DATE 01/1	5/2008			ty Building I		nstruction		PERMIT 000026616
APPLICANT	D TDENT	GIEBIEG	be i rommently	osted on Tremises	PHONE	386.397.054		700020010
ADDRESS	697	SE HOLLY TERRA	ACF	LAKE CIT		360.397.034.	FL	32025
OWNER		PETTYJOHN	ice	EARE CIT	PHONE	386.752.079		32023
ADDRESS	1811	SW CR 242-A		LAKE CIT		500.752.075	- FL	32025
CONTRACTO		RENT GIEBEIG			PHONE	386.397.054		-
LOCATION O	F PROPER	TY 47 S, L 24	2, THEN .5 MIL	ES ON THE LEFT, I	T IS ACRO	oss	_	
		FROM SV	V CREWS FARM	// TERR				
TYPE DEVEL	OPMENT	SFD,UTILITY		ESTIMATED CO	OST OF CO	NSTRUCTION	87	7800.00
HEATED FLO	OR AREA	1756.00	TOTA	AL AREA 2764.00	1	HEIGHT	18.80	STORIES 1
FOUNDATION	CONC	CRETE WAL	LS FRAMED	ROOF PITCH	6/12	F	LOOR	SLAB
LAND USE &	ZONING	RSF-2			MAX	. HEIGHT	35	•
Minimum Set I	Back Requir	ments: STREET-	FRONT	25.00	REAR	15.00	SIDE	10.00
NO. EX.D.U.	1	FLOOD ZONE	x	DEVELOPM	MENT PERM	MIT NO.		
PARCEL ID	19-4S-17-	08566-000	SUBD	IVISION				
LOT	BLOCK	PHASE	UN	UIT	TOTA	AL ACRES -2	1.42	
			R282811523		ant	halala		
Culvert Permit 1	No.	Culvert Waiver (Contractor's Licen	nse Number	110	Applicant/Owne	er/Contrac	ctor
EXISTING		07-0981	BL		/	Н		N
Driveway Conn	ection	Septic Tank Number	LU &	& Zoning checked by	App	roved for Issuar	nce	New Resident
COMMENTS:	FLOOR O	NE FOOT ABOVE T	HE ROA. NOC	ON FILE.				
-								
3				to		Check # or 0	Cash	3517
		FOR BL	JILDING & Z	ONING DEPAR	RTMENT	ONLY		(footer/Slab)
Temporary Pow	/er		Foundation			Monolithic		(100tc1/3lab)
		date/app. by		date/app. by		-	c	date/app. by
Under slab roug	gh-in plumb			Slab		Sheathing	g/Nailing	
Framing		date/ap	2007 26 76 70	date/ap				date/app. by
	date/app		Kough-in plum	bing above slab and l	below wood			date/app. by
Electrical rough	n-in		Heat & Air Du	ıct		Peri. beam (Lin		
D		date/app. by		date/app.	by			date/app. by
Permanent power		e/app. by	C.O. Final	date/app. by		Culvert	dat	te/app. by
M/H tie downs, l	blocking, el	ectricity and plumbing	·			Pool	dat	старр. бу
Reconnection			Pump pole	ate/app. by	Utility Pol	e	date	/app. by
M/H Pole	d	ate/app. by	vel Trailer	date/app. by	3.50	date/app. b	70	
	e/app. by	_	vei Trailer	date/app. by	_	Re-roof	date/a	pp. by
BUILDING PER	MIT FEE S	440.00	CERTIFICATIO	ON FEE \$ 13.8	2	SURCHARG	E FEE \$	13.82
MISC. FEES \$				50.00 FIRE FEI			TE FEE \$	
FLOOD DEVEL	OPMENT F	FEE \$FLO	OD ZONE FEE \$	25.00 CULVER	-			EE 542.64
INSPECTORS (J. H.			OFFICE	111		JE 5 12.01
	-	0.10		CLLINKS	-		-	

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 080 Pt Date Received 110 By JW Permit # 26616
Zoning Official BLK Date 15-01-08 Flood Zone FEMA Map # N/A Zoning RSF-1
Land Use RV.L.D. Elevation N/A MFE alone R River N/A Plans Examiner KTIH Date 1-14-08
Comments
NOC JEH Deed or PA Site Plan State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. 6000000000000000000000000000000000000
Name Authorized Person Signing Permit Trent Greberg Phone 397-0545
Address 697 SE Holly Terrace Lake City FL 32025
Address 697 SE Holly Terrace Lake City FL 32025 Owners Name James Dwight Pettyjohn Phone 752-0791
911 Address 18/1 5W CR 242 A, C.C.41 32025
Address 697 SE Holly Terrace Lake City FL 32025
Address 697 SE Holly Terrace Lake City FL 32025
Fee Simple Owner Name & Address James Dwight Pettijohn
Bonding Co. Name & Address
Architect/Engineer Name & Address Freeman Design
Mortgage Lenders Name & Address
Circle the correct power company – FL Power & Light – Clay Elec – Suwannee Valley Elec. – Progress Energy
10 11 17 22511 000
Property ID Number 19-45-17-08 566 = Bot Estimated Cost of Construction 100,000
Subdivision NameLotBlockUnitPhase
Driving Directions 47 South left on 242 go 1/2 mile
on left across from sw crows farm ter.
Number of Existing Dwellings on Property
Total Morage zor old
Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height 18 8"
Actual Distance of Structure from Property Lines - Front 260 Side 250 Side 429 Rear 1067
Number of Stories Heated Floor Area 1756 Total Floor Area 2764 Roof Pitch 6/12
Application to be a second of the second of

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

Tw Called TENT 1.15.08

Revised 11-30-07

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Fermitee)

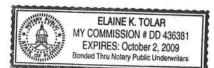
Contractor's License Number KR 282811523 Columbia County Competency Card Number 000141

Affirmed under penalty of perjury to by the Contractor and subscribed before me this

Personally known X

or Produced Identification

State of Florida Notary Signature (For the Contractor)



NOTICE OF COMMENCEMENT

STATE OF: Florida COUNTY OF: Columbia Inst:200812000216 Date:1/7/2008 Time:10:17 AM <u>LLFF</u>DC,P.DeWitt Cason,Columbia County Page 1 of 2

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statues, the following information is provided in this Notice of Legal Attached 1. Description of Property: 2. General Description of Improvement: Construction of Single Family Residence 3. Owner Information: a. Name and Address: <u>James Dwight Pettyjohn</u> 1811 SW County Road 242-A <u>Lake City</u>, Fl Interest in Property: Fee Simple_ b. Name and Address of Fee Simple titleholder (if other than Owner): C. Contractor (Name and Address): Trent Giebeig Construction, Inc. 697 SE Holly Terrace Lake City, Fl. 32025 4 5. Surety: Name and Address: N/A Amount of Bond: ь. 6. Lender (Name and Address): N/A Persons within the State of Florida designated by Owner upon notices or other documents may be 7. Served as provided by 713,13 (I)(a)(7), Florida Statues.

N/A In addition to himself, the Owner designates the following person to recieve a copy of the Lienor's 8. Notice as provided in 713.13 (I)(b), Florida Statues (Name and Address): 9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of Recording unless a different date is specified): Type Owner Name Sworn to and subscribed before me by the Owner (s) on this 32 day of 140. Type Name: ELAINE Notary Public, State of Florida COMMISSION EXPIRY / NUMBER: ELAINE K. TOLAR MY COMMISSION & DD 436381 Personally Known James Produced Identification EXPIRES: Oclober 2, 2009 Inded Ting Noting Public Underwrite Did Take an Oath / Did Not Take an Oath

THIS INSTRUMENT PREPARED BY:

MARLIN M. FEAGLE, ESQUIRE FEAGLE & FEAGLE, ATTORNEYS, P.A. 153 NE Madison Street Post Office Box 1653 Lake City, Florida 32056-1653 (as to form only)

Florida Bar No. 0173248

Inst:2004016340 Date:07/14/2004 Time:15:32

0.70 Doc Stamp-Deed :

DC,P.DeWitt Cason,Columbia County B:1020 P:2464

WARRANTY DEED

THIS INDENTURE, made this 28 day of ____

BILLY N. PETTYJOHN, a married person not residing on the property, whose mailing address is 6960 South U.S. Highway 441, Lake City, Florida 32025; and EUGENE PETTYJOHN and his wife, IMOGENE PETTYJOHN, whose mailing address is 1536 SW Packard Street, Lake City, Florida 32025, parties of the first part, Grantor, and **DWIGHT PETTYJOHN**, whose mailing address is 1811 SW County Road 242, Lake City, Florida 32025, parties of the second part, Grantee,

WITNESSETH:

That said grantor, for and in consideration of the sum of TEN AND NO/100 (\$10.00) DOLLARS, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs, successors and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

A parcel of land lying in the Northeast 1/4 of Section 28, Township 4 South, Range 17 East, Columbia County, Florida, being more particularly described as follows:

Commence at the Northeast corner of Section 28, Township 4 South, Range 17 East, Columbia County, Florida, and run S 88°16'41" W along the North line of said Section 28 a distance of 184.08 feet to a line being parallel with the East line of said Section 28; thence run S 01°10'50" E along said parallel line 923.18 feet to the POINT OF BEGINNING; thence continue S 01°10'50" E still along said parallel line 1165.95 feet to the South line of a parcel of land as described in Official Records Book 775, Page 2237, as recorded in the public records of Columbia County, Florida; thence run S 87°06'23" W along said South line 1527.61 feet to the East right-of-way line of State Road Number 25 (U.S. Highway Number 441); thence run N 30°00'37" W along said East right-of-way line 15.83 feet to the South line of a parcel of land as described in Official Records Book 775, Page 2238, as recorded in the public records of Columbia County, Florida; thence run N 85°58'18" E along said South line 348.54 feet to the East line of said parcel of land described in Official Records Book 775, Page 2238, as recorded in the public records of Columbia County, Florida; thence run N 30°03'51" W along said East line 798.52 feet to the North line of a parcel of land as described in Official Records Book 775, Page 2238, as recorded in the public records of Columbia County, Florida; thence run S 86°34'29" W along said North line 349.52 feet to the East right-of-way line of State Road Number 25 (U.S. Highway Number 441); thence run N 30°00'37" W along said East right-of-way line 794.87 feet to a line being parallel with the North line of said Section 28; thence run N 88°16'41" E along said parallel line 734.96 feet; thence run S 01°43'19" E a distance of 223.19 feet to a line being parallel with the North line of said Section 28; thence run N 88°16'41" E along said parallel line 1567.74 feet to the **POINT OF BEGINNING**. Containing 48.15 acres, more or less.

AND

A parcel of land lying in the SE 1/4 of Section 19, Township 4 South, Range 17 East, Columbia County, Florida, being more particularly described as follows:

Commence at the Southeast corner of Section 19, Township 4 South, Range 17 East, Columbia County, Florida, and run N 00°32'55" W along the East line of said Section 19 a distance of 43.61 feet to a point on the arc of a curve concave to the North having a radius of 17148.73 feet and central angle of 00°04'23", being on the North right-of-way line of County Road Number 242; thence run Westerly along said arc of curve, also still along said North right-of-way line 21.85 feet to the Point of Tangency of said curve; thence run N 89°08'24" W still along said North right-of-way line 824.01 feet to a line being parallel with the East line of said Section 19, also being the **POINT OF BEGINNING**; thence continue N 89°08'24" W still along said North right-of-way line 739.91 feet; thence run N

Inst:2004016340 Date:07/14/2004 Time:15:32

Doc Stamp-Deed: 0.70

____DC,P.DeWitt Cason,Columbia County B:1020 P:2466

00°48'01" W a distance of 1361.76 feet to the South line of Pine Knoll, a subdivision recorded in Plat book 3, Page 100 of the public records of Columbia County, Florida; thence run N 89°34'02" E along said South line 420.76 feet to the Southeast corner of said subdivision; thence run S 01°16'21" W a distance of 251.53 feet; thence run S 87°55'00" E a distance of 328.86 feet to said line being parallel with the East line of said Section 19; thence S 00°32'55" E along said parallel line 1156.50 feet to the **POINT OF BEGINNING**. Containing 21.42 acres, more or less.

N.B. No portion of the above described property constitutes the constitutional homestead of Grantor, Billy N. Pettyjohn.

Tax Parcel No.: 28-4S-17-08833-000 and 19-4S-17-08566-000

TOGETHER WITH all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

AND the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2003.

Inst:2004016340 Date:07/14/2004 Time:15:32 DC,P.DeWitt Cason,Columbia County B:1020 P:2467 Doc Stamp-Deed :

(SEAL)

IN WITNESS WHEREOF, the said Grantors have signed and sealed these presents the

day and year first above written.

Signed, sealed and delivered in the presence of:

Witness

Maerin feagle

Print or type name

DIANES. EDENFIELD

Print or type name

Signed, sealed and delivered in the presence of:

Print or type name

DIANES. EDENFIELD

Print or type name

DC,P.DeWitt Cason,Columbia County B:1020 P:2468 STATE OF FLORIDA COUNTY OF COLUMBIA The foregoing instrument was acknowledged before me this 28th day of , 2004, by BILLY N. PETTYJOHN who is personally known to me or who has produced a Florida driver's license as identification. MY COMMISSION # DD112002 EXPIRES Notary Public, State of Florida May 26, 2006 BONDED THRU TROY FAIN INSURANCE, INC. My Commission Expires: SEAL) STATE OF FLORIDA COUNTY OF COLUMBIA The foregoing instrument was acknowledged before me this 28, 2004, by EUGENE PETTYJOHN and his wife, IMOGENE PETTYJOHN, who are personally known to me or who have produced Florida driver's licenses as identification.

Notary Public, State of Florida

My Commission Expires:

Diane S. Edenfield

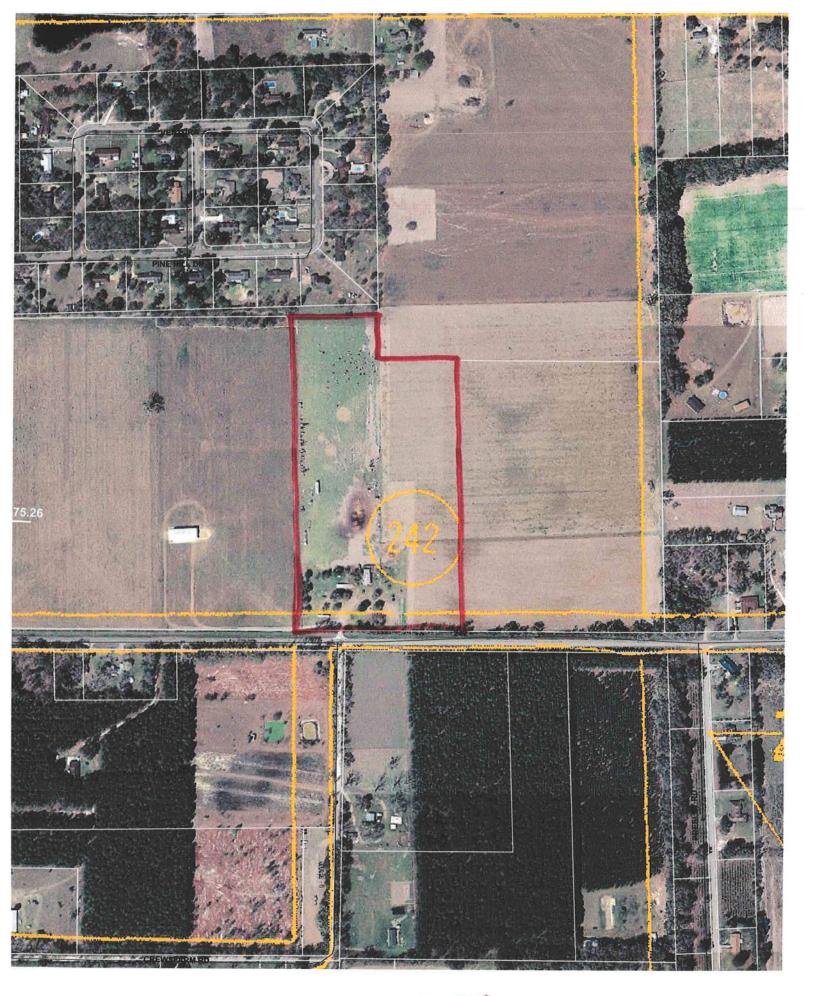
Y COMMISSION # DD112002 EXPIRES May 26, 2006 BONDED THRU TROY FAIN INSURANCE, INC.

(NOTAF

Inst:2004016340 Date:07/14/2004 Time:15:32

0.70

Doc Stamp-Deed :



0801-41

Columbia County Property Appraiser DB Last Updated: 11/15/2007

2008 Proposed Values

Tax Record

Property Card

Interactive GIS Map

Search Result: 1 of 1

Print

Parcel: 19-4S-17-08566-000

Owner & Property Info

Owner					
Owner	's	Na	me	PE	TTYJOH

Owner's Name	PETTYJOHN D	PETTYJOHN DWIGHT						
Site Address								
Mailing Address	1811 SW CR LAKE CITY, F							
Use Desc. (code)	PASTURELAN (006200)							
Neighborhood	19417.00	Tax District	2					
UD Codes	МКТА06	Market Area	06					
Total Land Area	21.420 ACRE	S						
Description	FT, E 339.24	OF SEC, RUN W 1507.4 FT, S 182 FT, E 1132.5 X 21.42 AC DESC IN O	6 FT, S 1138					

GIS Aerial



Property & Assessment Values

Total Appraised Value		\$3,855.00
XFOB Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
Ag Land Value	cnt: (1)	\$3,855.00
Mkt Land Value	cnt: (0)	\$0.00

\$128,520.00			
\$3,855.00			
\$3,855.00			
\$0.00			
\$3,855.00			

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price			
NONE									

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
			NONE			***************************************

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
				NONE		

Land Breakdown

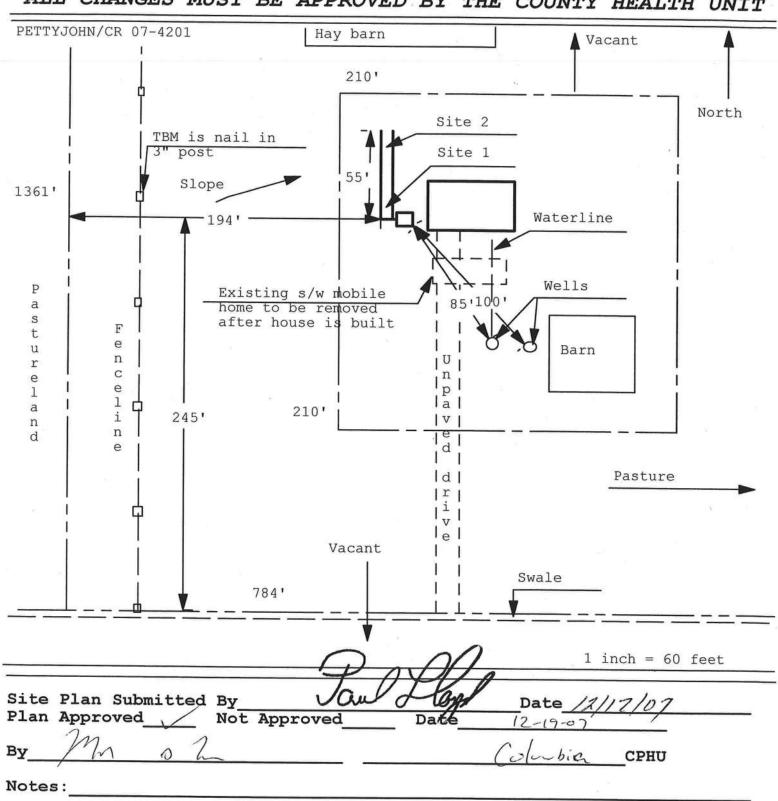
Lnd Code	e Desc Units		Adjustments	Eff Rate	Lnd Value	
006200	PASTURE 3 (AG)	21.420 AC	1.00/1.00/1.00/1.00	\$180.00	\$3,855.00	
009910	MKT.VAL.AG (MKT)	21.420 AC	1.00/1.00/1.00/1.00	\$0.00	\$128,520.00	

Columbia County Property Appraiser

DB Last Updated: 11/15/2007

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



p.2

Water Wells Pumps & Service Phone: (386) 752-6677 Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place Lake City, FL 32025 www.lynchwelldrilling.com

November 6, 2007

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the above-referenced well:

Size of Pump Motor:

1 Horse Power

Size of Pressure Tank:

81-Gallon Bladder Tank

Cycle Stop Valve Used:

No

Should you require any additional information, please contact us.

Sincerely,

Linda Newcomb

Lynch Well Drilling, Inc.

Linda Newcomb

Project Name:

Address:

DATE:

Pettyjohn Residence

T. Geibeig

Lake City

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Builder:

Permitting Office:

	∟ake City, FI			Permit Number:	
Owner:				Jurisdiction Number:	22/000
Climate Zone: N	North				
New construction or a	existing	New	12.	Cooling systems	
2. Single family or mult	i-family	Single family		Central Unit	Cap: 32.0 kBtu/hr
3. Number of units, if m		1	1		SEER: 13.00
Number of Bedrooms		3	h	N/A	SEEK. 15.00
5. Is this a worst case?	2	Yes		37.4.4	-
Conditioned floor are	a (fl²)	1756 ft²		N/A	_
7. Glass type 1 and area:				10/4	_
a. U-factor:			12	Heating austoms	_
(or Single or Double	DEEAULT) 70 CD	escription Area		Heating systems	C 22.01 D: #
b. SHGC:	DEFAULT) /a.(Db	le Default) 140.0 ft	a.	Electric Heat Pump	Cap: 32.0 kBtu/hr
	TAILT) 51			NT.	HSPF: 8.50
(or Clear or Tint DE	FAULT) 7b.	(Clear) 140.0 ft ²	b.	N/A	
8. Floor types	mention with the control	CALCO HAR HAR FOR HAR HAR HAR HAR HAR			_
a. Slab-On-Grade Edge	Insulation	R=0.0, 166.0(p) ft	c.	N/A	<u> </u>
b. N/A		_			<u> 22 - 2</u> 1
c. N/A		_	14.	Hot water systems	
Wall types			a.	Electric Resistance	Cap: 20.0 gallons
a. Frame, Wood, Exterio	or	R=13.0, 1556.0 ft ²			EF: 0.94
b. N/A			b.	N/A	5-22-500 (1994) AMARIN (1994)
c. N/A		_	53000		_
d. N/A		_	c.	Conservation credits	
e. N/A		_	2000	(HR-Heat recovery, Solar	_
10. Ceiling types		_	1	DHP-Dedicated heat pump)	
a. Under Attic		R=30.0, 1756.0 ft ²	15	HVAC credits	PT, CF,
b. N/A		10 50.0, 1750.0 It	0255035 2	(CF-Ceiling fan, CV-Cross ventilation	
c. N/A		_		HF-Whole house fan,	11,
11. Ducts		_			
	AII. Intonion	S D-60 660 A		PT-Programmable Thermostat,	
a. Sup: Con. Ret: Con.	Ari: interior	Sup. R=6.0, 66.0 ft		MZ-C-Multizone cooling,	
b. N/A		_		MZ-H-Multizone heating)	
		_			
		Total as-built p	oints:	18971	
Glass/F	loor Area: 0.08	Total base p			S
		i otal base p	Oli its.	24960	
I hereby certify that the	plans and specific	ations covered by	Rev	riew of the plans and	
this calculation are in co	ompliance with the	Florida Energy		cifications covered by this	OF THE STATE
Codo	(4)	0,		culation indicates compliance	STATE OF THE
PREPARED BY:	John 411	otes	with	the Florida Energy Code.	12 8
				ore construction is completed	E E
DATE: 12-2	24-07			building will be inspected for	13 1
I hereby certify that this	building, as desig	ned, is in compliance		pliance with Section 553.908	
with the Florida Energy				ida Statutes.	NA TOUS A
	and and the thirt		CMP15112		OD WE THE
OWNER/AGENT:			BU	ILDING OFFICIAL:	

DATE:

PERMIT #:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI,

BASE	AS-BUILT					
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area		verhang t Len Hgt	Area X S	РМ Х	SOF	= Points
.18 1756.0 18.59 5876.0	2.Double, Clear 3.Double, Clear 4.Double, Clear 5.Double, Clear	1.0 6.0 MW 1.0 6.0 ME 1.0 6.0 MW 1.0 MW 1	0 15.0 0 15.0 0 60.0 0 36.0	25.97 25.97 29.56 42.75 42.75 40.16	0.97 0.97 0.97 0.96 0.42 0.96	227.0 379.0 431.0 2469.0 643.0 193.0
WALL TYPES Area X BSPM = Points	Туре	R-Value		X SPIV	l =	Points
Adjacent 0.0 0.00 0.0 Exterior 1556.0 1.70 2645.2	1. Frame, Wood, Exterior	13.0	1556.0	1.50		2334.0
Base Total: 1556.0 2645.2	As-Built Total:		1556.0			2334.0
DOOR TYPES Area X BSPM = Points	Туре		Area >	K SPM	=	Points
Adjacent 0.0 0.00 0.0 Exterior 60.0 6.10 366.0	1.Exterior Insulated 2.Exterior Insulated		20.4 39.6	4.10 4.10		83.6 - 162.4
Base Total: 60.0 366.0	As-Built Total:		60.0			246.0
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X SPI	M X SC	M =	Points
Under Attic 1756.0 1.73 3037.9	1. Under Attic	30.0	1756.0 1.73	3 X 1.00		3037.9
Base Total: 1756.0 3037.9	As-Built Total:		1756.0			3037.9
FLOOR TYPES Area X BSPM = Points	Туре	R-Value	e Area X	SPM	=	Points
Slab 166.0(p) -37.0 -6142.0 Raised 0.0 0.00 0.0	Slab-On-Grade Edge Insulation	0.0	166.0(p	-41.20		-6839.2
Base Total: -6142.0	As-Built Total:		166.0			-6839.2
INFILTRATION Area X BSPM = Points			Area X	SPM	=	Points
1756.0 10.21 17928.8			1756.0	10.21	(a-1)	17928.8

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI, PERMIT #:

BASE			AS-BUILT					
Summer Ba	se Points: 2	23711.8	Summer As-Built Points:	21049.4				
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Cooling Points				
23711.8	0.3250	7706.3	(sys 1: Central Unit 32000btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Int(AH),R6.0(INS) 21049 1.00 (1.00 x 1.147 x 0.91) 0.260 0.902 21049.4 1.00 1.044 0.260 0.902	5155.4 5155.4				

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI,

PERMIT #:

BASE		AS-BUILT								
GLASS TYPES .18 X Conditioned X BWPM = P Floor Area	oints	Type/SC		hang Len	Hgt	Area X	WF	РМ Х	WOF	= Points
.18 1756.0 20.17	6375.0	1.Double, Clear	NW	1.0	6.0	9.0	2	4.30	1.00	218.0
		2.Double, Clear	NW	1.0	6.0	15.0	2	4.30	1.00	364.0
1		3.Double, Clear	NE	1.0	6.0	15.0		3.57	1.00	353.0
		4.Double, Clear	SE	1.0	6.0	60.0		4.71	1.04	914.0
1		5.Double, Clear	SE	10.0	6.0	36.0		4.71	2.38	1258.0
		6.Double, Clear	SW	1.0	6.0	5.0	70	6.74	1.02	85.0
		As-Built Total:				140.0				3192.0
WALL TYPES Area X BWPM =	Points	Туре		R-	√alue	Area	Х	WPM	=	Points
Adjacent 0.0 0.00	0.0	1. Frame, Wood, Exterior			13.0	1556.0		3.40		5290.4
Exterior 1556.0 3.70	5757.2									
NAME OF STREET STREET										
Base Total: 1556.0	5757.2	As-Built Total:				1556.0				5290.4
DOOR TYPES Area X BWPM =	Points	Туре				Area	Х	WPM	=	Points
Adjacent 0.0 0.00	0.0	1.Exterior Insulated				20.4		8.40		171.4
Exterior 60.0 12.30	738.0	2.Exterior Insulated				39.6		8.40		332.6
Base Total: 60.0	738.0	As-Built Total:				60.0				504.0
CEILING TYPES Area X BWPM =	Points	Туре	R-	√alue	Ar	ea X W	PM :	x wc	M =	Points
Under Attic 1756.0 2.05	3599.8	1. Under Attic		3	80.0	1756.0 2	.05 X	(1.00		3599.8
Base Total: 1756.0	3599.8	As-Built Total:				1756.0				3599.8
FLOOR TYPES Area X BWPM =	Points	Туре		R-\	/alue	Area	Х	WPM	=	Points
Slab 166.0(p) 8.9	1477.4	1. Slab-On-Grade Edge Insu	lation		0.0	66.0(p	50	18.80		3120.8
Raised 0.0 0.00	0.0							- Control Control		
Base Total:	1477.4	As-Built Total:				166.0				3120.8
INFILTRATION Area X BWPM =	Points					Area :	X	WPM	=	Points
1756.0 -0.59	-1036.0					1756.0		-0.59		-1036.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI, PERMIT #:

BASE			AS-BUILT						
Winter Base	Points:	16911.4	Winter As-Built Points:	14671.0					
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	Heating Points					
16911.4	0.5540	9368.9	(sys 1: Electric Heat Pump 32000 btuh ,EFF(8.5) Ducts:Con(S),Con(R),Int(A 14671.0 1.000 (1.000 x 1.169 x 0.93) 0.401 0.950 14671.0 1.00 1.087 0.401 0.950	H),R6.0 6078.8 6078.8					

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI, PERMIT #:

BASE			AS-BUILT									
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier :	X Credit :	
3		2635.00		7905.0	20.0	0.94	3		1.00	2578.94	1.00	7736.8
				5.3	As-Built To	otal:						7736.8

	CODE COMPLIANCE STATUS												
BASE				AS-BUILT									
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
7706		9369		7905		24980	5155		6079		7737		18971

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FI, PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK		
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.			
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.			
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.			
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.			
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.			
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.			

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.8

The higher the score, the more efficient the home.

, , Lake City, FI,

	New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms	New Single family 1 3	= '	Cooling systems a. Central Unit b. N/A	Cap: 32.0 kBtu/hr SEER: 13.00	_
5.6.7.	Is this a worst case? Conditioned floor area (ft²) Glass type ¹ and area: (Label reqd.	$Yes \\ 1756 ft^2$ by 13-104.4.5 if not default)		N/A		_
	U-factor: (or Single or Double DEFAULT) SHGC:	Description Area 7a. (Dble Default) 140.0 ft ²	_ *	Heating systems . Electric Heat Pump	Cap: 32.0 kBtu/hr HSPF: 8.50	_
	(or Clear or Tint DEFAULT) Floor types Slab-On-Grade Edge Insulation	7b. (Clear) 140.0 ft ² R=0.0, 166.0(p) ft		o. N/A o. N/A		_
c. 9.	N/A N/A Wall types		_ _ 14.	Hot water systems . Electric Resistance	Cap: 20.0 gallons	_
b. c. d.	Frame, Wood, Exterior N/A N/A N/A N/A	R=13.0, 1556.0 ft ²	_	. N/A . Conservation credits (HR-Heat recovery, Solar	EF: 0.94	
10. a. b. c.	Ceiling types Under Attic N/A N/A	R=30.0, 1756.0 ft ²	_ 15. 	DHP-Dedicated heat pump) HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan,	PT, CF,	_
a.	Ducts Sup: Con. Ret: Con. AH: Interior N/A	Sup. R=6.0, 66.0 ft	_	PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		
Cons in th	tify that this home has complie struction through the above end is home before final inspection d on installed Code compliant	ergy saving features which n. Otherwise, a new EPL I	h will be in	stalled (or exceeded)	OF THE STAIR	A CANA
	der Signature:		Date:	F	O CO	RUDA
Addı	ress of New Home:		City/FL Z	ip:	GOD WE TRUST	9

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCPB v4.5.2)

BUILDING INPUT SUMMARY REPORT

FLOORS PROJECT	Title: Pettyjohn F Owner: (blank) # of Units: 1 Builder Name: T. Geibeig Climate: North Permit Office: Lake City Jurisdiction #: (blank) # Floor Type 1 Slab-On-Grade Edge Insulation	New/Existing: Bedrooms: Conditioned A Total Stories: Worst Case: Rotate Angle:	3 Subdivision:	N/A N/A N/A (blank) (blank) Lake City, FI,
CEILINGS	# Ceiling Type 1 Under Attic Credit Multipliers: None	R-Val Area Base Area Units 30.0 1756.0 ft² 1756.0 ft² 1	# System Type 1 Central Unit Credit Multipliers: Ceil Fn, PT	Efficiency Capacity SEER: 13.00 32.0 kBtu/hr
WALLS	# Wall Type 1 Frame - Wood	Location R-Val Area Units Exterior 13.0 1556.0 ft ² 1	# System Type 1 Electric Heat Pump Credit Multipliers: PT	Efficiency Capacity HSPF: 8.50 32.0 kBtu/hr
	# Panes Tint Ornt 1 Double Clear N 2 Double Clear N 3 Double Clear E 4 Double Clear S 5 Double Clear S 6 Double Clear W	Area OH Length 9.0 ft ² 1.0 ft 6.0 ft 1 6.0 ft 1 15.0 ft ² 1.0 ft 6.0 ft 1 15.0 ft ² 1.0 ft 6.0 ft 1 30.0 ft ² 1.0 ft 6.0 ft 2 36.0 ft ² 10.0 ft 6.0 ft 1 5.0 ft ² 1.0 ft 6.0 ft 1	# Supply Return Location 1 Cond. Cond. Interior Credit Multipliers: None # System Type EF Cap 1 Electric Resistance 0.94 20.6	6.0 66.0 ft Conservation Type Con. EF
WINDOWS			# Use Default? Annual Opt	erating Cost Electric Rate N/A
MISC		TOTAL CONTRACTOR TO THE STATE OF THE STATE O	nnects: N/A stem Proposed: No resent?: No	Pool Size: 0 Pump Size: 0.00 hp Dryer Type: Electric Stove Type: Electric Avg Ceil Hgt:

EnergyGauge® (Version: FLRCPB v4.5.2)

Residential System Sizing Calculation

Summary Project Title:

Project Title: Pettyjohn Residence Code Only Professional Version Climate: North

Lake City, FI

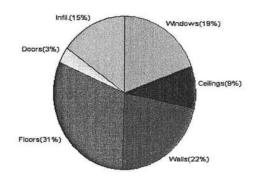
12/31/2007

				1-1011-0	• •
Location for weather data: Gaine	sville - Def	aults: Latitu	ude(29) Altitude(152 ft.) Temp Ran	ge(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)		
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	23125	Btuh	Total cooling load calculation	18064	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	138.4	32000	Sensible (SHR = 0.75)	148.8	24000
Heat Pump + Auxiliary(0.0kW)	138.4	32000	Latent	412.3	8000
			Total (Electric Heat Pump)	177.1	32000

WINTER CALCULATIONS

Winter Heating Load (for 1756 sqft)

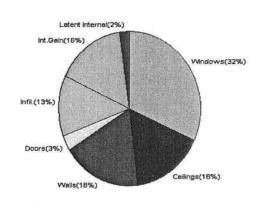
Load component			Load	
Window total	140	sqft	4507	Btuh
Wall total	1556	sqft	5110	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	1756	sqft	2069	Btuh
Floor total	166	sqft	7248	Btuh
Infiltration	84	cfm	3414	Btuh
Duct loss			0	Btuh
Subtotal		- 1	23125	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			23125	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1756 sqft)

Load component			Load	
Window total	140	sqft	5738	Btuh
Wall total	1556	sqft	3246	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	1756	sqft	2908	Btuh
Floor total		70	0	Btuh
Infiltration	42	cfm	784	Btuh
Internal gain			2860	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain		- 1	16124	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			1540	Btuh
Latent gain(ventilation)		- 1	0	Btuh
Latent gain(internal/occu	pants/othe	r)	400	Btuh
Total latent gain			1940	Btuh
TOTAL HEAT GAIN			18064	Btuh



Version 8
For Florida residences only

EnergyGauge® System Sizing	
EnergyGauge® System Sizing PREPARED BY: (SULLI) (MOH)	
DATE:	_

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title: Pettyjohn Residence

Code Only

Professional Version

Climate: North

Lake City, FI

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

This calculation is for Worst Case. The house has been rotated 225 degrees.

12/31/2007

Cam	nonené	I aada	Sau Milla	I- 11
COIII	houeur	Loads	TOL AAUO	le House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	SW	9.0	32.2	290 Btuh
2	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
3	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	NE	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	NE	36.0	32.2	1159 Btuh
6	2, Clear, Metal, 0.87	SE	5.0	32.2	161 Btuh
	Window Total		140(sqft)		4507 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1556	3.3	5110 Btuh
	Wall Total		1556	*SANSONO	5110 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	264 Btuh
2	Insulated - Exterior		40	12.9	513 Btuh
	Door Total		60	MORNEY.	777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1756	1.2	2069 Btuh
	Ceiling Total		1756		2069Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	166.0 ft(p)	43.7	7248 Btuh
	Floor Total		166		7248 Btuh
			Envelope Su	btotal:	19710 Btuh
Infiltration	Туре	ACH X Volu	ıme(cuft) walls(sqft) CFM=	
Stational, including a fundamental of	Natural	0.32	15804 1556	84.3	3414 Btuh
Ductload			(DI	_M of 0.000)	0 Btuh
All Zones		Sens	ible Subtotal All	Zones	23125 Btuh

WHOLE HOUSE TOTALS		
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	23125 Btuh 0 Btuh 23125 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Lake City, FI

Pettyjohn Residence

Code Only Professional Version Climate: North

12/31/2007

EQUIPMENT		
Electric Heat Pump	#	32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Lake City, FI

Code Only
Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 225 degrees.

12/31/2007

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	SW	9.0	32.2	290 Btuh
2	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
3	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	NE	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	NE	36.0	32.2	1159 Btuh
6	2, Clear, Metal, 0.87	SE	5.0	32.2	161 Btuh
	Window Total		140(sqft)		4507 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1556	3.3	5110 Btuh
	Wall Total		1556	CHANNE I	5110 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	264 Btuh
2	Insulated - Exterior		40	12.9	513 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1756	1.2	2069 Btuh
	Ceiling Total		1756		2069Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	166.0 ft(p)	43.7	7248 Btuh
	Floor Total		166		7248 Btuh
		z	one Envelope Su	ototal:	19710 Btuh
Infiltration	Туре	ACH X Volu	ime(cuft) walls(sqft)	CFM=	
	Natural	0.32	15804 1556	84.3	3414 Btuh
Ductload	Average sealed, Supply(R6.0-Cond.), Return(R6.0-Cond)DLM of 0.000) 0 Btuh				
Zone #1		23125 Btuh			

WHOLE HOUSE TOTA	8	
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	23125 Btuh 0 Btuh 23125 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Lake City, FI

Project Title: Pettyjohn Residence Code Only Professional Version Climate: North

12/31/2007

EQUIPMENT		
Electric Heat Pump	#	32000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal) (U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title: Pettyjohn Residence

Code Only Professional Version

Lake City, FI

Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F This calculation is for Worst Case. The house has been rotated 225 degrees.

12/31/2007

Component Loads for Whole House

	Type*	Overhang		Window Area(sqft)		HTM		Load			
Window	Pn/SHGC/U/InSh/ExSh/IS	Omt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, B-D, N,F	SW	1ft.	6ft.	9.0	0.0	9.0	19	43	391	Btul
2	2, Clear, 0.87, B-D, N,F	SW	1ft.	6ft.	15.0	2.0	13.0	19	43	601	Btul
3	2, Clear, 0.87, B-D, N,F	NW	1ft.	6ft.	15.0	0.0	15.0	19	41	612	Btul
4	2, Clear, 0.87, B-D, N,F	NE	1ft.	6ft.	60.0	0.0	60.0	19	41	2447	Btul
5	2, Clear, 0.87, B-D, N,F	NE	10ft.	6ft.	36.0	0.0	36.0	19	41	1468	Btul
6	2, Clear, 0.87, B-D, N,F	SE	1ft.	6ft.	5.0	0.0	5.0	19	43	217	Btul
	Window Total				140 (sqft)				5738	Btul
Walls	Туре	R-Value/U-Val		-Value	Area	(sqft)		НТМ	Load	V	
1	Frame - Wood - Ext			13.0/0	0.09	1556.0			2.1	3246	Btuh
	Wall Total				1556 (sqft)				3246	Btuh	
Doors	Туре				Area (sqft)		HTM		Load		
1	Insulated - Exterior		5			20	.4		9.8	200	Btuh
2	Insulated - Exterior					39	.6		9.8	388	
	Door Total				60 (sqft)			588	Btuh		
Ceilings	Type/Color/Surface		R-Va	lue		Area(sqft)			HTM	Load	
1	Vented Attic/DarkShingle			30.0		1756.0		756.0 1.7		2908	Btuh
	Ceiling Total					1756 (sqft)				2908	
Floors	Туре		R-Va	lue		Size		HTM		Load	
1	Slab On Grade	0.0				166 (ft(p))			0.0	0	Btuh
	Floor Total						0 (sqft)			0	Btuh
						Er	velope S	Subtotal:		12479	Btuh

Residential Load - Component Details (continued) Project Title: Code Only Pettyjohn Residence Professional Version

Climate: North

Lake City, FI

12/31/2007

Infiltration	Type SensibleNatural	ACH 0.16	Volume(cu		l area(sq 1556	ft) CFM= 84.3	Load 784	Btuh
Internal gain		Occupants 2	Btu X	h/occu 230	pant +	Appliance 2400	Load 2860	Btuh
				Sens	sible Enve	elope Load:	16124	Btuh
Duct load					(DGM o	f 0.000)	0	Btuh
				Sensil	ble Load	All Zones	16124	Btuh

Residential Load - Component Details (continued)
Project Title: Code Only

Project Title: Pettyjohn Residence

Professional Version Climate: North

Lake City, FI

12/31/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	16124	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	16124	Btuh
¥:	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	16124	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	1540	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (2 people @ 200 Btuh per person)	400	Btuh
	Latent other gain	0	Btuh
	Latent total gain	1940	Btuh
	TOTAL GAIN	18064	Btuh

EQUIPMENT		
1. Central Unit	#	32000 Btuh

Residential Load - Component Details (continued)

Project Title: Pettyjohn Residence

Code Only Professional Version Climate: North

Lake City, FI

12/31/2007

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details Project Title: Code C

Pettyjohn Residence

Professional Version Climate: North

Lake City, FI

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F This calculation is for Worst Case. The house has been rotated 225 degrees.

12/31/2007

Component Loads for Zone #1: Main

	Type*		Over	hang	Window Area(sqft)		HTM		Load		
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, B-D, N,F	SW	1ft.	6ft.	9.0	0.0	9.0	19	43	391	Btuh
2	2, Clear, 0.87, B-D, N,F	SW	1ft.	6ft.	15.0	2.0	13.0	19	43	601	Btuh
3	2, Clear, 0.87, B-D, N,F	NW	1ft.	6ft.	15.0	0.0	15.0	19	41	612	Btuh
4	2, Clear, 0.87, B-D, N,F	NE	1ft.	6ft.	60.0	0.0	60.0	19	41	2447	Btuh
5	2, Clear, 0.87, B-D, N,F	NE	10ft.	6ft.	36.0	0.0	36.0	19	41	1468	Btuh
6	2, Clear, 0.87, B-D, N,F	SE	1ft.	6ft.	5.0	0.0	5.0	19	43		Btuh
	Window Total				140 (sqft)					5738 Btuh	
Walls	Type		R-Va	alue/U	I-Value	/alue Area(sqft)			HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	155	6.0		2.1	3246	Btuh
	Wall Total					155	6 (sqft)			3246	Btuh
Doors	Туре					Area			HTM	Load	
1	Insulated - Exterior					20	3 N N		9.8	200	Btuh
2	Insulated - Exterior					39	.6		9.8	388	Btuh
	Door Total					60 (sqft)					Btuh
Ceilings	Type/Color/Surface		R-Value Area(sqft)			HTM	Load				
1	Vented Attic/DarkShingle			30.0		175	571 105 775		1.7	2908	Btuh
	Ceiling Total					A. (50) 780	6 (sqft)			2908	
Floors	Туре		R-Va	alue	Size			НТМ	Load	Dian	
1	Slab On Grade			0.0			6 (ft(p))		0.0	0	Btuh
	Floor Total			0.0			0 (sqft)		0.0	75	Btuh
	. 100. 1010.										
			es es		Zone Envelope Subtotal:		ibtotal:	12479	Btuh		
nfiltration	Туре		Α	CH	Volum	e(cuft) v	vall area	(saft)	CFM=	Load	
	SensibleNatural			0.16		15804	1556	(1)	42.1	784	Btuh
Internal		(Occup	ants		Btuh/oc	cupant	F	Appliance	Load	
gain				2		X 23	700		2400	2860	Btuh
	Sensible Envelope Load:						16124	Btuh			
Duct load	Average sealed, Supply	(R6.0-0	Cond.), Ret	urn(R6.	0-Cond))	(DGM o	f 0.000)	0	Btuh
	Sensible Zone Load 16124 Btuh						Btuh				

Residential Load - Component Details (continued)

Project Title: Pettyjohn Residence Code Only **Professional Version** Climate: North

12/31/2007

Lake City, FI

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	16124	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	16124	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	16124	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	1540	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (2 people @ 200 Btuh per person)	400	Btuh
	Latent other gain	0	Btuh
	Latent total gain	1940	Btuh
	TOTAL GAIN	18064	Btuh

EQUIPMENT							
1. Central Unit	#	32000 Btuh					

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

Project Title: Pettyjohn Residence

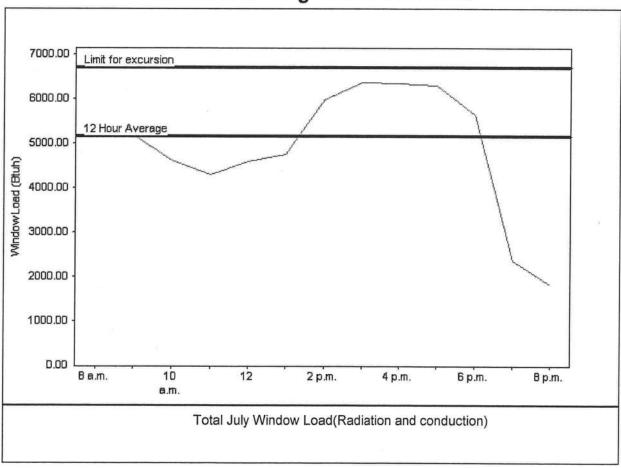
Code Only Professional Version Climate: North

12/31/2007

Lake City, FI

Weather data for: Gainesville - Defaults							
Summer design temperature	92 F	Average window load for July	5164 Btuh				
Summer setpoint	75 F	Peak window load for July	6375 Btuh				
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	6713 Btuh				
Latitude	29 North	Window excursion (July)	None				

WINDOW Average and Peak Loads



The midsummer window load for this house does not exceed the window load excursion limit. This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: Delelue SM Otto

DATE: 12

X-04-07

ACCA HRIVURL

EnergyGauge® FLRCPB v4.5.2



BUILDING CODE COMPLIANCE OFFICE (BCCO) PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Tamko Roofing Products, Inc. P.O. Box 1404 **Joplin, MO 64802**

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: TAMKO Heritage Declaration & Heritage XL Roof Shingles

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This consists of pages 1 through 4.

The submitted documentation was reviewed by Frank Zuloaga, RRC



NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03

Page 1 of 4

ROOFING ASSEMBLY APPROVAL

Category:

Roofing

Sub-Category:

07310 Composition Shingles

Materials

Dimensional

Deck Type:

Wood

1. Scope:

This approves Tamko Heritage Declaration and Heritage XL Asphalt Shingles, manufactured by Tamko Roofing Products, Inc. as described in this Notice of Acceptance.

2. PRODUCT DESCRIPTION

Product	Dimensions	Test Specifications	Product Description
Heritage Declaration & Heritage XL	12" x 36"		A heavy weight dimensional asphalt shingle.

3. EVIDENCE SUBMITTED:

Test Agency	Test Identifier	Test Name/Report	Date
PRI Asphalt Technologies, Inc.	TAS 100	TAP-066-02-01 TAP-073-02-01	01/09/03 05/20/03
Underwriters Laboratories, Inc. Underwriters Laboratories, Inc.	ASTM D 3462 TAS 107	R2919 03CA08442	06/12/03 06/12/03

4. LIMITATIONS

- 4.1 Fire classification is not part of this acceptance; refer to a current Approved Roofing Materials Directory for fire ratings of this product.
- 4.2 Shall not be installed on roof mean heights in excess of 33 ft.
- 4.3 All products listed herein shall have a quality assurance audit in accordance with the Florida Building Code and Rule 9B-72 of the Florida Administrative Code.

5. INSTALLATION

- 5.1 Shingles shall be installed in accordance with Roofing Application Standard RAS 115.
- 5.2 The manufacturer shall provide clearly written application instructions.
- 5.3 Exposure and course layout shall be in compliance with Detail 'A', attached.
- 5.4 Nailing shall be in compliance with Detail 'B', attached.

6. LABELING

5.1 Shingles shall be labeled with the Miami-Dade Logo or the wording "Miami-Dade County-Product Control Approved".

7. BUILDING PERMIT REQUIREMENTS

- 7.1 Application for building permit shall be accompanied by copies of the following:
 - 7.1.1 This Notice of Acceptance.
 - 7.1.2 Any other documents required by the Building Official or the applicable Building Code in order to properly evaluate the installation of this system.

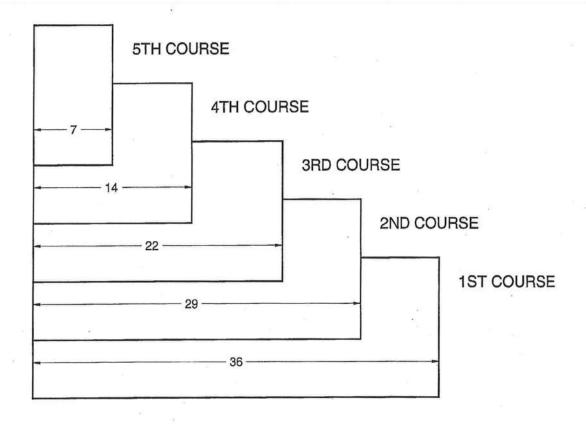


NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03 Page 2 of 4

DETAIL A

HERITAGE DECLARATION & XL

All dimensions are in inches.





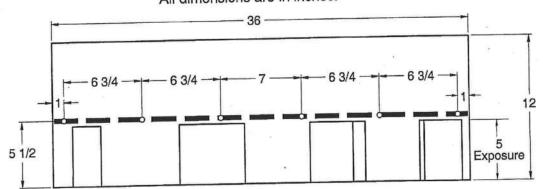
NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03

Page 3 of 4

DETAIL B

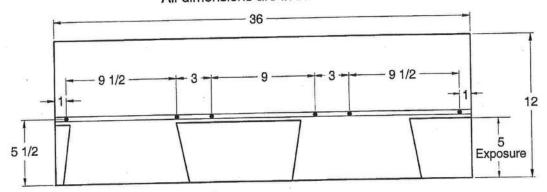
HERITAGE DECLARATION

12" x 36" LAMINATED SHINGLE All dimensions are in inches.



HERITAGE XL

12" x 36" LAMINATED SHINGLE All dimensions are in inches.



END OF THIS ACCEPTANCE



NOA No.: 03-0620.01 Expiration Date: 09/04/08 Approval Date: 09/04/03

Page 4 of 4



MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation 108 Mutzfeld Road Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by the BCCO and accepted by the Building Code and Product Review Committee (BCPRC) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The BCCO (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BCPRC reserves the right to revoke this acceptance, if it is determined by BCCO that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the South Florida Building Code, 1994 Edition for Miami-Dade County or Florida Building Code.

DESCRIPTION: Outswing Glazed Residential Steel Door w\Sidelites

APPROVAL DOCUMENT: Drawing No. S-2003, titled "Therma-Tru Wood edge Outswing", sheets 1 through 6 to 6, prepared by RW Consulting, dated 3/9/01, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

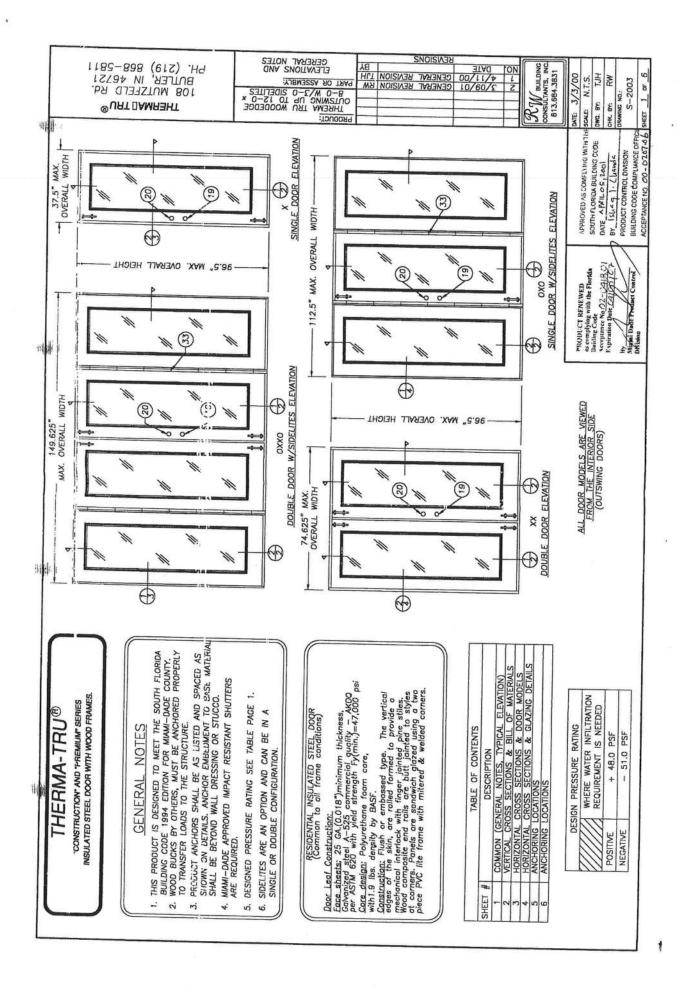
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

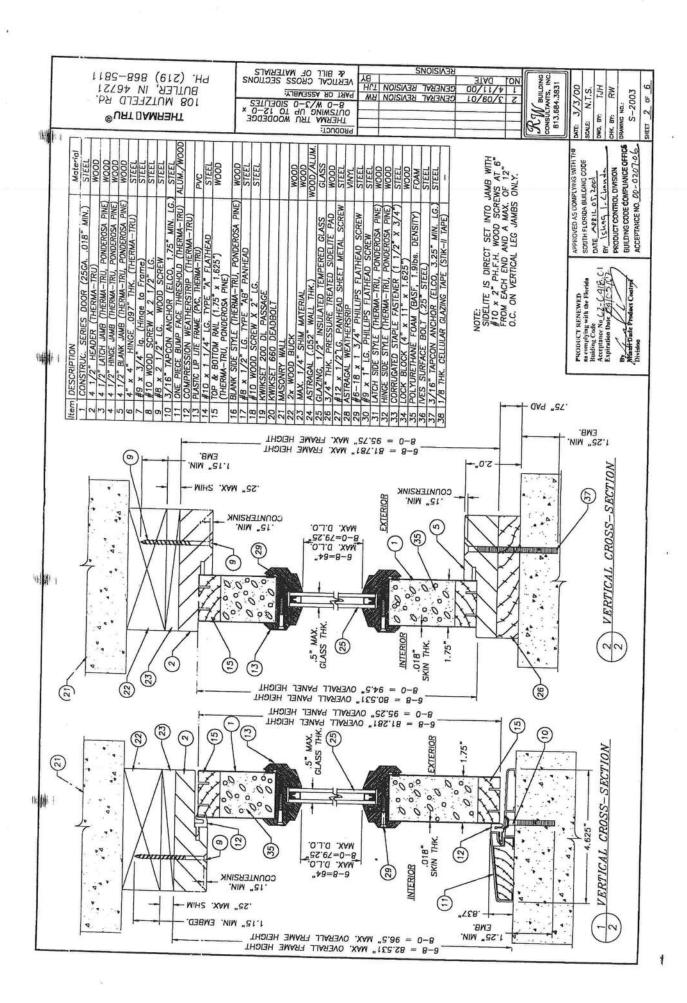
This NOA renews NOA # 00-0207.06 and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Raul Rodriguez.

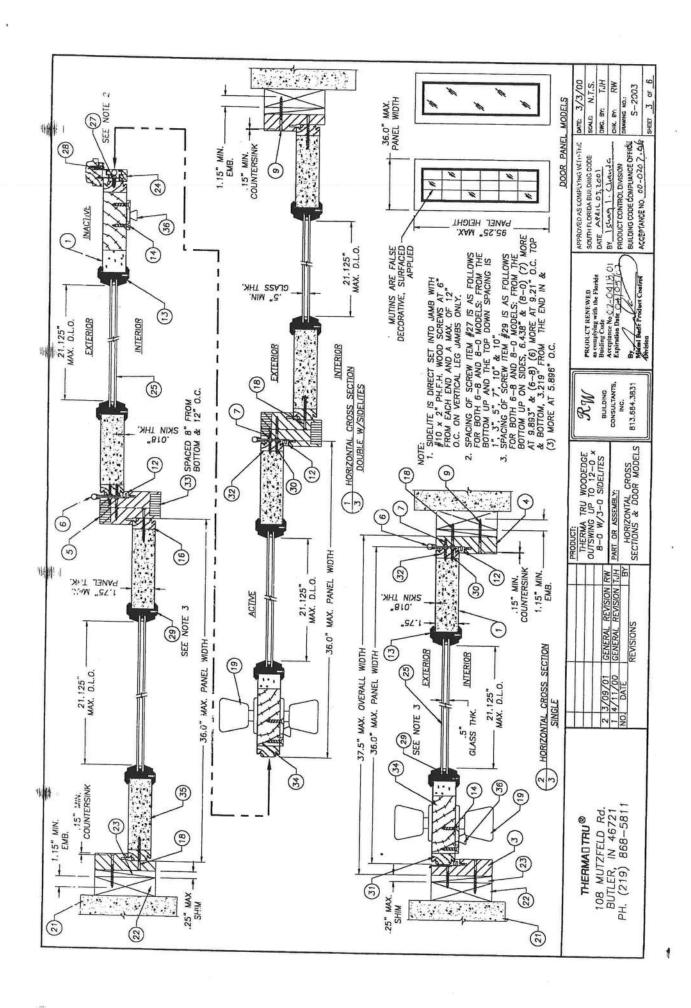


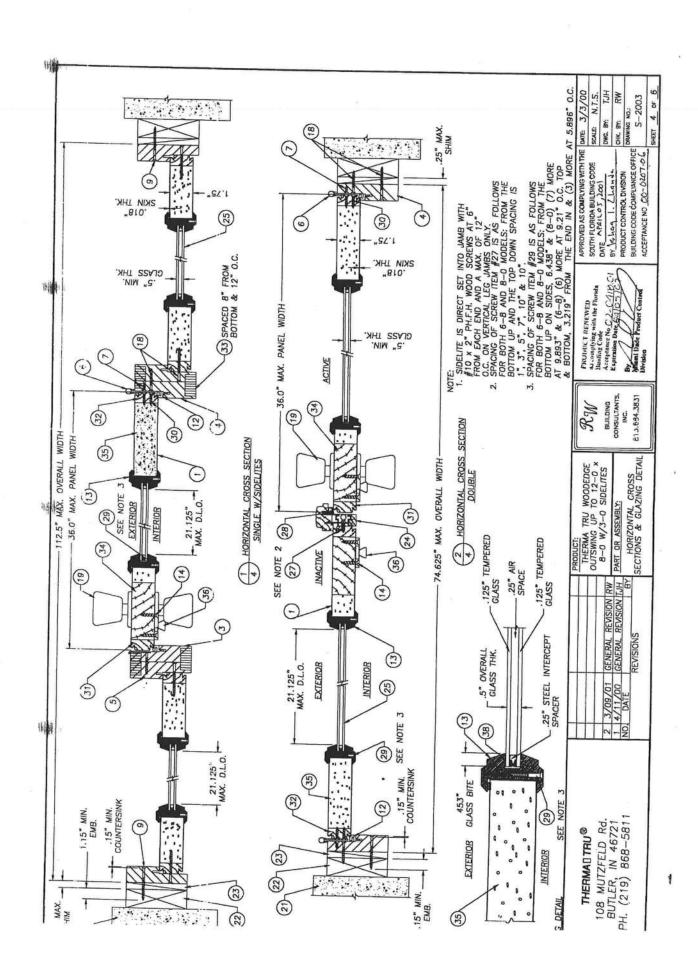
NOA No 02-0418.01 Expiration Date: April 05, 2007 Approval Date: May 23, 2002

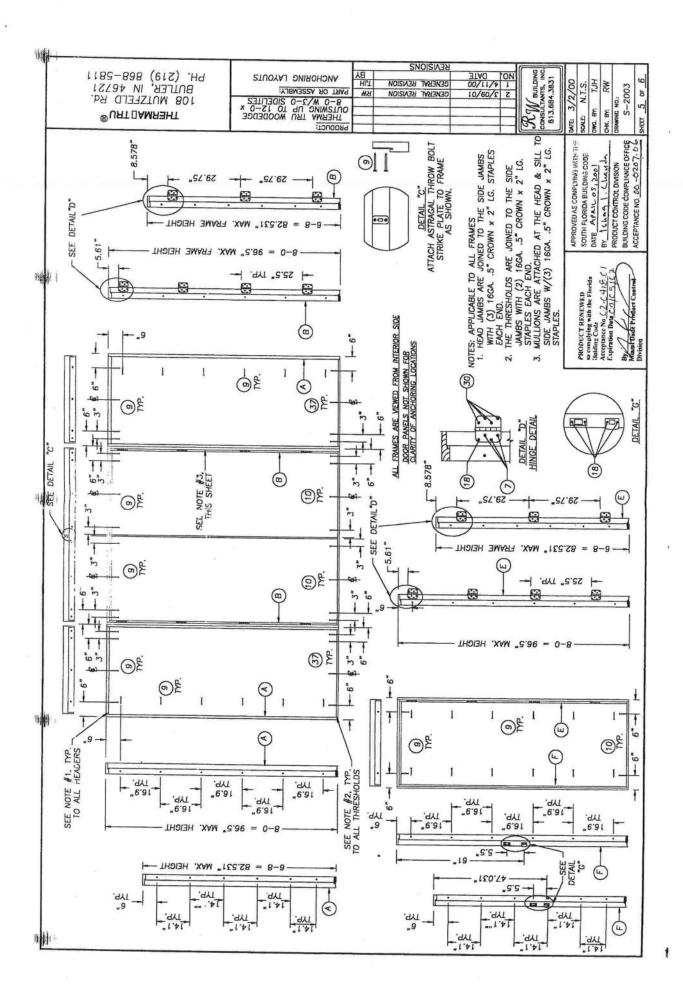
Page 1

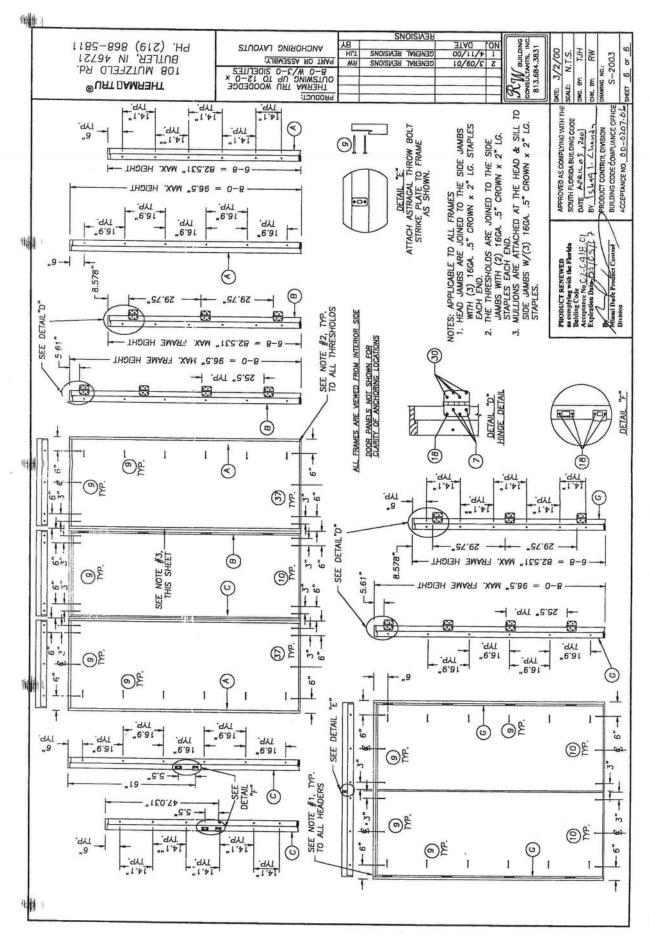














MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Clopay Building Products Co. 8585 Duke Blvd. Mason, OH 45040

SCOPE: This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code. This product is approved as described herein, and has been designed to comply with the Florida Building Code including the High Velocity Hurricane Zone.

DESCRIPTION: Sectional Garage Door 16'- 2" Wide.

APPROVAL DOCUMENT: Drawing No. 101300, titled "Double Car Hurricane Pan Door", dated 02/15/95 with last revision on 01/06/04, sheets 1 and 2 of 2, prepared by Clopay Building Products Co, signed and sealed by M. W. Westerfield, P.E., bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

LIMITATION: This approval requires the manufacturer to do testing of all coils used to fabricate door panels under this Notice of Acceptance. A minimum of 2 specimens shall be cut from each coil and tensile tested according to ASTM E-8 by a Dade County approved laboratory selected and paid by the manufacturer. Every 3 months, four times a year, the manufacturer shall mail to this office: a copy of the tested reports with confirmation that the specimen were selected from coils at the manufacturer production facilities. And a notarized statement from the manufacturer that only coils with yield strength of 38000 psi or more shall be used to make door panels for Dade County under this Notice of Acceptance

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA renews NOA # 03-0829.05 and consists of this page, evidence page as well as the approval document mentioned above.

03/23/06

The submitted documentation was reviewed by Candido F. Font PE.

NOA No 05-1212.02

Expiration Date: March 26, 2007 Approval Date: March 23, 2006

Page 1

Clopay Building Products Co.

NOTICE OF ACCEPTANCE: EVIDENCE PAGE

A. DRAWINGS

1. Drawing prepared by Clopay Building Products Co., titled "Double Car Hurricane Pan Door", Drawing No. 101300, dated 02/15/95, with last revision on 01/06/2004, sheets 1 through 2 of 2, signed and sealed by M.W. Westerfield, PE.

B. TESTS

1. Test report of large missile impact test per PA 201 and cyclic wind pressure test per PA 203 of "Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-408, dated 01/25/95, signed and sealed by H. M. Medina, PE.

2. Test report of Uniform Static Air Pressure Test Per PA 202 on "Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-407, dated

01/24/95, signed and sealed by H. M. Medina, PE.

3. Test report of Forced Entry Resistance per section 3603.2(b)5 on "Garage Door" prepared by Hurricane Engineering Testing, Inc. report No. HETI 95-407f, dated 01/25/95, signed and sealed by H. M. Medina, PE.

C. CALCULATIONS

1. Calculations dated 01/20/95; pages 1 and 2, prepared by M. W. Westerfield, PE, signed and sealed by M. W. Westerfield, PE.

2. Calculations dated 02/24/95, page 1, prepared M.W. Westerfield, PE, signed and sealed by M.W. Westerfield, PE.

D. MATERIAL CERTIFICATIONS

- 1. Test report of Tensile Test per ASTM E 8, report No. HETI 94-T59, prepared by Hurricane Engineering & Testing, Inc., dated 02/06/95, signed and sealed by H.M. Medina, PE.
- 2. Test report of Salt Spray Test per ASTM D1654 & ASTM B117, report No. 9EM-1144, prepared by Q.C. Metallurgical, Inc., dated 06/03/99, signed and sealed by K. Grate.

E. STATEMENTS.

1. Affidavit of yield strength compliance prepared by R. D. Shifflett employed by Clopay Building Products Co., notarized on 01/11/2001 by B. H. Schuler.

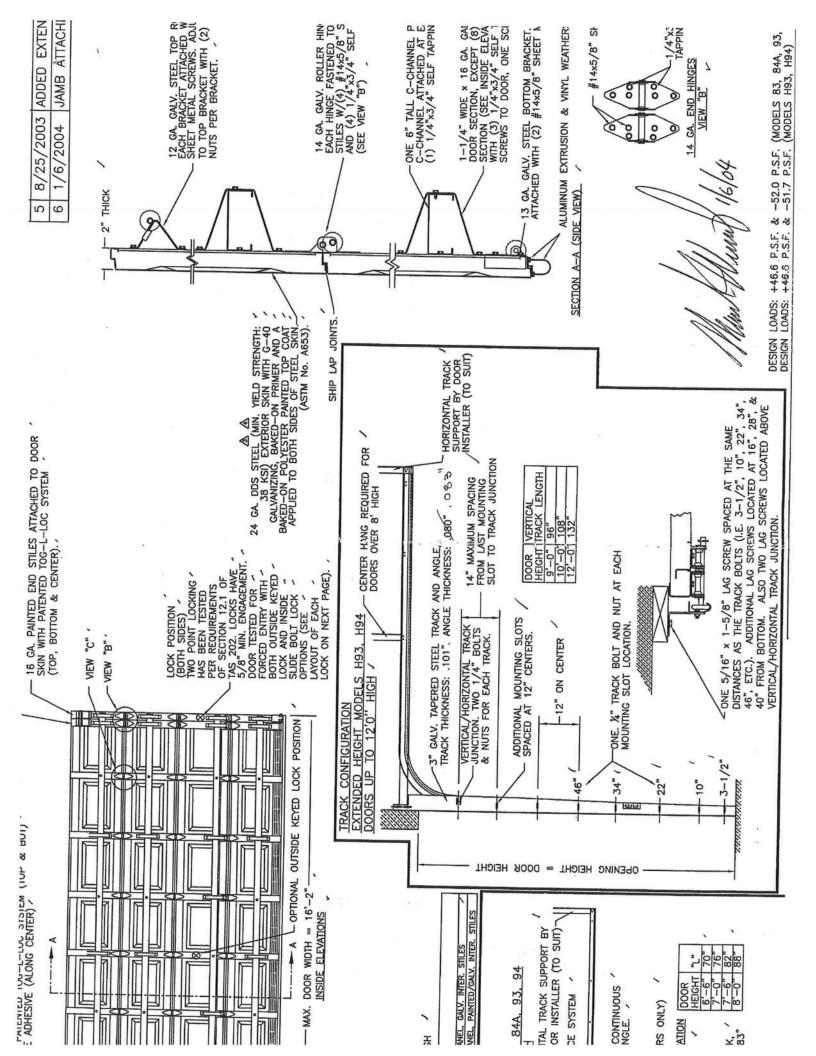
F. QUALITY ASSURANCE.

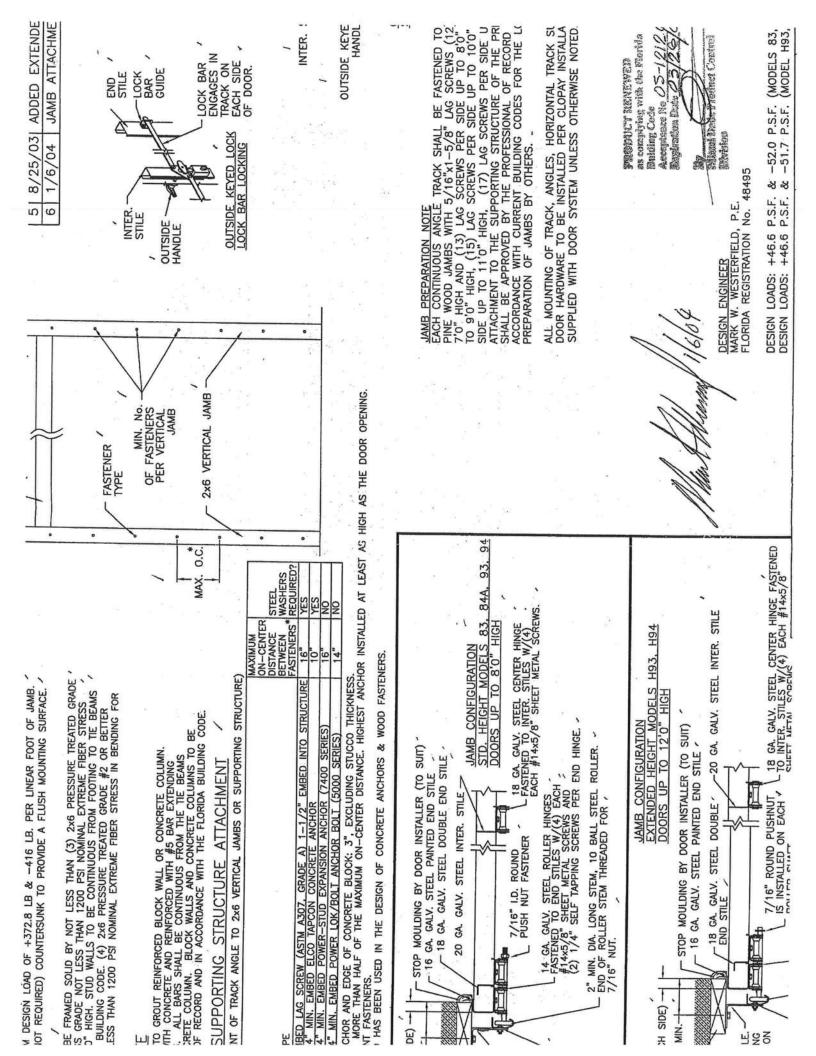
1. Building Code Compliance Office.

Candido F. Font, PE.

Senior Product Control Division NOA No 05-1212.02

Expiration Date: March 26, 2007 Approval Date: March 23, 2006







BUILDING CODE COMPLIANCE OFFICE (BCCO) PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation 108 Mutzfeld Road Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by the BCCO and accepted by the Building Code and Product Review Committee (BCPRC) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

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This product is approved as described herein, and has been designed to comply with the South Florida Building Code, 1994 Edition for Miami-Dade County or Florida Building Code.

DESCRIPTION: Outswing Glazed Residential Steel Door w\Sidelites

APPROVAL DOCUMENT: Drawing No. S-2003, titled "Therma-Tru Wood edge Outswing", sheets 1 through 6 to 6, prepared by RW Consulting, dated 3/9/01, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

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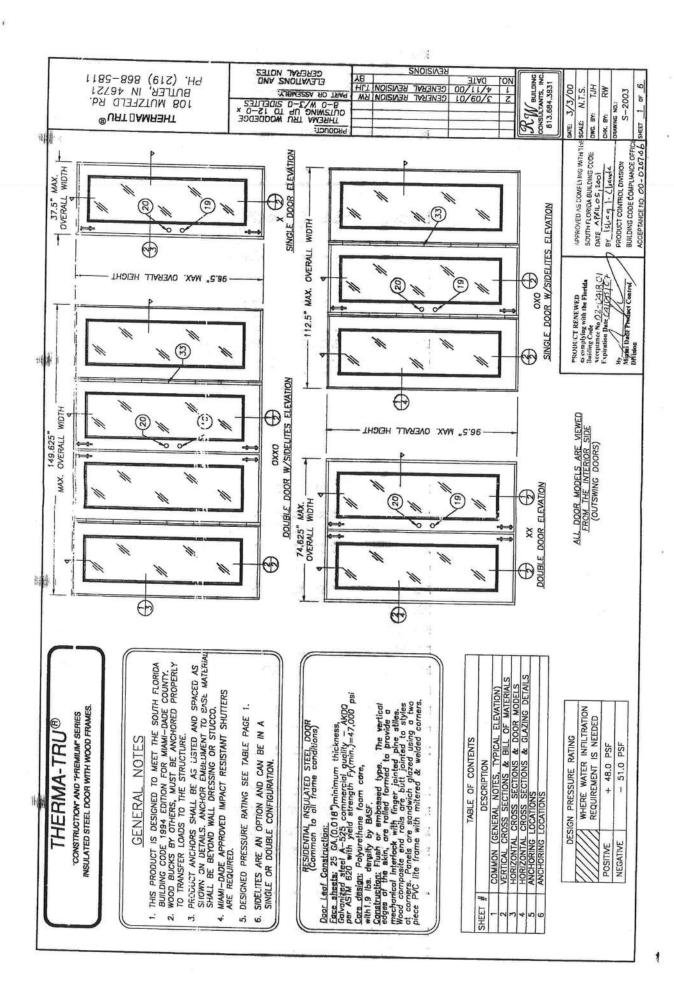
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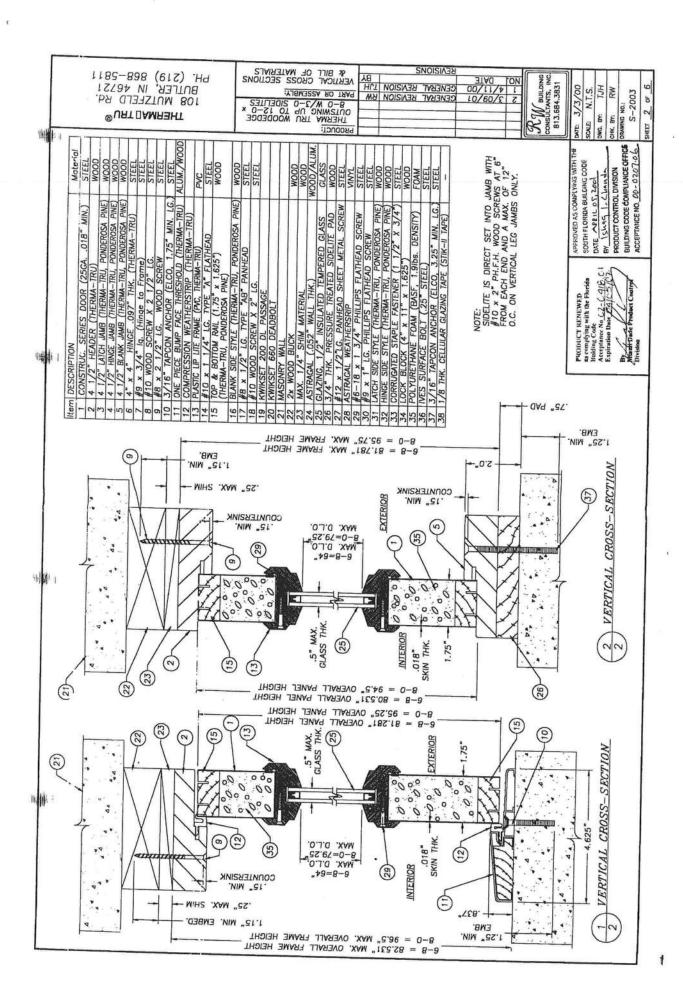
This NOA renews NOA # 00-0207.06 and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Raul Rodriguez.

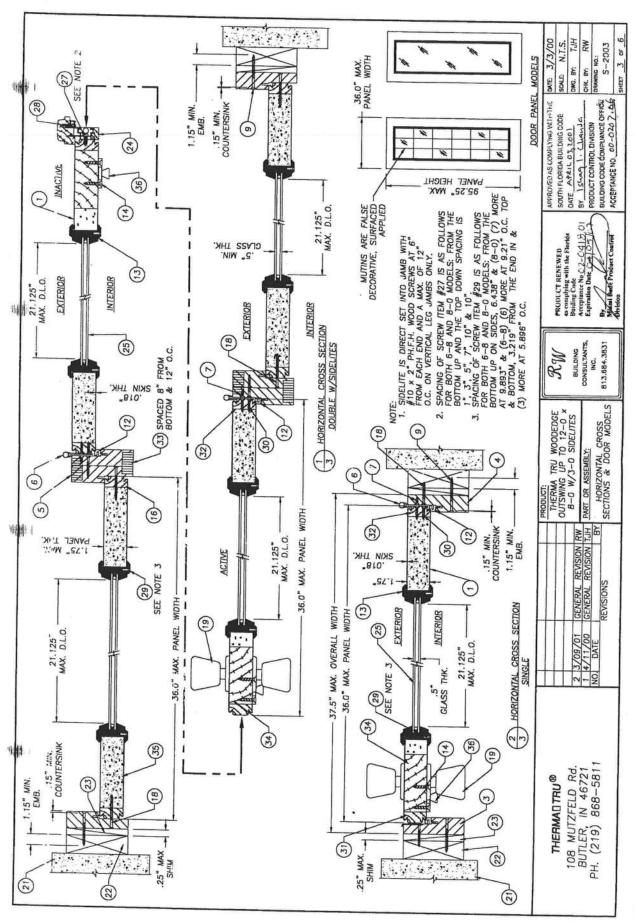


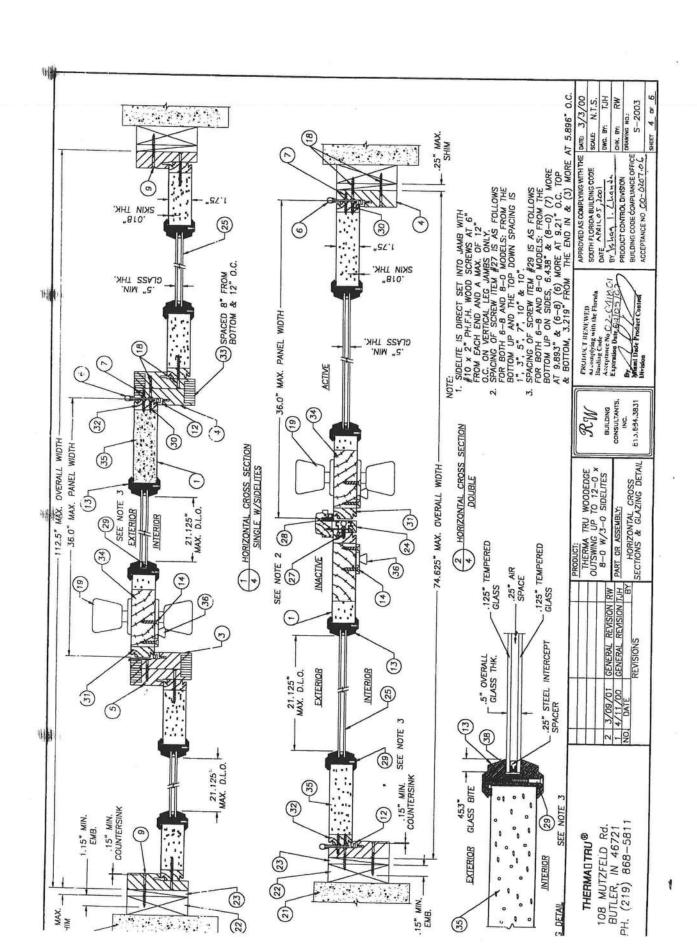
NOA No 02-0418.01 Expiration Date: April 05, 2007 Approval Date: May 23, 2002

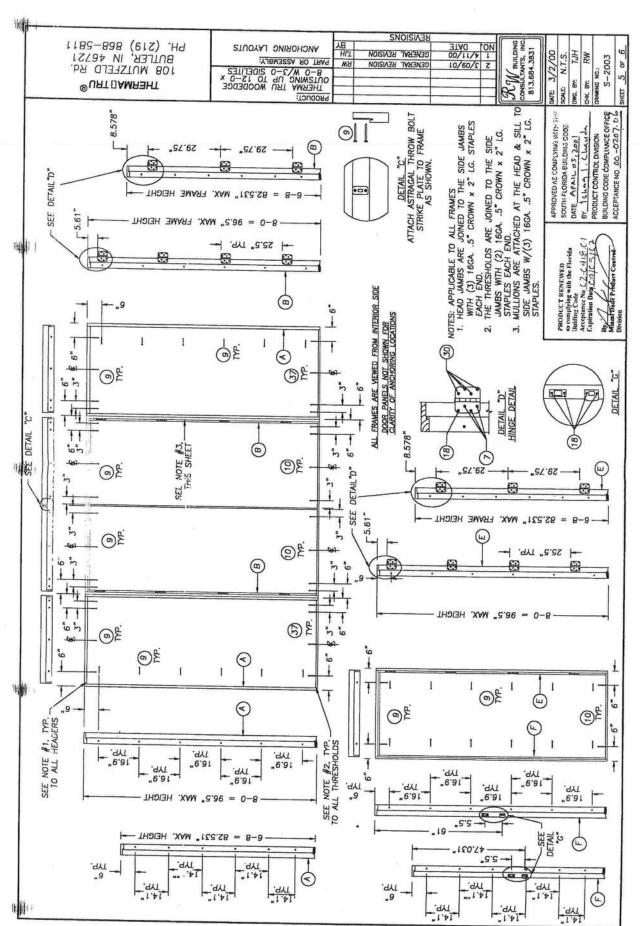
Page 1

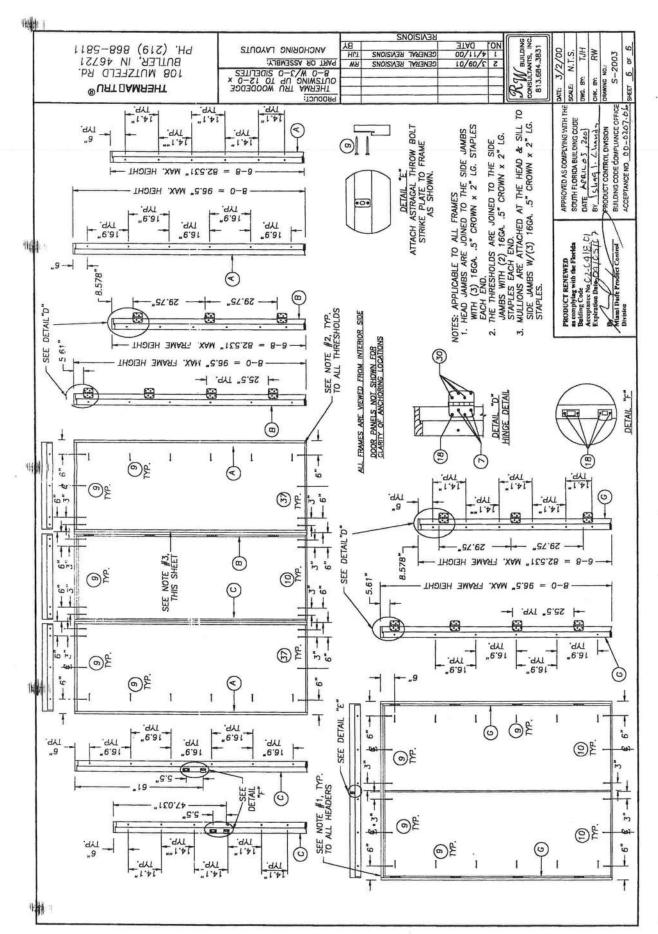














Project Information for:

L264817

Builder:

Trent Giebeig Construction, Inc.

Address:

1811 Southwest County Road 242A

Lake City, FL 32025

County:

Columbia

Truss Count:

21

Design Program: MiTek 20/20 6.3 **Building Code:** FBC2004/TPI2002 Truss Design Load Information:

Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

> Brian T. Giebeig Florida Registered Residential Contractor License No. RR282811523 Address: Trent Giebeig Construction, Inc. 462 Southwest Fairlington Court Lake City, Florida 320

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

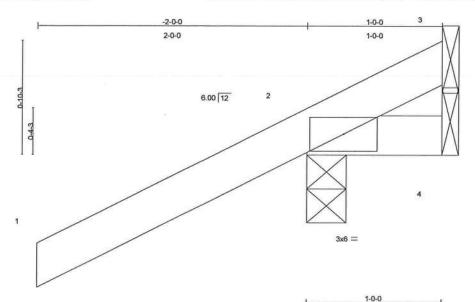
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1922967	CJ1	1/8/08
2	J1922968	CJ3	1/8/08
3	J1922969	CJ5	1/8/08
4	J1922970	EJ7	1/8/08
5	J1922971	HJ9	1/8/08
6	J1922972	T01	1/8/08
7	J1922973	T01G	1/8/08
8	J1922974	T02	1/8/08
9	J1922975	T03	1/8/08
10	J1922976	T03G	1/8/08
11	J1922977	T04	1/8/08
12	J1922978	T04G	1/8/08
13	J1922979	T05	1/8/08
14	J1922980	T05G	1/8/08
15	J1922981	T06	1/8/08
16	J1922982	T07	1/8/08
17	J1922983	T07G	1/8/08
18	J1922984	T08	1/8/08
19	J1922985	T09	1/8/08
20	J1922986	T10	1/8/08
21	J1922987	T11	1/8/08



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	CJ1	JACK	4	1		J1922967
\$i*		J. I.S. I.			Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)	3 5 X - 6			(Signature)	100000	Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

1-0-0

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1) Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 Ib uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3. Continued on page 2

January 8,2008

Scale: 1.5"=1"

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	CJ1	JACK	4	1		J1922967
P					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:17 2008 Page 2

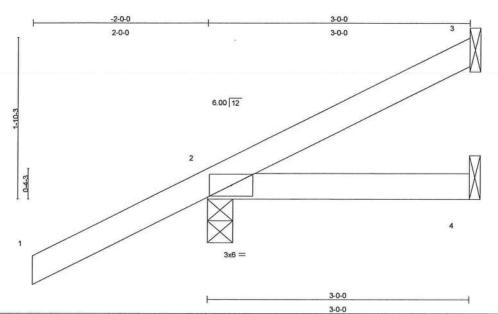
LOAD CASE(S) Standard

ulius Lee 1928 Design Engineer Johda PE No. 34868 193 Ceastal Bay Blvd Joynton Beach, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	CJ3	JACK	4	1		J1922968
v v	000	0.1011			Job Reference (optional)	

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LOADING (psf) **SPACING** 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** in (loc) TCLL 20.0 0.29 Plates Increase 1.25 TC Vert(LL) 0.01 2-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.012-4 >999 240 BCLL 10.0 WB 0.00 Rep Stress Incr YES Horz(TL) -0.003 n/a n/a BCDL Code FBC2004/TPI2002 5.0 (Matrix) Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4) Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

January 8,2008

Scale = 1:12.5

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	14000000
L264817	CJ3	JACK	4	1		J1922968
O.			1000		Job Reference (optional)	

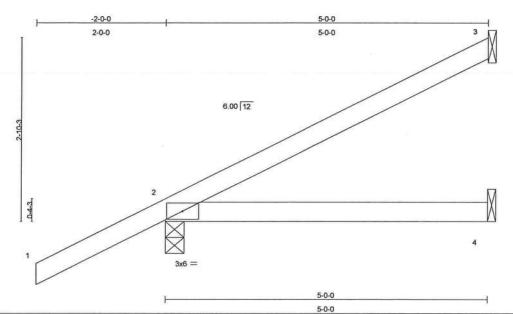
6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:18 2008 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	WWW.WW.WW
L264817	CJ5	JACK	4	1		J1922969
Đ					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	2002	(Mat	rix)						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

Julius Les Truss Design Engineer Florida FE No. 34869 1109 Ceastal Bay Blyd Boynton Besch, FL 33435

January 8,2008

Scale = 1:16.9

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	CJ5	JACK	4	1		J1922969
P	000	o, to it		20	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:18 2008 Page 2

LOAD CASE(S) Standard

Julius Las Truse Design Engineer Florida PE No. 24868 100 Coustell Bay Blod Boynton Besch. FL 33435



Job L264817	Truss	Truss Type MONO TRI		Qty 4	Ply 1	GIEBEIG -	PETTYJOHN RES	S.	J1922970
L204817	EJ/	IVIONO TRI	Joo	4	1	Job Referen	ice (optional)		
Builders FirstS	ource, Lake City, FI 320	55	6.30	0 s Apr 19 2006	MiTek In	dustries, Inc.	Tue Jan 08 15:57	15 2008	Page 1
	-2-0-0			7-0	0				
	2-0-0	1		7-0-	-0		3 17		Scale = 1:20.8
									Scale = 1.20.6
			6.00	12					
	10-3				//				
	e e			//					
		2							
	24.3						——————————————————————————————————————		
	14	A					4		
	1	3x6 =					-		
				7-0-	0				
				7-0-	0		10		
LOADING (ps	0 Plates Increase	2-0-0 1.25	CSI TC 0.50	DEFL Vert(LL)	0.33	loc) I/defl 2-4 >250	360 MT2	ATES 20	GRIP 244/190
TCDL 7. BCLL 10.			BC 0.45 WB 0.00		-0.16 -0.00	2-4 >501 3 n/a	240 n/a		
BCDL 5.			(Matrix)			** 1200 TA		ght: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-94(load case 6), 2=-224(load case 6), 4=-65(load case 5) Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-131/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.58

NOTES

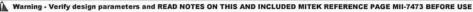
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 224 lb uplift at joint 2 and 65 lb uplift at joint 4.

LOAD CASE(S) Standard



Julius Les Truss Cesign Engineer 1906 Cessell Bay Blvd 1906 Cessell Bay Blvd Boynton Besch, FL 33436

January 8,2008

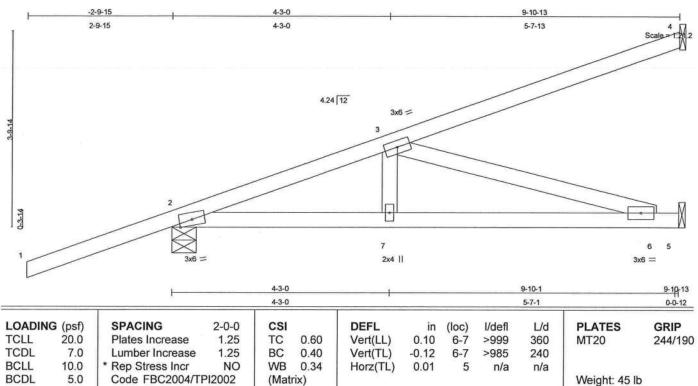


Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	HJ9	MONO TRUSS	2	1	J1	1922971
ě.		mene mese	_		Job Reference (optional)	
Builders FirstS	ource, Lake City, FI	32055 6.30	00 s Feb 15 2006	MiTek	Industries, Inc. Fri Jan 04 13:34:19 2008 P	age 1
	2.9.15	430			0.10.13	



LUMBER
TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 7-11-9 oc

bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical

Max Horz 2=269(load case 3)

Max Uplift 4=-233(load case 3), 2=-401(load case 3), 5=-181(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-647/363, 3-4=-105/65 BOT CHORD 2-7=-535/598, 6-7=-535/598, 5-6=0/0

WEBS 3-7=-94/190, 3-6=-623/557

JOINT STRESS INDEX

2 = 0.75, 3 = 0.22, 6 = 0.17 and 7 = 0.13

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.

Julius Les Truss Design Engineer Flonda PE No. 34869 1100 Ceastel Bay 6176 Boynton Beach, Ft. 95401

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	HJ9	MONO TRUSS	2	1		J1922971
	1111			8	Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

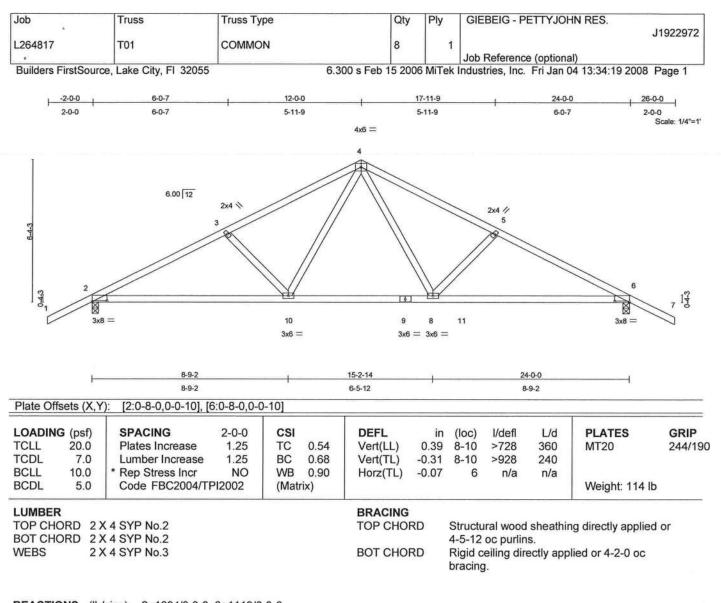
Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truse Design Engineer Flonda PE No. 34899 I 100 Coastel Bey Blvd





REACTIONS (lb/size) 2=1094/0-3-8, 6=1119/0-3-8

Max Horz 2=107(load case 6)

Max Uplift 2=-705(load case 6), 6=-721(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1808/2332, 3-4=-1589/2274, 4-5=-1638/2341, 5-6=-1857/2399,

6-7=0/47

BOT CHORD 2-10=-1913/1542, 9-10=-1265/1101, 8-9=-1265/1101, 8-11=-1973/1586,

6-11=-1973/1586

WEBS 3-10=-288/286, 4-10=-954/560, 4-8=-1081/652, 5-8=-288/287

JOINT STRESS INDEX

2 = 0.77, 3 = 0.33, 4 = 0.80, 5 = 0.33, 6 = 0.77, 8 = 0.51, 9 = 0.49 and 10 = 0.51

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Truss Design Engineer Floride PE No. 34966 1100 Ceastel Bay Blord Boynton Beach, FL 33435

Continued on page 2

January 8,2008



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Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T01	COMMON	0			J1922972
1204017	101	COMMON	0	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:20 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 2 and 721 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 10-11=-70(F=-60), 6-11=-10

Julius Lee Trues Cesign Engineer Flonds PE No. 34869 1109 Coastal Bay Blvd



Job Truss Type Ply GIEBEIG - PETTYJOHN RES. Truss Qty J1922973 L264817 T01G **GABLE** 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:21 2008 Page 1 -2-0-0 12-0-0 17-11-9 24-0-0 26-0-0 2-0-0 6-0-7 5-11-9 5-11-9 6-0-7 Scale = 1:47.3 8x10 || 5 6.00 12 6x8 < 6x8 / 5x8 > 3x8 II 3x8 14 3x6 = 12 11 10 29 5x8 = 3x6 = 8x10 = 3x6 = 5x8 > 8-9-2 15-2-14 24-0-0 8-9-2 6-5-12 8-9-2 Plate Offsets (X,Y): [2:0-3-15,0-0-3], [2:0-11-8,0-1-12], [8:0-3-15,0-0-3], [8:0-11-8,0-1-12], [11:0-5-0,0-4-8] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 TC Plates Increase 1.25 0.71 Vert(LL) 0.30 10-12 >932 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.43 Vert(TL) -0.23 10-12 >999 240 **BCLL** 10.0 Rep Stress Incr NO WB 0.40 Horz(TL) -0.088 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 195 lb LUMBER BRACING TOP CHORD 2 X 6 SYP No.1D *Except* TOP CHORD Structural wood sheathing directly applied or 1-3 2 X 4 SYP No.2, 7-9 2 X 4 SYP No.2 4-1-14 oc purlins. **BOT CHORD** 2 X 6 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 4-0-5 oc 2 X 4 SYP No.3 WEBS bracing. **OTHERS** 2 X 4 SYP No.3 **WEBS** T-Brace: 2 X 4 SYP No.3 -5-12, 5-10 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1933/0-6-0, 8=1958/0-6-0

Max Horz 2=118(load case 6) Max Uplift 2=-1457(load case 6), 8=-1474(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-70/108, 2-3=-3422/4535, 3-4=-3313/4465, 4-5=-2850/3968, 5-6=-2903/4042, TOP CHORD

6-7=-3369/4541, 7-8=-3478/4612, 8-9=-70/108

BOT CHORD 2-12=-3949/3028, 11-12=-2414/1934, 10-11=-2414/1934, 10-29=-4020/3079,

8-29=-4020/3079

WEBS 4-12=-808/1013, 5-12=-1502/946, 5-10=-1633/1041, 6-10=-813/1019 ulius Las russ Design Engineer 190 24868 190 Caastal Bay Blvd evnton Beach, FL 33436

JOINT STRESS INDEX

2 = 0.89, 2 = 0.89, 3 = 0.00, 3 = 0.75, 3 = 0.87, 4 = 0.22, 5 = 0.74, 6 = 0.22, 7 = 0.00, 7 = 0.88, 7 = 0.75, 8 = 0.89, 8 = 0.89, 10 = 0.89,2 = 0.89, 2 = 0.89, 3 = 0.00, 3 = 0.75, 3 = 0.07, 4 = 0.22, 5 = 0.74, 5 = 0.22, 7 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 21 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 24 = 0.33, 25 = 0.33 Continue 0.33 page = 20.68, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33 and 28 = 0.33

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T01G	GABLE	1	1		J1922973
*					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1457 lb uplift at joint 2 and 1474 lb uplift at joint 8.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

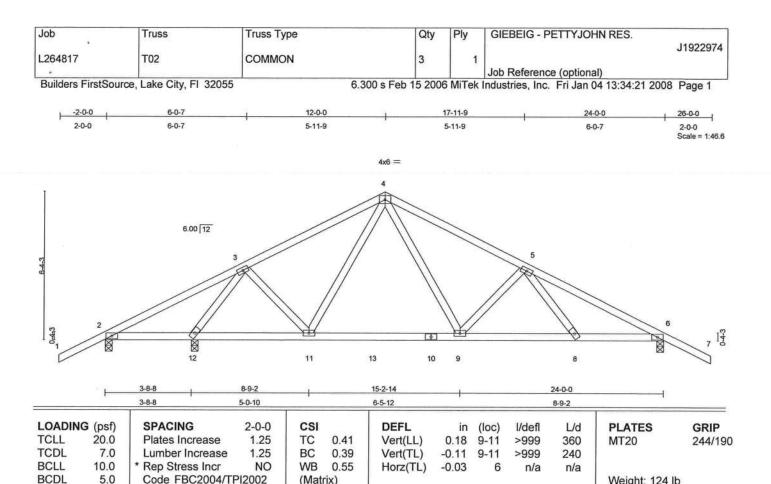
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-12=-10, 12-29=-70(F=-60), 8-29=-10

Julius Lee Trues Design Engineer Ponda PE No. 34869 1109 Coastal Bay Blvd





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

5.0

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD Structural wood sheathing directly applied or

5-6-5 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-6-9 oc

bracing.

REACTIONS (lb/size) 2=88/0-3-8, 6=823/0-3-8, 12=1303/0-3-8

Code FBC2004/TPI2002

Max Horz 2=107(load case 6)

Max Uplift 2=-81(load case 11), 6=-538(load case 7), 12=-833(load case 6)

Max Grav 2=137(load case 10), 6=823(load case 1), 12=1303(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-467/425, 3-4=-859/1243, 4-5=-972/1401, 5-6=-1232/1655, 6-7=0/47 **BOT CHORD**

2-12=-314/562, 11-12=-441/471, 11-13=-623/642, 10-13=-623/642, 9-10=-623/642,

(Matrix)

8-9=-1190/1010, 6-8=-1304/1031

WEBS 3-11=-516/388, 4-11=-364/238, 4-9=-664/359, 5-9=-316/369, 3-12=-1334/1706,

5-8=-194/144

JOINT STRESS INDEX

2 = 0.56, 3 = 0.68, 4 = 0.75, 5 = 0.68, 6 = 0.56, 8 = 0.46, 9 = 0.44, 10 = 0.22, 11 = 0.44 and 12 = 0.46

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; ICDL=4.2pst; BCDL=3.0pst; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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January 8,2008



Weight: 124 lb

Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T02	COMMON	3	1		J1922974
		Common			Job Reference (optional)	

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NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 2, 538 lb uplift at joint 6 and 833 lb uplift at joint 12.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

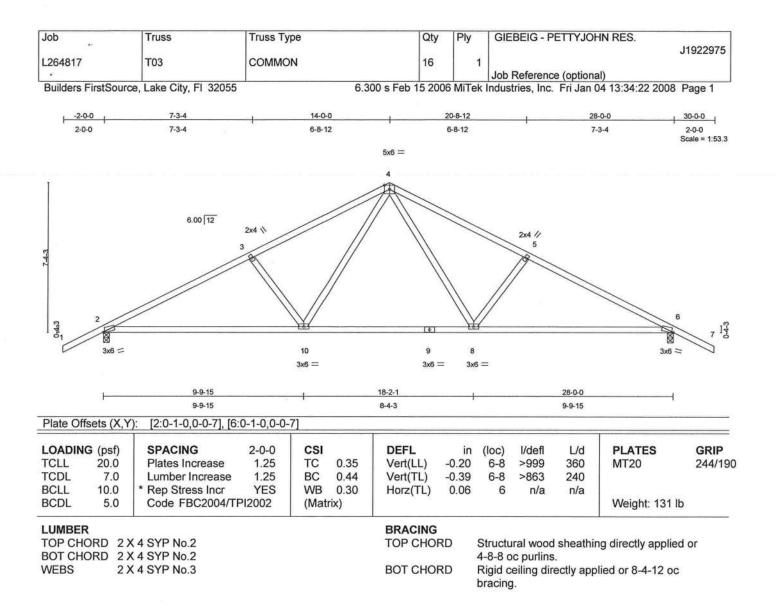
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-12=-10, 12-13=-70(F=-60), 6-13=-10

Julius Lee Truss Design Engineer Florida PE No. 34868 1109 Coestal Bay Blyri





REACTIONS (lb/size) 2=1003/0-3-8, 6=1003/0-3-8

Max Horz 2=119(load case 6)

Max Uplift 2=-288(load case 6), 6=-288(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1544/844, 3-4=-1316/817, 4-5=-1316/817, 5-6=-1544/844, 6-7=0/47

BOT CHORD 2-10=-566/1301, 9-10=-243/881, 8-9=-243/881, 6-8=-566/1301

WEBS 3-10=-358/332, 4-10=-249/451, 4-8=-249/451, 5-8=-358/332

JOINT STRESS INDEX

2 = 0.80, 3 = 0.33, 4 = 0.60, 5 = 0.33, 6 = 0.80, 8 = 0.42, 9 = 0.30 and 10 = 0.42

NOTES

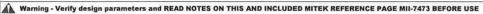
1) Unbalanced roof live loads have been considered for this design.

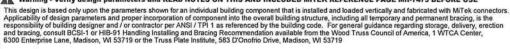
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Trues Design Engineer Florida PE No. 24888 1 100 Coastal Bay Blvd Boynton Beach, FL 33435







Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	CONTRACTOR OF THE PARTY OF THE
L264817	T03	COMMON	16	1		J1922975
	100	COMMON	10		Job Reference (optional)	

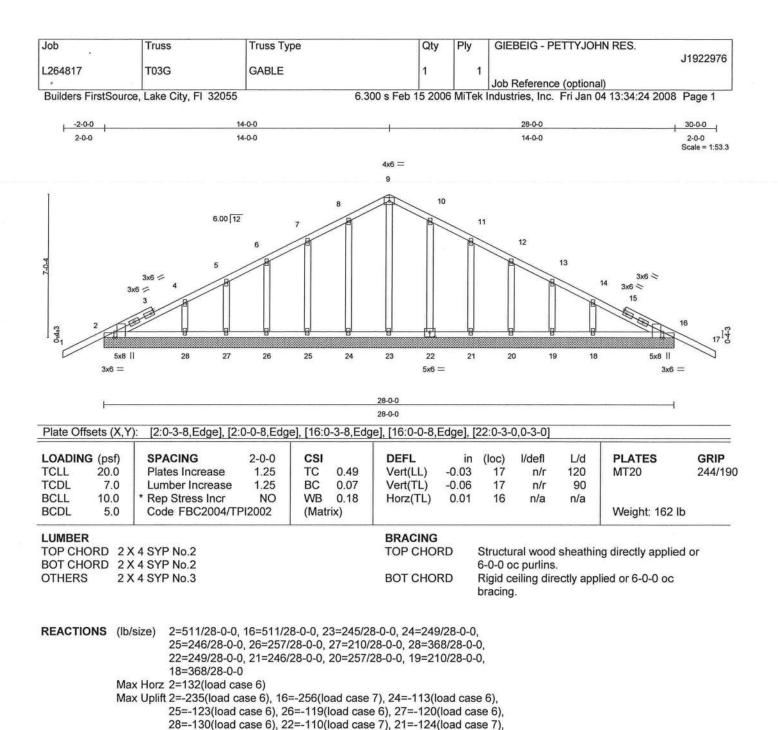
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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 2 and 288 lb uplift at joint 6.

LOAD CASE(S) Standard





Julius Lee Truse Design Engineer Florida PE No. 34569 1109 Cassial Bay Blvd Boynton Besch, FL 35435

January 8,2008

Continued on page 2



18=368(load case 1)

20=-119(load case 7), 19=-118(load case 7), 18=-136(load case 7)

24=253(load case 10), 25=246(load case 1), 26=257(load case 10), 27=210(load case 1), 28=368(load case 1), 22=253(load case 11), 21=246(load case 1), 20=257(load case 11), 19=210(load case 1), 20=257(load case 11), 25=210(load case 11), 25=21

Max Grav 2=511(load case 1), 16=511(load case 1), 23=245(load case 1),

Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	green secretarians and consider
L264817	T03G	GABLE	1	1		J1922976
•					Job Reference (optional)	

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-22/99, 2-3=-103/48, 3-4=-114/86, 4-5=-64/87, 5-6=-50/117, 6-7=-53/157, 7-8=-52/217, 8-9=-54/267,

9-10=-54/267, 10-11=-52/217, 11-12=-53/157, 12-13=-50/100, 13-14=-64/48, 14-15=-58/86, 15-16=-96/11,

16-17=-22/99

BOT CHORD 2-28=-2/162, 27-28=-2/162, 26-27=-2/162, 25-26=-2/162, 24-25=-2/162, 23-24=-2/162, 22-23=-2/162,

21-22=-2/162, 20-21=-2/162, 19-20=-2/162, 18-19=-2/162, 16-18=-2/162

WEBS 9-23=-225/0, 8-24=-233/165, 7-25=-226/184, 6-26=-235/181, 5-27=-196/167, 4-28=-333/223, 10-22=-233/165,

11-21=-226/184, 12-20=-235/181, 13-19=-196/167, 14-18=-333/223

JOINT STRESS INDEX

2 = 0.63, 2 = 0.18, 3 = 0.00, 3 = 0.40, 3 = 0.40, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.24, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.00, 15 = 0.40, 15 = 0.40, 16 = 0.63, 16 = 0.18, 18 = 0.33, 19 = 0.33, 1

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 235 lb uplift at joint 2, 256 lb uplift at joint 16, 113 lb uplift at joint 24, 123 lb uplift at joint 25, 119 lb uplift at joint 26, 120 lb uplift at joint 27, 130 lb uplift at joint 28, 110 lb uplift at joint 22, 124 lb uplift at joint 21, 119 lb uplift at joint 20, 118 lb uplift at joint 19 and 136 lb uplift at joint 18.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

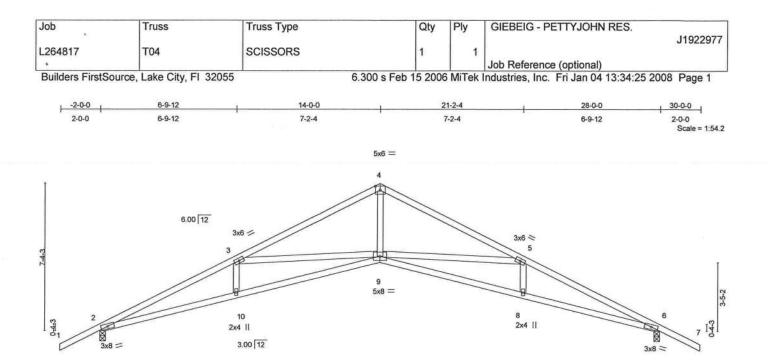
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-9=-114(F=-60), 9-17=-114(F=-60), 2-16=-10

Julius Lee Truse Design Engineer Flonda PE No. 3-1869 j 109 Goastal Bay Blvd





6-9-12 6-9-12 7-2-4 7-2-4 SPACING LOADING (psf) 2-0-0 CSI DEFL L/d **PLATES** GRIP in (loc) I/defl TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) 0.30 9-10 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.56 Vert(TL) -0.559-10 >609 240 **BCLL** 10.0 Rep Stress Incr 0.76 YES WB Horz(TL) 0.39 6 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 127 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

21-2-4

TOP CHORD Structural wood sheathing directly applied or

28-0-0

3-4-12 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-11-11 oc

bracing.

REACTIONS (lb/size) 2=1003/0-3-8, 6=1003/0-3-8

6-9-12

Max Horz 2=118(load case 6)

Max Uplift 2=-288(load case 6), 6=-288(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2902/1397, 3-4=-2042/933, 4-5=-2042/933, 5-6=-2902/1397,

6-7=0/46

BOT CHORD 2-10=-1098/2585, 9-10=-1102/2589, 8-9=-1102/2589, 6-8=-1098/2585

WEBS

3-10=0/210, 3-9=-820/580, 4-9=-502/1343, 5-9=-820/580, 5-8=0/210

14-0-0

JOINT STRESS INDEX

2 = 0.67, 3 = 0.39, 4 = 0.72, 5 = 0.39, 6 = 0.67, 8 = 0.33, 9 = 0.83 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 8,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T04	SCISSORS	1	1		J1922977
*	101	CONCONIC	,		Job Reference (optional)	

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NOTES

- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 2 and 288 lb uplift at joint 6.

LOAD CASE(S) Standard



Qty Ply GIEBEIG - PETTYJOHN RES. Job Truss Truss Type J1922978 L264817 T04G **GABLE** Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jan 08 16:00:26 2008 Page 1 28-0-0 14-0-0 -2-0-0 30-0-0 2-0-0 14-0-0 14-0-0 2-0-0 Scale = 1:51.6 4x6 = 9 10 6.00 12 11 12 13 5 3x6 > 3x6 = 14 3x6 > 15 4x10 = 28 27 23 22 4x10 = 5x6 = 4-0-0 28-0-0 4-0-0 24-0-0 Plate Offsets (X,Y): [2:0-3-12,0-2-0], [16:0-3-12,0-2-0], [22:0-3-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** GRIP in (loc) I/defl 20.0 1.25 TC 0.57 TCLL Plates Increase Vert(LL) 0.01 2-28 >999 360 MT20 244/190 Lumber Increase TCDL 7.0 1.25 BC 0.08 Vert(TL) -0.01 16-18 >999 240 BCLL 10.0 Rep Stress Incr NO WB 0.18 Horz(TL) 0.01

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2

5.0

OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

Weight: 162 lb

n/a

16

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS

(lb/size) 2=535/0-3-8, 16=513/24-0-0, 23=241/24-0-0, 24=249/24-0-0, 25=247/24-0-0, 26=254/24-0-0, 27=222/24-0-0, 28=336/24-0-0, 22=249/24-0-0, 21=246/24-0-0,

(Matrix)

20=257/24-0-0, 19=210/24-0-0, 18=369/24-0-0

Max Horz 28=132(load case 6)

Code FBC2004/TPI2002

Max Uplift 2=-294(load case 6), 16=-254(load case 7), 24=-112(load case 6), 25=-122(load case 6), 26=-123(load case 6), 27=-103(load case 6), 28=-189(load case 6), 22=-110(load case 7), 21=-124(load case 7), 20=-119(load case 7), 19=-118(load case 7), 18=-135(load case 7)

Max Grav 2=535(load case 1), 16=513(load case 1), 23=241(load case 1), 24=253(load case 10), 25=247(load case 1), 26=254(load case 10), 27=222(load case 1), 28=336(load case 1), 22=253(load case 11), 21=246(load case 1), 20=257(load case 11), 19=210(load case 11), 18=369(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-23/100, 2-3=-107/41, 3-4=-106/72, 4-5=-66/79, 5-6=-56/111, 6-7=-58/158, 7-8=-58/217

, 8-9=-59/267, 9-10=-58/267, 10-11=-58/217, 11-12=-58/158, 12-13=-55/100, 13-14=-69/49,

14-15=-65/82, 15-16=-101/17, 16-17=-22/99

2-28=0/202, 27-28=0/163, 26-27=0/163, 25-26=0/163, 24-25=0/163, 23-24=0/163,

22-23=0/163, 21-22=0/163, 20-21=0/163, 19-20=0/163, 18-19=0/163, 16-18=0/163

9-23=-221/0, 8-24=-233/165, 7-25=-227/184, 6-26=-233/181, 5-27=-205/166,

4-28=-308/226, 10-22=-233/165, 11-21=-226/184, 12-20=-235/181, 13-19=-196/167, 14-18=-334/223

Engineer 5. 34866 1 Bay Blvd ch. FL 33435

January 8,2008

Continued on page 2

BOT CHORD

WEBS

🗥 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T04G	GABLE	1	1		J1922978
					Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.68, 3 = 0.00, 3 = 0.41, 4 = 0.41, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.25, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.00, 15 = 0.41, 15 = 0.41, 16 = 0.68, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.20, 23 = 0.34, 24 = 0.34, 25 = 0.34, 27 = 0.34 and 28 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2, 254 lb uplift at joint 16, 112 lb uplift at joint 24, 122 lb uplift at joint 25, 123 lb uplift at joint 26, 103 lb uplift at joint 27, 189 lb uplift at joint 28, 110 lb uplift at joint 22, 124 lb uplift at joint 21, 119 lb uplift at joint 20, 118 lb uplift at joint 19 and 135 lb uplift at joint 18.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

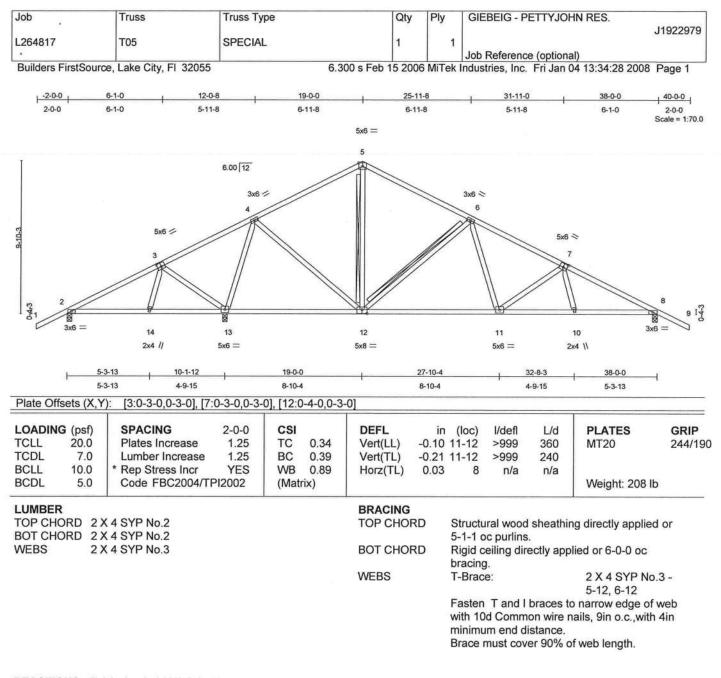
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-9=-114(F=-60), 9-17=-114(F=-60), 2-16=-10

Julius Lee Truse Design Engineer Florida FE No. 24868 1100 Coestel Bay Blvd Acunton Besch, FL 02425





REACTIONS (lb/size) 2=231/0-3-8, 13=1492/0-3-8, 8=923/0-3-8

Max Horz 2=148(load case 6)

Max Uplift 2=-231(load case 6), 13=-473(load case 6), 8=-291(load case 7) Max Grav 2=319(load case 10), 13=1492(load case 1), 8=923(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-130/264, 3-4=-235/550, 4-5=-552/427, 5-6=-552/427, 6-7=-1103/658

, 7-8=-1432/730, 8-9=0/47

BOT CHORD 2-14=-216/79, 13-14=-228/111, 12-13=-106/295, 11-12=-236/845, 10-11=-488/1194

, 8-10=-478/1206

3-14=-210/143, 3-13=-369/502, 4-13=-1250/757, 4-12=-255/654, 5-12=-64/183,

6-12=-580/448, 6-11=-128/343, 7-11=-335/269, 7-10=0/140

Truss Design Engineer Florida FE No. 34869 1109 Coastal Bay Blvd Boynton Beach, FL 93495

JOINT STRESS INDEX

WEBS

Continued 63 3 3 59, 4 = 0.46, 5 = 0.64, 6 = 0.46, 7 = 0.59, 8 = 0.63, 10 = 0.33, 11 = 0.25, 12 = 0.56, 13 = 0.25 and 14 = 0.33 January 8,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofino Drive, Madison, WI 53719.



Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T05	SPECIAL	1	1		J1922979
		J			Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

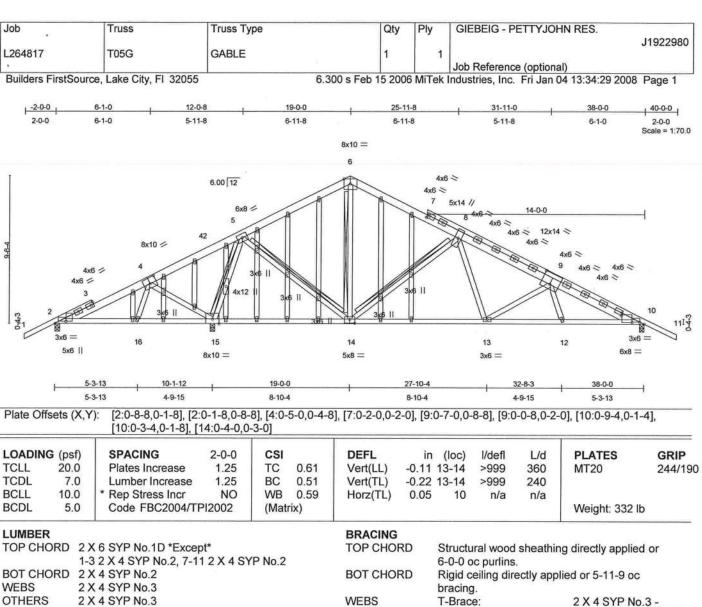
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 2, 473 lb uplift at joint 13 and 291 lb uplift at joint 8.

LOAD CASE(S) Standard





2 X 4 SYP No.3

T-Brace:

5-15, 5-14, 6-14, 8-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=589/0-3-8, 15=2795/0-3-8, 10=1179/0-3-8

Max Horz 2=-165(load case 7)

Max Uplift 2=-517(load case 6), 15=-1666(load case 6), 10=-667(load case 7) Max Grav 2=657(load case 10), 15=2795(load case 1), 10=1179(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-56/100, 2-3=-264/321, 3-4=-153/261, 4-42=-485/611, 5-42=-669/909, TOP CHORD

5-6=-1219/1172, 6-7=-1008/1027, 7-8=-1103/1052, 8-9=-1716/1372,

9-10=-2049/1438, 10-11=0/47

BOT CHORD 2-16=-173/137, 15-16=-138/124, 14-15=0/172, 13-14=-926/1436,

12-13=-1133/1769, 10-12=-1125/1778

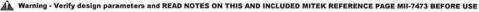
4-16=-208/131, 4-15=-743/965, 5-15=-2432/2201, 5-14=-744/1049, 6-14=-179/145,

8-14=-722/601, 8-13=-83/329, 9-13=-308/217, 9-12=0/126

Continued on page 2

WEBS

January 8,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building oed. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T05G	GABLE	1	1		J1922980
	1.000				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:29 2008 Page 2

JOINT STRESS INDEX

2 = 0.81, 2 = 0.77, 3 = 0.00, 3 = 0.45, 3 = 0.45, 4 = 0.37, 5 = 0.51, 6 = 0.70, 7 = 0.00, 7 = 0.48, 7 = 0.48, 8 = 0.28, 8 = 0.30, 8 = 0.30, 8 = 0.30, 8 = 0.30, 9 = 0.13, 9 = 0.23, 9 = 0.23, 9 = 0.23, 10 = 0.64, 10 = 0.57, 12 = 0.33, 13 = 0.38, 14 = 0.60, 15 = 0.35, 16 = 0.33, 17 = 0.69, 18 = 0.33, 19 = 0.33, 20 = 0.69, 21 = 0.33, 22 = 0.33, 23 = 0.69, 24 = 0.33, 25 = 0.33, 26 = 0.34, 27 = 0.33, 28 = 0.33, 29 = 0.33, 30 = 0.33, 31 = 0.56, 32 = 0.33, 33 = 0.33, 34 = 0.33, 35 = 0.33, 36 = 0.33, 37 = 0.33, 38 = 0.69, 39 = 0.33, 40 = 0.33 and 41 = 0.69

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 517 lb uplift at joint 2, 1666 lb uplift at joint 15 and 667 lb uplift at joint 10.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-42=-114(F=-60), 6-42=-141(F=-87), 6-7=-141(F=-87), 7-11=-54, 2-10=-10

Julius Law Truss Design Engineer Florida PE No. 34868 1 100 Grastal Bay Blvd Govnton Beach, Ft. 03435



Job	Truss	Truss Typ	oe .	Qt	Ply	GIEB	EIG - PE	TTYJOHN	N RES.	J1922981
.264817	T06	SPECIAL		2		1				01022001
Builders FirstSourc	e, Lake City, FI 32055	5	6.	300 s Feb 15 2	006 MiT			(optional) Fri Jan 04		8 Page 1
	, , ,						18			3
	8-11 10-3-8	15-0-				31-2		-	38-0-0	40-0-0
2-0-0 5-	8-11 4-6-13	4-9-0	3-11-8	5-0-0		7-2-	-4		6-9-12	2-0-0 Scale = 1:71.2
				5x6 =						
				6						
		6.00 12	//							
			5		7					
		3x6 =			18					
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				9	12		A.			Ŋ
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31	- H	4	13	9 9			9	11	A DE	1014
- // 🔞		60	5x8 =						•	, /
3x6 =	15	14		142		20			3xt	= 7
3x6 =	15	14 5x14 =		12		3x6	=		3x6 5x6 =	
		5x14 =	3.00	*					5x6 =	12 7
j5-	8-11 10-1-12	5x14 = 10 ₁ 3-8 13-11-8	3.00	24-0-0		31-2-	-4	-1	5x6 =	1
j <u>5-</u>	8-11 10-1-12 8-11 4-5-1	5x14 = 10 ₁ 3-8 13-11-8 0-1-12 3-8-0	3.00 14-7-12 0-8-4	24-0-0 9-4-4	1		-4	-1	5x6 =	1
Plate Offsets (X,Y	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8	5x14 = 10 ₁ 3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0	3.00 14-7-12 0-8-4 0], [9:0-7-2,0-1	24-0-0 9-4-4 -3]		31-2- 7-2-	-4 4	· · ·	5x6 = 38-0-0 6-9-12	ł
Plate Offsets (X,Y)	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8	5x14 = 10 ₁ 3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1	24-0-0 9-4-4 -3]		31-2- 7-2- (loc)	-4 4 I/defl	L/d	5x6 = 38-0-0 6-9-12 PLATES	GRIP
Plate Offsets (X,Y) COADING (psf) FCLL 20.0	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8 SPACING Plates Increase	5x14 = 10 ₁ 3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46	24-0-0 9-4-4 3] DEFL Vert(LL)	0.17	31-2- 7-2- (loc) 11-12	-4 4 I/defl >999	360	5x6 = 38-0-0 6-9-12	ł
Plate Offsets (X,Y) OADING (psf) CLL 20.0 CDL 7.0	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase	5x14 = 10 ₁₃ -8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50	24-0-0 9-4-4 -3] DEFL Vert(LL) Vert(TL)	0.17 -0.31	31-2- 7-2- (loc) 11-12 12-13	-4 4 I/defl		5x6 = 38-0-0 6-9-12 PLATES	GRIP
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8 SPACING Plates Increase	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46	24-0-0 9-4-4 3] DEFL Vert(LL)	0.17	31-2- 7-2- (loc) 11-12 12-13	1/defl >999 >999	360 240	5x6 = 38-0-0 6-9-12 PLATES	GRIP 244/190
Plate Offsets (X,Y) COADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCDL 5.0	8-11 10-1-12 8-11 4-5-1): [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	24-0-0 9-4-4 -3] DEFL Vert(LL) Vert(TL) Horz(TL)	0.17 -0.31 0.14	31-2- 7-2- (loc) 11-12 12-13	1/defl >999 >999	360 240	5x6 = 38-0-0 6-9-12 PLATES MT20	GRIP 244/190
Plate Offsets (X,Y) OADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	24-0-0 9-4-4 -3] DEFL Vert(LL) Vert(TL)	0.17 -0.31 0.14	(loc) 11-12 12-13 9	1/defl >999 >999 n/a	360 240 n/a	5x6 = 38-0-0 6-9-12 PLATES MT20	GRIP 244/190
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 CMBER TOP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o	I/defl >999 >999 n/a	360 240 n/a sheathing	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257	GRIP 244/190
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 CMBER TOP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL)	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei	I/defl >999 >999 n/a	360 240 n/a sheathing	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257	GRIP 244/190
Plate Offsets (X,Y) OADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCDL 5.0 LUMBER FOP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing.	I/defl >999 >999 n/a al wood ac purlins illing dire Except:	360 240 n/a sheathing	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257 g directly appled or 5-6-2 or	GRIP 244/190
Plate Offsets (X,Y) OADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 UMBER OP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei	I/defl >999 >999 n/a al wood ac purlins illing dire Except:	360 240 n/a sheathing	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257	GRIP 244/190
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 CMBER TOP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHO	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing. 1 Row at T-Brace:	I/defl >999 >999 n/a al wood ac purlins illing dire Except:	360 240 n/a sheathing ctly applie	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257 g directly appl ed or 5-6-2 or 9-12 2 X 4 SYP 1 6-13	GRIP 244/190
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 CMBER COP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHO	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing. 1 Row at T-Brace:	I/defl >999 >999 n/a al wood ac purlins illing dire Except t midpt	360 240 n/a sheathing c. ctly applie	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257 g directly appl ed or 5-6-2 or 9-12 2 X 4 SYP 1 6-13 narrow edge	GRIP 244/190 'Ib ied or control of web
Plate Offsets (X,Y) COADING (psf) CLL 20.0 CDL 7.0 BCLL 10.0 BCDL 5.0 CMBER COP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHO	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing. 1 Row at T-Brace:	I/defl >999 >999 n/a al wood ac purlins illing dire Except t midpt T and I b Commo	sheathing control of the control of	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257 g directly appl ed or 5-6-2 or 9-12 2 X 4 SYP 1 6-13	GRIP 244/190 'Ib ied or control of web
Plate Offsets (X,Y) COADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCDL 5.0 COMBER FOP CHORD 2 X BOT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHO	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing. 1 Row at T-Brace: Fasten with 10d minimum	I/defl >999 >999 n/a al wood ac purlins illing dire Except t midpt T and I to Common end dis	sheathing control applies braces to an wire na stance.	January Septiments of the septiment of t	GRIP 244/190 ' Ib ied or No.3 -
Plate Offsets (X,Y) OADING (psf) CLL 20.0 CDL 7.0 CLL 10.0 CDL 5.0 UMBER OP CHORD 2 X OT CHORD 2 X	B-11 10-1-12 B-11 4-5-1 D: [3:0-3-0,0-3-0], [8 SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/T	5x14 = 10,3-8 13-11-8 0-1-12 3-8-0 3:0-3-0,0-3-0 2-0-0 1.25 1.25 YES	3.00 14-7-12 0-8-4 D], [9:0-7-2,0-1 CSI TC 0.46 BC 0.50 WB 0.76	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHO	0.17 -0.31 0.14	(loc) 11-12 12-13 9 Structura 4-2-12 o Rigid cei bracing. 1 Row at T-Brace: Fasten with 10d minimum	I/defl >999 >999 n/a al wood ac purlins illing dire Except t midpt T and I b Common end dis ust cove	sheathings. ctly applied or wire nastance. or 90% of	5x6 = 38-0-0 6-9-12 PLATES MT20 Weight: 257 g directly appl ed or 5-6-2 or 9-12 2 X 4 SYP 1 6-13 narrow edge	GRIP 244/190 'Ib ied or No.3 -

REACTIONS (lb/size) 2=-171/0-3-8, 14=2043/0-3-8, 9=773/0-3-8

Max Horz 2=148(load case 6)

Max Uplift 2=-384(load case 11), 14=-522(load case 6), 9=-270(load case 7) Max Grav 2=131(load case 10), 14=2043(load case 1), 9=773(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-274/1065, 3-4=-592/1385, 4-5=-41/356, 5-6=0/357, 6-7=-1025/607,

7-8=-1042/428, 8-9=-1974/946, 9-10=0/46

BOT CHORD 2-15=-908/386, 14-15=-910/391, 13-14=-1280/861, 12-13=0/321, 11-12=-691/1742, 3

9-11=-688/1740

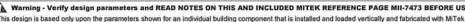
3-15=-196/176, 3-14=-372/496, 4-14=-1495/760, 4-13=-513/1256, 5-13=-257/250,

6-13=-890/287, 6-12=-522/1152, 7-12=-355/357, 8-12=-830/639, 8-11=0/204

January 8,2008

Continued on page 2

WEBS





ulius Lee ruse Design Engineer londs PE No. 34866 100 Coastel Bey Stor world leach, FL 33436

Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T06	SPECIAL	2	1		J1922981
	100	OI LOIAL			Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.40, 3 = 0.58, 4 = 0.87, 5 = 0.33, 6 = 0.48, 7 = 0.33, 8 = 0.70, 9 = 0.63, 9 = 0.21, 11 = 0.33, 12 = 0.90, 13 = 0.54, 14 = 0.81, 15 = 0.33, 16 = 0.15, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33 and 28 = 0.33

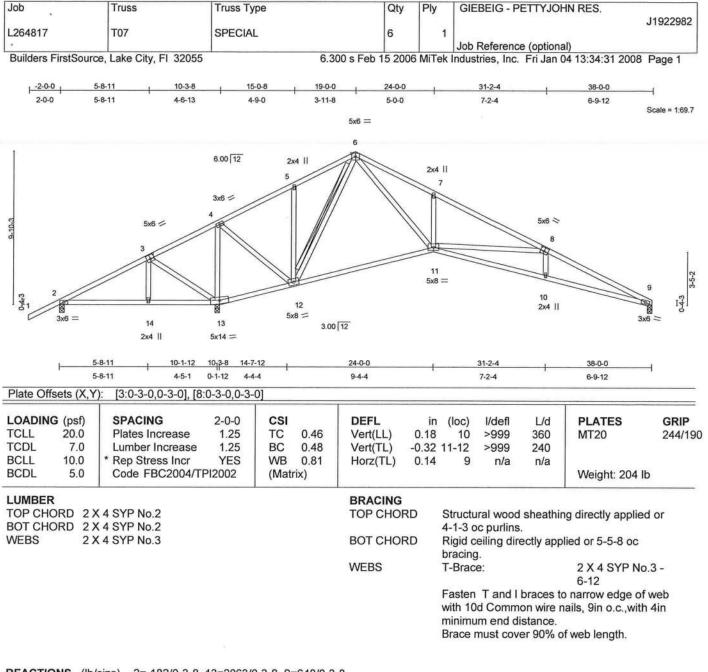
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 2, 522 lb uplift at joint 14 and 270 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Clesian Engineer Flonda PE No. 34868 1100 Ceastal Bay Blvd Boynton Beach, FL 33435





REACTIONS (lb/size) 2=-182/0-3-8, 13=2063/0-3-8, 9=648/0-3-8

Max Horz 2=161(load case 6)

Max Uplift 2=-395(load case 11), 13=-534(load case 6), 9=-163(load case 7) Max Grav 2=128(load case 10), 13=2063(load case 1), 9=648(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-394/1089, 3-4=-713/1409, 4-5=-100/369, 5-6=0/369, 6-7=-1024/626,

7-8=-1042/450, 8-9=-2029/1055

BOT CHORD 2-14=-930/417, 13-14=-931/422, 12-13=-1303/897, 11-12=0/265, 10-11=-873/1796,

9-10=-876/1799

3-14=-196/176, 3-13=-372/499, 4-13=-1509/833, 4-12=-586/1270, 5-12=-256/249,

6-12=-904/362, 6-11=-584/1159, 7-11=-351/352, 8-11=-884/724, 8-10=0/207

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L 264817	T07	SPECIAL	6	1		J1922982
,	1.01	or zon z			Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.40, 3 = 0.58, 4 = 0.88, 5 = 0.33, 6 = 0.49, 7 = 0.33, 8 = 0.77, 9 = 0.63, 10 = 0.33, 11 = 0.90, 12 = 0.55, 13 = 0.82 and 14 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 2, 534 lb uplift at joint 13 and 163 lb uplift at joint 9.

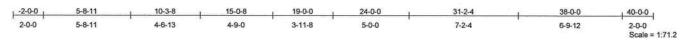
LOAD CASE(S) Standard

Julius Lee Truss Cession Engineer Flonda PE No. 34899 1 190 Cessial Bay Blvd Boynton Beach, FL 93495



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
1.004047	T070	CARLE				J1922983
L264817	T07G	GABLE	1	1		
					Job Reference (optional)	

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8x10 =

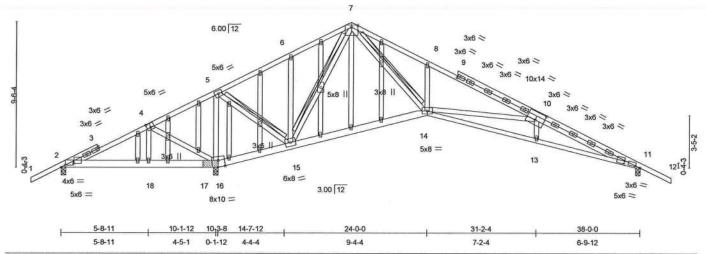


Plate Offsets (X,Y): ,0-0-1], [16:0-5-0, Edge]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.99	Vert(LL)	0.30	13-14	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.42	13-14	>791	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.21	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 296 lb	

LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or

BOT CHORD 2 X 6 SYP No.1D *Except* 4-2-1 oc purlins.

11-14 2 X 4 SYP No.2 **BOT CHORD**

Rigid ceiling directly applied or 5-1-9 oc **WEBS** 2 X 4 SYP No.3 *Except* bracing.

5-16 2 X 4 SYP No.2 **WEBS** T-Brace: 2 X 4 SYP No.3 -

2 X 4 SYP No.3 5-15, 7-15, 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-84/0-3-8, 16=3727/0-4-6 (0-3-8 + bearing block), 11=937/0-3-8

Max Horz 2=168(load case 6)

OTHERS

WEBS

Max Uplift 2=-281(load case 11), 16=-2432(load case 6), 11=-585(load case 7) Max Grav 2=205(load case 10), 16=3727(load case 1), 11=937(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-60/107, 2-3=-969/1655, 3-4=-1096/1821, 4-5=-1742/2359, 5-6=-146/392,

6-7=-118/366, 7-8=-1915/1575, 8-9=-1748/1353, 9-10=-1859/1342,

10-11=-2773/1813, 11-12=0/46

BOT CHORD 2-18=-1538/1078, 17-18=-1539/1083, 16-17=-1539/1083, 15-16=-2166/1904,

14-15=-137/471, 13-14=-1494/2494, 11-13=-1492/2487

4-18=-182/152, 4-16=-577/774, 5-16=-2860/2423, 5-15=-1779/2306, 6-15=-629/698

7-15=-1479/1003, 7-14=-1213/1774, 8-14=-597/625, 10-14=-783/551,

Continued on pagle 213=0/205

January 8,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T07G	GABLE	1	1		J1922983
*	10.0	O, IDEE		in 18	Job Reference (optional)	

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JOINT STRESS INDEX

 $2 = 0.57, \ 2 = 0.86, \ 3 = 0.00, \ 3 = 0.47, \ 3 = 0.47, \ 4 = 0.72, \ 5 = 0.80, \ 6 = 0.36, \ 7 = 0.58, \ 8 = 0.33, \ 9 = 0.00, \ 9 = 0.40, \ 9 = 0.26, \ 9 = 0.26, \ 9 = 0.26, \ 10 = 0.28, \ 10 = 0.29, \ 10 = 0.29, \ 10 = 0.29, \ 11 = 0.66, \ 11 = 0.66, \ 13 = 0.33, \ 14 = 0.88, \ 15 = 0.78, \ 16 = 0.95, \ 16 = 0.00, \ 17 = 0.00, \ 17 = 0.00, \ 18 = 0.33, \ 19 = 0.33, \ 20 = 0.43, \ 21 = 0.33, \ 22 = 0.33, \ 23 = 0.65, \ 24 = 0.33, \ 25 = 0.33, \ 26 = 0.33, \ 27 = 0.33, \ 28 = 0.33, \ 29 = 0.33, \ 30 = 0.54, \ 31 = 0.33, \ 32 = 0.33, \ 33 = 0.33, \ 34 = 0.33, \ 35 = 0.33, \ 36 = 0.33, \ 37 = 0.45$

NOTES

- 1) 2 X 6 SYP No.1D bearing block 12" long at jt. 16 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 4) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2, 2432 lb uplift at joint 16 and 585 lb uplift at joint 11.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

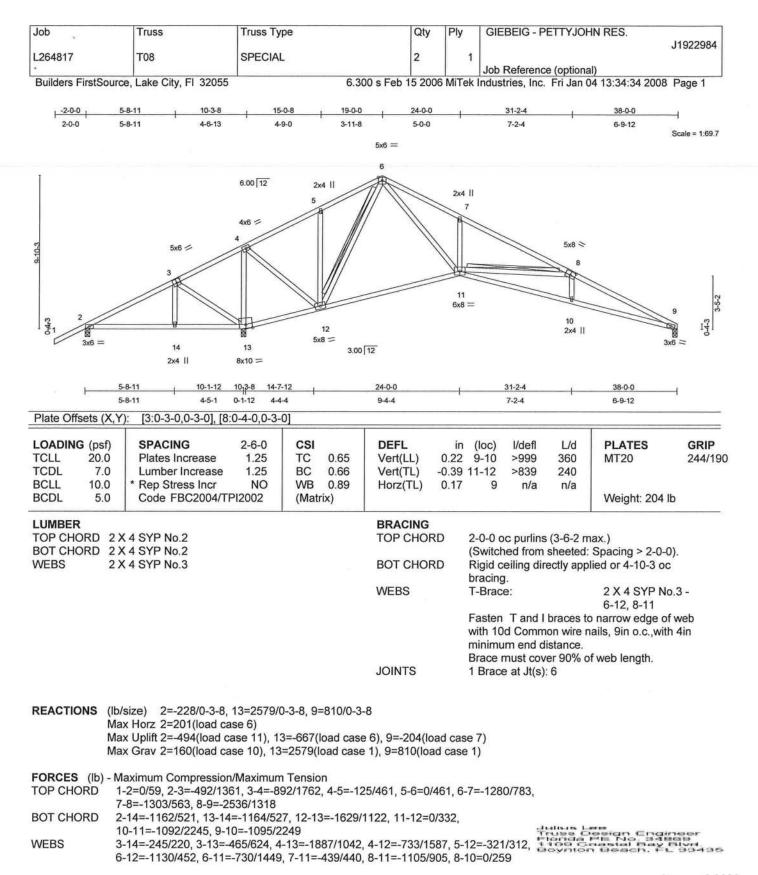
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-114(F=-60), 5-7=-141(F=-87), 7-8=-141(F=-87), 8-12=-54, 2-16=-10, 14-16=-10, 11-14=-10

Julius Lee Trues Design Engineer Flonda PE No. 24868 1 100 Geasiel Bay Blyd Boynton Besch, FL 23436





Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	Т08	SPECIAL	2	1		J1922984
	100	OF LOIAL	-		Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.50, 3 = 0.73, 4 = 0.73, 5 = 0.33, 6 = 0.61, 7 = 0.33, 8 = 0.64, 9 = 0.79, 10 = 0.33, 11 = 0.81, 12 = 0.68, 13 = 0.95 and 14 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 494 lb uplift at joint 2, 667 lb uplift at joint 13 and 204 lb uplift at joint 9.

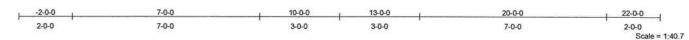
LOAD CASE(S) Standard

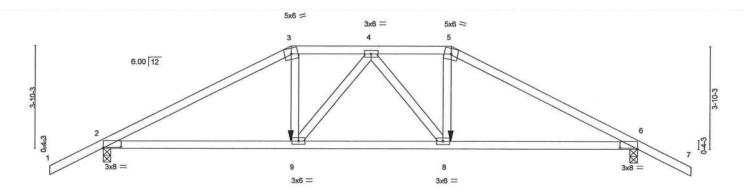
Julius Les Truss Design Engineer Flonda PE No. 34860 1 100 Caestal Bay Blyd Boynton Beach, Ft. 33436



Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	Т09	HIP	1	1		J1922985
	105	-	,		Job Reference (optional)	

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7-0-0	13-0-0	20-0-0	
7-0-0	6-0-0	7-0-0	- 1

Plate Of	fsets (X,Y	'): [2:0-8-0,0-0-6], [6:	0-8-0,0-0-	6]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.14	2-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.19	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 91 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		3-9-2 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-3-12 oc bracing.

REACTIONS (lb/size) 2=1381/0-3-8, 6=1381/0-3-8

Max Horz 2=77(load case 5)

Max Uplift 2=-721(load case 5), 6=-721(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2393/1090, 3-4=-2079/1015, 4-5=-2079/1015, 5-6=-2393/1090,

6-7=0/47

BOT CHORD 2-9=-938/2052, 8-9=-985/2169, 6-8=-904/2052

WEBS 3-9=-371/719, 4-9=-269/143, 4-8=-269/143, 5-8=-370/719

JOINT STRESS INDEX

2 = 0.74, 3 = 0.62, 4 = 0.38, 5 = 0.62, 6 = 0.74, 8 = 0.46 and 9 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

EbAll hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Les Truss Design Engineer Florida FE No. 34869 1 109 Coastel Bay Slvri Boynton Beach, FL 33431

January 8,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T09	HIP	1	1		J1922985
					Job Reference (optional)	-

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 721 lb uplift at joint 2 and 721 lb uplift at joint 6.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

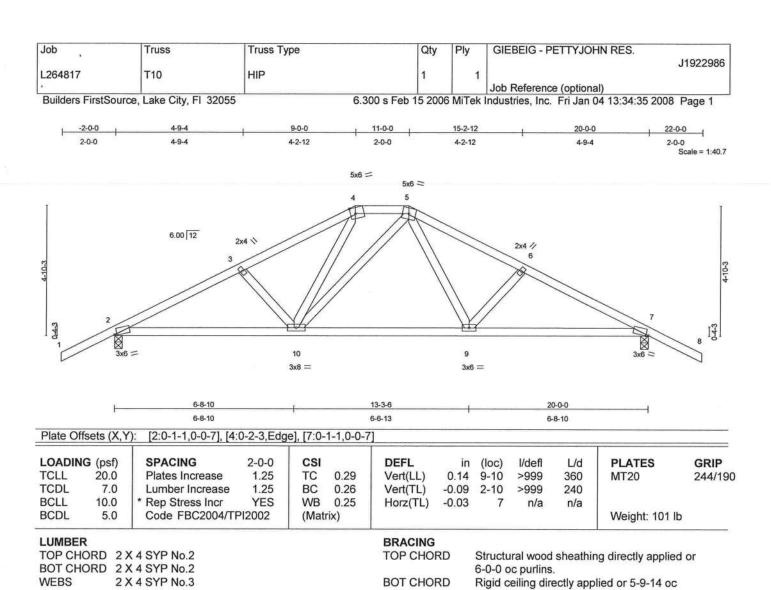
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-9=-10, 8-9=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 9=-411(F) 8=-411(F)

Julius Lee Truse Design Engineer Flonda PE No. 34869 1109 Coastal Bay Blvd





bracing.

REACTIONS (lb/size) 2=747/0-3-8, 7=747/0-3-8

Max Horz 2=-89(load case 7)

Max Uplift 2=-476(load case 6), 7=-476(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1075/1415, 3-4=-908/1375, 4-5=-661/1044, 5-6=-914/1384,

6-7=-1075/1419, 7-8=0/47

BOT CHORD 2-10=-1110/900, 9-10=-726/657, 7-9=-1113/901

WEBS 3-10=-208/215, 4-10=-534/271, 5-10=-127/136, 5-9=-548/278, 6-9=-207/216

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.44, 5 = 0.31, 6 = 0.33, 7 = 0.75, 9 = 0.44 and 10 = 0.50

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Les Trues Design Engineer Flonds PE No. 34866 1180 Crastel Bay Blvd. Boynton Beach, FL 33435

3) Provide adequate drainage to prevent water ponding.

Continued on page 2





Job .	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T10	HIP	1	1		J1922986
		1	(°)		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:35 2008 Page 2

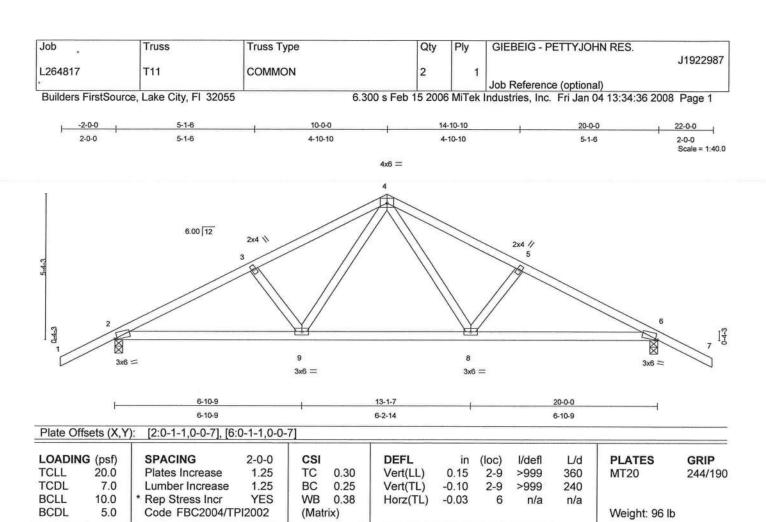
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida Ple No. 24868 190 Coestal Bay Blvd 190495 Environ Besch, FL 90495





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-9-13 oc

bracing.

REACTIONS (lb/size) 2=747/0-3-8, 6=747/0-3-8

Max Horz 2=-95(load case 7)

Max Uplift 2=-481(load case 6), 6=-481(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1066/1411, 3-4=-910/1394, 4-5=-910/1394, 5-6=-1066/1411,

6-7=0/47

BOT CHORD 2-9=-1104/891, 8-9=-652/612, 6-8=-1104/891

WEBS 3-9=-237/229, 4-9=-602/300, 4-8=-602/300, 5-8=-237/229

JOINT STRESS INDEX

2 = 0.75, 3 = 0.33, 4 = 0.53, 5 = 0.33, 6 = 0.75, 8 = 0.42 and 9 = 0.42

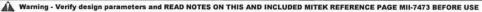
NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collinger on page 2

January 8,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job ,	Truss	Truss Type	Qty	Ply	GIEBEIG - PETTYJOHN RES.	
L264817	T11	COMMON	2	1		J1922987
	(5.00.20)				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jan 04 13:34:36 2008 Page 2

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 481 lb uplift at joint 2 and 481 lb uplift at joint 6.

LOAD CASE(S) Standard

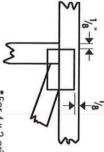


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless securely seat. plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

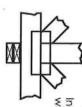
perpendicular to slots. Second dimension is the length parallel The first dimension is the width

LATERAL BRACING



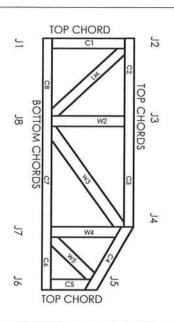
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

96-31, 96-67

BOCA

ICBO 3907, 4922

9667, 9432A

SBCCI

561

NER

WISC/DILHR

960022-W, 970036-N



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each
- ω joint and embed fully. Avoid knots and wane at joint locations. Place plates on each face of truss at each
- 4 at 1/2 panel length (± 6" from adjacent joint.) Unless otherwise noted, locate chord splices
- S Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 6 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- 7. practice is to camber for dead load deflection. is the responsibility of truss fabricator. General Camber is a non-structural consideration and
- 00 shown indicate minimum plating requirements Plate type, size and location dimensions
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 unless otherwise noted ft. spacing, or less, if no ceiling is installed,
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling erection and installation of trusses.
- © 1993 MiTek® Holdings, Inc.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

		MAX GABLE VERTICAL LENGTH	
		MAX GABLE VERTICAL LENGTH SPACING SPECIE SPECIE SPECIE O.C. SPI 12" O.C. 16" O.C. SPI 12" O.C. HF 12" O.C. SPI HF 12" O.C. SPI DFI 12" O.C. SPI 12" O.C. SPI 14" SP 15" SP 15" SP 15" SP 16" O.C. SPI 17" SP 18" SP 1	
		SPACING SPECIES OF SPE	VAC
j.	H=WARNI BRACING PLATE (OF ANCE T THESE T STRUCTU	### ### ### ### ### ### ### ### ### ##	4
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	MARKING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, MAIDLING, SUPPTING, INSTALLING AND BRACING. REFER TO BEST 14-21 (BUILDING COMPONENT SAFETY INFORMING), PUBLICING INSTALLING AND INFORMATION, MAISIN, VIOLANDISTO, VIOLAND	COUP A GROUP B 5 10 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0	BDACE
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34869 Of Florida	JLIUS LFF'S CONS. ENGINEERS P.A. DELRAY BEACH, PL SEAL-2161		,
	P.A.S	CROUP A 112 11 12	
MAX. TOT.		BRACE STORY B 13 3 1 13 3 1 13 3 1 13 3 1 13 3 1 13 3 1 13 1 13 1	
TOT. LD. 60 PSF SPACING 24.0"		BRACING GRO SPRUCE-PINE SPRUCE-PINE 1	20

ATES.	EAX, SPLICE, AND HEEL PI
2.5X4	EATER THAN 11' 8"
2004	EATER THAN 4 D', BUT
K4 DR EX3	SS THAN 4' 0" 1
NO SPLICE	VERTICAL LENGTH
SIZES	ABLE VERTICAL PLATE

			NAPERT PRACTICES PRIDE TO PERFORMING SHALL HAVE PROPERTY ATTACHED OPERLY ATTACHED RIGID CESSIONS	TY INFORMATION, PUBLISHED BY TP) (TRUSS VI. 53719) AND VICA (VIDIO TRUSS COUNCIL	TING, HAND ING SHIPPING, INSTALL ING AND
No: 34869 STATE OF FLORIDA			DELRAY BEACH, PL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF				
		-ENG	DRWG MIEK SID GABLE 15 E HT	DATE 11/26/03	REF ASCEY-02-CABI3015

ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, II 1.00, EXPOSURE Q

ING GROUP SPECIES AND GRADES:

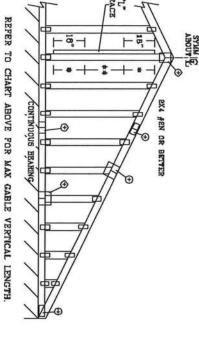
GROUP A:

HEM-FIR
AZ STUD
53 STANDARD

SOUTHERN PINE #3 STUD

STANDARD

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	STANDARD	STUD	#3	#22	# 3	STANDARD	STUD	£ 3	\$1 / #2	STANDARD	STUD	† 3	#2	\$ 3	STANDARD	STUD	#3	\$1 / #2	STANDARD	STUD	# 3	2#	# 1	STANDARD	STUD	#3	\$1 / #2	GRADE	BRACE
	4.0	4.	4. 2,	4. 4.	4 5	3' 11"	3' 11"			3. 8.	3 8	3. 8,	3' 11"	4.0	3. 7.	3' 7"	3' 7"	3.	3' 0"	3' 3"	3. 3.	3' 6"			3' 1"		3.	BRACES	S _O
	5 6	6 4	6' 6"	6' 11"	5' 11 "	5' 4"	6.3	6 3°	6' 11"	4. 9.	5 6	5. 3.		6 4	4' 8"	5' 6"	UI.	6. 4.	3' 10"	4' 6"					4' 5,		5. 6,	GROUP A	(1) 1X4 '
	5, 6,	6' 4"	6' 5"		7' 6"	5' 4"	6. 3.	6 3°	7' 2'	4' 9"	5° 6"	6' 7"	8' 10"	B' 10"	4' 8"	6. 5.	5) 5	6. 6.		4' 6"	4. 6.	5' 11"	5' 11"		4, 5,	4. 5"	6. 8.	GROUP B	"L" BRACE .
	7' 3"	8. 3.	8' 3"	8' 3"		7' 1°	a' 3"	8' 3"	8' 3"	6' 3"	7' 3"	7' 4"	7' 8"	7' 6"	6. 5.	7. 2.	7' 2"	7' 6'	5" 1"	5' 11"	6. 0.	6, 8,	6' B"	6. 0.	5' 10"	6, 10.	6 6	GROUP A	(1) 2X4 ·
TATAS	7' 3"	8 6"	100	8' 11"	B' 11.	7' 1"	8 3	e' 3"	8' 6'		7' 3"	7 4"	B' 1"	B' 1"	6. 2.	7' 2"	7' 2"	7' 8"	5. 1.	5' 11"	5.0"	7' 0"	7' 0"	5. 0.	5' 10"	5' 10"	6. 9.	GROUP B	(1) 2X4 "L" BRACE .
2	8.8	9. 10.	9' 10"	9' 10"		9, 6,			9' 10"		8' 11"	8, 11	8' 11"	8, 11,	8. 3	8' 11"	8' 11"	8" 11"	8° 11"	7' 10"	7' 10"	7' 10"	7' 10"		7' 10"	7' 10"	7 10	GROUP A	(2) 2X4 "L"
	9, 9,	10' 4"	10' 4"	10' 7"	10' 7"	9, 6,		9' 10"	10. 1.	8' 5"	8' 5"	8. 9.		8' 7"	8, 3,	8' 11"	8' 11"	9. 2.	6, 11,	8'0"	8. 1.	٦,	8,5		7' 10"	7' 10"	8.0.	GROUP	BRACE **
	11' 4"	12' 11"	12, 11,	12, 11,	12' 11"		12. 10.		12' 11"	8, 8,	11, 4,	11. 2.	11. 9.	11, 8,	9. 7"	11, 1,,	- 7	11' 9"	8' 0"	8 3	9. 4.	10′ 3"	10' 3"	7' 10"	9' 1"	9' 1"	10' 3"	B GROUP A	(1) 2X6
	11' 4"	13. 1.	13' 3'	13' 11"	13' 11"	11' 1"	12' 10"		13. 4.			11' 6'	12' 8"	12' B"	8. 4.	11, 1,,	11' 2"	12' 1"	8.0.	8' 3"	9 4.	11, 1,	11' 1"	7' 10"	9′ 1"	9' 1"	10' 7'	A GROUP B	"L" BRACE .
	14' 0"	14. 0.	14' 0"	14' 0"	14 0	14' 0"	14. 0"	14 0"	14. 0.	13' 3"	14 0	14. 0.	14. 0"	14. 0	12. 11.	14. 0*	14' 0"	14. 0.	10' 10"	12' 3"	12. 3.	12' 3"	12' 3"	10' 7"	12' 3*	12' 3"	12' 3"	B GROUP A GROUP	(2) 2XB "L" BRACE
	14' 0"	14. 0	14' 0"	14' 0"	14' 0"	14' 0"	14. 0"	14 0	14. 0"	13' 3"	14. 0.	14. 0.	14, D.	14' D"	12. 11.	14' D"	14' O"	14. 0.	10' 10"	12' 6"	12' 8'			10' 7"	12' 3"	12' 3"	12. 7.	GROUP B	BRACE **
CADAL DAY BAND BAND TOWN	CARLE BAIN BIRDONNE 104	CONTINUING BRABING /A		LIVE LOAD DEPLECTION CRY	CABLE INCOS D	d salida aldvo			1/2	6.8	SOUTHERN PINE		#1 & E	HEX-F	GROOM			C. R. P.	COLUS	\$53	DOUGLAS FIR-LARCH	-	STIME STATE	_11	GROUI		BRACING GROUP SPE		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR SBOJ
AT EACH END. MAX WEB

GABLE TRUSS

TOTAL LENGTH IS 14".

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DIT-L #2 OR
BETTER DIAGONAL
BRACE, SINGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

CONNECT DIAGONAL AT AMERICAL WEB.

BLE TRUSS DETAIL NOTES:

HEM-PIR FI & BIR GROUP B:

DOUGLAS FIR-LARCH

ASLE END SUPPORTS LOAD FROM 4' 0"
OUTLODKERS WITH E' 0" OVERHANG, OR 12"
PLYWOOD OVERHANG. e uplift connections for 180 plf over inuous bearing (6 psf to dead load). AD DEPLECTION CRITERIA IS L/240.

ATTACH EACH 'L' BRACE WITH 104 NAILS.

FOR (1) 'L' BRACE: SPACE NAILS AT 2° O.C.

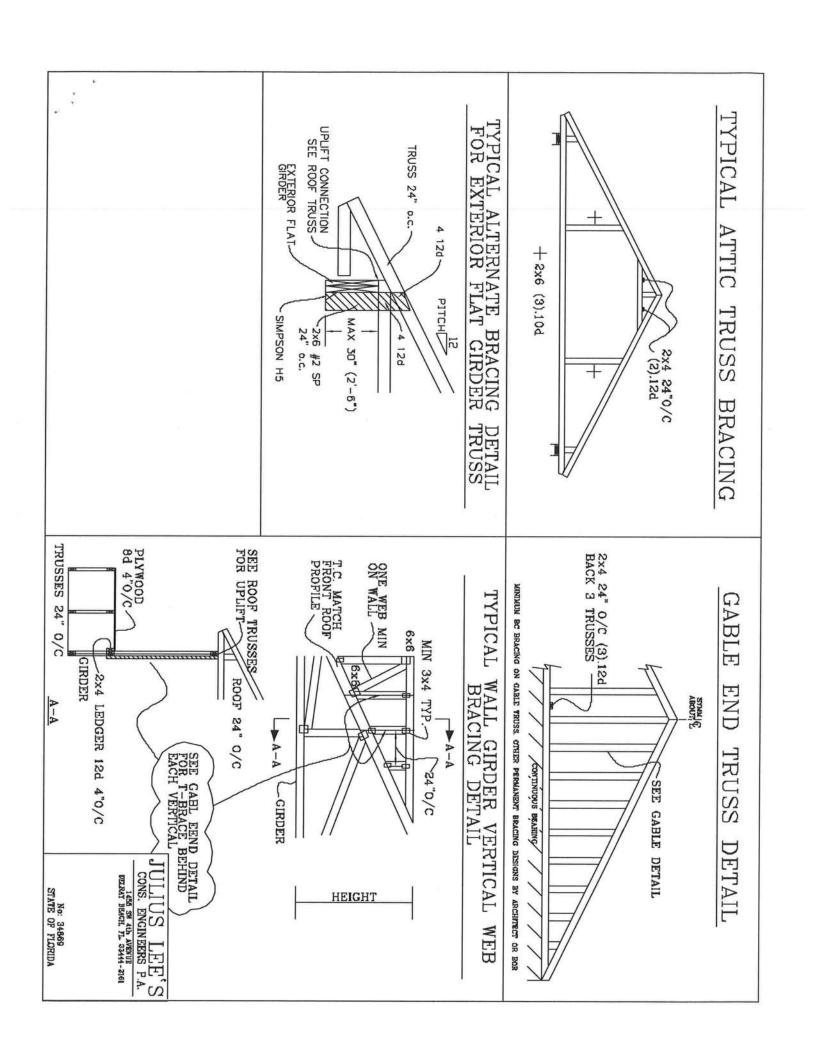
FOR (2) 'L' BRACE: SPACE NAILS AT 3° O.C.

FOR (2) 'L' BRACES: SPACE NAILS AT 3° O.C.

IN 18° END ZONES AND 6° O.C. BETWEEN ZDNES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH:

VERTICAL LENGTH 1
LESS THAN 4' 0" 1
GREATER THAN 4' D', BUT
LESS THAN 11' B' + REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. GREATER THAN 11' 6" GABLE VERTICAL PLATE SIZES NO SPLICE 2.5X4 22

			PLATE INSTITUTE, 388 D'ONDRIO DE, SUTTE 200, HAUSSIN, ME 537193 AND MICA (MODD TRUSS COLACIL OF ANERICA, 6300 ENTERPRISE LM, MAISSIN, ME 337193 FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNETINGS INVERT OFFICE INTEREST TO THE PROPERTY BEAUTY BROSSON ASSESSED.	MANARONGIM TRUSSIS REDUIRE EXTREME CARE IN FARRICATING, HANDLING, SUPPRING, INSTALLING AND BRACING. RETER TO BESI 1-03 CBUILDING COMPONENT SAFETY (MFDRANTION), PUBLISHED BY FPI CIRUSS	
No: 34869 STATE OF FLORIDA			DELRAY BEACH, PL 33444-2161	70	S, HHI SIIIIIII
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF			<u> </u>	
		-ENG	DWG MITEK STD GABLE 30' E H7	DATE 11/26/03	REF ASCE7-02-GAB13030



TOP CHORD BOT CHORD WEBS 2X4 2X4 800 222 BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

UP

30,

3

88 5

Ħ >

4XB 2X4

5X6

5X6

5X6 3X5 58

2.5X4

2.6X4

.5X3 5X4

1.5X4

1.5X4 5X6

TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

H U 0

4XB

5X5 .5X4

5X5

OR 3X6 TRULOX AT 4'

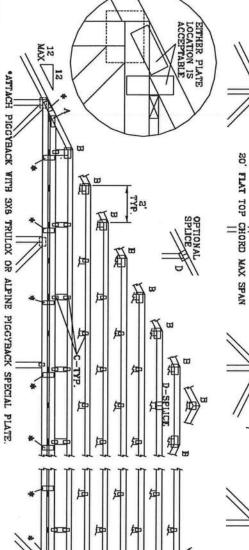
00,

ATTACH TRULOX PLATES WITH (8) 0.120 X 1.575 EQUAL PER FACE PER PLY. (4) NAILS IN EACH | BE CONNECTED. REFER TO DRAWING 160 TL FOR

NAILS, OR MEMBER TO

INFORMATION

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. MAX SIZE OF ZXIZ



	WEB BRACING CHART
WEB LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 84 NAILS AT 4" OC.
10' 70 14'	2x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LISS. * PICCYBACK SPECIAL PLATE N,

l	
	THIS
	DRAWING
	REPLACES
	DRAWINGS
	634,016
	634,017
	& 847.0
	45

8 1/4"

		NAVARHINGM: TRUSKIS REQUIRE EXTROE CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BACING. REFER TO DESIL IND GRILLING COMPOUNT SAFETY INFORMATION, PAIR CHICA BY TRY (TRUCK) PLATE INSTITUTE, 250 DEPOURDO IS, SUITE OR MADISON, V., 52779 AND HOTTO, NOTED REUSS COLUMNS OF AMERICA, 6500 INTERPRISE IN, HANDISON, VI, 327799 FOR SAFETY PRACTICES PRIGE TO DESIGN ON THE PROPERTY ATTACHED STRUCTURAL PARCIES AND BOTTOM CHOOD SHALL HAVE A PROPERTY ATTACHED RIGHT OF CHICA.
STATE OF FLORIDA		JULIUS LEE'S P.A.
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.
		MAX LOADING REF PIGGYBACK 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC. DATE 09/12/07 DRWGMITEK STD PIGGY

VALLEYTRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

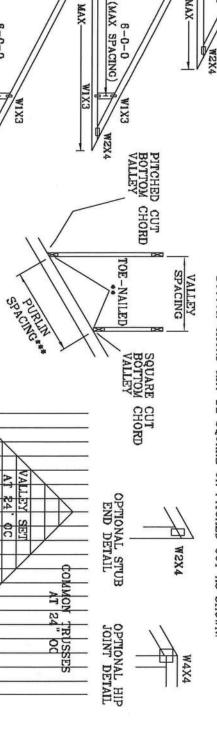
CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

12 NAX. WZX4

W2X4

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN NOT EXCEED 12'0".



W1X3

12 MAX.

12

W4X4

8-0-0 MAX

12 MAX.

12

W4X4

16-0-0 MAX-

W1X3

6-0-0

W1X3

W1X3 W5X4/SPL

(MAX SPACING

W1X3

W2X4

COMMON

TRUSSES

24

AT 24"

000

PARTIAL FRAMING PLAN

6-0-0

MAYARINGM. TRUSEES REQUIRE EXTREME CARE IN FABRICATING, HANDING, SHIPPING, INSTALLING AND BRADING. REFER TO BESEL HOU GRUIL DRIE (CEPONICHE SAFETY MEDIANTIDIN, PUBLICISED BY TPI (TRUSE PLAITE INSTITUTE, 582) DEPOCIATID BY, SUITE 280, MADISON, VJ. 53799 AND WICE AVOCID TRUSE COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VI. 53799 FOR SAFETY PRACTICES PRIDE TO PERFORMO, THESE FUNCTIONS, UNLESS OTHERWISE MORGATE, TPO EXPORT SHALL HAVE ROPERSLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING

20-0-0 MAX (++)

JULIUS LEE'S DELRAY BEACH, FL 35444-2161 No: 34869 STATE OF FLORIDA S BC LL BC TC TC SPACING DUR.FAC. 1.25 PL E DI 32 U 20 0 THIS DRAWING REPLACES DRAWING A105 20 24 1.25 40 0 PSF PSF PSF DRWG PSF DATE PSF REF -ENG VALTRUSS1103 11/26/03 VALLEY DETAIL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

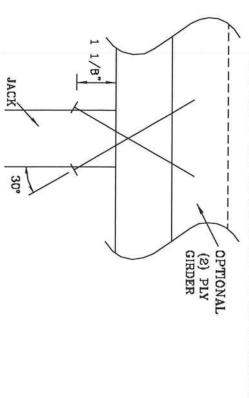
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

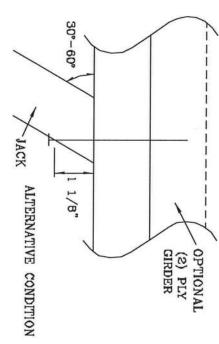
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

-	ט	4	ယ	N	TOE-NAILS	NUMBER OF
"	493#	394#	296#	187#	1 PLY	SOUTHE
	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
110011	452#	361#	271#	181#	1 PLY	DOUGLAS
1000	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
Попо	390#	312#	234#	156#	1 PLY	HEM-FIR
4.00	£07#	406#	304#	203#	2 PLIES	-FIR
70477	#VBE	307#	230#	154#	1 PLY	SPRUCE
#00#	#80k	397#	298#	189#	2 PLIES	SPRUCE PINE FIR

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL P	PLATE INSTITU	BRACING. REF				
NG. REPER TO BEST 1-03 CHUILING CHRPORNY SAFETY (NTOWNTIDE), SUPPING, INSTALLING AND NETTER TO BEST 1-03 CHUILING CHRPORNY SAFETY (NTOWNTIDE), PUBLISHED BY FRY CRUISS INSTITUTE, 583 PHOCHEL DE, SUTTE 260, ANDISON, VI. 53719) AND YEAR AUGUST PRIESS CONCIL. CRICA, 6300 ENTERPRIST, MAGISSM, VI. 33719) AND SAFETY PARCYTICES PRIENT OF PREPERNAL FOURTIDES. UNICESS OTHERWISE (NDICATED, TOP CHRB) SHALL HAVE PEPERLY ATTACHED RIGID CELLING.									
STATE OF FLORIDA	No: 34889			DELRAY BEACH, FL 83444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S			
SPACING	DUR. FA	TOT. LD.	BC LL	BC DL	TC DL	TC LL			
	FAC. 1.00								
		PSF	PSF	PSF	PSF	PSF REF			
			-ENG JL	DRWG	DATE	REF			
			JL	CNTONAIL1103	DATE 09/12/07	TOE-NAIL			

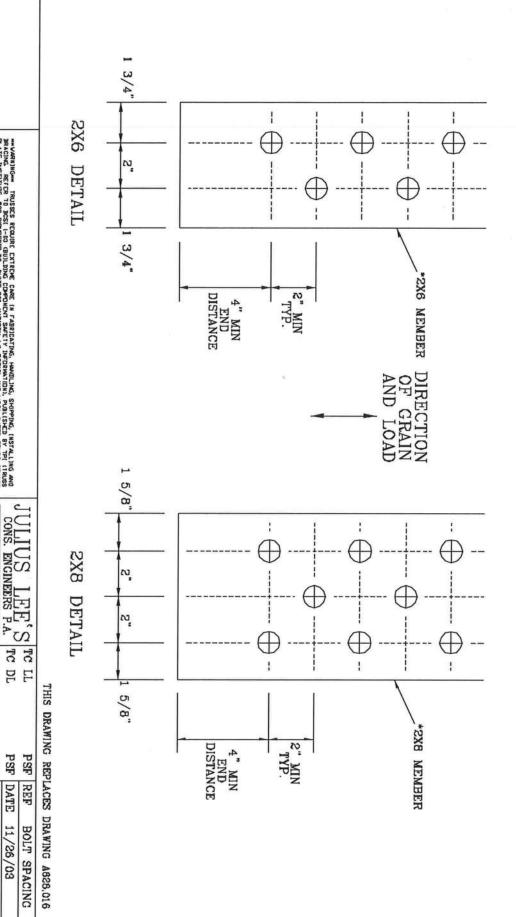
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



DELRAY BEACH, FL 33444-2161

BC DL

DRWG -ENG

CNBOLTSP1103

BC LL

PSF PSF

No: 34869 STATE OF FLORIDA

SPACING

TOT. LD.

TRULOX CONNECTION

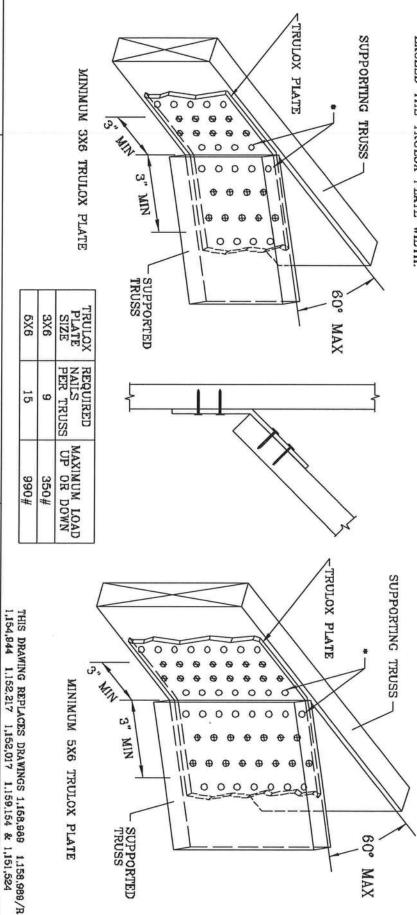
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST NAILS MAY BE OMITTED FROM THESE ROWS EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER

MAX



3

JULIUS LEE'S DELRAY BEACH, IL 33444-2151

LEE'S

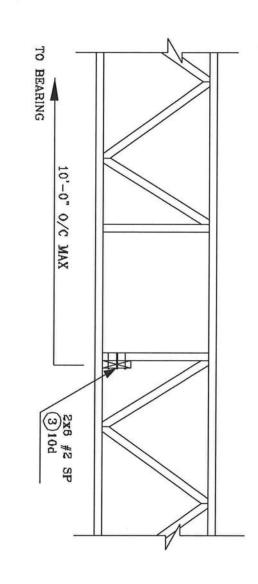
DRWG DATE REF

CNTRULOX1103 11/26/03 TRULOX

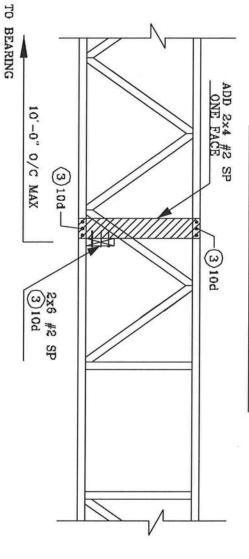
-ENG

No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

1455 SM 4th AMENUE
1455 SM 4th AMENUE
1455 SM 4th AMENUE
1455 SM 4th AMENUE
1555 SM 4th AMEN

No: 34869 STATE OF FLORIDA

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. = 26616 Section 1: General Information (Treating Company Information) Company Name: __Aspen Peat Control, Inc. Company Address: 301 NW Colo Tarraco City State Zip Company Business License No. ________ Company Phone No. _ FHA/VA Case No. (if any) _ Section 2: Builder Information Section 3: Property Information Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) Type of Construction (More than one box may be checked) Slab ☐ Basement ☐ Crawl Other Approximate Depth of Footing: Outside ___ Type of Fill Section 4: Treatment Information Date(s) of Treatment(s) Brand Name of Product(s) Used EPA Registration No. _ Approximate Final Mix Solution % Approximate Size of Treatment Area: Sg. ft. Linear ft. Linear ft. of Masonry Voids Approximate Total Gallons of Solution Applied ☐ Yes No Was treatment completed on exterior? Yes ☐ No Service Agreement Available? Note: Some state laws require service agreements to be issued. This form does not preempt state law. Attachments (List) _ Comments Certification No. (if required by State law) Name of Applicator(s) The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Authorized Signature



COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 19-4S-17-08566-000

Building permit No. 000026616

Use Classification SFD,UTILITY

Permit Holder B.TRENT GIEBEIG

Owner of Building JAMES PETTYJOHN

1811 SW CR 242-A, LAKE CITY, FL Location:

Date: 04/25/2008

173.76

Total:

Waste: 100.50

Fire:

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

