

[illegible]

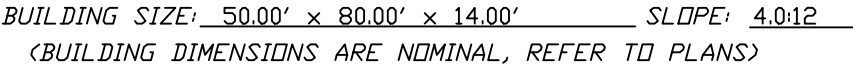
The following conditions apply in the event that these drawings are used as approval drawings:

- It is imperative that any changes to these drawings:
- 1) Be made in contrasting ink.
 - 2) Have all instances of change clearly indicated.
 - 3) Be legible and unambiguous.
- B) Dated signature is required on all pages.
- C) Manufacturer reserves the right to re-submit drawings with extensive or complex changes required to avoid misfabrications. This may impact the delivery schedule.
- D) Approval of these drawings indicates conclusively that the manufacturer has correctly interpreted the contract requirements, and further constitutes agreement that the building as drawn, or as drawn with indicated changes represents the total of the materials to be supplied by manufacturer.
- E) Any changes noted on the drawings not in conformance with the terms and requirements of the contract between manufacturer and its customer are not binding on manufacturer unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation. Manufacturer recognizes that rubber stamps are routinely used in indicating approval, disapproval, rejection, or mere review of the drawings submitted. However, manufacturer does not accept changes or additions to contractual terms and conditions that may appear with the use of a stamp or similar indication of approval, disapproval, etc. Such language applied to the manufacturer's drawings by the customer, architect, engineer, or any other party will be considered as unacceptable alterations to these drawing notes, and will not alter the contractual rights and obligations existing between manufacturer and its customer.

The building manufacturer has commitment to manufacture quality building components that can be safely erected, however, the safety commitment and job site practices of the erector are beyond the control of the building manufacturer. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, state and federal safety and health standards, whether standard statutory or customary, should always be followed to help insure worker safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

The proper tightening and inspection of all fasteners is the responsibility of the erector. All high strength (A325, A490) bolts and nuts must be tightened by the "turn-of the nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation and inspection procedures are compatible prior to the start of erection.

is the responsibility of the builder/contractor to insure that all project plans and specifications comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that the building manufacturer or its design engineer is acting as the engineer of record or design professional for a construction project. The contractor must secure all required approval and permits from the appropriate agency as required. Approval of the manufacturer's drawings and calculations indicate that the building manufacturer correctly interpreted and applied the requirements of the contract drawings and specifications. (sect. 4.2.1 AISC code of standard practices, 9th ed.) Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (sect. 3.3 AISC code of standard practice 9th ed.) Design considerations of any material in the structure which are not furnished by the building manufacturer are the responsibility of the contractors and engineers other than the building manufacturer's engineer unless specifically indicated. The contractor is responsible for all erection of steel and associated work in compliance with the building manufacturer's "for erection installation" drawings. Products shipped to builder or his customer shall be inspected by builder immediately upon arrival. Claims for shortages or defective material, if not packaged, must be made to the manufacturer in writing within five (5) days after receipt of the shipment. However, if a defect is of such nature that reasonable visual inspection would fail to disclose it, then the claim must be made within five (5) days after the builder learns of the defect. The manufacturer will not be liable for any defect unless claim is made one (1) year after date of the original shipment by the manufacturer to builder or his customer. The manufacturer will be given a reasonable opportunity to inspect defective materials upon receipt of claim by builder. If a defect is of such nature that it can be remedied by a field operation at the job site without the necessity of returning the material to the manufacturer, then upon written authorization of the manufacturer, the builder may repair or cause the material to be repaired and the manufacturer will reimburse the builder for the cost of the repair in accordance with the written authorization. Unless noted otherwise, all bracing as shown and provided by the manufacturer for this building is required and shall be installed by the erector as a permanent part of the structure. Temporary supports, such as temporary guys, braces, false work, cribbing or other elements required for the erection operation will be determined and furnished and installed by the erector. These temporary supports will secure the steel framing, or any partly assembled steel framing, against loads comparable in intensity to those for which the structure was designed, resulting from wind, seismic forces and erection operations, but not the loads resulting from the performance of work by or the acts of others, nor such unpredictable loads as those due to tornado, explosion or collision. (sect. 7.9.1 AISC code of standard practice, 9th ed.) Design of gutter and downspout is a function of the rainfall intensity and area to be drained. Design parameters utilized are in accordance with the 2002 low rise building systems manual and/or the 12th edition of the architectural graphic standards, as applicable. Proper owner maintenance dictates that the drainage system be kept free of debris and/or ice at all times to ensure proper function of the gutter and downspout. In those cases where the owner/tenant of a property is unwilling or unable to provide proper maintenance, elimination of gutter should be considered as an alternative.



This is to certify that this structure is designed utilizing the loads indicated and applied as required by the building code shown below. The certification is limited to the structural design of the framing and covering parts manufactured by the building manufacturer and is specified in the contract. Accessory items such as doors, window, louvers, translucent panels, and ventilators are not included. Also excluded are other parts of the project not provided by the building manufacturer such as foundations, masonry walls, mechanical equipment and erection of the building. The building should be erected on a properly designed foundation in accordance with the building manufacturer's design manual, the attached drawings and good erection practices.

Design Code FBC 23 Risk Category II - Normal

General Loads	
Roof Dead Load (D)	<u>2.50</u> psf
Roof Collateral Load (C)	<u>1.00</u> psf
Roof Live Load (Lr)	<u>20.00</u> psf
Tributary Live Load Reduction	<u>Yes</u>

Snow Load	
Flat-Roof Snow Load (Pf)	5.00 psf
Ground Snow Load (Pg)	3.00 psf
Snow Exposure Factor (Ce)	1
Snow Load Importance Factor (Is)	1.0000
Thermal Factor (Ct)	1.2000

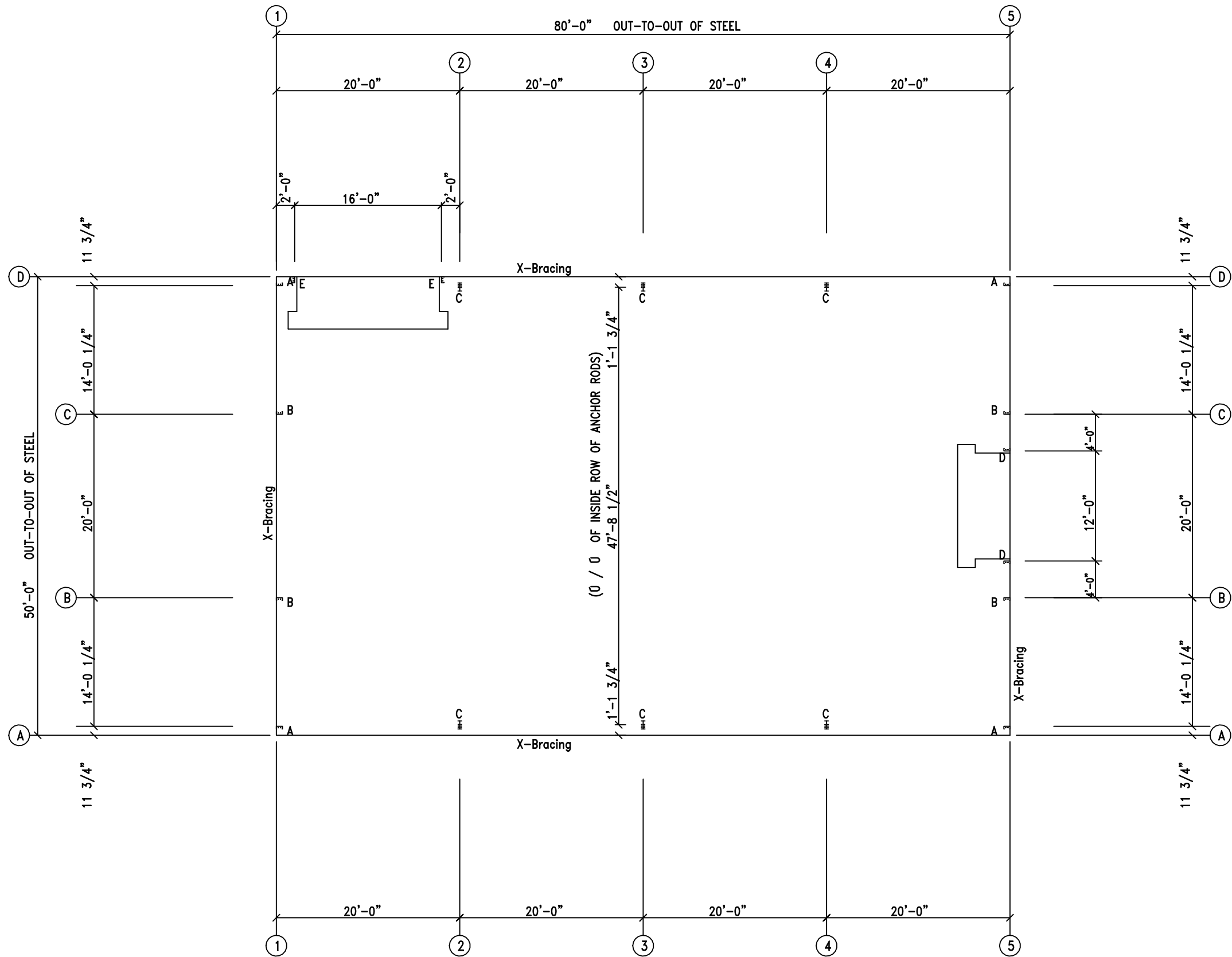
Wind Load

Ultimate Wind Speed (3 sec. gust) (V_{ult})	<u>130</u> mph
Nominal Wind Speed (V_{asn})	<u>100.70</u> mph
Wind Exposure Category	<u>B</u>
Enclosure Classification	<u>Enclosed</u>
Internal pressure coefficient	<u>0.18</u> / -0.18

Seismic Load		
Seismic Importance Factor (Ie)	<u>1.00</u>	
Mapped Spectral Response Accelerations	<u>Ss 0.12</u>	<u>S1 0.06</u>
Site Class	<u>D</u>	
Design Spectral Response Accelerations	<u>Ses 0.10</u>	<u>Su1 0.08</u>
Seismic Design Category	<u>B</u>	
Basic Seismic-Force-Resisting System(s)	<u>STEEL DMF</u>	/ DCBF
Analysis Procedure	<u>ELF</u>	

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	100
RF SEIS:	50
WIND BENT SEIS:	50

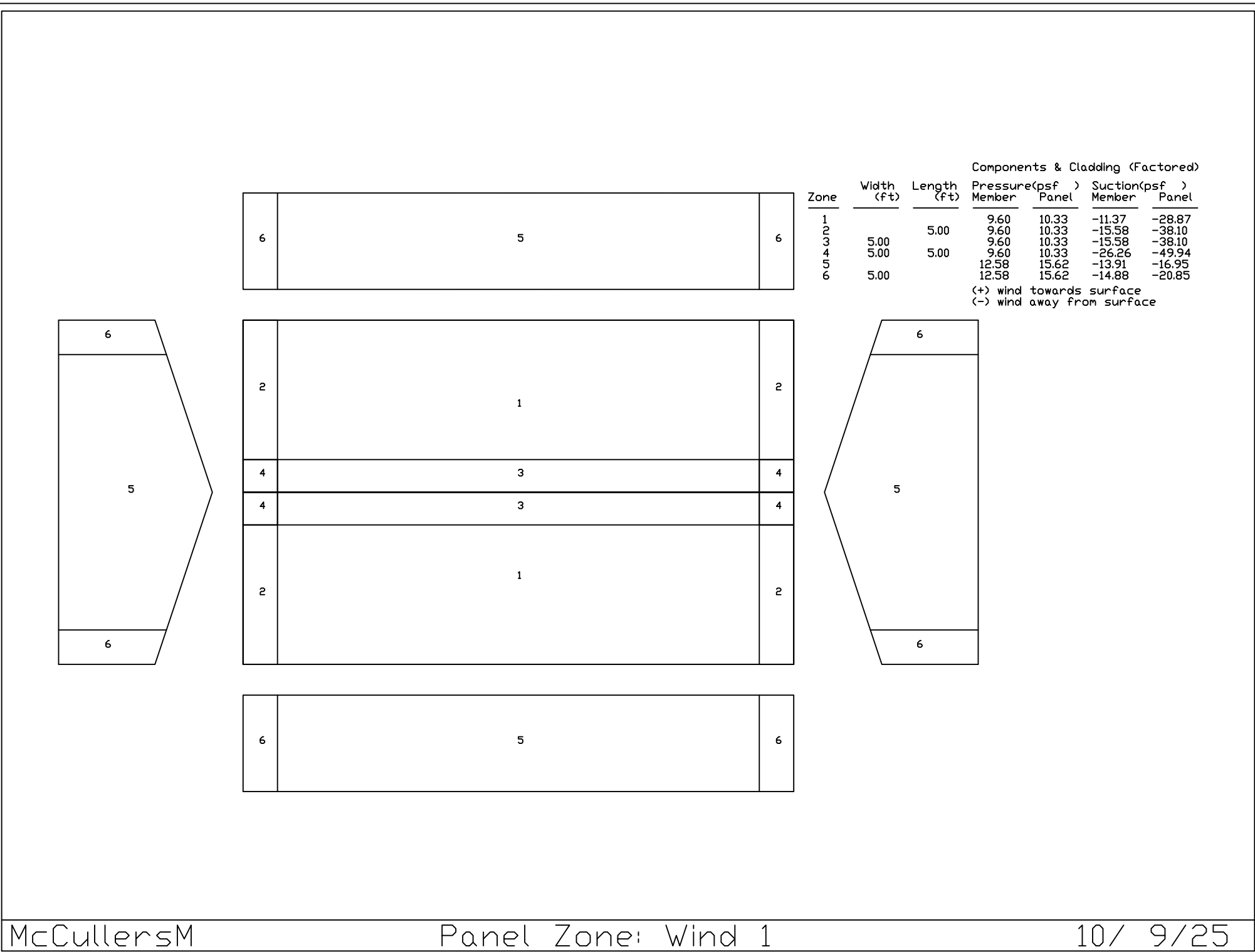
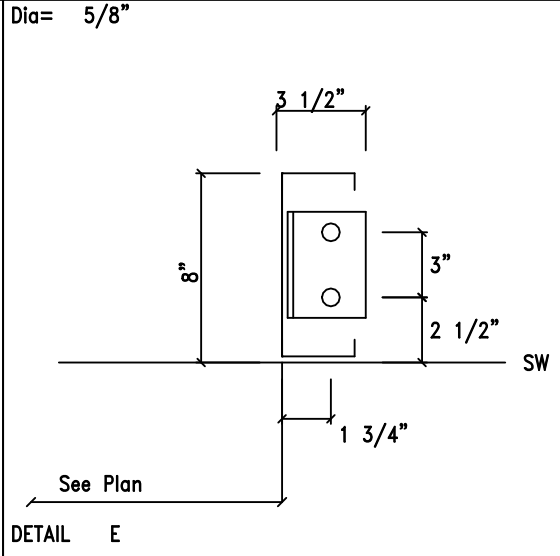
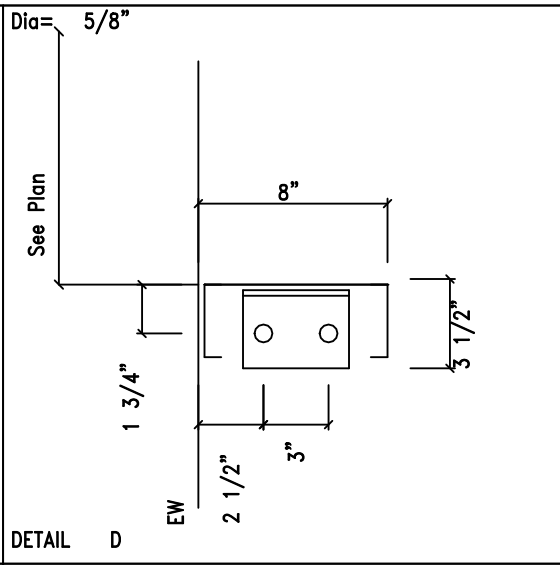
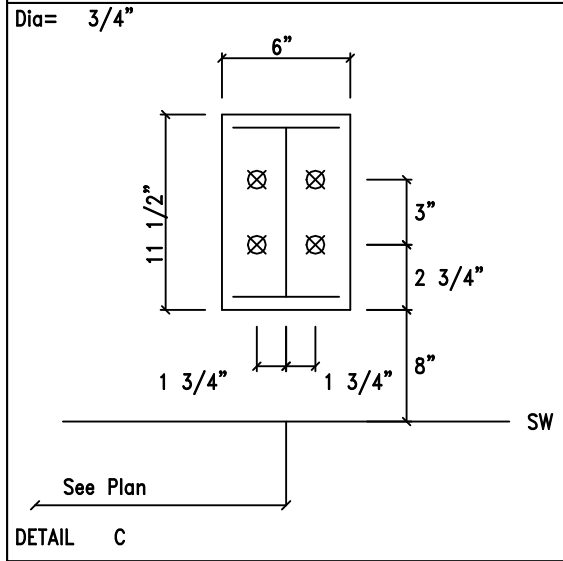
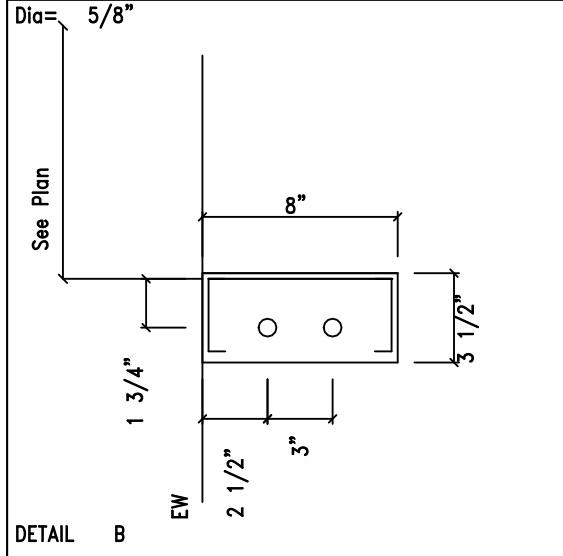
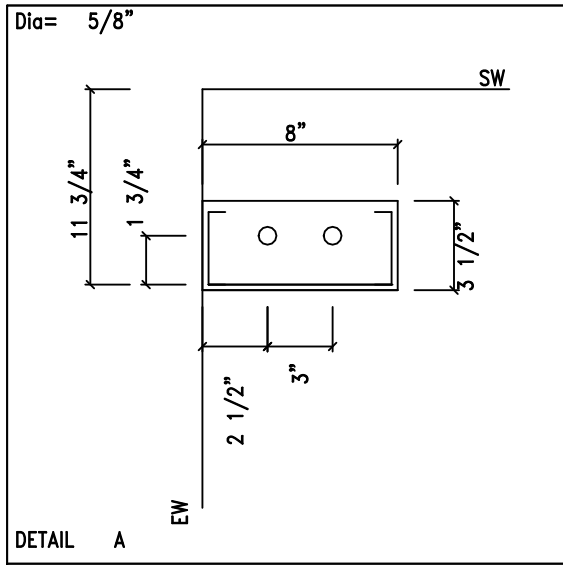
THE PROJECT DESIGNER IS NOT THE METAL BUILDING MANUFACTURER, THE METAL BUILDING DESIGNER, OR THE METAL BUILDING ENGINEER. THE ENGINEER WHOSE SEAL APPEARS ON THE METAL BUILDING PLANS IS A SPECIALTY ENGINEER AND NOT THE PROJECT DESIGNER OR THE PROJECT ENGINEER OF RECORD. THE ENGINEER WHOSE SEAL APPEARS ON THE METAL BUILDING PLANS DOES NOT HAVE FAMILIARITY WITH THE PHYSICAL JOBSITE LOCATION AND THEREFORE CANNOT BE IDENTIFIED AS, SERVE AS, OR QUALIFY AS THE PROJECT DESIGNER OR ENGINEER OF RECORD.



ANCHOR ROD PLAN
NOTE: All Base Plates @ 100'-0" (FINISH FLOOR)(UNLESS NOTED)

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION								
					OWNER OR PROJECT		Mike McCullers			
					JOBSITE LOCATION		100 SW Chase Ct.			
							Fort White FL 32038			
					CAD BY		DATE		SCALE	
					DET		10/ 9/25		N.T.S.	
					BUILDING SIZE		50.00' x 80.00' x 14.00'		JOB NO.	
							McCullersM		SHEET NO.	
									2 of 15	





Components & Cladding (Factored)						
Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Suction(psf) Panel	Pressure(psf) Member	Suction(psf) Panel
1			9.60	10.33	-11.37	-28.87
2			9.60	10.33	-15.58	-38.10
3	5.00	5.00	9.60	10.33	-15.58	-38.10
4	5.00	5.00	9.60	10.33	-26.26	-49.94
5			12.58	15.62	-13.91	-16.95
6	5.00		12.58	15.62	-14.88	-20.85

(+) wind towards surface
(-) wind away from surface

REVISIONS												
NO.	DATE	DESCRIPTION	BY	CK'D								
A	10/ 9/25	FOR PERMIT / CONSTRUCTION										

OWNER OR PROJECT		Mike McCullers	
JOBSITE LOCATION		100 SW Chase Ct.	
CAD BY		DET	
CK'D BY		10/ 9/25	
DATE		N.T.S.	
SCALE			

BUILDING SIZE		50.00' x 80.00' x 14.00'	
JOB NO.		McCullersM	
SHEET NO.		3 of 15	

LET'S GET BUILDING

GABLE

STEEL

ENDWALL COLUMN:

BASIC COLUMN REACTIONS (k)														
Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	D	0.2	0.1	1.1	0.3	0.0	-1.3	0.0	-1.6	0.0	-0.6	0.0	-0.8	-1.1
1	C	0.8	0.2	4.0	1.0	-1.9	-6.6	0.0	-1.3	-1.6	-5.1	0.0	-0.4	-3.2
1	B	0.8	0.2	4.0	1.0	0.0	-1.3	1.9	-6.6	0.0	-0.4	1.6	-5.1	-3.2
1	A	0.2	0.1	1.1	0.3	0.0	-1.6	0.0	-1.3	0.0	-0.8	0.0	-0.6	-1.1
Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	Seis Long Vert	-MIN_SNOW-- Horz	-MIN_SNOW-- Vert
1	D	1.2	0.0	-2.1	0.0	-1.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2
1	C	3.5	0.0	-3.3	-0.9	-3.4	-0.1	-0.1	0.0	0.1	0.0	0.0	0.0	0.6
1	B	3.5	0.9	-3.4	0.0	-3.3	0.0	0.1	0.1	-0.1	0.0	0.0	0.0	0.6
1	A	1.2	0.0	-1.4	0.0	-2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2
Frm Line	Col Line	E1UNB_SL_L-- Horz	E1UNB_SL_L-- Vert	E1UNB_SL_R-- Horz	E1UNB_SL_R-- Vert									
1	D	0.0	0.3	0.0	0.1									
1	C	0.0	1.0	0.0	0.4									
1	B	0.0	0.4	0.0	1.0									
1	A	0.0	0.1	0.0	0.3									

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
5	A	0.2	0.1	1.1	0.3	-1.9	-3.8	0.0	1.3	-1.6	-2.7	0.0	1.7	-1.1
5	B	0.8	0.2	4.0	1.0	0.0	-2.5	1.9	-5.9	0.0	-1.6	1.6	-4.3	-3.2
5	C	0.8	0.2	4.0	1.0	0.0	-2.8	0.0	-4.9	0.0	-1.6	0.0	-3.7	-3.2
5	D	0.2	0.1	1.1	0.3	0.0	-1.8	0.0	-1.3	0.0	-0.9	0.0	-0.6	-1.1
Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	Seis Long Vert	-MIN_SNOW-- Horz	-MIN_SNOW-- Vert
5	A	1.2	0.0	-0.7	-0.9	-2.6	-0.1	-0.2	0.0	0.2	0.0	0.0	0.0	0.2
5	B	3.5	0.9	-5.5	0.0	-1.4	0.0	0.2	0.1	-0.2	0.0	0.0	0.0	0.6
5	C	3.5	0.0	-2.6	0.0	-4.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.6
5	D	1.2	0.0	-1.4	0.0	-2.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2
Frm Line	Col Line	E2UNB_SL_L-- Horz	E2UNB_SL_L-- Vert	E2UNB_SL_R-- Horz	E2UNB_SL_R-- Vert									
5	A	0.0	0.3	0.0	0.1									
5	B	0.0	1.0	0.0	0.4									
5	C	0.0	0.4	0.0	1.0									
5	D	0.0	0.1	0.0	0.3									

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Column_Reactions(k)														Bolt(in)		Base_Plate(in)		Grout (in)	
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick							
1	D	6	0.7	-1.1	7	-0.6	-1.1	2	0.625	3.500	8.000	0.375	0.0						
1		1	0.0	1.4	6	0.7	-1.1												
1	C	8	2.1	-3.4	9	-1.9	-1.5	2	0.625	3.500	8.000	0.375	0.0						
1		1	0.0	5.0	8	2.1	-3.4												
1	B	10	2.1	-3.4	7	-1.9	-1.5	2	0.625	3.500	8.000	0.375	0.0						
1		1	0.0	5.0	10	2.1	-3.4												
1	A	11	0.7	-1.1	9	-0.6	-1.1	2	0.625	3.500	8.000	0.375	0.0						
1		1	0.0	1.4	11	0.7	-1.1												
5	A	8	0.7	-2.1	9	-0.6	-1.4	2	0.625	3.500	8.000	0.375	0.0						
5		12	0.5	1.9	8	0.7	-2.1												
5	B	10	2.1	-3.0	7	-1.9	-2.8	2	0.625	3.500	8.000	0.375	0.0						
5		1	0.0	5.0	10	2.1	-3.0												
5	C	10	2.1	-2.5	9	-1.9	-2.5	2	0.625	3.500	8.000	0.375	0.0						
5		1	0.0	5.0	10	2.1	-2.5												
5	D	11	0.7	-1.2	9	-0.6	-1.2	2	0.625	3.500	8.000	0.375	0.0						
5		1	0.0	1.4	11	0.7	-1.2												

ANCHOR BOLT SUMMARY

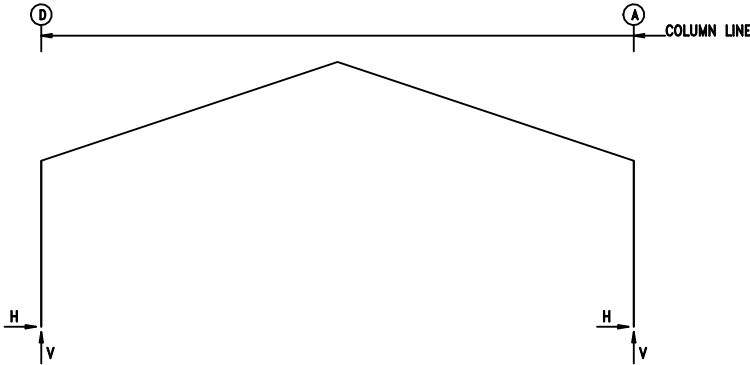
Qty	Locate	Dia (in)	Type	Bend Len (in)	Proj (in)
○ 8	Jamb	5/8"	A307		3.00
○ 16	Endwall	5/8"	A307		3.00
⊗ 24	Frame	3/4"	A307		3.00

BUILDING BRACING REACTIONS

Wall		Col Line		± Reactions(k)				Panel_Shear (lb/ft)	
Loc	Line			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Sels
L_EW	1	C,B		1.9	1.6	0.1	0.1		
F_SW	A	2,3		4.4	2.7	0.4	0.3		
R_EW	5	A,B		1.9	2.3	0.1	0.2		
B_SW	D	3,2		4.4	2.7	0.4	0.3		

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

FRAME LINES: 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR RODS, & BASE PLATES

Column_Reactions(k)														Bolt(in)		Base_Plate(in)		Grout (in)	
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick							
2*	D	1	4.0	8.4	2	-3.6	-5.5	4	0.750	6.000	11.50	0.500	0.0						
2*		4	0.0	-6.3															
2*	A	3	3.6	-5.5	1	-4.0	8.4	4	0.750	6.000	11.50	0.500	0.0						
2*		1	-4.0	8.4	5	0.0	-6.3												
2*	Frame lines:		2	3	4														

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column	-----Dead-----		---Collateral---		-----Live-----		-----Snow-----		---Wind_Left1---		---Wind_Right1---	
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	0.8	1.8	0.3	0.5	2.9	6.0	0.6	1.2	-6.7	-11.1	-0.9	-8.3
2*	A	-0.8	1.8	-0.3	0.5	-2.9	6.0	-0.6	1.2	0.9	-8.3	6.7	-11.1
Frame Line	Column	---Wind_Left2---		---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		Seismic_Right	
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	-6.2	-6.5	-0.3	-3.7	-0.7	-12.4	-2.1	-11.2	-0.1	-0.1	0.1	0.1
2*	A	0.3	-3.7	6.2	-6.5	2.1	-11.2	0.7	-12.4	-0.1	0.1	0.1	-0.1
Frame Line	Column	---Seismic_Long---		---MIN_SNOW---		F1UNB_SL_L--		F1UNB_SL_R--					
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert				
2*	D	0.0	-0.3	0.7	1.5	0.5	1.1	0.5	0.7				
2*	A	0.0	-0.3	-0.7	1.5	-0.5	0.7	-0.5	1.1				

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

Width (ft) = 50.0

Length (ft) = 80.0

Eave Height (ft) = 14.0/ 14.0

Roof Slope (rise/12) = 4.00/ 4.00

Roof Dead Load (psf) = 2.5

Wall Dead Load (psf) = 2.0

Left Endwall (psf) = 2.0

Right Endwall (psf) = 2.0

Front Sidewall (psf) = 2.0

Back Sidewall (psf) = 2.0

Roof Live Load (psf) = 20.0

Frame Live Load (psf) = 12.0

Collateral Load (psf) = 1.0

Snow Load (psf) = 5.0

Wind Speed (mph) = 130.0

Wind Code = FBC 23 (IBC 24)

Exposure = B

Closure = Enclosed

Internal Wind Coeff = -0.18, +0.18

Risk Category = II - Normal

Importance - Wind = 1.00

Importance - Seismic = 1.00

Seismic Design Category = B

Seismic Coeff (Sms) = 0.15
- Loading conditions are:

1 Dead+Collateral+Live

2 0.6Dead+0.6Wind_Left1

3 0.6Dead+0.6Wind_Right1

4 0.6Dead+0.6Wind_Long1L

5 0.6Dead+0.6Wind_Long2L

6 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L

7 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L

8 0.6Dead+0.6Wind_Left1+0.6Wind_Suction

9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

10 0.6Dead+0.6Wind_Right1+0.6Wind_Suction

11 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

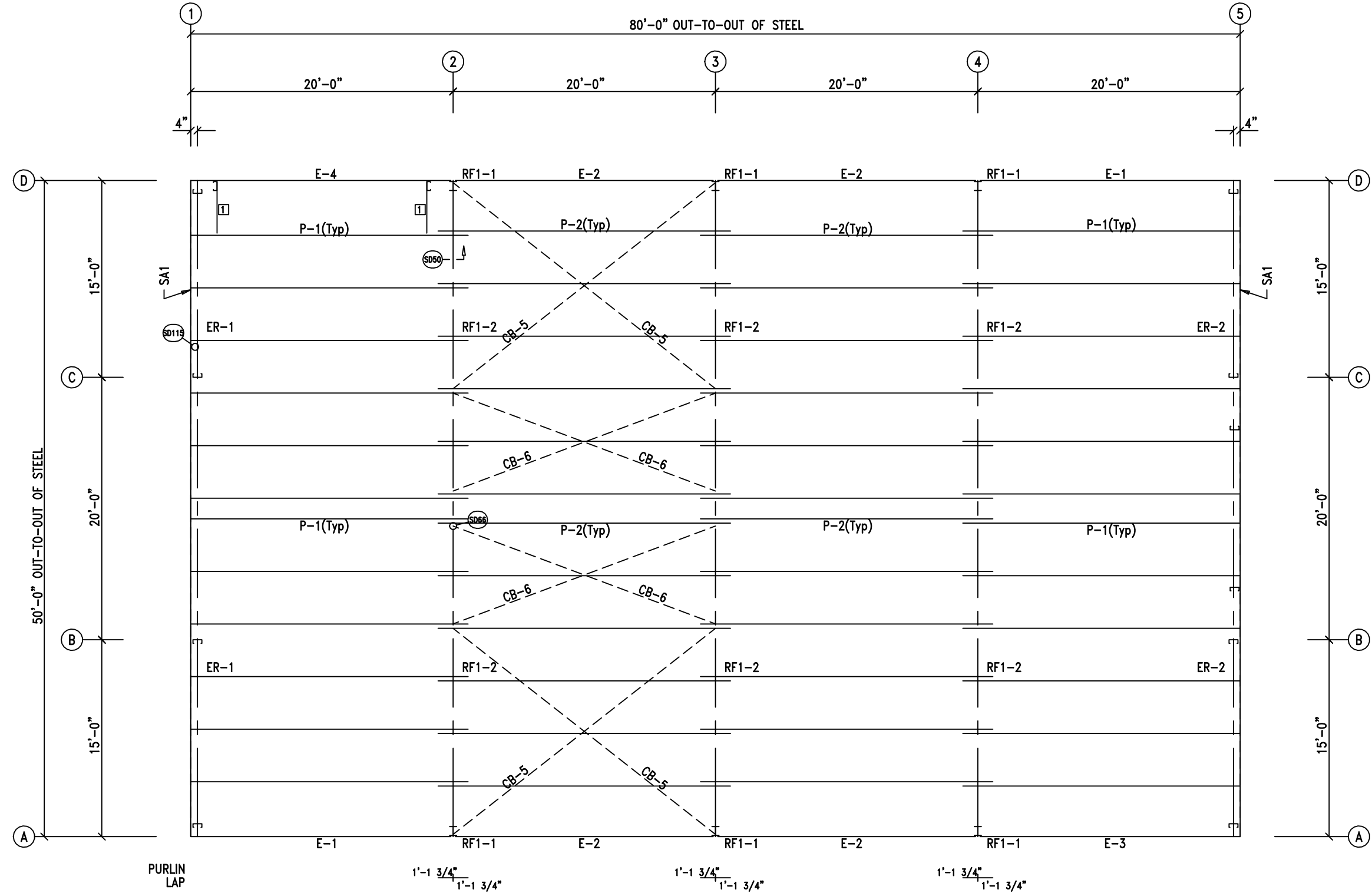
12 Dead+Collateral+0.75Live+0.45Wind_Right2+0.45Wind_Suction

REVISIONS												
NO.	DATE	DESCRIPTION	BY	CK'D								
A	10/ 9/25	FDR PERMIT / CONSTRUCTION										
						JOB SITE LOCATION			BUILDING SIZE			
						Fort White FL 32038			50.00' x 80.00' x 14.00'			
									GABLE STEEL LET'S GET BUILDING			
					CAD BY	DATE	SCALE	JOB NO.	SHEET NO.			
					DET	10/ 9/25	N.T.S.	McCullersM	4 of 15			

TRIM TABLE				
ROOF PLAN				
◇ ID	QUAN	PART	LENGTH	DETAIL
	4	FL-56	20'-2"	TD9

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z16	21'-1 1/2"
P-2	8X25Z16	22'-3 1/2"
E-1	8.50E14	19'-11 1/2"
E-2	8.50E14	19'-11 1/2"
E-3	8.50E14	19'-11 1/2"
E-4	8.50E14	19'-11 1/2"
CB-5	RD0500	24'-9"
CB-6	RD0500	21'-4"

CONNECTION PLATES	
ROOF PLAN	
□ ID	MARK/PART
1	JB-1



ROOF FRAMING PLAN

FL-56 : 20'-2"
(4)

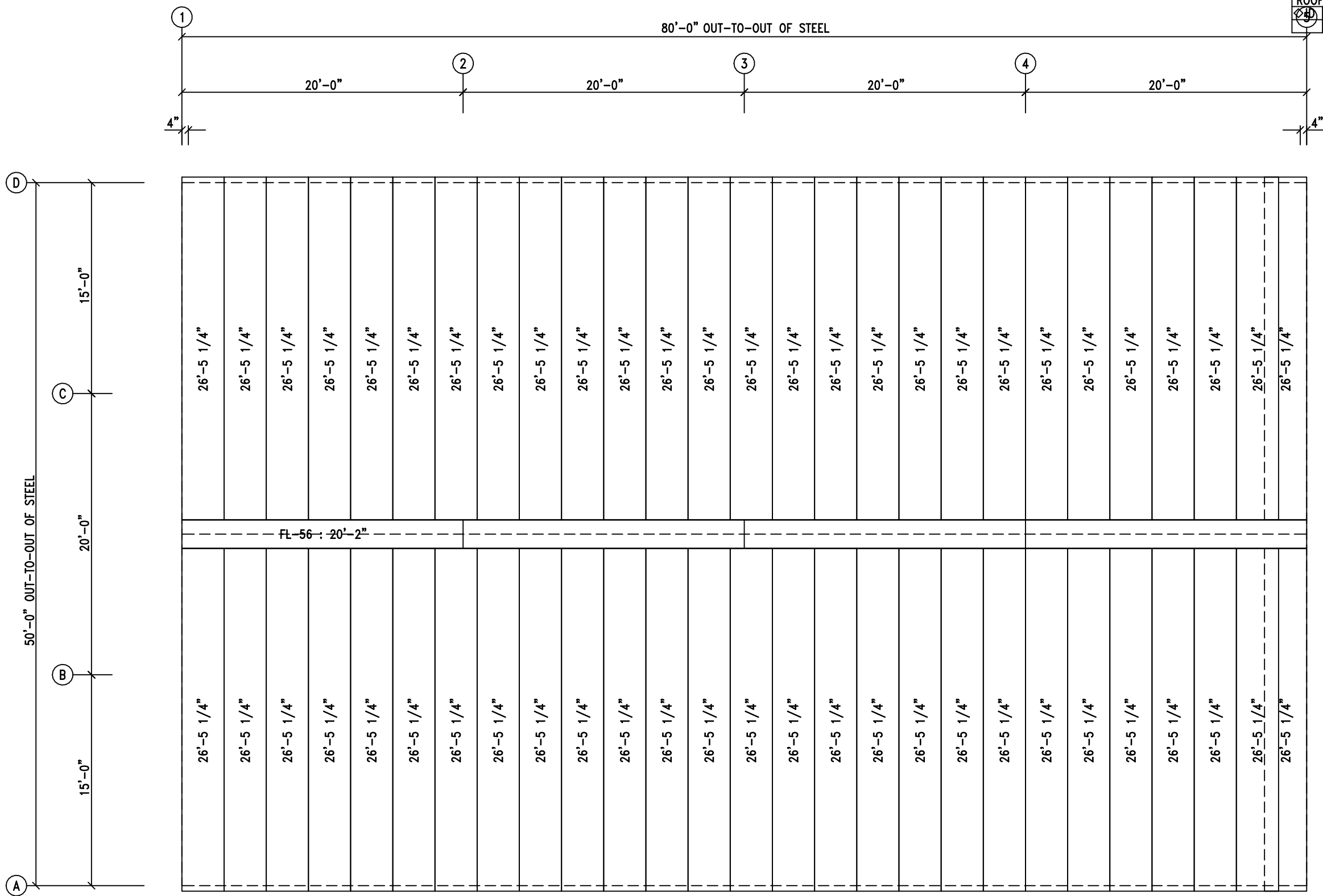
26'-5 1/4" (27)
26'-5 1/4" (27)

ROOF SHEETING

PANELS: 26 Ga. PBR
Galvalume

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION								
					OWNER OR PROJECT		Mike McCullers			
					JOBSITE LOCATION		100 SW Chase Ct.			
							Fort White FL 32038			
					CAD BY		DATE		SCALE	
					DET		10/ 9/25		N.T.S.	
					BUILDING SIZE		50.00' x 80.00' x 14.00'		JOB NO.	
							McCullersM		SHEET NO.	
									5 of 15	





TRIM TABLE				
QUAN	PART	LENGTH	DETAIL	
4	FL-56	20'-2"	TD9	

ROOF SHEETING PLAN
PANELS: 26 Ga. PBR - Galvalume

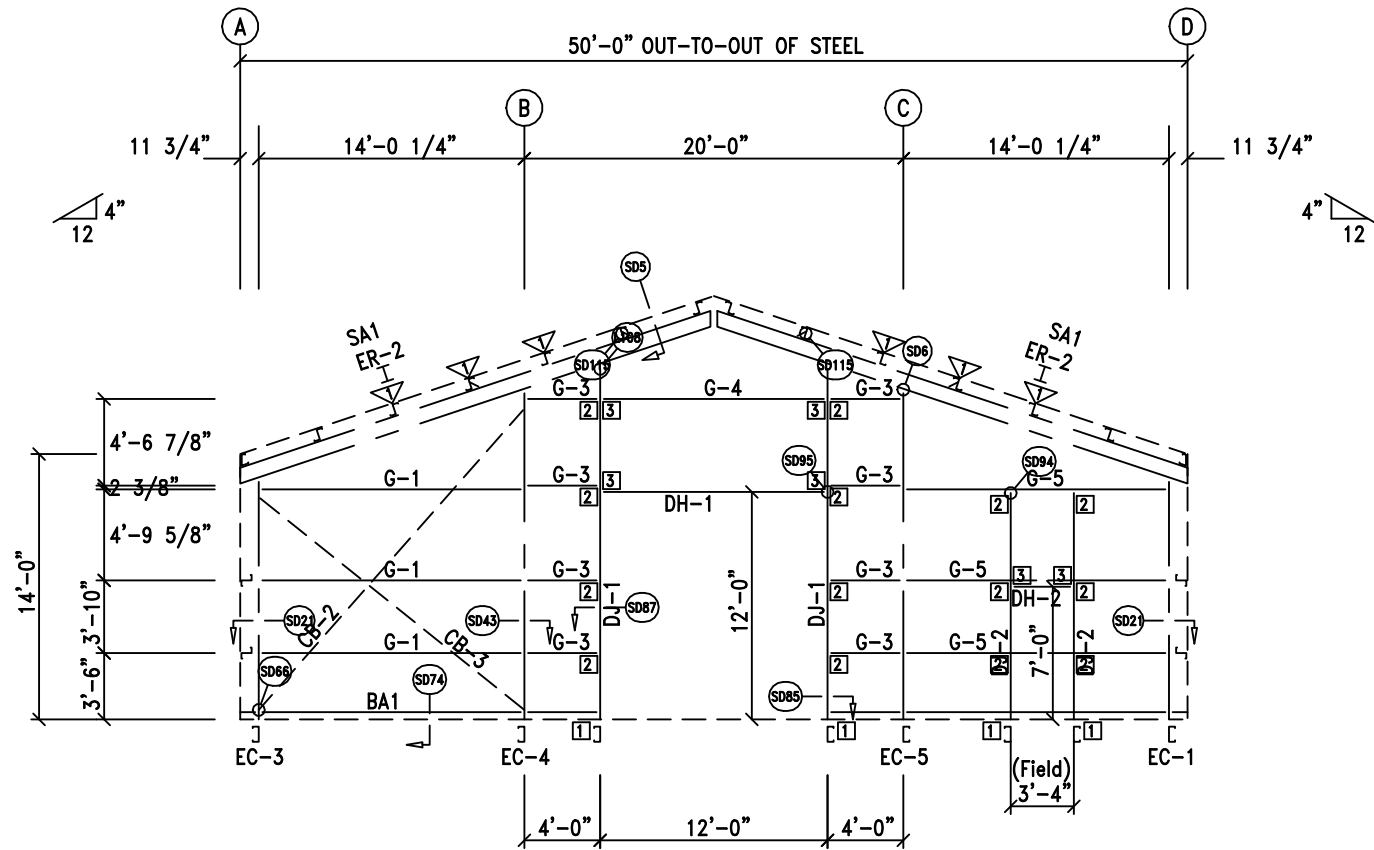
REVISIONS						
NO.	DATE	DESCRIPTION	BY	CK'D	OWNER OR PROJECT	
A	10/ 9/25	FOR PERMIT / CONSTRUCTION				
					JOBSITE LOCATION	Mike McCullers 100 SW Chase Ct. Fort White FL 32038
					CAD BY	DATE
					DET	10/ 9/25
					SCALE	N.T.S.
					BUILDING SIZE	50.00' x 80.00' x 14.00'
					JOB NO.	McCullersM
					SHEET NO.	6 of 15



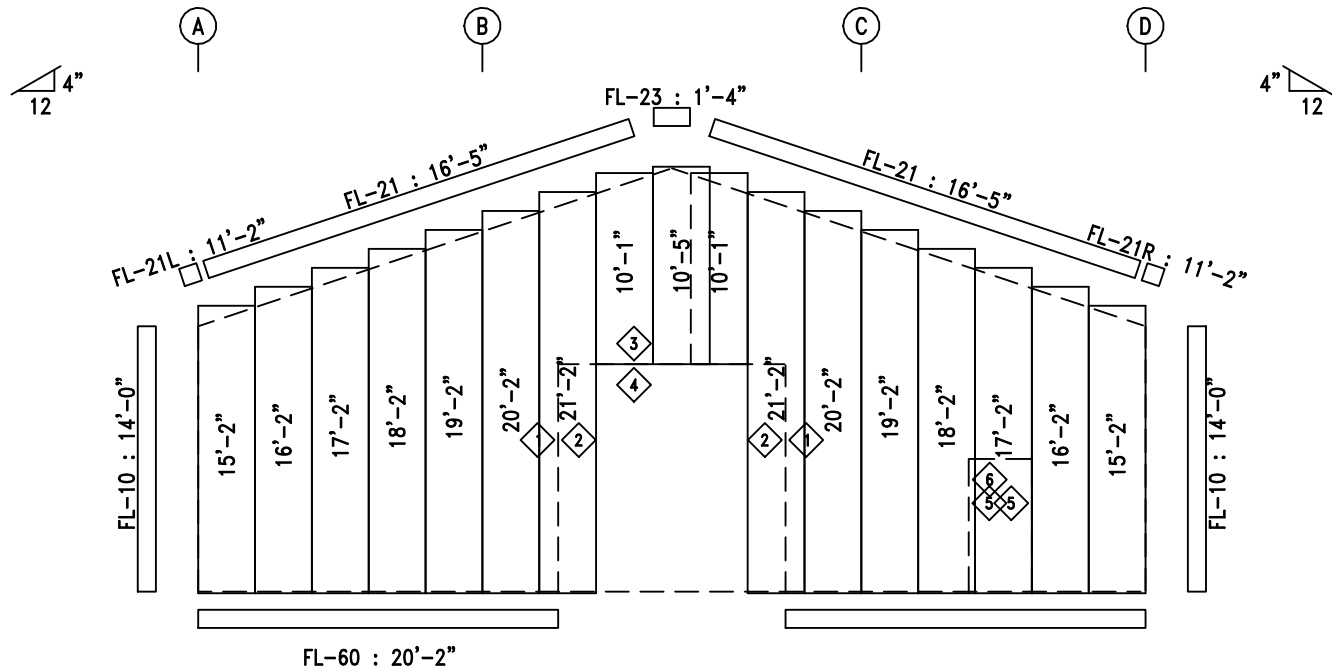
PANELS: 26 Ga. PBR – Ash Gray

FLANGE BRACE TABLE		
FRAME LINE 1		
▽ID	MARK	LENGTH
1	FB1A	2'-9 3/4"

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION			OWNER OR PROJECT	Mike McCullers				
					JOB SITE LOCATION	100 SW Chase Ct. Fort White FL 32038				
					CAD BY DET	CK'D BY	DATE 10/ 9/25	SCALE N.T.S.	BUILDING SIZE 50.00' x 80.00' x 14.00' OVER HUNG SIZE IS HOODING, PLATE RISE TO FLAND	SHEET NO. 8 of 15



ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. PBR - Ash Gray

TRIM TABLE				
FRAME LINE 5				
◇ ID	QUAN	PART	LENGTH	DETAIL
1	1	FL-21	16'-5"	TD35
	1	FL-21L	11'-2"	TD13
	1	FL-23	1'-4"	
	1	FL-21R	11'-2"	TD85
	1	FL-10	14'-0"	TD40
	2	FL-60	20'-2"	
2	1	FL-55	12'-2"	
3	1	FL-48	12'-3"	TD51
4	1	FL-55	12'-4"	
5	1	FL-52	12'-4"	TD52
6	1	FL-48	7'-3"	TD51
	1	FL-52	3'-8"	TD52

BOLT TABLE				
FRAME LINE 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-2/ER-2	8	A325	5/8"	1 3/4"
Columns/Raf	2	A325	5/8"	1 1/4"
Jamb	2	A307	1/2"	1"

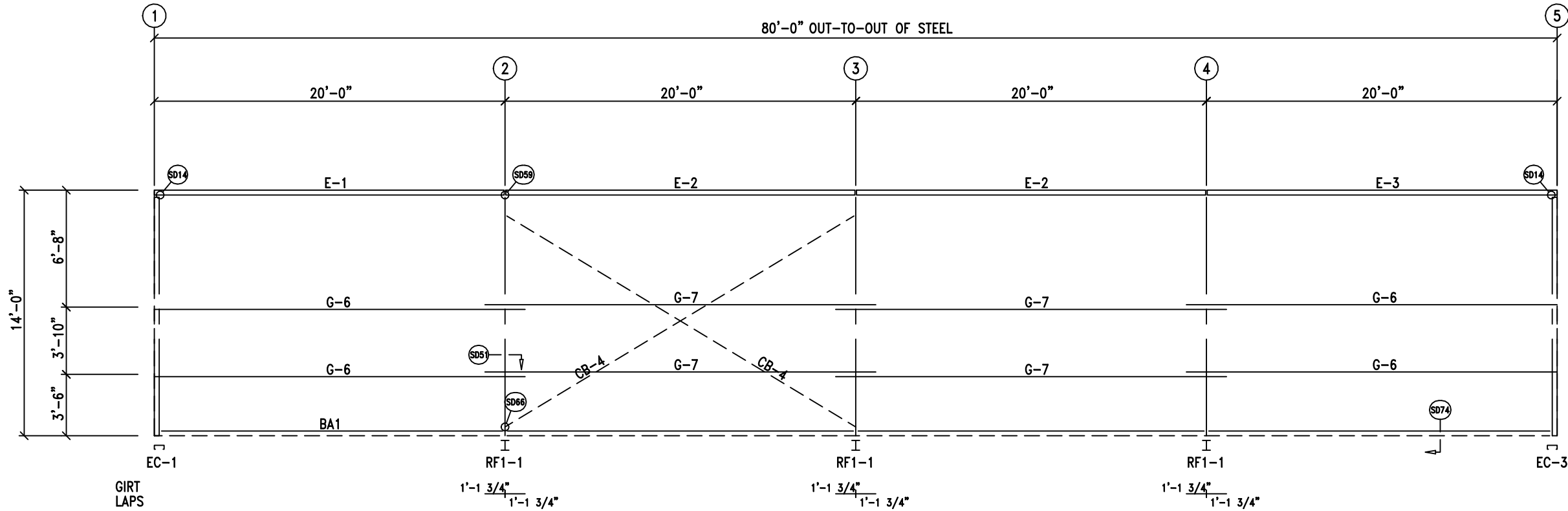
MEMBER TABLE		
FRAME LINE 5		
MARK	PART	LENGTH
EC-1	8X35C16	12'-10 1/8"
EC-3	8X35C16	12'-10 1/8"
EC-4	8X35C12	17'-6 1/4"
EC-5	8X35C12	17'-6 1/4"
ER-2	W8X10	26'-3 15/16"
DJ-1	8X35C14	18'-10"
DJ-2	8X35C16	11'-9 3/8"
DH-1	8X25C16	11'-11 1/2"
DH-2	8X25C16	3'-3 1/2"
G-1	8X25Z16	13'-8"
G-3	8X25Z16	3'-7 3/4"
G-4	8X25Z16	11'-11 1/2"
G-5	8X25Z14	13'-8"
CB-2	RD0500	21'-6"
CB-3	RD0500	18'-3"

FLANGE BRACE TABLE		
FRAME LINE 5		
▽ ID	MARK	LENGTH
1	FB1A	2'-9 3/4"

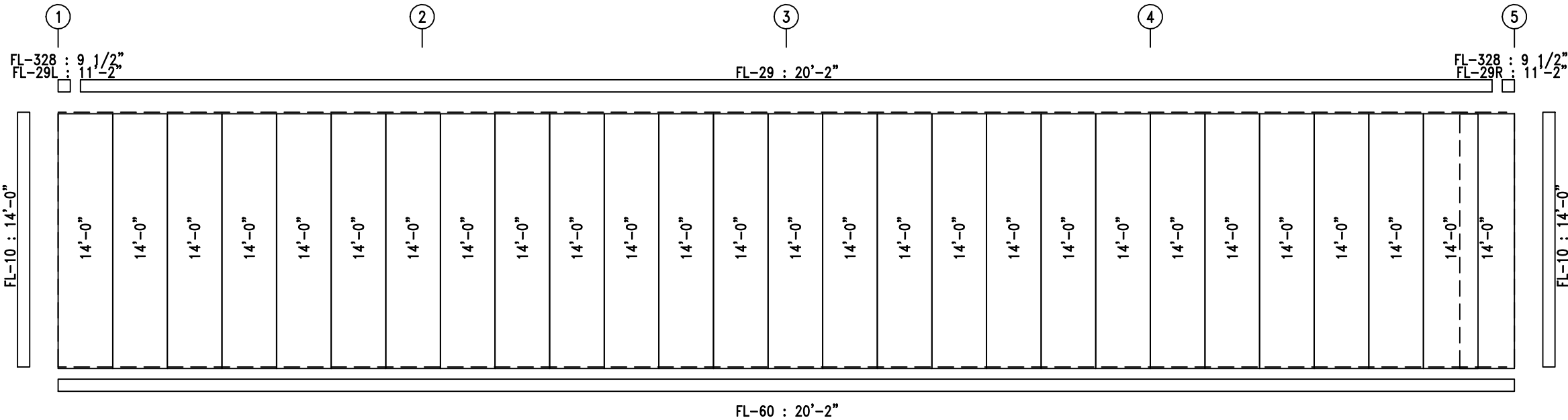
CONNECTION PLATES	
FRAME LINE 5	
□ ID	MARK/PART
1	CL-104
2	CL-103
3	CL-100

REVISIONS									
NO.	DATE	DESCRIPTION	BY	CK'D					
A	10/ 9/25	FOR PERMIT / CONSTRUCTION							
					OWNER OR PROJECT	Mike McCullers			
					JOB SITE LOCATION	100 SW Chase Ct. Fort White FL 32038			
					CAD BY	CK'D BY	DATE	SCALE	BUILDING SIZE
					DET		10/ 9/25	N.T.S.	50.00' x 80.00' x 14.00' (BUILDING SIZE IS NOMINAL, PLEASE REFER TO PLANS)
								JOB NO.	SHEET NO.
								McCullersM	9 of 15





SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. PBR - Ash Gray

TRIM TABLE FRAME LINE A				
◇ ID	QUAN	PART	LENGTH	DETAIL
	3	FL-29	20'-2"	TD16
	1	FL-29L	11'-2"	TD85
	1	FL-328L	9 1/2"	TRIM_324
	1	FL-29R	11'-2"	TRIM_322
	1	FL-328R	9 1/2"	
	1	FL-10	14'-0"	TD40
	4	FL-60	20'-2"	

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	8.50E14	19'-11 1/2"
E-2	8.50E14	19'-11 1/2"
E-3	8.50E14	19'-11 1/2"
G-6	8X25Z16	21'-1 1/2"
G-7	8X25Z16	22'-3 1/2"
CB-4	RD0500	23'-6"

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION								
					OWNER OR PROJECT					
					Mike McCullers					
					JOBSITE LOCATION					
					100 SW Chase Ct.					
					Fort White FL 32038					
					CAD BY					
					CK'D BY					
					DATE					
					10/ 9/25					
					SCALE					
					N.T.S.					

OWNER OR PROJECT

JOBSITE LOCATION

CAD BY

DET

Mike McCullers

100 SW Chase Ct.

Fort White FL 32038

DATE

10/ 9/25

SCALE

N.T.S.

BUILDING SIZE

50.00' x 80.00' x 14.00'

JOB NO.

McCullersM

SHEET NO.

11 of 15

GABLE

STEEL

LET'S GET BUILDING

BUILDING SIZE

50.00' x 80.00' x 14.00'

JOB NO.

McCullersM

SHEET NO.

11 of 15

Section Thru Rake w/Hot Rolled Rafter SD5	Cold Form Column to Hot Rolled Rafter SD6	Eave Strut to Hot Rolled Rafter Connection SD14	Section at "C" Corner Column SD21	Girt to "C" Endwall Column Connection SD43

Interior Bay Purlin Framing SD50	Interior Bay Girt Framing SD51	Eave Strut at Interior Column SD59

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION								

OWNER OR PROJECT	Mike McCullers		
JOB SITE LOCATION	100 SW Chase Ct.		
CAD BY	CK'D BY	DATE	SCALE
DET		10/ 9/25	N.T.S.

BUILDING SIZE	50.00' x 80.00' x 14.00'
JOB NO.	McCullersM
SHEET NO.	12 of 15

	Notes: - Rotate each zee as required to alternate between long and short flanges. - Lap connection bolts are omitted for clarity.	ZEE LAP DETAIL @ THE LONG AND SHORT FLANGES	DRAWING NO. SDG1		DRAWING NO. SDG2		DRAWING NO. TD1	NOTE: Screw patterns shown satisfy U.L. 90 requirements for roof.	DRAWING NO. TD7
FIELD LOCATED BRACING SLOT		ZEE LAPS		COLATERAL LOAD CONNECTIONS BY OTHERS		Fastener Location "PBR" Panel at Wall		Fastener Location "PBR" Panel at Roof	

	DRAWING NO. TD9		DRAWING NO. TD13		DRAWING NO. TD16		DRAWING NO. TD35		
Flat Ridge Detail - PBR For a Greater Than 3:12 Roof Slope		PBR Standard Trim Detail		Low Eave Detail - PBR Sculpture Eave Trim - Sheeted Wall		Rake Detail - PBR Standard Rake - Sheeted Wall			

REVISIONS										
NO.	DATE	DESCRIPTION	BY	CK'D						
A	10/ 9/25	FOR PERMIT / CONSTRUCTION								
					OWNER OR PROJECT					Mike McCullers
					JOBSITE LOCATION					100 SW Chase Ct.
										Fort White FL 32038
					CAD BY		DATE	SCALE	BUILDING SIZE	
					DET		10/ 9/25	N.T.S.	50.00' x 80.00' x 14.00'	
										JOB NO. McCullersM
										SHEET NO. 14 of 15



