DATE 02/15/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026765

APPLICANT MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

ADDRESS 1924 SW NAUTILUS ROAD LAKE CITY FL 32024

OWNER MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

ADDRESS 1924 SW NAUTILUS ROAD LAKE CITY FL 32024

CONTRACTOR MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

LOCATION OF PROPERTY 47-S TO WALTER LITTLE RD,TL CROSS OVER C-240 TO OLD WIRE RD
TO NAUTILUS,TL AROUND CURVE ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 171300.00

HEATED FLOOR AREA 3426.00 TOTAL AREA 4559.00 HEIGHT 20.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH _____ FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 24-5S-16-03706-004 SUBDIVISION _____

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 1.00

000001558 _____ _____ Schenetter Desravines

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

WAIVER 07-0823 BLK JTH Y

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD. LEGAL NON-CONFORMING LOT OF RECORD.

Check # or Cash 1049

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 860.00 CERTIFICATION FEE $ 22.80 SURCHARGE FEE $ 22.80

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ _____ **TOTAL FEE** 980.60

INSPECTORS OFFICE _____ CLERKS OFFICE MSS

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

980.60 | 50.00

For Office Use Only Application # 0710-18 Date Received 10/9/07 By G Permit # 1558 / LC765

Application Approved by - Zoning Official BLK Date 10.10.07 Plans Examiner OK JTH Date 12-12-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments See sheet 1 OF PLAN FOR SITE PLAN JTH Legal Non-Conforming Lot of Record

EHV - NOC 12-12-07 Received

Dropped off by Linda Roder 752-2281 FAX 752-2282

Applicants Name MOISES DESRAVINES Phone 561 432-5425

Address 1924 SW NAUTILUS ROAD LAKE CITY, FL 32024

Owners Name ~~1924~~ SAME Phone 752-3979

911 Address ~~1924~~ 1980 SW NAUTILUS ROAD LAKE CITY, FL 32024

Contractors Name owner builder Phone 752-3979

Address 1924 SW Nautilus Rd Lake City FL 32024

Fee Simple Owner Name & Address MOISES DESRAVINES 4609 Bowman St. Lake Worth FL 33463

Bonding Co. Name & Address none

Architect/Engineer Name & Address PRECISION DRAFTING + DESIGN, INC 34 VISTA DEL RIO BOYNTON, FL

Mortgage Lenders Name & Address none

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 24-5S-16-03706-004 Estimated Cost of Construction $110,000

Subdivision Name N/A Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 47 S, L on Walter Little Road, Cross 240, onto Old Wire Road, L on Nautilus, around curve on right

Type of Construction Residential SFD Number of Existing Dwellings on Property 0

Total Acreage 1 Lot Size 1 acre Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front ~~30'~~ Side ~~42'~~ Side ~~42'~~ Rear ~~42'~~

Total Building Height 20' Number of Stories 1 Heated Floor Area ~~3585~~ 3426 Roof Pitch 1/4

TOTAL 4559

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. CK# $1049 (1050)

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

X Moise Derravines

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 10 day of October 2007.

Personally known ____ or Produced Identification ✓

Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

Contractor Signature

Contractors License Number

Competency Card Number

NOTARY STAMP/SEAL

Linda R. Roder

Notary Signature

left message with

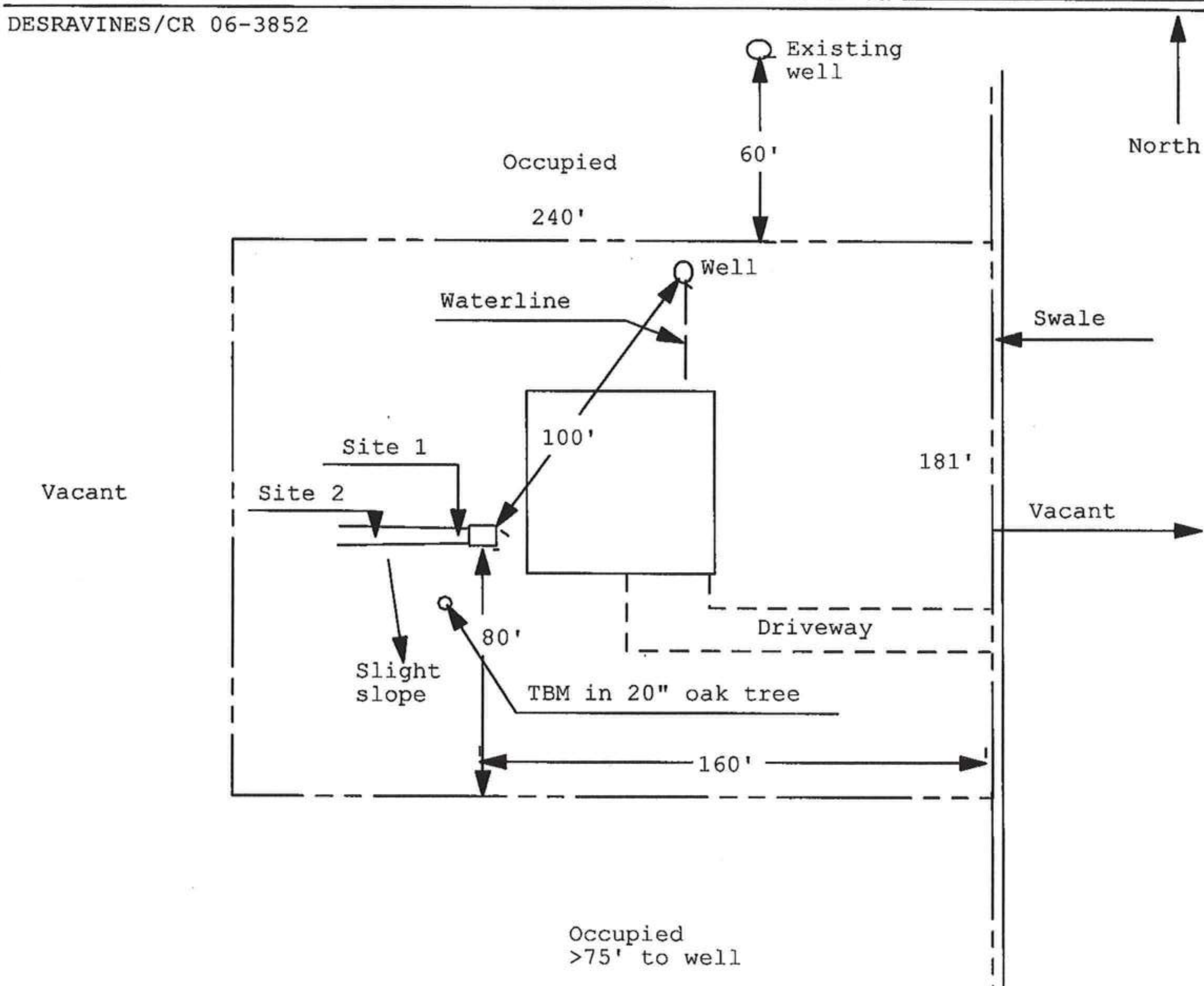
07-1018

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-0823

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

DESRAVINES/CR 06-3852



1 inch = 50 feet

Site Plan Submitted By Paul Lloyd **Date** 1/24/07

Plan Approved ✓ **Not Approved** ____ **Date** 10-22-07

By [signature] Columbia **CPHU**

Notes:

North FL Permit
382 SW Kemp ct
Lake City FL 32024

07-1018

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 24-5S-16-03706-004

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes. the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: 1924 SW Nautilus Lake City FL 32024
   a) Street *(job)* Address:
2. General description of improvements: single family dwelling
3. Owner Information
   a) Name and address: Moises Desravines 1924 SW Nautilus Lake City FL 32024
   b) Name and address of fee simple titleholder (if other than owner) NA
   c) Interest in property home site
4. Contractor Information
   a) Name and address: Owner builder Moises Desravines
   b) Telephone No.: Fax No. (Opt.)
5. Surety Information
   a) Name and address: NA
   b) Amount of Bond:
   c) Telephone No.:

Inst:200712025966 Date:11/26/2007 Time:10:39 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 1

6. Lender
   a) Name and address: NA
   b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address: NA
   b) Telephone No.: Fax No. (Opt.)

8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
   a) Name and address: NA
   b) Telephone No.: Fax No. (Opt.)

9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified):**

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

10. Moises Desravines
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Moises Desravines
Print Name

The foregoing instrument was acknowledged before me , a Florida Notary, this 21 day of November, 20 07, by:
Linda Roder as notary **(type of authority, e.g. officer, trustee, attorney fact) for** **(name of party on behalf of whom instrument was executed).**

Personally Known ____ OR Produced Identification ✓ Type DL

Notary Signature Linda R. Roder Notary Stamp or Seal:

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Prepared as to form only
No title search
PREPARED BY/RETURN TO:
John J. Kendron
ROBINSON, KENNON & KENDRON, P.A.
P. O. Box 1178
Lake City, FL 32056-1178

Inst:2006028971 Date:12/08/2006 Time:16:00
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1104 P:986

QUITCLAIM DEED

THIS INDENTURE, made the 7th day of December, in the year 2006, between **LUCY MAE LEWIS**, an unremarried widow, of the State of Florida, whose post office address is 2065 SW Nautilus Road, Lake City, Florida 32024, as party or parties of the first part, hereinafter called Grantor, and **MOISE M. DESRAVINES** and **SCHENETTER G. DESRAVINES,** his wife, whose mailing address is 1924 SW Nautilus Road, Lake City, Florida 32024, as party or parties of the second part, hereinafter called Grantees(the words "Grantor" and "Grantee" to include their respective heirs, legal representatives, successors, and assigns where the context requires or permits).

WITNESSETH THAT: Grantor, for and in consideration of the sum of $10.00 and other valuable considerations, in hand paid at and before the sealing and delivery of this Deed, the receipt of which is hereby acknowledged, by these presents does hereby remise, convey, and forever QUITCLAIM unto the Grantees the following described land, lying, situate and being in **Columbia** County, Florida, to wit:

TOWNSHIP 5 SOUTH, RANGE 16 EAST

SECTION 24: A part of the North ½ of the NW ¼ of Section 24, Township 5 South, Range 16 East, more particularly described as:
COMMENCE at the SE corner of the North ½ of the NW ¼ of said Section 24 and run N 0°48'14" E along the East line thereof, 181.5 feet for a POB; thence continue N 0°48'14" E, 181.5 feet; thence N 89°41'42" W, 240.0 feet; thence S 0°48'14" W, 181.5 feet; thence S 89°41'42" E, 240.0 feet to the POB, Columbia County, Florida.

PARCEL NO. 24-5S-16-03706-004

TO HAVE AND TO HOLD the described premises to Grantees, so that neither Grantor nor any person or persons claiming under Grantor shall at any time, by any means or ways, have, claim, or demand any right or title to the premises or appurtenances, or any rights thereof.

IN WITNESS WHEREOF, the Grantor has signed and sealed this Deed on the date written above.

Signed, sealed, and delivered in the presence of:

Witness
JOHN J KENDRON
Print Witness Name

Witness
MARY DRIGGERS
Print Witness Name

Lucy Mae Lewis
LUCY MAE LEWIS, Grantor

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of December, by **LUCY MAE LEWIS**, who is personally known to me or who has produced L200-522-31-701-0 as identification.

NOTARY SIGNATURE

(SEAL)

John J. Kendron
MY COMMISSION # DD233329 EXPIRES
July 20, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(✓) Single Family Dwelling
( ) Two-Family Residence
( ) Farm Outbuilding
( ) Other_____
( ) New Construction
( ) Addition, Alteration, Modification or other Improvement

NEW CONSTRUCTION OR IMPROVEMENT

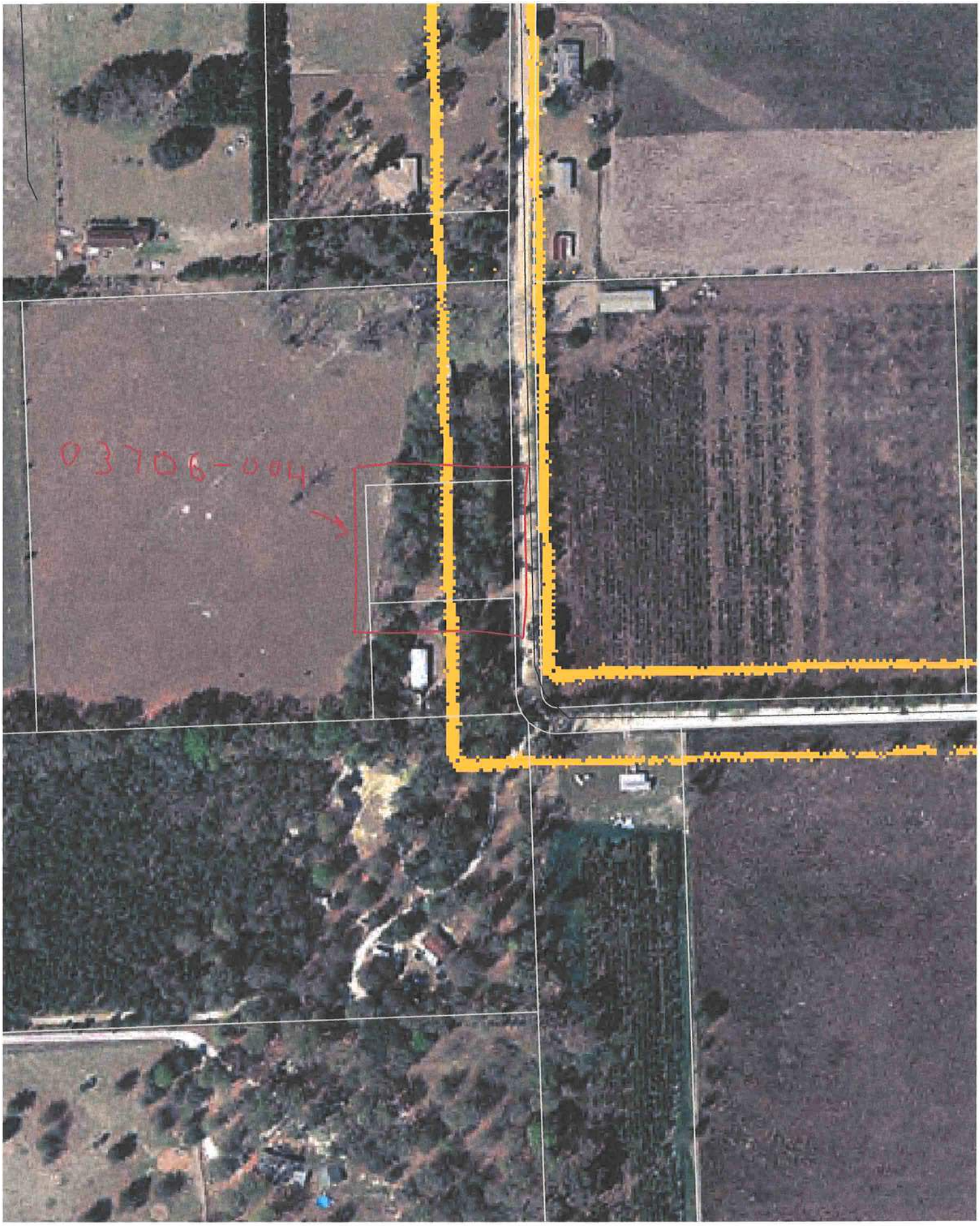
I MOISES DESRAVINES , have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number_____

Moise Desravines
Signature Date

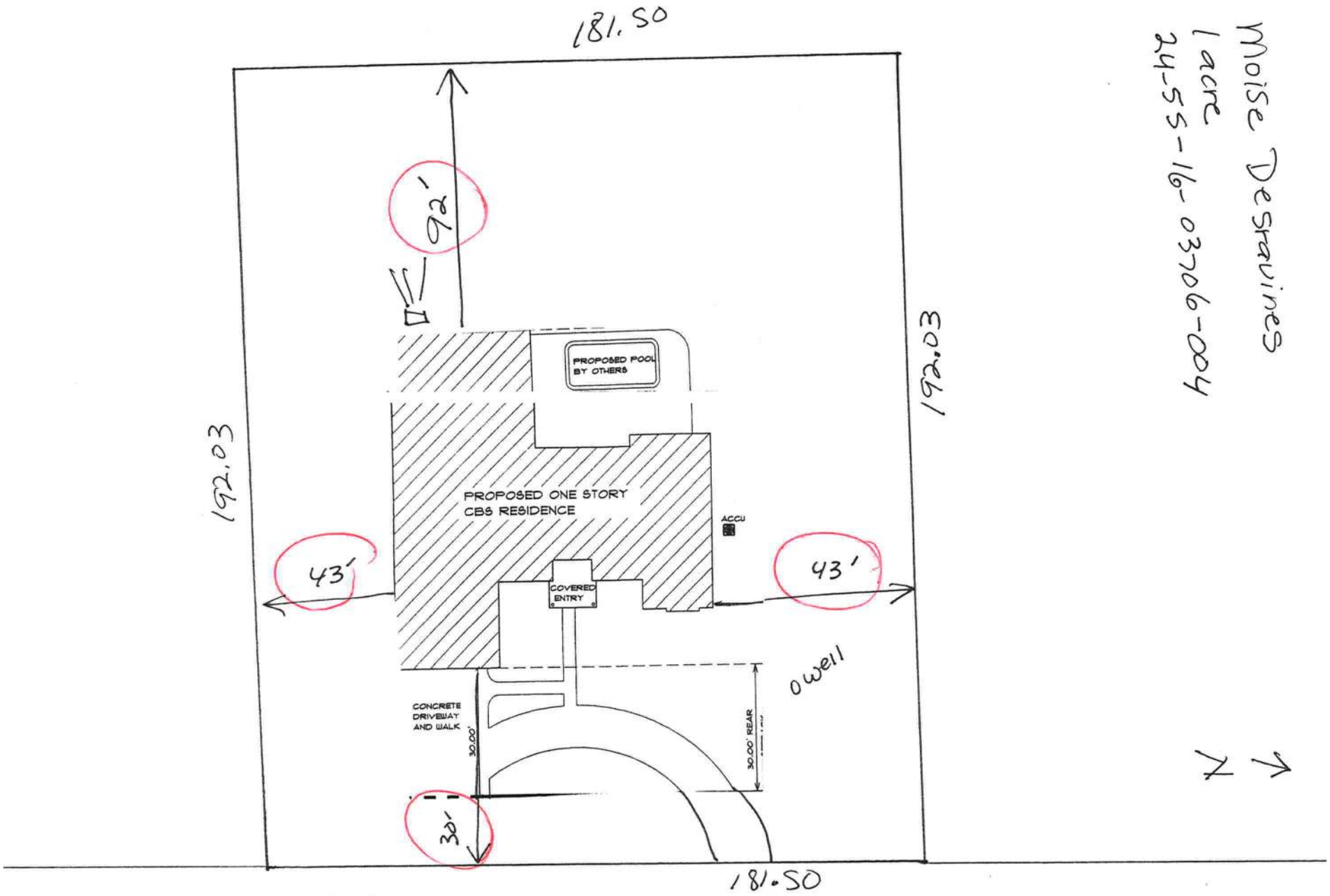
FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 10-10-07 Building Official/Representative_____



0710-18


181.50
192.03
192.03
181.50
92'
PROPOSED POOL
BY OTHERS
PROPOSED ONE STORY
CBS RESIDENCE
ACCU
43'
43'
COVERED
ENTRY
owell
CONCRETE
DRIVEWAY
AND WALK
30.00'
30.00' REAR
30'
SW Nautilus Rd
Moise Desravines
1 acre
24-55-16-03706-004
N

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1634
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

This instrument was prepared by:

PREPARED BY:
SMITH & SMITH, P.A.
P. O. BOX 1792
LAKE CITY, FLA. 32055

Warranty Deed (STATUTORY FORM — SECTION 689.02 F.S.)

This Indenture, Made this 2nd day of July 19 80 **Between**

DOREATHA GAINER of the County of Columbia, State of Florida, grantor*, and ELIZABETH HILL YOUNG whose post office address is Lake City of the County of Columbia, State of Florida, grantee*.

BOOK 460 PAGE 27 OFFICIAL RECORDS

Witnesseth, That said grantor, for and in consideration of the sum of TEN AND NO/100-----------($10.00)---Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 5 SOUTH - RANGE 16 EAST

SECTION 24: A part of the North 1/2 of the NW 1/4 of Section 24, Township 5 South, Range 16 East, more particularly described as follows:

COMMENCE at the SE Corner of the North 1/2 of the NW 1/4 of said Section 24 and run N 0°48'14" E, along the East line thereof 181.5 feet for a POB. Thence continue N 0° 48'14" E, 181.5 feet; thence N 89°41'42" W, 240.0 feet; thence S 0°48'14" W, 181.5 feet; thence S 89°41'42"E; 240.0 feet to the POB. Columbia County, Florida; Containing 1.0 acres more or less. Subject to existing road occupational right-of-way, 1980 taxes, visible easements, recorded easemtns and applicable zoning regulations, if any.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

* "Grantor" and "grantee" are used for singular or plural, as context requires.

In Witness Whereof, Grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Clanna B. Brown (witness signature)
Dorothy E. Snyder (witness signature)

Doreatha Gainer (Seal)
DOREATHA GAINER
(Seal)
(Seal)
(Seal)

FILED FOR RECORD '81 JAN -2 P3:00 OR 460 PAGE 27 810014

STATE OF FLORIDA
COUNTY OF COLUMBIA

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared DOREATHA GAINER to me known to be the person described in and who executed the foregoing instrument and acknowledged before me that she executed the same.

WITNESS my hand and official seal in the County and State last aforesaid this 2nd day of July 19 80.

Dorothy E. Snyder
Notary Public

My commission expires:

NOTARY PUBLIC STATE OF FLORIDA AT LARGE
MY COMMISSION EXPIRES SEPT. 19 1983
BONDED THRU GENERAL INS. UNDERWRITERS

STATE OF FLORIDA DOCUMENTARY STAMP TAX
DEPT. OF REVENUE JAN-2'81 PB. 11120 = 0 4. 0 0
0 3 0 1 2 2

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0710-18

Moise Desravines Contractor, Owners/Builder: Property ID# 24-5s-16-03706-004

On the date of October 10, 2007 application 0710-18 and plans for construction of a single family dwelling were reviewed. The following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0710-18 and when making reference to this application.

1. The wind design information, note number seven requires the minimum soil bearing pressure to equal 2,500 pound per square foot. The Florida Building Code table 1804.2 establishes the allowable load-bearing values of soils for foundation. Soils such as sand, silty sand, clayey sand, silty gravel and clayey gravel are general classification of soils in the area which the structure will be constructed upon. The code provides that a maximum of 2,000 pound per square foot as the load bearing capacity of these type soils. To comply with the table 1804.2 please have Mr. Luis Lopez your structural engineer design a foundation which will sufficiently support the structure within a load bearing capacity of 2,000 pound per square foot. Are have a registered professional conduct subsurface explorations at the project site upon which foundations are to be constructed, a sufficient number (not less

than four, one boring on each corner of the building foundation) borings should be made to a depth of not less than 10 feet below the level of the foundations to provide assurance of the soundness of the foundation bed and its load-bearing capacity.

2. On sheet three of the plans identify which windows in each bedroom will serve as emergency escape and rescue openings. Show compliance with section R310 of the Florida Residential Code.

   R310.1 Emergency escape and rescue required.

   Basements with habitable space and every sleeping room shall have at least one operable emergency escape and rescue opening. Where basements contain one or more sleeping rooms, emergency egress and rescue openings shall be required in each sleeping room, but shall not be required in adjoining areas of the basement. Where emergency escape and rescue openings are provided they shall have a sill height of not more than 44 inches (1118 mm) above the floor. Where a door opening having a threshold below the adjacent ground elevation serves as an emergency escape and rescue opening and is provided with a bulkhead enclosure, the bulkhead enclosure shall comply with Section 310.3. The net clear opening dimensions required by this section shall be obtained by the normal operation of the emergency escape and rescue opening from the inside. Emergency escape and rescue openings with a finished sill height below the adjacent ground elevation shall be provided with a window well in accordance with Section R310.2. The emergency escape and rescue opening shall be permitted to open into a screen enclosure, open to the atmosphere, where a screen door is provided leading away from the residence.

   R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m2).

   Exception: Grade floor openings shall have a minimum net clear opening of 5 square feet (0.465 m2).

   R310.1.2 Minimum opening height: The minimum net clear opening height shall be 24 inches (610 mm).

   R310.1.3 Minimum opening width: The minimum net clear opening width shall be 20 inches (508 mm).

   R310.1.4 Operational constraints: Emergency escape and rescue openings shall be operational from the inside of the room without the use of keys or tools.

3. Show on the foundation plans the required lintel length and type for each opening (windows, egress doors (garage doors) within the load bearing concrete block walls.

4. The typical wall section on sheet six of the plans requires each truss to be anchored and references sheet nine of the plans . Sheet nine of the plans were not submitted with this application. Please verify the anchor up lift capacity for each roof truss. The typical wall section shows that the roof covering will be cement barrel tiles. Verify in writing that the trusses are so design to support these cement barrel tiles. Structural note: The structure is design for a 140 MPH wind speed; the trusses are design only to withstand a 110 MPH wind speed.
5. Within the master bath truss system the truss layout shows a section of the truss system will be field framed. Please submit a drawing detailing this roof truss section. Include the lumber type, size and anchoring method which will be used to field frame this roof truss section.
6. In reference to the electrical riser detail shown on sheet seven of the plans. Please note that an electrical service overcurrent protection device for the main electrical service is required. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

Project Information for:	L213487		
Builder:	DESRAVINES (O/B)	Date:	1/24/2007
Lot:	N/A	Start Number:	**1205**
Subdivision:	1924 SW NAUTILUS RD		
County or City:	COLUMBIA COUNTY		L213487
Truss Page Count:	24		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**60**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

Owner Builder
Address: N/A
N/A
Designer: **104**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0124071205	1/24/2007				
2	CJ1A	0124071206	1/24/2007				
3	CJ3	0124071207	1/24/2007				
4	CJ3A	0124071208	1/24/2007				
5	CJ5	0124071209	1/24/2007				
6	CJ5A	0124071210	1/24/2007				
7	EJ4	0124071211	1/24/2007				
8	EJ7	0124071212	1/24/2007				
9	EJ7A	0124071213	1/24/2007				
10	EJ7B	0124071214	1/24/2007				
11	EJ7C	0124071215	1/24/2007				
12	EJ7D	0124071216	1/24/2007				
13	HJ3	0124071217	1/24/2007				
14	HJ9	0124071218	1/24/2007				
15	HJ9A	0124071219	1/24/2007				
16	T01	0124071220	1/24/2007				
17	T01	0124071221	1/24/2007				
18	T02	0124071222	1/24/2007				
19	T03	0124071223	1/24/2007				
20	T04	0124071224	1/24/2007				
21	T05	0124071225	1/24/2007				
22	T06	0124071226	1/24/2007				
23	T07	0124071227	1/24/2007				
24	T08	0124071228	1/24/2007				

JAN 2 4 2007

Dwg.#0124071205

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ1	JACK	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:27 2006 Page 1

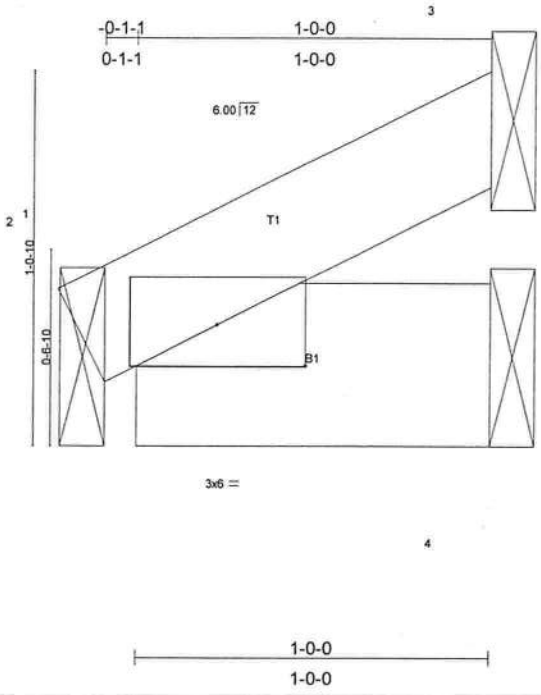


Plate Offsets (X,Y): [2:0-3-0,0-1-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.02	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.00	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 4 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=68/Mechanical, 4=15/Mechanical, 3=43/Mechanical
Max Horz 2=30(load case 5)
Max Uplift2=-16(load case 5), 4=-9(load case 3), 3=-29(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4/0, 2-3=-29/19
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 2, 9 lb uplift at joint 4 and 29 lb uplift at joint 3.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071206

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ1A	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:32 2006 Page 1

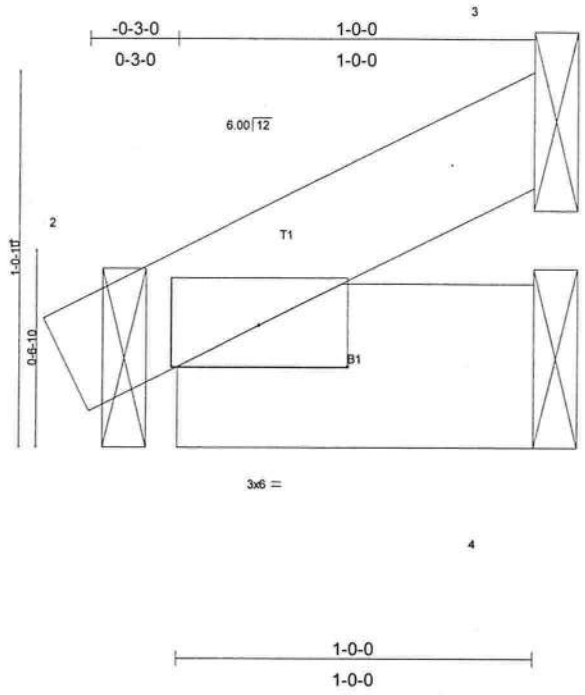


Plate Offsets (X,Y): [2:0-3-0,0-1-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.02	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.00	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 5 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=93/Mechanical, 4=15/Mechanical, 3=39/Mechanical
Max Horz 2=30(load case 5)
Max Uplift2=-27(load case 5), 3=-25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-0/0, 2-3=-27/17
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 2 and 25 lb uplift at joint 3.

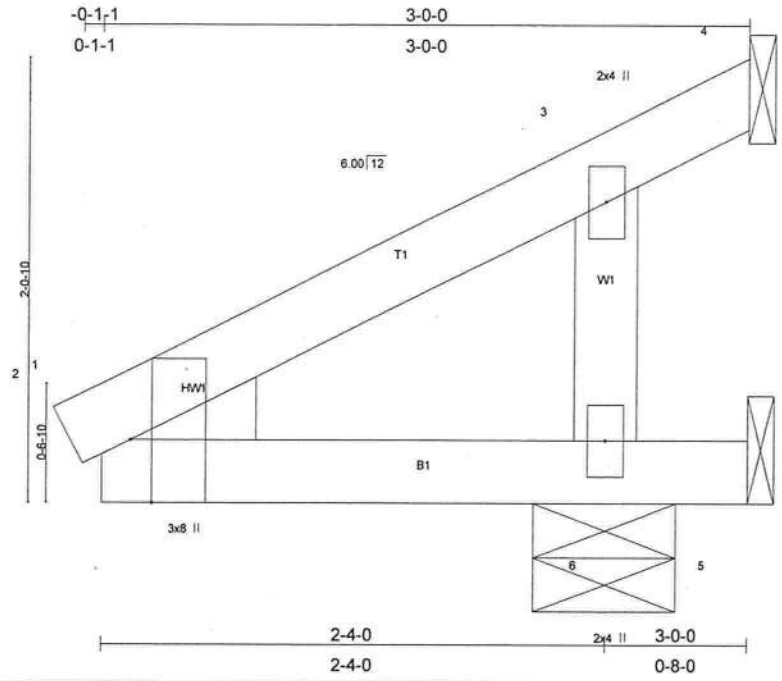
LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071207

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ3	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:36 2006 Page 1



Scale = 1:10.1

*Uplift in gravity load case

Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.00	6	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	0.00	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.05	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-259/Mechanical, 5=-276/Mechanical, 6=895/0-8-0
Max Horz 6=79(load case 5)
Max Uplift4=-259(load case 1), 5=-276(load case 1), 6=-280(load case 5)
Max Grav 4=66(load case 5), 5=63(load case 5), 6=895(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-110/93, 3-4=-140/69
BOT CHORD 2-6=-0/119, 5-6=0/0
WEBS 3-6=-493/353

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 259 lb uplift at joint 4, 276 lb uplift at joint 5 and 280 lb uplift at joint 6.

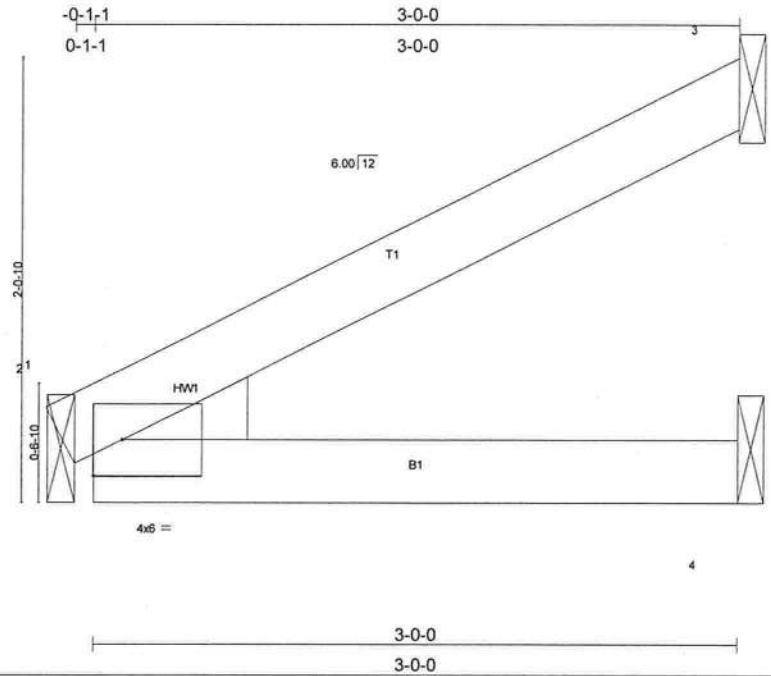
LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071208

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ3A	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:40 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=129/Mechanical, 2=184/Mechanical, 4=43/Mechanical
Max Horz 2=79(load case 5)
Max Uplift3=-78(load case 5), 2=-56(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7/0, 2-3=-74/53
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 3, 56 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071209

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ5	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:44 2006 Page 1

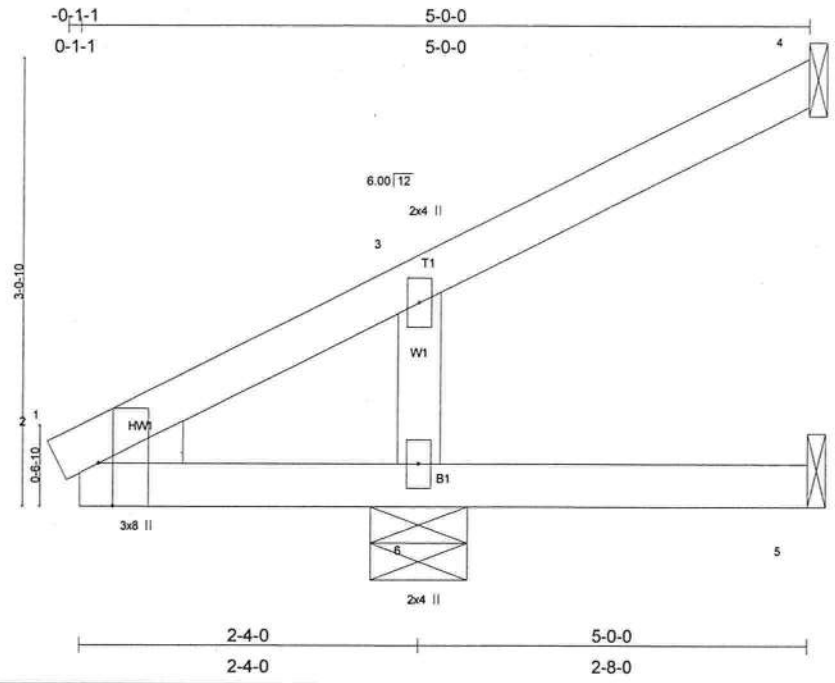


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.00	5-6	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	0.01	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.08	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=46/Mechanical, 5=-22/Mechanical, 6=576/0-8-0
Max Horz 6=125(load case 5)
Max Uplift4=-55(load case 5), 5=-38(load case 6), 6=-198(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-182/95, 3-4=-84/10
BOT CHORD 2-6=-0/188, 5-6=0/0
WEBS 3-6=-373/268

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 4, 38 lb uplift at joint 5 and 198 lb uplift at joint 6.

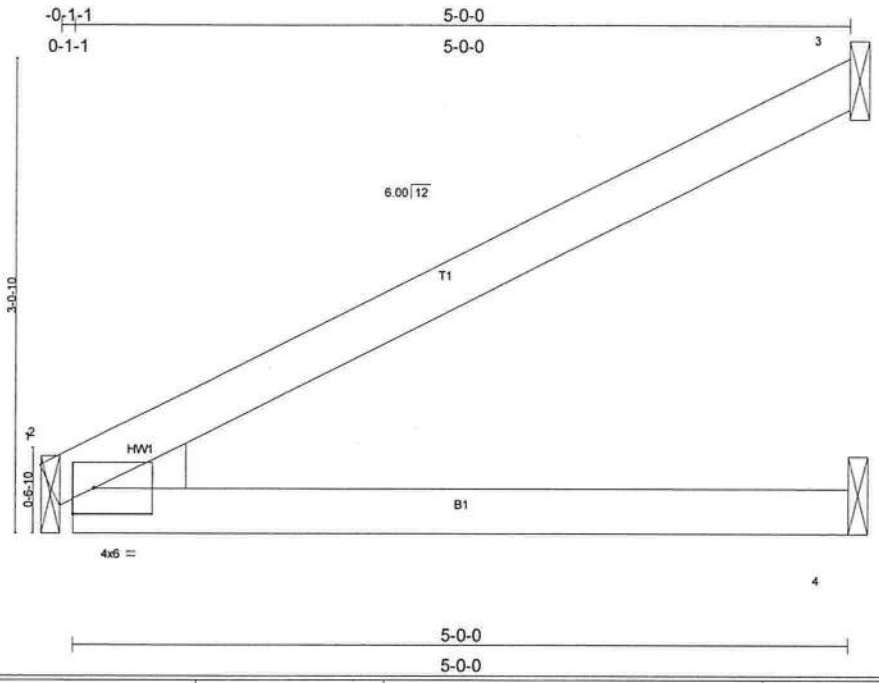
LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071210

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	CJ5A	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:49 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.09	2-4	>629	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.08	2-4	>735	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=219/Mechanical, 2=304/Mechanical, 4=73/Mechanical
Max Horz 2=125(load case 5)
Max Uplift3=-127(load case 5), 2=-102(load case 5), 4=-47(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7/0, 2-3=-116/85
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 3, 102 lb uplift at joint 2 and 47 lb uplift at joint 4.

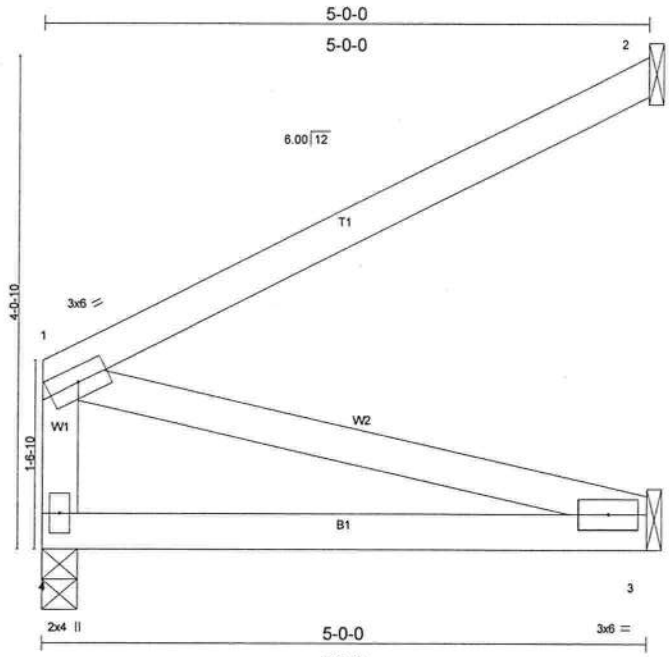
LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071211

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ4	MONO TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:53 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.03	3-4	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=216/Mechanical, 4=288/0-3-8, 3=73/Mechanical
Max Horz 4=110(load case 5)
Max Uplift2=-118(load case 5), 4=-18(load case 5), 3=-15(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-100/77, 1-4=-216/59
BOT CHORD 3-4=-166/0
WEBS 1-3=-0/172

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 2, 18 lb uplift at joint 4 and 15 lb uplift at joint 3.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071212

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ7	MONO TRUSS	42	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:53:59 2006 Page 1

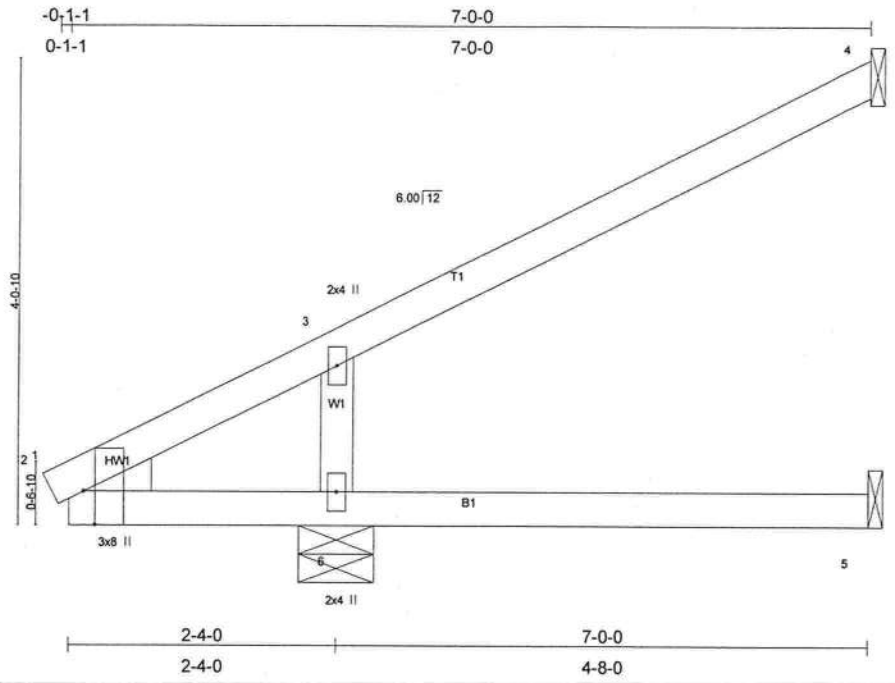


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.06	5-6	>992	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	0.05	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.07	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=157/Mechanical, 5=44/Mechanical, 6=639/0-8-0
Max Horz 6=171(load case 5)
Max Uplift4=-110(load case 5), 5=-58(load case 6), 6=-226(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-170/107, 3-4=-115/52
BOT CHORD 2-6=-0/171, 5-6=0/0
WEBS 3-6=-464/198

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 4, 58 lb uplift at joint 5 and 226 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071213

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ7A	SPECIAL	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:04 2006 Page 1

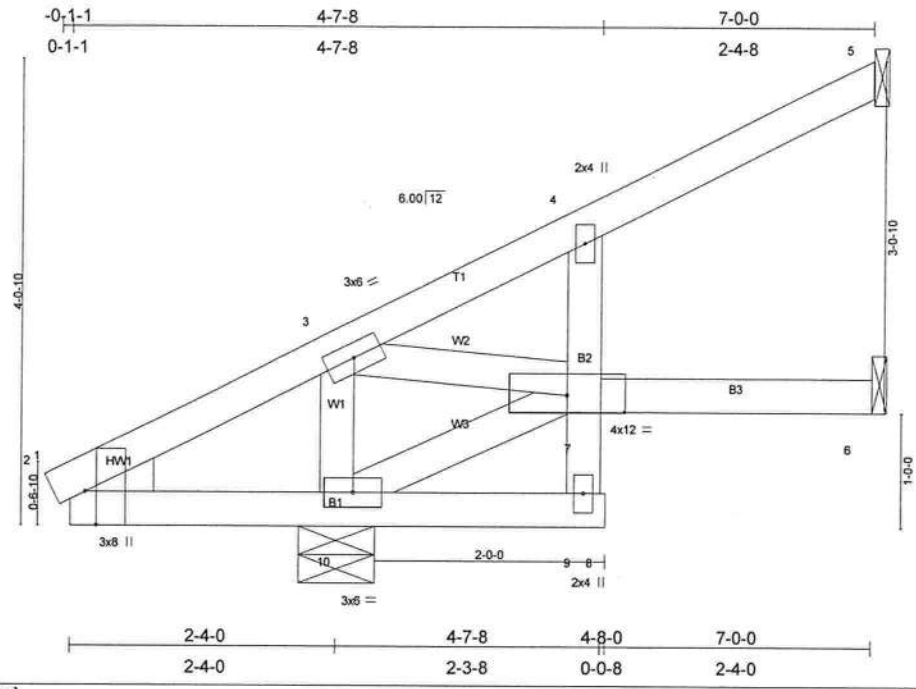


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.03	8	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.04	8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=128/Mechanical, 6=76/Mechanical, 10=642/0-8-0
Max Horz 10=171(load case 5)
Max Uplift5=-79(load case 5), 6=-29(load case 5), 10=-168(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-161/274, 3-4=-139/11, 4-5=-44/48
BOT CHORD 2-10=-181/156, 9-10=-36/38, 8-9=0/0, 7-9=0/24, 4-7=-127/62, 6-7=-4/2
WEBS 7-10=-248/23, 3-10=-424/157, 3-7=-42/259

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 5, 29 lb uplift at joint 6 and 168 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071214

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ7B	SPECIAL	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:09 2006 Page 1

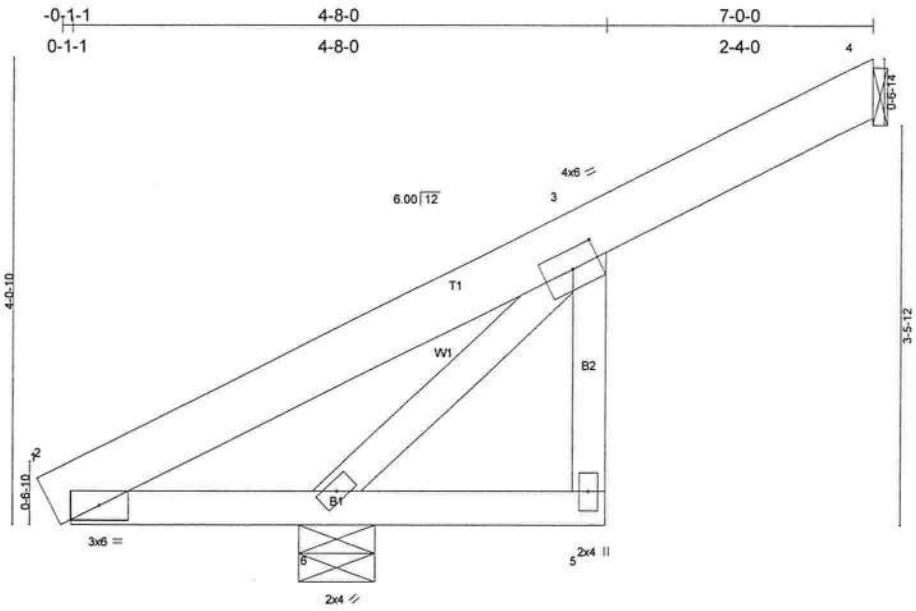


Plate Offsets (X,Y): [3:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.04	3	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.07	5	>846	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=203/Mechanical, 6=615/0-8-0
Max Horz 6=167(load case 5)
Max Uplift4=-122(load case 5), 6=-165(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3/0, 2-3=-290/616, 3-4=-64/79
BOT CHORD 2-6=-491/305, 5-6=0/0, 3-5=0/18
WEBS 3-6=-705/231

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 4 and 165 lb uplift at joint 6.
4) Load case(s) 1 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-50, 2-3=-90, 3-4=-110(F=-20), 2-5=-30

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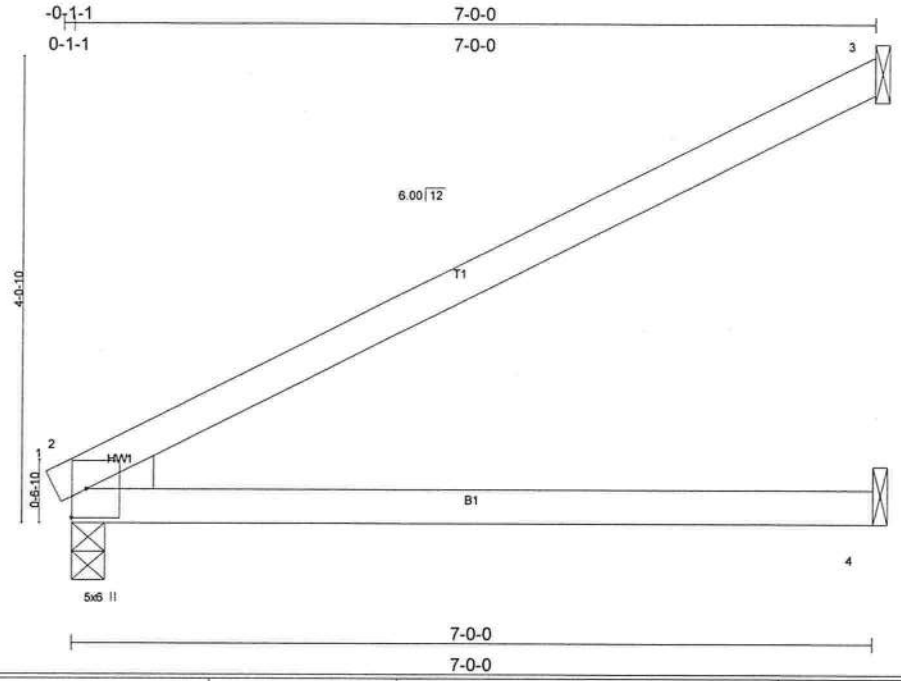
Dwg.#0124071215

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ7C	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:13 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	-0.15	2-4	>536	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.35	2-4	>235	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=277/Mechanical, 2=423/0-3-8, 4=130/Mechanical
Max Horz 2=171(load case 5)
Max Uplift3=-149(load case 5), 2=-58(load case 5), 4=-5(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7/0, 2-3=-160/103
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 3, 58 lb uplift at joint 2 and 5 lb uplift at joint 4.

LOAD CASE(S) Standard

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Dwg.#0124071216

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	EJ7D	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:18 2006 Page 1

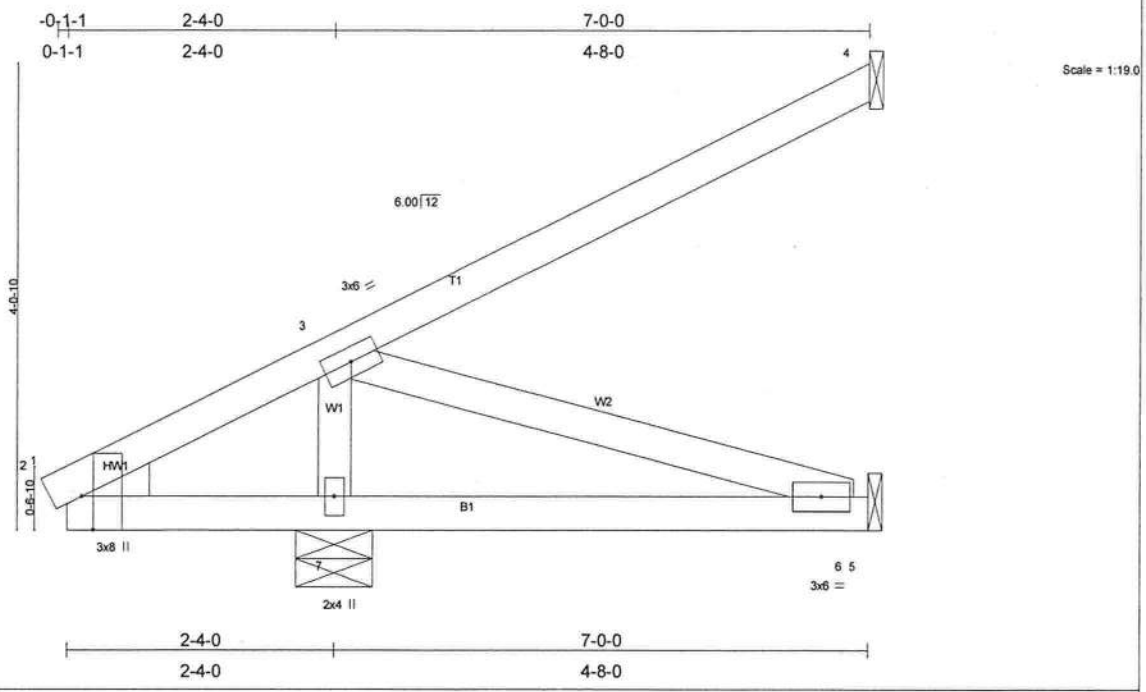

Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.01	6-7	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.02	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=166/Mechanical, 7=879/0-8-0, 6=-48/Mechanical
Max Horz 7=171(load case 5)
Max Uplift4=-91(load case 5), 7=-232(load case 5), 6=-48(load case 1)
Max Grav 4=166(load case 1), 7=879(load case 1), 6=10(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-210/476, 3-4=-111/56
BOT CHORD 2-7=-324/183, 6-7=-324/38, 5-6=0/0
WEBS 3-7=-733/255, 3-6=-40/339

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 4, 232 lb uplift at joint 7 and 48 lb uplift at joint 6.
4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 159 lb down and 42 lb up at 0-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-50, 2-4=-90, 2-6=-30
Concentrated Loads (lb)
Vert: 2=-159(F)

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
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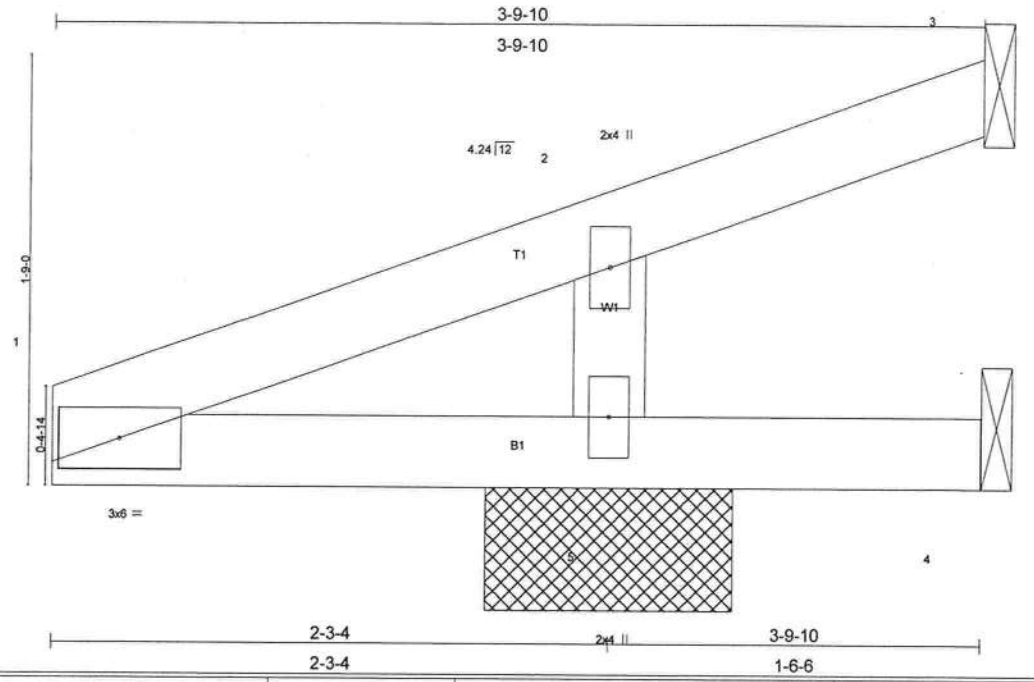
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071217

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	HJ3	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:23 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	0.00	5	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	0.00	5	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.02	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 4=1/Mechanical, 5=181/1-0-1
Max Horz 5=26(load case 2)
Max Uplift3=-28(load case 5), 4=-16(load case 7), 5=-112(load case 2)
Max Grav 3=31(load case 1), 4=21(load case 2), 5=181(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-30/20, 2-3=-23/6
BOT CHORD 1-5=-11/26, 4-5=0/0
WEBS 2-5=-125/61

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 16 lb uplift at joint 4 and 112 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Trapezoidal Loads (plf)
Vert: 1=0(F=45, B=45)-to-3=-86(F=2, B=2), 1=0(F=15, B=15)-to-4=-29(F=1, B=1)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071218

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	HJ9	MONO TRUSS	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:28 2006 Page 1

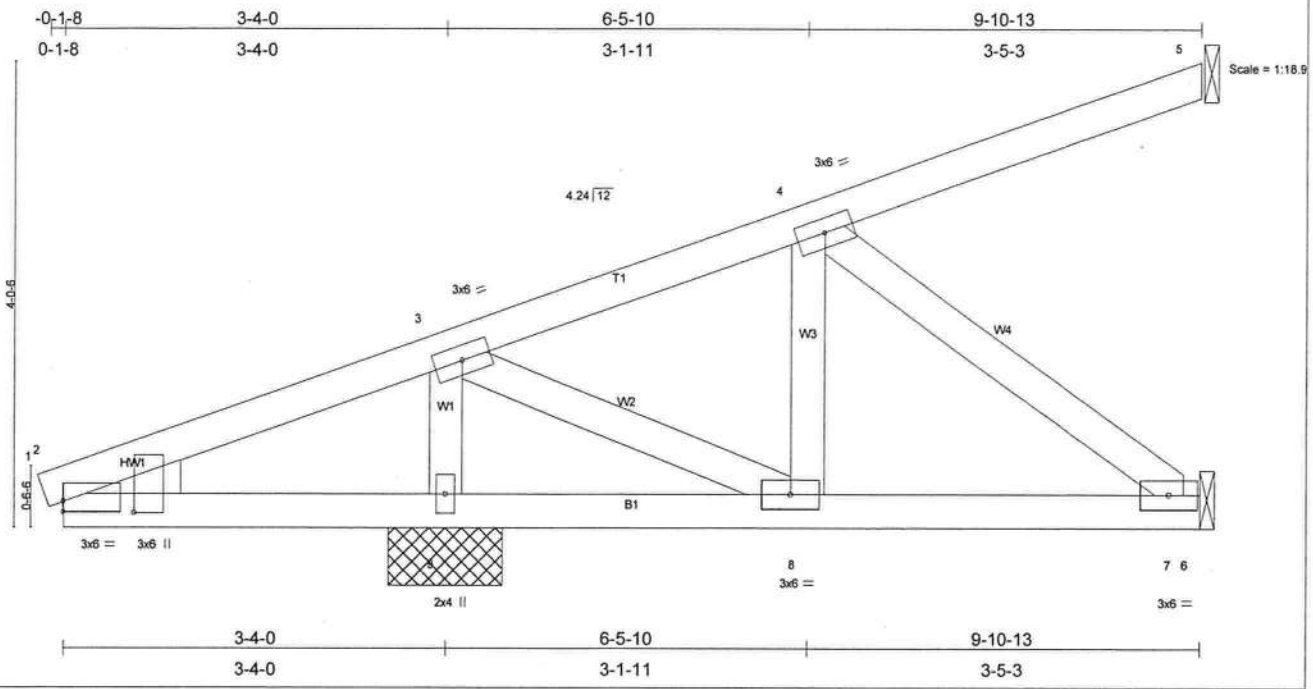

Plate Offsets (X,Y): [2:0-0-0,0-1-1], [2:0-1-3,0-7-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	0.03	7-8	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.06	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.19	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=4/Mechanical, 9=711/1-0-1, 7=754/Mechanical
Max Horz 9=-108(load case 7)
Max Uplift9=-231(load case 2), 7=-151(load case 2)
Max Grav 5=48(load case 7), 9=711(load case 1), 7=754(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/5, 2-3=-52/122, 3-4=-480/120, 4-5=0/16
BOT CHORD 2-9=-125/50, 8-9=-53/65, 7-8=-83/450, 6-7=0/0
WEBS 3-9=-423/108, 3-8=-160/552, 4-8=-184/515, 4-7=-567/104

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 9 and 151 lb uplift at joint 7.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=40(F=90)
Trapezoidal Loads (plf)
Vert: 2=0(F=15, B=15)-to-6=-297(F=-133, B=-133)

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071219

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	HJ9A	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:32 2006 Page 1

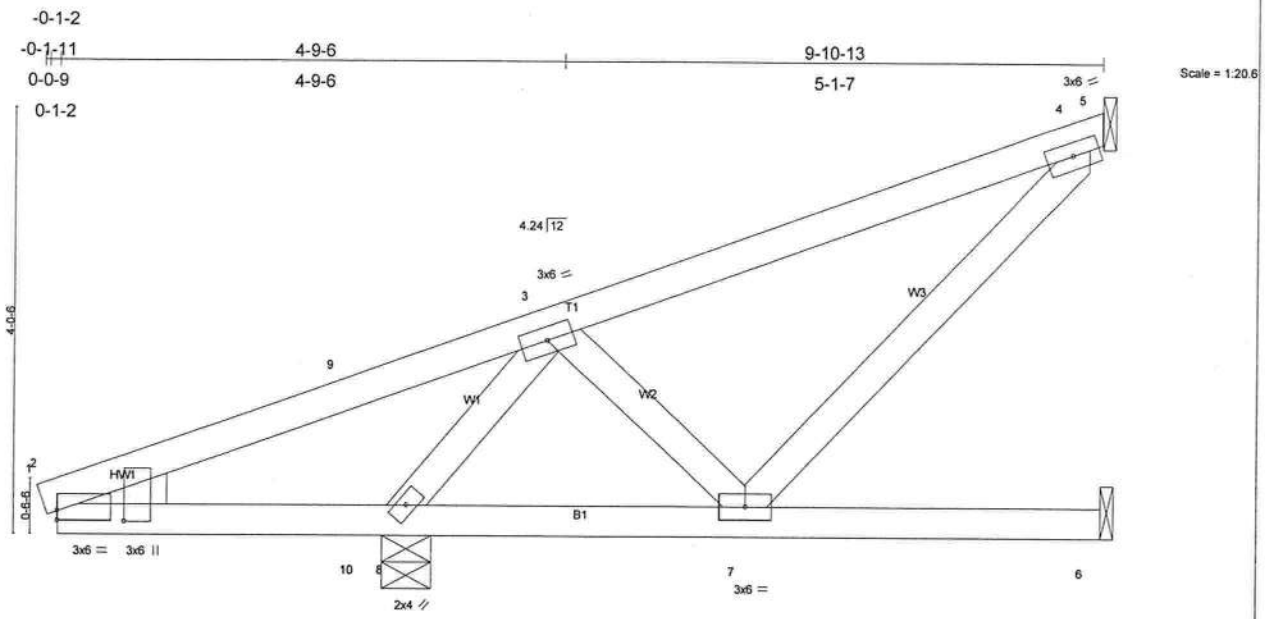

Plate Offsets (X,Y): [2:0-0-0,0-1-1], [2:0-1-3,0-7-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.01	6-7	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.19	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 6=68/Mechanical, 8=886/0-5-11, 5=237/Mechanical
Max Horz 8=127(load case 2)
Max Uplift6=-23(load case 2), 8=-386(load case 2), 5=-165(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-249/90, 2-9=-372/951, 3-9=-368/966, 3-4=-44/109, 4-5=-56/78
BOT CHORD 2-10=-856/353, 8-10=-856/353, 7-8=-192/62, 6-7=0/0
WEBS 3-7=-92/206, 4-7=-73/0, 3-8=-1071/385

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 6, 386 lb uplift at joint 8 and 165 lb uplift at joint 5.
4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 159 lb down and 57 lb up at -0-1-11, and 159 lb down and 57 lb up at -0-1-11 on top chord. The design/selection of such connection device(s) is the responsibility of others.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Concentrated Loads (lb)
Vert: 1=-318(F=-159, B=-159)
Trapezoidal Loads (plf)
Vert: 1=40(F=45, B=45)-to-2=37(F=43, B=43), 2=-3(F=43, B=43)-to-9=-66(F=12, B=12), 9=0(F=45, B=45)-to-5=-159(F=-35, B=-35), 2=0(F=15, B=15)-to-10=-22(F=4, B=4), 10=0(F=15, B=15)-to-6=-53(F=-11, B=-11)

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071220

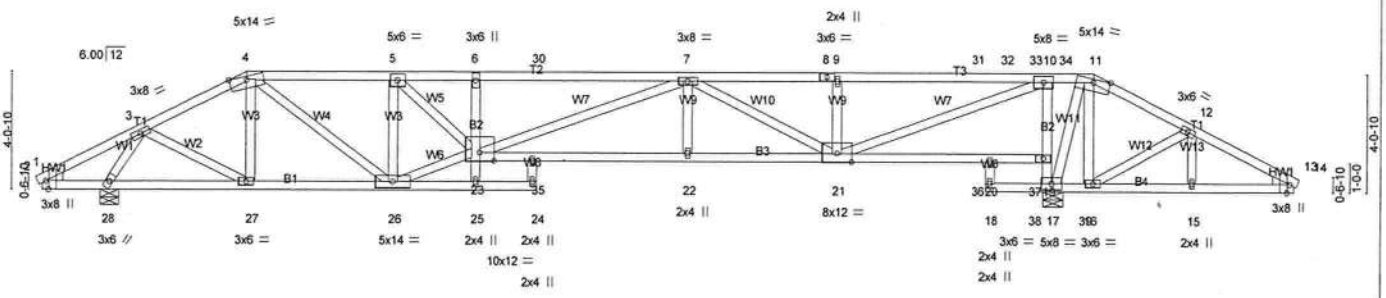
Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T01	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:38 2006 Page 1

-0-1-1 3-6-13 | 7-0-0 | 12-0-4 | 15-0-8 | 22-2-5 | 27-4-3 | 36-2-0 | 39-7-3 | 43-2-0 43-3-1

0-1-1 3-6-13 | 3-5-3 | 5-0-4 | 3-0-4 | 7-1-13 | 5-1-13 | 8-9-13 | 3-5-3 | 3-6-13 0-1-1

Scale = 1:74.7
Camber = 9/16 in



36-2-0

2-4-0 | 7-0-0 | 12-0-4 | 15-0-8 | 17-0-8 | 22-2-5 | 27-4-3 | 32-6-0 | 34-10-0 | 39-7-3 | 43-2-0

2-4-0 | 4-8-0 | 5-0-4 | 3-0-4 | 2-0-0 | 5-1-13 | 5-1-13 | 5-1-13 | 2-4-0 | 1-4-0 | 3-5-3 | 3-6-13

Plate Offsets (X,Y): [2:0-3-8,Edge], [13:0-3-8,Edge], [23:0-6-0,0-3-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	0.40	24	>973	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.92	24	>425	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.22	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 490 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2, W7 2 X 4 SYP No.2, W7 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 17=5213/0-8-0, 28=3989/0-8-0
Max Horz 28=106(load case 3)
Max Uplift17=-1787(load case 5), 28=-1462(load case 3)
Max Grav 17=5213(load case 1), 28=4031(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-131/321, 3-4=-5036/1987, 4-5=-6817/2874, 5-6=-9994/4191, 6-30=-10295/4318, 7-30=-10295/4318, 7-8=-5973/2581, 8-9=-5973/2581, 9-31=-5973/2581, 31-32=-5973/2581, 32-33=-5973/2581, 10-33=-5973/2581, 10-34=-475/1287, 11-34=-475/1287, 11-12=-374/948, 12-13=-171/435, 13-14=-6/0
BOT CHORD 2-28=-220/151, 27-28=-907/2077, 26-27=-1801/4454, 25-26=-212/398, 24-25=0/0, 23-25=-87/285, 6-23=-821/349, 23-35=-3749/9019, 22-35=-3749/9019, 21-22=-3749/9019, 21-36=-1280/681, 20-36=-1280/681, 20-37=-1280/681, 19-37=-1280/681, 17-19=-3741/1422, 10-19=-3530/1361, 18-38=0/0, 17-38=0/0, 17-39=-766/446, 16-39=-766/446, 15-16=-299/176, 13-15=-299/176
WEBS 3-27=-1016/2688, 4-27=-239/191, 4-26=-1339/2949, 5-26=-3823/1616, 23-26=-2819/6795, 5-23=-1826/4406, 7-23=-643/1478, 7-22=-150/351, 7-21=-3556/1382, 9-21=-730/286, 10-21=-2907/7338, 11-17=-1518/573, 11-16=-421/1151, 12-16=-588/328, 12-15=-89/155, 3-28=-4472/1620, 18-20=-117/47

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1787 lb uplift at joint 17 and 1462 lb uplift at joint 28.
7) Girder carries tie-in span(s): 3-3-9 from 17-8-0 to 32-6-0; 3-3-9 from 17-0-8 to 32-6-0; 3-11-0 from 17-0-8 to 32-6-0; 3-11-0 from 17-0-8 to 32-6-0
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 770 lb down and 139 lb up at 36-2-0, and 770 lb down and 139 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T01	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:38 2006 Page 2

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-50, 2-4=-90, 4-30=-188(F=-97), 30-31=-128(F=-38), 31-32=-132(F=-42), 32-33=-95(F=-5), 33-34=-188(F=-97), 11-34=-90, 11-13=-90, 13-14=-50, 2-27=-30, 25-27=-62(F=-33), 24-25=-62(F=-33), 23-35=-62(F=-33), 35-36=-49(F=-19), 20-36=-49(F=-19), 20-37=-0(F), 19-37=-33(F), 18-38=-30(F=-0), 17-38=-62(F=-33), 17-39=-62(F=-33), 13-39=-30

Concentrated Loads (lb)

Vert: 27=-770(F) 16=-770(F)

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

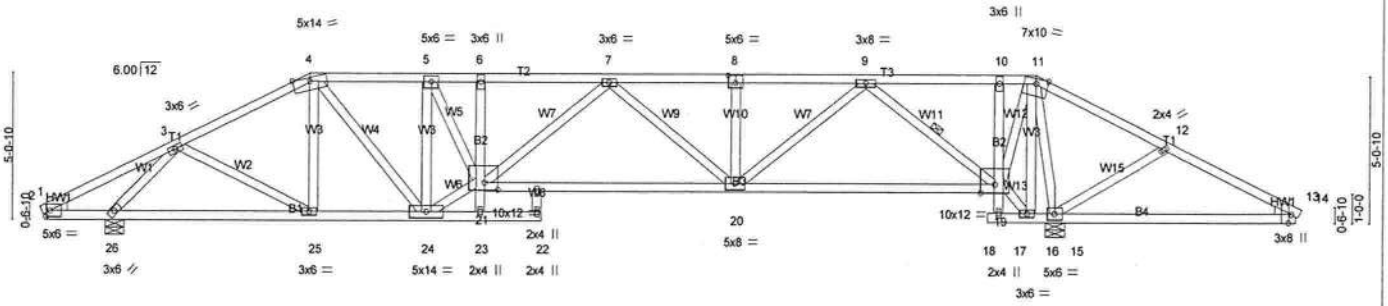
Dwg.#0124071222

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T02	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:43 2006 Page 1

-0-1-1	4-6-13	9-0-0	13-0-4	19-4-12	23-9-0	28-3-0	34-2-0	38-7-3	43-2-0	43-3-1
0-1-1	4-6-13	4-5-3	4-0-4	6-4-8	4-4-4	4-6-0	5-11-0	4-5-3	4-6-13	0-1-1

Scale = 1:74.9
Camber = 3/8 in



2-4-0	9-0-0	13-0-4	17-0-8	19-4-12	23-9-0	28-3-0	32-6-0	34-2-0	43-2-0
2-4-0	6-8-0	4-0-4	4-0-4	2-4-4	4-4-4	4-6-0	4-3-0	1-8-0	8-4-0

34-10-0
0-8-0

Plate Offsets (X,Y): [8:0-3-0,0-3-0], [13:0-3-8,Edge], [21:0-5-8,0-2-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.43	20-21	>907	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.68	20-21	>573	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.23	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 9-19

REACTIONS (lb/size) 15=3125/0-8-0, 26=2147/0-8-0
Max Horz 26=171(load case 4)
Max Uplift15=-1235(load case 6), 26=-808(load case 4)
Max Grav 15=3125(load case 1), 26=2189(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-155/318, 3-4=-2504/1300, 4-5=-2839/1525, 5-6=-3827/2038, 6-7=-3901/2084, 7-8=-3455/1844, 8-9=-3455/1844, 9-10=-338/306, 10-11=-279/275, 11-12=-427/1083, 12-13=-315/632, 13-14=-6/0
BOT CHORD 2-26=-193/180, 25-26=-939/1537, 24-25=-1240/2151, 23-24=-95/78, 22-23=0/0, 21-23=-25/138, 6-21=-237/104, 20-21=-1982/3917, 19-20=-1106/2179, 17-19=-47/150, 10-19=-228/104, 17-18=0/0, 16-17=-90/208, 15-16=-591/415, 13-15=-444/293
WEBS 3-25=-337/708, 4-25=-151/119, 4-24=-507/1064, 5-24=-2260/1132, 21-24=-1736/3129, 5-21=-1148/2215, 16-19=-676/397, 11-19=-921/1851, 11-16=-266/211, 11-15=-2202/972, 12-15=-530/298, 3-26=-2578/1161, 7-21=-48/125, 7-20=-676/268, 9-20=-984/1727, 9-19=-2435/1238, 8-20=-385/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1235 lb uplift at joint 15 and 808 lb uplift at joint 26.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

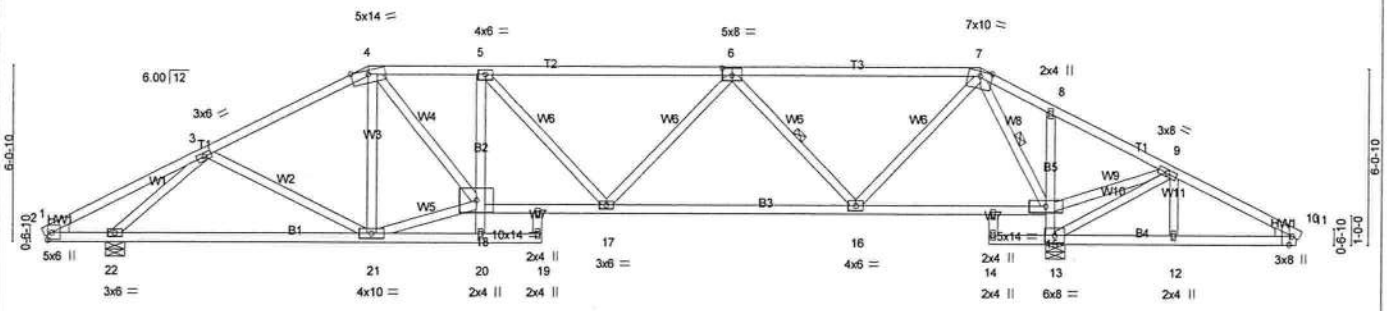
Dwg.#0124071223

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T03	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:48 2006 Page 1

-0-1-1	5-6-13	11-0-0	15-0-8	23-7-4	32-2-0	38-11-3	43-2-0	43-3-1
0-1-1	5-6-13	5-5-3	4-0-8	8-6-12	8-6-12	6-9-3	4-2-13	0-1-1

Scale = 1:75.0
Camber = 1/4 in



2-4-0	11-0-0	15-0-8	17-0-8	19-3-14	27-10-10	32-6-0	34-10-0	38-11-3	43-2-0
2-4-0	8-8-0	4-0-8	2-0-0	2-3-6	8-6-12	4-7-6	2-4-0	4-1-3	4-2-13

Plate Offsets (X,Y): [6:0-4-0,0-3-0], [10:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	0.33	16-17	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.47	16-17	>838	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.16	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 255 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-11-5 oc bracing.
WEBS 1 Row at midpt 6-16, 7-15

REACTIONS (lb/size) 13=3181/0-8-0, 22=2143/0-8-0
Max Horz 22=140(load case 4)
Max Uplift13=-1247(load case 6), 22=-774(load case 4)
Max Grav 13=3181(load case 1), 22=2181(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-175/276, 3-4=-2497/1295, 4-5=-3171/1669, 5-6=-3044/1653, 6-7=-1746/948, 7-8=-539/1318, 8-9=-583/1246, 9-10=-217/552, 10-11=-6/0
BOT CHORD 2-22=-137/207, 21-22=-976/1844, 20-21=-133/131, 19-20=0/0, 18-20=-6/103, 5-18=-391/232, 17-18=-1483/3245, 16-17=-1241/2764, 15-16=-253/513, 13-15=-2983/1573, 8-15=-101/113, 13-14=0/0, 12-13=-406/242, 10-12=-386/222
WEBS 3-21=-150/390, 4-21=-574/188, 18-21=-1020/2077, 4-18=-730/1633, 6-17=-334/471, 6-16=-1576/720, 7-15=-2894/1411, 9-15=-459/421, 9-13=-285/91, 9-12=-187/178, 3-22=-2676/1129, 5-17=-336/108, 7-16=-1094/1891

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1247 lb uplift at joint 13 and 774 lb uplift at joint 22.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

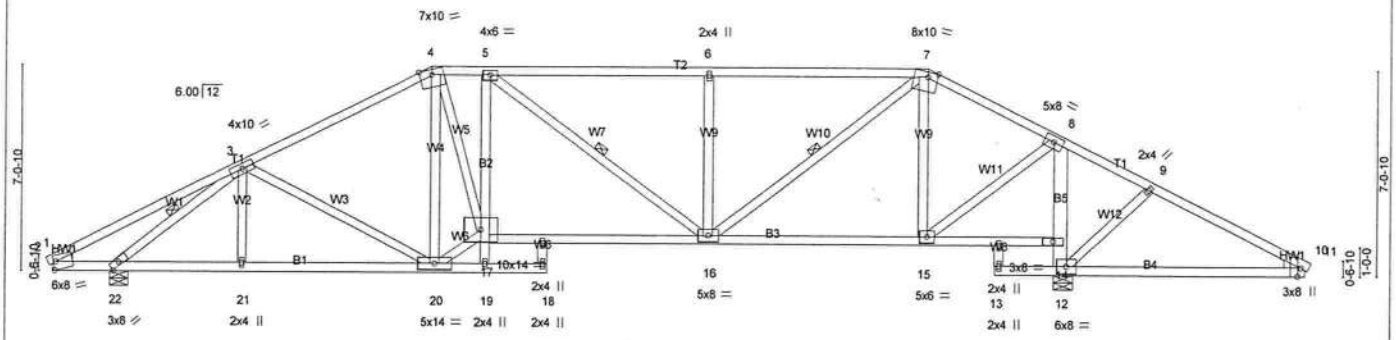
Dwg.#0124071224

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T04	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:53 2006 Page 1

-0-1-1	6-6-13	13-0-0	15-0-8	22-7-4	30-2-0	37-9-3	43-2-0	43-3-1
0-1-1	6-6-13	6-5-3	2-0-8	7-6-12	7-6-12	7-7-3	5-4-13	0-1-1

Scale = 1:74.7
Camber = 1/4 in



2-4-0	7-8-0	13-0-0	15-0-8	17-0-8	22-7-4	30-2-0	32-6-0	34-10-0	43-2-0
2-4-0	5-4-0	5-4-0	2-0-8	2-0-0	5-6-12	7-6-12	2-4-0	2-4-0	8-4-0

Plate Offsets (X,Y): [2:0-1-0,Edge], [7:0-4-3,Edge], [10:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.26	16-17	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.43	18	>910	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.09	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 264 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-2 oc bracing.
WEBS 1 Row at midpt 5-16, 7-16, 3-22

REACTIONS (lb/size) 12=3191/0-8-0, 22=2135/0-8-0
Max Horz 22=156(load case 4)
Max Uplift12=-1276(load case 6), 22=-737(load case 4)
Max Grav 12=3191(load case 1), 22=2166(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-109/382, 3-4=-2394/1229, 4-5=-2608/1416, 5-6=-2379/1307, 6-7=-2379/1307, 7-8=-1082/556, 8-9=-427/1061, 9-10=-373/776, 10-11=-6/0
BOT CHORD 2-22=-211/163, 21-22=-1121/2071, 20-21=-1121/2071, 19-20=-60/23, 18-19=0/0, 17-19=0/68, 5-17=-272/257, 16-17=-1194/2668, 15-16=-406/929, 14-15=-1153/695, 12-14=-2663/1334, 8-14=-2561/1240, 12-13=0/0, 10-12=-560/355
WEBS 3-21=-177/133, 3-20=-74/159, 4-20=-1184/437, 17-20=-1223/2417, 4-17=-863/1892, 5-16=-404/169, 6-16=-729/306, 7-16=-981/1868, 7-15=-1308/544, 8-15=-1230/2441, 9-12=-470/257, 3-22=-2916/1408

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1276 lb uplift at joint 12 and 737 lb uplift at joint 22.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0124071225

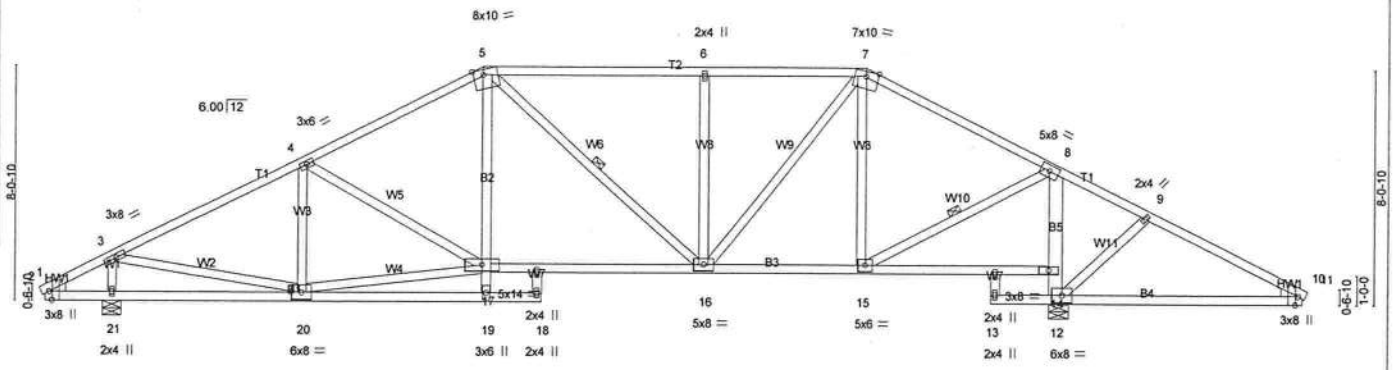
Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:54:58 2006 Page 1

2-4-0 | -0-1-1 | 8-9-5 | 15-0-0 | 22-7-4 | 28-2-0 | 37-9-3 | 43-2-0 | 43-3-1

0-1-1 | 6-5-5 | 6-2-11 | 7-7-4 | 5-6-12 | 9-7-3 | 5-4-13 | 0-0-1

Scale = 1:74.7
Camber = 3/16 in



2-4-0 | 8-9-5 | 15-0-0 | 17-0-8 | 22-7-4 | 28-2-0 | 32-6-0 | 34-10-0 | 43-2-0

2-4-0 | 6-5-5 | 6-2-11 | 2-0-8 | 5-6-12 | 5-6-12 | 4-4-0 | 2-4-0 | 8-4-0

Plate Offsets (X,Y): [2:0-3-8,Edge], [5:0-4-3,Edge], [10:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	0.23	16-17	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.34	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 264 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-0 oc bracing.
WEBS 1 Row at midpt 5-16, 8-15

REACTIONS (lb/size) 12=3176/0-8-0, 21=2143/0-8-0
Max Horz 21=163(load case 4)
Max Uplift12=-1292(load case 6), 21=-700(load case 4)
Max Grav 12=3176(load case 1), 21=2161(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-54/77, 3-4=-2441/1233, 4-5=-2442/1263, 5-6=-1933/1112, 6-7=-1933/1112, 7-8=-1348/725, 8-9=-425/1058, 9-10=-373/779, 10-11=-6/0
BOT CHORD 2-21=-56/44, 20-21=-220/142, 19-20=-120/132, 18-19=0/0, 17-19=-101/167, 5-17=-424/560, 16-17=-920/2117, 15-16=-446/1118, 14-15=-1058/641, 12-14=-2661/1323, 8-14=-2525/1181, 12-13=0/0, 10-12=-564/355
WEBS 3-21=-1971/901, 3-20=-988/2071, 4-20=-572/177, 4-17=-59/155, 5-16=-276/118, 6-16=-607/262, 7-16=-668/1285, 7-15=-974/373, 8-15=-1188/2408, 9-12=-451/249, 17-20=-980/1958

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1292 lb uplift at joint 12 and 700 lb uplift at joint 21.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

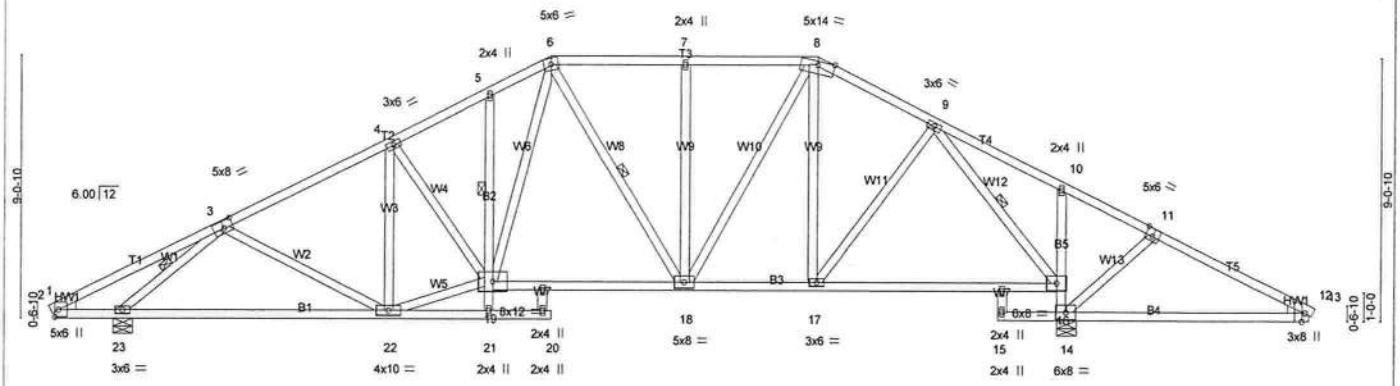
Dwg.#0124071226

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:55:03 2006 Page 1

-0-1-1	5-9-1	11-4-8	15-0-8	17-0-0	21-7-0	26-2-0	30-3-2	37-9-3	43-2-0	43-3-1
0-1-1	5-9-1	5-7-8	3-8-0	1-11-8	4-7-0	4-7-0	4-1-2	7-6-1	5-4-13	0-1-1

Scale = 1:75.0
Camber = 3/16 in



2-4-0	11-4-8	15-0-8	17-0-8	26-2-0	32-6-0	34-10-0	43-2-0
2-4-0	9-0-8	3-8-0	2-0-0	9-1-8	6-4-0	2-4-0	8-4-0

Plate Offsets (X,Y): [3:0-3-12,0-3-0], [11:0-3-0,0-3-4], [12:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.28	22-23	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.29	22-23	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.05	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 286 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 5-19
WEBS 1 Row at midpt 3-23, 6-18, 9-16

REACTIONS (lb/size) 14=3182/0-8-0, 23=2142/0-8-0
Max Horz 23=179(load case 4)
Max Uplift14=-1307(load case 6), 23=-698(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-174/272, 3-4=-2423/1266, 4-5=-2428/1305, 5-6=-2341/1343, 6-7=-1724/1034, 7-8=-1724/1034, 8-9=-1496/864, 9-10=-424/1176, 10-11=-437/1069, 11-12=-389/797, 12-13=-6/0
BOT CHORD 2-23=-130/211, 22-23=-967/1855, 21-22=-90/34, 20-21=0/0, 19-21=0/90, 5-19=-134/97, 18-19=-762/1848, 17-18=-484/1255, 16-17=-266/760, 14-16=-2767/1379, 10-16=-297/144, 14-15=0/0, 12-14=-578/368
WEBS 3-22=-112/268, 4-22=-509/114, 19-22=-1143/2207, 4-19=-11/127, 6-19=-568/924, 8-18=-459/925, 8-17=-556/145, 9-17=-441/898, 11-14=-454/250, 3-23=-2638/1102, 6-18=-247/108, 9-16=-2786/1310, 7-18=-419/181

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1307 lb uplift at joint 14 and 698 lb uplift at joint 23.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

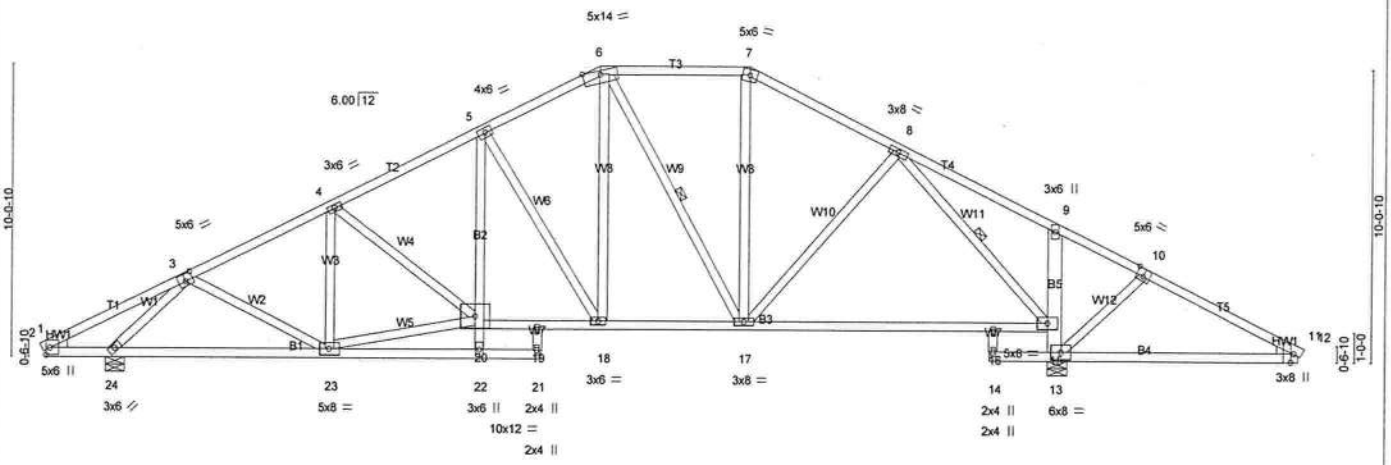
Dwg.#0124071227

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:55:08 2006 Page 1

-0-1-1	4-10-15	9-8-4	15-0-8	19-0-0	24-2-0	29-4-0	34-6-0	37-9-3	43-2-0	43-3-1
0-1-1	4-10-15	4-9-5	5-4-4	3-11-8	5-2-0	5-2-0	5-2-0	3-3-3	5-4-13	0-1-1

Scale = 1:75.0
Camber = 1/8 in



2-4-0	9-8-4	15-0-8	17-0-8	19-0-0	24-2-0	32-6-0	34-6-0	43-2-0
2-4-0	7-4-4	5-4-4	2-0-0	1-11-8	5-2-0	8-4-0	2-0-0	8-8-0

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-3-0,0-3-4], [11:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.28	16-17	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.26	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.06	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 9-13
WEBS 1 Row at midpt 6-17, 8-15

REACTIONS (lb/size) 13=3097/0-8-0, 24=2098/0-8-0
Max Horz 24=172(load case 4)
Max Uplift13=-1325(load case 6), 24=-686(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-159/311, 3-4=-2369/1191, 4-5=-2372/1187, 5-6=-1812/992, 6-7=-1296/819, 7-8=-1549/869, 8-9=-409/1265, 9-10=-418/1051, 10-11=-364/776, 11-12=-6/0
BOT CHORD 2-24=-182/187, 23-24=-781/1556, 22-23=-159/241, 21-22=0/0, 20-22=-56/76, 5-20=-362/580, 19-20=-821/2023, 18-19=-820/2022, 17-18=-572/1562, 16-17=-259/846, 15-16=-259/846, 13-15=-2427/1070, 9-15=-289/145, 13-14=0/0, 11-13=-562/349
WEBS 3-23=-243/535, 4-23=-464/147, 20-23=-858/1835, 4-20=-21/122, 5-18=-884/478, 6-18=-492/826, 6-17=-570/213, 7-17=-228/240, 8-17=-312/685, 8-15=-2855/1255, 10-13=-460/259, 3-24=-2498/1066, 14-16=-143/121, 19-21=-11/70

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1325 lb uplift at joint 13 and 686 lb uplift at joint 24.

LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

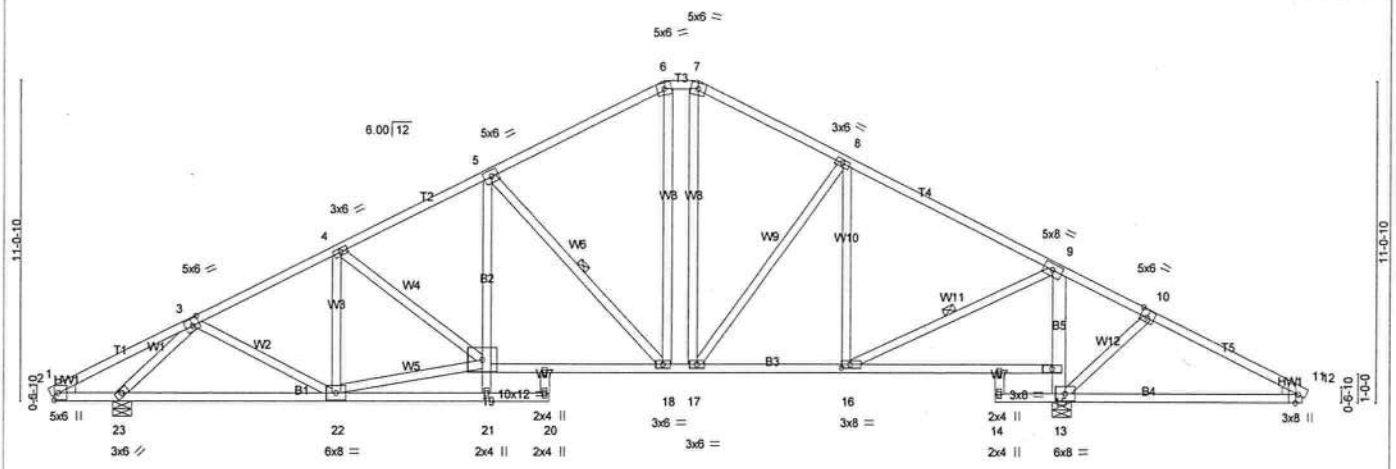
Dwg.#0124071228

Job	Truss	Truss Type	Qty	Ply	MOISE DESRAVINES
L213487	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Oct 19 12:55:13 2006 Page 1

-0-1-1	4-11-1	9-8-8	15-0-8	21-0-0	22-2-0	27-4-0	37-9-3	43-2-0	43-3-1
0-1-1	4-11-1	4-9-8	5-4-0	5-11-8	1-2-0	5-2-0	10-5-3	5-4-13	0-1-1

Scale = 1:75.0
Camber = 1/4 in



2-4-0	9-8-8	15-0-8	17-0-8	21-0-0	22-2-0	27-4-0	32-6-0	34-10-0	43-2-0
2-4-0	7-4-8	5-4-0	2-0-0	3-11-8	1-2-0	5-2-0	5-2-0	2-4-0	8-4-0

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [10:0-3-0,0-3-4], [11:0-3-8,Edge], [16:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.23	18-19	>999	240	MT20	244/190
TCDL	25.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.39	18-19	>991	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.03	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 286 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-8 oc bracing.
WEBS 1 Row at midpt 5-18, 9-16

REACTIONS (lb/size) 13=3189/0-8-0, 23=2138/0-8-0
Max Horz 23=212(load case 4)
Max Uplift13=-1342(load case 6), 23=-721(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/0, 2-3=-159/308, 3-4=-2430/1243, 4-5=-2516/1287, 5-6=-1649/915, 6-7=-1358/862, 7-8=-1624/918, 8-9=-1454/746, 9-10=-424/1055, 10-11=-375/786, 11-12=-6/0
BOT CHORD 2-23=-178/187, 22-23=-856/1600, 21-22=-65/0, 20-21=0/0, 19-21=-63/144, 5-19=-483/748, 18-19=-925/2190, 17-18=-474/1358, 16-17=-423/1161, 15-16=-1021/618, 13-15=-2674/1301, 9-15=-2522/1146, 13-14=0/0, 11-13=-569/358
WEBS 3-22=-256/543, 4-22=-544/155, 19-22=-1044/2138, 4-19=0/113, 5-18=-1234/673, 6-18=-385/491, 7-17=-249/329, 8-17=-105/364, 8-16=-918/312, 9-16=-1150/2402, 10-13=-443/246, 3-23=-2552/1111

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=17ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever left and right exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1342 lb uplift at joint 13 and 721 lb uplift at joint 23.

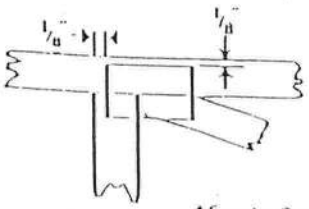
LOAD CASE(S) Standard

JANUARY 24,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

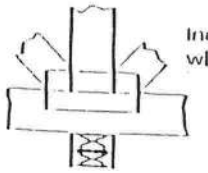
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

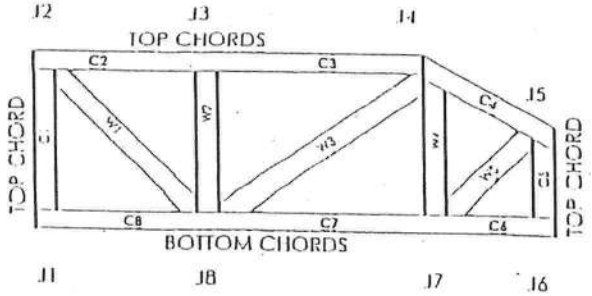
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek

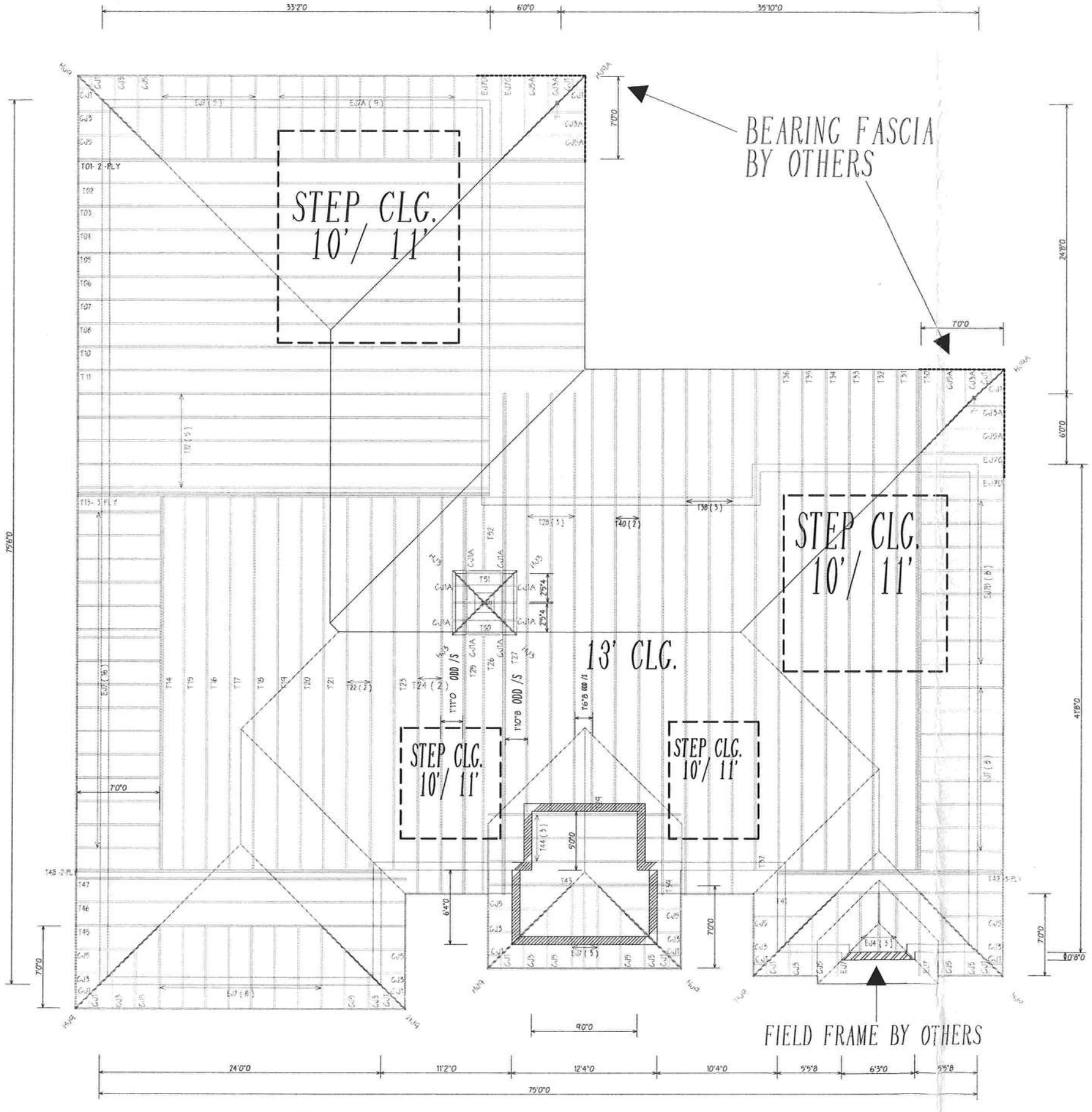
TEE LOK

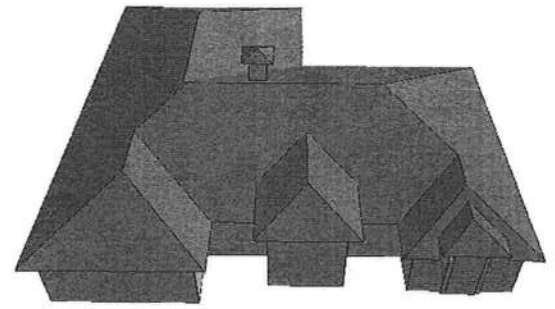
General Safety Notes

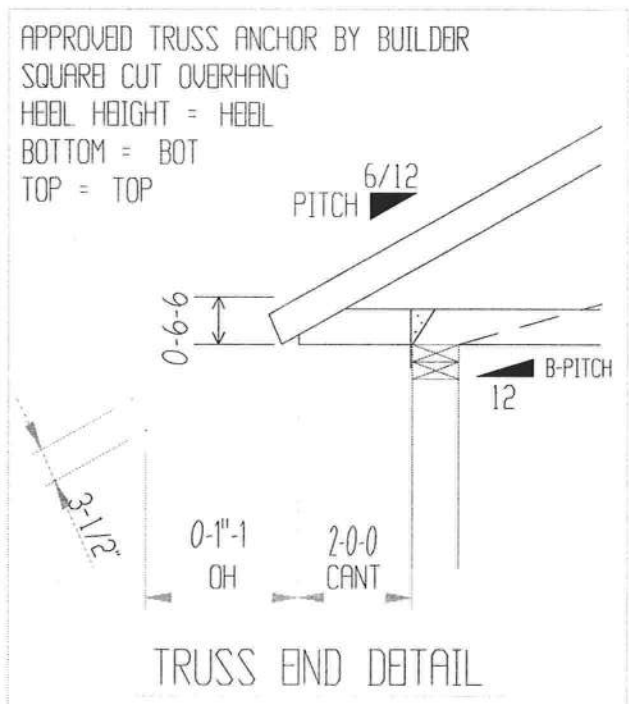
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ALL 10' CLG'S EXCEPT
WHERE NOTED.







HANGERS

26-HTU26
7-HGUS26-2
1-HGUS28-3

BEARING HEIGHT SCHEDULE

	8'-0"
	13'-0"
	11'-6"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: Date:

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: MOSIE DESRAVINES
LEGAL ADDRESS: LAKE CITY
MODEL: CUSTOM
REVISION:
SCALE: NTS
DATE: 10/20/2006
DRAWN BY: TJR
JOB #: L213487

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056 Tel. (386) 755-3633 • Fax (386) 752-5456
4784 Rosselle Street • Jacksonville, FL 32254 Tel. (904) 381-8901 • Fax (904) 381-8902
2230 Greensboro Highway • Quincy, FL 32351 Tel. (850) 442-3495 • Fax (850) 442-4008

November 1, 2007

Mr. Moise M. Desravines
1980 SW Nautilus Road
Lake City, Florida 32024

Reference: Report of Subsurface Exploration
Moise & Schenetter Desravines Residence
1924 SW Nautilus Road
Lake City, Florida 32024
Cal-Tech Project No. 07-00526-01

Dear Mr. Desravines:

Cal-Tech Testing, Inc. (CTI) has completed the subsurface exploration and engineering evaluation for the proposed residence located at 1924 SW Nautilus Road in Lake City, Columbia County, Florida. Our work was verbally authorized by you on October 30, 2007.

Introduction

The purpose of this exploration was to develop information concerning the site and subsurface conditions in order to evaluate site preparation requirements and foundation support alternatives for the proposed residence. This report briefly describes the field activities and presents our findings.

We understand, the proposed construction will include a one-story building with attached garage, swimming pool, and associated driveway and landscaped areas. We anticipate the construction will consist of a combination siding or stucco veneer exterior with wood framed walls, and wood-truss roofing system. Detailed structural loading information has not been provided; however, we assume that bearing walls and individual column loads will not exceed 3 kips per lineal foot and 25 kips, respectively. We assume that soil-supported floor loads (dead load plus live load) will not exceed 125 psf.

We understand approximately 2 to 4 feet of new fill will be needed at the site to achieve finished subgrade elevations. In addition, we anticipate the concrete floor slab will be supported by about 2 feet of newly placed fill (stem wall footing).

Site Conditions

The existing site conditions within the limits of the proposed residence were observed by our drill crew on October 31, 2007. At the time of our site visit, the ground surface was grass covered, and the site topography was relatively level with elevation difference of approximately two feet across the proposed building area.

Field Program

Our field program consisted of performing three (3) Standard Penetration Test (SPT) borings extending to a depth of 15 feet below the existing ground surface. The borings were performed at the approximate locations indicated on the attached Field Exploration Plan (Figure No. 1).

The sampling and penetration procedures of the Standard Penetration Test (SPT) boring was accomplished in general accordance with ASTM D-1586, using a power rotary drill rig. The SPT boring was performed by driving a standard 1-3/8'' I.D. and 2.0'' O.D. split spoon sampler with a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6 inch increments, were recorded. The penetration resistance or "*N*" value is the summation of the last two 6 inch increments and is illustrated on the attached boring log adjacent to their corresponding depths. The penetration resistance is used as an index to derive soil parameters from various empirical correlations.

Subsurface Conditions

In general, the soil profile as disclosed by SPT borings B-1, B-2, and B-3 consisted of about 9 to 12 inches of gray, silty fine sand (SP-SM) with trace of organic (TOPSOIL), this surficial cover was underlain by about 3 feet of light grayish tan, silty fine sand (SM-SP). Beneath these soils, the soil profile consisted about 11 feet of light gray and reddish tan, mottled, clayey fine sand (SC). These soils have a very loose to very firm relative density with Standard penetration resistance or "*N*" values ranging from 3 to 28 Blows Per Foot (BPF).

For a more detailed description of the subsurface conditions encountered, please refer to the attached Generalized Subsurface Profile (Figure No. 1). Note that the transition between soil types may be gradual and not abrupt as indicated by the boring logs; therefore, the thickness of soil layers should be considered approximate.

Groundwater

The depth to the groundwater was measured at the boring locations at the time of completion of drilling. The groundwater table was not encountered in any of the SPT borings. We note that due to the relatively short time frame of the field exploration, the groundwater may not have had sufficient time to stabilize. For a true groundwater level reading, piezometers may be required. In any event, fluctuation in groundwater levels should be expected due to seasonal climatic changes, construction activity, rainfall variations, surface water runoff, and other site-specific factors. Since groundwater

level variations are anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on the assumption that variations will occur.

Discussion and Recommendations

The subject site is considered acceptable for the support of the proposed residence on a conventional shallow foundation system. Provided individual column footings and continuous wall footings bear on compacted acceptable existing soils or newly placed structural fill soils, the shallow foundation may be designed using an allowable net soil bearing pressure of 2,000 psf.

Due to the varying density of the upper soils, it is recommended the exposed subgrade be proofrolled and proof compacted to a depth of 4 feet below the existing ground surface prior to concrete placement (including bottom of footings and slab areas). This may require the overexcavation and recompaction of the upper 4 feet of the existing soils. Soils should be proof compacted to a minimum of 95% of the modified Proctor maximum dry density (ASTM D-1557). All properly compacted structural fill should consist of non-plastic, inorganic, granular soil containing less than 10 percent material passing the 200 mesh sieve (i.e., relatively clean sand).

Driveway subgrade should be compacted to a density of at least 98 percent of the modified Proctor maximum dry density (ASTM D-1557) to a depth at least 12 inches below the bottom of the concrete slab. Control joints should be provided for crack control every 10 feet. Concrete used for driveway construction should have a minimum 28-day compressive strength of 3,000 psi.

In the swimming pool area, consideration should be given to the groundwater. Although the groundwater table was not encountered within any of the SPT borings, should a swimming pool bottom be located near or below the groundwater table, a drainage blanket should be provided below the pool to allow collection of the water and discharging by either gravity or by a sump and pump system. This will be essential during periods when the pool is drained.

The exploration and recommendations are based upon subsurface conditions encountered at a specific locations and time as presented within this report. However, subsurface conditions may exist that differ from our findings. We request that we be notified if dissimilar subsurface conditions are encountered.

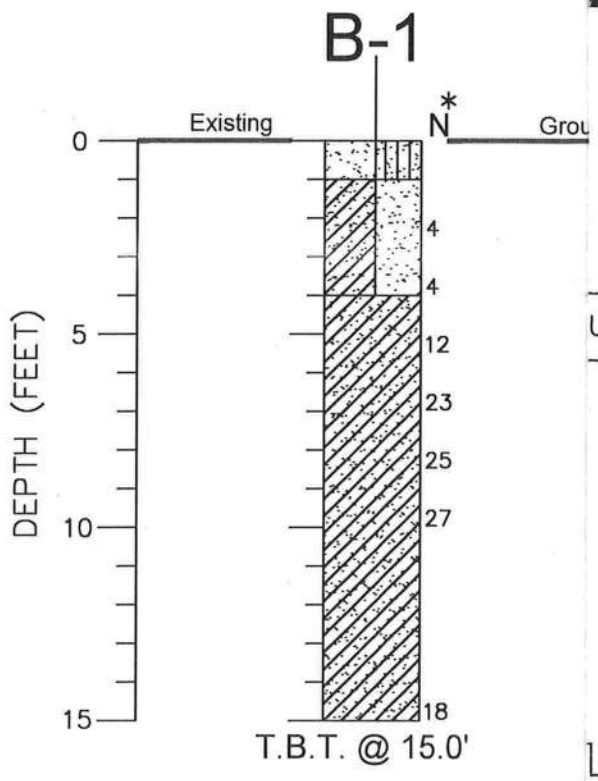
We appreciate the opportunity to be of service on this project and look forward to a continued association. Should you have questions concerning this report or if we may be further service, please contact this office.

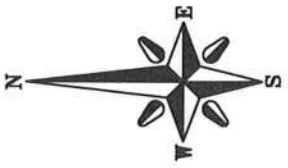
Respectfully submitted,
Cal-Tech Testing, Inc.

David B. Brown
Executive Vice President

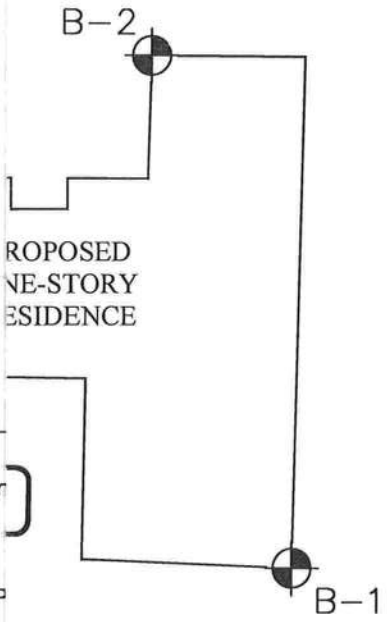
Nabil O. Hmeidi, P.E.
Senior Geotechnical Engineer
Licensed, Florida No. 57842

APPENDIX





US ROAD



ROPOSED
NE-STORY
ESIDENCE

LEGEND

Gray, silty fine sand (SP-SM), trace organ

Light grayish tan, silty fine sand, (SM-SP

Light gray and reddish tan, mottled, clay

* Standard Penetration Resistance (Blows/ft.)
Measured Using a Manual Hammer System

Groundwater Level Measured at Completio

T.B.T. Test Boring Terminated

A.B.T. Auger Boring Terminated

15.0' Termination Depth

ormed by CTI on 10/31/2007

FIELD EXPLORATION PLAN
&
GENERALIZED SUBSURFACE PROFILE

Project No. 07-00526-01	DATE: 11/01/2007	FIGURE: 1
DRAWN:	APPROVED:	SCALE: N.T.S.

Florida Building Code Online

The Florida Department of Community Affairs
Building Code Information System

FLORIDA BUILDING CODE

Overview | User Registration | Organization Registration | User Authorization | Organization Search | Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Cancel Search

Result List for Organizations

Displaying 1-1 of 1

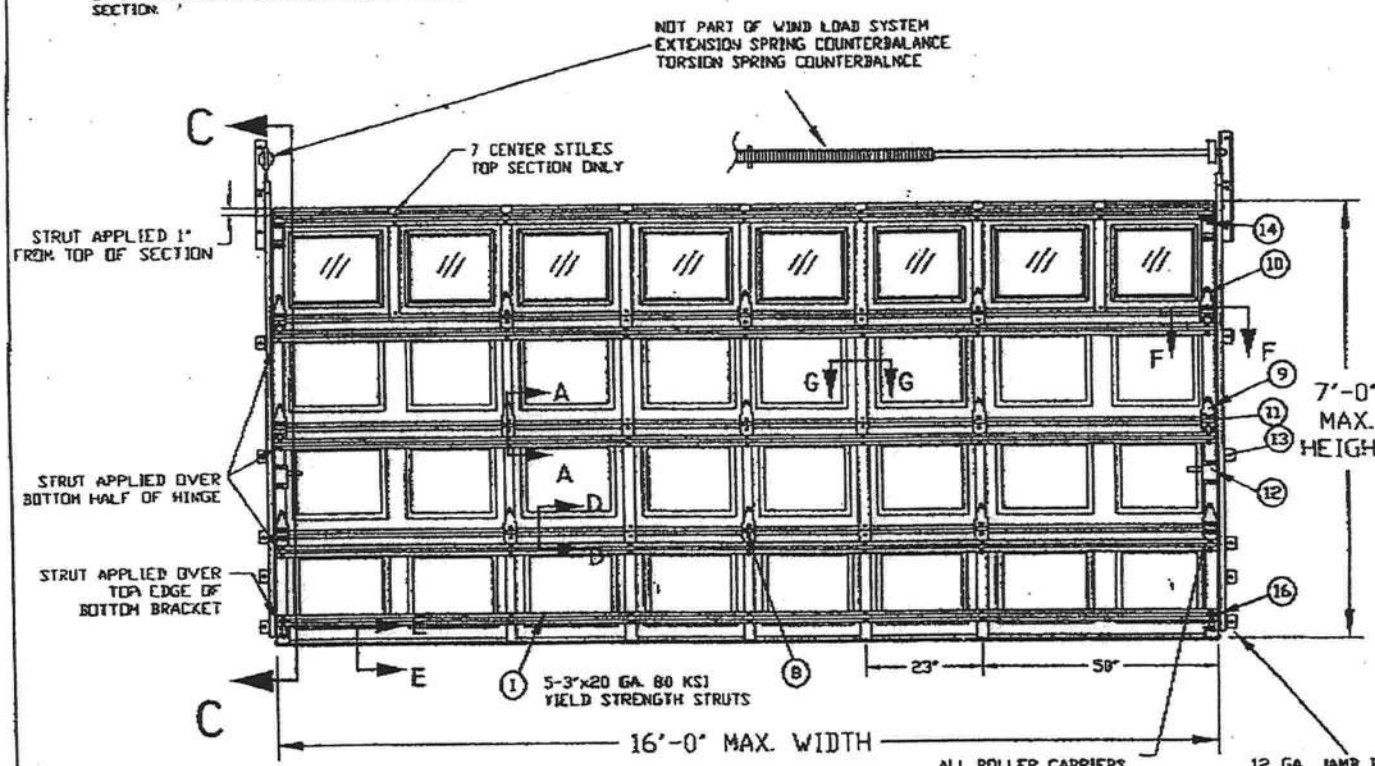
Name	City	Contact	Phone	Type	Expire	Status
General American Door	Montgomery	James Campbell	6308593000	Product Manufacturer	01/01/2099	Approved
Org Code: PDM System ID: 3585			Site Link: www.gadco.com			

Displaying 1-1 of 1

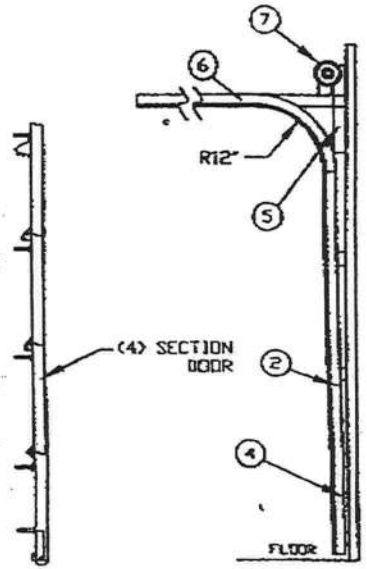
REV.	DATE	BY	DESCRIPTION
A - A5	11-10-00	DW	SEE E.C.N. 831

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5¼" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. DROP IN TYPE OF INSULATION IS OPTIONAL

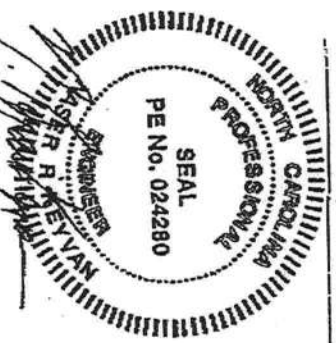


INSIDE ELEVATION



SEC C-C
VERTICAL TRACK, (16 GA.)

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s) of dimensions and installation(s) of the door as tested.



LISTED
SBCCI PST & ESI
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 (00293)

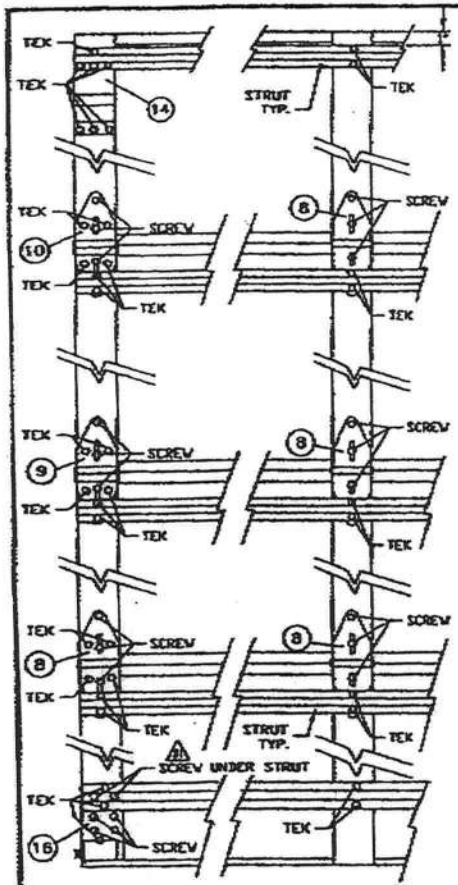
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN ⚠
SERIES 7524, EXTERIOR STEEL =.024" MIN ⚠
(TESTED) WITH WINDOWS

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

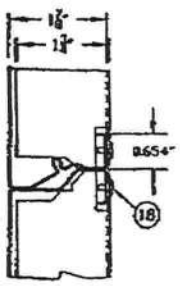
GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: 1/3 | APPROVED BY: | DRAWN BY: R WIEDMAN
DATE: 10-20-00 | REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER: PAGE 1 OF 2 | DRAWING NUMBER: W13220-1

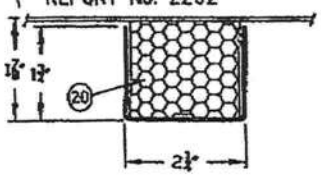


FASTENER ARRANGEMENT

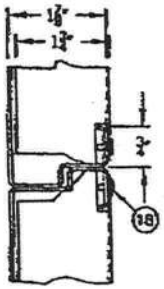
LISTED
SBCI PST & ESI
REPORT No. 2202



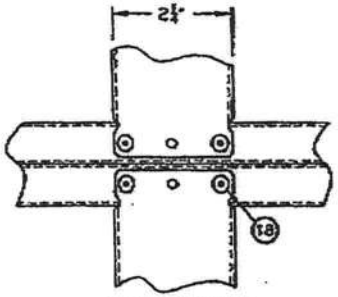
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)



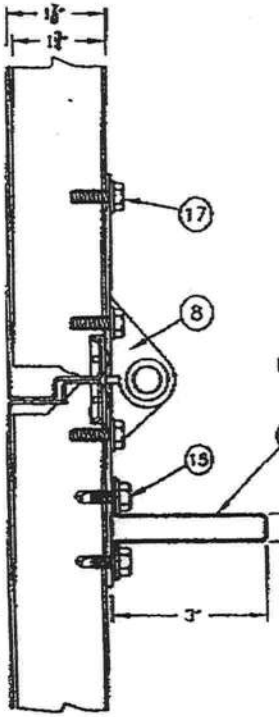
SEC G-G
CENTER STILE
20 GA GALVANIZED



SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)

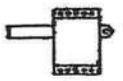


PAN ATTACHMENT TO STILE

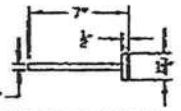


SEC A-A

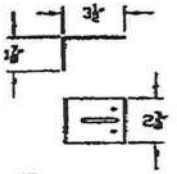
TRACK
16 GA (055 MIN.)



(12) SIDE LOCK

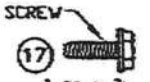


(11) ROLLER 10 BALLS

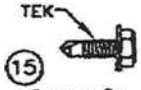


(4) #6 JAMB BRACKET

(1) 5-3"x20 GA 80 KSI YIELD STRENGTH HEMMED STRUT APPLIED WITH 2 TEK SCREWS PER HINGE OR STILE LOCATION (14 PER STRUT, MINIMUM)

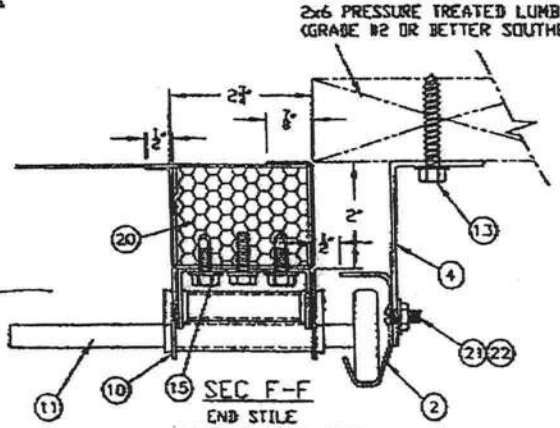


(17) 1/4-20 X 3/4 HEX WASHERHEAD SCREW

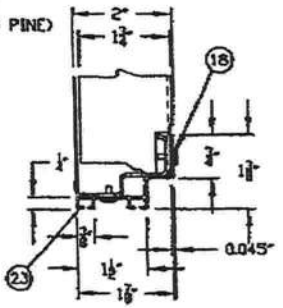


(15) 1/4-20 X 3/4 HWH TEK SCREW WITH #2 REDUCED POINT

ITEM	PART NO.	DESCRIPTION
23	3485	BOTTOM RAIL 6063 T6 ALUM EXT.
22	1391	1/4"-20 X 5/8" RIBBED NECK SCREW
21	4044	1/4"-20 SERRATED FLANGE NUT
20		1 LB DENSITY ROLLER BLOCK EPS
19		
18	4385	POP RIVET, 9/32" STEEL
17	4229	HEX WASHERHEAD SCREW 1/4"-20 X 3/4"
16	L399RLBR	BOTTOM ROLLER BRACKET
15	0138	RWH TEK SCREW 1/4-20 X 3/4 W/ REDUCED PT.
14	3875	TOP SUPPORT BRACKET
13	4043	5/16" X 3-5/8" LAG SCREW
12	4710	SIDE LOCK
11	4305	LONG STEM ROLLER 2"
10	4203	ROLLER CARRIER HINGE #3 (14 GA)
9	4202	ROLLER CARRIER HINGE #2 (14 GA)
8	4201	ROLLER CARRIER HINGE #1 (14 GA)
7		COUNTER BALANCE (TORSION BAR & SPRINGS)
6		HORIZONTAL TRACK & ANGLE (16 GA)
5		VERTICAL FLAG ANGLE
4	4585-6	TRACK BRACKET #6 (12 GA)
3		
2		VERTICAL TRACK (16 GA)
1		3" HEMMED STRUT (20 GA) MIN. YIELD STR. 80KSI



SEC F-F
END STILE
20 GA PAINTED STEEL



SEC E-E

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASSER R. KAYYAN

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

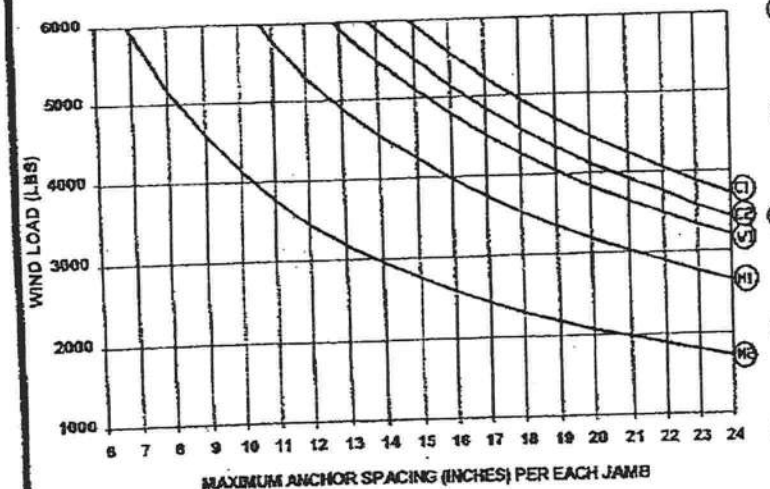
GADCO®
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE
DATE: 11-7-00
DRAWN BY: D. WIEDMAN
REVISED: (B) 12-8-00

16' X 7' MAX. RAISED PANEL STEEL DOOR-WINDLOAD ±20 PSF

PAGE 2 OF 2

DRAWING NUMBER: W13220-2



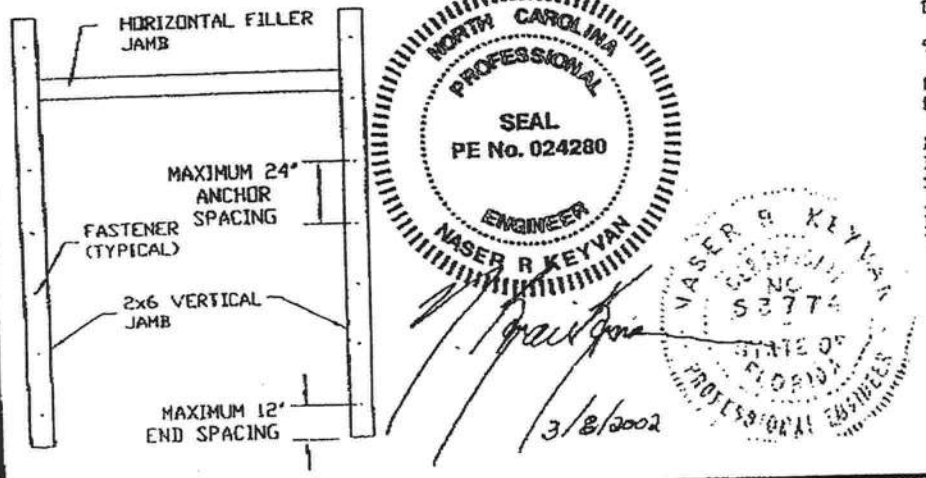
DESIGN LOAD (LBS) / FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS / FT² X (16 FT WIDE X 8 FT HIGH) = 3840 LBS

- (C1) USE 22" SPACING
- (C2) USE 21" SPACING
- (W1) USE 19" SPACING
- (M1) USE 16" SPACING
- (M2) USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

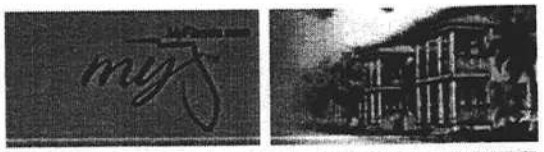
11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GADCO®

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 8-30-99		REVISED
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	MASTER DRAWING NUMBER: A10560	DRAWING NUMBER

FLORIDA DEPARTMENT OF
Community Affairs



Product Approval
USER: Public User

FL #	FL5108
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	

Product Manufacturer	MI Windows and Doors
Address/Phone/Email	650 W Market St Gratz, PA 17030 (717) 365-3300 ext 2101 surich@miwd.com
Authorized Signature	Steven Urich surich@miwd.com
Technical Representative Address/Phone/Email	
Quality Assurance Representative Address/Phone/Email	

W in door

A·L·I

(Validator / Operations Administrator)

AAMA
CERTIFICATION PROGRAM

AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc.
P.O. Box 370
Gratz, PA 17030-0370

Attn: Bill Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

1. The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION	RECORD OF PRODUCT TESTED				LABEL ORDER NO.
AAMA/NWWDA 101/I.S. 2-97 H-R55*-36x62					
COMPANY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM SIZE TESTED		
			FRAME	SASH	
MI Windows & Doors, Inc. (Oldsmar, FL) MI Windows & Doors, Inc. (Smyrna, TN)	MTL-8 MTL-9	185/3185 SH (Fin) (AL)(O/X)(OG) (ASTM)	3'0" x 5'2"	2'10" x 2'7"	By Request

2. This Certification will expire May 14, 2008 and requires validation until then by continued listing in the current AAMA Certified Products Directory.

3. Product Tested and Reported by: Architectural Testing, Inc.

   Report No.: 01-50360.02

   Date of Report: June 14, 2004

NOTE: PLEASE REVIEW, AND ADVISE ALI IMMEDIATELY IF DATA, AS SHOWN, NEEDS CORRECTION.

Date: August 1, 2005

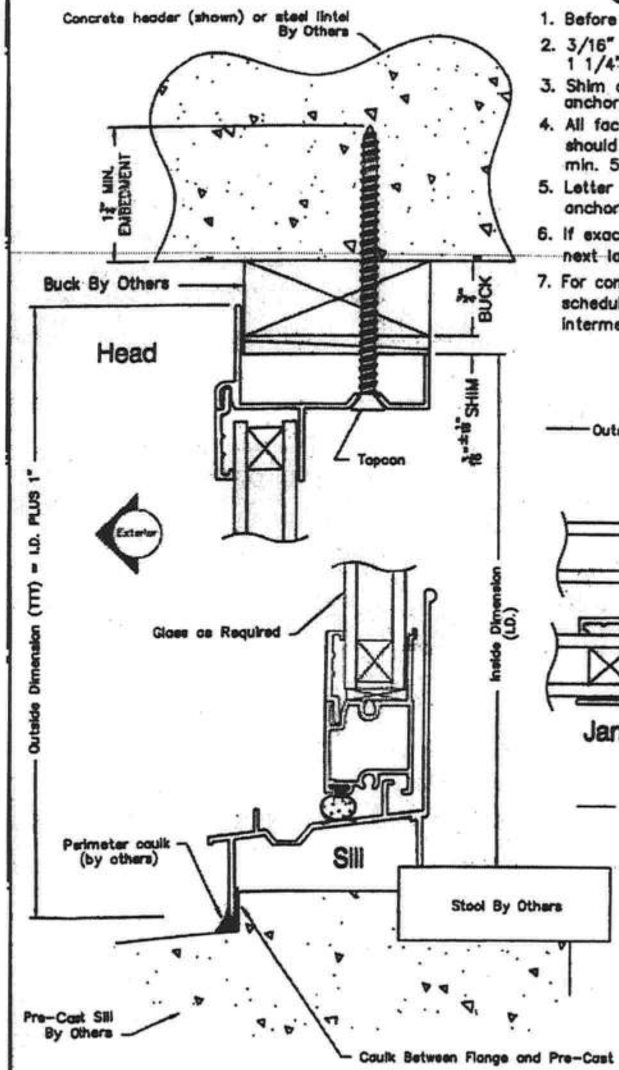
Validated for Certification:

Associated Laboratories, Inc.

Authorized for Certification:

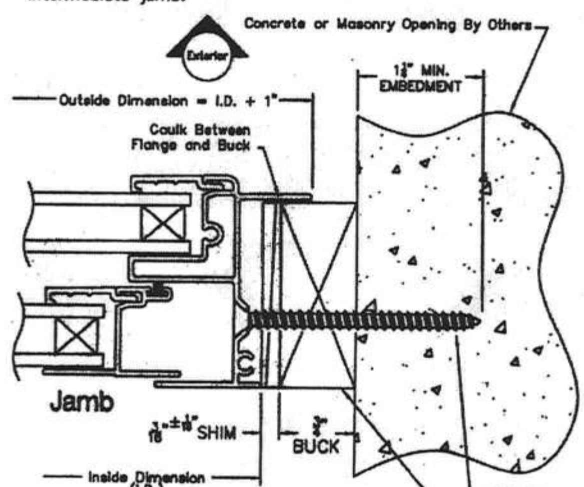
American Architectural Manufacturers Association

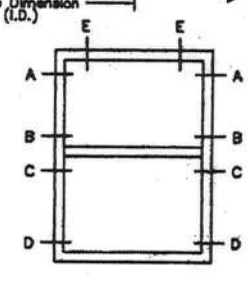
cc: AAMA
JGS/df
ACP-04 (Rev. 5/03)



"ONE BY" (3/4") BUCKS (SHOWN)

1. Before installation, caulk back of flange, or face of buck.
2. 3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.
3. Shim as required with load bearing shims at each installation anchor as shown.
4. All factory applied holes not designated for Tapcon anchor should be filled with #10 screws of sufficient length to provide min. 5/8" embedment into wood buck.
5. Letter designations on the Tapcon location chart indicate where anchors are to be installed using the elevation as a key.
6. If exact window size is not given, use anchor quantity for next larger window in chart.
7. For continuous head and sill twins & triples, use the same fastener schedule for each unit in the main frame except ignore the intermediate jamb.





"TWO BY" (1 1/2") BUCKS

"TWO BY" bucks are engineered and fastened to the masonry opening BY OTHERS.

Follow the same instructions and fastener requirements for "one by" bucks except use #10 screws of sufficient length for 1 1/4" minimum embedment into buck.

MI HOME PRODUCTS BetterBilt / Capitol

* TAPCON LOCATION CHART				
CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS		
		UP TO DP35	DP35.1 TO DP55	DP55.1 TO DP69.3
12	18 1/8 x 25	A, D & E	A, D & E	A, D & E
13	18 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
14	18 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
15	18 1/8 x 62	A, D & E	A, D & E	A, D & E
16	18 1/8 x 71	A, D & E	A, D & E	A, D & E
17	18 1/8 x 83	A, D & E	A, D & E	A, D & E
1/2 32	25 1/2 x 25	A, D & E	A, D & E	A, D & E
1/2 33	25 1/2 x 37 3/8	A, D & E	A, D & E	A, D & E
1/2 34	25 1/2 x 49 5/8	A, D & E	A, D & E	A, D & E
1/2 35	25 1/2 x 62	A, D & E	A, D & E	A, D & E
1/2 36	25 1/2 x 71	A, D & E	A, D & E	A, D & E
1/2 37	25 1/2 x 83	A, D & E	A, D & E	A, C, D & E
22	36 x 25	A, D & E	A, D & E	A, D & E
23	36 x 37 3/8	A, D & E	A, D & E	A, D & E
24	36 x 49 5/8	A, D & E	A, D & E	A, D & E
25	36 x 62	A, D & E	A, D & E	A, D & E
26	36 x 71	A, D & E	A, D & E	A, C, D & E
27	36 x 83	A, D & E	A, C, D & E	A, C, D & E
32	52 1/8 x 25	A, D & E	A, D & E	A, D & E
33	52 1/8 x 37 3/8	A, D & E	A, D & E	A, D & E
34	52 1/8 x 49 5/8	A, D & E	A, D & E	A, D & E
35	52 1/8 x 62	A, D & E	A, C, D & E	A, C, D & E
36	52 1/8 x 71	A, D & E	A, C, D & E	A, B, C, D & E
37	52 1/8 x 83	A, D & E	A, C, D & E	A, B, C, D & E
2040	23 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
2050	23 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
2060	23 3/8 x 71 5/8	A, D & E	A, D & E	A, D & E
2070	23 3/8 x 83 5/8	A, D & E	A, D & E	A, D & E
3040	35 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
3050	35 3/8 x 59 5/8	A, D & E	A, D & E	A, D & E
3060	35 3/8 x 71 5/8	A, D & E	A, D & E	A, C, D & E
3070	35 3/8 x 83 5/8	A, D & E	A, C, D & E	A, C, D & E
4040	47 3/8 x 47 5/8	A, D & E	A, D & E	A, D & E
4050	47 3/8 x 59 5/8	A, D & E	A, D & E	A, C, D & E
4060	47 3/8 x 71 5/8	A, D & E	A, C, D & E	A, C, D & E
4070	47 3/8 x 83 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4450	51 3/8 x 59 5/8	A, D & E	A, D & E	A, C, D & E
4460	51. 3/8 x 71 5/8	A, D & E	A, C, D & E	A, B, C, D & E
4470	51 3/8 x 83 5/8	A, D & E	A, C, D & E	A, B, C, D & E

*"TAPCON" TYPE HARDENED MASONRY SCREWS INCLUDE TAPCON, RAWL, & SIMPSON

SYM	REVISION	DATE	BY
A	REMOVED SILL INSTALLATION ANCHOR SCREWS	7/16/04	SB

Product Technology Corporation, Winter Park, Florida 32789

MI HOME PRODUCTS
GRATZ, PA

TITLE: 185/3185 SINGLE HUNG FLANGE FRAME INSTALLATION DETAILS & FASTENER SCHEDULE

PREPARED BY: PTC Product Technology Corporation
Phone 407.622.6334 Fax 407.622.6336

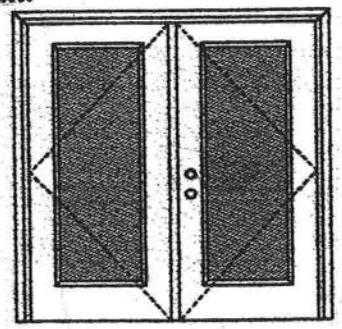
DWN: DRL	DATE: 06/15/04
SCALE: N.T.S.	DWG. NO: MIHP0059
REV. LETTER: A	SHEET: 1 OF 1

XX
Glazed Outswing Unit

COP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*

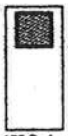

106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

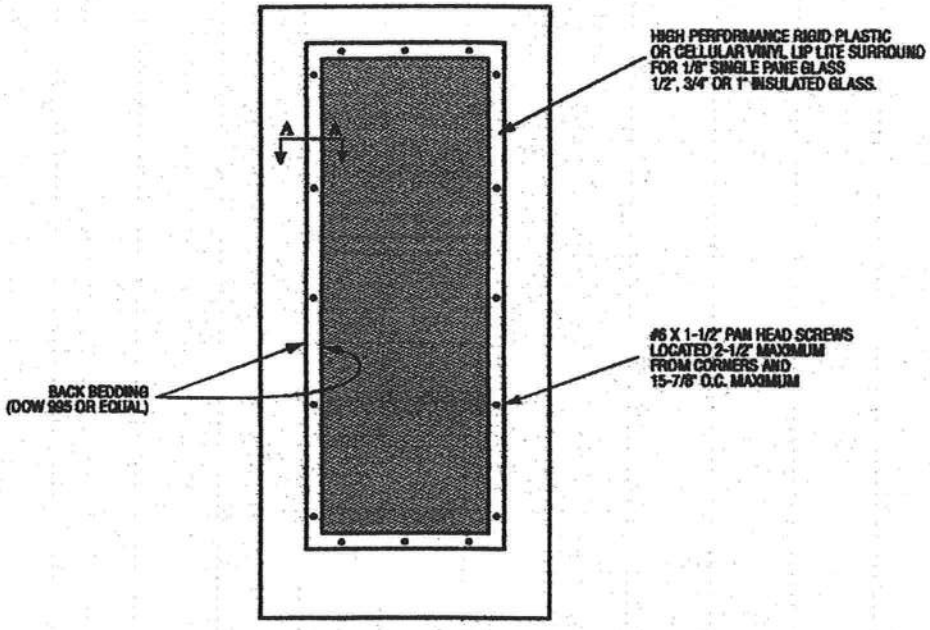
Johnson EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

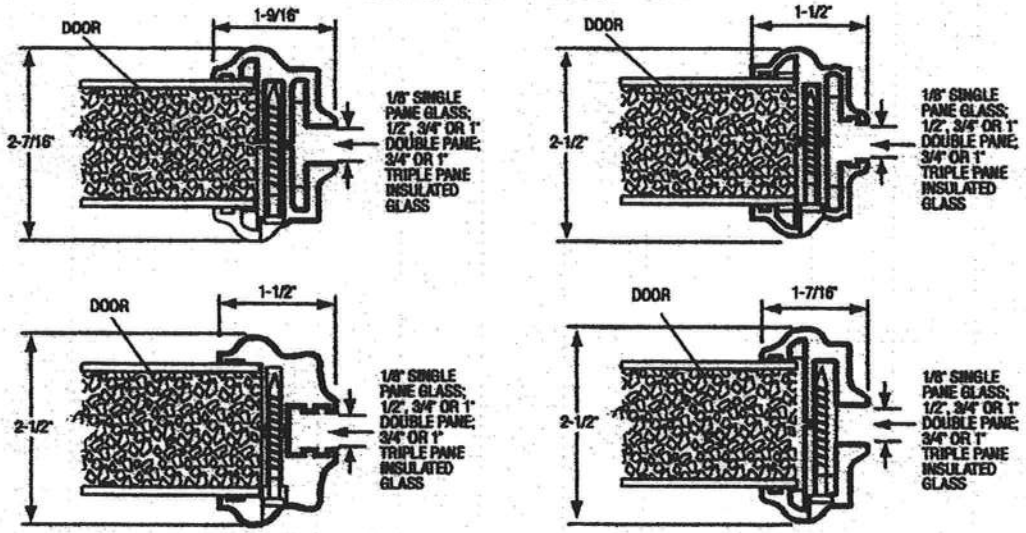
PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

GLASS INSERT IN DOOR OR SIDELITE PANEL



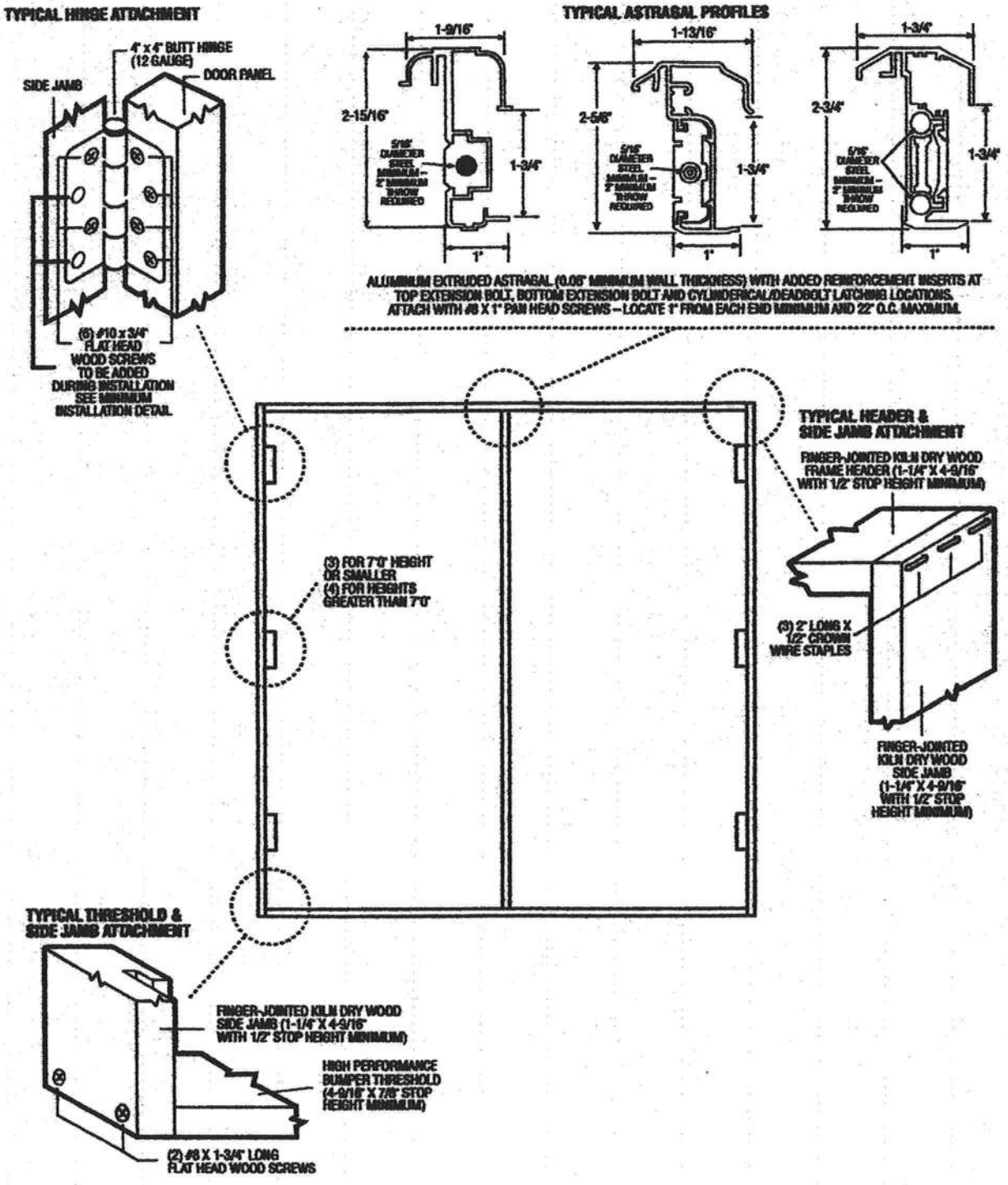
SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



XX
Unit

MAD-WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR



XX
Glazed Outswing Unit

COP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

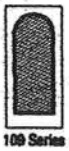
3/4 GLASS:

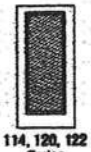

404 Series

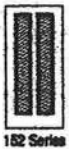

416 Series

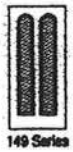

450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson EntrySystems

March 29, 2002
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PREMDOR Collection Premium Quality Doors

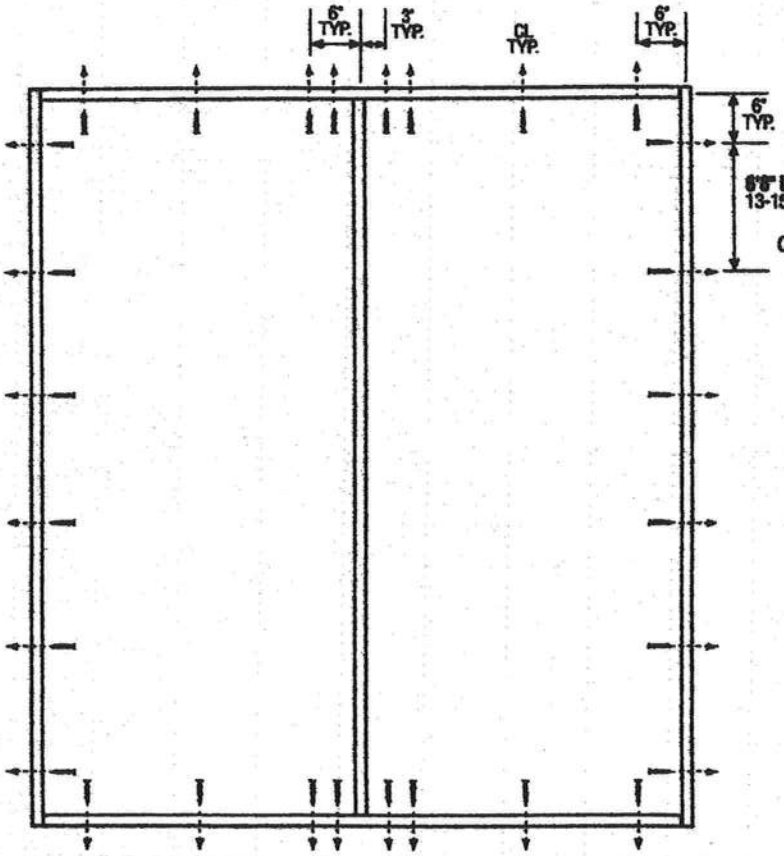
Exclusively from Masonite

Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

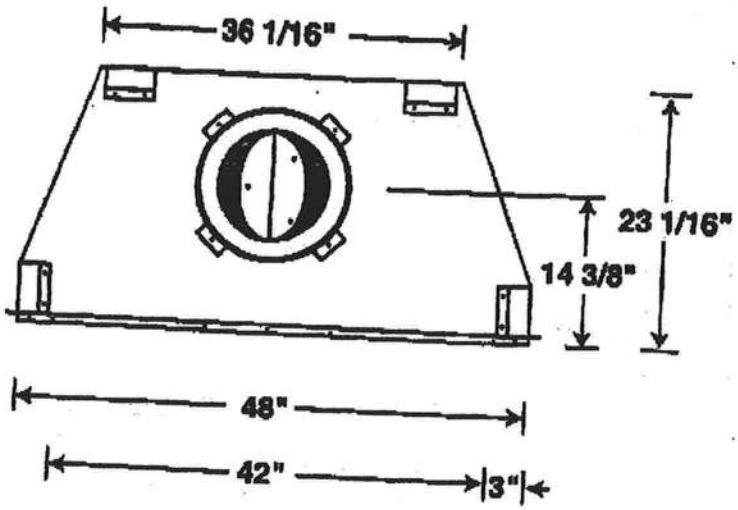
Notes:

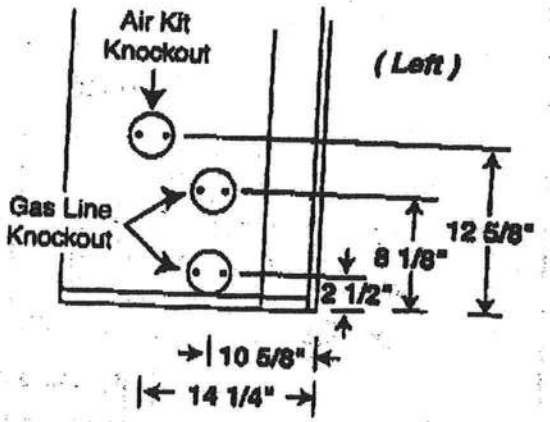
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

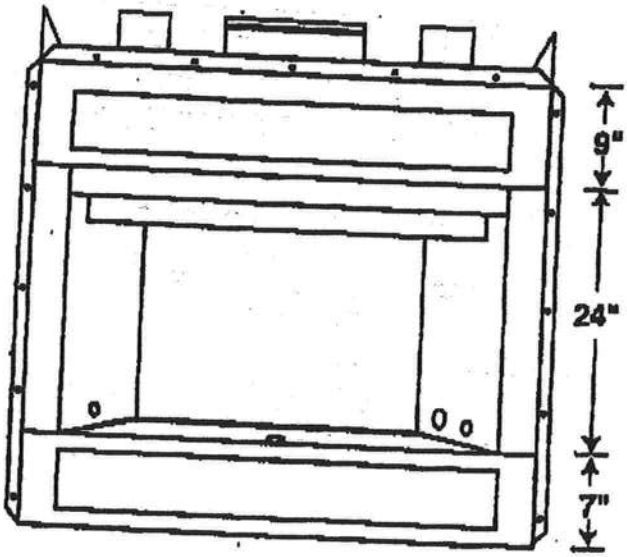
Craftsman

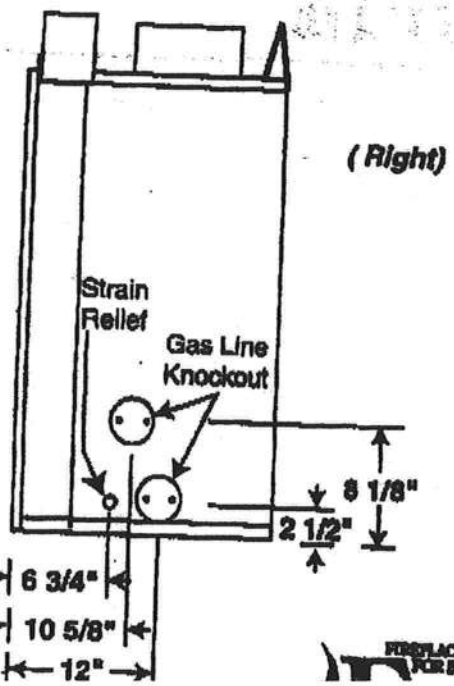
42" Woodburning Fireplac

Vent Pipe Size	10"
Min. Pipe Clearance	1"
Min. System Height	14' 6"
- w/ Single Offset	14' 6"
- w/ Two Offsets	22' 0"
Max. Dist. Between Elbows	6' 0"
Max. System Height	50' 0"









Product Approval

USER: Public User

Product Approval Menu > Product or Application Search > Application List > **Application Detail**

FL #	FL282-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	

Product Manufacturer	Entegra Roof Tile Corporation
Address/Phone/Email	819 S Federal Highway Suite 300 Stuart, FL 34994 (772) 223-0005 ext 1256 rzummo@rooftile.com
Authorized Signature	Rosemarie Zummo rzummo@rooftile.com
Technical Representative	Scott Johnson
Address/Phone/Email	819 S. Federal Hwy # 300 Stuart, FL 34994 (772) 410-5428 sjohnson@rooftile.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Roofing Tiles
Compliance Method	Certification Mark or Listing
Certification Agency	Miami-Dade BCCO - CER

Referenced Standard and Year (of Standard)	**Standard**	**Ye**
	TAS 112	19
Equivalence of Product Standards Certified By		
Product Approval Method	Method 1 Option A	
Date Submitted	07/20/2005	
Date Validated	07/20/2005	
Date Pending FBC Approval	07/25/2005	
Date Approved	08/24/2005	

Summary of Products

FL #	Model, Number or Name	Description
282.1	Concrete Flat Roofing Tile	Flat profile, interlocking, high pr concrete roof tile equipped with For direct deck or batten nail on adhesive set application.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0417.09 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:
282.2	Estate "S" Tile	Low profile, interlocking, extrud tile equipped with two nail hole ribs. For direct deck or battenec or adhesive set.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0703.04 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:
282.3	Skandia Roof Tile	Flat profile concrete roof tile for battened nail-on
Limits of Use (See Other) **Approved for use in HVHZ:**		**Certification Agency Certifica** **Installation Instructions**

Approved for use outside HVHZ: **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 00.1106.03 Can be installed in HVHZ		Verified By:
282.4	Valencia Spanish "S" Concrete	High profile, interlocking, one-p concrete roof tile equipped with For direct deck nail-on, mortar s set applications.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0417.08 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:

Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Comfort Survey & Home Analysis

Name Moise Desravines

Address 1924 SW Nautilus Rd.

City Lake City State FL. Zip 32024

Home Phone (386) 752-3979 Work Phone (____) _____

Email Address _____

COMPLETED BY COOKS HEAT & AIR

BERNARD L. JOHNSON

1924 SW NAUTILUS RD

Accu-Size Cooling & Heating Analysis

Cooling Load (Heat Gain) - 95 Degree Day

Square Footage of Windows			Heat Gain
North (single)		X 26 =	
North (double)	118	X 21 =	2478
NE & NW (single)		X 45 =	
NE & NW (double)		X 35 =	
East & West (single)		X 60 =	
East & West (double)	215	X 49 =	10,535
SE & SW (single)		X 50 =	
SE & SW (double)		X 40 =	
South (single)		X 36 =	
South (double)	36	X 25 =	900

Square Footage of Doors			Heat Gain
Wood (no storm door)		X 13 =	
Wood (w/ storm door)		X 9 =	
Insulated Metal Door	66	X 6 =	396

Wall Perimeter ___ X Ht. __ = 3300
Less glass area 369 Net Wall 2931

Square Footage of Net Walls			Heat Gain
No insulation	2931	X 6 =	17586
R-13 (3.5" insulation)		X 3 =	
R-19 (6" insulation)		X 2 =	

Square Footage of Ceiling			Heat Gain
No insulation		X 22 =	
R-11 (3" insulation)		X 4.1 =	
R-19 (6" insulation)		X 2.6 =	
R-30 (10" insulation)	3426	X 1.6 =	5481

Square Footage of Floor			Heat Gain
No insulation		X 3 =	
Carpet (no insulation)		X 2 =	
R-11 (3"+ insulation)		X 1 =	
Floor on Slab		X 0 =	0

Infiltration/Ventilation			Heat Gain
Home square feet	3426	X 3.5 =	11,991

Internal Gains			Heat Gain
Number of People	4	X 530 =	2120
Kitchen & Bath Allowance			1250

Subtotal BTU/h Heat Gain = 52,737

Gains from Ductwork		Heat Gain
In crawl Space - (Subtotal BTUh X .09)		
In Attic - (Subtotal BTUh X .13)		6855

Total BTU/h Heat Gain = 59,592

Heating Load (Heat Loss) - 20 Degree Day

Square Footage of Windows			Heat Loss
Single Glass		X 71 =	
Double Glass		X 50 =	

Square Footage of Doors			Heat Loss
Single Glass Patio		X 76 =	
Double Glass Patio		X 53 =	
Wood (no storm door)		X 56 =	
Wood (w/ storm door)		X 33 =	
Insulated Metal Door		X 25 =	

Square Footage of Net Walls			Heat Loss
Frame (no insulation)		X 15 =	
Frame (3.5" insulation)		X 5 =	
Frame (6" insulation)		X 4 =	
Masonry (no insulation)		X 28 =	
Masonry (1" insulation)		X 8 =	

Square Footage of Ceiling			Heat Loss
No insulation		X 20 =	
R-11 (3" insulation)		X 5 =	
R-19 (6" insulation)		X 3 =	
R-30 (10" insulation)		X 2 =	

Sq. Ft. of Floor over Crawl area			Heat Loss
No insulation		X 13 =	
Carpet (no insulation)		X 8 =	
R-11 (3"+ insulation)		X 5 =	

Sq. Ft. of Floor over Basement			Heat Loss
No insulation		X 1.5 =	
Carpet or insulation		X .8 =	

Perimeter* of Slab Floor			Heat Loss
Slab (no insulation)		X 41 =	
Slab (edge insulation)		X 18 =	

Infiltration/Ventilation			Heat Loss
Home square feet		X 3.7 =	

Subtotal BTU/h Heat Loss =

Losses from Ductwork		Heat Loss
In crawl Space - (Subtotal BTUh X .10)		
In Attic - (Subtotal BTUh X .08)		

Total BTU/h Heat Loss =

80% Furnace Efficiency loss	X .25 =	
90%+ Furnace Efficiency loss	X .12 =	

Total BTU/h Input Needed =

12,000 / 59,592 = 5 TON UNIT & 1.5 TON

FLORIDA DEPARTMENT OF Community Affairs

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FLORIDA DEPARTMENT OF Community Affairs

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- HOUSING & COMMUNITY DEVELOPMENT
- EMERGENCY MANAGEMENT
- OFFICE OF THE SECRETARY

Product Approval

USER: Public User

Product Approval Menu > Product or Application Search > Application List > **Application Detail**

FL #	FL282-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	Entegra Roof Tile Corporation
Address/Phone/Email	819 S Federal Highway Suite 300 Stuart, FL 34994 (772) 223-0005 ext 1256 rzummo@rooftile.com
Authorized Signature	Rosemarie Zummo rzummo@rooftile.com
Technical Representative	Scott Johnson
Address/Phone/Email	819 S. Federal Hwy # 300 Stuart, FL 34994 (772) 410-5428 sjohnson@rooftile.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Roofing Tiles
Compliance Method	Certification Mark or Listing
Certification Agency	Miami-Dade BCCO - CER

Referenced Standard and Year (of Standard)	**Standard**	**Ye**
	TAS 112	19
Equivalence of Product Standards Certified By		
Product Approval Method	Method 1 Option A	
Date Submitted	07/20/2005	
Date Validated	07/20/2005	
Date Pending FBC Approval	07/25/2005	
Date Approved	08/24/2005	

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Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0417.09 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:
282.2	Estate "S" Tile	Low profile, interlocking, extrud tile equipped with two nail hole ribs. For direct deck or battenec or adhesive set.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0703.04 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:
282.3	Skandia Roof Tile	Flat profile concrete roof tile for battened nail-on
Limits of Use (See Other) **Approved for use in HVHZ:**		**Certification Agency Certifica** **Installation Instructions**

Approved for use outside HVHZ: **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 00.1106.03 Can be installed in HVHZ		Verified By:
282.4	Valencia Spanish "S" Concrete	High profile, interlocking, one-p concrete roof tile equipped with For direct deck nail-on, mortar s set applications.
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** Miami-Dade NOA # 01-0417.08 Can be installed in HVHZ		**Certification Agency Certifica** **Installation Instructions** Verified By:

Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Occupancy	Threshold	Exception
All occupancies	Buildings three stories or more in height	One- and two-family dwellings, parking garages, telecommunication spaces
Assembly (A-1, A-3, A-4)	Fire area > 12,000 sq.ft. or fire area occupant load > 300 or fire area above/below level of exit discharge. Multitheater complex (A-1 only)	Participant sport areas at level of exit discharge, places of worship with fixed seating and not part of a mixed occupancy A-3, A-4
Assembly (A-2)	Fire area > 5,000 sq.ft. or fire area occupant load > 300 or fire area above/below level of exit discharge.	None
Assembly (A-5)	Accessory areas > 1,000 sq.ft.	None
Educational (E)	Fire area > 20,000 sq.ft. or level of exit discharge.	Existing buildings
Factory (F-1) Mercantile (M) Storage (S-1)	Fire area > 12,000 sq. ft. or Building > 3 stories or Combined fire area > 24,000 sq.ft. Woodworking > 2,500 sq.ft. (F-1 only). Bulk storage of tires > 20,000 cu.ft. (S-1 only).	None
High hazard (H-1, H-2, H-3, H-4, H-5)	Sprinklers required.	None
Institutional (I-1, I-2, I-3)	Sprinklers required.	None
Residential (R)	Sprinklers required.	None
Repair garage (S-1)	Fire area > 12,000 sq.ft. or ≥ 2 stories (including basement) with fire area > 10,000 sq.ft. or repair garage servicing vehicles in basement.	None
Parking garage (S-2)	Enclosed automobile parking—sprinklers required. Commercial trucks/buses parking area > 5,000 sq.ft.	None
Covered malls (402.8)	Sprinklers required.	Attached open parking structures.
High rises (403.2, 403.3)	> 75 feet above vehicle access.	Airport traffic control towers, open garages. A-5
Unlimited area buildings (507)	A-3, A-4, B, E, F, M, S: 1 story. B, F, M, S: 2 story.	One story F-2 or S-2.

Note: Thresholds located in Section 903.2 unless noted. See also Table 903.2.13 for additional required suppression systems.

For SI: 1 foot = 304.8 mm, 1 square foot = 0.0929 m^2.

Figure 903.2
SUMMARY OF OCCUPANCY-RELATED AUTOMATIC SPRINKLER THRESHOLDS

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001558

DATE: 02/15/2008 BUILDING PERMIT NO. 26765

APPLICANT MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

ADDRESS 1924 SW NAUTILUS ROAD LAKE CITY FL 32024

OWNER MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

ADDRESS 1924 SW NAUTILUS ROAD LAKE CITY FL 32024

CONTRACTOR MOISE & SCHENETTER DESRAVINES PHONE 386.752.3979

LOCATION OF PROPERTY 47-S TO WALTER LITTLE RD,TL CROSS OVER C-240 TO OLD WIRE RD

TO NAUTILUS,TL AROUND CURVE ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

PARCEL ID # 24-5S-16-03706-004

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Schenetter Desravines

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: Scott Phillips DATE: 2-20-08

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

RECEIVED
FEB 19 2008
By

COLUMBIA COUNTY GATEWAY TO FLORIDA