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	248	19 U	56 R

Jt	Brg Size	Required
A	46.3"	0"-to- 46"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -B	0.07	28 C	0.00 0.07

--------Bottom Chords---------

A -C	0.04	0 T	0.00 0.04

------------Webs-------------

C -B	0.01	71 C WindLd

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.10

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

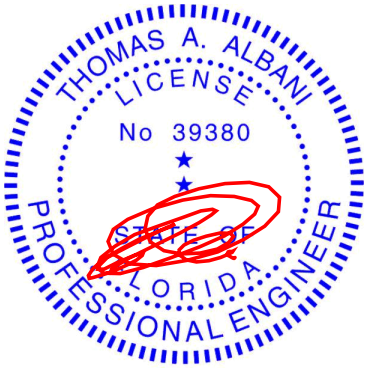
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	2.0x 4.0	Ctr	Ctr	0.13
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 71 Lbs
Max tens. force 25 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

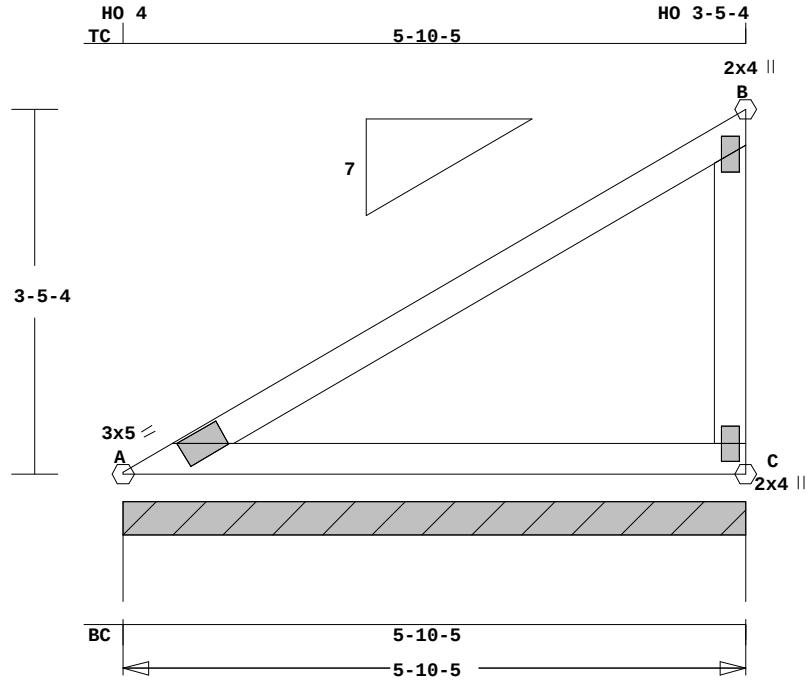


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V9	1	VLM	51005	7	0	0	T20412095

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.553" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 28.2 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.25	2x 4	SP-#2
BC	0.15	2x 4	SP-#2
WB	0.05	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5-10- 5
or	48.0"	0- 0- 0	5-10- 5
BC	Cont.	0- 0- 0	5-10- 5
or	70.3"	0- 0- 0	5-10- 5

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	396	30 U	91 R

Jt	Brg Size	Required
A	70.3"	0"-to- 70"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
A -B	0.25	47 C	0.00 0.25
A -C	0.15	0 T	0.00 0.15
C -B	0.05	113 C	WindLd

Top Chords
Bottom Chords
Webs

CT Defl -0.03" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.17

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

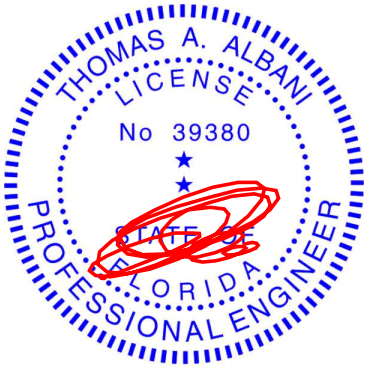
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	2.0x 4.0	Ctr	Ctr	0.13
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

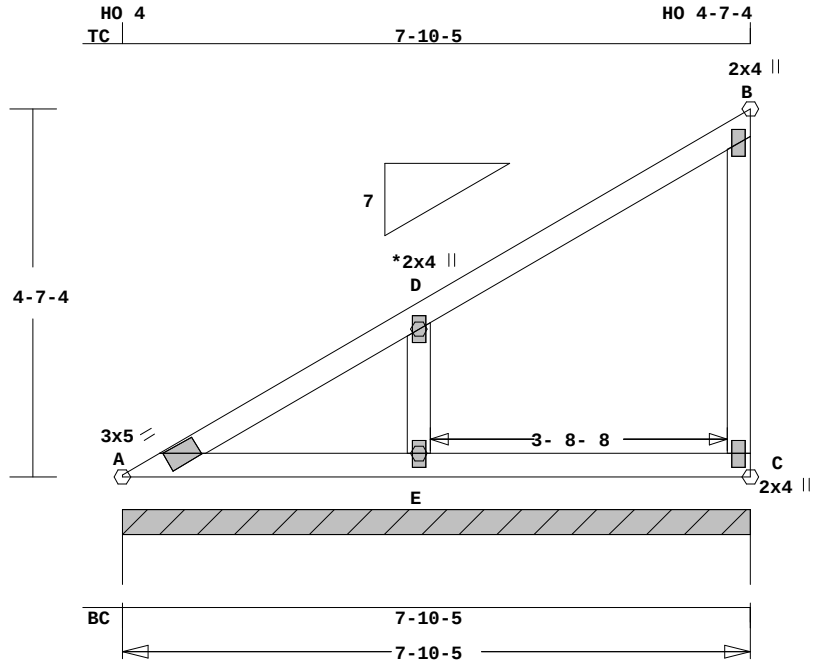
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 113 Lbs
Max tens. force 41 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V10	1	VLM	71005	7	0	0	T20412096

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.416" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 41.6 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.14	2x 4	SP-#2
BC	0.09	2x 4	SP-#2
WB	0.20	2x 4	SP-#3
GW	0.10	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7-10- 5
or	48.0"	0- 0- 0	7-10- 5
BC	Cont.	0- 0- 0	7-10- 5
or	94.3"	0- 0- 0	7-10- 5

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	544	42 U	126 R

Jt	Brg Size	Required
A	94.3"	0"-to- 94"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -D	0.10	94 C	0.00 0.10
D -B	0.14	52 C	0.00 0.14
--------Bottom Chords---------			
A -E	0.06	0 T	0.00 0.06
E -C	0.09	0 T	0.00 0.09
------------Webs------------			
C -B	0.20	99 C	0.00 0.20
----------Gable Webs----------			
E -D	0.10	215 C	0.00 0.10

CT Defl -0.01" in E -C L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in D -B 0.13

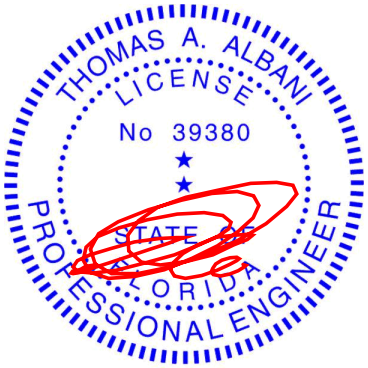
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	2.0x 4.0	Ctr	Ctr	0.56
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	2.0x 4.0	Ctr	Ctr	0.55

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 215 Lbs
Max tens. force 95 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

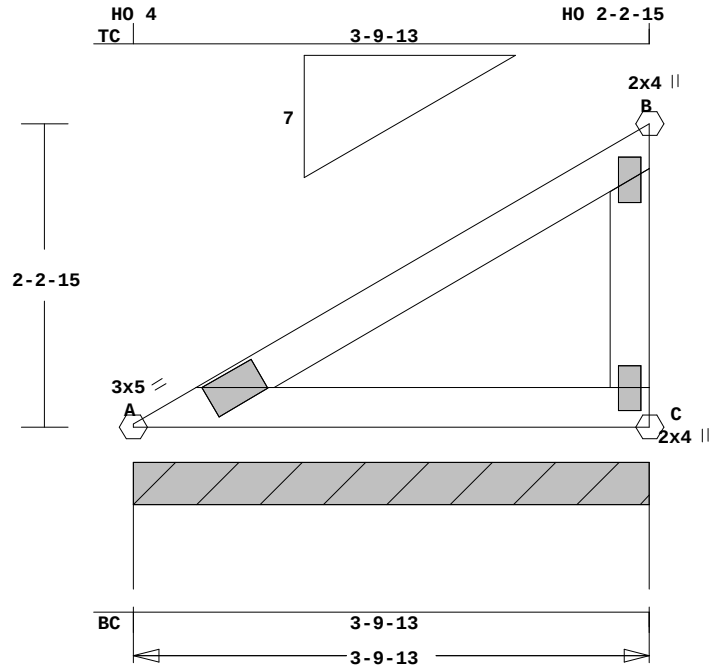


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V11	1	VLM	30913	7	0	0	T20412097

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.704" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 17.6 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.04	2x 4	SP-#2
WB	0.01	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 9-13
or	45.8"	0- 0- 0	3- 9-13
BC	Cont.	0- 0- 0	3- 9-13
or	45.8"	0- 0- 0	3- 9-13

psf-Ld	Dead	Live	
TC	7.0	20.0	
BC	10.0	0.0	
TC+BC	17.0	20.0	
Total	37.0	Spacing	24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	245	19 U	55 R

Jt	Brg Size	Required
A	45.8"	0"-to- 46"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -B	0.07	28 C	0.00 0.07

--------Bottom Chords---------

A -C	0.04	0 T	0.00 0.04

------------Webs-------------

C -B 0.01 70 C WindLd

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

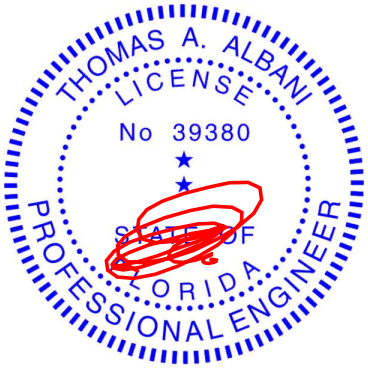
Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
B	MT20	2.0x 4.0	Ctr	Ctr	0.13
C	MT20	2.0x 4.0	Ctr	Ctr	0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 70 Lbs
Max tens. force 25 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.

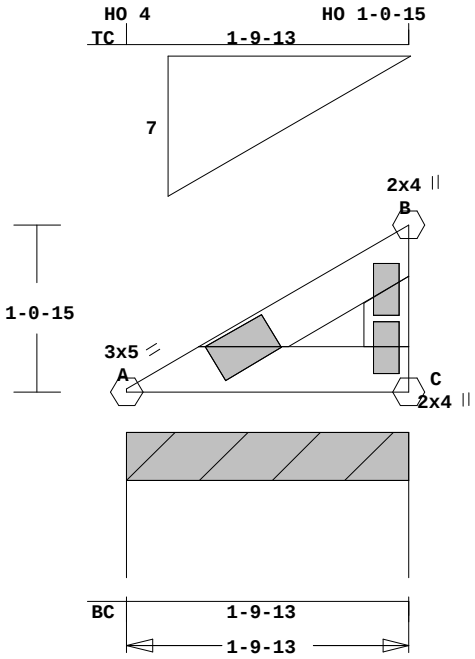


Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V12	1	VLM	10913	7	0	0	T20412098

Adams Residence



ALL PLATES ARE MT2020

Scale: 0.808" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 7.3 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.00	2x 4	SP-#2
WB	0.00	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	1- 9-13
or	21.8"	0- 0- 0	1- 9-13
BC	Cont.	0- 0- 0	1- 9-13
or	21.8"	0- 0- 0	1- 9-13

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	97	7 U	20 R

Jt	Brg Size	Required
A	21.8"	0"-to- 22"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd

----------Top Chords----------

A -B	0.00	9	C

--------Bottom Chords---------

A -C	0.00	0	T

------------Webs-------------

C -B	0.00	27	C	WindLd

CT Defl 0.00" in ----- L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -B 0.02

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

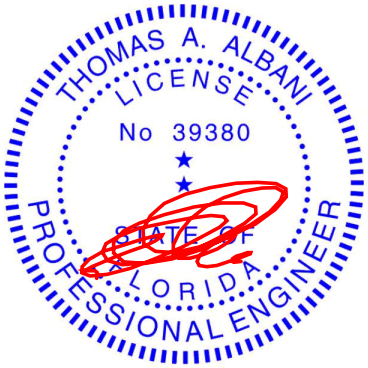
Jt Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr 0.45
B	MT20	2.0x 4.0	Ctr	Ctr 0.12
C	MT20	2.0x 4.0	Ctr	Ctr 0.12

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.

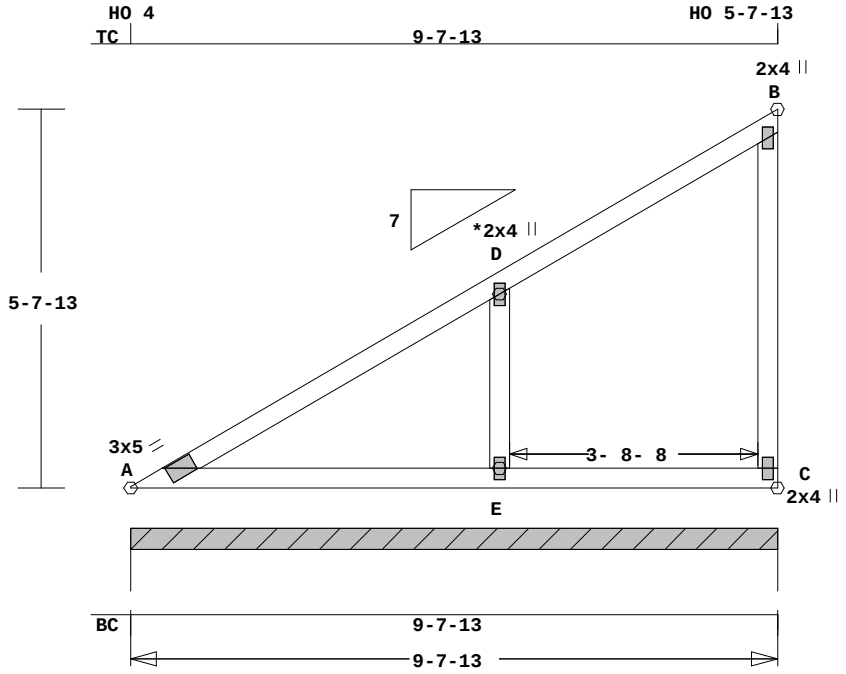
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 27 Lbs
Max tens. force 11 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: June 8,2020

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
200342	V13	1	VLM	90713	7	0	0	T20412099

Adams Residence



ALL PLATES ARE MT2020

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.349" = 1'

MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 52.9 LBS

Online Plus -- Version 30.0.071
RUN DATE: 08-JUN-20

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

	CSI	-Size-	----Lumber----
TC	0.19	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.17	2x 4	SP-#3
GW	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 7-13
or	48.0"	0- 0- 0	9- 7-13
BC	Cont.	0- 0- 0	9- 7-13
or	115.8"	0- 0- 0	9- 7-13

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor		1.25
Plate Duration Factor		1.25

	Fb	Fc	Ft	Emin
TC	1.15	1.10	1.10	1.10
BC	1.10	1.10	1.10	1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	677	52 U	158 R

Jt	Brg Size	Required
A	115.8"	0"-to- 116"

Plus 12 Wind Load Case(s)
Plus 1 LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -D	0.19	113 C	0.00 0.19
D -B	0.17	57 C	0.00 0.17
Bottom Chords			
A -E	0.12	0 T	0.00 0.12
E -C	0.11	0 T	0.00 0.11
Webs			
C -B	0.17	97 C	0.00 0.17
Gable Webs			
E -D	0.06	249 C	0.03 0.03

CT Defl -0.01" in A -E L/999
LL Defl 0.00" in ----- L/999
Shear // Grain in A -D 0.14

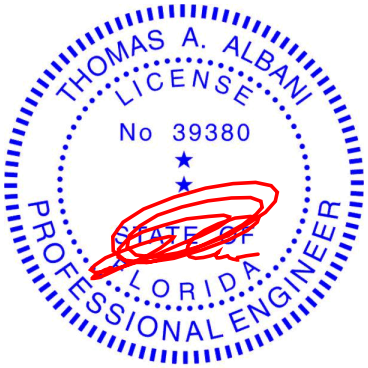
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - MS18 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x 5.0	Ctr	Ctr	0.45
D	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	2.0x 4.0	Ctr	Ctr	0.60
E	MT20	2.0x 4.0	Ctr	Ctr	0.00
C	MT20	2.0x 4.0	Ctr	Ctr	0.74

REVIEWED BY:
MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2017
TPI 2014
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-10
Truss is designed as a Main Wind-Force Resistance System - Directional.
Wind Speed: 130 mph
Risk Category : II
Mean Roof Height: 15-0
Exposure Category: B
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 4.0 psf
BC Dead Load : 6.0 psf
Max comp. force 249 Lbs
Max tens. force 111 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 2.0 which is used to calculate Vert(CT) deflection per ANSI/TPI 1.



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Date:

June 8,2020

ONLINE PLUS GENERAL NOTES & SYMBOLS

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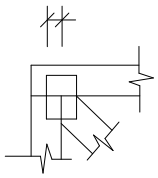
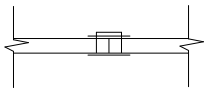


PLATE LOCATION

Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

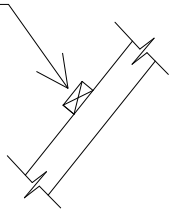
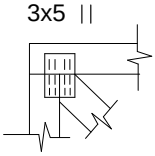


PLATE SIZE AND ORIENTATION

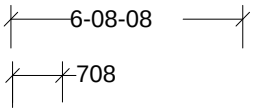
3x5 ||

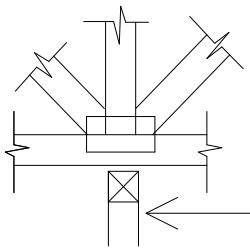


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6'-8.5" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before trusses are installed. If necessary, shim bearings to assure solid contact with truss.

Metal connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on Truss Design Drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Mitek Industries Inc. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to "Building Component Safety Information" (BCSI 1) as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records. When truss hangers are specified on the Truss Design Drawing, they must be installed per manufacturer's details and specifications.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS MANUFACTURER.

MiTek Industries, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

Tel: 813-972-1135
Fax: 813-971-6117

JOBSITE PACKAGE

IMPORTANT DOCUMENTS ENCLOSED

PLEASE REVIEW

WARNING: The handling, storing, installing, restraining and diagonal bracing of structural building components requires specialized training, clearly implemented procedures, and careful planning and communication among the contractor, crane operator and installation crew. Handling and installing components without appropriate training, planning and communication greatly increases the probability of an accident resulting in property damage, serious personal injury and/or death.

Prior to component installation, the documents should be examined and disseminated to all appropriate personnel, in addition to proper training and a clear understanding of the installation plan, any applicable fall protection requirements, and the intended restraint and bracing requirements. **Trusses over 60 feet in length are very dangerous to install and may require complex temporary and permanent bracing. Please consult a Registered Design Professional.**

Examine the building, the building's structural framing system, bearing locations and related installation conditions. Begin component installation only after any unsatisfactory conditions have been corrected. Do not cut, modify or repair components. Report any damage before installation.

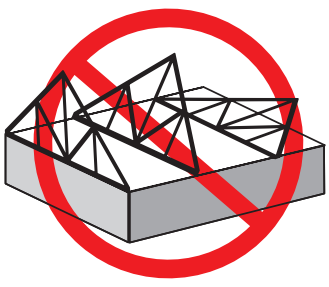
The enclosed documents are offered as minimum guidelines only. Nothing contained in this jobsite package should be construed in any manner as expanding the scope of responsibility of, or imposing any additional liabilities on, the component manufacturer.

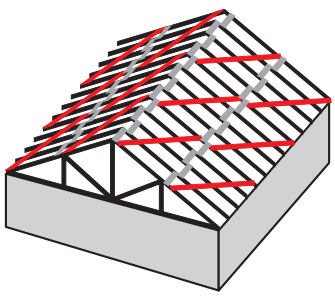
ADVERTENCIA: El manejo, almacenamiento, instalación, restricción y arriostre diagonal de componentes estructurales de construcción requieren entrenamiento especializada, procedimientos claramente implementados y planificaron y comunicación clara entre el contratista, operador de grúa, y los obreros de instalación. El manejar e instalar los componentes sin entrenamiento suficiente, planificación y comunicación adecuadas aumenta la probabilidad de un accidente que resulta en dareo a propiedad, herida seria o muerte.

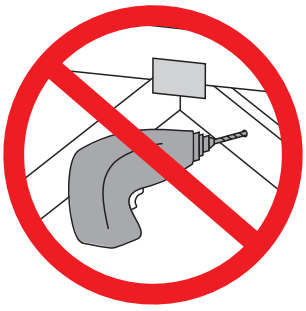
Antes de la instalación de componentes, los documentos adjuntos deben ser examinados y difundidos a todo el personal apropiado, además del entrenamiento pertinente y un claro entendimiento del plan de instalación, de todo requisito aplicable de la protección contra la caída y de los requisitos previstos de arriostre y restricción. **La instalación de trusses más de 60 pies de largo es muy peligrosa. Consulte a un Profesional de Diseño Registrado.**

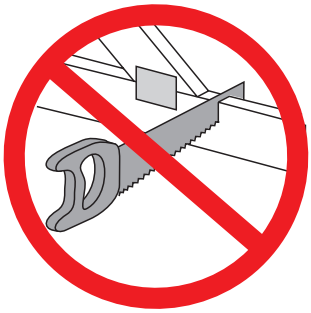
Examine la estructura, el sistema armazón estructural de edificio, ubicaciones de soporte e las condiciones de instalación correspondientes y comenzar con la instalación de los componentes sólo después de haber corregido toda condición insatisfactoria. No corte, modifique ni repare los componentes y informe cualquier daño descubierto antes de proceder a la instalación.

Los documentos adjuntos se ofrecen solamente como directrices mínimas. Nada de lo incluido en este paquete debe interpretarse de manera que exceda el alcance de la responsabilidad del fabricante de componentes, ni en forma tal que imponga responsabilidades adicionales sobre éste.









To view a non-printing PDF of this document, visit www.sbcindustry.com/jobsitecs.

FOR DESIGN RESPONSIBILITIES SEE REVERSE

BCSI-B1 SUMMARY SHEET GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES

SPANS OVER 60' MAY REQUIRE COMPLEX PERMANENT BRACING. PLEASE ALWAYS CONSULT A PROFESSIONAL ENGINEER

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses* for more detailed information.**

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet – Permanent Restraint/Bracing of Chords & Web Members* for more information. All other permanent bracing design is the responsibility of the Building Designer.**

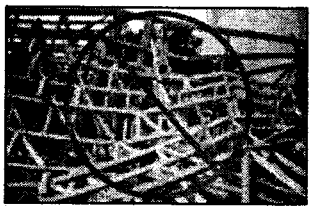
NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal*** para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 – Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios*** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

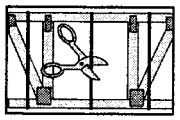
⚠ The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.



⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Lleve guantes y lentes protectores cuando corte los empaques.

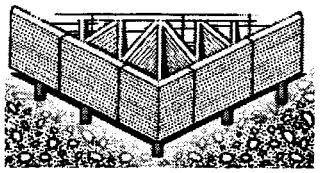


HANDLING — MANEJO

⚠ Avoid lateral bending. — Evite la flexión lateral.

⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



☑ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information pertaining to handling and jobsite storage of trusses.

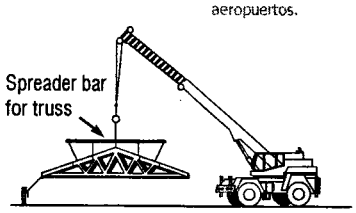
Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detras de la pila de los trusses a 8 hasta 10 pies en el centro.

Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal*** para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

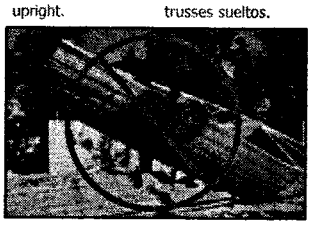


☑ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.

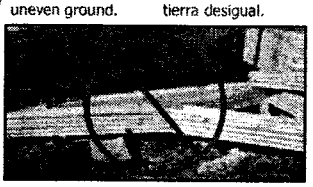
⊘ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



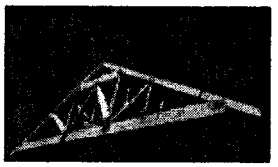
⊘ Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES
RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

⚠ Warning! Don't overload the crane.
¡Advertencia! ¡No sobrecargue la grúa!



⊘ Never use banding alone to lift a bundle.
Do not lift a group of individually banded bundles.

Nunca use sólo los empaques para levantar un paquete.
No levante un grupo de paquetes empaqueandos individualmentes.

☑ A single lift point may be used for bundles with trusses up to 45'.
Two lift points may be used for bundles with trusses up to 60'.
Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes hasta 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

⚠ Warning! Do not over load supporting structure with truss bundle.

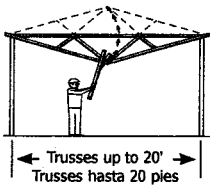
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

☑ Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND
INSTALACIÓN DE TRUSSES INDIVIDUALES POR LA MANO

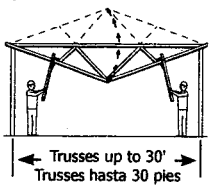
☑ Trusses 20' or less, support at peak.

Soporte del pico los trusses de 20 pies o menos.



☑ Trusses 30' or less, support at quarter points.

Soporte de los cuartos de tramo los trusses de 30 pies o menos.



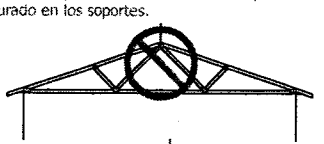
HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

☑ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

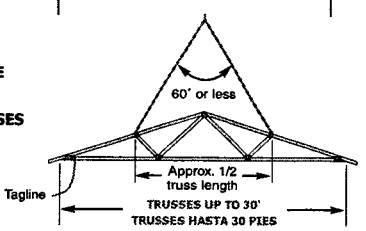
Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

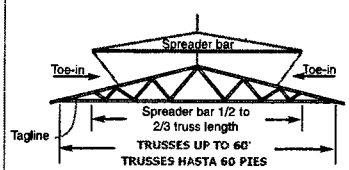
⚠ Warning! Using a single pick-point at the peak can damage the truss.

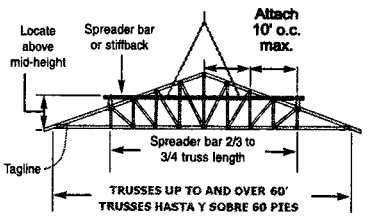
¡Advertencia! El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES
RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



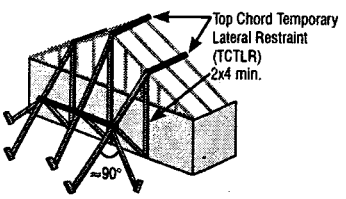




TEMPORARY RESTRAINT & BRACING
RESTRICCIÓN Y ARRIOSTRE TEMPORAL

⚠ Refer to BCSI-B2 Summary Sheet – Truss Installation & Temporary Restraint/Bracing*** for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Restricción/Arriostre Temporal*** para más información.

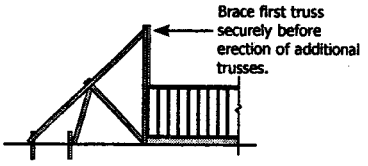


☑ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).

⊘ Do not walk on unbraced trusses.

No camine en trusses sueltos.



HOJA RESUMEN DE LA GUIA DE BUENA PRACTICA PARA EL MANEJO, INSTALACION, RESTICCION Y ARRIOSTRE DE LOS TRUSSES

VANOS MAS DE 60' PIES PUEDEN REQUERIR ARRIOSTRE PERMANENTE COMPLEJO. POR FAVOR, SIEMPRE CONSULTE A UN INGENIERO PROFESSIONAL

BCSI-B1 SUMMARY SHEET GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES

SPANS OVER 60' MAY REQUIRE COMPLEX PERMANENT BRACING. PLEASE ALWAYS CONSULT A PROFESSIONAL ENGINEER

STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

☑ 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estabilice los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing*** for more information.

Vea el resùmen BCSI-B2 - Instalación de Trusses y Restricción/Arriostre Temporal*** para más información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

☑ This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

1) TOP CHORD — CUERDA SUPERIOR

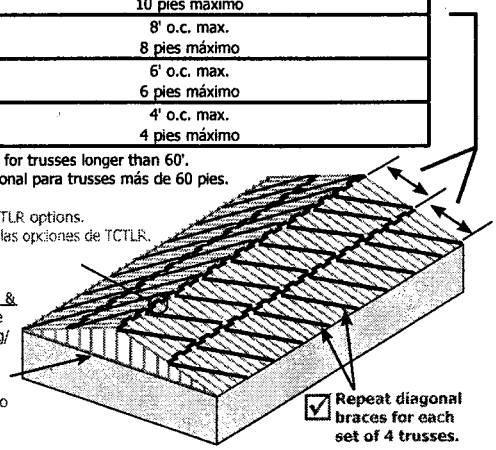
Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCTLR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un Ingeniero Profesional para trusses más de 60 pies.

☑ See BCSI-B2*** for TCTLR options.
Vea el BCSI-B2*** para las opciones de TCTLR.

⚠ Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members*** for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.***



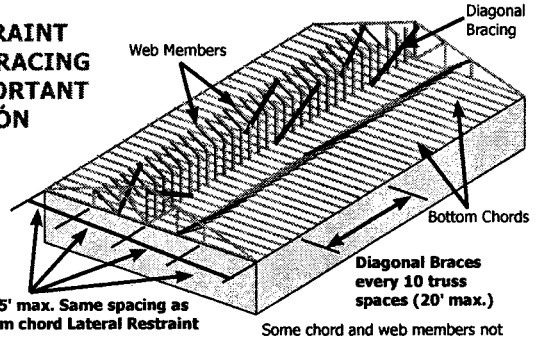
☑ Repeat diagonal braces for each set of 4 trusses.

Repita los arrisotres diagonales para cada grupo de 4 trusses.

Ground bracing not shown for clarity.

2) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

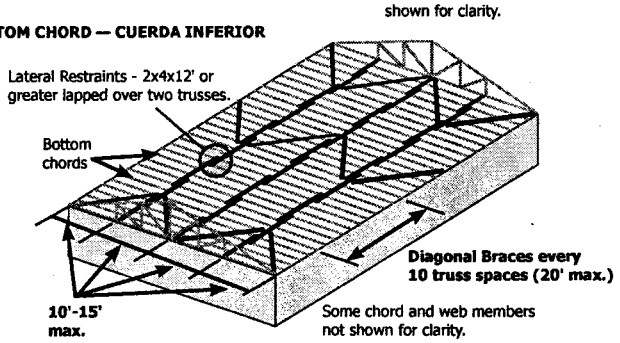
⚠ **LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT ¡LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!**



⚠ 10'-15' max. Same spacing as bottom chord Lateral Restraint

Some chord and web members not shown for clarity.

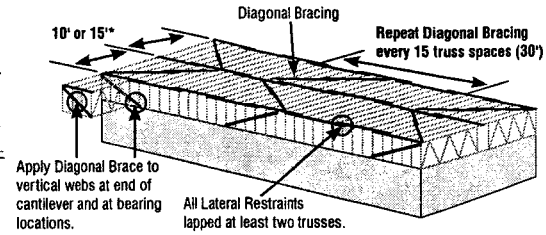
3) BOTTOM CHORD — CUERDA INFERIOR



RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

⚠ Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses*** for more information.

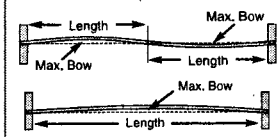
Vea el resumen BCSI-B7 – Restricción/Arriostre Temporal y Permanente para Trusses de Cuerdas Paralelas*** para más información.



*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACIÓN

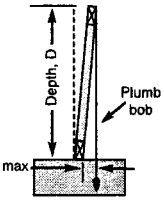
☑ Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



Out of Plane	
Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

Out of Plumb	
D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

☑ Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



CONSTRUCTION LOADING — CARGA DE CONSTRUCCIÓN

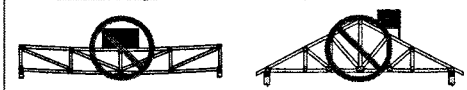
⚠ Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

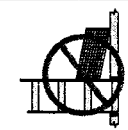
No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

Maximum Stack Height for Material on Trusses	
Material	Height
Gypsum Board	12"
Plywood or OSB	16"
Asphalt Shingles	2 bundles
Concrete Block	8"
Clay Tile	3-4 tiles high

⊘ Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading*** for more information.

No exceda las alturas máximas de montón. Vea el resumen BCSI-B4 Carga de Construcción*** para más información.





⊘ Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.

⊘ Never stack materials near a peak.
Nunca amontone los materiales cerca de un pico.

☑ Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

☑ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

Truss bracing not shown for clarity.

ALTERATIONS — ALTERACIONES

⚠ Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.***

Vea el resumen BCSI-B5 Daños de Trusses, Modificaciones en la Obra y Errores de Instalación.***

⊘ Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningún miembro estructural de un truss, a menos que esté específicamente permitido en el Dibujo del Diseño del Truss.

⚠ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

***Contact the Component Manufacturer for more information or consult a Professional Engineer for assistance.
To view a non-printing PDF of this document, visit www.sbcindustry.com/b1.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

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HOJA RESUMEN DE LA GUIA DE BUENA PRACTICA PARA EL MANEJO, INSTALACION, RESTICCION Y ARRIOSTRE DE LOS TRUSSES

VANOS MAS DE 60' PIES PUEDEN REQUERIR ARRIOSTRE PERMANENTE COMPLEJO. POR FAVOR, SIÉMPRE CONSULTE A UN INGENIERO PROFESSIONAL

B2 Truss Installation & Temporary Restraint/Bracing
Instalación de Trusses & Restricción/Arriostre Temporal

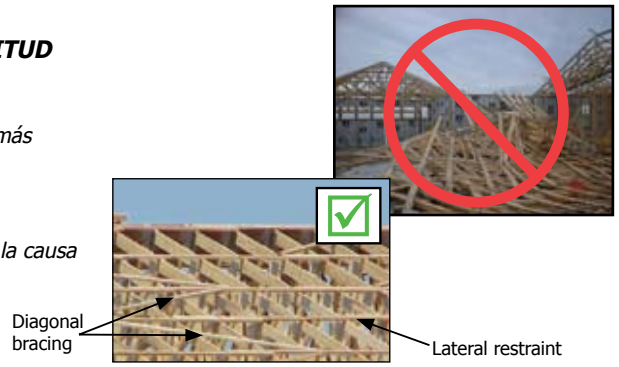
FOR TRUSSES UP TO 2'-0" ON-CENTER AND 80'-0" IN LENGTH
PARA TRUSSES HASTA 2 PIES EN CENTRO Y HASTA 80 PIES DE LONGITUD

WARNING! Spans over 60' require more complex temporary installation restraint/bracing. Consult a registered design professional.

Los tramos más de 60 pies requieren restricción/arriostre de instalación temporal más complejo. Consulte a un profesional registrado de diseño.

DANGER! Disregarding handling, installing, restraining and bracing safety recommendations is the major cause of truss erection/installation accidents.

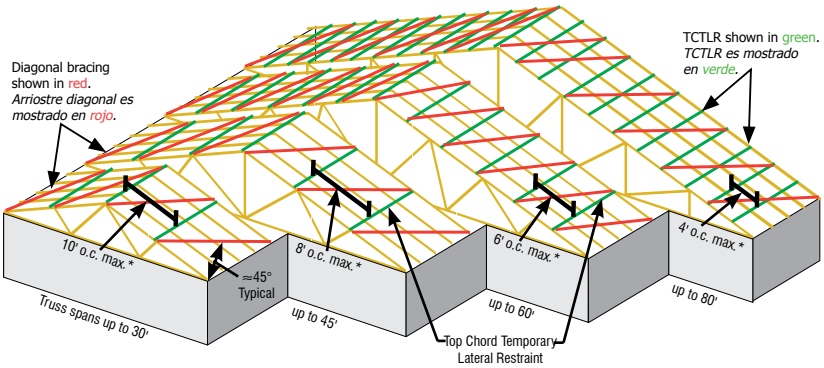
El no seguir las recomendaciones de manejo, instalación, restricción y arriostre es la causa principal de los accidentes durante la erección/instalación de trusses.



NOTICE Lateral restraint is NOT adequate without diagonal bracing.
La Restricción Lateral NO es adecuada sin el Arriostre Diagonal.

☑ Always diagonally brace for safety!
¡Siempre arriostre diagonalmente para seguridad!

MAXIMUM SPACING FOR TOP CHORD TEMPORARY LATERAL RESTRAINT (TCTLR)
EL ESPACIAMIENTO MÁXIMO PARA LA RESTRICCIÓN LATERAL TEMPORAL DE LA CUERDA SUPERIOR (TCTLR)



The graphic at left shows the maximum on-center spacing (see * at left) of TCTLR based on truss span from the table in Step 2 on page 2.

- Ground bracing not shown for clarity.
- Apply diagonal bracing or structural sheathing immediately. For spans over 60' applying structural sheathing immediately is the preferred method.

*El dibujo a la izquierda muestra el espaciamiento máximo en el centro (vea * a la izquierda) del TCTLR basado en los tramos de trusses de la tabla en el Paso 2 en la página 2.*

- *No se muestra el arriostre de tierra para claridad.*
- *Aplique inmediatamente el Arriostre Diagonal o el Entablado Estructural (structural sheathing). Para tramos más de 60 pies el método preferido es entablarlos inmediatamente.*

CHECK THESE ITEMS BEFORE STARTING ERECTION/INSTALLATION AND CORRECT AS NEEDED
REVISE ESTOS PUNTOS ANTES DE EMPEZAR LA ERECCIÓN/INSTALACIÓN Y CORRÍJALOS CUANDO ES NECESARIO

☑ Building dimensions match the construction documents.
Las dimensiones del edificio concuerdan con los documentos de construcción.

☑ Bearing supports (e.g., walls, columns, headers, beams, etc.) are accurately and securely installed, plumb and properly braced.
Los soportes que sostienen cargas (ej., paredes, columnas, vigas de cabezera, vigas, etcétera) son instalados seguramente y con precisión, y son nivelados y arriostrados apropiadamente.

☑ Hangers, tie-downs, restraint and bracing materials are on site and accessible.
Los colgadores (hangers), soportes de anclaje (tie-downs) y materiales de restricción y arriostre están accesibles en la obra.

☑ Erection/installation crew is aware of installation plan and lateral restraint/diagonal bracing requirements.
El personal de erección/instalación es consciente del plan de instalación y los requisitos de restricción/arriostre.

☑ Multi-ply trusses, including girders, are correctly fastened together prior to lifting into place.
Los trusses de varias capas, incluyendo travesaños, son fijados juntos correctamente antes de levantarlos en lugar.

☑ Any truss damage is reported to truss manufacturer. Refer to **BCSI-B5**.*** Do not install damaged trusses unless instructed to do so by the building designer, truss designer or truss manufacturer.

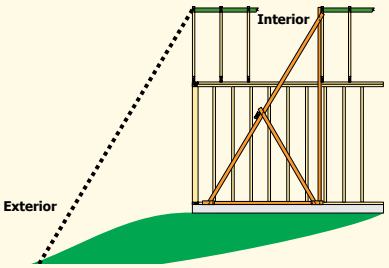
*Algún daño a los trusses ha sido reportado al fabricante de trusses. Vea el resumen **BCSI-B5**.*** No instale trusses dañados a menos que se dijeren el diseñador del edificio, el diseñador del truss o el fabricante del truss.*

☑ Trusses are the correct dimension.
Los trusses son la dimensión correcta.

☑ Tops of bearing supports are flat, level and at the correct elevation.
La parte superior de los soportes de cojinete son planas, niveladas y a la elevación correcta.

☑ Jobsite is clean and neat, and free of obstructions.
La obra está limpia, ordenada y sin obstrucciones.

☑ Ground bracing procedure for first truss is based on site and building configuration.
El procedimiento de Arriostre de Tierra para el primer truss es basado en el terreno y la configuración del edificio.



If ground level is too far from truss for exterior ground bracing, use interior ground bracing.

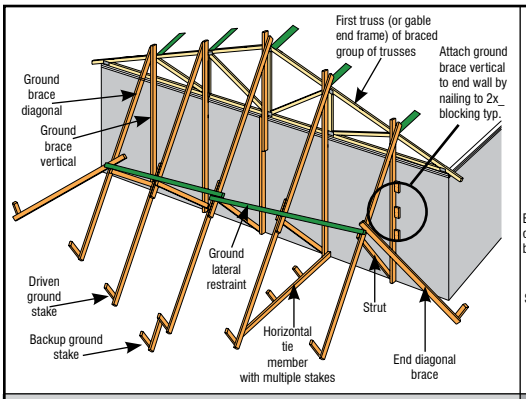
Si el nivel del terreno es demasiado lejos para usar Arriostre de Tierra exterior, use Arriostre de Tierra interior.

STEPS TO SETTING TRUSSES

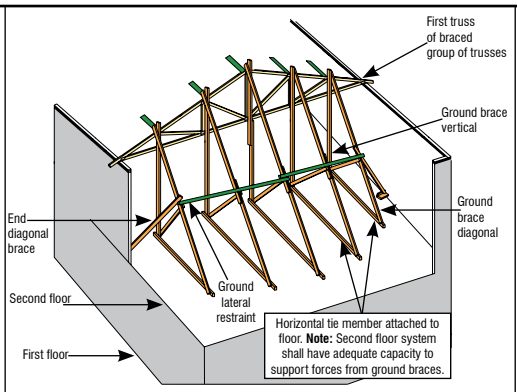
PASOS PARA EL MONTAJE DE TRUSSES

1. Establish Ground Bracing Procedure: Exterior or Interior

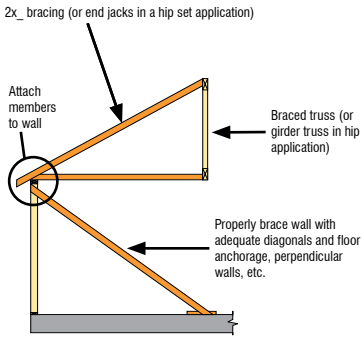
Establezca el Procedimiento de Arriostre de Tierra: Exterior o Interior



EXTERIOR GROUND BRACING
ARRIOSTRE DE TIERRA EXTERIOR



INTERIOR GROUND BRACING
ARRIOSTRE DE TIERRA INTERIOR



INTERIOR GROUND BRACING TO END WALL
ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO

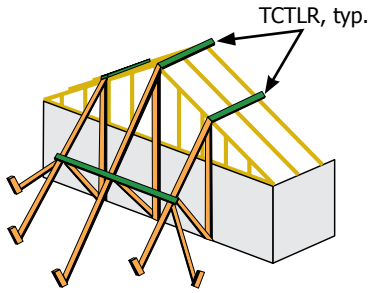
2. Determine the locations for TCTLR and Ground Braces

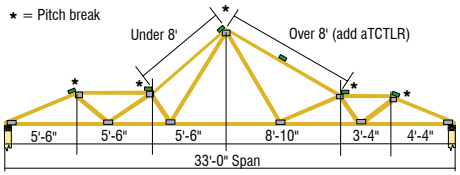
Determine las ubicaciones para TCTLR y los Arriostres de Tierra

☑ Use truss span to determine spacing for top chord temporary lateral restraint (TCTLR) from table at right.
Use el tramo del truss para determinar el espaciamiento para restricción lateral temporal de la cuerda superior (TCTLR) en la tabla a la derecha.

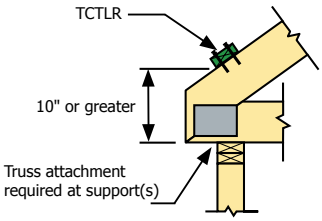
Maximum Top Chord Temporary Lateral Restraint Spacing**	
Truss Span	**TCTLR Spacing**
Up to 30'	10' on-center maximum
30' - 45'	8' on-center maximum
45' - 60'	6' on-center maximum
60' - 80'*	4' on-center maximum

***Consult a registered design professional for trusses longer than 60'.**
****For trusses spaced greater than 2' o.c., see also BCSI-B10.**

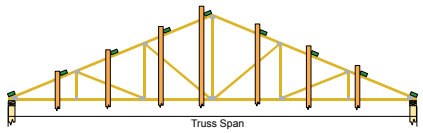




☑ Locate additional TCTLR at each pitch break.
Localice TCTLR adicional en cada rotura de inclinación.



☑ Locate additional TCTLR over bearings if the heel height is 10" or greater.
Localice TCTLR adicional sobre los soportes si la altura del talón (heel height) es de 10 pulgadas o más.

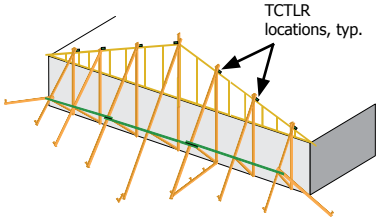


☑ Locate a ground brace vertical at each TCTLR location.
Localice una vertical de arriostre de tierra en cada lugar de TCTLR.

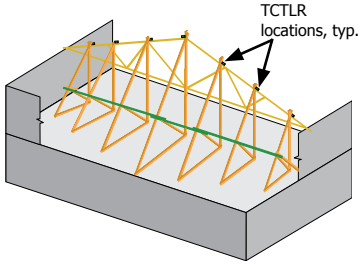
3. Set First Truss and Fasten Securely to Ground Braces

Coloque el Primer Truss y Fíjelo Seguramente a los Arriostres de Tierra

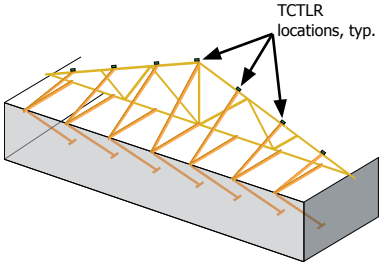
☑ Set first truss (or gable end frame) and fasten securely to ground braces and to the wall, or as directed by the building designer. Examples of first truss installed include:
Coloque el primer truss (o armazón hastial) y fíjelo seguramente a las verticales de arriostre de tierra y a la pared, o como se dirige el diseñador del edificio. Ejemplos del primer truss instalado incluyen:



EXTERIOR GROUND BRACING
ARRIOSTRE DE TIERRA EXTERIOR



INTERIOR GROUND BRACING
ARRIOSTRE DE TIERRA INTERIOR



INTERIOR GROUND BRACING TO END WALL
ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO

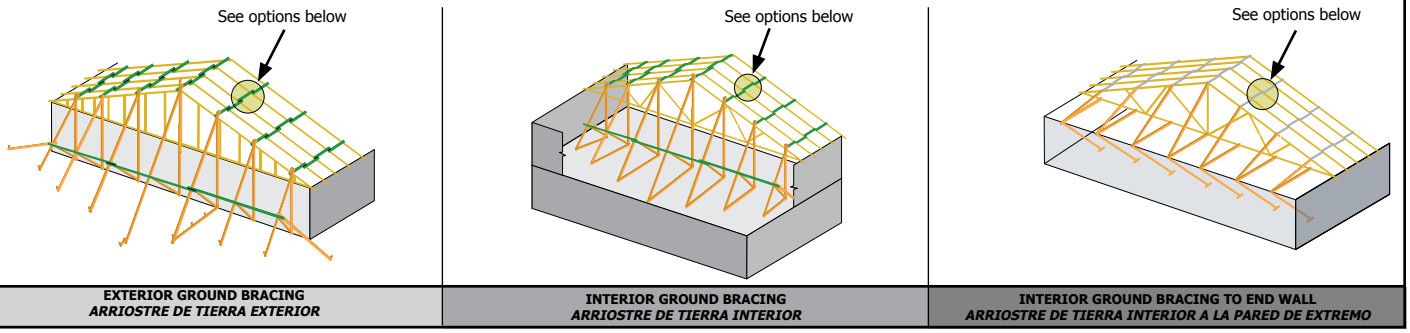
⚠ **CAUTION!** First truss must be attached securely to all bearings and to all required ground braces prior to removing the hoisting supports.

El primer truss tiene que ser sujetado seguramente a todos soportes y a todas arriostres de tierra requeridos, antes de quitar los soportes de levantamiento.

4. Set Trusses 2, 3, 4 & 5 with TCTLR in Line with Ground Bracing

Coloque los Trusses 2, 3, 4 y 5 con TCTLR en Línea con los Arriostres de Tierra

☑ Attach trusses securely at all bearings, shimming bearings as necessary. Examples of first five trusses set include:
Sujete seguramente los trusses a todos los soportes, rellenar sólidamente los soportes como sea necesario. Ejemplos de los primeros cinco trusses colocados incluyen:

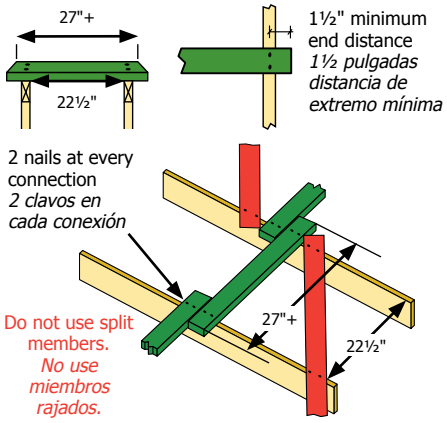


EXTERIOR GROUND BRACING *ARRIOSTRE DE TIERRA EXTERIOR*	INTERIOR GROUND BRACING *ARRIOSTRE DE TIERRA INTERIOR*	INTERIOR GROUND BRACING TO END WALL *ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO*

NOTICE The following three (3) Short Member Temporary Lateral Restraint options **require that the diagonal bracing be installed continuously**.
Las siguientes tres opciones para instalar la Restricción Lateral Temporal de los Miembros Cortos requieren que el arriostre diagonal está instalado continuamente.

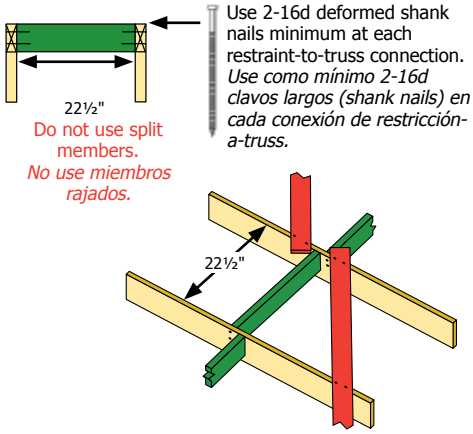
Option 1
Short member temporary lateral restraint installed on top of trusses

Opción 1
Restricción lateral temporal de los miembros cortos instalados encima de trusses



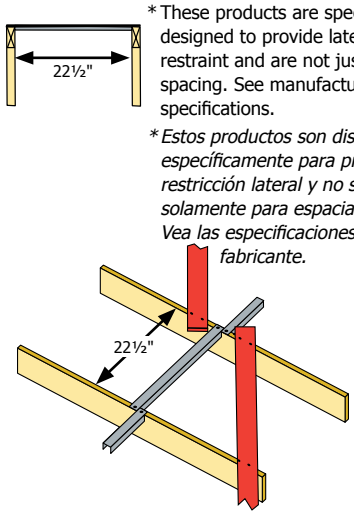
Option 2
Short member temporary lateral restraint installed between trusses

Opción 2
Restricción lateral temporal de los miembros cortos instalados entre trusses



Option 3
Proprietary metal restraint products*

Opción 3
*Productos de metal para restricción patentados**



⚠ **CAUTION!** Each truss must be attached securely at each bearing and all TCTLR installed before removing the hoisting supports.
Cada truss tiene que ser sujetado seguramente en cada soporte y todas las TCTLR instaladas antes de quitar los soportes de levantar.

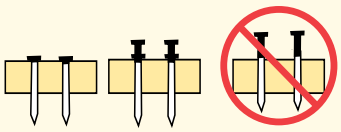
LATERAL RESTRAINT/BRACING MATERIAL AND CONNECTIONS
CONNEXIONES Y MATERIALES DE RESTRICCIÓN/ARRIOSTRE LATERAL

☑ Minimum size of bracing and lateral restraint material is 2x4 stress-graded lumber or approved proprietary metal restraint/bracing, unless otherwise specified by the building designer.
El tamaño del material de restricción lateral y arriostre debe ser por lo menos 2x4 madera graduada por esfuerzo o restricción/arriostre de metal patentado aprobado, a menos que especifique el diseñador del edificio.

☑ All bracing and lateral restraint members must be connected to each truss with at least 2 nails (see minimum sizes shown below), except for the short member restraints shown in Step 4, Option 2 (see above), which require 2-16d deformed-shank (i.e., ring- or screw-shank) nails.
Todos los miembros de restricción lateral y arriostre tienen que ser conectados a cada truss con un mínimo de 2 clavos (ver los tamaños mínimos mostrados abajo) excepto para las restricciones de miembros cortos mostrados en el Paso 4, Opción 2 (vea arriba), cuales requieren 2-16d clavos con largos deformados (Ej. Largos de anillos o tornillos).

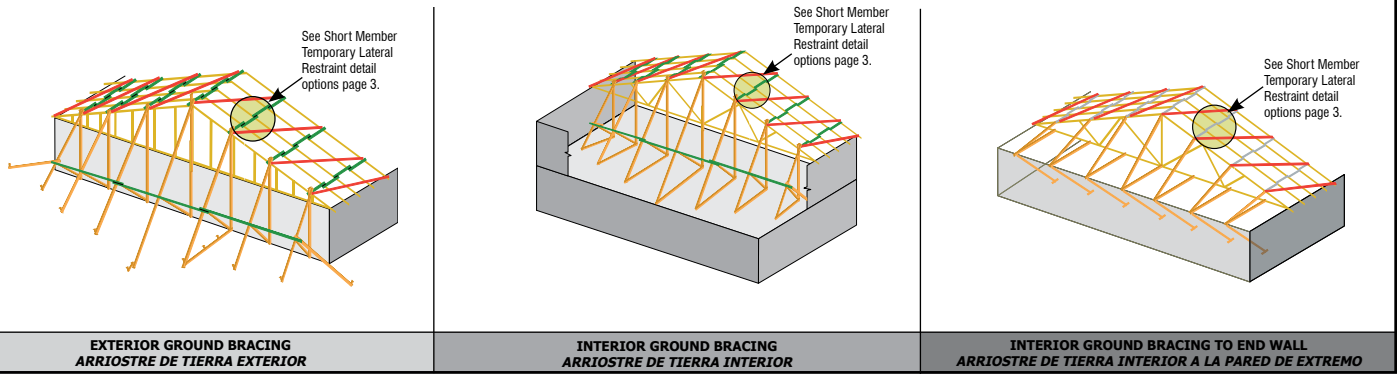
☑ Drive nails flush, or use double-headed nails for easiest removal.
Clave los clavos al raso, o use clavos de dos cabezas para quitarlos más fácilmente.

10d (0.128x3")
12d (0.128x3.25")
16d (0.131x3.5")



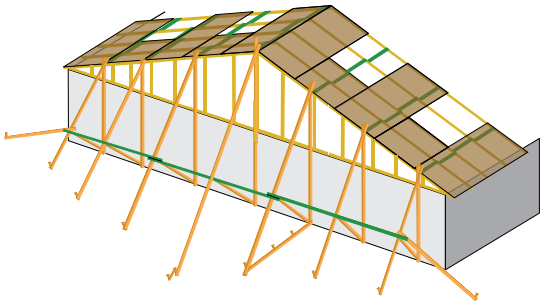
5. Install Top Chord Diagonal Bracing
Instale el Arriostre Diagonal de la Cuerda Superior

☑ Attach diagonal bracing to the first five trusses. Examples of diagonal bracing on first five trusses include:
Coloque el arriostre diagonal a los primeros cinco trusses. Ejemplos de arriostre diagonal en los primeros cinco trusses incluyen:



EXTERIOR GROUND BRACING *ARRIOSTRE DE TIERRA EXTERIOR*	INTERIOR GROUND BRACING *ARRIOSTRE DE TIERRA INTERIOR*	INTERIOR GROUND BRACING TO END WALL *ARRIOSTRE DE TIERRA INTERIOR A LA PARED DE EXTREMO*

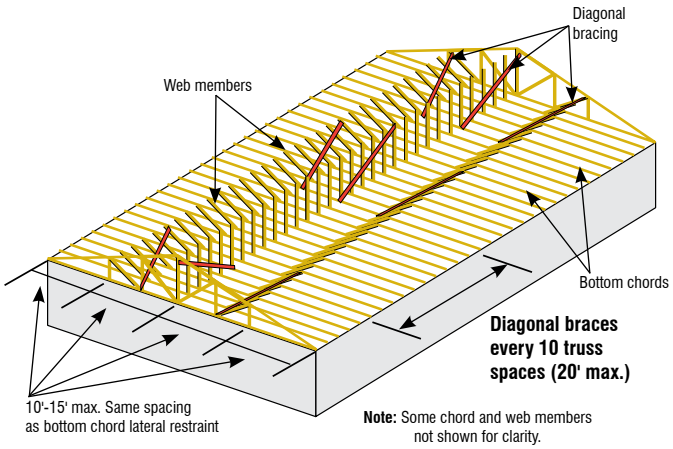
☑ Or start applying structural sheathing. Example of structural sheathing installed on first five trusses.
O empiece en aplicar el entablado estructural. Ejemplo de entablado estructural instalado en los primeros cinco trusses.



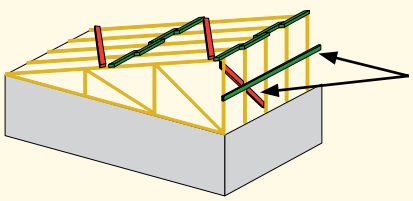
6. Install Web Member Diagonal Bracing
Instale el Arriostre Diagonal de Miembros Secundarios

☑ Temporary web member diagonal bracing acts with the top chord and bottom chord temporary lateral restraint and diagonal bracing to form triangulation perpendicular to the plane of the truss and prevents trusses from leaning or dominoing.
El arriostre diagonal temporal de los miembros secundarios trabaja con la restricción lateral y el arriostre diagonal temporales de la cuerda superior e inferior para formar una triangulación perpendicular al plano del truss y evita que los trusses se inclinen o caigan como dominós.

☑ Install diagonal bracing at about 45° on web members (verticals whenever possible) located at or near rows of bottom chord lateral restraint. Web diagonal bracing must extend from the top chord to the bottom chord. Repeat at the interval shown in the Figure below.
Instale el arriostre diagonal a aproximadamente 45 grados en los miembros secundarios (verticales cuando sea posible) colocados en o cerca de las filas de restricción lateral de la cuerda inferior. Arriostre diagonal para los miembros secundarios tiene que extender de la cuerda superior a la cuerda inferior. Repita a los intervalos mostrados en la Figura a la derecha.



NOTICE The requirements for web member permanent individual truss member restraint are specified on the truss design drawing (TDD). Refer to **BCSI-B3** for more information.***
*Los requisitos para la restricción permanente de miembros individuales de truss para miembros secundarios son especificados en el dibujo del diseño de truss. Vea el resumen **BCSI-B3** para más información.****



NOTICE Mono trusses, deep flat trusses and other types of trusses with deep ends also require temporary lateral restraint and diagonal bracing on the long web members at the deep end of the truss.
Los trusses de una sola pendiente, trusses planos y profundos y otros tipos de trusses con extremos profundos también requieren restricción lateral temporal y arriostre diagonal en los miembros secundarios largos al parte profundo del truss.

BCSI - B3 SUMMARY SHEET PERMANENT RESTRAINT/ BRACING OF CHORDS AND WEB MEMBERS

SPANS OVER 60' MAY REQUIRE COMPLEX PERMANENT BRACING. PLEASE ALWAYS CONSULT A PROFESSIONAL ENGINEER

WARNING! Disregarding Permanent Restraint/Bracing is a major cause of truss field performance problems and has been known to lead to roof or floor systems collapse.
¡ADVERTENCIA! *Descuidar el Arriostre/Restricción Permanente es una causa principal de problemas de rendimiento del truss en campo y había conocido a llevar al derrumbamiento del sistema del techo o piso.*

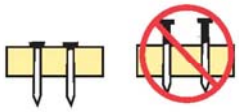
CAUTION! Spans over 60' may require complex permanent bracing. Please always consult a Registered Design Professional.
¡CAUTELA! *Tramos sobre 60 pies pueden requerir arriostre permanente complejo. Por favor, siempre consulte a un Profesional Registrado de Diseño.*

RESTRAINT/BRACING MATERIALS & FASTENERS
MATERIALES Y CIERRES DE RESTRICCIÓN/ARRIOSTRE

☑ Common restraint/bracing materials include wood structural panels, gypsum board sheathing, stress-graded lumber, proprietary metal products, and metal purlins and straps.
Materiales comunes de arriostrar/restringir incluyen paneles estructurales de madera, entablado de yeso, madera graduada por esfuerza, productos de metal patentados, y vigas de soporte y tiras de metal.

MINIMUM ATTACHMENT REQUIREMENTS FOR LUMBER RESTRAINT/BRACING[1,2]

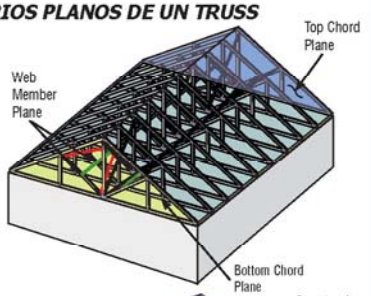
Lumber Size	Minimum Nail Size	Minimum Number of Nails per Connection
2x4 stress-graded	10d (0.128x3") 12d (0.128x3.25") 16d (0.131x3.5")	2
2x6 stress-graded	10d (0.128x3") 12d (0.128x3.25") 16d (0.131x3.5")	3



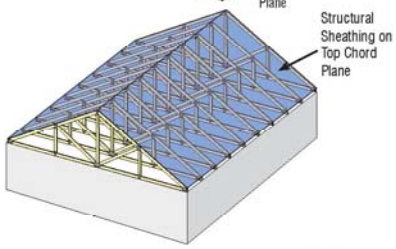
[1] Other attachment requirements may be specified by the Truss Designer or Building Designer.
[2] The size and attachment for bracing materials such as wood structural panels, gypsum board sheathing, proprietary metal restraint/bracing products, and metal purlins and straps are provided by the Building Designer.

PERMANENT BRACING FOR THE VARIOUS PLANES OF A TRUSS
ARRIOSTRE PERMANENTE PARA VARIOS PLANOS DE UN TRUSS

☑ Permanent Bracing is important because it,
a) prevents out-of-plane buckling of truss members,
b) helps maintain proper truss spacing, and
c) resists and transfers lateral loads from wind and seismic forces.
El arriostre Permanente es importante porque,
a) impide el torcer fuera-de-plano de los miembros del truss,
b) ayuda en mantener espaciamiento apropiado de los trusses, y
c) resiste y pasa las cargas laterales de viento y fuerzas sísmicas aplicadas al sistema del truss.



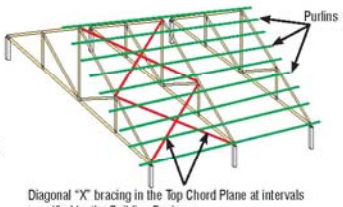
☑ Trusses require Permanent Bracing within ALL of the following planes:
1. Top Chord Plane
2. Bottom Chord Plane
3. Web Member Plane
Trusses requieren Arriostre Permanente dentro de TODOS los siguientes planos:
1. Plano de la Cuerda Superior
2. Plano de la Cuerda Inferior
3. Plano del Miembro Secundario



CAUTION! Without Permanent Bracing the truss, or a portion of its members, will buckle (i.e., fail) at loads far less than design.
¡CAUTELA! *Sin el Arriostre Permanente, del truss, o un parte de los miembros, torcerán (ej. fallarán) de cargas muchas menos que las cargas que el truss es diseñado a llevar.*

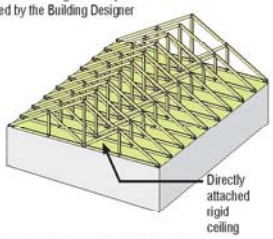
1. PERMANENT BRACING FOR THE TOP CHORD PLANE
1. ARRIOSTRE PERMANENTE PARA EL PLANO DE LA CUERDA SUPERIOR

☑ Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced.
Use contrachapado, panel de fibras orientadas (OSB), o vigas de soporte de madera o metal que son arriostrados apropiadamente.



☑ The Truss Design Drawing (TDD) provides information on the assumed support for the top chord.
El Dibujo del Diseño de Truss (TDD) provee información sobre el soporte supuesto para la cuerda superior.

☑ Fastener size and spacing requirements and grade for the sheathing, purlins and bracing are provided in the building code and/or by the Building Designer.
El tamaño de cierre y requisitos de espaciamiento y grado para el entablado, vigas de soporte y arriostre son provistos en el código del edificio y/o por el Diseñador del Edificio.



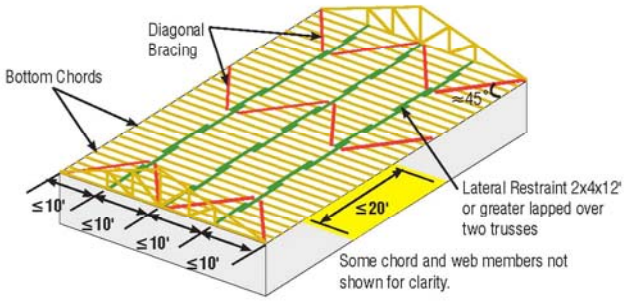
2. PERMANENT BRACING FOR THE BOTTOM CHORD PLANE
2. ARRIOSTRE PERMANENTE PARA EL PLANO DE LA CUERDA INFERIOR

☑ Use rows of continuous Lateral Restraint with Diagonal Bracing, gypsum board sheathing or rigid ceiling.
Use filas de Restricción Lateral Continua con Arriostre Diagonal, entablado de yeso o techo rigído.

☑ The TDD provides information on the assumed support for the bottom chord.
El TDD provee información sobre el soporte supuesto para la cuerda inferior.

☑ **Install bottom chord permanent Lateral Restraint at the spacing indicated on the TDD and/or by the Building Designer with a maximum of 10' on center.**

Instale Restricción Lateral permanente de la cuerda inferior al espaciamiento indicado en el TDD y/o por el Diseñador del Edificio con un máximo de 10 pies en el centro.



Lateral Restraint and Diagonal Bracing used to brace the Bottom Chord Plane.

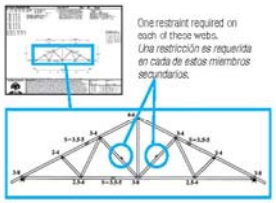
3. PERMANENT BRACING FOR THE WEB MEMBER PLANE
3. ARRIOSTRE PERMANENTE PARA EL PLANO DEL MIEMBRO SECUNDARIO

Web Member Permanent Bracing collects and transfers buckling restraint forces and/or lateral loads from wind and seismic forces. The same bracing can often be used for both functions.
Arriostre Permanente de los Miembros Secundarios recogen y pasan fuerzas de restricción de torcer y/o cargas laterales de viento y fuerzas sísmicas. A menudo el mismo arriostre puede ser usado para ambos funciones.

Individual Web Member Permanent Restraint & Bracing
Restricción y Arriostre Permanente de Miembros Secundarios Individuales

☑ Check the TDD to determine which web members (if any) require restraint to resist buckling.
Revisa el TDD para determinar cuales miembros secundarios (si algunos) requieren restricción para resistir el torcer.

☑ Restrain and brace with,
A. Continuous Lateral Restraint & Diagonal Bracing, or
B. Individual Member Web Reinforcement.
Restrinja y arriostre con,
A. Restricción Lateral Continua y Arriostre Diagonal, o
B. Refuerzo de Miembros Secundarios Individuales.

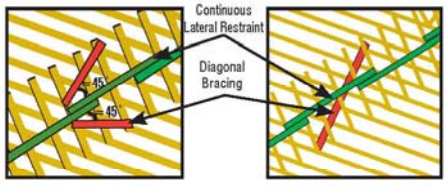


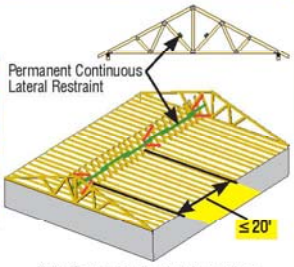
A. Continuous Lateral Restraint (CLR) & Diagonal Bracing
A. Restricción Lateral Continua (CLR) y Arriostre Diagonal

☑ Attach the CLR at the locations shown on the TDD.
Sujete el CLR en las ubicaciones mostrados en el TDD.

☑ Install the Diagonal Bracing at approximately 45° to the CLR and position so that it crosses the web in close proximity to the CLR. Attach the Diagonal Brace as close to the Top and Bottom Chords as possible and to each web it crosses. **Repeat every 20' or less.**
Instale el Arriostre Diagonal a aproximadamente 45 grados al CLR y lo coloque para que cruce la cuerda muy cerca del CLR. Sujete el Arriostre Diagonal como cercano a las cuerdas inferior y superior como sea posible y a cada cuerda que lo cruza. ***Repita cada 20 pies o menos.***

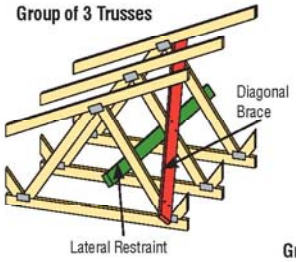
EXAMPLES OF DIAGONAL BRACING WITH CONTINUOUS LATERAL RESTRAINT



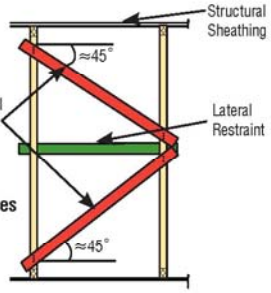


Note: Some chord and web members not shown for clarity.

Group of 3 Trusses



Group of 2 Trusses



☑ Lateral Restraint & Diagonal Bracing can also be used with small groups of trusses (i.e., three or less). Attach the Lateral Restraint & Diagonal Brace to each web member that they cross.
Restricción Lateral y Arriostre Diagonal también puede ser usado con grupos pequeños de trusses (ej. tres o menos). Sujete la Restricción Lateral y el Arriostre Diagonal a cada miembro secundario que los cruzan.

RESTRICCION / ARRIOSTRE PERMANENTE DE LAS CUERDAS Y LOS MIEMBROS SECUNDARIOS

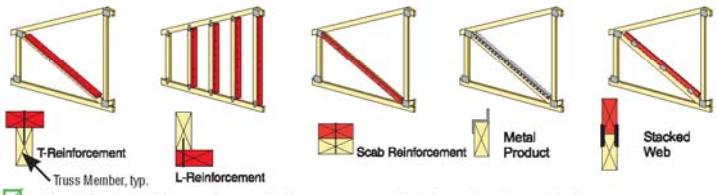
TRAMOS DE 60' PIES PUEDEN REQUERIR ARRIOSTRE PERMANENTE COMPLEJO.POR FAVOR,SIEMPRE CONSULTE A UN PROFESIONAL DE DISENO REGISTRADO.

BCSI-B3 SUMMARY SHEET PERMANENT RESTRAINT / BRACING OF CHORDS AND WEB
SPANS OVER 60' MAY REQUIRE COMPLEX PERMANENT BRACING. PLEASE ALWAYS CONSULT A PROFESSIONAL ENGINEER

ALWAYS DIAGONALLY BRACE THE CONTINUOUS LATERAL RESTRAINT!
¡SIEMPRE ARRIOSTRE LA RESTRICCIÓN LATERAL CONTINUA DIAGONALMENTE!

B. Individual Web Member Reinforcement
B. Refuerzo de Miembros Secundarios Individuales

T-, L-, Scab, I-, U-Reinforcement, proprietary metal reinforcement and stacked web products provide an alternative for resisting web buckling.
T-, L-, costra, I-, U-Refuerzo, refuerzo de metal patentando y productos de miembros secundarios amontonados proveen una alternativa para resistir el torcer de los miembros secundarios.

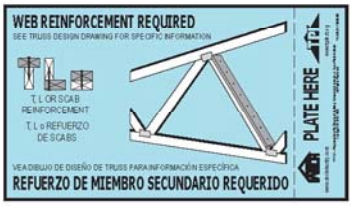


☑ The following table may be used unless more specific information is provided.
La siguiente tabla puede ser usada a menos que información más específica está provista.

WEB REINFORCEMENT FOR SINGLE PLY TRUSSES[1]

Specified CLR	Size of Truss Web	Type & Size of Web Reinforcement				Grade of Web Reinforcement	Minimum Length of Web Reinforcement	Minimum Connection of Web Reinforcement to Web
		T	L	Scab[2]	I			
1 Row	2x4	2x4	2x4	2x4		Same species and grade or better than web member	90% of web or extend to within 6" (150 mm) of end of web member, whichever is greater	16d Gun nails (0.131x3.5") @ 6" (150 mm) on-center[2]
	2x6	2x6	2x6	2x6				
	2x8	2x8	2x8	2x8				
2 Rows	2x4	---	---	---	2-2x4			
	2x6	---	---	---	2-2x6			
	2x8	---	---	---	2-2x8			

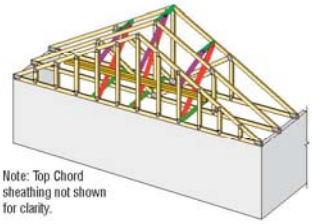
[1]Maximum allowable web length is 14' (4.3 m).
[2]For Scab Reinforcement use 2 rows of 10d Gun nails (0.120x3") at 6" (150 mm) on-center to attach reinforcement to web.

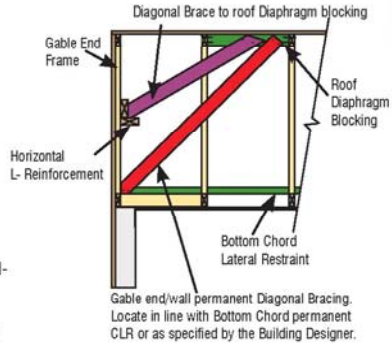


☑ Some Truss Manufacturers mark the locations of the web Lateral Restraint or reinforcement on the truss using tags similar to those above.
Algunos Fabricantes de Trusses marcan en el truss las ubicaciones de refuerzo o Restricción Lateral de miembros secundarios con etiquetas similares a las arriba.

Web Member Plane Permanent Building Stability Bracing to Transfer Wind & Seismic Forces
Arriostre de Estabilidad Permanente del Edificio del Plano de Miembros Secundarios para Desplazar Fuerzas de Viento y Fuerzas Sísmicas

☑ The web member restraint or reinforcement specified on a TDD is required to resist buckling under vertical loads. Additional restraint and bracing is typically required to transfer lateral loads due to wind and/or seismic forces. This restraint and bracing is typically provided by the Building Designer.
La restricción o refuerzo de miembros secundarios especificada en un TDD es requerido a resistir el torcer bajo cargas verticales. Restricción y arriostre adicional es requerido típicamente para pasar cargas laterales debidas a fuerzas de viento y/o fuerzas sísmicas. Esta restricción y arriostre es típicamente provisto por el Diseñador del Edificio.

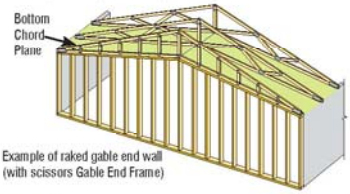




☑ Some Truss Designers provide general design tables and details to assist the Building Designer in determining the Bracing required to transfer lateral loads due to wind and/or seismic forces from the Gable End Frame into the roof and/or ceiling diaphragm.
Algunos Diseñadores de Trusses proveen tablas y detalles de diseño generales para asistir el Diseñador del Edificio en determinar el Arriostre requerido para pasar cargas laterales debidas a fuerzas de viento y/o fuerzas sísmicas del Armazón Hastial al diafragma del techo.

Gable End Frames and Sloped Bottom Chords
Armazones Hastiales Y Cuerdas Inferiores Pendientes

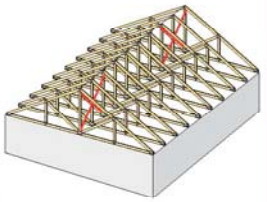
☑ The Gable End Frame should always match the profile of the adjacent trusses to permit installation of proper Bottom Chord Plane restraint & bracing unless special bracing is designed to support the end wall.
El Armazón Hastial siempre debe encajar el perfil de los trusses contiguous para permitir la instalación de restricción y arriostre apropiada de la Cuerda Inferior a menos que arriostre especial es diseñado para soportar la pared de extremo.



⚠ **CAUTION!** Using a flat Bottom Chord Gable End Frame with adjacent Trusses that have sloped Bottom Chords is prohibited by some building codes as adequate bracing of this condition is difficult and sometimes impossible. Special end wall bracing design considerations are required by the Building Designer if the Gable End Frame profile does not match the adjacent Trusses.
¡CAUTELA! *El uso de un Armazón Hastial de la Cuerda Inferior con Trusses contiguos cuales tienen Cuerdas Inferiores pendientes es prohibido por algunos códigos de edificios porque arriostre adecuado de esta condición es difícil y a veces imposible. Consideraciones especiales de diseño para el arriostre de la pared de extremo son requeridos por el Diseñador del Edificio si el perfil del Armazón Hastial no hace juego con los Trusses contiguos.*

PERMANENT BRACING FOR SPECIAL CONDITIONS
ARRIOSTRE PERMANENTE PARA CONDICIONES ESPECIALES
Sway Bracing—*Arriostre de "Sway"*

☑ "Sway" bracing is installed at the discretion of the Building Designer to help stabilize the truss system and minimize the lateral movement due to wind and seismic loads.
Arriostre de "Sway" está instalado por la discreción del Diseñador del Edificio para ayudar en estabilizar el sistema de trusses y para minimizar el movimiento lateral debido a cargas de viento y cargas sísmicas.

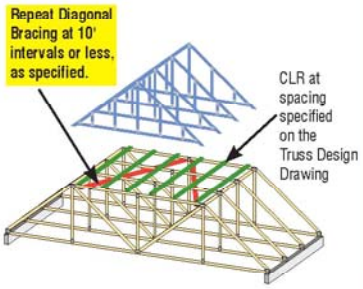


☑ Sway bracing installed continuously across the building also serves to distribute gravity loads between trusses of varying stiffness.
Arriostre de "Sway" que es instalada continuadamente al través del edificio también es usado para distribuir las cargas de gravedad entre trusses de rigidez variando.

Permanent Restraint/Bracing for the Top Chord in a Piggyback Assembly
Restricción/Arriostre Permanente para la Cuerda Superior en un Ensamblaje de Piggyback

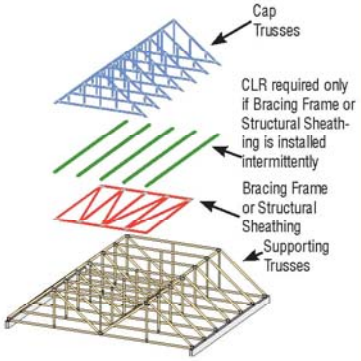
☑ Provide restraint and bracing by:
- using rows of 4x2 stress-graded lumber CLR and Diagonal Bracing, or
- connecting the CLR into the roof diaphragm, or
- adding Structural Sheathing or Bracing Frames, or
- some other equivalent means.

Provee restricción y arriostre por:
- *usando filas de 4x2 CLR madera graduada por esfuerza y Arriostre Diagonal, o*
- *conectando el CLR al diafragma del echo, o*
- *añadiendo Entablado Estructural o Arm zanes de Arriostre, o*
- *algunos otros métodos equivalentes.*



☑ Refer to the TDD for the maximum assumed spacing for attaching the Lateral Restraint to the top chord of the supporting truss.
Refiere al TDD para el espaciamiento máximo supuesto para sujetar la Restricción Lateral a la cuerda superior del truss soportante.

☑ The TDD provides the assumed thickness of the restraint and minimum connection requirements between the cap and the supporting truss or restraint.
El TDD provee el grosor supuesto de la restricción y los requisitos de conexión mínimos entre la capa y el truss soportante o la restricción.



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RESTRICCION / ARRIOSTRE PERMANENTE DE LAS CUERDAS Y LOS MIEMBROS SECUNDARIOS
TRAMOS MAS DE 60' PIES PUEDEN REQUERIR ARRIOSTRE PERMANENTE COMPLEJO. POR FAVOR, SIEMPRE CONSULTE A UN PROFESIONAL DE DISENO REGISTRADO

B4 Construction Loading
Cargas de Construcción

Construction loads are those loads imposed on the unfinished building as a result of the construction process. Typical construction loads include the weight of the workers, equipment, and building materials, to name a few. For example, a bundle of plywood sheathing or gypsum board stacked on trusses temporarily creates construction loads.

Cargas de construcción son las cargas que están impuestas a los edificios incompletos como resultado del proceso de construcción. Cargas de construcción típicas incluyen el peso de los trabajadores, el equipo y los materiales de construcción, etcétera. Por ejemplo, un paquete de entablado contrachapado o tabla de yeso apilados temporalmente sobre los trusses crean cargas de construcción.

☑ Make sure that the truss assembly is properly restrained and braced according to the guidelines in **BCSI-B1***** and **BCSI-B2***** before placing any construction loads on them. Construction loads shall only be placed on fully restrained and braced structures.

*Asegúrese que el montaje del truss está adecuadamente restringido y arriostrado según las pautas en **BCSI-B1***** y **BCSI-B2***** antes de colocar alguna carga de construcción en la estructura. Solamente coloquen cargas de construcción arriba de estructuras cuales son restringidos y arriostrados completamente.*

⚠ **WARNING!** Stacking excessive amounts of construction materials on floor or roof trusses is an unsafe practice. Property damage, personal injury and/or death are possible if this warning is not heeded.

***¡ADVERTENCIA!** Apilando cantidades excesivas de cargas de construcción sobre trusses de piso u techo es una práctica peligrosa. Daño a la propiedad, herida personal y/o muerte son posibles si no sigue esta advertencia.*

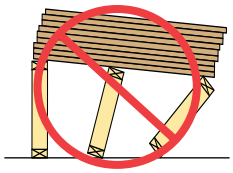
NOTICE Trusses that have been over-stressed due to excessive construction loading will usually show excessive sagging (deflection) and at least a portion of this deflection will remain even after the load has been removed. In more severe cases, broken truss members and/or failed truss joints may result.

Los trusses que han sidos demasiado estresados debido a cargas de construcción excesivas usualmente demuestran una desviación excesiva, y por lo menos una parte de este desviación se quedarán aún después de que la carga se haya quitada. En casos más severos, miembros quebrados del truss y/o junturas falladas pueden resultar.

CONSTRUCTION LOADING DO'S AND DON'TS
QUE HACER Y NO HACER CON LAS CARGAS DE CONSTRUCCIÓN

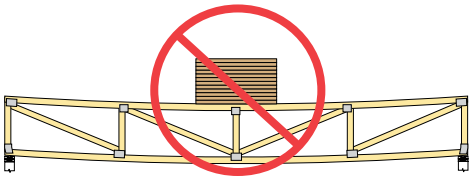
⃠ **DON'T** stack materials on unbraced trusses.

***NO** amontone materiales sobre trusses que no esten arriostrados.*



⃠ **DON'T** overload the trusses.

***NO** sobrecargue los trusses.*

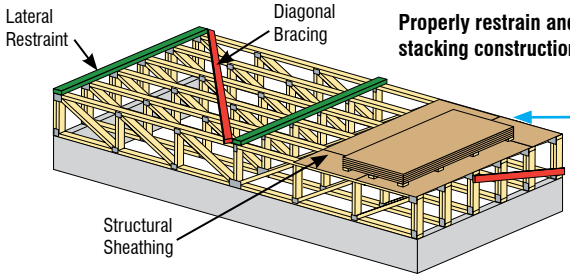


⃠ **DON'T** exceed stack heights listed in the table.

***NO** exceda la altura de montón indicada en la tabla que sigue.*

Maximum Stack Height for Material on Trusses ***Maximua Altura de Montón para Material encima de los Trusses***	
Material – *Material*	**Height – Altura**
Gypsum Board – *Tabla de Yeso*	12" – *12 pulgadas*
Plywood or OSB – *Madera Contrachapada u OSB*	16" – *16 pulgadas*
Asphalt Shingles – *Teja de Asfalto*	2 bundles – *2 paquetes*
Concrete Block – *Bloque de Hormingón*	8" – *8 pulgadas*
Clay Tile – *Teja de Arcilla*	3-4 tiles – *3-4 azulejos*

Note: Limit stacking periods to approximately one week, unless alternative information is provided by the Building Designer, Truss Designer or Truss Manufacturer.



Properly restrain and brace trusses before stacking construction materials on them.

B4ConstLoad 090520

General detail for repair of broken webs, chords, and damaged or missing chord splices that meet the following conditions:

* Webs must be SPF or better and 2x6 or smaller.
* Chords must be SPF or better and 2x6 or smaller.
* Scab must be the same size as the broken member.
* Scab must be same grade or better as the broken member.
* No more than two broken or cracked members per truss.
* Truss must be single ply.
* Perimeter of chord break area must be a minimum distance X from any heel or peak, and minimum of 6" away from any interior joint locations.
* Perimeter of web break area must be a minimum distance X from web joint.

C = Maximum length of damaged area (not to exceed 12"). S = Overall length of scab (shaded). Must be equal or greater than 2(X)+C.

X = Minimum length of scab at each end of break area.

Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.

For all lumber, plates, web bracing, etc., refer to original drawing sealed by Robbins Engineering, Inc.

*Apply all nails so as to avoid damaging of lumber and loosening of plates at joints. Minimum end distance of 2" must be provided for all nails.

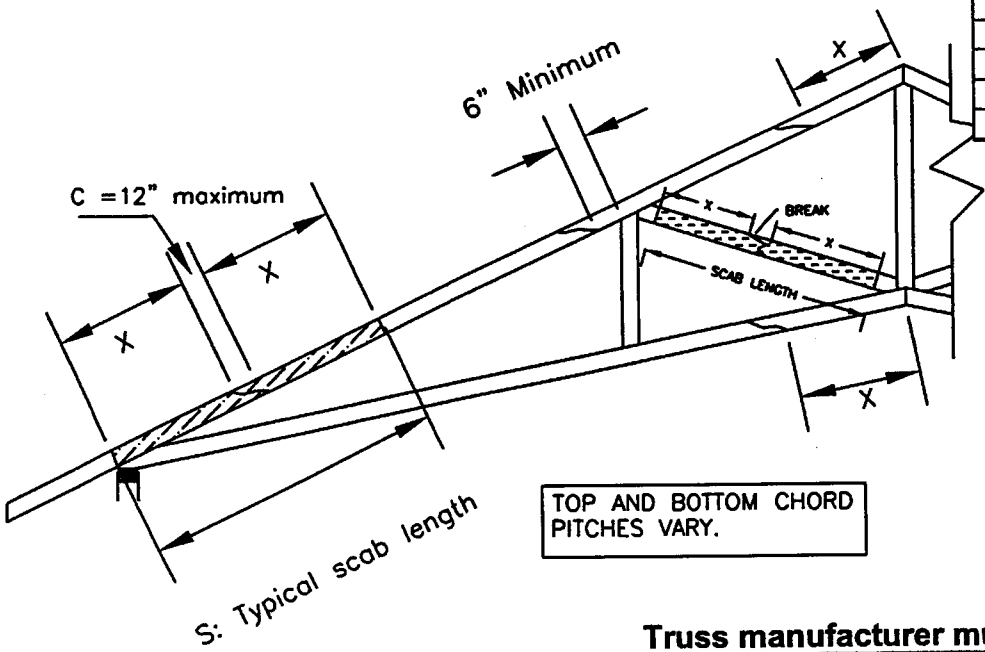
*Use 1 row of 10d common nails spaced 3" on center into 2x3 scabs.

*Use 2 rows of 10d common nails spaced 3" on center in each row and staggered into 2x4 and 2x5 scabs.

*Use 3 rows of 10d common nails spaced 3" on center in each row and staggered into 2x6 scabs.

NOTE: Apply scab to one face of truss using nailing as specified above. If desired, two scabs may be applied, one to each face of truss; use 6" nail spacing if scabs are applied to each face.

2x3 SCABS			2x4 or 2x5 SCABS		
MINIMUM X DISTANCE (in.)	MIN. NUMBER OF NAILS AT EACH END OF	MAX. FORCE (lbs) AT 1.15 LOAD DURATION	MINIMUM X DISTANCE (in.)	MIN. NUMBER OF NAILS AT EACH END OF BREAK	MAX. FORCE (lbs) AT 1.15 LOAD DURATION
14	4	460	10	4	460
20	6	690	16	8	920
26	8	920	22	12	1380
32	10	1150	28	16	1840
38	12	1380	34	20	2300
44	14	1610	40	24	2760
50	16	1840	46	28	3220
56	18	2070	52	32	3680
62	20	2300	58	36	4140
68	22	2530	64	40	4600



TOP AND BOTTOM CHORD PITCHES VARY.

2x6 SCABS		
MINIMUM X DISTANCE (in.)	MIN. NUMBER OF NAILS AT EACH END OF	MAX. FORCE (lbs) AT 1.15 LOAD DURATION
10	6	690
16	12	1380
22	18	2070
28	24	2760
32	28	3220
36	32	3680
40	36	4140
44	40	4600
48	44	5060
52	48	5520

PHILIP J. O'REGAN LICENSE No 58126 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Robbins Engineering
6904 Parke East Blvd
Tampa, FL 33610
FL Cert #5595

Truss manufacturer must be notified if field conditions do not match this general detail.

ROBBINS

Engineering Inc.

P.O.Box 280055, Tampa, FL 33682

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "V", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG

Checked By: PO

Rev. Date: 12/02/05

Dwg. No: GD-69

General detail for repair of broken, damaged or cut chords of PC42 floor trusses that meet the following conditions.

* Chord size must be maximum 4x2 and southern pine species.
* Scab must be at minimum the same size and grade as broken chord. Wide face of scab must be attached to narrow face of chord member.
* Truss must be single ply.
* No more than two broken or cracked chords per truss.
* Perimeter of break area must be minimum of X distance from end of truss. and minimum of 4" away from any plates at panel points. All plates must be intact and fully embedded.

C = Maximum length of damaged area or cut section not to exceed 12".
S = Overall length of scab member must be equal or greater than 2(X)+C.
X = Minimum length of scab member at each end of break area.
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.
For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Attach the scab with one row of 10d common nails at 3" on center into 4x2 chord members. Axial force and number of nails may be doubled or X distance divided by 2, if scabs applied to each face of truss at the same location. Minimum end distance of 3" must be provided for all members.

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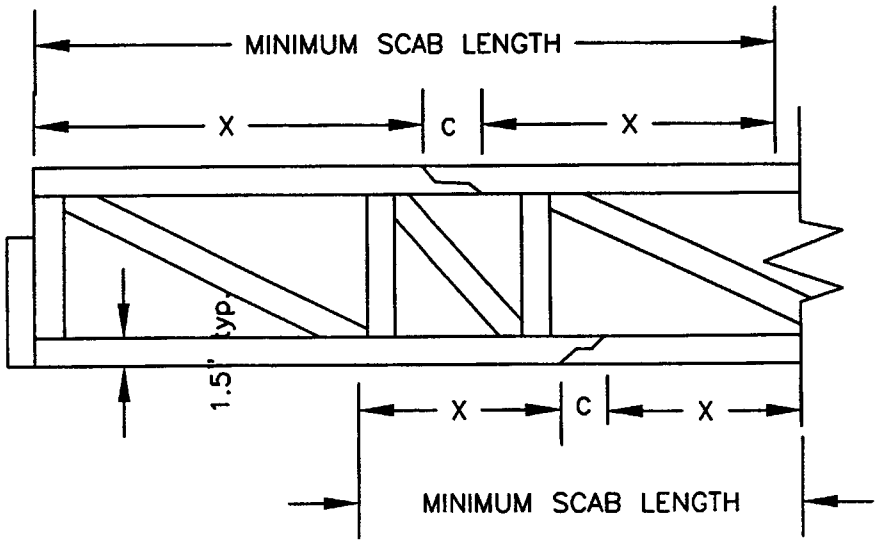
FL Cert #5555



TOTAL NUMBER OF NAILS AT EACH END OF BREAK	X (INCHES)	MAXIMUM AXIAL FORCE IN CHORD MEMBER (LBS.)
4	18	512
6	24	768
8	30	1024
10	36	1280
12	42	1536
14	48	1792
16	54	2048
18	60	2304
20	66	2560
22	72	2816
24	78	3072

Truss manufacturer must be notified if field conditions do not match this general detail.

ROBBINS
Engineering Inc.
P.O.Box 280055, Tampa, FL 33682

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "v", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

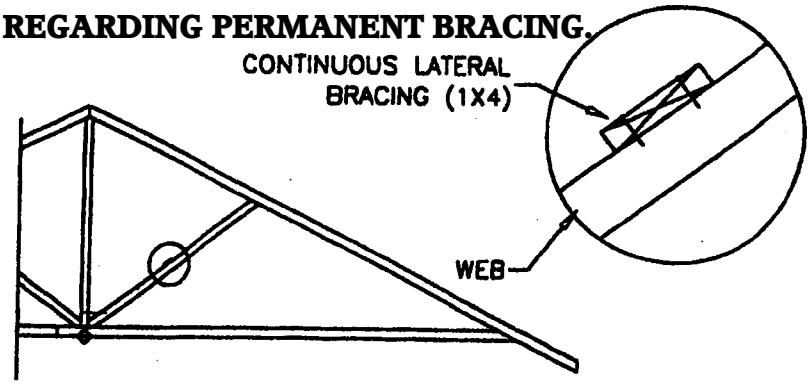
Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Rev. Date: 5/17/04
Dwg. No: GD-62-FL

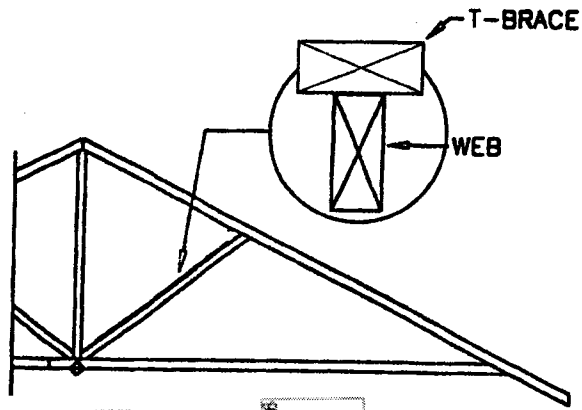
PERMANENT WEB BRACING

SEE INSIDE DESIGN MANUAL FOR BCSI PERMANENT BRACING INSTALLATION INSTRUCTIONS AND FOR FURTHER INFORMATION. CONSULT THE ENGINEER OF RECORD FOR EXPLICIT INSTRUCTION REGARDING PERMANENT BRACING.



CONTINUOUS LATERAL BRACING

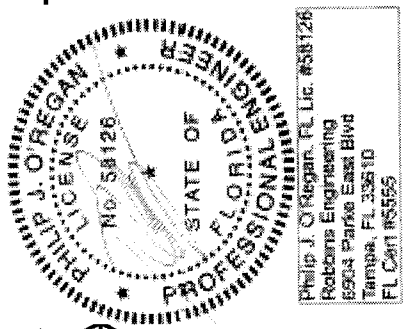
1x4 #3 HEM–FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

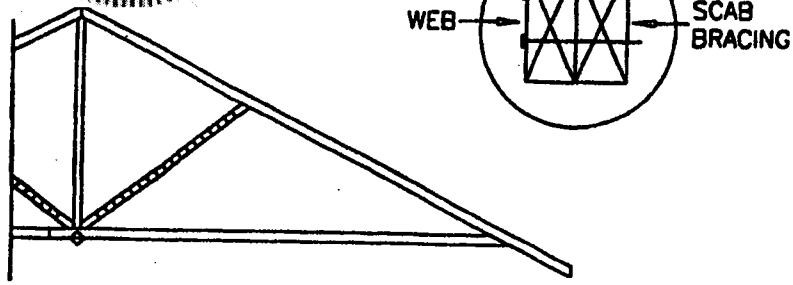


T-BRACE

THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.





SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.