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Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-5687
Job Description: Mike Tillotson	
Address: 361 sw ridge street , LAKE CITY, FL 32024	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res	IntelliVIEW Version: 20.01.01A JRef #: 1X662150008
Wind Standard: ASCE 7-16 Wind Speed (mph): 130 Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 3 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	162.21.1341.13353	A01
3	162.21.1341.21227	A03
5	GBLLETIN0118	

Item	Drawing Number	Truss
2	162.21.1341.17340	A02
4	A14015ENC160118	



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General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

lc = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

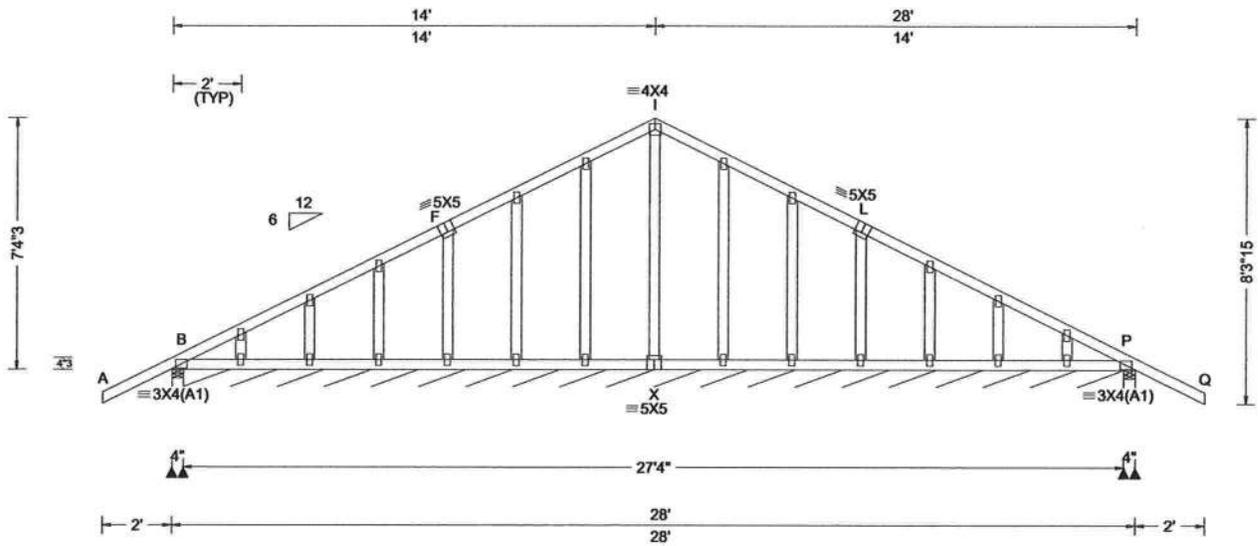
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): -0.001 AD 999 240 VERT(CL): -0.002 AD 999 180 HORZ(LL): 0.003 M - - HORZ(TL): 0.004 M - - Creep Factor: 2.0 Max TC CSI: 0.325 Max BC CSI: 0.066 Max Web CSI: 0.129 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs), or * = PLF Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 306 /- /- /177 /85 /231 B* 72 /- /- /40 /14 /- P 306 /- /- /221 /74 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 B Brg Width = 328 Min Req = - P Brg Width = 4.0 Min Req = 1.5 Bearings B, B, & P are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

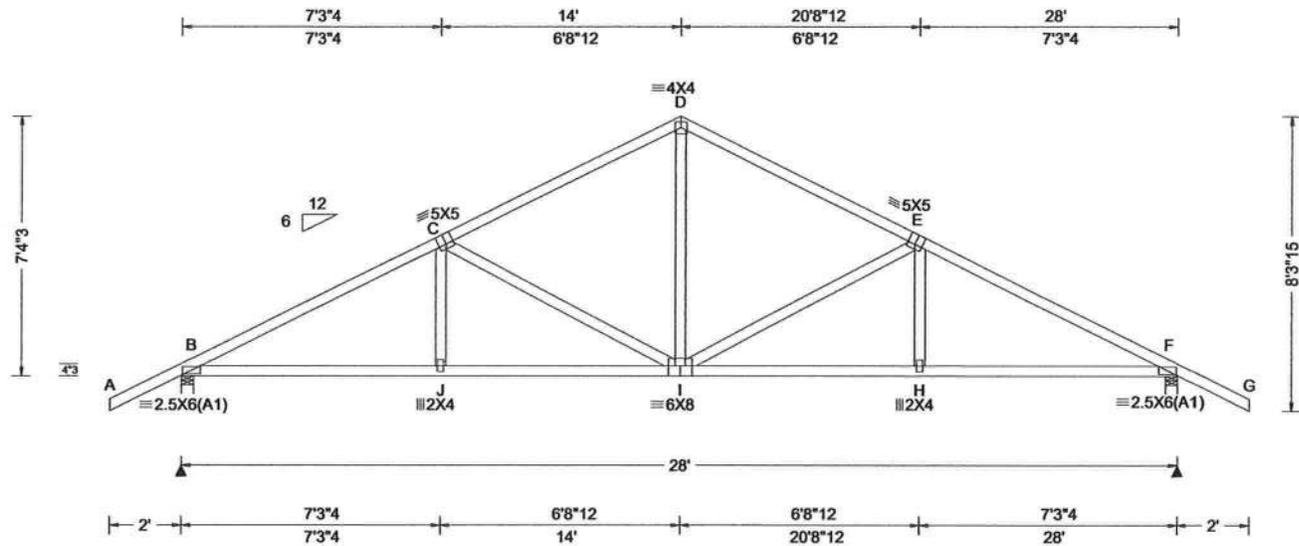
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
The overall height of this truss excluding overhang is 7-4-3.



FL REG# 278, Yoonhwak Kim, FL PE #86367
06/11/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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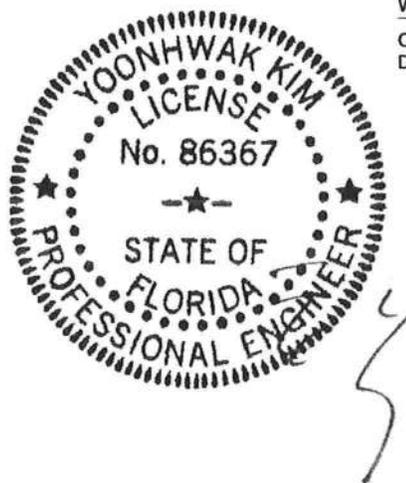


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.083 I 999 240 VERT(CL): 0.167 I 999 180 HORZ(LL): 0.036 H - - HORZ(TL): 0.073 H - - Creep Factor: 2.0 Max TC CSI: 0.489 Max BC CSI: 0.671 Max Web CSI: 0.680 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1283 /- /- /778 /232 /231 F 1283 /- /- /778 /232 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 F Brg Width = 4.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 789 -1962 D - E 677 -1365 C - D 677 -1365 E - F 788 -1962 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - J 1671 -557 I - H 1668 -538 J - I 1668 -559 H - F 1671 -537 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - I 351 -609 I - E 351 -609 D - I 731 -261
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

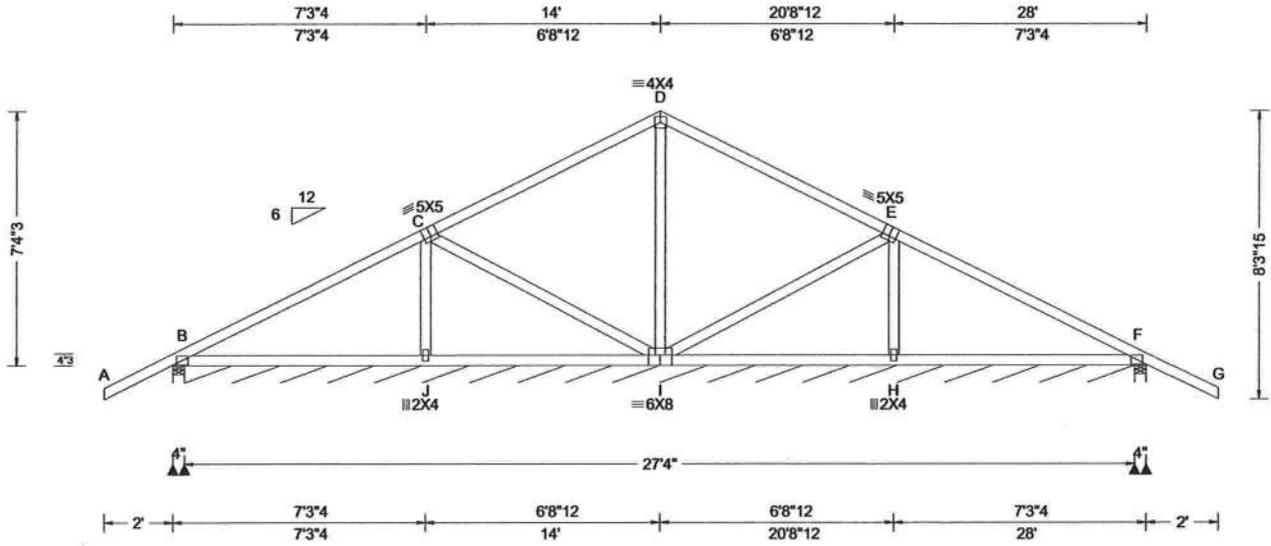
Additional Notes
The overall height of this truss excluding overhang is 7-4-3.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.010 J 999 240 VERT(CL): 0.021 J 999 180 HORZ(LL): 0.006 J - - HORZ(TL): 0.012 J - - Creep Factor: 2.0 Max TC CSI: 0.622 Max BC CSI: 0.446 Max Web CSI: 0.263 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs), or * = PLF <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>480</td> <td>-</td> <td>-</td> <td>/291</td> <td>/106</td> <td>/231</td> </tr> <tr> <td>B*</td> <td>59</td> <td>-</td> <td>-</td> <td>/32</td> <td>/9</td> <td>-</td> </tr> <tr> <td>F</td> <td>480</td> <td>-</td> <td>-</td> <td>/342</td> <td>/106</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 B Brg Width = 328 Min Req = - F Brg Width = 4.0 Min Req = 1.5 Bearings B, B, & F are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Loc</th> <th>Web</th> <th>Tens.Comp.</th> <th>Web</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>J - C</td> <td>180</td> <td>-380</td> <td>E - H</td> <td>178 -380</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	480	-	-	/291	/106	/231	B*	59	-	-	/32	/9	-	F	480	-	-	/342	/106	-	Loc	Web	Tens.Comp.	Web	Tens. Comp.	J - C	180	-380	E - H	178 -380
Loc	Gravity			Non-Gravity																																												
	R+	/R-	/Rh	/Rw	/U	/RL																																										
B	480	-	-	/291	/106	/231																																										
B*	59	-	-	/32	/9	-																																										
F	480	-	-	/342	/106	-																																										
Loc	Web	Tens.Comp.	Web	Tens. Comp.																																												
J - C	180	-380	E - H	178 -380																																												

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes
 All plates are 3X4(A1) except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 7-4-3.



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ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Gable Stud Reinforcement Detail

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing	2x4 Gable Vertical Species	Brace		(1) 2x4 'L' Brace												(2) 2x6 'L' Brace		(2) 2x6 'L' Brace	
		Grade	No Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
12" O.C.	SPF	#1 / #2	4' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF	Stud	4' 1"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
16" O.C.	SP	Standard	4' 1"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#1	4' 6"	7' 4"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
24" O.C.	SP	#2	4' 3"	7' 3"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	Stud	4' 2"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
36" O.C.	SPF	#1 / #2	4' 11"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF	Stud	4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
48" O.C.	SP	Standard	4' 8"	6' 11"	7' 5"	9' 3"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#1	5' 1"	8' 5"	8' 9"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
60" O.C.	SP	#2	4' 11"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	Stud	4' 9"	7' 4"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
72" O.C.	SPF	#1 / #2	5' 5"	9' 2"	9' 6"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF	Stud	5' 1"	9' 0"	9' 4"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
84" O.C.	SP	Standard	5' 1"	8' 0"	8' 6"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#1	5' 5"	9' 2"	9' 6"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
96" O.C.	SP	#2	5' 3"	8' 5"	9' 0"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	Stud	5' 3"	8' 5"	9' 0"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		

Bracing Group Species and Grades:

Group A:		Group B:	
Spruce-Pine-Fir	Hem-Fir	Southern Pine	Standard
#1 / #2	#2	#1	#2
Stud	Stud	Stud	Stud
#3	Standard	Standard	Standard

Douglas Fir-Larch

#3	Standard
Stud	Standard

Group A:

#1	#2
Stud	Stud
Standard	Standard

Group B:

#1	#2
Stud	Stud
Standard	Standard

Douglas Fir-Larch

#1	#2
Stud	Stud
Standard	Standard

Southern Pine

#1	#2
Stud	Stud
Standard	Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 psf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlofters with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For 1) 'L' brace space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For 2) 'L' brace space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

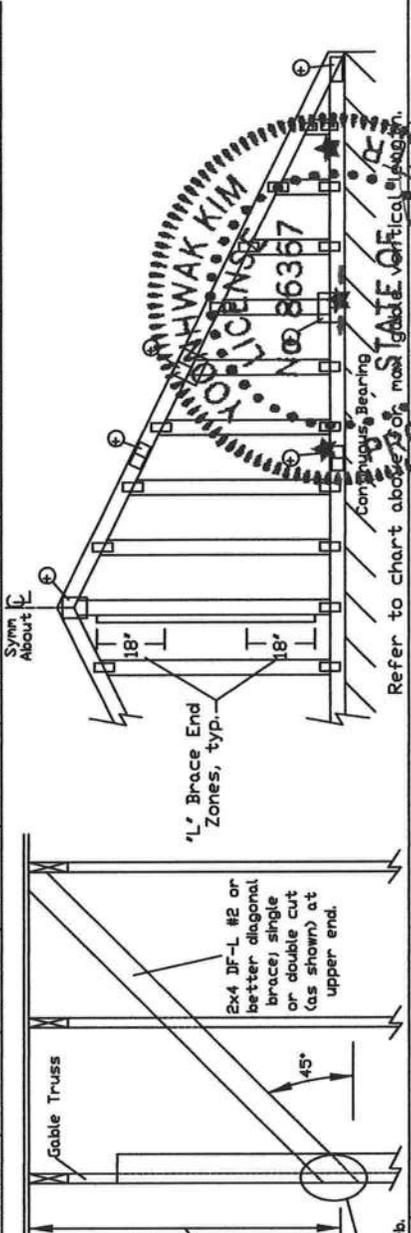
Gable Vertical Plate Sizes

Vertical Length	No. Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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DRWG	A14015ENC160118



Refer to chart above for max gable vertical spacing.

MAX. TOT. L.D. 60 PSF

MAX. SPACING 24.0'

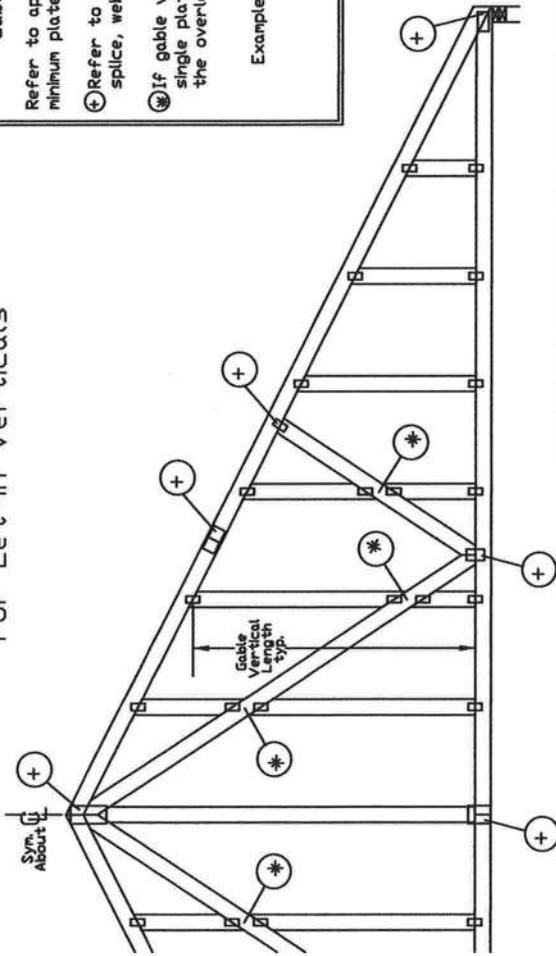
ALPINE
 AN ITW COMPANY

514 Earth City Expressway
 Suite 242
 Earth City, MO 63045

For more information see this job's general notes page and these web sites: www.alpineitw.com, www.tpi.com, www.sbcna.org, www.icb.com

1/26/18

Gable Detail For Let-in Verticals

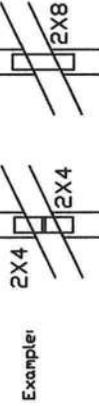


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

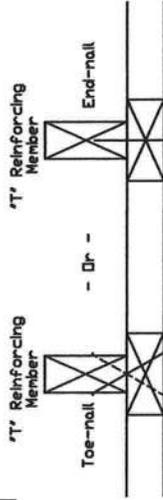
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Example:

'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.
'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x6	30 %
2x4	20 %

Example:

ASCE 7-10 Wnd Speed = 120 mph
Mean Roof Height = 30 ft, $K_{zt} = 1.00$
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

- End Driver Nails:
10d Common (0.148"x 3", min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

- Toenailed Nails:
10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

- A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

- A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A22015ENC100118, A24015ENC100118, A26015ENC100118, A28015ENC100118, A30015ENC100118, A32015ENC100118, A34015ENC100118, A36015ENC100118, A38015ENC100118, A40015ENC100118, A42015ENC100118, A44015ENC100118, A46015ENC100118, A48015ENC100118, A50015ENC100118, A52015ENC100118, A54015ENC100118, A56015ENC100118, A58015ENC100118, A60015ENC100118, A62015ENC100118, A64015ENC100118, A66015ENC100118, A68015ENC100118, A70015ENC100118, A72015ENC100118, A74015ENC100118, A76015ENC100118, A78015ENC100118, A80015ENC100118, A82015ENC100118, A84015ENC100118, A86015ENC100118, A88015ENC100118, A90015ENC100118, A92015ENC100118, A94015ENC100118, A96015ENC100118, A98015ENC100118, A100015ENC100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Guiding Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have property attached structural sheathing and bottom chord shall have property attached rigid ceiling. BCSI shall provide minimum lateral restraint of webs and shall have property attached rigid ceiling. BCSI shall provide minimum lateral restraint of webs and shall have property attached rigid ceiling. Refer to drawings 150A-2 for standard plate positions. Refer to drawings 150A-2 for standard plate positions.

Alpine, a division of ITV Building Components Group, Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing the drafter, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

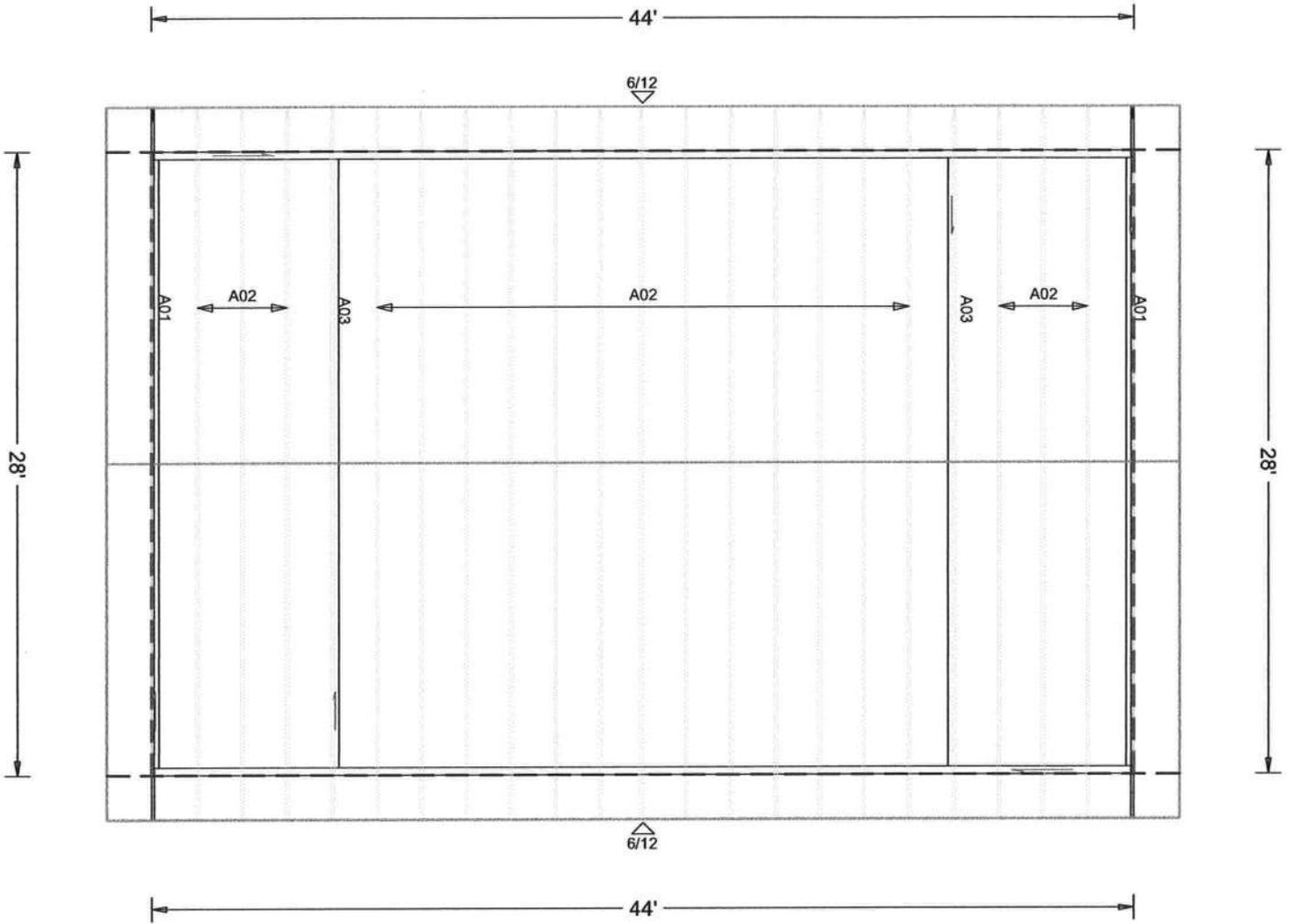
For more information see this job's general notes page and these web sites:
ALPINE: www.alpine.com TPI: www.tpi.com SBCA: www.sbcas.com ID: www.id.com

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118
MAX. TOT. LD. 60 PSF	
DUR. FAC. ANY	
MAX. SPACING 24.0"	



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JOB #: 21-5687

Job Name: Mike Tillotson
 Customer: Contractor
 Designer: Kelly Caudill
 ADDRESS: 361 sw ridge street
 SALESMAN: Fill in later
 : <Not Found>

JOB NO:
 21-5687

PAGE NO:
 1 OF 1