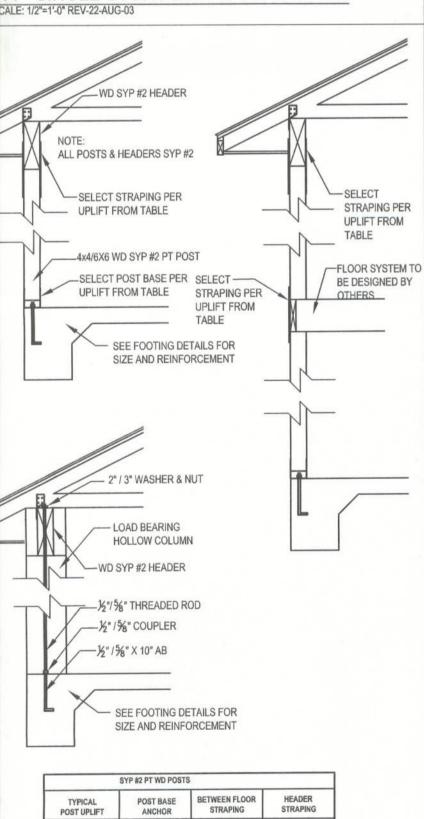
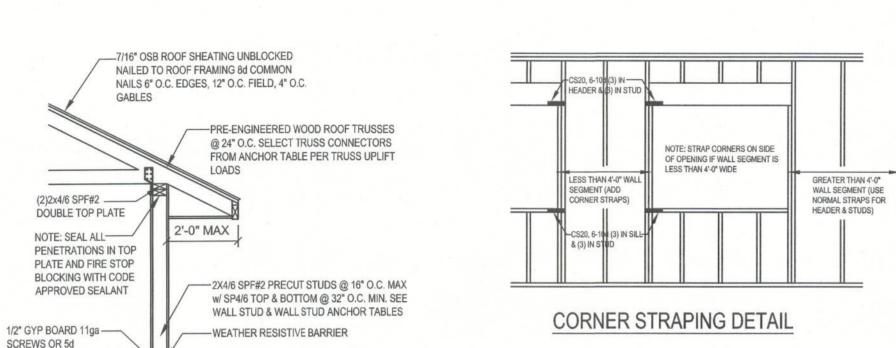


V1 - SINGLE STORY EXT. WALL SECTION



	SYP #2 PT WD POSTS			
TYPICAL	POST BASE	BETWEEN FLOOR	HEADER	
POST UPLIFT	ANCHOR	STRAPING	STRAPING	
555 LB	ABA44 W/ (6)-10d &	(2) LSTA21 W/	(2) LSTA21 W/	
	⅓" AB	(6)-10d EA.	(6)-10d EA.	
720 LB	ABA66 W/ (8)-16d	(2) LSTA21 W/	(2) LSTA21 W/	
	&%" AB	(8)-10d EA.	(8)-10d EA.	
2200 LB	ABU44 W/ (12)-16d,	(2) LSTA21 W/	(2) LSTA21 W	
	(2) 1/2 BOLTS & 1/4 "AB	(16)-10d EA.	(16)-10d EA.	
2300 LB	ABU66 W/ (12)-16d,	(2) LSTA21 W/	(2) LSTA21 W/	
	(2)½ BOLTS & %*AB	(16)-10d EA.	(16)-10d EA.	
	HOLLOW COLUMN			
1500 LB	½" X 10" AB ATT THREADED ROD W THRU COLUMN & WASHER &			
2300 LB	%" X 10" AB ATT THREADED ROD W THRU COLUMN & WASHER &			

W12 - PORCH HEADER ANCHORS SCALE: N.T.S. REV-18-JUL-03



-5/16" HARDIEPANEL FULLY BLOCKED.

—4" CONCRETE FLOOR SLAB REINFORCED

ON CHAIRS @ 2" DEPTH OR FIBER MESH

CONC., 6-MIL POLY VAPOR BARRIER W/ 6'

SPACING

32" O.C.

16" O.C.

TS20 NAILED

2X4 OUTRIGGER @ 48" OC.-

BLOCKING REQUIRED BETWEEN OUTRIGGERS 2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -

LAPS SEALED W/ POLY TAPE OVER

TERMITE TREATED & COMPACTED FILL

W/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED

6d @ 6" O.C. (FLASH OR SEAL ALL

JOINTS & PENETRATIONS)

COOLER NAILS 7" O.C.

2X4/6 PT PINE SOLE PLATE

ANCHORED W/ 1/2"x10"----

TABLE & WITHIN 8" FROM

CORNERS EACH WAY

ANCHOR BOLTS W/ 2x2x.140"

STEEL WASHER SPACING PER

FOUNDATION

RUSS UPLIFT & MAX ANCHOR BOLT

48" O.C.

48" O.C.

32" O.C.

24" O.C.

TTI31 W/ 5/8" X 7"

WEDGE ANCHOR

W43 - SINGLE STORY EXT. WALL SECTION wi

HARDIEPANEL SIDING (SIDES & REAR ONLY)

2X4 SCAB CONT. TOP-

4 - 10d NAILS OR 4 -

ALL CONNECTIONS

2X4 SCAB IF VERT.

CONT. 2X4X8' #2 SYP -

LATERAL BRACE @ 48"

2X4 BLOCKING @ 48" OC.

-PORTAL FRAME WALL

-1/2" SHEATHING FILLER

CS20 WRAP OVER TOP

PLATE ON OUTSIDE OF

SHEATHING (7) -10d IN

HEADER ON EACH SIDE

STUD, (7) - 10d IN

8d @ 3" O.C.

SECTION DETAIL

W37 - GARAGE DOOR SHEARWALL DETAIL

SEGMENT TYP.

BETWEEN GABLE AND

FIRST TRUSS.

(2) 16d NAIL @ 24" O.C.-

8d NAIL @ 6" O.C. IN ALL-

CORNER FRAMING DETAIL

FASTEN PLATE TO

NAILS @ 3" O.C.

SCALE: 1/2"=1'-0" REV-07-NOV-03

HEADER WITH TWO

ROWS OF 16d SINKKER

PANEL EDGES NOT PART

OF PORTAL FRAME

WEB IS NOT PRESENT

.131"x 3.25" TYPICAL AT

CHORD@ 8' FROM

то воттом

GABLE

NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6

MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT

GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.

10'-0" WALL HIGHT

770 LB

950 LB

1270 LB

1500 LB

SCALE: 1/2"=1'-0" REV-22-AUG-03

WALL STUD TABLE

2 x 4 @ 16" OC TO 11'-9" WALL HEIGHT

2 x 4 @ 12" OC TO 13'-0" WALL HEIGHT

2 x 6 @ 16" OC TO 18'-10" WALL HEIGH

2 x 6 @ 12" OC TO 20.0 ' WALL HEIGHT

STUD ANCHOR TABLE

SP4 / SP6 SPACING

32" O.C.

32" O.C.

16" O.C.

16" O.C.

DETAILS

EDGE, 10" O.C. FIELD

Note: Walls sheathed with 5/16" Hardiepanel siding · Install 5/16" Hardiepanel siding in accordance with Florida Building Commission Product Approval, PTID 889 T ner405.pdf. Corrosion resistant fasteners with 3/8" min edge distance and 2" min comer distance and spacing as shown on detail. All board edges supported by framing. Max height to width ratio is 2:1 or strap corners of opening for 3.5:1 aspect ratio.

· Maximum wind speed for 5/16" Hardiepanel siding is 120 mph for 6d at 6"OC per NER-405. Table 2a. · Shear capacity for walls with SPF#2 studs 16"OC sheathed with 5/16" Hardiepanel siding (6d at 6"OC edges, 12"OC field = 157 plf, 6d at 6"OC = 200 plf; 6d at 4"OC = 233 plf) per NER-405, Table 3.

SLAB TO DRAIN

1'-0"

F2 - PORCH SLAB

SCALE: 1/2"=1'-0" REV-22-AUG-03

- 6"x6"W1.4XW1.4 W.W.M. PLACED @ 2"

DEPTH ON CHAIRS OR FIBERMESH

@ 28 DAYS

W/ POLY TAPE

(1) #5 CONTINUOUS

PROCTOR

-----4" CONCRETE SLAB 2500 - PSI @ 28 DAYS

-6"x6"W1.4XW1.4 W.W.M. PLACED @ 2"

_(1) #5 CONTINUOUS

TO MIN 95% MOD. PROCTOR

W/ POLY TAPE

F12 - NON - BEARING STEP FOOTING

SHEATHING

2X4 BARGE

.131 X 3 🗗 -

CONT 2X4 SCAB FROM TOP

O BOTTOM CHORD @

VERTICAL IF HIGHER THAN

X-BRACING (PROVIDE

ADDITIONAL 2X4'5 @

48 TO FORM AN"

CS20 WRAP OVER TOP -

PLATE ON OUTSIDE OF

SHEATHING (7) -10d IN

HEADER ON EACH SIDE

CORNER NEXT TO

FULLY SHEATHED

FRAMING DETAIL

WALL SEE CORNER

MINIMUM WIDTH

16" FOR 8'-0" WALL

20" FOR 10'-0" WALL

24" FOR 12'-0" WALL

(6:1 ASPECT RATIO)

STUD, (7) - 10d IN

NOTE: ALL MEMBERS SHALL BES

2X4 X-BRACE @ 6'-0" OC.

W10 - TYPICAL GABLE END (X-BRACING)

RAFTER CONT.

-7/16" STRUCTURAL ROOF

HURRICANE

-CLIP H-2.5 OR

EQUAL 48" OC.

TOP CHORD

OF GABLE END

TRUSS DROP 3

TOE NAIL TRUSS TO

-BOTTOM CHORD OF

GABLE END TRUSS

-(2) - 2X4 TOP PLATE

- SIMPSON LSTA

24 @ 48" OC.

SEE DETAIL W1

HEADER

-MINIMUM (2) 2X12 WITH 1/2" OSB SPACER.

O.C. ALONG EACH EDGE

FASTEN PLATE TO HEADER WITH TWO

ROWS OF 16d SINKKER NAILS @ 3" O.C.

COMMON NAILS IN 3" GRID PATTERN IN

HEADER AND 3" O.C. IN ALL FRAMING

(STUD, AND SILLS) TYP.

-1/2"x10" ANCHOR BOLTS W 2x2x.140" STEEL WASHER

FOUNDATION

FRONT ELEVATION

FASTEN SHEATHING TO HEADER WITH 8d

-FOR A PANEL SPLICE (IF NEEDED), PANEL EDGES SHALL

BE BLOCKED, AND OCCUR WITHIN 24" OF MID-HEIGHT.

REQUIRED. IF 2X_BLOCKING IS USED, THE 2X 'S MUST

BE NAILED TOGETHER WITH (3) 16d SINKERS.

ONE ROW OF TYP. SHEATHING-TO-FRAMING NAILING IS

SPLIT WOOD

BUILT UP WITH 16d COMMON NAILS @ 16"

COM @8" OC.

DOUBLE PLATE w/ 16d

- 6 MIL VAPOR BARRIER W/ 6" LAPS SEALED

- TERMITE TREATED FILL, EA. LIFT COMPACTED

DEPTH ON CHAIRS OR FIBERMESH

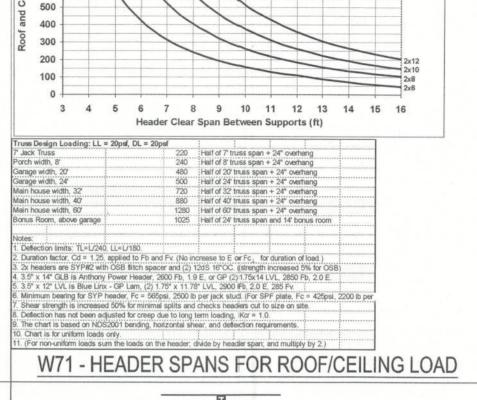
-4" CONCRETE SLAB 2500 - PSI

- 6 MIL VAPOR BARRIER W/ 6" LAPS SEALED

TERMITE TREATED FILL, EA, LIFT

COMPACTED TO MIN 95% MOD.

HOUSE SLAB



PRE-ENGINEERED WOOD ROOF

TRUSSES @ 24" O.C. SELECT_

TRUSS CONNECTORS FROM

ANCHOR TABLE PER TRUSS

SPACING PER TABLE

4" CONCRETE SLAB 2500 - -

(2) #5 CONTINUOUS-

400 LB

600 LB

1000 LB

ROOF TRUSS-

2X4 / 2X6 STUDS -

AT 16" OC. SPF #2

4" CONCRETE SLAB 2500 - -

(2) #5 CONTINUOUS-

PSI AT 28 DAYS

48" O.C.

48" O.C.

32* O.C.

WEDGE ANCHOR

SCALE: 1/2"=1'-0" REV-22-AUG-0:

1/2" X 7" WEDGE —

ANCHORS @ 48" O.C.

F4 - INTERIOR BEARING FOOTING

- 7/16" OSB WALL

SHEATHING

8d COMMON

NAILS 6" O.C.

EDGE, 12"O.C.

FIELD

W25 - INTERIOR SHEAR WALL DETAIL

N5 - TRUSS UPLIFT CONNECTOR TABLE REV-25-AUG-03

FULL'Y BLOCKED.

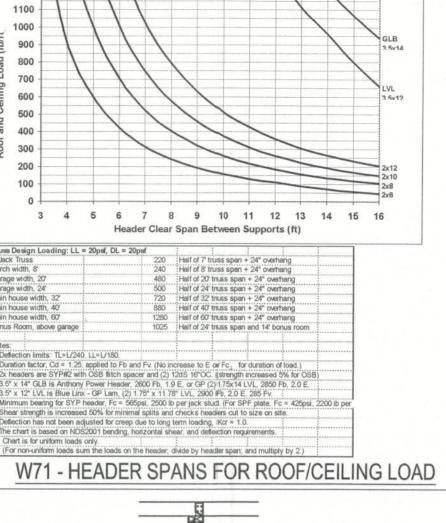
48" O.C.

32" O.C.

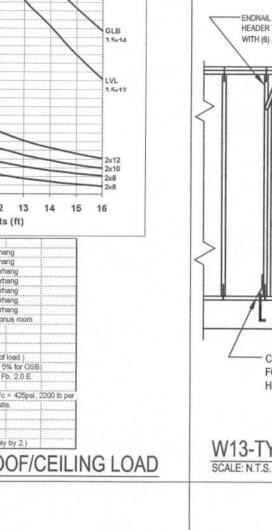
16° O.C.

SHEATHING

UPLIFT LOADS



Header Span vs Load



- 2X4 / 2X6 STUDS AT 16" OC. SPF #2

SP4 / SP6 SPACING PER TABLE

NOTE: SP2 TOP & SP1 BOTTOM

-6"X6" W1.4XW1.4 W.W.M.

PLACED AT 2" DEPTH ON

CHAIRS OR FIBERMESH

6 MIL VAPOR BARRIER

WITH 6" LAPS SEALED

WITH POLY TAPE

TERMITE TREATED FILL

EACH LIFT COMPACTED

TO MIN. 95% MOD.

PROCTOR

H2.5A

H10

HTS20

STUD PACK

OSB WALL

-STRAP STUDS W/ SP4/

-6"X6" W1.4XW1.4 W.W.M.

PLACED AT 2" DEPTH ON

CHAIRS OR FIBERMESH

6 MIL VAPOR BARRIER

WITH 6" LAPS SEALED

WITH POLY TAPE

ETERMITE TREATED FILL

TO MIN. 95% MOD.

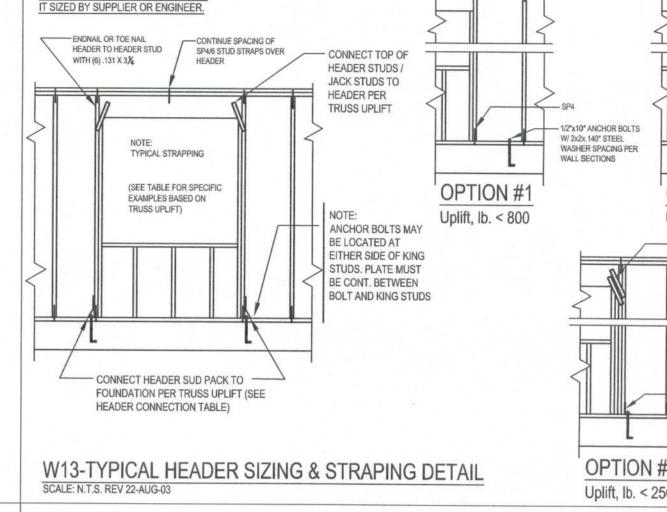
PROCTOR

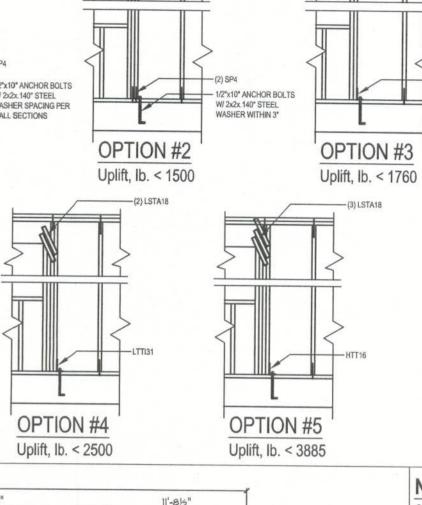
EACH LIFT COMPACTED

SP6 @ 48" O.C.

ALTERNATE FOR SP4/6

STRAP STUDS TOP AND BOTTOM W/





oad Bearing Header Sizing Methods (BY BUILDER Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5. Use supplier published data or Southern pine span tables. 3. For engineered lumber beams have suppliers engineer size beam. Jack Studs and King Studs (BY BUILDER) Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5. 5. Use one jack stud for every 3000 lb vertical load. 6. Total king plus jack studs = studs needed to be there if no opening was there.

Header Uplift Connections (BY BUILDER) Calculate the uplift at each end of the header by summing the moments of all truss uplifts and

dividing by the length of the header.

Option #	Uplift, lb.	Top Connector		Bottom Connector		
#1	< 800	End nail or toe nail w/6131"x3.25"		SP4, 6-10dx1 ½"	690	
#2	< 1500	LSTA12, 10-10d	755	(2) SP4, 6-10dx11/2", 1/2" AB	1380	
#3	< 1750	LSTA18, 14-10d		LTT20B, 10-16d ½" AB	1750	
#4	< 2500	(2) LSTA18, 14-10d		LTTI31, 18-10d 1/2 "x10" AB	2185	
#5	< 3885	(3) LSTA18, 14-10d	3480	HTT16, 18-16d, % "x10" AB	4175	

2001, TABLE 2308.3A r Spans For Exterior Bearing Walls orting Roof+Ceiling (20psf+20psf)		Header Spans (ft-in)		Building Width / Truss Span (ft)			
				20		28	
				NJ	Span	NJ	Span
		2-2x4	3-6	1	3-2	1	2-10
		2-2x6	5-5	1	4-8	1	4-2
S: NJ = Number of jack studs d to support each end. Building s measured perpendicular to		2-2x8	6-10	1	5-11	2	5-4
		2-2x10	8-5	2	7-3	2	6-6
		2-2x12	9-9	2	8-5	2	7-6
ge. For widths between those	Header Sizes	3-2x8	8-4	1	7-5	1	6-8
spans may be interpolated. are based on uniform loads on		3-2x10	10-6	1	9-1	2	8-2
are pased on uniform loads on		3-2x12	12-2	2	10-7	2	9-5
		4-2x8	9-2	1	8-4	1	7-6
		4-2x10	11-8	1	10-6	1	9-5
		4-2x12	14-1	1	12-2	2	10-11

N2-GENERAL NOTES

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'C = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

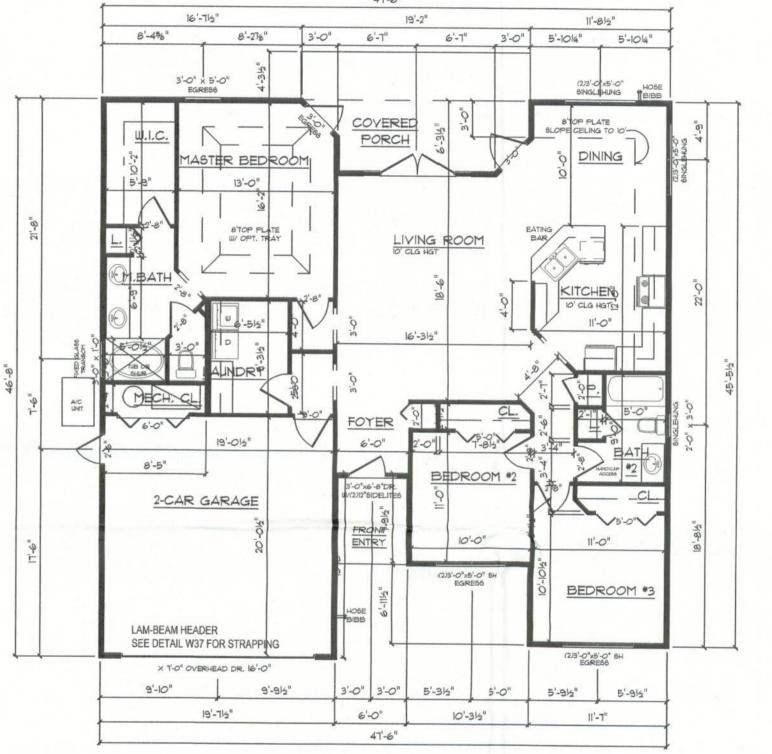
CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAPS SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23

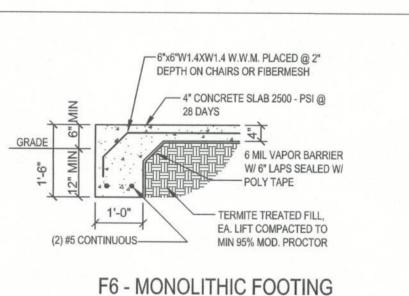
STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, OR EXAMPLE NOT ENDORSEMENT, AN FOLIVALENT SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64";

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.



w/6-.131"x3.25"



2-CAR GARAGE & MECH.CL = 430 S.F TOTAL AREA UNDER ROOF = 1974 6.F.

of the wind load engineer's scope of work.

velocity and design pressures.

capacity unless visual observation or soils test proves otherwise

truss-to-truss connections, and load reactions for all bearing locations.

connection, call the wind load engineer immediately.

A/C LIVING AREA

FRONT ENTRY

* FLOOR PLAN *

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the

UILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part

Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing

Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind

Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path

Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details,

structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not

engineer of record for the trusses and did not design the trusses or delegate to the truss designer.

= 1500 S.F.

= 44 S.F.

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disosway P.E.

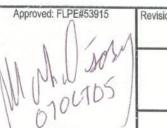
POB 868, Lake City, FL 32056 Phone: (386) 754-5419 Fax: (386) 754-6749 Email: windloadengineer@bellsouth.net

Location: Lot #26 Country Side Estates S/D Lake City, Florida

The Samuel Model

Builder: Bryan Zecher Construction

Designer: Teena Ruffo



Sheet S-1 of 2 Sheets Windload Engineering

N4-WIND LOAD DESIGN DATA (Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with

SCALE: 1/2"=1'-0" REV-22-AUG-03

>10% slope.)

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter	
320	455	H3	4-8d	4-8d	
245	350	H5A	3-8d	3-8d	
535	600	H2.5A	5-8d	5-8d	
620	720	H10	6-10dx:1½"	6-10dx 1½"	
850	990	LTS12	8-8dx11 ½"	8-8dx 11/5"	
1245	1450	HTS20	10-10d or12-10dx11/2"	10-10d or 12-10dx 1½"	
1265	1470	H16, H16-2	10-10dix1½"	2-10dx 1½"	
1785	2050	LGT2	14-10dl Sinker	16-16d Sinker	
3655	4200	MGT	%" Thid. Rod	22-10d	
SPF	SYP	Strap Connector	Tio One Member	To Other Member	
760	885	SP4	6-10dx:1½"	N/A	
865	1005	CS20	9-8d orr 7-10d	9-8d or 7-10d	
1085	1265	LSTA18-24	7-10d	7-10d	
1170	1360	SPH4	12-10dix1½"	N/A	
1420	1650	CS16	14-8d cor 11-10d	14-8d or 11-10d	
SPF	SYP	Column Anchor	TTo Foundation	To Column / Truss	
1160	1350	LTT19	%"x 166" AB	8-16d Sinkers	
1985	2310	LTTI31	%"x 166" AB	18-10dx 1½"	
2385	2775	HD2A	%"x 165" AB	2-5%" Bolts	
3590	4175	HTT16	%"x 16}" AB	18-16d	
1975	2300	ABU66	%"x 163" AB	12-16d	

can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's

Installation instructions must be followed to achieve rated loads.. All connections exposed directly to the weather shall be not dipped

galvanized after fabrication. Loads are increased for wind duration. Strap uplift may be reduced proportionally to number of nails.

ee spec sheet for alternate nail sizes (10d=84*16d, 10dx11/4*=80*10d, 10d=12d=16d sinker), SPF=86*SYP

Building Category Internal pressure Coefficient N/A (Enclosed Building not in the high velocity hurricane zone Building not in the wind-borne debris region Mean Roof Height < 30 ftRoof Angle 10-45 degrees Components And Cladding Wind Pressures (FBC Table1606.2 B&C **Total Shear Wall Segments** all wall segments with full height han 1: 3.5 (plus special shear wall 7A & 3.17B with table 3.17E ustment for type II shear wall (or equivalent calculation) REV-27-Jun-0

Basic Wind Speed Wind Exposure Wind Importance Factor

* Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions; including interior bearing walls. Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads. DOCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or Zone Effective Wind Area (ft2) architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and 4 21.8 -23.6 18.5 -20.4 is the responsibility off the builder. 5 21.8 -29.1 18.5 -22.6 COPYRIGHTS AND IPROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves ts common law copyrights and property right in these instruments of service. This document is 4"min for 8'-0"H wall 2'-10"min for 10'-0"H wall not to be reproduced, altered or copied in any form or manner without first the express written permission and conse Transverse Longitudinal dequired 45.1' 28.7' of Mark Disosway. ated dimensions surpercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not All exterior walls are type II shear walls proceed without clariffication. CTUAL SHEAR WALL length is the total WINDLOAD ENGINEER: Mark Disosway, PE No.53915 sheathing and width to height ratio gre gments if noted.) REQUIRED SHEAR ALL length is from WFCM-2001, table

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 rind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. nis drawing is not valid for construction unless raised seal is affixed.

REV-06-OCT-03

Job # 508103a