A New Warehouse for MAYO FERTILIZER

Lake City;, Florida

FLORIDA BUILDING CODES

FACILITY DESCRIPTION

BUILDING LOCATION – SEE SITE PLAN FOR SPECIFIC ADDRESS
LAND USE –
BUILDING CONSTRUCTION TYPE – TYPE II–B, UNSPRINKLERED
BUILDING USE AND CLASSIFICATION – STORAGE (S–1)
ALLOWABLE HEIGHT / BUILDING HEIGHT – 1 STORY
TYPE OF ROOF – METAL ROOF

BUILDING AND SITE INFORMATION

ZONING – INDUSTRIAL
LAND USE – BUSINESS (EXISTING)
PARKING LOT CRITERIA (EXISTING)

AREA TABULATION

EXISTING BUILDING FIRE AREA	4,500 SF
ADDITION BUILDING FIRE AREA	9,466 SF
TOTAL	13,966 SF

OCCUPANCY CLASSIFICATION AND LOAD

OCCUPANCY CLASSIFICATION STORAGE (S–1) – WAREHOUSE

WAREHOUSE	9,000 SF/500 = 18
LOADING DOCK	466 SF/500 = 5
TOTAL–	23 OCCUPANTS

PLUMBING PROVISIONS

OCCUPANT LOAD : 23 OCCUPANTS

PLUMBING REQUIREMENTS

EXISTING TOILET LOCATED WITHIN THE PREMISES 500 FT. AWAY FROM PROPOSED WAREHOUSE

EGRESS REQUIREMENTS

EXIT ACCESS TRAVEL DISTANCE
2010 FBC TABLE 1016.1 – NON–SPRINKLERED

MAXIMUM DISTANCE TO EXIT (REQUIRED / PROVIDED) – 300'–0" / 83'–2"
MINIMUM NUMBER OF EXITS (REQUIRED / PROVIDED) – 2 / 2
MINIMUM WIDTH OF MAIN EXIT – 36" = 150 PERSON CAPACITY

STRUCTURAL LOADS

ROOF LIVE LOADS –	20	PSF REDUCIBLE
COLLATEAL LOAD –	3	PSF
FLOOR LIVE LOADS –	250	PSF
WIND SPED –	120	PSF
BASIC VELOCITY PRESSURE	32.4	PSF @ 30'–0" MEAN ROOF HT.

WIND LOADS

DESIGN CRITERIA		
NOMINAL WIND SPEED Vasd	93 MPH	
ULTIMATE WIND SPEED Vult	120 MPH	
WIND RISK CATEGORY	II	
EXPOSURE CATEGORY	C	
INTERNAL WIND COEFFICIENT	+0.18 & –0.18	
ALL COMPONENTS INCLUDING DOORS AND WINDOWS SHALL BE DESIGNED TO THE FOLLOWING PRESSURES:		
EFFECTIVE AREA SQ FT.	PRESSURE (PSF) OUT	IN
0	31.1	–33.7
0	29.8	–32.4
0	27.9	–30.3
00	26.6	–29.2

DESIGN CODE REFERENCE

BUILDING CODE – FLORIDA BUILDING CODE – 2010 EDITION
ELECTRICAL CODE – NATIONAL ELECTRIC CODE – 2008 EDITION
PLUMBING CODE – FLORIDA PLUMBING CODE – 2010 EDITION
MECHANICAL CODE – FLORIDA MECHANICAL CODE – 2010 EDITION
FIRE CODE – FLORIDA FIRE PREVENTION CODE – 2010 EDITION
GAS CODE – FLORIDA FUEL GAS CODE – 2010 EDITION
NFPA 58 – LIQUEFIED PETROLIUM GAS – (LATEST EDITION)
NFPA 13 – SPRINKLER SYSTEM INSTALLATION (LATEST EDITION)
NFPA 72 – NATIONAL FIRE ALARM CODE (LATEST EDITION)
NFPA 72E AUTOMATIC DETECTION DEVICES
NFPA LIFE SAFETY CODE (LATEST EDITION)
NFPA 80 – FIRE DOORS AND WINDOWS (LATEST EDITION)
OSHA
ACI–318–08 BUILDING REQUIREMENTS FOR REINFORCED CONCRETE
AISC CODE OF STANDARD PRACTICE (MANUAL OF STEEL CONSTRUCTION, 9th EDITION)
S–310 SPECIFICATIONS FOR THE DESIGN FABRICATION AND ERECTION OF STEEL
STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, ADOPTED BY THE STEEL JOIST INSTITUTE AND THE AISC
SPECIFICATIONS FOR STRUCTURAL JOISTS USING ASTM A–325 BOLTS
STEEL DUCT INSTITUTE DESIGN MANUAL
STRUCTURAL WELDING CODE AWS D1.1
AISI SPECIFICATIONS FOR THE DESIGN OF LIGHT GAGE COLD FORMED STRUCTURAL STEEL MEMBERS FOR METAL DECKS
METAL BUILDING MANUFACTURERS ASSOCIATIONS DESIGN PRACTICES MANUAL
ASHRAE FOR MECHANICAL WORK
ASME BOILER CODES

CONSTRUCTION NOTES

GENERAL

G01 – THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
G02 – THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, ALL WORKERS AND ALL OTHER PEOPLE PRESENT DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION.
G03 – THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR ANCHORS, EMBEDS, SUPPORTS, OR ANY OTHER ITEMS WHICH MAY EFFECT STRUCTURAL DRAWINGS.

FOUNDATION

F01 – ALL ORGANIC MATERIALS, REFUSE MATERIALS AND SOFT AREAS IN SOIL SHALL BE REMOVED AND THE SOIL PREPARED FOR AN ALLOWABLE BEARING PRESSURE OF 2000psi.
F02 – SHOULD THE CONTRACTOR DISCOVER ANY CONDITION WHICH COULD PREVENT THE ATTAINMENT OF THE STATED DESIGN PRESSURE, HE SHALL NOTIFY THE ARCHITECT OR ENGINEER IMMEDIATLY.

SLAB ON GRADE

F03 – COMPACT INTERIOR FILL TO 95% MINIMUM MAX DRY DENSITY (MODIFIED PROCTOR, ASTM D1557–58T OPTIMUM MOISTURE CONTENT). SOIL COMPACTION SHALL BE FIELD CONTROLLED BY A REPRESENTATIVE FROM A QUALIFIED LABORATORY APPROVED BY THE ENGINEER. EACH LAYER OF FILL SHALL NOT EXCEED 10" AND SHALL BE COMPACTED PRIOR TO THE PLACEMENT OF THE NEXT FILL LAYER.
F03 – ALL FLOOR SLABS SHALL BE PLACED UPON A 4" THICK SAND LAYER FOR FINE GRADING.

CONCRETE AND REINFORCING

C01 – CONCRETE WORK SHALL CONFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318–08).
C02 – CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH AS FOLLOWS:
FOUNDATIONS, FILLED CELLS AND CONCRETE BEAMS – 3000psi
SLABS ON GRADE– 3000psi
C03 – ALL REBAR SHALL CONFORM TO ASTM–615, GRADE 40, AND SHALL LAP A MIMIMUM OF 25" AT ALL JOINTS. SLABS WITH FIBER MESH OR WELDED WIRE FABRIC SHALL CONFORM TO ASTM A–185–02.
C04 – COVER FOR ALL REINFORCING STEEL SHALL BE AS FOLLOWS (UNLESS NOTES):
ALL FOOTINGS – 3"
SLABS ON GRADE – 1–1/2" FROM TOP OF CONCRETE
ALL BEAMS – 1–1/2" ON STIRRUPS
C05 – VERTICAL REBAR WALL REINFORCING SHALL BE A STANDARD HOOK WITH A 25" PROJECTION ABOVE SLAB AND HAVE A MINIMUM 7" EMBEDMENT.

MASONRY

M01 – ALL MASONRY CONSTRUCTION SHALL COMFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530–08).
M02 – ALL CONCRETE BLOCKS SHALL CONFORM TO ASTM C 90. 128 DAY STRENGTH MUST EQUAL 2000psi, FM SHALL EQUAL 1500psi LAID IN A RUNNING BOND.
M03 – ALL MORTAR SHALL BE TYPE S OR M.
M04 – REINFORCE ALL CMU WALLS WITH A CONTINUOUS HORIZONTAL BOND BEAM GROUTED SOLID AND REINFORCE WITH A MINIMUM OF (1) #5 REBAR WITH A 25" OVERLAP AT EACH JOINT.
M05 – WHERE SHOWN, ALL VERTICAL CELLS OF BLOCK MASONRY SHALL BE FILLED WITH 3000psi GROUT HAVING A 28 DAY STRENGTH OF 3000psi AND A GROUND SLUMP NOT LESS THAN 8". REINFORCE WITH A MINIMUM OF (1) #5 VERTICAL REBAR WITH A MINIMUM 25" OVERLAP AT EACH JOINT.
M06 – GROUT FOR FILLED CELLS SHALL BE POURED OR PUMPED IN LIFTS NOT TO EXCEED 10'–0" IN HEIGHT AND SHALL BE CONSOLIDATED AT THE TIME OF POURING BY RODDING OR VIBRATING.
M07 – PROVIDE KNOCKOUTS IN CMU AT TH BASE OF EACH FILLED CELL TO ALLOW VISUAL VERIFICATION OF COMPLETE GROUT PENETRATION.

STRUCTURAL LUMBER

L01 – ALL STRUCTURAL AND LOAD BEARING WALLS SHALL HAVE A MINIMUM FB OF 1200psi.
L02 – WALLS HIGHER THAN 9'–0" SHALL HAVE INTERMEDIATE BRIDGING SPACED NOT GREATER THAN 72" APART.
L03 – ALL LOAD BEARING WALLS SHALL HAVE A DOUBLE SYP No.2 TOP PLATE.
L04 – ALL WALLS ON MASONRY OR CONCRETE SHALL HAVE SOLE SATL SOLUTION TREATED SOLE PLATES.

STRUCTURAL STEEL

S01 – STEEL WORK SHALL CONFORM TO THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (USE THE LATEST EDITION).

DRAWING INDEX

ARCHITECTURAL

11/19/2013 4:08 PM

DRAWING ISSUE: FOR PERMIT
ISSUE DATE: 19 NOV 2013
REVISED:

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REGISTRATIONS:
STATE OF FLORIDA AR 12,999
STATE OF GEORGIA 5044

PROJECT

A NEW WAREHOUSE FOR:

MAYO FERTILIZER

LAKE CITY, FLORDA

DRAWN BY: ESB
CHECKED BY:
APPROVED BY: DPK
ARCHITECT'S PROJECT No.:

SHEET TITLE:
COVER SHEET
BUILDING CODE DATA

SHEET NUMBER
C1

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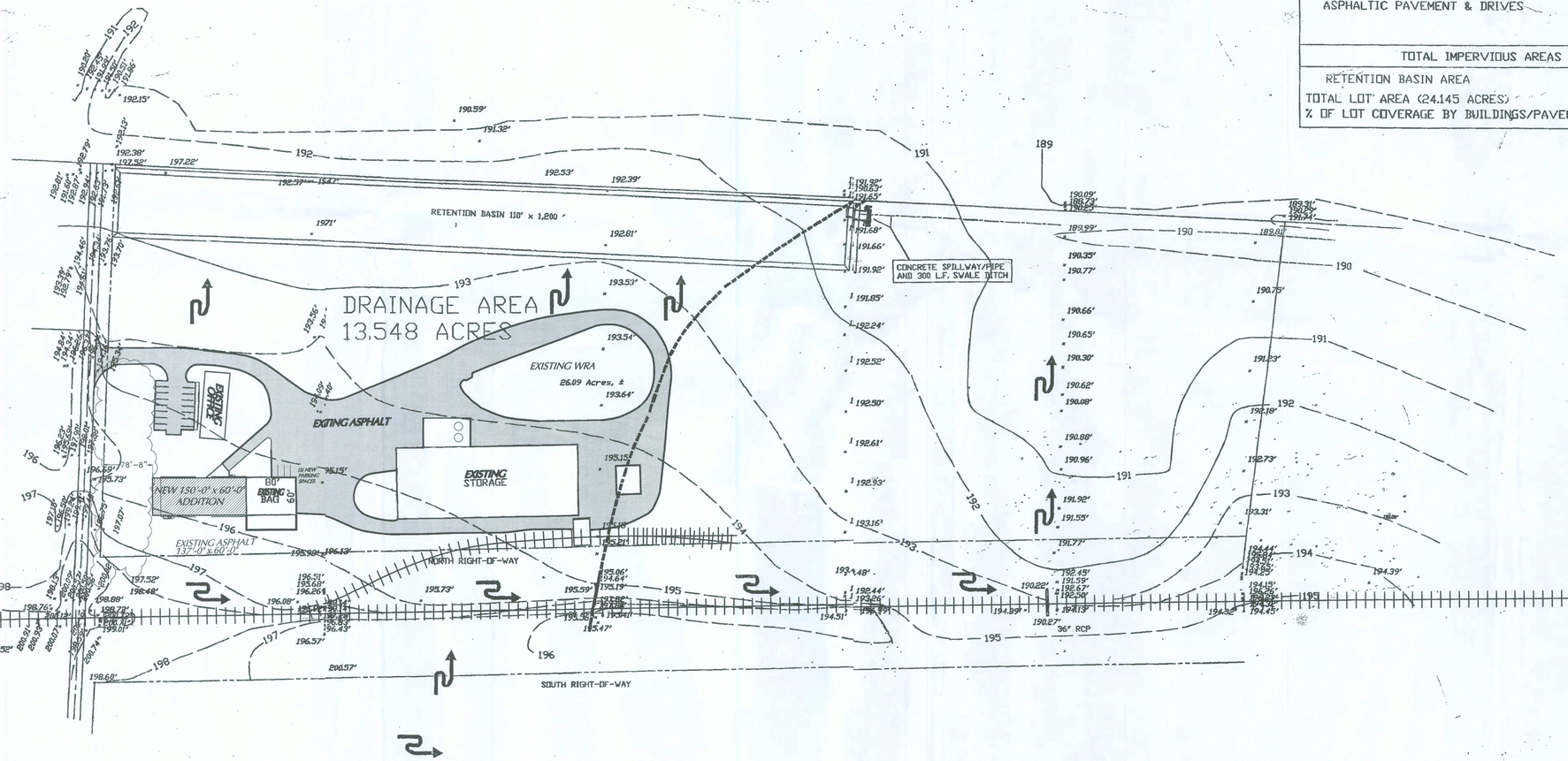
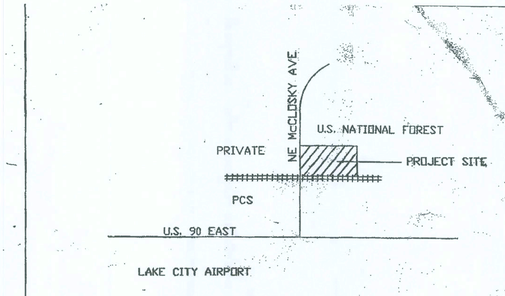
SITE PLAN

Scale: 1" = 100'

DECRIPTION:
ALL THAT PORTION OF THE NE 1/4 OF THE NW 1/4, AND THE NW 1/4 OF THE NE 1/4 OF SECTION 36, TOWNSHIP 3 SOUTH, RANGE 17 EAST, WHICH LIES EAST OF FOREST SERVICE ROAD NO. 236 AND NORTH OF THE SEABOARD COASTLINE RAILROAD, AND IS DESCRIBED MORE PARTICULARLY AS FOLLOWS: COMMENCE AT THE NW CORNER OF SAID SECTION 36 AND RUN THENCE N.86°48'07"E., ALONG THE NORTH BOUNDARY OF SAID SECTION 36 A DISTANCE OF 1675.73 FEET TO THE EASTERLY RIGHT-OF-WAY OF FOREST SERVICE ROAD NO. 236 AND THE POINT OF BEGINNING; THENCE CONTINUE N.86°48'07"E., STILL ALONG THE NORTH BOUNDARY OF SAID SECTION 36 A DISTANCE OF 701.08 FEET TO THE NE CORNER OF SAID NE 1/4 OF THE NW 1/4; THENCE N.86°50'08"E., STILL ALONG SAID NORTH BOUNDARY OF SAID SECTION 36 A DITANCE OF 1187.42 FEET TO THE NE CORNER OF SAID NW 1/4 OF THE NE 1/4; THENCE S.01°05'33"W., ALONG THE EAST LIE OF SAID NW 1/4 OF NE 1/4 A DISTANCE OF 502.11 FEET TO THE NORTHERLY RIGHT-OF-WAY OF SEABOARD COSTLINE RAILROAD; THENCE S.82°58'21"W., ALONG SAID RAILROAD RIGHT-OF-WAY A DISTANCE OF 1847.50 FEET TO THE EASTERLY RIGHT-OF-WAY OF FOREST SERVICE ROAD NO. 236; THENCE N.03°48'36"W., ALONG SAID EASTERLY RIHT-OF-WAY A DISTANCE OF 624.90 FEET TO THE POINT OF BEGINNING.





LOCATION MAP

SITE DEVELOPMENT INFORMATION	
IMPERVIOUS AREAS:	
OFFICE BUILDING	2,400 S.F.
BAG BUILDING	4,800 S.F.
STORAGE/BLENDING BUILDING	35,214 S.F.
CONCRETE PAVEMENT/SIDEWALK	462 S.F.
ASPHALTIC PAVEMENT & DRIVES	45,116 S.F.
TOTAL IMPERVIOUS AREAS	87,992 S.F.
RETENTION BASIN AREA	132,000 S.F.
TOTAL LOT AREA (24.145 ACRES)	1,051,751 S.F.
% OF LOT COVERAGE BY BUILDINGS/PAVEMENT	8.4 %

KEEN ENGINEERING & SURVEYING, INC.
LIVE OAK, FLORIDA 32060
9863 CR 417
904/362-4787
ENG. LIC. EB 3761

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LAKE CITY, FLORIDA

DRAWN BY: ESB
CHECKED BY:
APPROVED BY: DPK
ARCHITECT'S PROJECT No.:

SHEET TITLE:
SITE PLAN

SHEET NUMBER
SP-1

MAYO FERTILIZER - LAKE CITY BRANCH
LAKE CITY, FLORIDA

SITE DEVELOPMENT PLAN
MISC. NOTES, REFERENCES AND INSTRUCTIONS

SHEET No.	DRAWN BY:	DATE
SP-1	ESB	11/12/04

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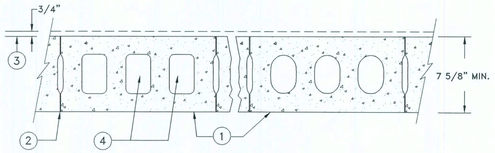


DESIGN No. U904

NTS

BEARING WALL RATING – 3 HR
NON-BEARING WALL RATING – 3 HR

HORIZONTAL SECTION

1. **CONCRETE BLOCKS*** – VARIOUS DESIGNS. CLASSIFICATION C-3 (3 HR)
   SEE CONCRETE BLOCKS CATEGORY FOR LIST ELIGIBLE MANUFACTURERS.

2. **MORTAR** – BLOCKS LAID IN FULL BED OF MORTAR, NOM 3/8 IN THICK, OF NOT LESS THAN 2-1/4 AND NOT MORE THAN 3-1/2 PARTS OF CLEAN, SHARP SAND TO ONE PART PORTLAND CEMENT (PROPORTIONED BY VOLUME) AND NOT MORE THAN FIFTY PERCENT HYDRATED LIME (BY CEMENT VOLUME). VERTICAL JOINTS STAGGERED.

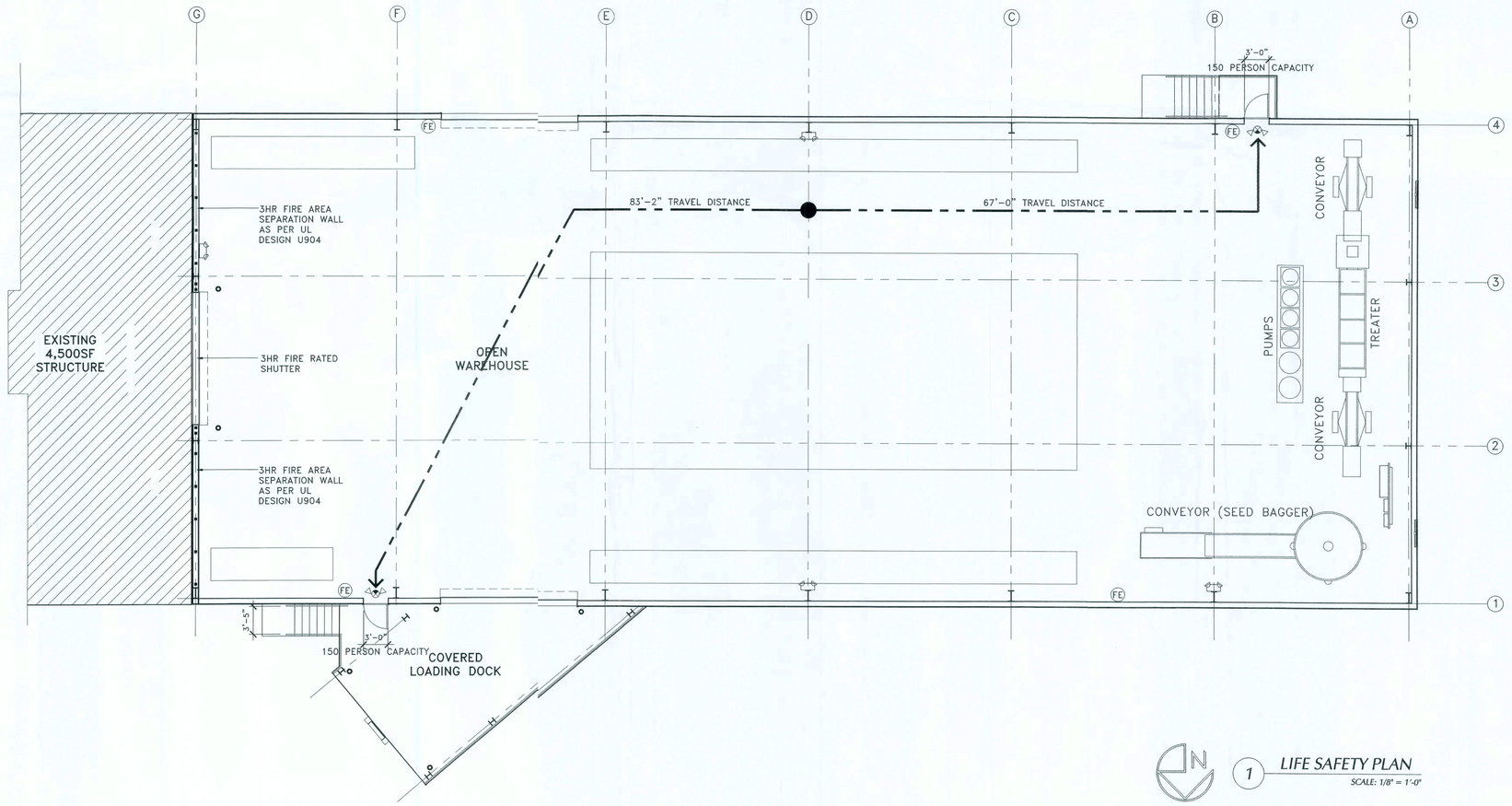
3. **PORTLAND CEMENT STUCCO OR GYPSUM PLASTER** – ADD 1/2 HR TO CLASSIFICATION IF USED. ATTACHED TO CONCRETE BLOCKS (ITEM 1).

4. **LOOSE MASONRY FILL**– IF ALL CORE SPACES ARE FILLED WITH LOOSE, DRY, EXPANDED SLAG, EXPANDED CLAY OR SHALE (ROTARY KILN PROCESS), WATER REPELLANT VERMICULITE MASONRY FILL INSULATION OR SILICONE TREATED PERLITE LOOSE FILL INSULATION, ADD 1 HR TO CLASSIFICATION.

5. **FOAMED PLASTIC*** – (OPTIONAL – NOT SHOWN) – 1-1/2 IN THICK MAX, 4 FT WIDE SHEATHING ATTACHED TO CONCRETE BLOCKS (ITEM 1).
   BPB AMERICA INC
   BPB CELOTEX – TYPE THERMAX

* BEARING THE UL CLASSIFICATION MARKING

SYMBOL LEGEND	
	COMBO ILLUMINATED EXIT LIGHT AND 2-HEAD EMERGENCY LIGHT W/ BATTERY BACK UP
	ILLUMINATED EXIT LIGHT W/ BATTERY BACK UP
	ILLUMINATED DIRECTIONAL EXIT LIGHT W/ BATTERY BACK UP
	WALL MOUNTED, 2-HEAD EMERGENCY LIGHT W/ BATTERY BACK UP
(S)	SMOKE DETECTOR
(FE)	20# CLASS D



1 LIFE SAFETY PLAN
SCALE: 1/8" = 1'-0"

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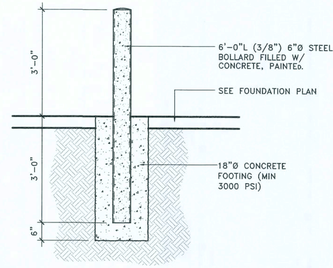

2 *TYPICAL BOLLARD DETAIL*
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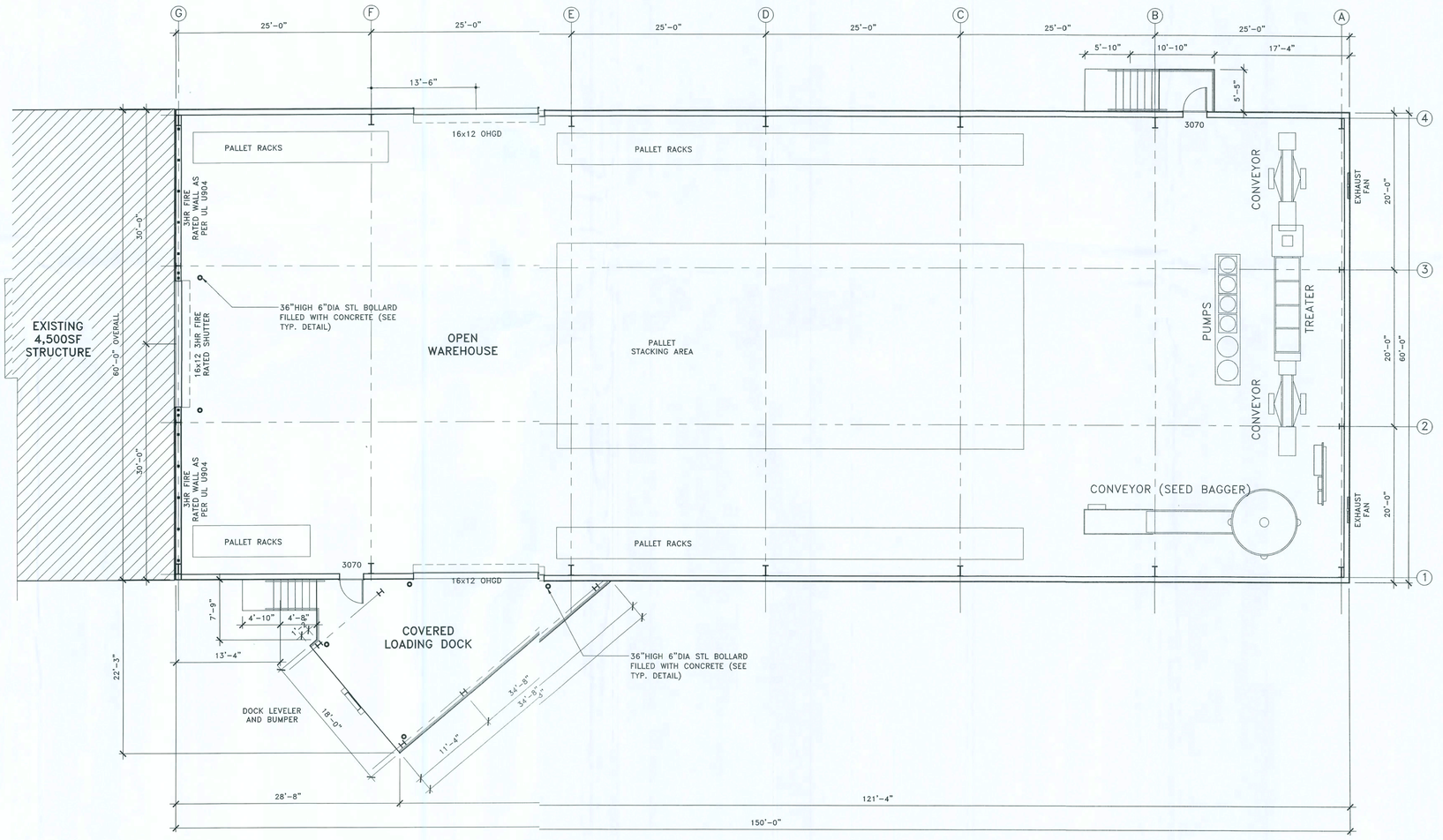

AREA CALCULATION

MAIN BUILDING –	9,000 SF
COVERED DOCK –	466 SF
TOTAL AREA –	9,466 SF

1 *FLOOR PLAN*
SCALE: 1/8" = 1'-0"

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FLOOR PLAN

SHEET NUMBER

A1.0

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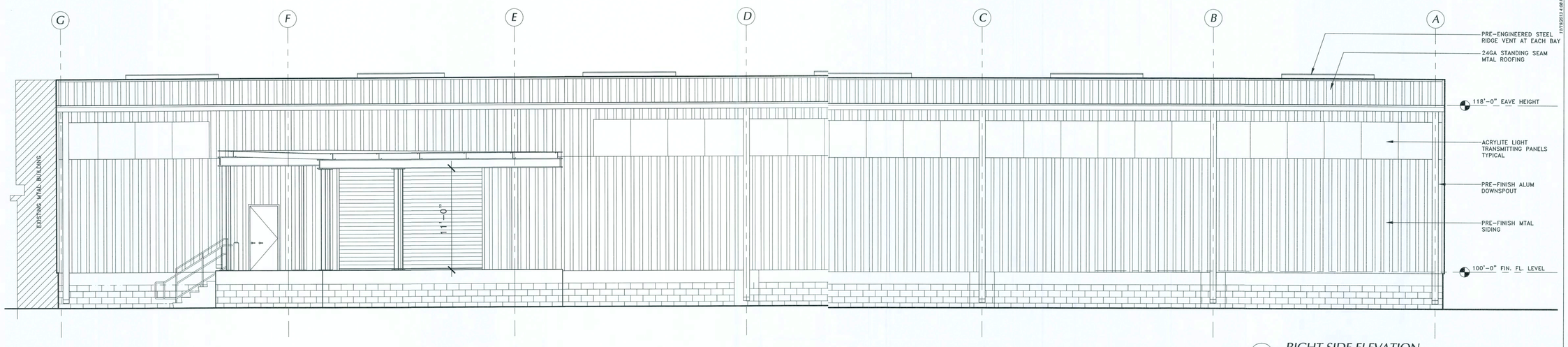
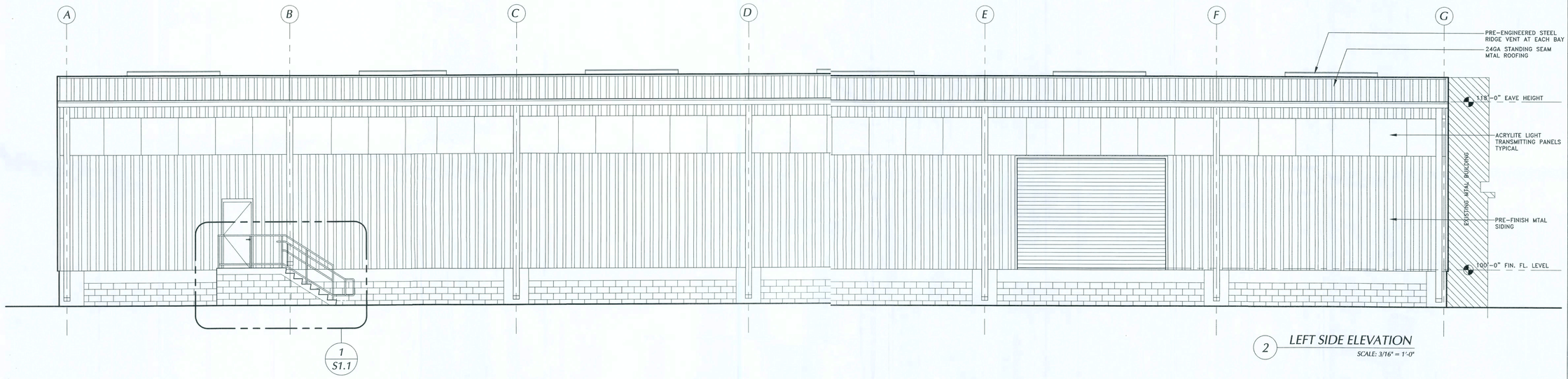
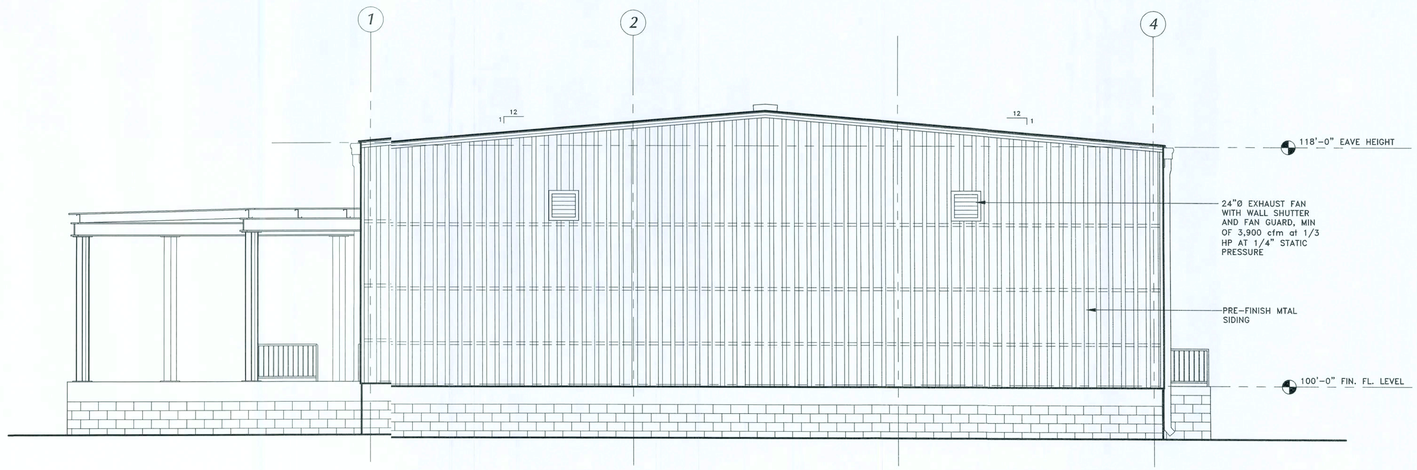


3 RIGHT SIDE ELEVATION
SCALE: 3/16" = 1'-0"



2 LEFT SIDE ELEVATION
SCALE: 3/16" = 1'-0"



1 REAR ELEVATION
SCALE: 3/16" = 1'-0"

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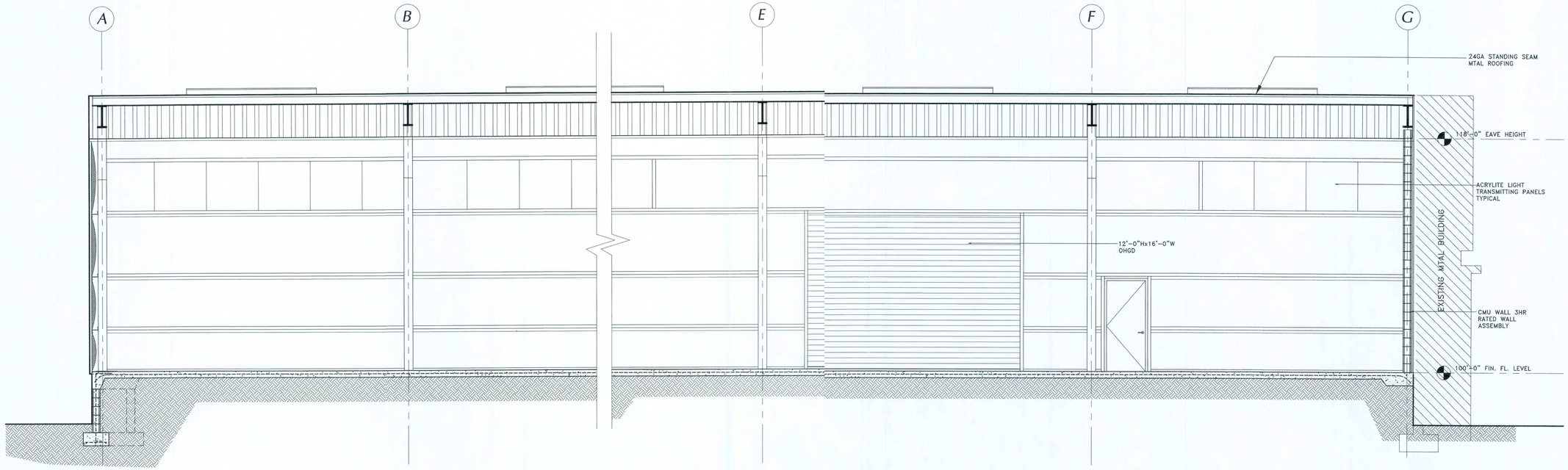
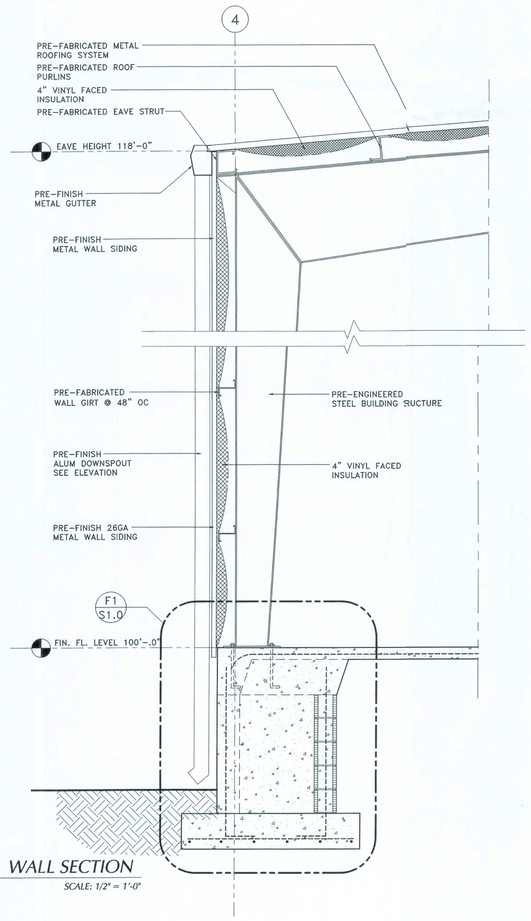
BUILDING EXTERIOR ELEVATION

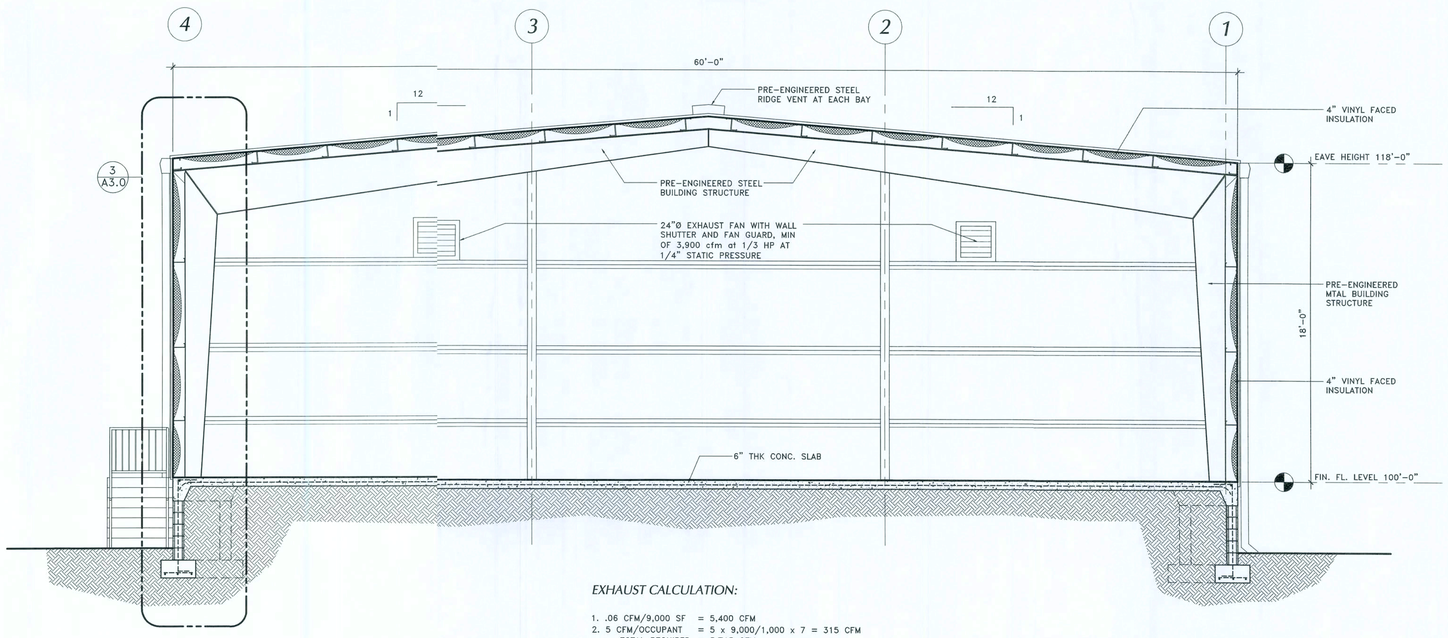
SHEET NUMBER
A2.0



2 *BUILDING SECTION*
SCALE: 1/4" = 1'-0"



3 *WALL SECTION*
SCALE: 1/2" = 1'-0"



EXHAUST CALCULATION:

1. .06 CFM/9,000 SF = 5,400 CFM
2. 5 CFM/OCCUPANT = 5 x 9,000/1,000 x 7 = 315 CFM
   TOTAL REQUIRED = 5,715 CFM

1 *BUILDING SECTION*
SCALE: 1/4" = 1'-0"

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SHEET TITLE:
BUILDING SECTIONS

SHEET NUMBER
A3.0

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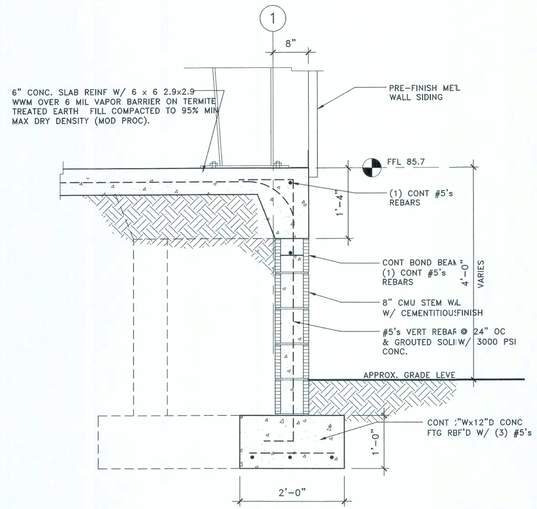
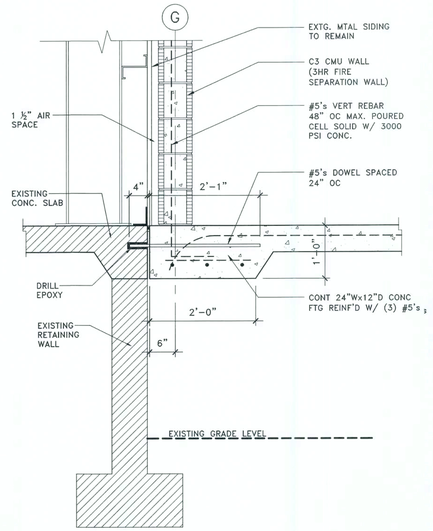


F3 FOUNDATION DETAIL
SCALE: 3/4" = 1'-0"



F4 FOUNDATION DETAIL
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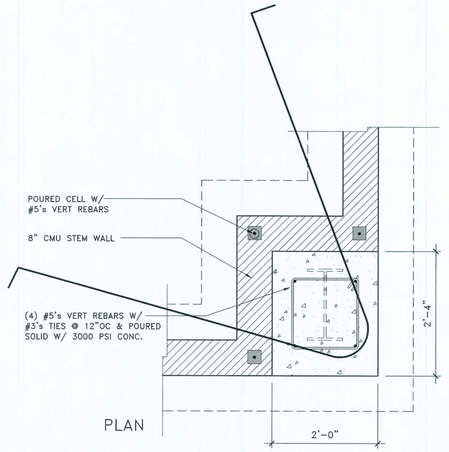
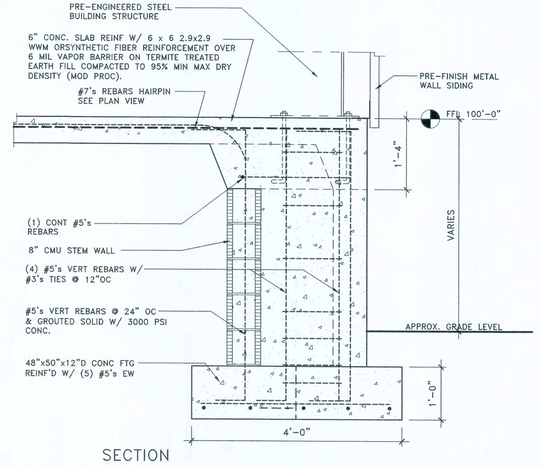
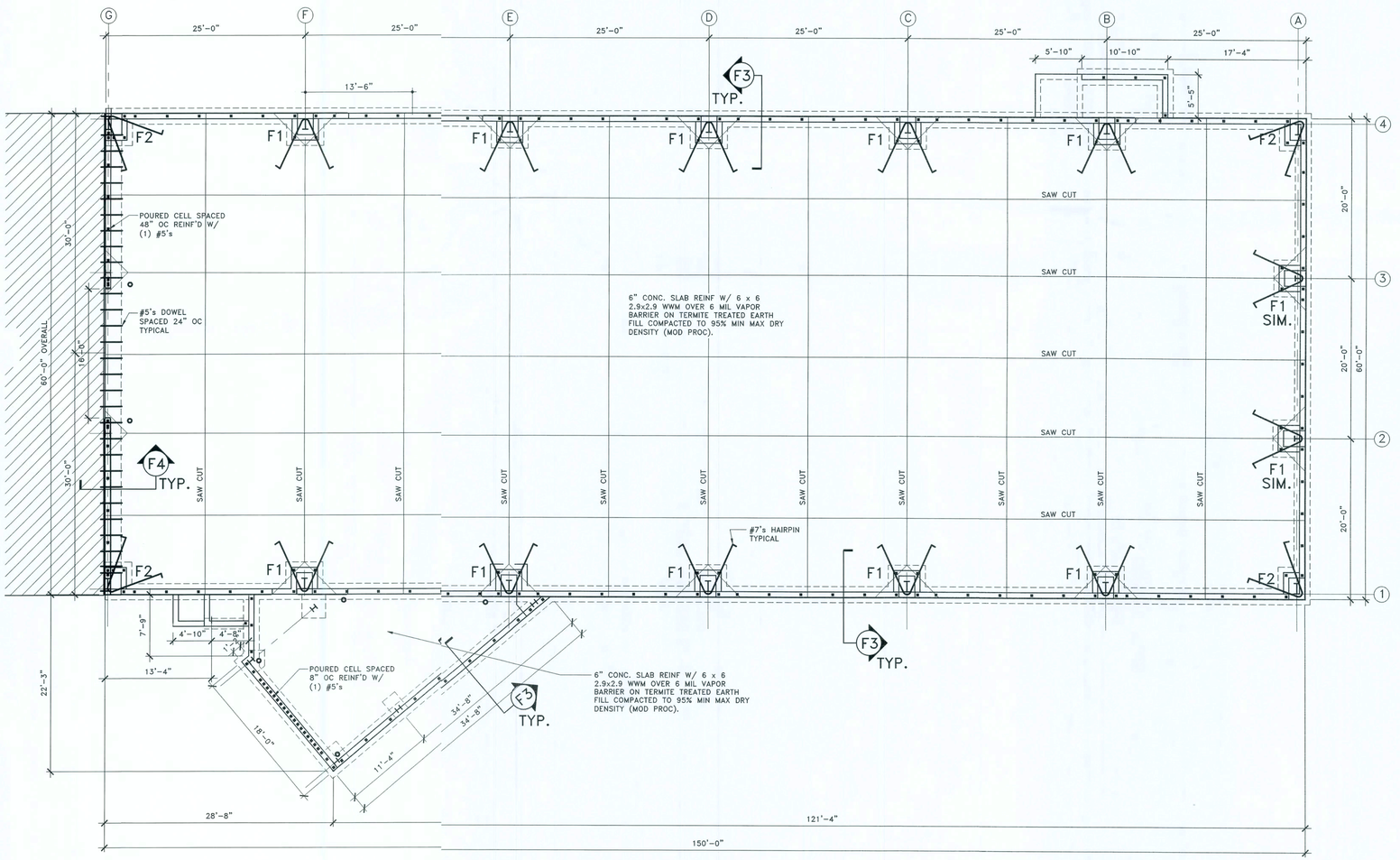
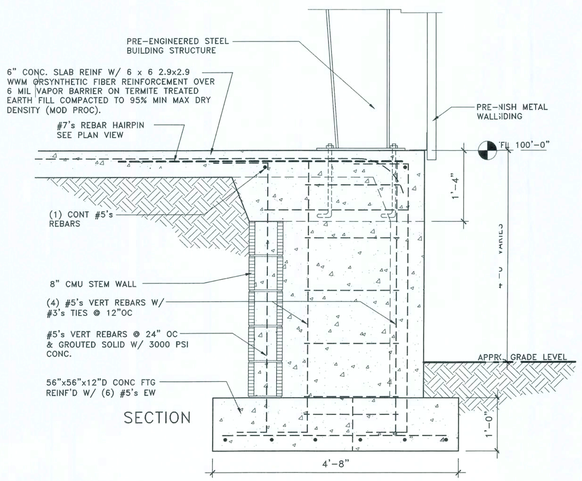




F2 FOUNDATION DETAIL
SCALE: 3/4" = 1'-0"





F1 FOUNDATION DETAIL
SCALE: 3/4" = 1'-0"

AREA CALCULATION

MAIN BUILDING –	9,000 SF
COVERED DOCK –	824 SF
TOTAL AREA –	9,824 SF

1 FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

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DRAWING ISSUE: FOR PERMIT
ISSUE DATE: 19 NOV 2013
REVISED:

SEAL

GENERAL CONTRACTOR

BRYANT CONSTRUCTION CO.

David P. King, Jr
ARCHITECT
7500 SW 61ST AVE
SUITE 400
OCALA, FL. 34476
352.873.3737 (PH)
352.873.0737 (FAX)

REGISTRATIONS:
STATE OF FLORIDA AR 12,999
STATE OF GEORGIA 5044

PROJECT

A NEW WAREHOUSE FOR:

MAYO FERTILIZER

LAKE CITY, FLORIDA

DRAWN BY: ESB
CHECKED BY:
APPROVED BY: DPK
ARCHITECT'S PROJECT No.

SHEET TITLE:
FOUNDATION PLAN

SHEET NUMBER
S1.0

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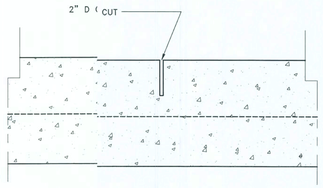
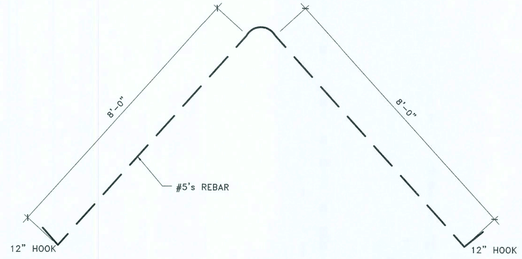
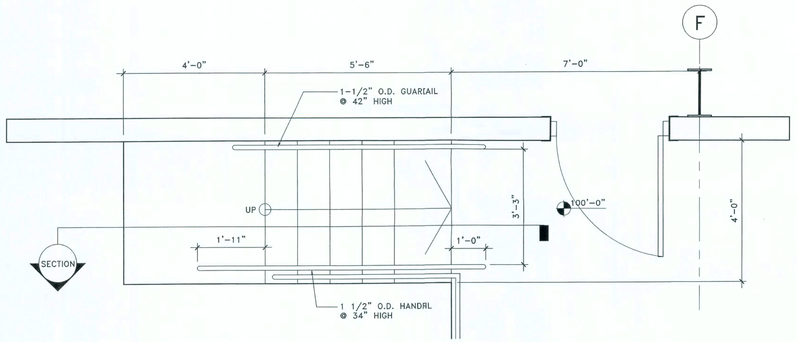
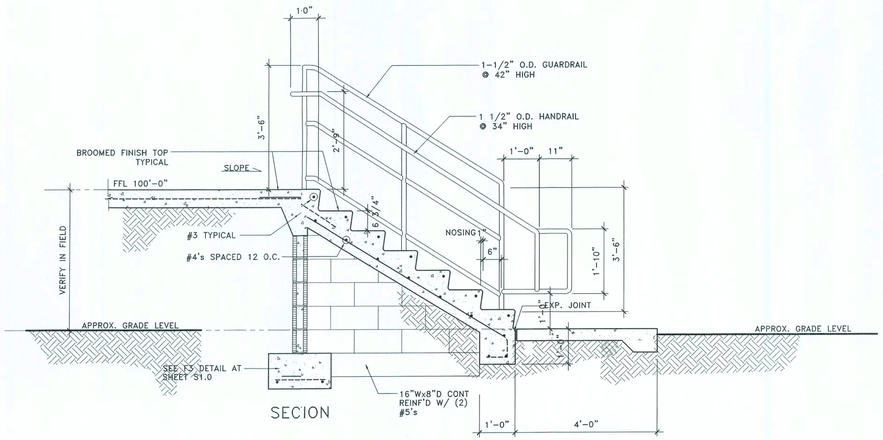


4 SAW CUT DETAIL
SCALE: 3" = 1'-0"

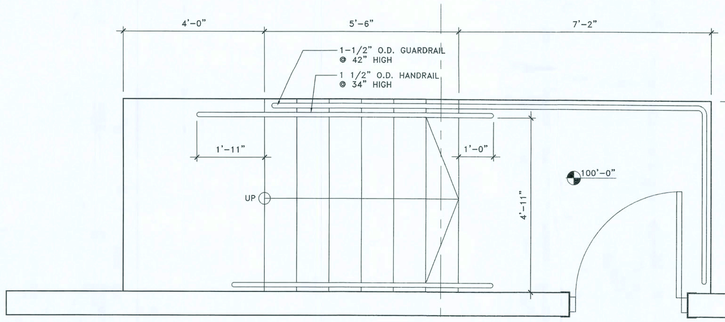


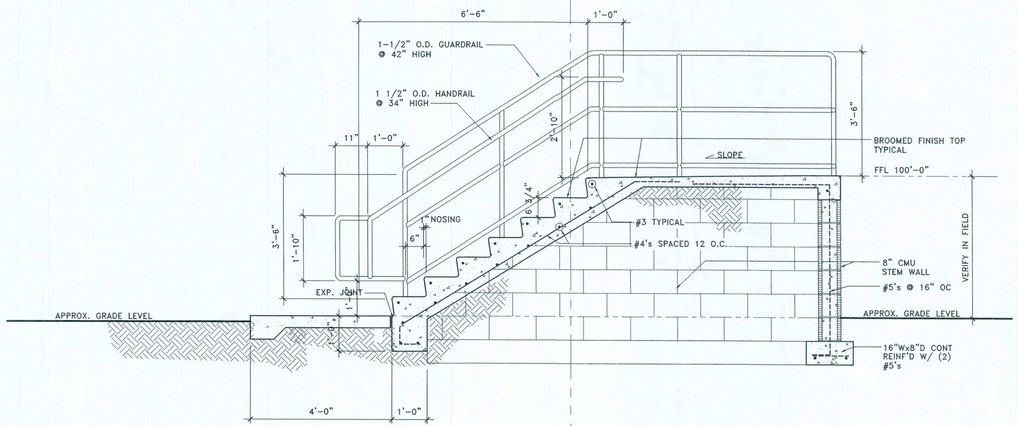
3 HAIR PIN DETAIL
SCALE: 1/2" = 1'-0"





2 STAIRCASE DETAIL
SCALE: 1/8" = 1'-0"





1 STAIRCASE DETAIL
SCALE: 1/8" = 1'-0"

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SEAL

19 NOV 2013

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REGISTRATIONS:
STATE OF FLORIDA AR 12,999
STATE OF GEORGIA 5044

PROJECT

A NEW WAREHOUSE FOR:

MAYO FERTILIZER

LAKE CITY, FLORIDA

DRAWN BY: ESB
CHECKED BY:
APPROVED BY: DPK
ARCHITECT'S PROJECT No:

SHEET TITLE:

FOUNDATION DETAIL

SHEET NUMBER

S1.1

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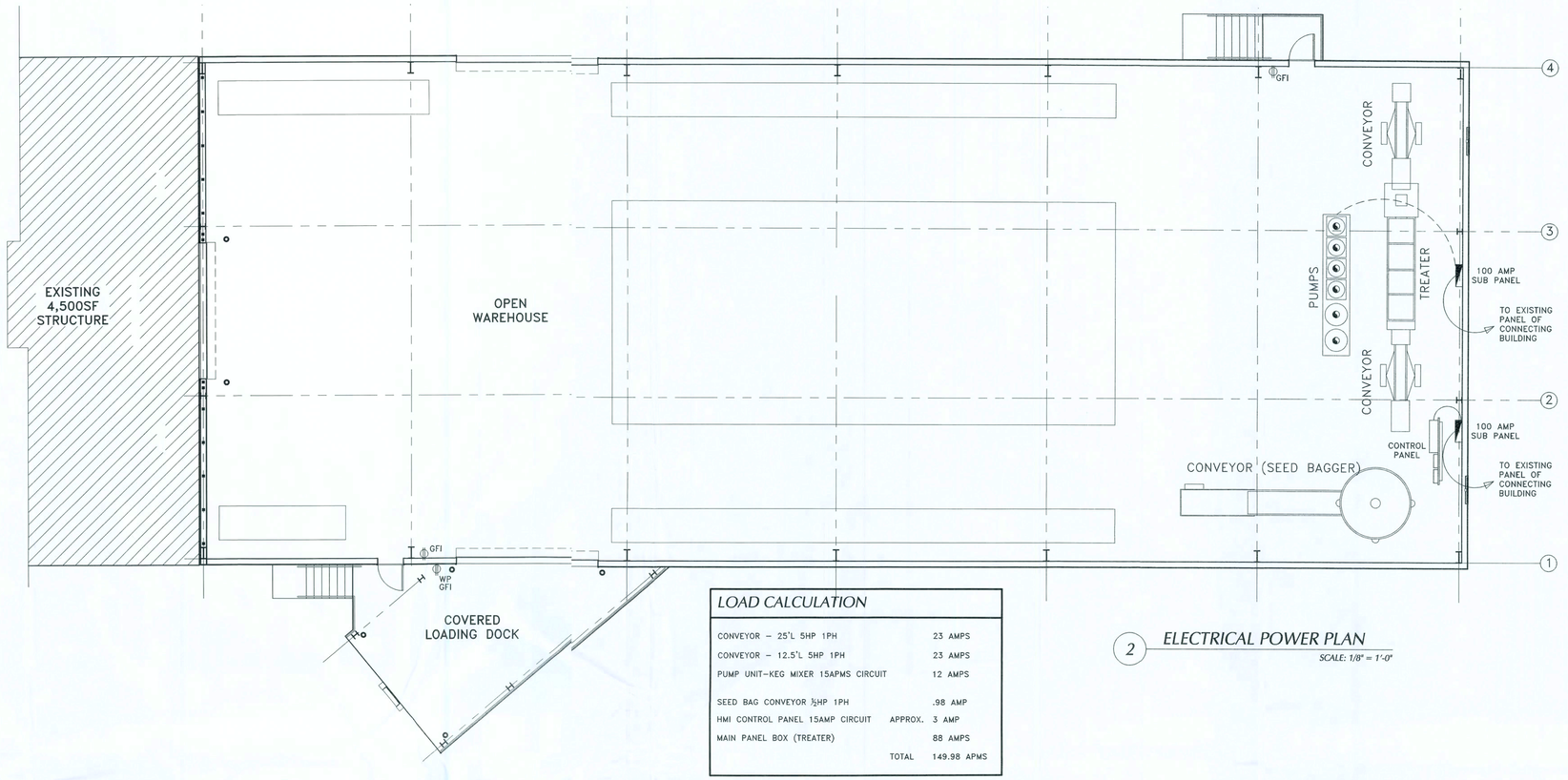
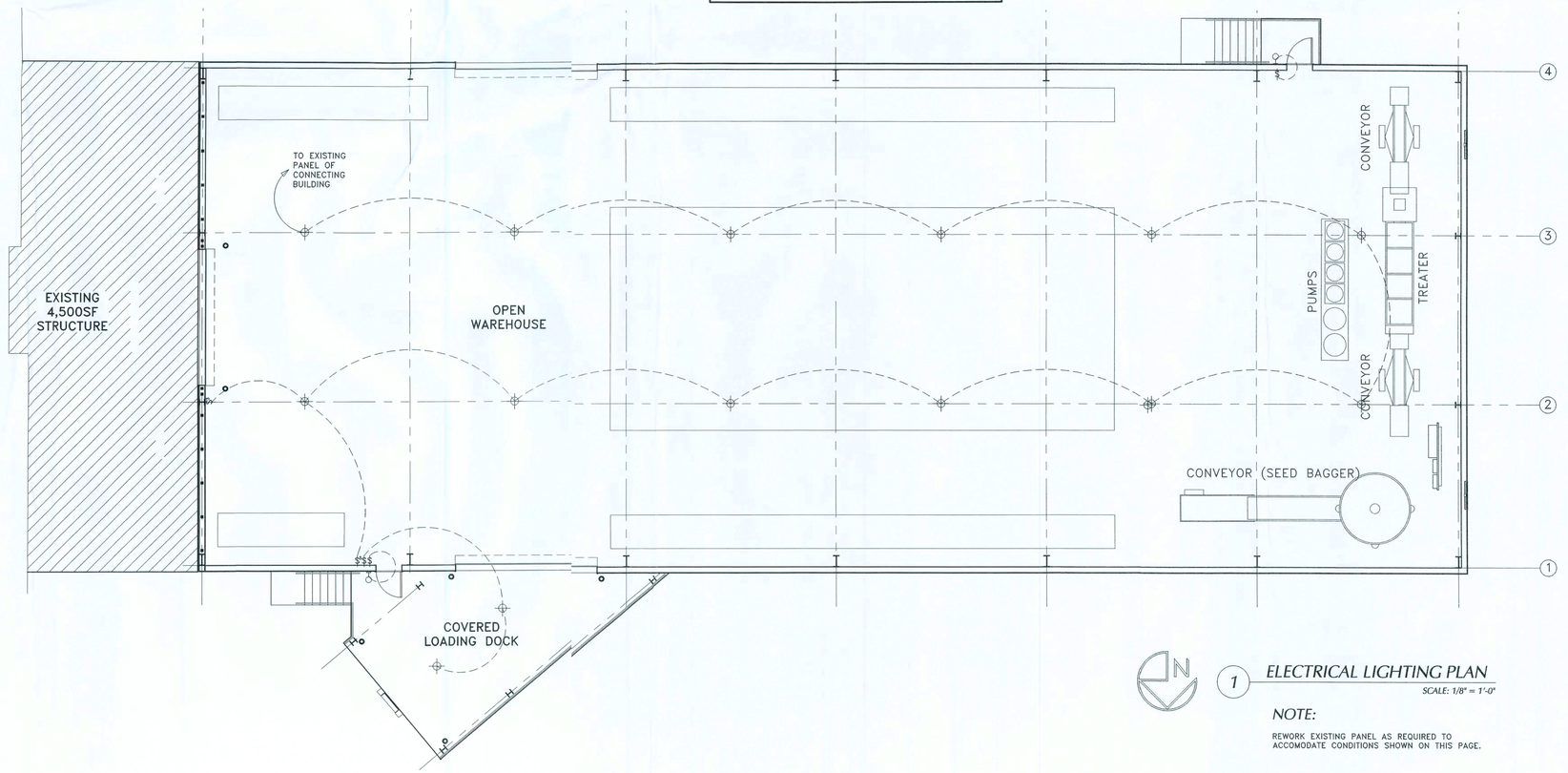


LOAD CALCULATION

CONVEYOR – 25'L 5HP 1PH		23 AMPS
CONVEYOR – 12.5'L 5HP 1PH		23 AMPS
PUMP UNIT–KEG MIXER 15APMS CIRCUIT		12 AMPS
SEED BAG CONVEYOR ½HP 1PH		.98 AMP
HMI CONTROL PANEL 15AMP CIRCUIT	APPROX.	3 AMP
MAIN PANEL BOX (TREATER)		88 AMPS
	TOTAL	149.98 APMS

2 *ELECTRICAL POWER PLAN*
SCALE: 1/8" = 1'-0"



ELECTRICAL SYMBOL KEY

	SURFACE MOUNTED LIGHT		240 OUTLET
	WALL MOUNTED LIGHT	$	SINGLE SWITCH
	PUMP MOTOR	$3	3–WAY SWITCH
	EXTERIOR FLOODLIGHT	$4	4–WAY SWITCH
	110 OUTLET	$RC	REMOTE CONTROL SWITCH
WP	WATER–PROOF OUTLET		ELEC. PANEL
GFI	GROUND FAULT OUTLET		METER

1 *ELECTRICAL LIGHTING PLAN*
SCALE: 1/8" = 1'-0"

NOTE:

REWORK EXISTING PANEL AS REQUIRED TO ACCOMODATE CONDITIONS SHOWN ON THIS PAGE.

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DRAWING ISSUE: FOR PERMIT
ISSUE DATE: 19 NOV 2013
REVISED:

SEAL

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352.873.0737 (FAX)

REGISTRATIONS:
STATE OF FLORIDA AR 12,999
STATE OF GEORGIA 5044

PROJECT

A NEW WAREHOUSE FOR:

MAYO FERTILIZER

LAKE CITY, FLORIDA

DRAWN BY: ESB
CHECKED BY:
APPROVED BY: DPK
ARCHITECT'S PROJECT No.

SHEET TITLE:

ELECTRICAL LIGHTING / POWER PLAN

SHEET NUMBER

E1.0

BUILDING LOAD SUMMARY

BUILDING CODE: FLORIDA BUILDING CODE ED: 2010

JOB LOCATION: LAKE CITY, FL

>>> ROOF LOADS:

Dead Load: 0.73 psf Live Load: 20 psf Tributary Reduction: X Y ___ N

Collateral Load: 0 psf Sprinkler Load: 0 psf

Ground Snow Load: N/A psf Flat roof Snow load: N/A psf

Snow exposure: N/A Snow Importance: N/A Thermal Factor: N/A

>>> WIND LOADS:

Wind Velocity (ULT): 130 mph Fastest Mile: N/A 3-sec Gust: X

Wind velocity (ASD): 101 mph Risk Category: II Wind Exposure: B

Enclosure: ■ Encl □ Partial □ Open Internal pressure coefficient: ± 0.18

Components and Cladding:

Zone1 +16.0 / -30.4 psf Zone 4 +27.8 / -30.1 psf

Zone2 +16.0 / -51.0 psf Zone 5 +27.8 / -37.1 psf

Zone3 +16.0 / -76.7 psf

>>> EARTHQUAKE LOADS:

Base Seismic Force Resisting System: N/A

Design Base Shear: N/A Analysis Used: N/A Seismic Importance: N/A

Risk Category: N/A Site Class: N/A Seismic Design Category: N/A

S(S): N/A g S(1): N/A g C(S): N/A R: N/A

>>> MEZZANINE LOADS:

Dead Load: N/A psf Live Load: N/A psf Reduction: N/A %

>>> OTHER LOADS:

NONE

DOOR PRESSURES (PSF)

QTY		DESCRIPTION	APPROVAL #	DESIGN INTERIOR	DESIGN EDGE	RESISTANCE
2	3070S-W	SOLID WALK DOOR WITH LEVER LOCKSET	FL10294	+17.3 / -18.9	+17.3 / -22.6	+50.0 / -50.0

>>> DEAN STEEL BUILDINGS, INC. ACTS AS A RESELLER OF THE ABOVE LISTED ACCESSORIES
>>> DEAN STEEL BUILDINGS, INC. CERTIFIES THE LOADS APPLIED DO NOT EXCEED THE MANUFACTURER'S STATED RESISTANCE PRESSURE.

SHEETING

	PANEL TYPE	APPROVAL #	MATL.	COLOR
ROOF	RIB-12	FL3774.1	26 GAGE	GL2
WALLS	RIB-12	FL3775.1	26 GAGE	??2
SOFFIT	RIB-12		26 GAGE	??

FRAMING SCREW

MARK	PART #	SPACING
◇6	HW5510	12" O.C.
◇6	HW5515	12" O.C.
◇6	HW5515	12" O.C.

STITCH SCREW

MARK	PART #	SPACING
◇3	HW5502	20" O.C.
◇3	HW5511	30" O.C.
◇3	HW5511	30" O.C.

FLASHNG

	MATL.	COLOR
GUTTER	26 GAGE	??
EAVE FLASHING	26 GAGE	??
RAKE	26 GAGE	??
DOWNSPOUTS	26 GAGE	??
CORNERS	26 GAGE	??
FRAMED OPENINGS	26 GAGE	??
OTHER	26 GAGE	??

ALTHOUGH WE TRY TO MAKE ALL DRAWINGS AS ACCURATE AS POSSIBLE, ANY DRAWING MARKED AS PERMIT OR APPROVAL HAS NOT BEEN FINAL DETAILED OR CHECKED. THE CUSTOMER ACCEPTS ALL RESPONSIBILITY FOR USING PERMIT OR APPROVAL DRAWINGS FOR ANY PURPOSE OTHER THAN OBTAINING A BUILDING PERMIT OR REVIEWING THE DETAILS FOR CONTRACT COMPLIANCE.

IMPORTANT --- PLEASE READ THE FOLLOWING:

>>> General
Dean Steel Buildings, Inc. standard product specifications apply, unless stipulated otherwise on the Dean Steel Buildin' purchase order. Dean Steel Buildings design practices, manufacturing processes, accessories and warranties will govern the work. A Dean Steel Buildings Standard Specificans Brochure is available upon request.

The builder/contractor is responsible for the setting of the achor bolts and the erection of the steel building compants in accordance with Dean Steel Buildings, Inc. "For Construction" drawings.

>>> Unloading Check-out and Shortages:
It is essential, while unloading your Dean Steel Building, to verify that all components listed on the shipping papers are actually shipped. The actual quantities of the items on each truck are circled on the shipping papers. Any items found to be shipped short, or in direct conflict with the shipping pape should be noted on the driver's report. This serves two purposes: 1) You, as the erector, will be aware that a particular item is missing and can temporarily work around it; 2) it lets u know that we have a problem and allow us to react quickly. This system eliminates the discovery that something is missing at the time you need it to erect the building. Taking the me to properly unload the job and check it allows for proper placing of the parts around the job site, which should expedite the erection process. Dean will do its utmost to fill any repted shortages as quickly as possible. Once the erector, or owner, signs that he has received the goods and accepted them as being complete, we can only assume that shortages abr this point are missing due to job site theft. All claims for damage or shortage must be presented, in writing, to the carrier-either Dean Steel Buildings, Inc., or common carrier, withinseven days after receipt of materials by purchaser. Failure to do so voids any claim.

>>> Storage and Protection of Materials:
A Galvanic action known as "white rust" may result when aluminum, galvanized, or the galvanized pre-painted coatin on piled flat sheets or nested formed sheets becomes wet from rain, condensation, or other causes. Under certain weather conditions this "white rust" can happen in as little as 24-48 kurs. Galvalume coated sheet is susceptible, as is galvanized sheet, to wet storage staining. However, due to the composition of the coating, the sheet surface will develop a dark gray disploration as opposed to the white oxide that develops on galvanized. Formed pre-painted sheets must be protected from moisture, in the same manner as plain galvanized or Galvalume sheets, if they are in contact with other sheets. The sheets must be properly packaged and stored. It is important upon receipt of material to examine packages for damage. Builders shuld take prompt action where cuts, tears, or other damage is evident. If moisture is present the panel should be dried at once.
Panels that cannot be stored out of the elements should be re-stacked individually and spacers put between the pals, so that individual panels can have air circulated around them (non-metallic spacers; i.e., wood, cardboard, etc.). Bundled panels should be off the ground sufficiently to prevent ring water from coming in contact with the panels. Bundled panels should also be slanted so that any condensation may be drained off. All bundled panels should be thoroughly covere with a waterproof canvas tarp. Do not use non-breathing materials such as plastic because they prevent air passage and tend to trap moisture in the bundle. Roof and side panels shuld be erected as soon as possible after their arrival at the job site. If prolonged job site storage will be required, the builder is advised to seek storage of the panels out of the elemes.

>>> Proper Erection Practices:

Dean requires that erection of its products be done by experienced pre-engineered metal building assemblers. Erectin and construction methods should be performed as outlined in the "American Institute of Steel Construction Code of Standard Practices for Steel Buildings and Bridges, Part 5." In addon, the erection policies and practices of both the SBA Independent Erectors Division, and MBMA erection practices must be understood and adhered to. The quality of erection has a dict bearing on the quality of the end product. If there are any questions as to these drawings on the Dean Steel Building system, please do not hesitate to contact our Customer srvice Department (239) 334-1051.

Temporaty supports or bracing required for the building erection is the responsibility of the erector to determine, furnh and install.

>>> Back Charge Claim Procedure:

Dean Steel Buildings, Inc., follows the back charge claim procedure adopted by MBMA and as outlined in the MBMA lw Rise Building Systems Manual, Common Industry Practices Section 6.10. The Customer Service Manager must be notified at once when a condition becomes apparent, which may resulin back charge by the builder or erector. Notification by phone must be confirmed in writing. Some approximation of the amount of the back charge must be established at this time, al a written authorization from the Customer Service Manager must be secured before the work is started.

Dean will not honor any field corrections or back charges unless prior notice has been given and agreed upon. All ccrepancies must be agreed upon, in writing, and accompanied with a Dean purchase order number before Dean will honor any back charges. Dean will then pay this agreed amount uponresentation of a final claim. Payment will be by credit memo to the Builder's account.

Any work which is undertaken without such notification and authorization, for which the builder expects to back chan Dean, will not be honored as a back charge.

Should a discrepancy exist, Dean may elect to do one of the following:

1) Ship material from its plant for field correction (freight allowed)
2) Purchase material locally (or allow builder to do so) for field correction
3) Modify existing materials previously shipped to conform to requirements
4) Return material to Dean's plant for exchange or modification requirements

When delivery is contracted by Dean, it is our carrier's intent to arrive on the job site at a pre-designated time an every effort will be made to do so. However, Dean will not accept any back charges due to late arrivals.

Dean Steel Buildings, Inc., shall not, however, be liable to builder in any way or for any reason on account of any ange in Dean Steel Buildings, Inc., product lines. Dean will not pay any back charges for delays that may be incurred due to shortages. Dean will not pay any claims on improper unloding of material, improper storage of material, or delays or damages caused by improper erection techniques. Dean Steel Buildings, Inc., may make changes from time to time in their pruct lines by discontinuing, altering, or modifying any or all of the products included therein and by adding new and additional products thereto.

#31656

COLUMBIA COUNTY BUILDING DEPARTMENT
Received
FILE COPY
Code Compliance
PLANS EXAMINER

WILLIAM ALLEN CAIN
LICENSE
No. 25722
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

ISSUE	REVISION	DATE	BY
		12/3/13	
P	PERMIT	12-30-13	R.M.M.

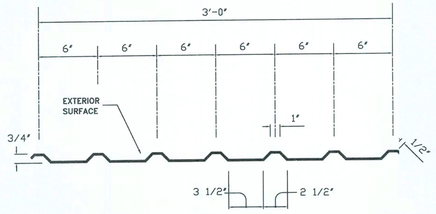
THIS SET OF DRAWINGS IS FOR
■ PERMIT
□ CONSTRUCTION
□ _____

DEAN STEEL BUILDINGS, INC.
2929 INDUSTRIAL AVE.
FORT MYERS, FLORIDA
33901

JOB NUMBER TV01359
DRAWN SIL DATE 12/26/13
CHECKED SIL DATE 12/26/13
SHEET NUMBER 1 OF 5

CUSTOMER : MAYO FERTILIZER, INC
PROJECT NAME : MAYO FERTILIZER, INC WAREHOUSE
DESCRIPTION :
JOB INFORMATION

RIB - 6 PANELS



Fy = 80 K.S.I.

RIB-6 PANEL SECTION PROPERTIES					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0182	0.0331	0.0127	0.0314
24 GA	0.0258	0.0258	0.0486	0.0179	0.0464

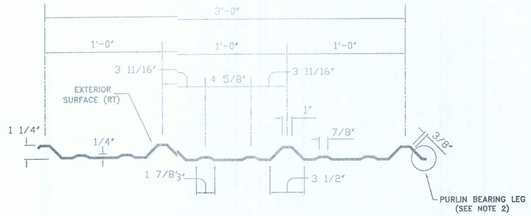
THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTEDBY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMER.

RS A 6 - 27 07 00
- RS: RIB-6, END CUT CODE
- A: A = SQUARE END, B = LEFT SLOPE, C = RIGHT SLOPE
- 6: 4 = 24GA, 6 = 26GA
- 27: LENGTH IN FEET
- 07: LENGTH IN INCHE
- 00: LENGTH I SIXTEENTHS
- END CUT SLOPE CODE

NOTE
1) SECTIONS PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COL-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.

2) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPEFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, G90 1.25 OZ./PER SF. MEETIN ASTM A525 OR GALVALUME,™ AZ55, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.

RIB - 12 PANELS
SHADOW PANELS (Reverse Rib)



PANEL SECTION PROPERTIES 80,000 PSI					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0294	0.0283	0.0371	0.0500
24 GA	0.0258	0.0465	0.0481	0.0520	0.0671

PANEL SECTION PROPERTIES 50,000 PSI					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
24 GA	0.0258	0.0547	0.0589	0.0542	0.0677

THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTED BY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMBER.

RT A 66 - 27 07 00
- RT: RIB-12, END CUT CODE
- A: A = SQUARE END, B = LEFT SLOPE, C = RIGHT SLOPE
- 66: 4 = 24GA, 6 = 26GA
- 27: LENGTH IN FEET
- 07: LENGTH IN INCHES
- 00: LENGTH IN SIXTEENTHS
- END CUT SLOPE CODE

NOTE:
1) SECTIONS PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.

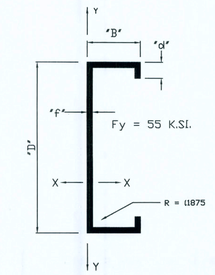
2) PANELS WITH PURLIN BEARING LEGS ARE AVAILABLE IN RIB-12 GALVALUME ONLY.

3) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, G90 1.25 OZ./PER SF. MEETING ASTM A525 OR GALVALUME,™ AZ55, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.

4) SHADOW PANEL IS INSTALLED AS A REVERSE RIB, THEREFORE THE LOAD-IN AND LOAD-OUT PROPERTIES ARE REVERSED.

CEE SECTION PROPERTIES						
'D'	'B'	MATERIAL	'F'	'd'	IX In.^{4}	SX In.^{3}
0'-8"	2 1/2"	14 GA	0.0747	3/4"	9.716	2.429
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.311	3.328
0'-10"	3 1/2"	14 GA	0.0747	3/4"	20.863	4.173
0'-10"	4"	12 GA	0.1046	3/4"	29.949	5.810

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER

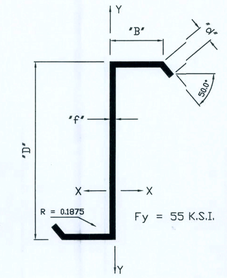


NOTE
1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
2) MINIMUN YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM.)

C H A-A 4 - 11 11 12
- C: CEE
- H: E OR H = 0'-8" DEEP, T = 0'-10" DEEP
- A-A: PUNCH PATTERN
- 4: 4 = 14 GA, 2 = 12 GA
- 11: LENGTH IN FEET
- 11: LENGTH IN INCHES
- 12: LENGTH IN SIXTEENTHS

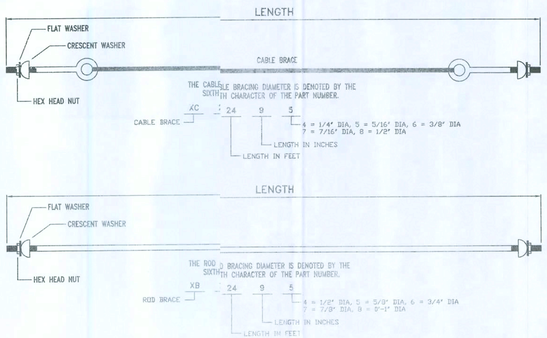
ZEE SECTION PROPERTIES						
'D'	'B'	MATERIAL	'F'	'd'	IX In.^{4}	SX In.^{3}
0'-8"	2 1/2"	16 GA	0.0598	3/4"	8.120	2.030
0'-8"	2 1/2"	15 GA	0.0673	3/4"	9.099	2.275
0'-8"	2 1/2"	14 GA	0.0747	3/4"	10.058	2.514
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.847	3.462
0'-10"	3 1/2"	14 GA	0.0747	3/4"	21.960	4.390
0'-10"	3 1/2"	12 GA	0.1046	3/4"	30.520	6.100

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER



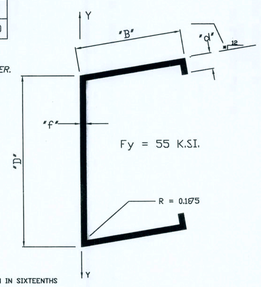
NOTE
1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
2) MINIMUN YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM.)

Z E A-A 4 - 27 07 00
- Z: ZEE
- E: E = 0'-8" DEEP, T = 0'-10" DEEP
- A-A: PUNCH PATTERN
- 4: 6 = 16 GA, 5 = 15 GA, 4 = 14 GA, 2 = 12 GA
- 27: LENGTH IN FEET
- 07: LENGTH IN INCHES
- 00: LENGTH IN SIXTEENTHS



EAVE STRUT SECTION PROPERTIES (POSITIVE BENDING)								
'D'	'B'	MATERIAL	'F'	'd'	IX In.^{4}	SX In.^{3}	IY In.^{4}	SY In.^{3}
0'-8"	0'-5"	14 GA	0.0747	3/4"	15.581	3.895	6.624	1.351
0'-8"	0'-5"	12 GA	0.1046	3/4"	21.335	4.979	9.341	1.970

THE SLOPE AND MATERIAL THICKNESS OF ALL EAVE STRUTS ARE DENOTED BY THE THIRD, FIFTH, AND SIXTH CHARACTER OF THE PART NUMBER.



NOTE
1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
2) MINIMUN YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM.)

ES N-N 4 C - 24 11 12
- ES: EAVE STRUT
- N-N: N = LOWSIDE SLOPE, R = HIGHSIDE SLOPE
- 4: 4 = 14 GA, 2 = 12 GA
- C: SLOPE CODE
- 24: LENGTH IN FEET
- 11: LENGTH IN INCHES
- 12: LENGTH IN SIXTEENTHS

SLOPE CODE SCHEDULE		
A = 0.0:12	H = 3.5:12	Q = 7.0 :12
B = 0.5:12	J = 4.0:12	R = 7.5 :12
C = 1.0:12	K = 4.5:12	S = 8.0 :12
D = 1.5:12	L = 5.0:12	T = 8.5 :12
E = 2.0:12	M = 5.5:12	U = 9.0 :12
F = 2.5:12	N = 6.0:12	V = 9.5 :12
G = 3.0:12	P = 6.5:12	W = 10.0:12

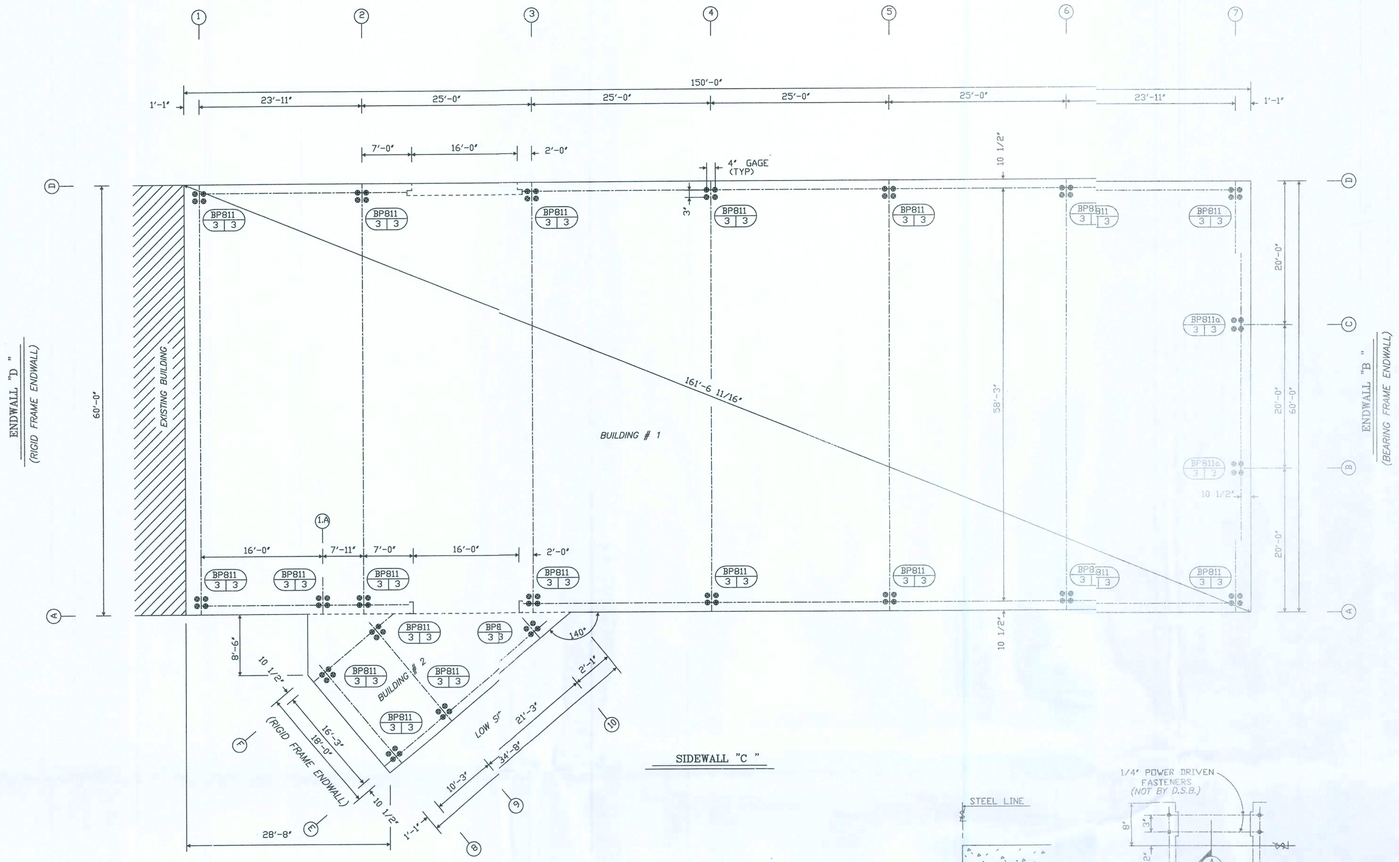
WILLIAM ALLEN CRAIG LICENSE No. 25722 STATE OF FLORIDA
12/31/13

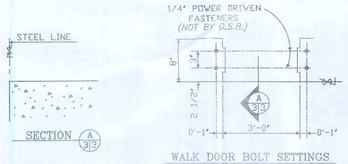
P	PERMIT	12-30-13	R.M.M.
ISSUE	REVISION	DATE	BY

DEAN STEEL BUILDINGS, INC.
2929 INDUSTRIAL AVE.
FORT MYERS, FLORIDA
33901

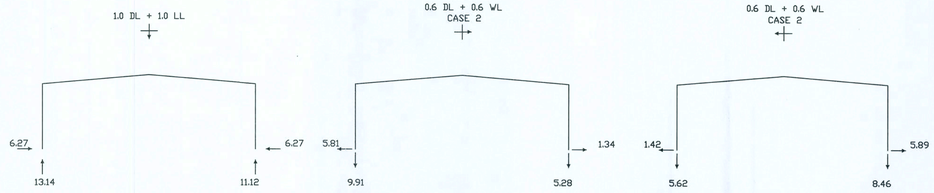
JOB NUMBER TV01359
DRAWN SIL DATE 12/26/13
CHECKED SIL DATE 12/26/13
SHEET NUMBER 2 OF 5

CUSTOMER : MAYO FERTILIZER, INC
PROJECT NAME : MAYO FERTILIZER, INC WAREHOUSE
DESCRIPTION :
MATERIAL PROPERTIES





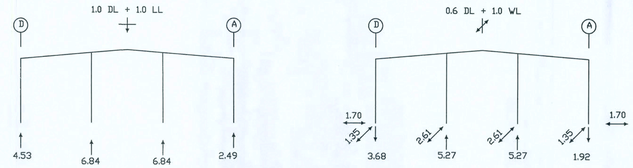
WALK DOOR BOLT SETTINGS
(2 REQUIRED, FIELD LOCATED)



FRAME REACTIONS AT FRAME LINES (1) THRU (6)

(ALL REACTIONS IN KIPS)

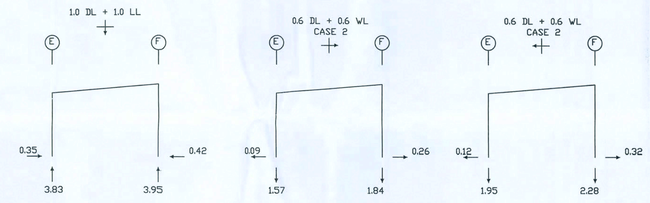
BUILDING # 1



COLUMN REACTIONS AT FRAME LINE (7)

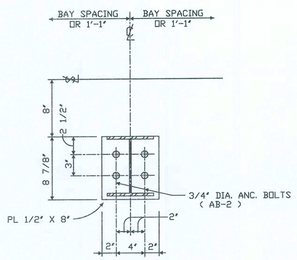
(ALL REACTIONS IN KIPS)

BUILDING # 1

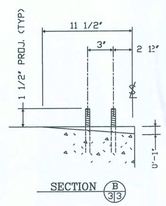


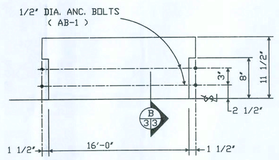
FRAME REACTIONS AT FRAME LINES (8) AND (9)

(ALL REACTIONS IN KIPS)



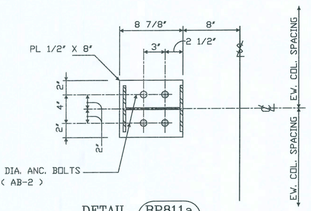
DETAIL BP811 (3 | 3)



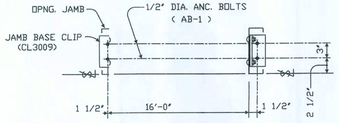


ANCHOR BOLT SETTING FOR O.H.D.

1) 16'-0" X 12'-0" FACTORY LOCATED



DETAIL BP811a (3 | 3)

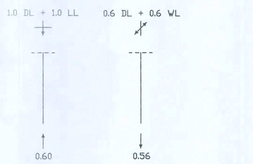


ANCHOR BOLT SETTING FOR O.H.D.

(1) 16'-0" X 12'-0" FACTORY LOCATED

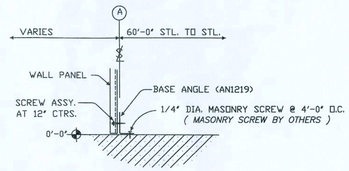
SOLDIER COLUMN REACTIONS AT LINE (1A)

(ALL REACTIONS IN KIPS)



SOLDIER COLUMN REACTIONS AT LINE (10)

(ALL REACTIONS IN KIPS)



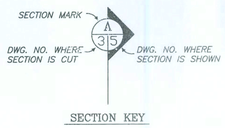
BASE DETAIL AT COMMON WALL

ANCHOR BOLTS BY OTHERS

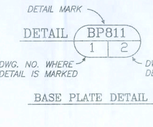
QTY.	PART #	BOLT SIZE	'A'	'B'	'C'	'D'	PROJ.
8	AB-1	1/2" X 0'-6"	1/2"	2"	0'-6"	1 3/4"	1 1/2"
88	AB-2	3/4" X 1'-3"	3/4"	3"	1'-3"	3"	2"
	AB-3	7/8" X 1'-3"	7/8"	4"	1'-3"	3 1/2"	2"
	AB-4	1" X 1'-6"	1"	4"	1'-6"	4"	3"
	AB-5	1" X 2'-2"	1"	4"	2'-2"	4"	3"
	AB-6	1 1/4" X 2'-0"	1 1/4"	4"	2'-0"	3 3/4"	3"

DEAN STEEL BUILDGS, INC. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MEGIALS, AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMJMENTS. ADEQUATE PROVISIONS SHALL BE MADE IN THE FOUNDATIODESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING @ ANY OTHER IMPOSED LOADS, ACCOUNTING FOR THE BEARING CAPACITY F THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

THE DISTANCE OF 'E ANCHOR BOLTS FROM THE EDGE OF THE SLAB IS BASED ON DSB STAIARDS. THE ACTUAL SLAB SIZE MAY BE MODIFIED TO ACCOMODATE ACTUA SITE CONDITIONS AND FOUNDATION DESIGN.

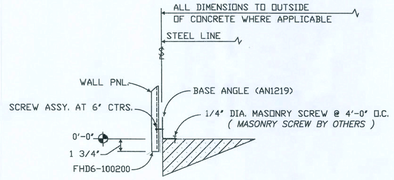


SECTION KEY



BASE PLATE DETAIL KEY

NOTE !
FINISHED FLOOR ELEVATION = 0'-0"
ALL BASE PLATES ARE AT ELEV. 0'-0"
UNLESS OTHERWISE NOTED.



BASE DETAIL

WILLIAM ALLEN CLARK LICENSE No. 25722 STATE OF FLORIDA PROFESSIONAL ENGINEER

ISSUE	REVISION	DATE	BY
		12/31/13	
0	FOR CONST.	12.24.2013	JVJ

DEAN STEEL BUILDINGS, INC.
2929 INDUSTRIAL AVE.
FORT MYERS, FLORIDA
33901

JOB NUMBER TV01359
DRAWN J.W.J. DATE 12. 24. 13
CHECKED J.W.J. DATE 12. 24. 13
SHEET NUMBER 2 OF

CUSTOMER : MAYO FERTILIZER, INC
PROJECT NAME : MANYO FERTILIZER, INC WAREHOUSE
DESCRIPTION :

ANCHOR BOLT PLAN
BASE PLATE DETAILS AND
FRAME REACTIONS

FIELD LOCATED ACCESSORY SCHEDULE

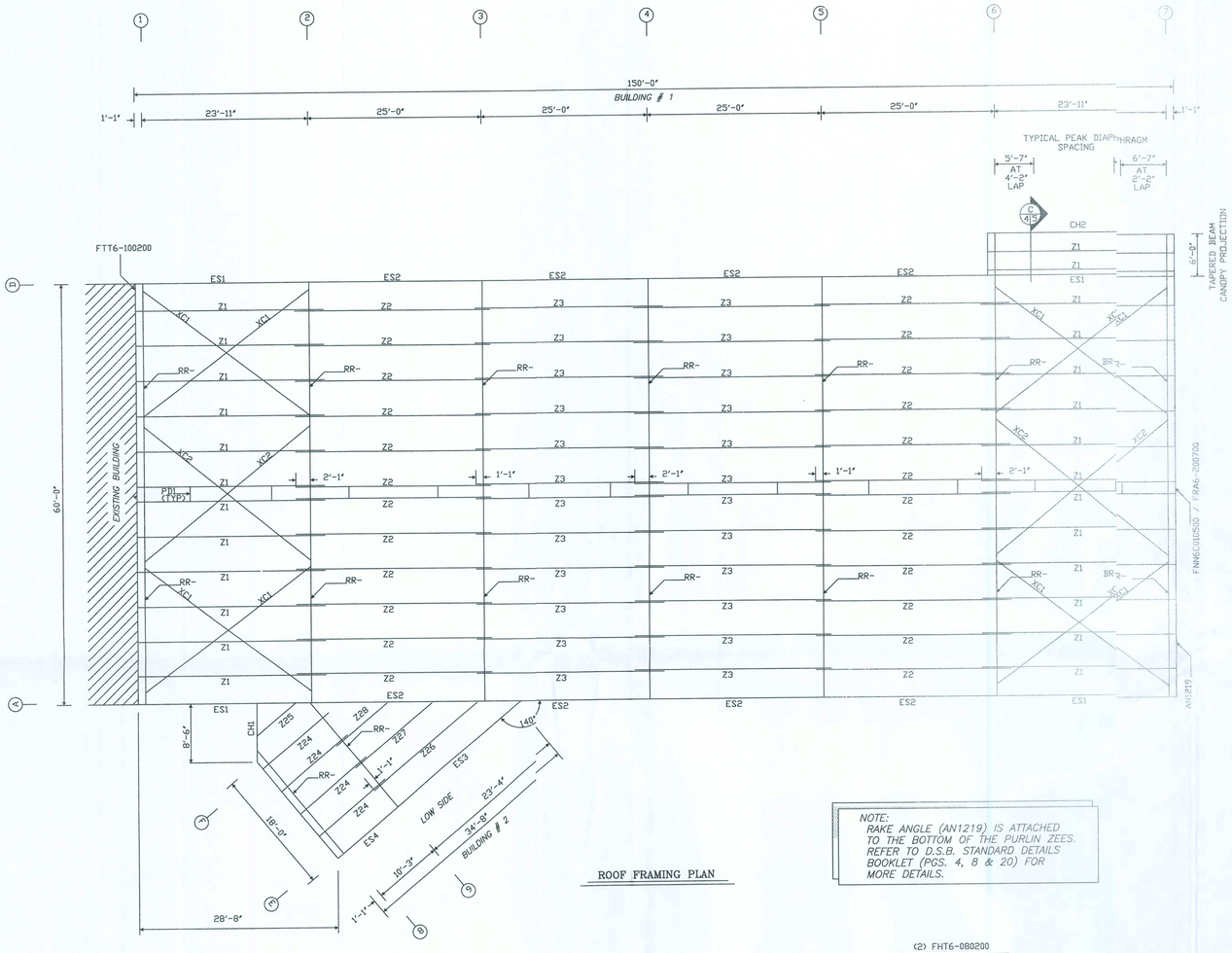
QTY.	MK.	DESCRIPTION
2	A	3070 CECO FOAM INSULATED DOOR LEAF W/FRAME & LEVER LOCKSET (CYLINDRICAL OPERATOR) (+50.00 / -50.00 PSF)
51	SK5789	3'-0" X 11'-1" TRANSLUCENT FIBER GLASS PNL. (S12) (S13)

NOTE !
REFER TO D.S.B. STD DETAIL BOOKLET (PG. 36) FR WALK DOOR INSTALLION DETAILS.

NOTE!
REFER TO DSB STD DETAIL BOOKLET (PG.39) FOR WALL LIGHT DETAILS

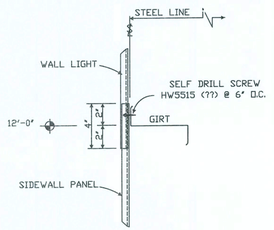
PEAK DIAPHRAGM REFERENCE TABLE		CABLE BRACING REFERENCE TABLE		FRAMED OPENING REFERENCE TABLE			EAVE STRUT REFERENCE TABLE	
REF NUMB	NPD	REF NUMB	NPD	REF NUMB	OPENING SIZE	NPD	REF NUMB	NPD
PD1	ANP4C020204	XC1	XC3094	FD1	16'-0" X 12'-0" (FACTORY LOCATED)	CHAHA2-17010 CHA-A2-15111:	ES1	ESN-N4C241114
		XC2	XC3164				ES2	ESN-N4C241112
							ES3	ESN-N4C230312

NOTES:
1. USE GROMMET BOLTS HW5526 @ 12" O.C. FOR WALL LIGHT TO WALL LIGHT CONNECTION.
2. USE GROMMET BOLTS HW5526 @ 12" O.C. TO ATTACH EAVE FLASHING (FEP6-100200) TO WALL LIGHT PANELS.
3. BUILDER TO FIELD CUT WALL LIGHTS (SK5789) PANELS (S5) TO REQUIPED LENGHT.

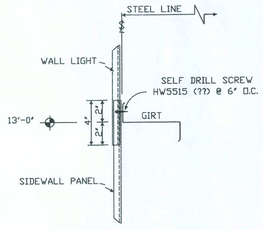


ROOF FRAMING PLAN

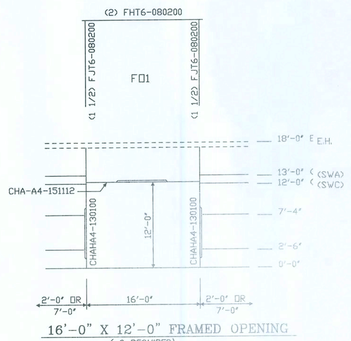
NOTE:
RAKE ANGLE (AN1219) IS ATTACHED TO THE BOTTOM OF THE PURLIN ZEES. REFER TO D.S.B. STANDARD DETAILS BOOKLET (PGS. 4, 8 & 20) FOR MORE DETAILS.



SECTION D 4/4 @ SW "C"

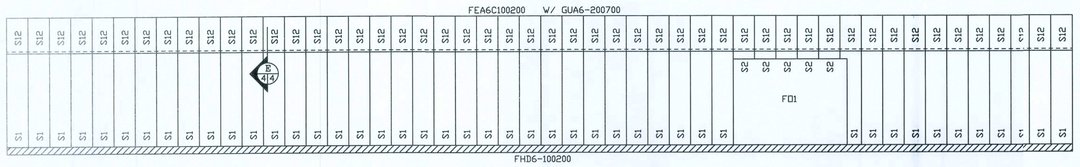


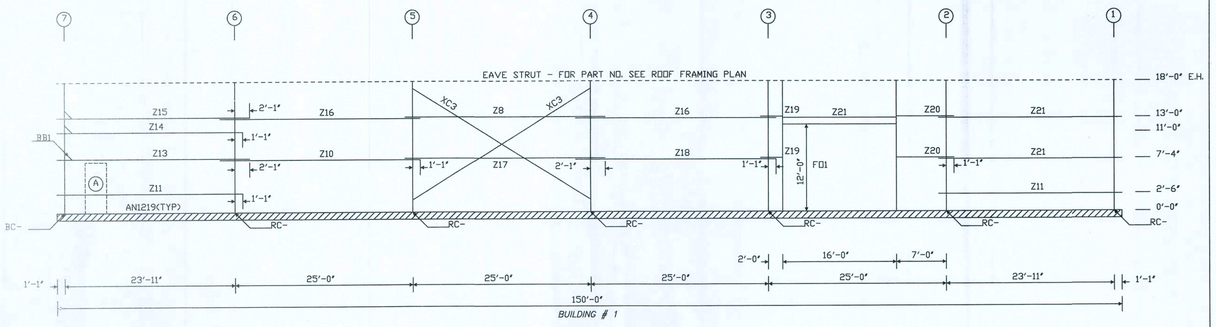
SETION F 4/4 @ SW "A"



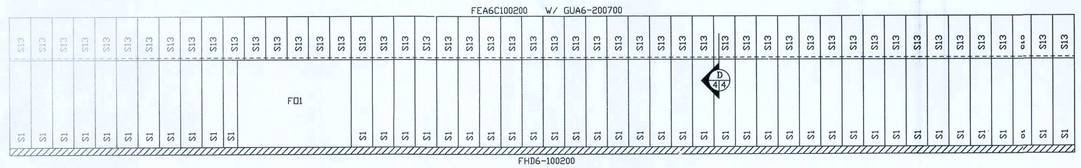
16'-0" X 12'-0" FRAMED OPENING
(2 REQUIRED)

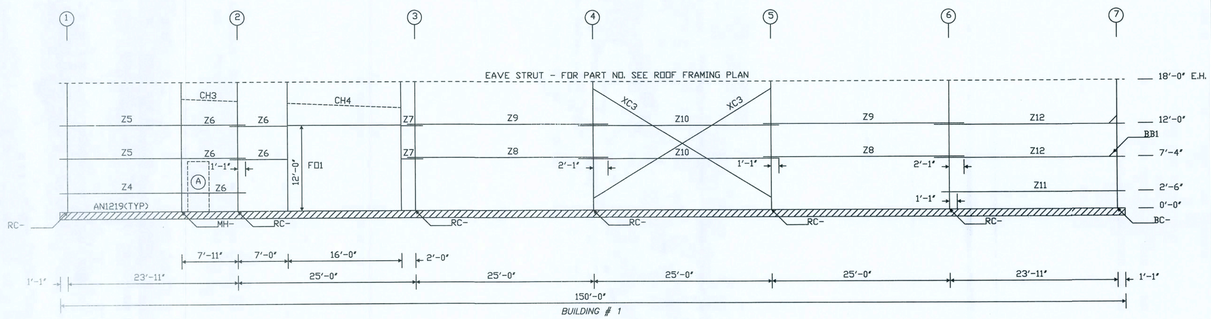
NOTE !
REFER TO D.S.B. STD. DETAIL BOOKLET (PGS. 37 & 38) FOR FRMD. OPNG. & TRIM DETAILS.





SIDEWALL "A"





SIDEWALL "C"

NOTE 1
DIMENSIONS ON THIS SHEET ARE STEEL DIMENSIONS AND DO NOT INCLUDE SHEET LEDGES, ETC. FOR CONCRETE DIMENSIONS SEE THE ANCHOR BOLT PLAN.

NOTE 2
SHEETING MUST BE ATTACHED TO ALL SECONDARY FRAMING PER DEAN STANDARD DETAILS. THIS INCLUDES ALL DOUBLE PURLINS AND GIRTS REGARDLESS OF LOCATION.

NOTE 3
THE TYPE AND MATL. THICKNESS OF ALL FLASHING IS DENOTED BY THE 1ST, 2ND, 3RD, AND 4TH CHARACTERS OF PART NUMBER. (EXAMPLE: FJT6-140100)
FJT = JAMB FLASHING FOR RIB-12, FRA = RAKE FLASHING (STD.), 6 = 26 GA., & 4 = 24 GA.
GUA = SCULPTURED GUTTER, FWR = WALL TO ROOF FLASHING
OCT = OUTSIDE CORNER FLASHING FOR RIB-12, ETC.

WILLIAM ALLEN CLAYTON LICENSE No. 25722 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/31/13

ISSUE	REVISION	DATE	BY
P	PERMIT	12-30-13	R.M.M.

DEAN STEEL BUILDINGS, INC.
2929 INDUSTRIAL AVE.
FORT MYERS, FLORIDA
33901

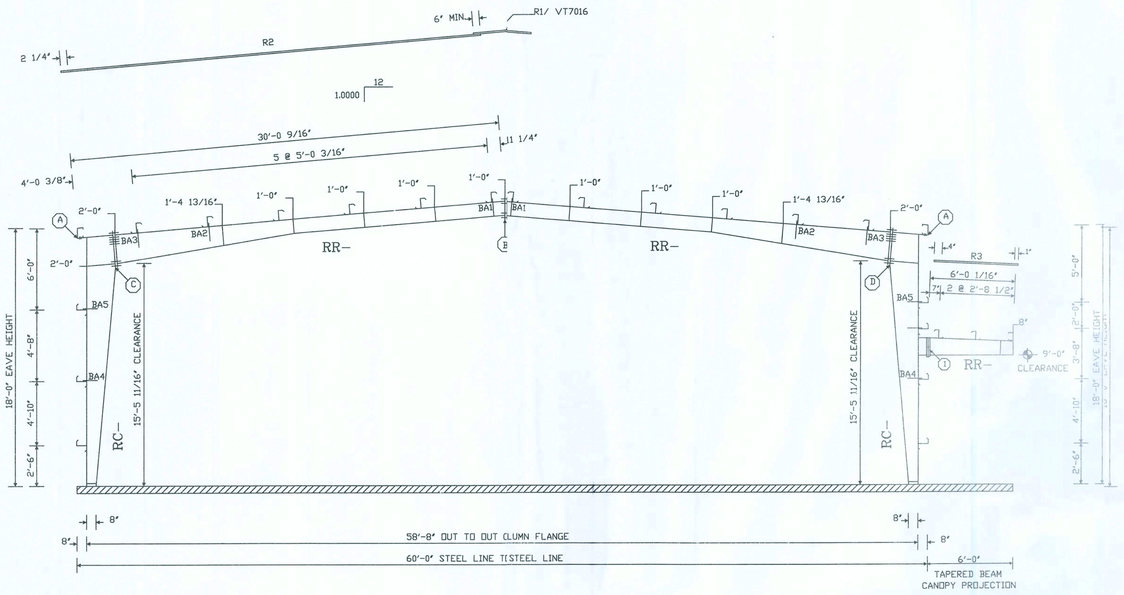
JOB NUMBER TV01359
DRAWN SIL 12/26/13
CHECKED SIL 12/26/13
SHEET NUMBER 4 OF 5

CUSTOMER : MAYO FERTILIZER, INC
PROJECT NAME : MAYO FERTILIZER, INC WAREHOUSE
DESCRIPTION :
ROOF FRAMING PLAN
SIDEWALL ELEVATIONS

FIELD LOCATED ACCESSORY SCHEDULE

VENTS			
8	VT7016	10'-0 LNG. X 0'-9" THROAT W/ BIRD SCREEN	GL.

NOTE !
REFER TO D.S.B. S\ CONST. DETAIL BOOKLET (P\ 41) FOR RIB 12 MONOVENT :TAILS.



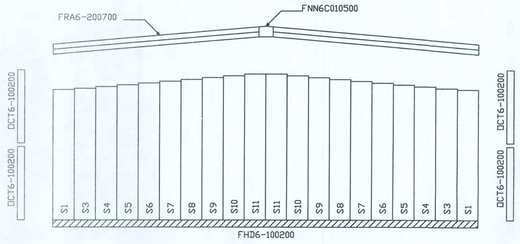
FRAME CROSS SECTION AT FRAME LINES (1) THRU (6)

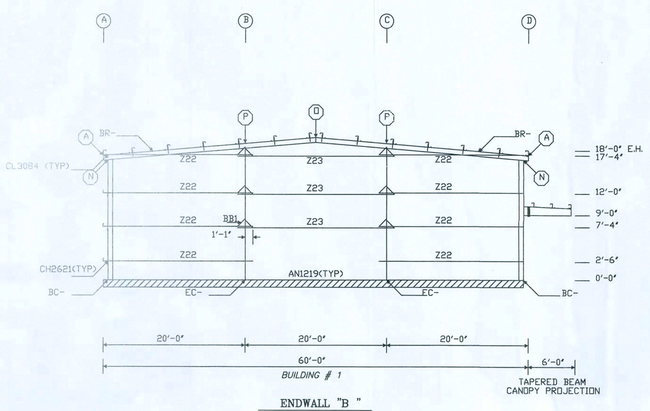
ROOF SHEETING REFERENCE TABLE	
REF NUMB	NPD
R1	RTZ6C020400
R2	RTA6-290800
R3	RTA6-051000
R4	RTA6-240608

FLANGE BRACE REFERENCE TABLE	
REF NUMB	NPD
BA1	ANF4-010911
BA2	ANF4-020210
BA3	ANF4-030107
BA4	ANF4-020013
BA5	ANF4-030004
BB1	ANF4-010603

WALL SHEETING REFERENCE TABLE	
REF NUMB	NPD
S1	RTA6-180100
S2	RTA6-051100
S3	RTA6-180300
S4	RTA6-180600
S5	RTA6-180900
S6	RTA6-190000
S7	RTA6-190300
S8	RTA6-190600
S9	RTA6-190900
S10	RTA6-200000
S11	RTA6-200300
S12	SK5789 (WALL LIGHTS
S13	SK5789 (WALL LIGHTS

ZEE REFERENCE TABLE	
REF NUMB	NPD
Z1	ZEA-A5-270100
Z2	ZEA-A5-280200
Z3	ZEA-A6-280200
Z4	ZEJ-J6-260812
Z5	ZEJ-J2-260812
Z6	ZEG-J6-070912
Z7	ZEX7G6-020912
Z8	ZEA-J5-280200
Z9	ZEA-J6-280200
Z10	ZEJ-A5-280200
Z11	ZEJ-J6-260100
Z12	ZEJ-A6-270100
Z13	ZEA-J5-270100
Z14	ZEA-J6-260100
Z15	ZEA-J6-270100
Z16	ZEJ-A6-280200
Z17	ZEA-J4-280200
Z18	ZEJ-A4-280200
Z19	ZEG7X6-020912
Z20	ZEJ-G6-070912
Z21	ZEJ-J2-260100
Z22	ZEJ-J6-200412
Z23	ZEJ-J6-220200

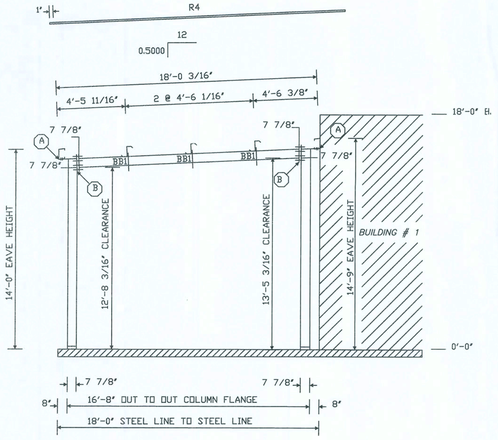




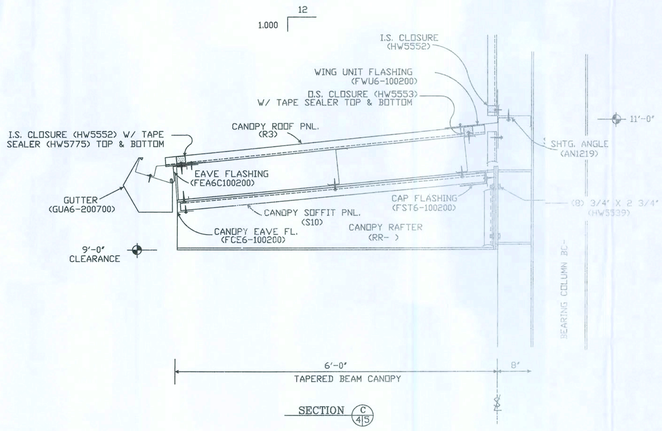
ENDWALL "B "

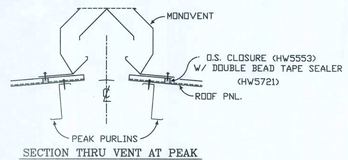
BOLT SCHEDULE		
CONN.	QTY.	SIZE
A	4	1/2" X 1 1/4" HW5532
B	8	3/4" X 2" HW5537
C	14	3/4" X 2" HW5537
D	14	3/4" X 2" HW5537
N	4	1/2" X 1 1/2" HW5604
O	4	3/4" X 2 1/4" HW5602
P	2	3/4" X 1 3/4" HW5536

NOTE: ALL BOLTS TO BE HIGH STRENGTH WITH NUT AND WASHER. CONNECTION "A" REQUIRES WASHER ON EAVE STRUT SIDE ONLY.

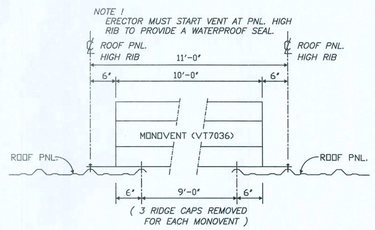


FRAME CROSS SECTION AT FRAME LINES (8) AND (9)





SECTION THRU VENT AT PEAK



MONOVENT DETAIL

NOTE:
1. THE HIGH STRENGTH BOLTS & WASHERS SPECIFIED FOR USE AT THE COLUMN & RAFTER CONNECTIONS CONFORM TO A.S.T.M. SPECIFICATION A-325. THESE BOLTS MUST BE TIGHTENED BY THE "TURN OF THE NUT" METHOD AS DESCRIBED IN SECTION 5(C) OF THE A.I.S.C. SPECIFICATION STRUCTURAL JOINTS USING A.S.T.M. A-325 OR A-490 BOLTS
2. ALL BOLTS TO BE HIGH STRENGTH WITH NUTS AND WASHERS. EXCEPT CONNECTION "A" REQUIRES A WASHER ON THE EAVE STRUT SIDE ONLY.
3. ALL DEPTHS SHOWN ARE WEB DEPTHS ONLY AND DO NOT INCLUDE FLANGE THICKNESSES.
4. FLANGE BRACES ARE 1 3/4" X 1 3/4" X 14 GA. ANGLE ALL FLG. BRACES ARE LOCATED ON BOTH SIDES OF THE FRAME UNLESS NOTED OTHERWISE. *PEAK PURLIN IS FLANGE BRACED ON ONE SIDE ONLY. FRAMES AT BLDG. ENDS REQUIRE FLG. BRACE ON ONE SIDE ONLY*
5. RIGID FRAME RAFTER PART NUMBERS ARE LOCATED ON THE TOP FLANGE OF THE UPHILL END OF THE RAFTER.
6. RIGID FRAME COLUMN PART NUMBERS ARE LOCATED ON THE OUTER FLANGE AT THE TOP OF THE COLUMN.
7. DIMENSIONS SHOWN ON CROSS-SECTION ARE MEASURED AT THE COLUMN BASES ONLY. THE WIDTH MEASURED AT THE EAVE CAN BE WIDER DEPENDING ON COLUMN OUTER FLANGE THICKNESS CHANGES.
8. RIGID FRAME IS SYMMETRICAL ABOUT CTR. LINE

WILLIAM ALLEN CLARK LICENSE No. 25722 STATE OF FLORIDA PROFESSIONAL ENGINEER

		12/31/13	
P	PERMIT	12-30-13	R.M.M.
ISSUE	REVISION	DATE	BY

DEAN STEEL BUILDINGS, INC.
2929 INDUSTRIAL AVE.
FORT MYERS, FLORIDA
33901

JOB NUMBER	TV01359
DRAWN	SIL 12/26/13
CHECKED	SIL 12/26/13
SHEET NUMBER	5 OF 5

CUSTOMER : MAYC FERTILIZER, INC
PROJECT NAME : MAYO 'ERTILIZER, INC WAREHOUSE
DESCRIPTION :
FRAME CROSS SECTION
ENDWALL ELEVATIONS