

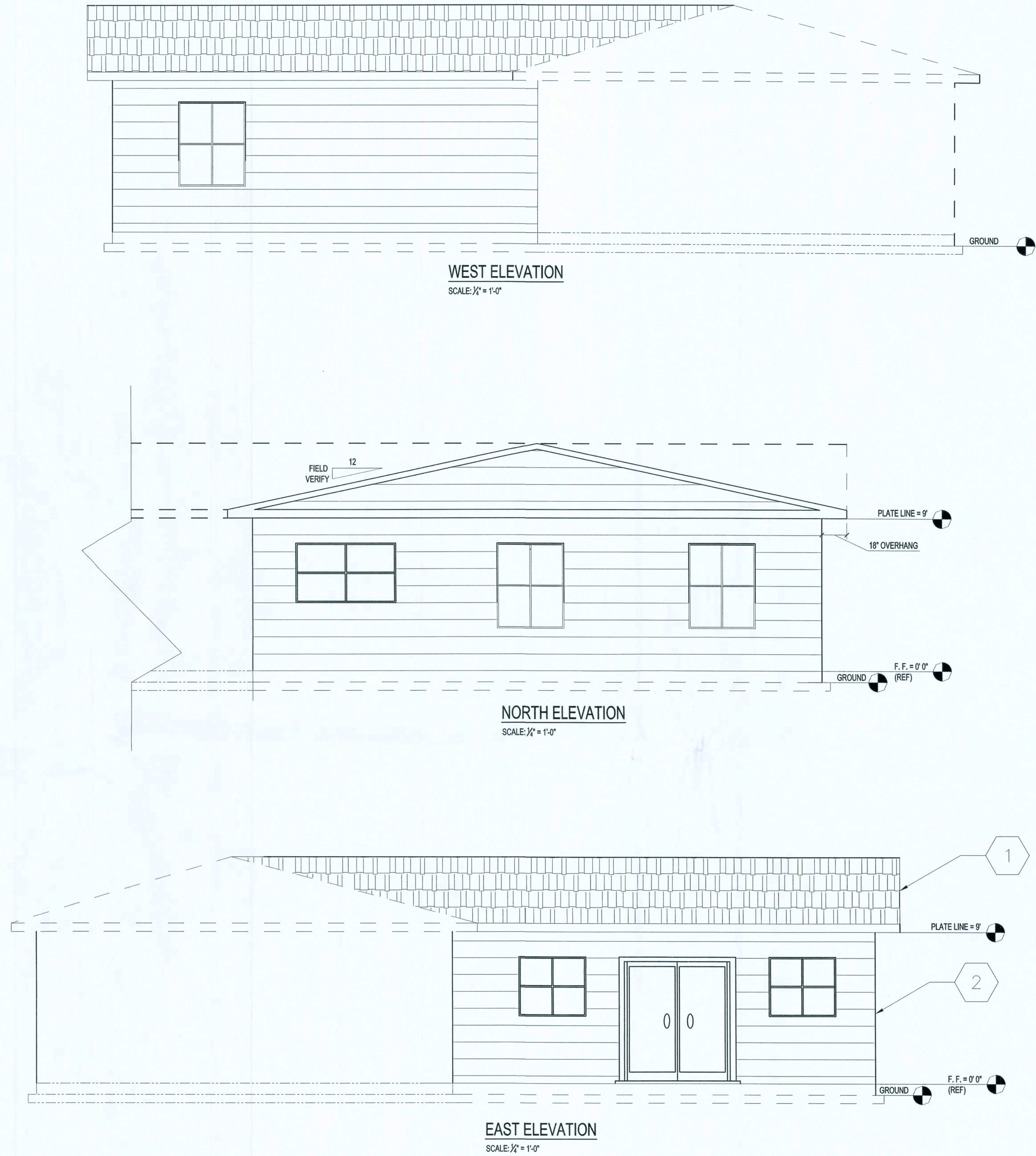
FLOOR PLAN
SCALE: 1/4" = 1'-0"

DOOR SCHEDULE			
SYMBOL	TYPE	SIZE	QUANTITY
(A)	INTERIOR CLOSET	2'8" X 6'8"	3
(B)	INTERIOR DOOR	3' X 6'8"	3
(C)	DBL EXTERIOR FRENCH DOORS	3' X 6'8"	2

WINDOW SCHEDULE				
SYMBOL	DESCRIPTION	SIZE	QUANTITY	NOTES
(1)	SINGLE HUNG	4' X 5'	3	EERO
(2)	SINGLE HUNG	4' X 3'6"	3	

NOTES
1) EERO = EMERGENCY ESCAPE AND RESCUE OPENING
2) ATTIC ACCESS = 3' X 2'
3) ALL ANGLED WALLS ARE 45° UNLESS OTHERWISE SPECIFIED

AREA INDEX
PROPOSED ADDITION = 870 S. F.



SHEET KEYNOTES

- 1 ARCH. SHINGLES TO MATCH EXISTING (PER OWNER)
- 2 8" VINYL SIDING TO MATCH EXISTING (PER OWNER)

REVISION NOTES		ISSUED FOR	
REV #	DATE	REV #	CONSTRUCTION
-	09-25		
PROJECT NUMBER PF06-242			
DRAWN BY FV			
CHECKED BY G.G.			
A-1			
SHEET ###			

P.O. Box 187
130 West Howard Street
Live Oak FL 32064
Phone: (386) 362-3678
Fax: (386) 362-6133
Gary Gill, PE 51942
Auth. # 9461



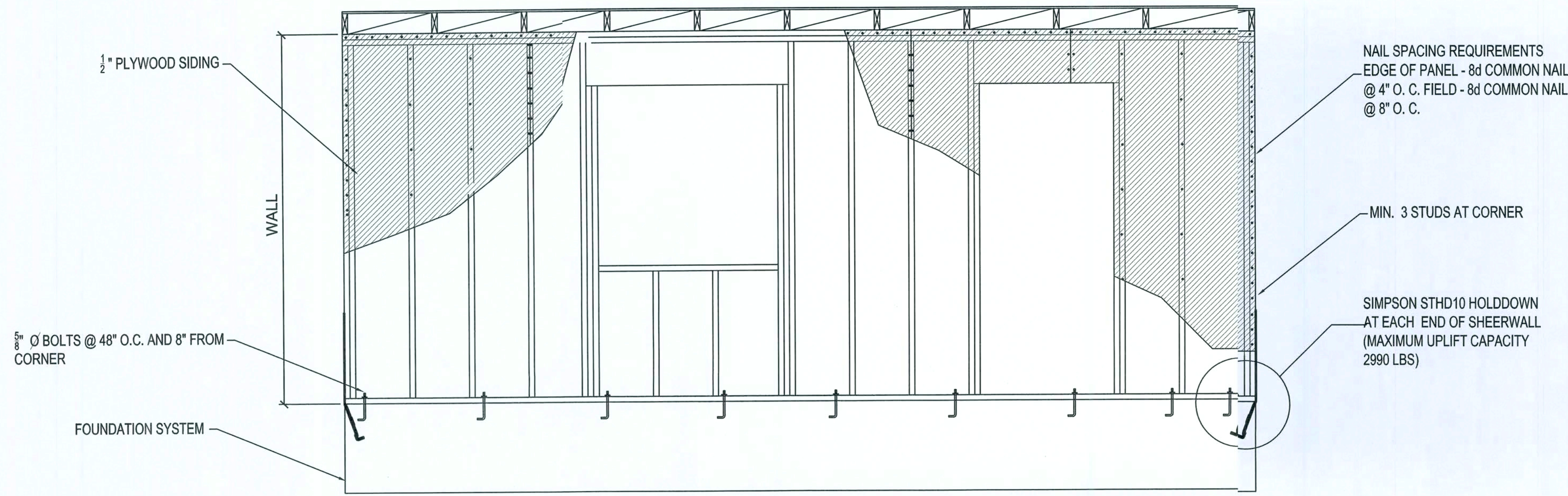
STEVEN'S RESIDENCE
NEW ADDITION

PLAN VIEW

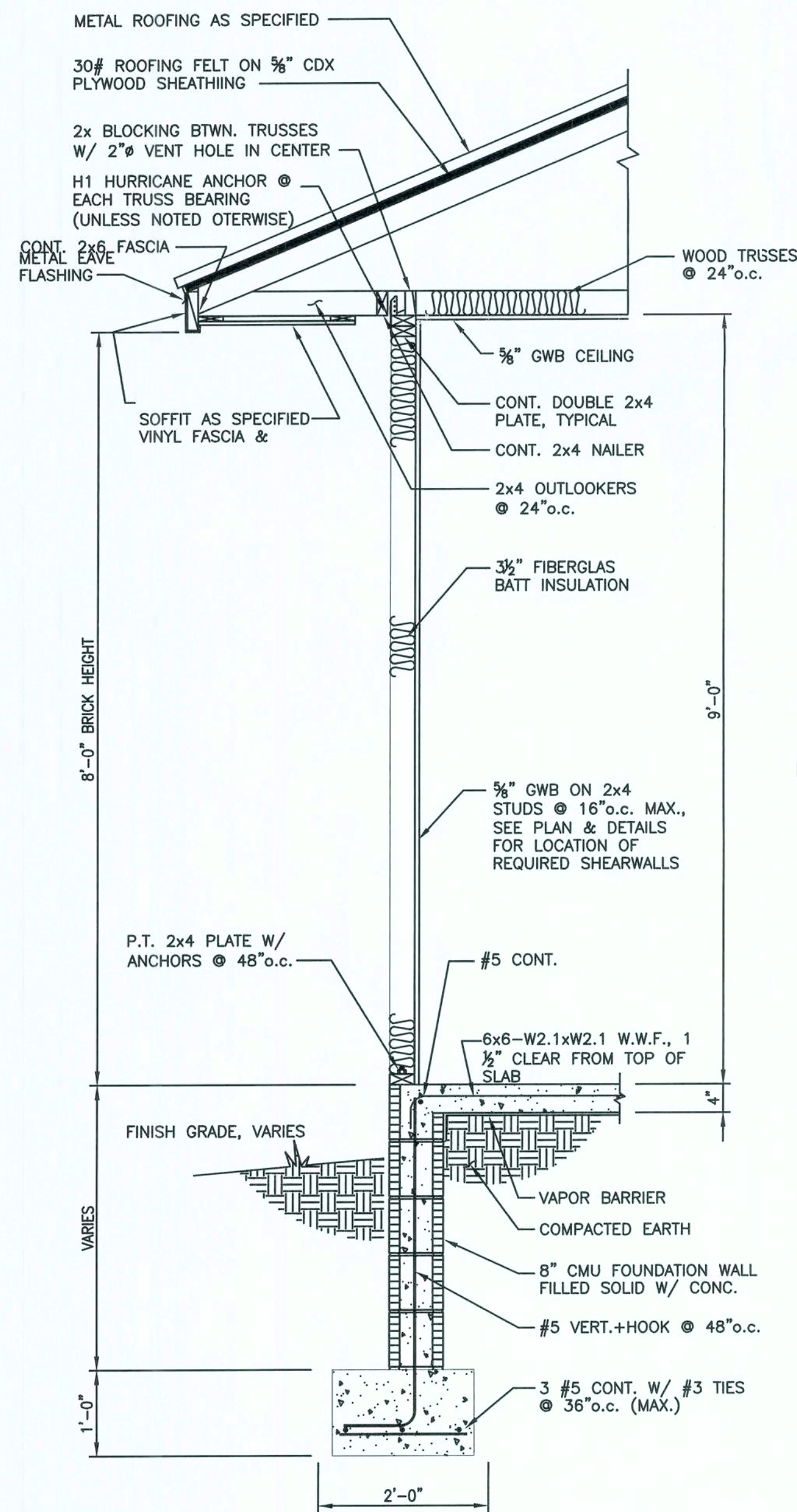


PROPOSED ELECTRICAL TO BE ADDED TO EXISTING PANEL

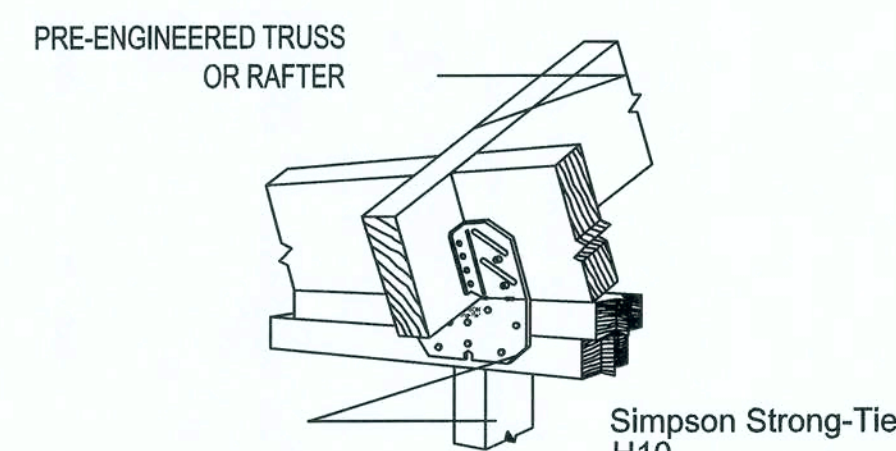
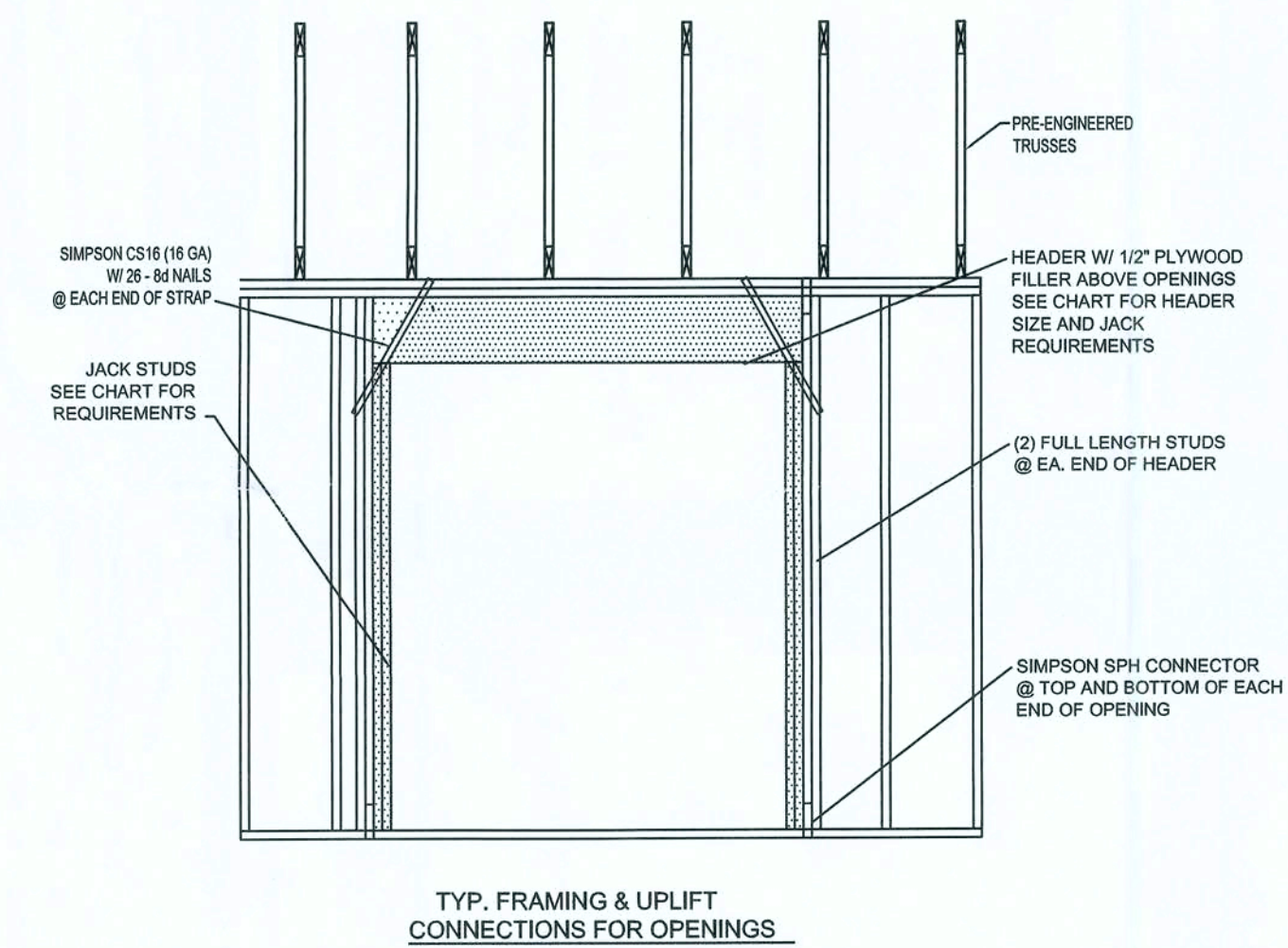
A-2



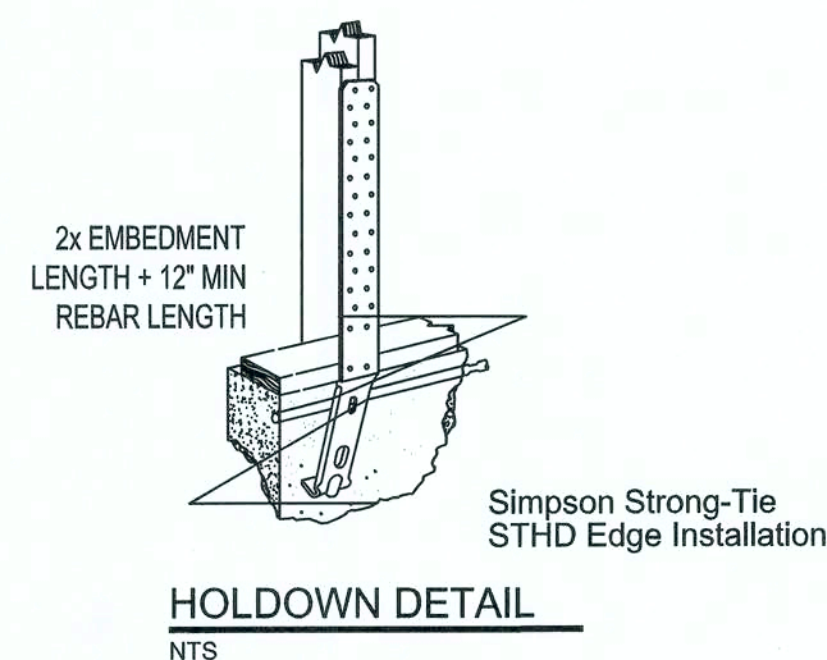
TYPICAL PERFORATED SHEERWALL
SCALE $\frac{3}{8}" = 1' 0"$



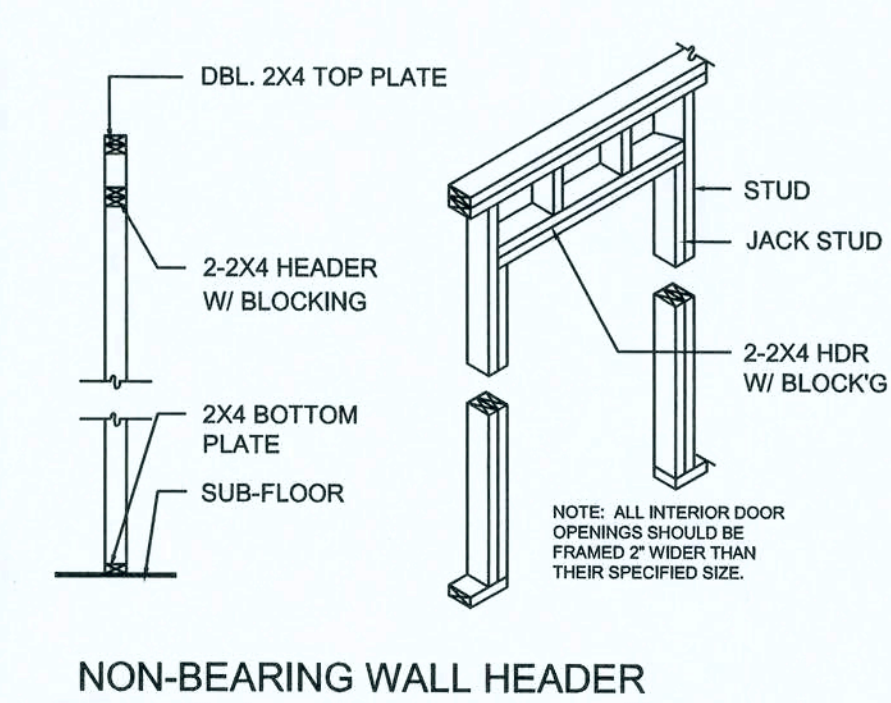
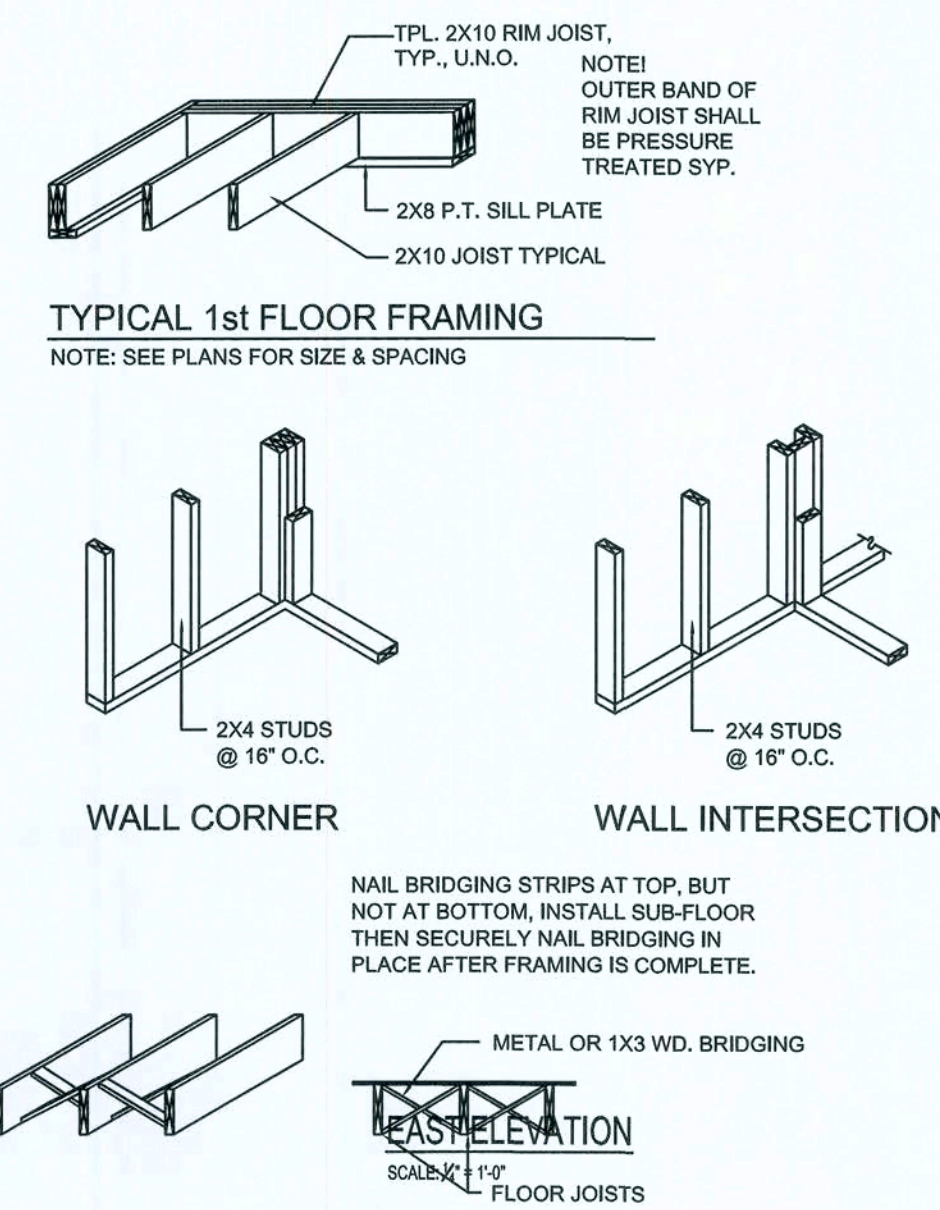
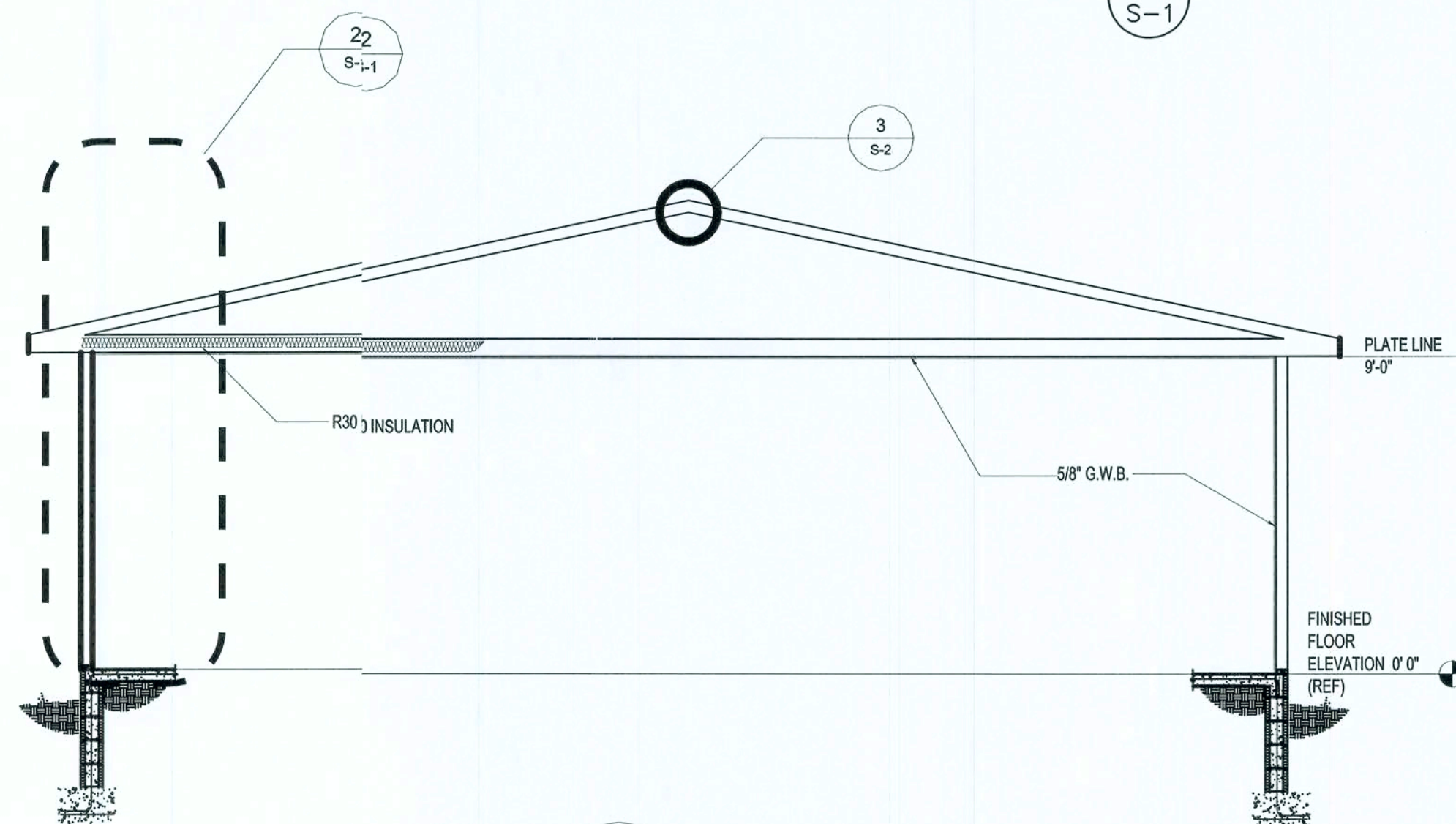
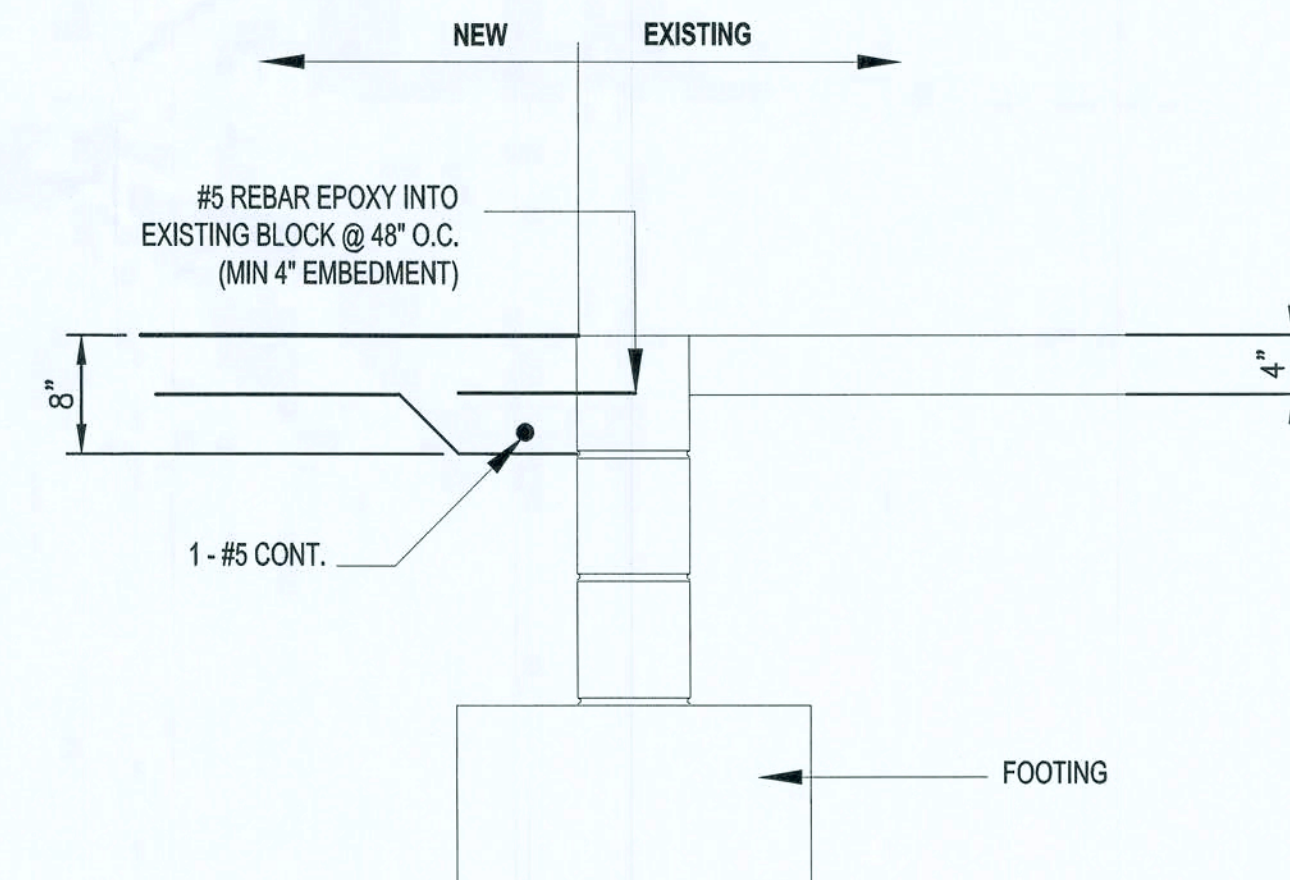
2 SECTION
S-1



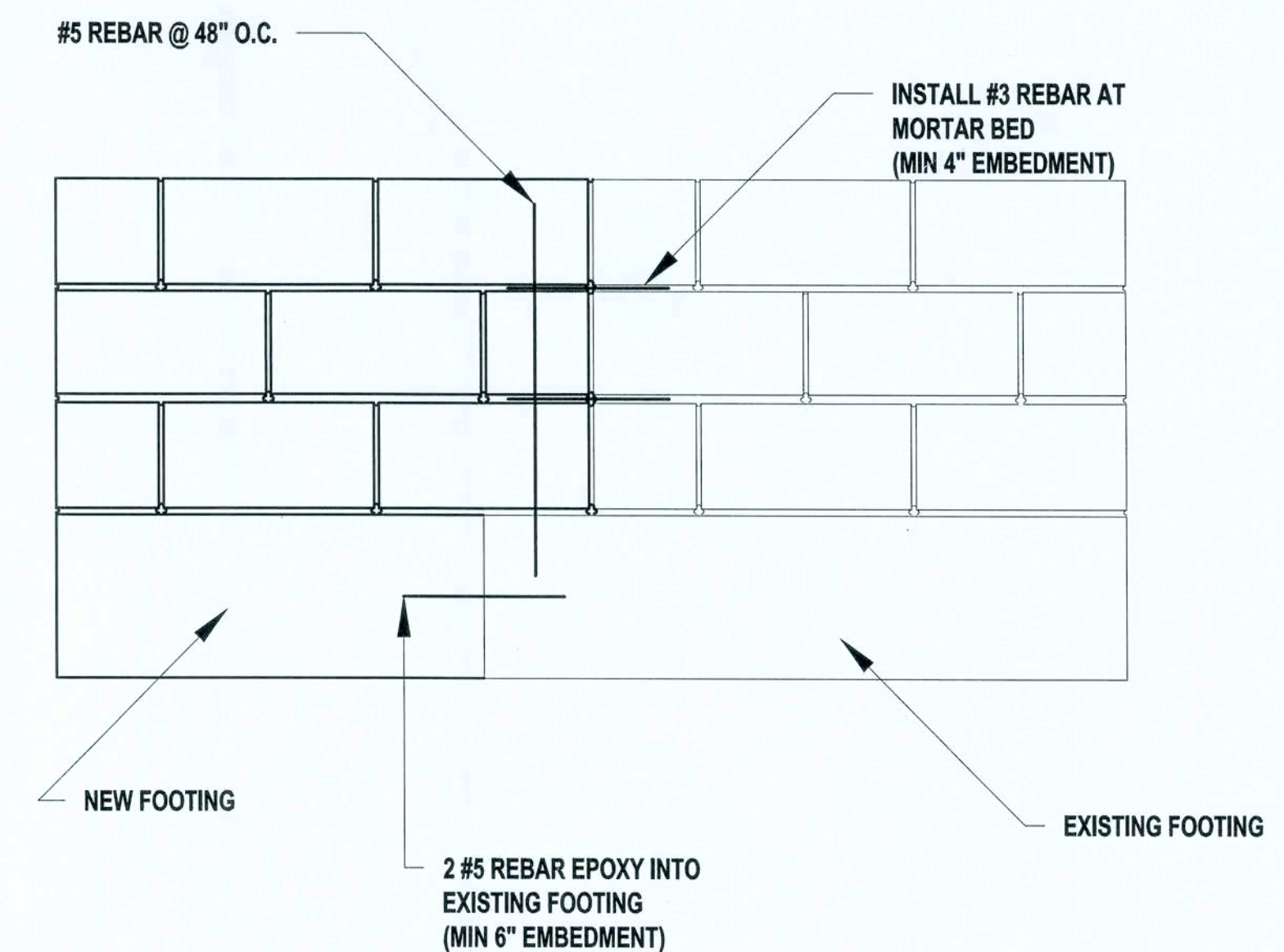
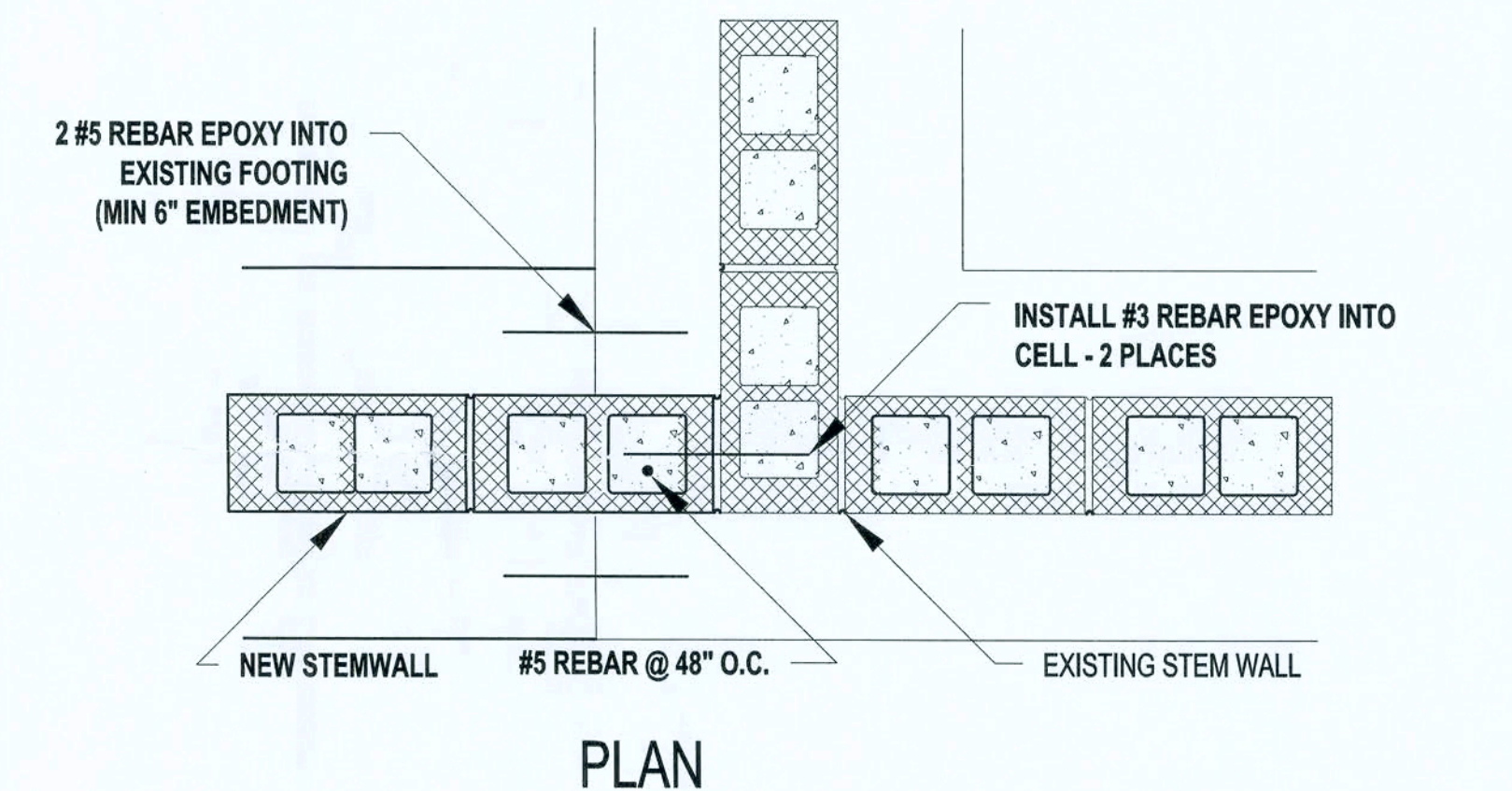
STUD TIES DETAILS
NTS



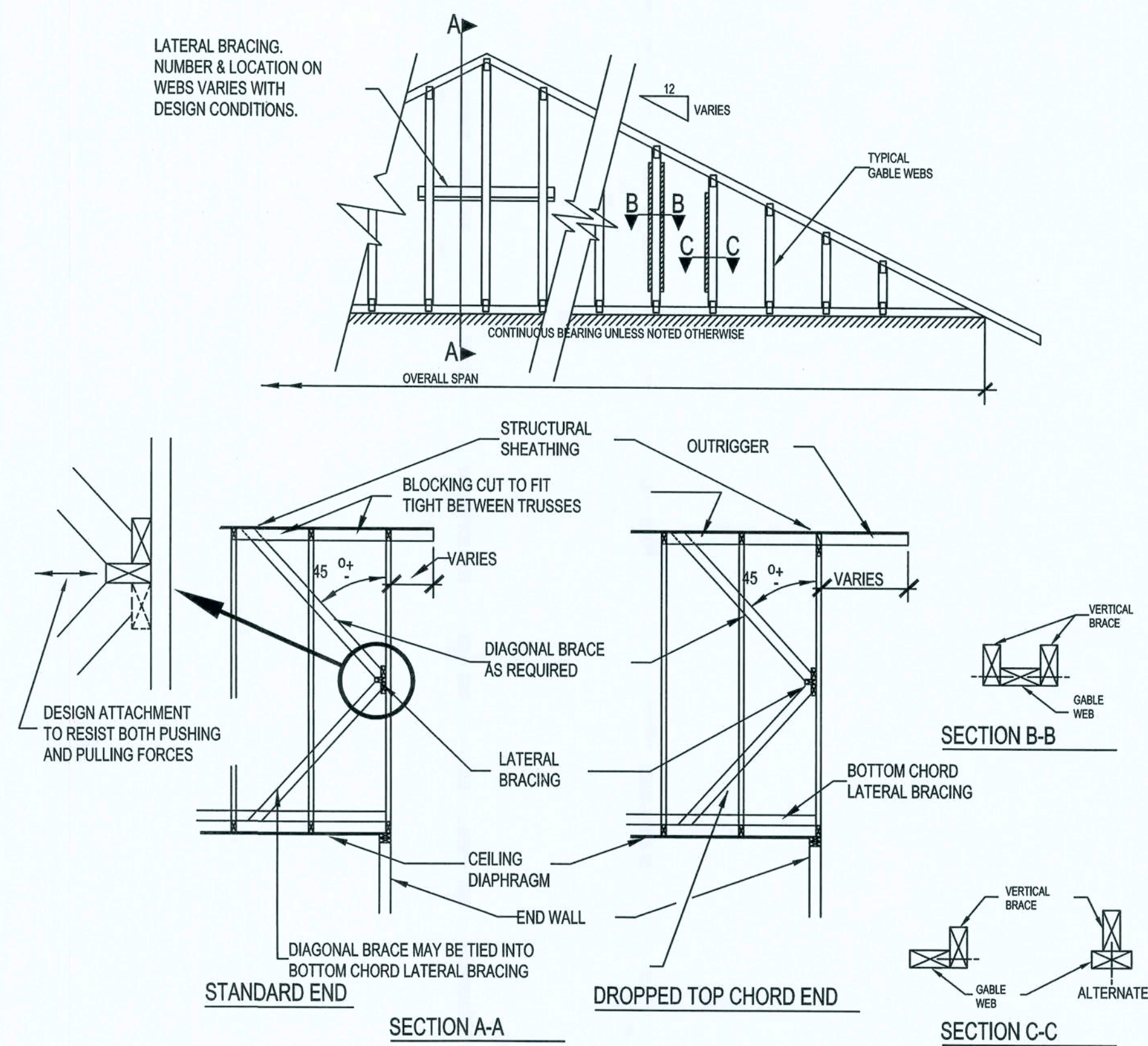
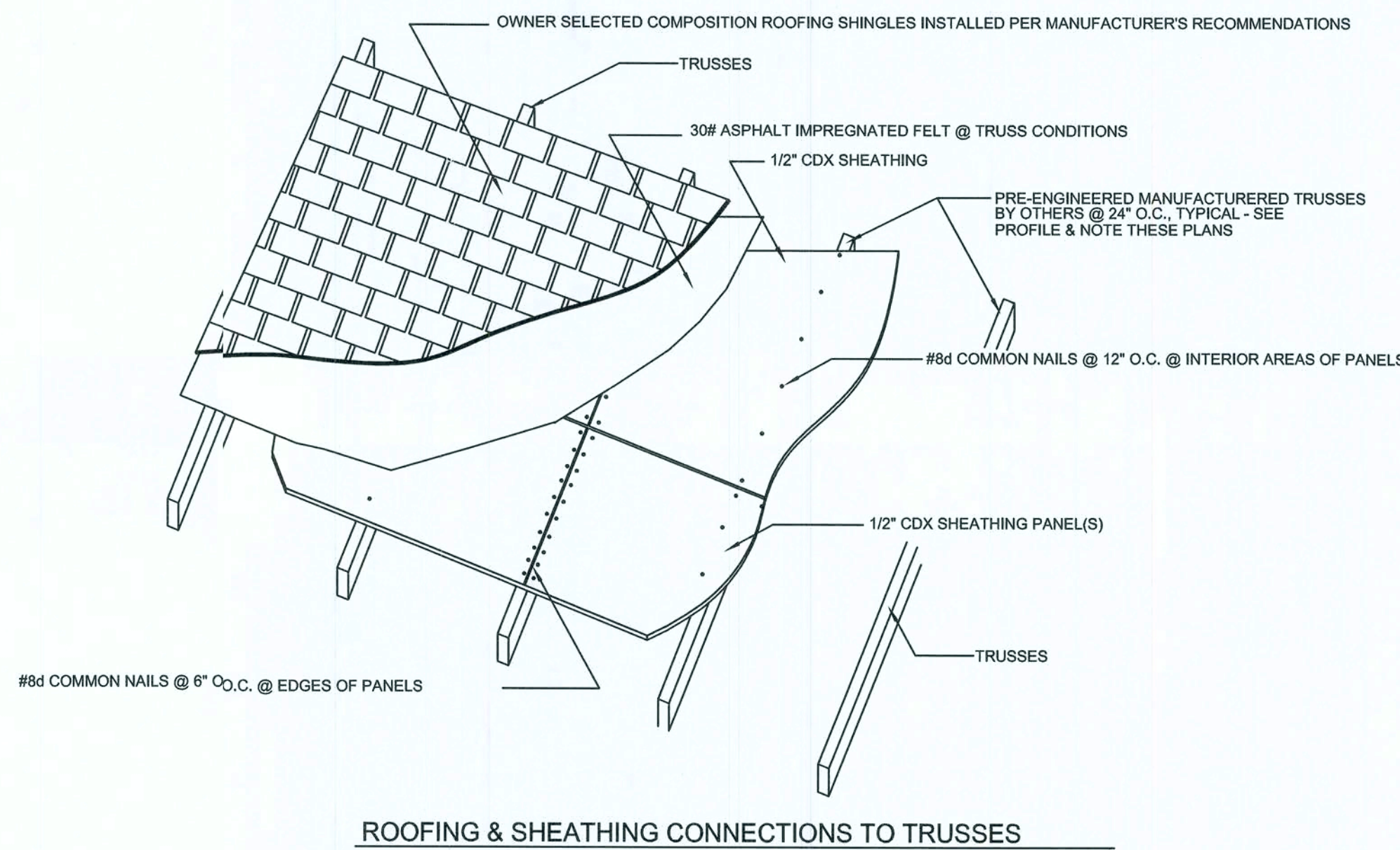
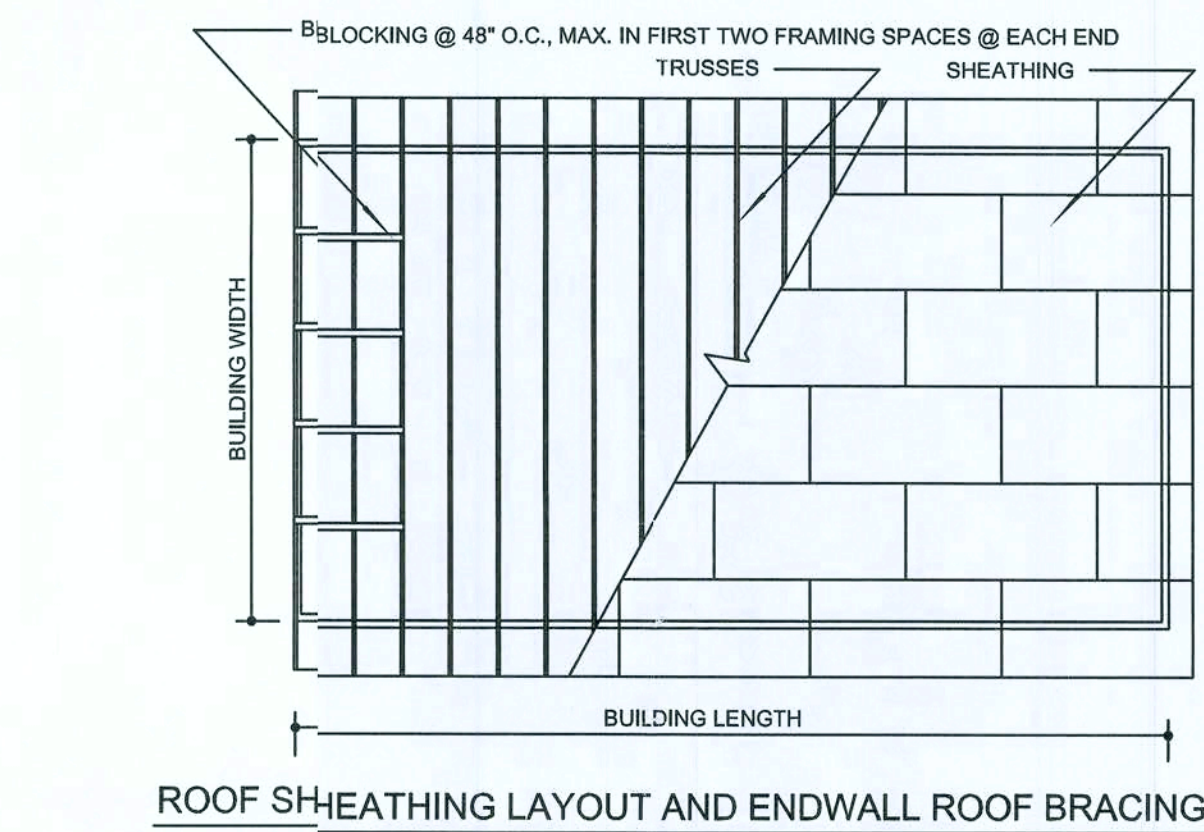
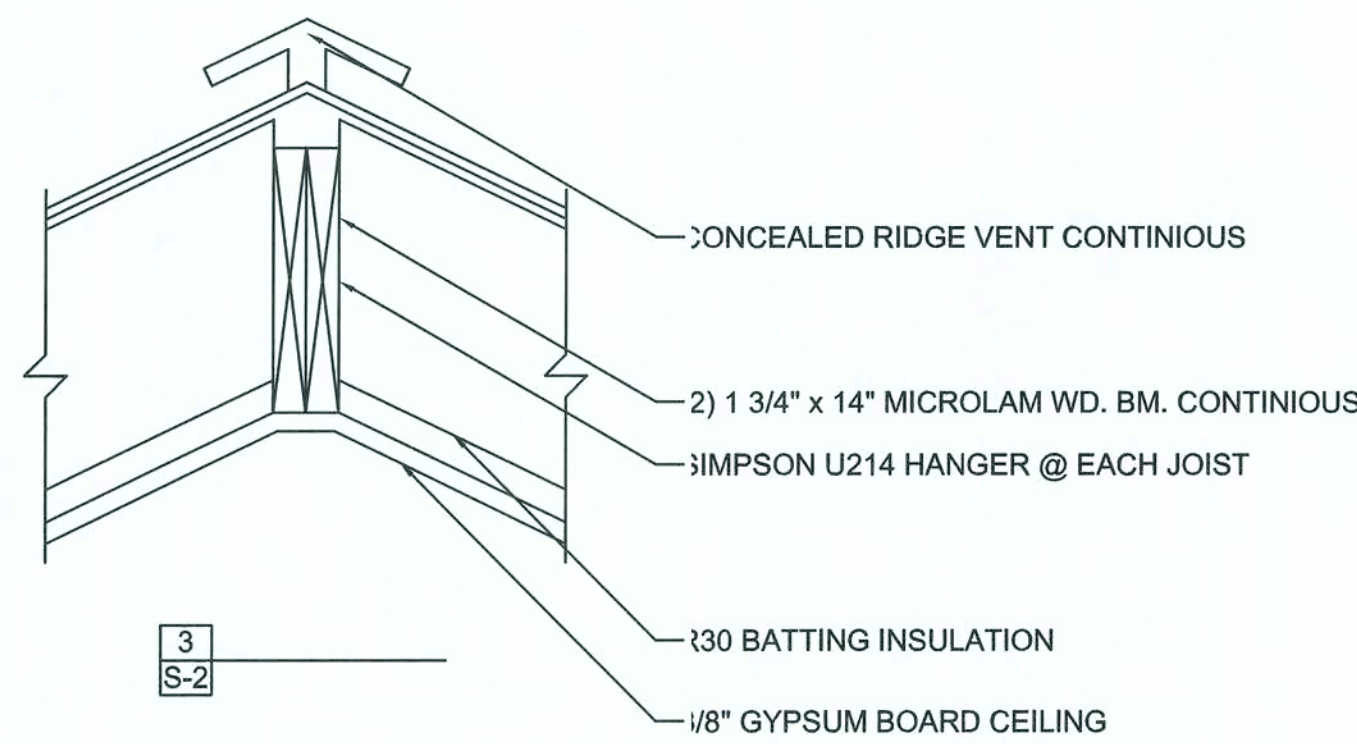
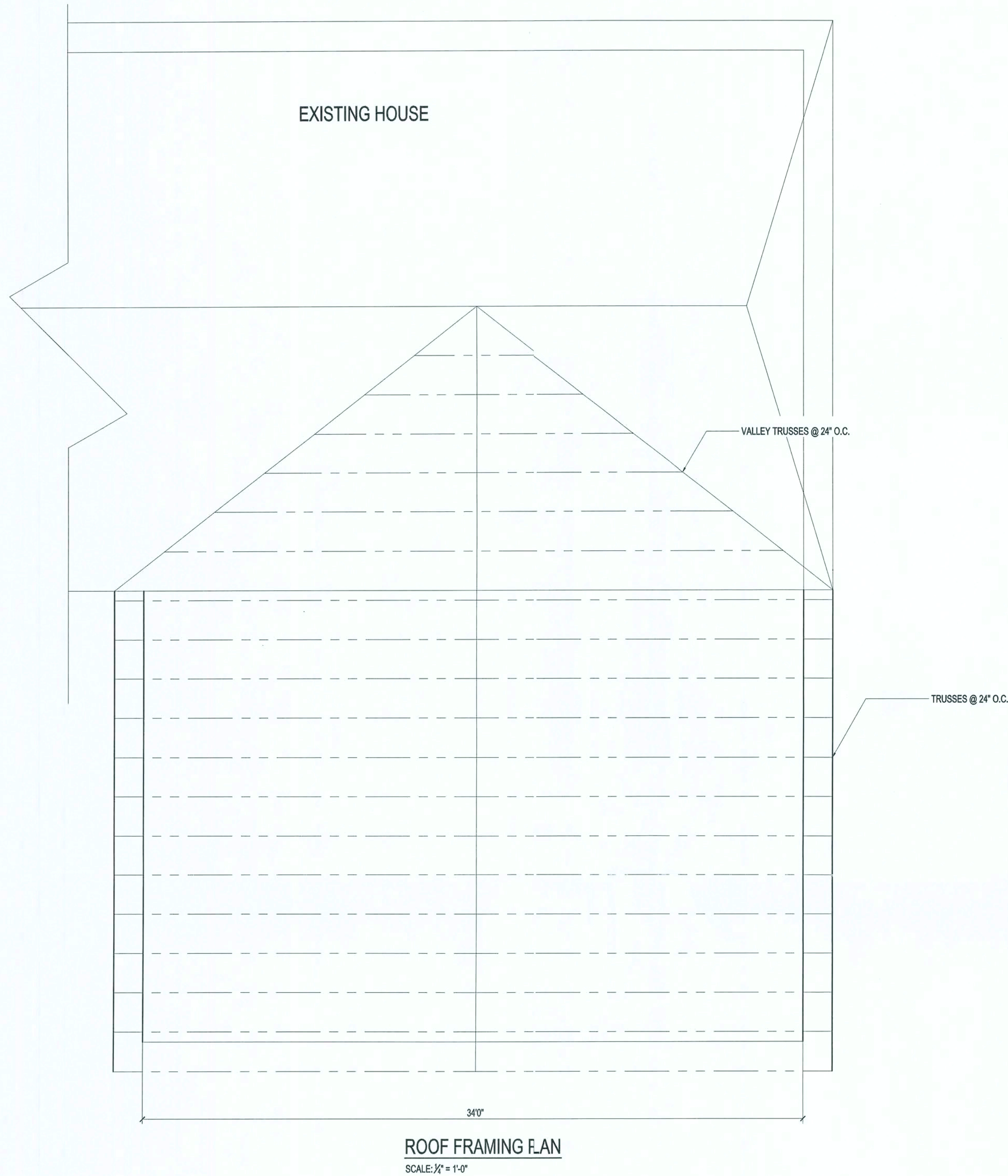
HOLDOWN DETAIL
NTS



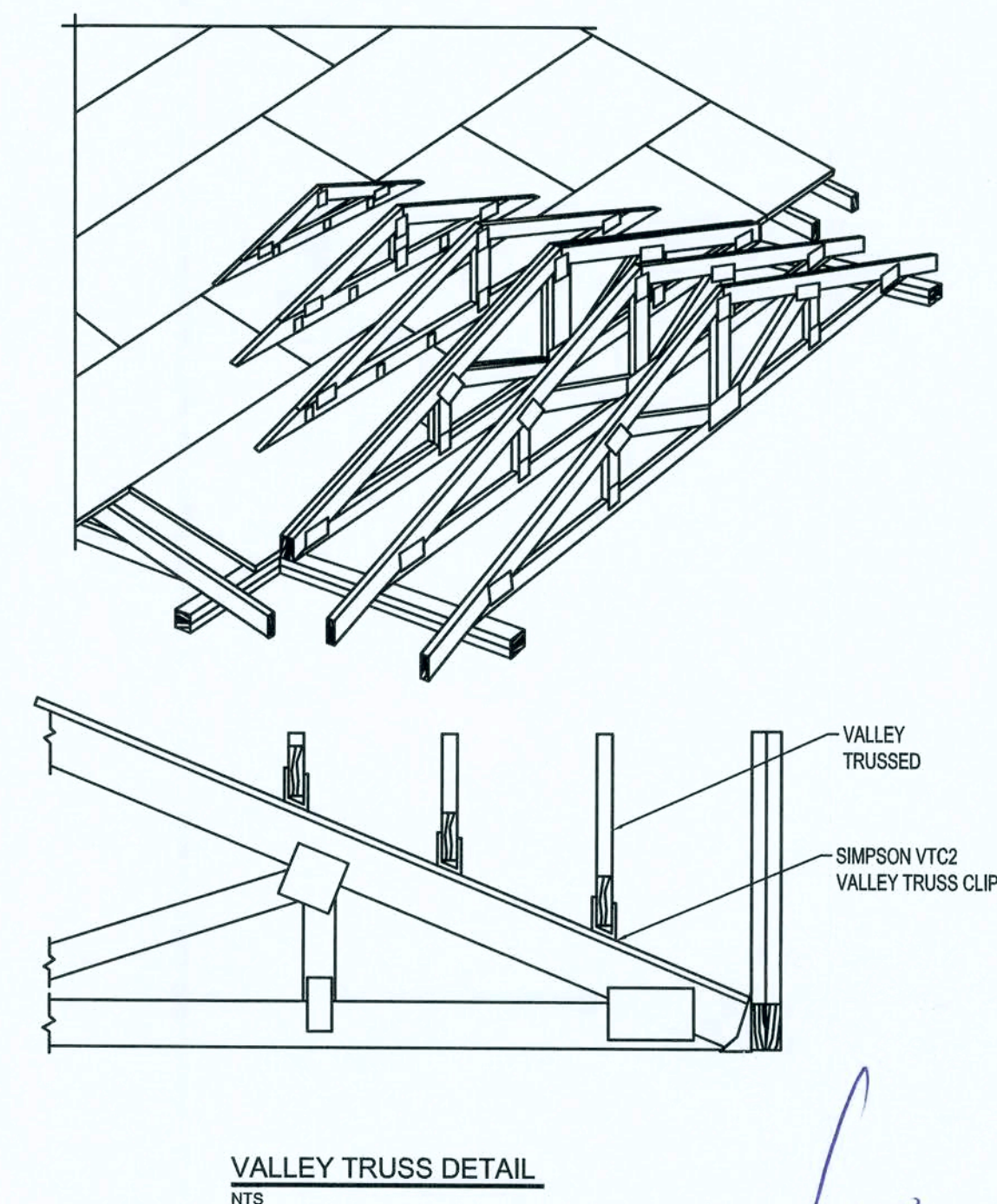
HEADER SPANS FOR EXTERIOR BEARING WALLS							
SUPPORTING:	HEADER SIZE	BUILDING WIDTH (FT)					
		20'		28'		36'	
		SPAN	# JACKS	SPAN	# JACKS	SPAN	# JACKS
ROOF, CEILING	2-2x4	3'-6"	1	3'-2"	1	2'-10"	1
	2-2x6	5'-5"	1	4'-8"	1	4'-2"	1
	2-2x8	6'-10"	1	5'-11"	2	5'-4"	1
	2-2x10	8'-5"	2	7'-3"	2	6'-6"	2
	2-2x12	9'-9"	2	8'-5"	2	7'-6"	2
	3-2x8	8'-4"	1	7'-5"	1	6'-8"	1
	3-2x10	10'-6"	1	9'-1"	2	8'-2"	1
	3-2x12	12'-2"	2	10'-7"	2	9'-5"	2
	4-2x8	9'-2"	1	8'-4"	1	9'-2"	1
	4-2x10	11'-8"	1	10'-6"	1	9'-5"	1
	4-2x12	14'-1"	1	12'-2"	2	10'-11"	1



1
S-1



- NOTES:
- 1) ACTUAL BRACING REQUIREMENTS WILL VARY DUE TO WIND LOAD, CODE CRITERIA, BUILDING HEIGHT, TRUSS SPAN, WEB LUMBER GRADE/SPECIES/ON CENTER SPACING AND OTHER VARIABLES. BRACING (AND ATTACHMENT) REQUIREMENTS SHOULD BE DESIGNED FOR EACH SPECIFIC JOB.
 - 2) CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL, AS WELL AS THE DESIGN AND SPECIFICATION OF TEMPORARY AND PERMANENT BRACING OF THE ROOF SYSTEM IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.



REVISION NOTES		ISSUED FOR	
REV #	DATE	CONSTRUCTION	
1	09-25		
P.O. Box 187 130 West Howard Street Live Oak FL 32064 Phone: (386) 362-3678 Fax: (386) 362-6133 Gary Gil, PE 51942 AUTH. # 9461 STRUCTURAL CIVIL ENGINEERS			
STEVEN'S RESIDENCE NEW ADDITION			
ROOF DETAILS			
PROJECT NUMBER PF06-242			
DRAWN BY FV			
CHECKED BY G.G.			
S-2			
SHEET ###			

DESIGN CRITERIA

DESIGN PER 2004 FLORIDA BUILDING CODE, UNLESS OTHERWISE NOTED.

LIVE LOADS:	
ROOFS AND CANOPIES:	0 TO 200 SF16PSF
	201 TO 600 SF14PSF
	OVER 600 SF12PSF
STAIRS.....	100PSF
FLOORS.....	50PSF
CORRIDORS.....	80PSF
LOBBIES.....	80PSF
BALCONIES.....	60PSF
PARTITION LOAD (DEAD LOAD).....	20PSF

WIND LOADS:	
BASIC WIND SPEED: (ASCE 7).....	110 MPH
MEAN ROOF HEIGHT.....	21 FT
WIND IMPORTANCE FACTOR (CATEGORY II).....	1.0
WIND EXPOSURE.....	B
ENCLOSURE CLASSIFICATION.....	ENCLOSED
INTERNAL PRESSURE COEFFICIENT.....	-0.18
DIRECTIONALITY FACTOR (Kd).....	0.85
SHAPE FACTORS.....	PER CODE

THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGION. IMPACT RESISTANT GLAZING IS NOT REQUIRED.

DESIGN WIND PRESSURES FOR COMPONENTS & CLADDING:			
WALLS & WALL OPENINGS			
TRIBUTARY AREA	INTERIOR ZONE	END ZONE	
	(> 6.3 ft FROM BLDG. CORNER)	(< 6.3 ft FROM BLDG. ORNER)	
10 sf	-23.61 / 21.7	-29.2 / 21.77	
25 sf	-22.31 / 20.5	-26.55 / 20.5	
(LINEARLY INTERPOLATE BETWEEN STATED VALUES)			
ROOFS & ROOF OPENINGS			
TRIBUTARY AREA	INTERIOR ZONE	END ZONE	
	(> 6.3 ft FROM BLDG. CORNER)	(< 6.3 ft FROM BLDG. ORNER)	
10 sf	-21.77/19.92	-25.46 / 19.92	
25 sf	-20.30 / 19.19	-23.99 / 19.19	
(LINEARLY INTERPOLATE BETWEEN STATED VALUES)			

CONCRETE (DESIGN PER CURRENT EDITION ACI 318)	
SLAB ON GRADE.....	F'C= 4000 PSI
FOOTINGS.....	F'C= 3000 PSI
ALL OTHER CONCRETE.....	F'C= 3000 PSI

ALL REINFORCING STEEL ASTM A615 GRADE 60

ALL WELDED WIRE FABRIC ASTM A185

CONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 530)	
COMPRESSIVE STRENGTH.....	F'M= 1500 PSI

STRUCTURAL STEEL (DESIGN PER CURRENT EDITION AISC), UNLESS OTHERWISE NOTED MATERLS SHALL BE AS FOLLOWS:

W-SHAPES.....	ASTM 992, Fy=50 KSI
OTHER SHAPES & PLATES.....	ASTM A36, Fy=36 KSI
HSS SQUARE & RECTANGULAR SHAPES.....	ASTM A500 GRADE B, Fy= 46 KSI
HSS ROUND SHAPES.....	ASTM A500 GRADE B, Fy= 42 KSI
STEEL PIPES.....	ASTM A53 GRADE B, Fy= 35 KSI
WELDING ELECTRODES.....	AWS A5.1 OR A5.5 SERIES E70
HIGH-STRENGTH BOLTS.....	1/2" ASTM A325
ANCHOR RODS.....	GRADE 36 ASTM F1554
WELDED STUDS.....	ASTM A108
DEFORMED BARS.....	ASTM A496
PAINT & PROTECTION.....	SSPC PAINT 25

SOIL BEARING (DESIGN MAXIMUM).....	1000PSF
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GENERAL NOTES

CONCRETE

UNLESS OTHERWISE NOTED ON THE DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:

FOOTINGS.....	3"
PILE CAPS.....	SEE TYPICAL DETAIL
GRADE BEAMS.....	3"
COLUMNS AND PEDESTALS (OVER VERTICAL REINF).....	2"
SLABS AND WALLS (EXPOSED TO EARTH, LIQUID OR WEATHER).....	2"
SLABS AND WALLS (NOT EXPOSED TO EARTH, LIQUID OR WEATHER).....	3/4"
CANOPY SLABS.....	1 1/2"
BEAMS (OVER MAIN REINFORCING).....	1 1/2"
SLABS ON GRADE.....	2" FROM TOP

ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 315 DURING THE PLACEMENT OF CONCREE.

UNLESS OTHERWISE NOTED, SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS FOLLOWS:
WELDED WIRE FABRIC.....WIRE SPACING PLUS 6"
REINFORCING BARS.....40 BAR DIAMETERS

ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE NOTD.

FOUNDATIONS

IF FOOTING EVALUATIONS SHOWN OCCUR IN A DISTURBED, UNSTABLE, OR UNSUITABLE SOIL, TH ENGINEER SHALL BE NOTIFIED.

STEPS IN WALL FOOTINGS SHALL NOT EXCEED A SLOPE OF (1) VERTICAL TO TWO (2) HORIZONTAL

PROVIDE A MINIMUM OF TWO #4 BARS IN TOP OF CONTINUOUS WALL FOOTINGS AT DOOR AND OTHER OPENINGS, 4'-0" LONGER THAN THE OPENING.

TRUSS FASTENER SCHEDULE

LOCATION	UPLIFT	FASTENER (1)	TRUSS	PLATE
ROOF TRUSS	<415#	1-H2.5	5-8d	5-8d
	<905#	1-H10	8-8dx 1 1/2	8-8dx 1 1/2
	<1200#	2-H2.5A	10-8d	10-8d
	<1470	1-H16	10-10dx 1 1/2	10-10d x 1 1/2

NOTES:

1) ALL CONNECTORS LISTED ARE SIMPSON STRONG-TIE, UON. OTHER MANUFACTURERS MAY BE SUBSTITUTED. SCREW SIZE AND NUMBER SHALL BE IN ACCORDANCE WITH MANUFACTURER'S CATALOG. ROOF TRUSS CLIPS SHALL BE SELECTED TO PROVIDE THE UPLIFT RESISTANCE SHOWN ON THE ROOF TRUSS SHOP DRAWINGS.
2) TRUSS ENGINEER MAY PROVIDE ALTERNATE CONNECTIONS.

SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

EMBEDMENT FOR EXPANSION BOLTS SHALL BE 3 1/2"Ø MINIMUM FOR 3/4" BOLTS IN CONCRETE, 5 1/2" IN GROUTED MASONRY. HILTI KWIK BOLT II OR EQUAL.

EPOXY GROUT SHALL BE POWER FAST CARTRIDGE SYSTEM BY RAWL, HY150 CARTRIDGE SYSTEM BY HILTI; (HILTI RES500, IF HOLE IS CORED INSTEAD OF DRILLED) OR APPROVED EQUAL, UON. EMBEDMENT SHALL BE 12 BAR DIAMETERS MINIMUM, UON. HOLES SHALL BE 1/2" LARGER THAN REBAR SIZE, AND 3/4" LARGER THAN THREADED ROD SIZE. HOLE SHALL BE BRUSHED OUT WITH BOTTLE BRUSH AND THEN BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURERS PRINTED INSTRUCTIONS.

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE OF THE PROJECT.

GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT / ENGINEER. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP OR HAVE BEEN MERELY "RUBBER STAMPED" SHALL BE RETURNED WITHOUT REVIEW.

CHANGES TO THE CONTRACT DOCUMENTS SHALL BE CLOUDON ON SHOP DRAWINGS OR REQUESTED IN WRITING. THE CONTRACTOR IS LIABLE FOR ANY DEVIATIONS UNLESS REVIEWED AND ACKNOWLEDGED BY THE ENGINEER. SHOP DRAWING SUBMITTALS SHALL ONLY BE CHECKED FOR CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS.

SPECIFICATIONS

CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION), EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS.

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS", AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS", AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AND AWS D1.1 "STRUCTURAL WELDING CODE", EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

A GEOTECHNICAL TESTING AND INSPECTION FIRM SHALL BE EMPLOYED TO PERFORM A SOIL SURVEY FOR SATISFACTORY AOIL MATERIALS, SAMPLING AND TESTING FOR QUALITY CONTROL AS PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THIS PROJECT. ALL EARTHWORK OPERATIONS SHALL BE PERFORMED TO THE SATISFACTION OF THE GEOTECHNICAL TESTING FIRM.

CONCRETE NOTES

- CONCRETE FOOTINGS AND SLABS
 - CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSF IN 28 DAYS.
 - REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.
 - WELDED WIRE MESH SHALL CONFORM TO ASTM A185.
 - PROVIDE A MINIMUM COVER OF 3" FOR REINFORCING STEEL WHEN THE CONCRETE IS PLACED DIRECTLY AGAINST THE GROUND. CONCRETE EXPOSED TO EARTH OR WEATHER SHALL HAVE A MINIMUM COVER OF 1 1/2" INCHES.
 - WELDED WIRE FABRIC SHALL HAVE A MINIMUM YIELD STRENGTH OF 65,000 psi.
 - MINIMUM WWF FOR SLAB ON GRADE SHALL BE 6x6-W1.4x1.4
 - A VAPOR RETARDER CONSISTING OF 6 MIL MINIMUM POLYETHYLENE WITH JOINTS LAPPED 6 INCHES AND SEALED WITH 2" APPROVED TAPE OR MASTIC, OR OTHER APPROVED MATERIALS HAVING A MAXIMUM PERM RATING OF 0.5

- SOIL PREPARATION AND PROPERTIES
 - AREA UNDER FOOTINGS, FOUNDATIONS, AND CONCRETE SLABS SHALL HAVE ALL VEGETATION, STUMPS, ROOTS, AND FOREIGN MATTERS SHALL BE REMOVED TO THEIR CONSTRUCTION.
 - FILL MATERIAL SHALL BE FREE OF VEGETATION AND FOREIGN MATERIAL
 - ALLOWABLE BEARING PRESSURE = 1500 psf

- CONCRETE SLAB SHALL HAVE A MINIMUM ALLOWABLE SLOPE TO ALLOW FOR WATER DRAINAGE FROM SHOWERS.

STEEL NOTES

STRUCTURAL STEEL (DESIGN PER CURRENT EDITION ACI 530) (UON) MATERIALS SHALL BE AS FOLLOWS:

W-SHAPES.....	ASTM 992, Fy=50 KSI
OTHER SHAPES & PLATES.....	ASTM A36, Fy=36 KSI
HSS SQUARE & RECTANGULAR SHAPES.....	ASTM A500 GRADE B, Fy=46 KSI
HSS ROUND SHAPES.....	ASTM A500 GRADE B, Fy=42 KSI
STEEL PIPES.....	ASTM A53 GRADE B, Fy=35 KSI
WELDING ELECTRODES.....	AWS A5.1 OR A5.5 SERIES E70
HIGH-STRENGTH BOLTS.....	1/2" ASTM A325
ANCHOR RODS.....	GRADE 36 ASTM F1554
WELDED STUDS.....	ASTM A108
DEFORMED BARS.....	ASTM A496
PAINT & PROTECTION.....	SSPC PAINT 25

SOIL BEARING (DESIGN MAXIMUM).....	2500 PSF
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PRODUCT APPROVAL SPECIFICATION TABLE

CATEGORY / SUBCATEGORY	MANUFACTURER	PROJECT DESCRIPTION	APPROVAL NUMBERS
EXTERIOR DOORS			
SWINGING	MASONITE INTERNATIONAL	METAL EDGE STEEL DOOR UNITS	19.1
ROLL UP	-		
WINDOWS			
SINGLE HUNG	BETTERBILT	FIN FRAME 52x72 MODEL 740/3740	663.13
ROOFING PRODUCTS			
ASPHALT SHINGLES	TAMKO	GLAS-SEAL AR - 3 TAB	1956.1
UDERLAYMENTS	TAMKO	MASTER SMOOTH -ASPHALT UNDERLAYMENT	1481.1
STRUCTURAL COMPONENTS			
WOOD CONNECTORS	SIMPSON STRONG-TIE		
	-	CS16	1901.4
	-	SPH6	538.35
	-	MSTC40	1901.64
	-	H10	474.109
	-	MSTCM40	1901.70
	-	LSSU410	474.248
	-	H6	474.119
TRUSS PLATES	ALPINE	CC46	1218.13
		LSTA36	1901.36
		IUT14	474.216
		AB66	474.10

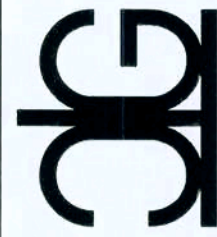
CONNECTION LISTING

MANUFACTURER	MODEL #	FASTENER COUNT	ALLOWABLE LOAD
SIMPSON STRONG TIE CO.	CS16	(26) 8d	1705
SIMPSON STRONG TIE CO.	SPH6	(10) 10d x 1 1/2	1240
SIMPSON STRONG TIE CO.	MTSC40	(52) 16d SINKERS	4335
SIMPSON STRONG TIE CO.	H10	(8) 8d x 1 1/2	905
SIMPSON STRONG TIE CO.	MSTCM40	(14) 16d SINKERS	2335
SIMPSON STRONG TIE CO.	LSSU410	FACE=(18) 16d JOIST=(12) 10D x 1 1/2	1150
SIMPSON STRONG TIE CO.	H6	(8) 8d	915
SIMPSON STRONG TIE CO.	CC46	BEAM=(4) 3/8" POST=(2)3/8"	2330
SIMPSON STRONG TIE CO.	LSTA36	(24) 10d	1640
SIMPSON STRONG TIE CO.	IUT14	(14) 10d x 1 1/2	245
SIMPSON STRONG TIE CO.	AB66	N/A	5335
SIMPSON STRONG TIE CO.	U26	HEADER=(6) 10d & (6) 16d JOIST=(4) 10d x 1 1/2	415
SIMPSON STRONG TIE CO.	ITT14	(6) 10d	1215
SIMPSON STRONG TIE CO.	U210	HEADER=(12) 16d JOIST=(6) 10d x 1 1/2	720

REVISION NOTES

DATE	09-25
REV #	-

P.O. Box 187
130 West Howard Street
Live Oak FL 32064
Phone: (386) 362-3678
Fax: (386) 362-6133
Gary Gill PE 51942
Auth. # 8461



STRUCTURAL CIVIL ENGINEERS

STEVEN'S RESIDENCE
NEW ADDITION

GENERAL NOTES

PROJECT NUMBER

PF06-242

DRAWN BY

FV

CHECKED BY

G.G.

S-3

SHEET #### #####