DATE 02/06/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000024113

APPLICANT LINDA RODER PHONE 386.752.2281

ADDRESS 387 SW KEMP CT LAKE CITY FL 32024

OWNER SPARKS CONTRACTORS,INC. PHONE 386.623.0575

ADDRESS 145 SW MORNING GLORY DR LAKE CITY FL 32024

CONTRACTOR JOSH SPARKS PHONE 386.623.057

LOCATION OF PROPERTY 90-W TO C-341,TL TO HOPE HENRY RD,TR TO MORNING GLORY DR,TR AND IT'S THE 2ND LOT ON THE L.

TYPE DEVELOPMENT SF/UTILITY ESTIMATED COST OF CONSTRUCTION 109950.00

HEATED FLOOR AREA 2199.00 TOTAL AREA 2948.00 HEIGHT 23.20 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 15-4S-16-03023-502 SUBDIVISION ROLLING MEADOWS

LOT 2 BLOCK PHASE UNIT TOTAL ACRES 0.54

000000962		CBC1252260		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
18"X32'MITERED	05-1273-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: MFE 101.0' PER PLAT. ELEVATION LETTER REQUIRED. NOC ON FILE.

Check # or Cash 3171

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 550.00 CERTIFICATION FEE $ 14.74 SURCHARGE FEE $ 14.74

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 679.48

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

2 Rolling Meadows

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only: Application # 0512-58 Date Received 12-27-05 By LH Permit # 96 2/2413
Application Approved by - Zoning Official BLK Date 28.12.05 Plans Examiner OK-STH Date 2-5-06
Flood Zone X Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Den.
Comments M.F.E. 101 Per PLAT Elevation Letter Required
CH#3171

Applicants Name Linda Roder / Melanie Roder Phone 752-2281
Address 387 S.W. Kemp Ct. Lake City FL 32024
Owners Name Sparks Contractors Inc. Phone 623-0575
911 Address 145 S.W. Morning Glory Dr. Lake City FL 32024
Contractors Name Sparks Contractors Inc. - Josh Sparks Phone 623-0575
Address P.O. Box 1479 Lake City FL 32056
Fee Simple Owner Name & Address NA
Bonding Co. Name & Address NA
Architect/Engineer Name & Address Ben Sparks / Mark Disosway
Mortgage Lenders Name & Address NA

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy
Property ID Number 15-4S-16E 03023-502 Estimated Cost of Construction 171,000
Subdivision Name Rolling Meadows Lot 2 Block ____ Unit ____ Phase ____
Driving Directions 90 W. L on 341 (Sisters welcome) R at Hope Henry, R on S.W. Morning Glory Dr. 2nd Lot on L

Type of Construction SFD Number of Existing Dwellings on Property 0
Total Acreage ____ Lot Size .54 Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 35' Side 23.5' Side 23.5' Rear 133'
Total Building Height 23'-2" Number of Stories 1 Heated Floor Area 2199 Roof Pitch 8-12
Porch 257 Garage 492 TOTAL 2948

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

Sworn to (or affirmed) and subscribed before me
this 5 day of December 20 05.
Personally known ✓ or Produced Identification ____

Contractor Signature
Contractors License Number CBC 1252260
Competency Card Number ____
NOTARY STAMP/SEAL

Notary Signature

*24113

Inst:2006002352 Date:02/01/2006 Time:10:46
MK DC,P.DeWitt Cason,Columbia County B:1072 P:1679

Permit No.
Tax Folio No. 15-4S-16-03023-502 & 15-4S-16-03023-504

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:

   Lots 2 and 4, ROLLING MEADOWS, a subdivision according to the plat thereof, as recorded in Plat Book 8, pages 45 and 46, public records of Columbia County, Florida.

2. General description of improvements: New Home Construction

3. Owner information:
   a) Name and Address: Sparks Contractors, Inc.
   P.O.Box 1479
   Lake City, FL 32056

   c) Interest in property: 100%

4. Contractor (name and address):
   Sparks Contractors, Inc.
   P.O. Box 1479
   Lake City, FL 32056

   a) Phone number: 386-755-9314
   b) Fax number: 386-755-7156

5. Surety Information:
   a) Name and Address:

   b) Phone number:
   c) Amount of bond:

6. Lender (name and address):
   Peoples State Bank
   350 SW Main Blvd.
   Lake City, FL 32025

   a) Phone number: 386-754-0002
   b) Fax Number 386-754-0031

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:

   William J. Haley, Esquire
   Brannon, Brown, Haley & Bullock, P.A.
   P.O. Box 1029
   Lake City, FL 32056-1029

   a) Phone number: 386-752-3213

8. Expiration date of Notice of Commencement: _____, 200___.

Sparks Contractors, Inc.

By: _____
Jacob D. Sparks, President
P.O. Box 1479
Lake City, FL 32056

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to and subscribed before me this 30th day of January, 2006, A.D. by Jacob D. Sparks, as President of Sparks Contractors, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me or whom produced FL Drivers License as identification.

Debbie G. Moore
Notary Public - State of Florida

Debbie G. Moore
Commission # DD400475
Expires March 16, 2009

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000962

DATE 02/06/2006 PARCEL ID # 15-4S-16-03023-502

APPLICANT LINDA RODER PHONE 386.752.2281

ADDRESS 387 SW KEMP CT LAKE CITY FL 32024

OWNER SPARKS CONTRACTORS INC. PHONE 386.623.0575

ADDRESS 145 SW MORNING GLORY DRIVE LAKE CITY FL 32024

CONTRACTOR JOSH SPARKS PHONE 386.623.0575

LOCATION OF PROPERTY 90-W TO C-341-TL TO HOPE HENRY RD,TR TO MORNING GLORY DR.

2ND LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLING MEADOWS 2

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

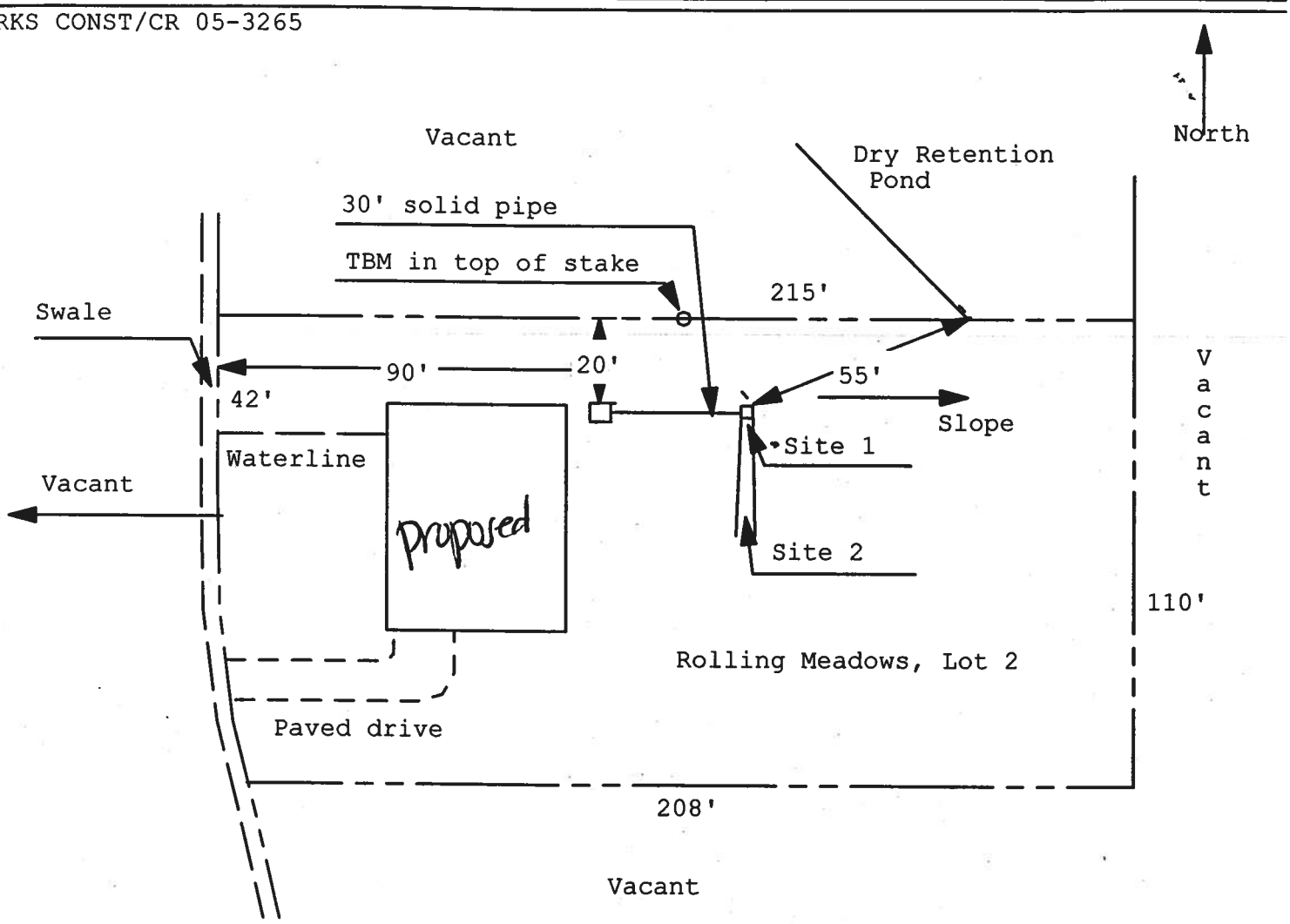
Amount Paid 25.00

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-1273N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

SPARKS CONST/CR 05-3265



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 12/9/05

Plan Approved X **Not Approved** ____ **Date** 12.30.05

By Sallie Braddy. ESI. COLUMBIA **CPHU**

Notes: _____

This instrument prepared by:
William J. Haley, Esquire
Brannon, Brown,
Haley & Bullock, P. A.
P. O. Box 1029
Lake City, FL 32056-1029

Inst:200503197O Date:12/27/2005 Time:11:48
Doc Stamp-Deed : 390.60
DC,P.DeWitt Cason,Columbia County B:1069 P:903

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 22 day of December, 2006, between **JERRY COOK,** a married man, who does not reside on the property, but who resides at 314 Cannon Creek Drive, Lake City, Florida 32055, hereinafter referred to as Grantor, and **SPARKS CONTRACTORS, INC,** a Florida corporation, having a mailing address of 162 SW Country Court, Lake City FL 32024, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of $10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in **Columbia** County, Florida, to-wit:

Lot(s) 2 and 4, **ROLLING MEADOWS,** a subdivision according to the plat thereof, as recorded in Plat Book 8, pages 45 and 46, public records of Columbia County, Florida.

PARCEL NO. Part of 15-4S-16-03023-005

SUBJECT TO: Taxes and special assessments for the year 2006 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

SUBJECT TO: That certain Mortgage to Peoples State Bank dated November 16, 2005, recorded November 17, 2005 in Official Records Book 1065, page 1012, public records of Columbia County, Florida

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Inst:2005031970 Date:12/27/2005 Time:11:48
Doc Stamp-Deed : 390.60
DC,P.DeWitt Cason,Columbia County B:1069 P:904

Signed, sealed and delivered
in the presence of:

Print Name: Josh Sparks

Jerry Cook

Print Name:

STATE OF FLORIDA
COUNTY OF COLUMBIA

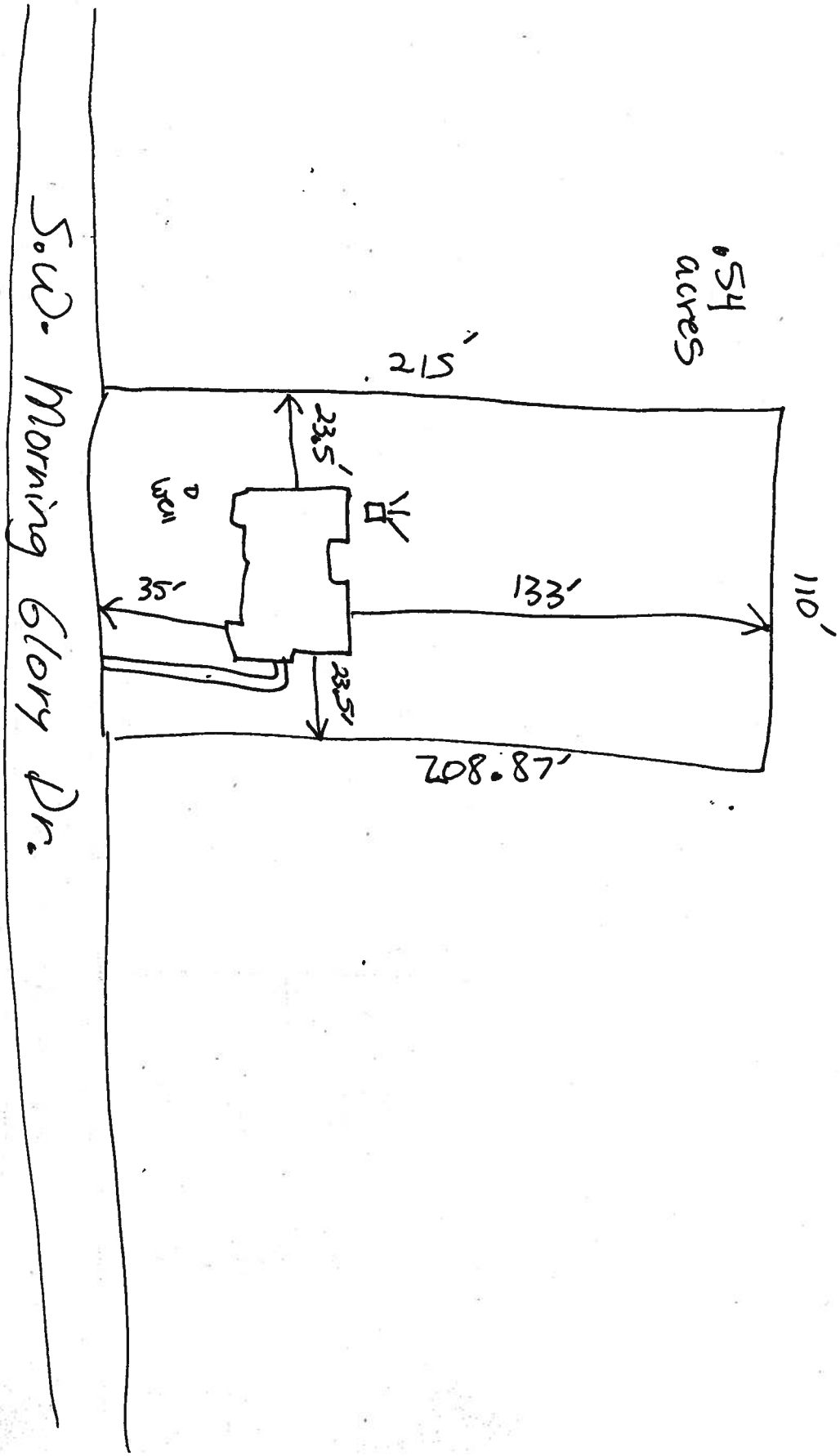
The foregoing instrument was acknowledged before me this 23 day of December, 2005, by Jerry Cook, who is personally known to me or whom produced _____, as identification.

Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

Notary Public, State of Florida

F:\WPDOCS\22032\00014\00151663.WPD

Site Plan
Lot 2 Rolling Meadows



FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**511234SparksConstruction**	Builder:	**Josh Sparks**
Address:	**Lot: 2, Sub: Rolling Meadows, Plat:**	Permitting Office:	Columbia
City, State:	**, FL**	Permit Number:	
Owner:	**Spec House**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 48.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 12.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	2199 ft²	c. N/A	
7. Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 229.0 ft²	13. Heating systems	
b. SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 229.0 ft²	a. Electric Heat Pump	Cap: 48.0 kBtu/hr
8. Floor types			HSPF: 7.80
a. Slab-On-Grade Edge Insulation	R=0.0, 236.0(p) ft	b. N/A	
b. N/A			
c. N/A		c. N/A	
9. Wall types			
a. Frame, Wood, Exterior	R=13.0, 1428.0 ft²	14. Hot water systems	
b. Frame, Wood, Adjacent	R=13.0, 318.0 ft²	a. Electric Resistance	Cap: 40.0 gallons
c. N/A			EF: 0.92
d. N/A		b. N/A	
e. N/A			
10. Ceiling types		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 2260.0 ft²	15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 250.0 ft		
b. N/A			

Glass/Floor Area: 0.15 Total as-built points: 32844 Total base points: 34050 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ben Sparks

DATE: 12-1-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Josh Sparks

DATE: 12-5-05

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2199.0	20.04	7932.2	Double, Clear	W	1.5	8.0	15.0	38.52	0.96	553.6	
				Double, Clear	W	1.5	4.0	4.0	38.52	0.82	126.0	
				Double, Clear	N	99.0	9.5	10.0	19.20	0.59	113.9	
				Double, Clear	W	13.5	9.5	30.0	38.52	0.46	529.2	
				Double, Clear	SW	12.0	8.0	14.0	40.16	0.42	238.8	
				Double, Clear	W	8.8	8.0	28.0	38.52	0.51	545.2	
				Double, Clear	S	99.0	9.5	20.0	35.87	0.43	309.8	
				Double, Clear	W	1.5	8.0	36.0	38.52	0.96	1328.7	
				Double, Clear	N	1.5	4.5	16.0	19.20	0.90	276.5	
				Double, Clear	E	1.5	8.0	56.0	42.06	0.96	2255.5	
				Double, Clear	E	7.5	9.5	20.0	42.06	0.58	485.9	
				Double, Clear	E	7.5	4.0	12.0	42.06	0.40	201.0	
				Double, Clear	NE	1.5	5.5	10.0	29.56	0.91	267.6	
				Double, Clear	E	1.5	5.5	15.0	42.06	0.90	565.5	
				Double, Clear	SE	1.5	5.5	10.0	42.75	0.86	368.1	
				Double, Clear	S	1.5	1.5	4.0	35.87	0.52	74.7	
				Double, Clear	S	1.5	5.5	30.0	35.87	0.83	895.4	
				As-Built Total:				**330.0**			**9135.6**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	318.0	0.70	222.6	Frame, Wood, Exterior	13.0	1428.0	1.50	2142.0
Exterior	1428.0	1.70	2427.6	Frame, Wood, Adjacent	13.0	318.0	0.60	190.8
Base Total:	**1746.0**		**2650.2**	**As-Built Total:**		**1746.0**		**2332.8**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	20.0	2.40	48.0	Exterior Insulated	80.0	4.10	328.0
Exterior	80.0	6.10	488.0	Adjacent Insulated	20.0	1.60	32.0
Base Total:	**100.0**		**536.0**	**As-Built Total:**	**100.0**		**360.0**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	2199.0	1.73	3804.3	Under Attic	30.0	2260.0	1.73 X 1.00	3909.8
Base Total:	**2199.0**		**3804.3**	**As-Built Total:**		**2260.0**		**3909.8**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT					
FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	=	Points
Slab	236.0(p)	-37.0	-8732.0	Slab-On-Grade Edge Insulation	0.0	236.0(p	-41.20		-9723.2
Raised	0.0	0.00	0.0						
Base Total:			**-8732.0**	**As-Built Total:**		**236.0**			**-9723.2**
INFILTRATION	Area X	BSPM	= Points			Area X	SPM	=	Points
	2199.0	10.21	22451.8			2199.0	10.21		22451.8
Summer Base Points: 28642.5				**Summer As-Built Points:**					**28466.8**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
				(sys 1: Central Unit 48000 btuh ,SEER/EFF(12.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
				28467	1.00	(1.09 x 1.147 x 1.00)	0.284	1.000	10122.4
28642.5	**0.4266**		**12218.9**	**28466.8**	**1.00**	**1.250**	**0.284**	**1.000**	**10122.4**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2199.0	12.74	5042.7	Double, Clear	W	1.5	8.0	15.0	20.73	1.01	314.4	
				Double, Clear	W	1.5	4.0	4.0	20.73	1.05	87.3	
				Double, Clear	N	99.0	9.5	10.0	24.58	1.03	252.5	
				Double, Clear	W	13.5	9.5	30.0	20.73	1.20	746.2	
				Double, Clear	SW	12.0	8.0	14.0	16.74	1.81	424.4	
				Double, Clear	W	8.8	8.0	28.0	20.73	1.18	683.8	
				Double, Clear	S	99.0	9.5	20.0	13.30	3.66	973.4	
				Double, Clear	W	1.5	8.0	36.0	20.73	1.01	754.5	
				Double, Clear	N	1.5	4.5	16.0	24.58	1.00	395.1	
				Double, Clear	E	1.5	8.0	56.0	18.79	1.02	1073.3	
				Double, Clear	E	7.5	9.5	20.0	18.79	1.22	460.3	
				Double, Clear	E	7.5	4.0	12.0	18.79	1.44	324.7	
				Double, Clear	NE	1.5	5.5	10.0	23.57	1.01	237.6	
				Double, Clear	E	1.5	5.5	15.0	18.79	1.04	293.5	
				Double, Clear	SE	1.5	5.5	10.0	14.71	1.11	163.8	
				Double, Clear	S	1.5	1.5	4.0	13.30	2.73	145.3	
				Double, Clear	S	1.5	5.5	30.0	13.30	1.15	457.6	
				As-Built Total:				**330.0**			**7787.7**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	318.0	3.60	1144.8	Frame, Wood, Exterior	13.0	1428.0	3.40	4855.2
Exterior	1428.0	3.70	5283.6	Frame, Wood, Adjacent	13.0	318.0	3.30	1049.4
Base Total:	**1746.0**		**6428.4**	**As-Built Total:**		**1746.0**		**5904.6**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	20.0	11.50	230.0	Exterior Insulated	80.0	8.40	672.0
Exterior	80.0	12.30	984.0	Adjacent Insulated	20.0	8.00	160.0
Base Total:	**100.0**		**1214.0**	**As-Built Total:**	**100.0**		**832.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X	WCM =	Points
Under Attic	2199.0	2.05	4507.9	Under Attic	30.0	2260.0	2.05 X	1.00	4633.0
Base Total:	**2199.0**		**4507.9**	**As-Built Total:**		**2260.0**			**4633.0**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT				
FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	236.0(p)	8.9	2100.4	Slab-On-Grade Edge Insulation	0.0	236.0(p	18.80	4436.8
Raised	0.0	0.00	0.0					
Base Total:			**2100.4**	**As-Built Total:**		**236.0**		**4436.8**
INFILTRATION	Area X	BWPM	= Points			Area X	WPM	= Points
	2199.0	-0.59	-1297.4			2199.0	-0.59	-1297.4
Winter Base Points:			**17996.1**	**Winter As-Built Points:**				**22296.7**
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier = Heating Points
				(sys 1: Electric Heat Pump 48000 btuh ,EFF(7.8) Ducts:Unc(S),Unc(R),Gar(AH),R6.0				
				22296.7	1.000	(1.069 x 1.169 x 1.00)	0.437	1.000 12181.3
17996.1	**0.6274**	**11290.7**		**22296.7**	**1.00**	**1.250**	**0.437**	**1.000 12181.3**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2635.00	10540.0	40.0	0.92	4	1.00	2635.00	1.00	10540.0	
				As-Built Total:						**10540.0**	

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points		Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	
12219	**11291**	**10540**	**34050**		**10122**	**12181**	**10540**	**32844**	

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 2, Sub: Rolling Meadows, Plat: , , FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.3

The higher the score, the more efficient the home.

Spec House, Lot: 2, Sub: Rolling Meadows, Plat: , , FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 48.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 12.00
4.	Number of Bedrooms	4	b.	N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	2199 ft²	c.	N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
a.	U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 229.0 ft²	13.	Heating systems	
b.	SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 229.0 ft²	a.	Electric Heat Pump	Cap: 48.0 kBtu/hr
8.	Floor types				HSPF: 7.80
a.	Slab-On-Grade Edge Insulation	R=0.0, 236.0(p) ft	b.	N/A	
b.	N/A				
c.	N/A		c.	N/A	
9.	Wall types		14.	Hot water systems	
a.	Frame, Wood, Exterior	R=13.0, 1428.0 ft²	a.	Electric Resistance	Cap: 40.0 gallons
b.	Frame, Wood, Adjacent	R=13.0, 318.0 ft²			EF: 0.92
c.	N/A		b.	N/A	
d.	N/A				
e.	N/A		c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10.	Ceiling types		15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a.	Under Attic	R=30.0, 2260.0 ft²			
b.	N/A				
c.	N/A				
11.	Ducts				
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 250.0 ft			
b.	N/A				

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

BUILDING INPUT SUMMARY REPORT

PROJECT

Title:	511234SparksConstruction	**Family Type:**	Single	**Address Type:**	Lot Information
Owner:	Spec House	**New/Existing:**	New	**Lot #:**	2
# of Units:	1	**Bedrooms:**	4	**Subdivision:**	Rolling Meadows
Builder Name:	Josh Sparks	**Conditioned Area:**	2199	**Platbook:**	(blank)
Climate:	North	**Total Stories:**	1	**Street:**	N/A
Permit Office:	(blank)	**Worst Case:**	Yes	**County:**	Columbia
Jurisdiction #:	(blank)	**Rotate Angle:**	(blank)	**City, St, Zip:**	, FL,

FLOORS

#	Floor Type	R-Val	Area/Perimeter	Units
1	Slab-On-Grade Edge Insulation	0.0	236.0(p) ft	1

DOORS

#	Door Type	Orientation	Area	Units
1	Insulated	Exterior	10.0 ft²	8
2	Insulated	Adjacent	20.0 ft²	1

CEILINGS

#	Ceiling Type	R-Val	Area	Base Area	Units
1	Under Attic	30.0	2260.0 ft²	2199.0 ft²	1

Credit Multipliers: None

COOLING

#	System Type	Efficiency	Capacity
1	Central Unit	SEER: 12.00	48.0 kBtu/hr

Credit Multipliers: None

WALLS

#	Wall Type	Location	R-Val	Area	Units
1	Frame - Wood	Exterior	13.0	1428.0 ft²	1
2	Frame - Wood	Adjacent	13.0	318.0 ft²	1

HEATING

#	System Type	Efficiency	Capacity
1	Electric Heat Pump	COP: 7.80	48.0 kBtu/hr

Credit Multipliers: None

WINDOWS

#	Panes	Tint	Ornt	Area	OH Length	OH Hght	Units
1	Double	Clear	N	15.0 ft²	1.5 ft	8.0 ft	1
2	Double	Clear	N	4.0 ft²	1.5 ft	4.0 ft	1
3	Double	Clear	E	10.0 ft²	99.0 ft	9.5 ft	1
4	Double	Clear	N	10.0 ft²	13.5 ft	9.5 ft	3
5	Double	Clear	NW	14.0 ft²	12.0 ft	8.0 ft	1
6	Double	Clear	N	28.0 ft²	8.8 ft	8.0 ft	1
7	Double	Clear	W	10.0 ft²	99.0 ft	9.5 ft	2
8	Double	Clear	N	18.0 ft²	1.5 ft	8.0 ft	2
9	Double	Clear	E	16.0 ft²	1.5 ft	4.5 ft	1
10	Double	Clear	S	28.0 ft²	1.5 ft	8.0 ft	2
11	Double	Clear	S	10.0 ft²	7.5 ft	9.5 ft	2
12	Double	Clear	S	12.0 ft²	7.5 ft	4.0 ft	1
13	Double	Clear	SE	10.0 ft²	1.5 ft	5.5 ft	1
14	Double	Clear	S	15.0 ft²	1.5 ft	5.5 ft	1
15	Double	Clear	SW	10.0 ft²	1.5 ft	5.5 ft	1
16	Double	Clear	W	4.0 ft²	1.5 ft	1.5 ft	1
17	Double	Clear	W	15.0 ft²	1.5 ft	5.5 ft	2

DUCTS

#	Supply Location	Return Location	Air Handler Location	Supply R-Val	Supply Length
1	Uncond.	Uncond.	Garage	6.0	250.0 ft

Credit Multipliers: None

WATER

#	System Type	EF	Cap.	Conservation Type	Con. EF
1	Electric Resistance	0.92	40.0	None	0.00

REFR.

#	Use Default?	Annual Operating Cost	Electric Rate
1	Yes	N/A	N/A

Residential System Sizing Calculation

Summary

Spec House

, FL

Project Title:
511234SparksConstruction

Class 3 Rating
Registration No. 0
Climate: North

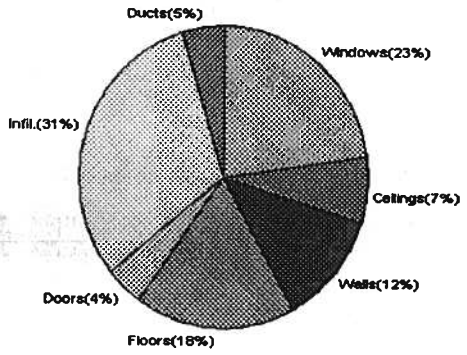
12/1/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**40874**	**Btuh**	**Total cooling load calculation**	**39337**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	117.4	48000	Sensible (SHR = 0.75)	126.0	36000
Heat Pump + Auxiliary(0.0kW)	117.4	48000	Latent	111.6	12000
			Total (Electric Heat Pump)	122.0	48000

WINTER CALCULATIONS

Winter Heating Load (for 2199 sqft)

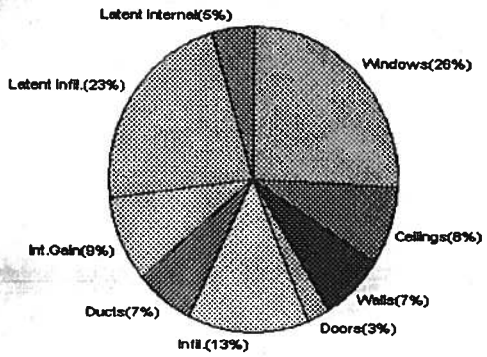
Load component			Load	
Window total	330	sqft	9339	Btuh
Wall total	1746	sqft	4936	Btuh
Door total	100	sqft	1654	Btuh
Ceiling total	2260	sqft	2938	Btuh
Floor total	236	ft	7458	Btuh
Infiltration	294	cfm	12603	Btuh
Subtotal			**38928**	**Btuh**
Duct loss			1946	Btuh
TOTAL HEAT LOSS			**40874**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 2199 sqft)

Load component			Load	
Window total	330	sqft	10255	Btuh
Wall total	1746	sqft	2815	Btuh
Door total	100	sqft	1014	Btuh
Ceiling total	2260	sqft	3209	Btuh
Floor total			0	Btuh
Infiltration	257	cfm	5090	Btuh
Internal gain			3600	Btuh
Subtotal(sensible)			**25984**	**Btuh**
Duct gain			2598	Btuh
Total sensible gain			**28582**	**Btuh**
Latent gain(infiltration)			8915	Btuh
Latent gain(internal)			1840	Btuh
Total latent gain			**10755**	**Btuh**
TOTAL HEAT GAIN			**39337**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: Ben Green

DATE: 12-1-05

System Sizing Calculations - Winter

Residential Load - Component Details

Spec House
, FL

Project Title:
511234SparksConstruction

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

12/1/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	15.0	28.3	424 Btuh
2	2, Clear, Metal, DEF	N	4.0	28.3	113 Btuh
3	2, Clear, Metal, DEF	E	10.0	28.3	283 Btuh
4	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	NW	14.0	28.3	396 Btuh
6	2, Clear, Metal, DEF	N	28.0	28.3	792 Btuh
7	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
8	2, Clear, Metal, DEF	N	36.0	28.3	1019 Btuh
9	2, Clear, Metal, DEF	E	16.0	28.3	453 Btuh
10	2, Clear, Metal, DEF	S	56.0	28.3	1585 Btuh
11	2, Clear, Metal, DEF	S	20.0	28.3	566 Btuh
12	2, Clear, Metal, DEF	S	12.0	28.3	340 Btuh
13	2, Clear, Metal, DEF	SE	10.0	28.3	283 Btuh
14	2, Clear, Metal, DEF	S	15.0	28.3	424 Btuh
15	2, Clear, Metal, DEF	SW	10.0	28.3	283 Btuh
16	2, Clear, Metal, DEF	W	4.0	28.3	113 Btuh
17	2, Clear, Metal, DEF	W	30.0	28.3	849 Btuh
	Window Total		330		9339 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1428	3.1	4427 Btuh
2	Frame - Adjacent	13.0	318	1.6	509 Btuh
	Wall Total		1746		4936 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		80	18.3	1466 Btuh
2	Insulated - Adjac		20	9.4	188 Btuh
	Door Total		100		1654Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2260	1.3	2938 Btuh
	Ceiling Total		2260		2938Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	236.0 ft(p)	31.6	7458 Btuh
	Floor Total		236		7458 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	21990(sqft)	294	12603 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			294	12603 Btuh

Totals for Heating	**Subtotal**	**38928 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1946 Btuh**
	Total Btuh Loss	**40874 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
511234SparksConstruction

Class 3 Rating
Registration No. 0
Climate: North

12/1/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Spec House
, FL

Project Title:
511234SparksConstruction

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 12/1/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	8	15.0	0.0	15.0	22	22	330 Btuh
2	2, Clear, DEF, N, N	N	1.5	4	4.0	0.0	4.0	22	22	88 Btuh
3	2, Clear, DEF, N, N	E	99	9.5	10.0	10.0	0.0	22	72	220 Btuh
4	2, Clear, DEF, N, N	N	13.5	9.5	30.0	0.0	30.0	22	22	660 Btuh
5	2, Clear, DEF, N, N	NW	12	8	14.0	0.0	14.0	22	50	700 Btuh
6	2, Clear, DEF, N, N	N	8.83	8	28.0	0.0	28.0	22	22	616 Btuh
7	2, Clear, DEF, N, N	W	99	9.5	20.0	20.0	0.0	22	72	440 Btuh
8	2, Clear, DEF, N, N	N	1.5	8	36.0	0.0	36.0	22	22	792 Btuh
9	2, Clear, DEF, N, N	E	1.5	4.5	16.0	3.0	13.0	22	72	1003 Btuh
10	2, Clear, DEF, N, N	S	1.5	8	56.0	56.0	0.0	22	37	1232 Btuh
11	2, Clear, DEF, N, N	S	7.5	9.5	20.0	20.0	0.0	22	37	440 Btuh
12	2, Clear, DEF, N, N	S	7.5	4	12.0	12.0	0.0	22	37	264 Btuh
13	2, Clear, DEF, N, N	SE	1.5	5.5	10.0	0.6	9.4	22	62	597 Btuh
14	2, Clear, DEF, N, N	S	1.5	5.5	15.0	15.0	0.0	22	37	330 Btuh
15	2, Clear, DEF, N, N	SW	1.5	5.5	10.0	0.6	9.4	22	62	597 Btuh
16	2, Clear, DEF, N, N	W	1.5	1.5	4.0	3.3	0.7	22	72	122 Btuh
17	2, Clear, DEF, N, N	W	1.5	5.5	30.0	6.7	23.3	22	72	1825 Btuh
	Window Total				330					10255 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Exterior	13.0	1428.0	1.7	2485 Btuh
2	Frame - Adjacent	13.0	318.0	1.0	331 Btuh
	Wall Total		1746.0		2815 Btuh

Doors	Type	Area	HTM	Load
1	Insulated - Exter	80.0	10.1	811 Btuh
2	Insulated - Adjac	20.0	10.1	203 Btuh
	Door Total	100.0		1014 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	2260.0	1.4	3209 Btuh
	Ceiling Total		2260.0		3209 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	236.0 ft(p)	0.0	0 Btuh
	Floor Total		236.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.70	21990	257.1	5090 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			257	5090 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	8	X 300	+	1200	3600 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
511234SparksConstruction

Class 3 Rating
Registration No. 0
Climate: North

12/1/2005

Totals for Cooling	**Subtotal**	**25984 Btuh**
	Duct gain(using duct multiplier of 0.10)	**2598 Btuh**
	Total sensible gain	**28582 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**8915 Btuh**
	Latent occupant gain (8 people @ 230 Btuh per person)	**1840 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**39337 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0512-58

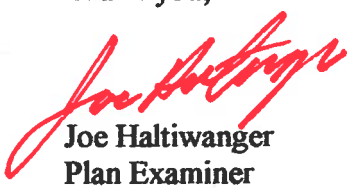
Sparks Construction Owner Sparks Construction lot 2 Rolling Meadows

On the date of December 27, 2005 application 0512-58 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0512-58 when making reference to this application.

1. Please submit an approved copy of the Columbia County Environmental Health Department site plan application for an on site waste water septic system.
2. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department.
3. On the dwelling elevation drawing show the height of the chimney flue above the roof peak and the distance from the nearest roof line intersection.

Thank you,



Joe Haltiwanger
Plan Examiner
Columbia County Building Department

Project Information for:	L141740		
Builder:	SPARKS CONSTRUCTION	Date:	11/30/2005
Lot:	LOT 2 ROLLING MEADOWS	Start Number:	1416
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	38		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

SPARKS, JOSHUA D. CBC 1252260
Address: 130 S W COUNTRY CT
LIVE OAK FL 32060 Designer: 115

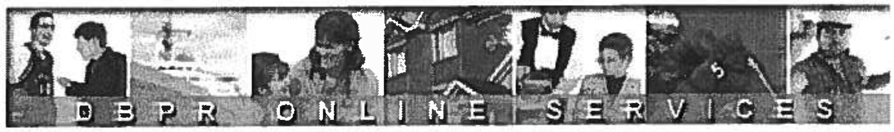
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	EJ7	1130051416	11/30/2005				
2	EJ7A	1130051417	11/30/2005				
3	PB01	1130051418	11/30/2005				
4	PB02	1130051419	11/30/2005				
5	PB03	1130051420	11/30/2005				
6	PB04	1130051421	11/30/2005				
7	PB05	1130051422	11/30/2005				
8	PB06	1130051423	11/30/2005				
9	PB07	1130051424	11/30/2005				
10	T01	1130051425	11/30/2005				
11	T01G	1130051426	11/30/2005				
12	T02	1130051427	11/30/2005				
13	T03	1130051428	11/30/2005				
14	T03G	1130051429	11/30/2005				
15	T04G	1130051430	11/30/2005				
16	T05	1130051431	11/30/2005				
17	T05A	1130051432	11/30/2005				
18	T05G	1130051433	11/30/2005				
19	T06	1130051434	11/30/2005				
20	T07	1130051435	11/30/2005				
21	T08	1130051436	11/30/2005				
22	T09	1130051437	11/30/2005				
23	T10	1130051438	11/30/2005				
24	T11	1130051439	11/30/2005				
25	T12	1130051440	11/30/2005				
26	T13	1130051441	11/30/2005				
27	T14	1130051442	11/30/2005				
28	T15	1130051443	11/30/2005				
29	T16	1130051444	11/30/2005				
30	T17	1130051445	11/30/2005				
31	T18	1130051446	11/30/2005				
32	T19	1130051447	11/30/2005				
33	T20	1130051448	11/30/2005				
34	T21	1130051449	11/30/2005				
35	T22	1130051450	11/30/2005				
36	T23	1130051451	11/30/2005				
37	T24	1130051452	11/30/2005				
38	V01	1130051453	11/30/2005				

NOV 30 2005



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NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051416

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	EJ7	MONO TRUSS	27	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:05 2005 Page 1

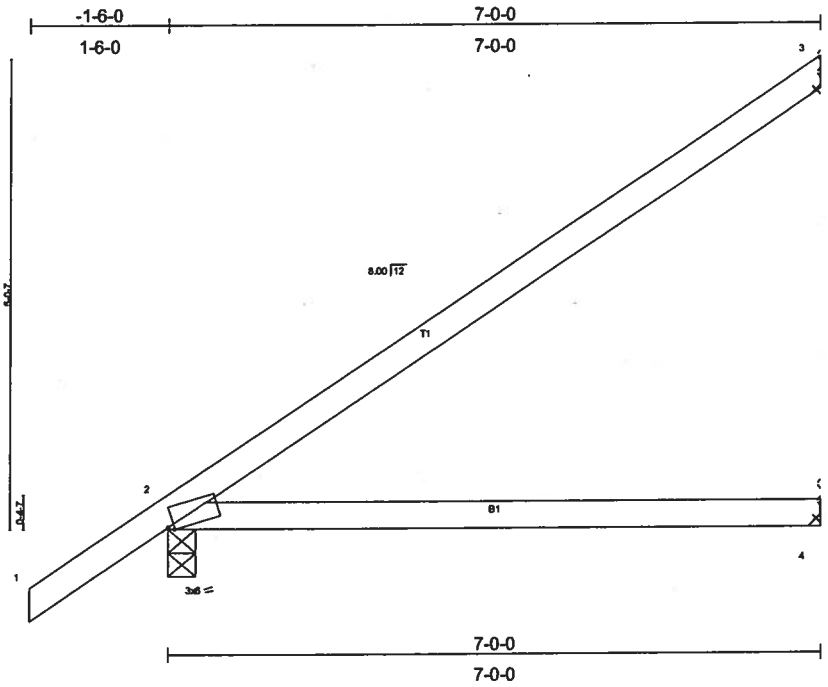


Plate Offsets (X,Y): [2:0-0-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.14	2-4	>592	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.23	2-4	>357	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=385/0-3-8, 4=110/Mechanical
Max Horz 2=277(load case 5)
Max Uplift3=-165(load case 5), 2=-141(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-133/73
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 3, 141 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051417

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	EJ7A	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:06 2005 Page 1

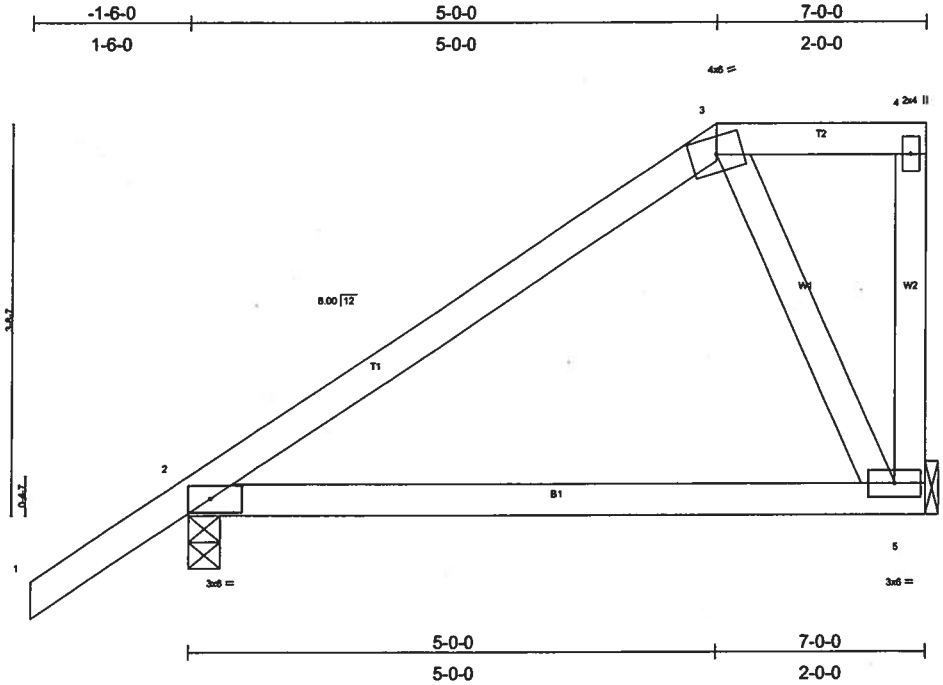


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.05	2-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.09	2-5	>894	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=271/Mechanical, 2=382/0-3-8
Max Horz 2=219(load case 5)
Max Uplift5=-108(load case 5), 2=-171(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-200/33, 3-4=-35/0, 4-5=-32/61
BOT CHORD 2-5=-98/123
WEBS 3-5=-185/219

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5 and 171 lb uplift at joint 2.

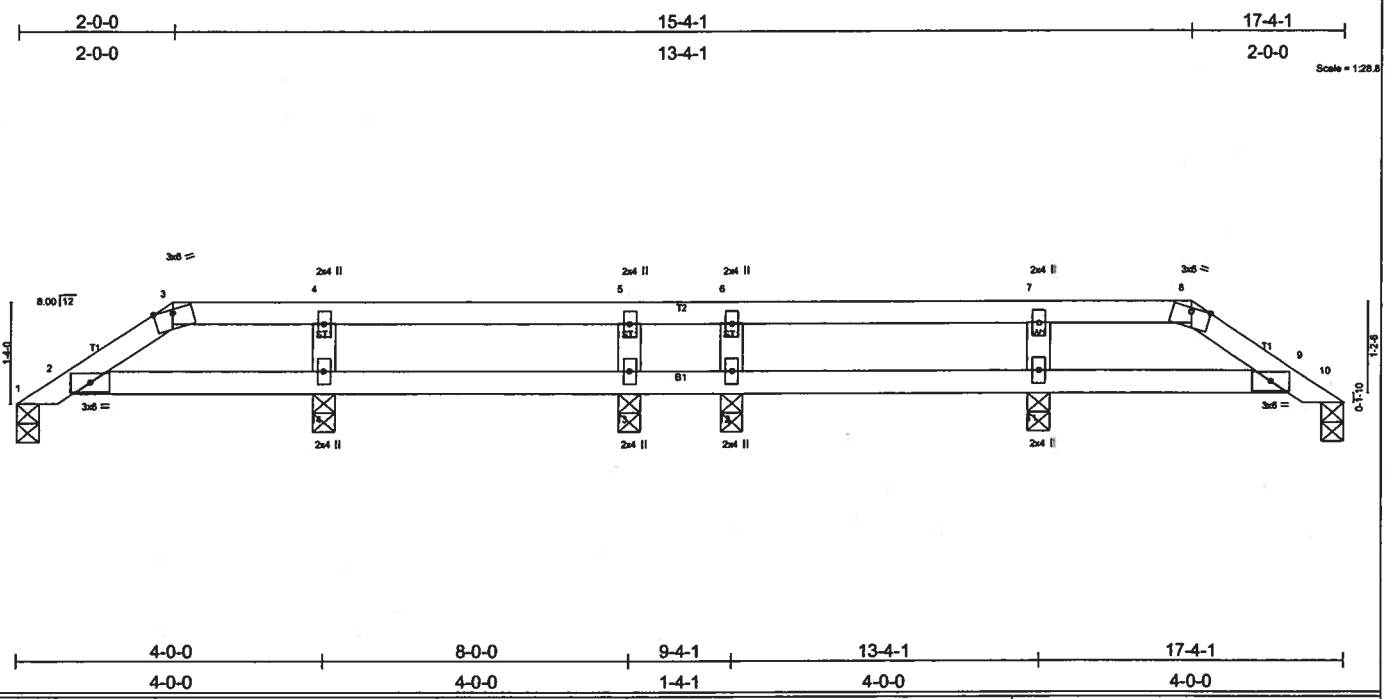
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051418

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB01	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:50:45 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.01	2-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.01	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 54 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=134/0-3-8, 13=208/0-3-8, 14=363/0-3-8, 12=208/0-3-8, 11=363/0-3-8, 10=134/0-3-8
Max Horz 1=-44(load case 3)
Max Uplift1=-50(load case 5), 13=-116(load case 3), 14=-172(load case 4), 12=-114(load case 4), 11=-163(load case 3), 10=-52(load case 6)
Max Grav 1=134(load case 1), 13=219(load case 10), 14=368(load case 9), 12=219(load case 9), 11=368(load case 10), 10=134(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-68/40, 2-3=-97/54, 3-4=-72/60, 4-5=-72/60, 5-6=-72/60, 6-7=-72/60, 7-8=-72/60, 8-9=-97/54, 9-10=-68/34
BOT CHORD 2-14=-25/72, 13-14=-25/72, 12-13=-25/72, 11-12=-25/72, 9-11=-25/72
WEBS 5-13=-143/126, 4-14=-223/184, 6-12=-143/125, 7-11=-223/179

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 116 lb uplift at joint 13, 172 lb uplift at joint 14, 114 lb uplift at joint 12, 163 lb uplift at joint 11 and 52 lb uplift at joint 10.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

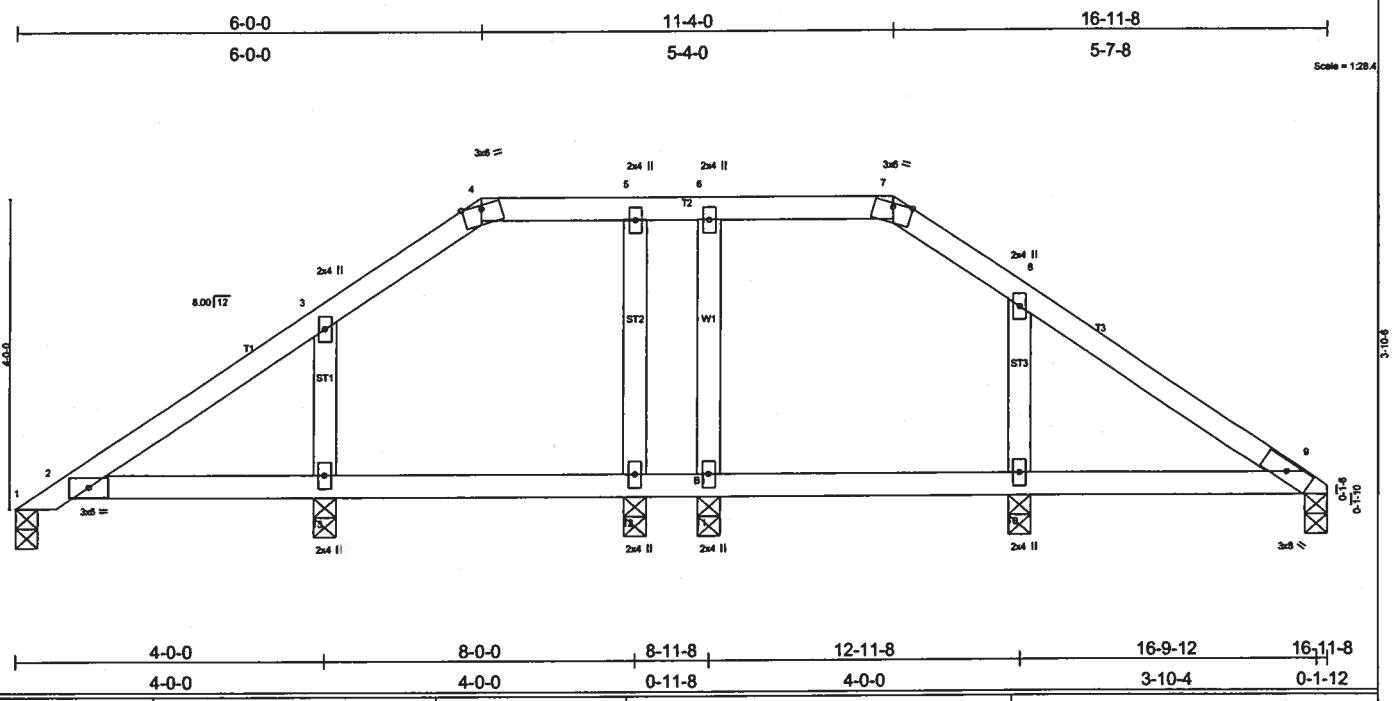
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051419

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB02	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:52:16 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.02	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=141/0-3-8, 9=155/0-3-8, 12=170/0-3-8, 13=375/0-3-8, 10=367/0-3-8, 11=182/0-3-8
Max Horz 1=135(load case 4)
Max Uplift1=-38(load case 3), 9=-28(load case 3), 12=-58(load case 4), 13=-195(load case 5), 10=-183(load case 6), 11=-86(load case 4)
Max Grav 1=141(load case 1), 9=157(load case 10), 12=170(load case 1), 13=376(load case 9), 10=367(load case 1), 11=187(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-119/131, 2-3=-80/123, 3-4=-110/97, 4-5=-47/88, 5-6=-47/88, 6-7=-47/88, 7-8=-112/98, 8-9=-104/82
BOT CHORD 2-13=-66/94, 12-13=-66/94, 11-12=-66/94, 10-11=-66/94, 9-10=-66/94
WEBS 5-12=-110/85, 3-13=-221/198, 8-10=-220/190, 6-11=-124/101

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 28 lb uplift at joint 9, 58 lb uplift at joint 12, 195 lb uplift at joint 13, 183 lb uplift at joint 10 and 86 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051420

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB03	VALLEY	1	1	Job Reference (optional)

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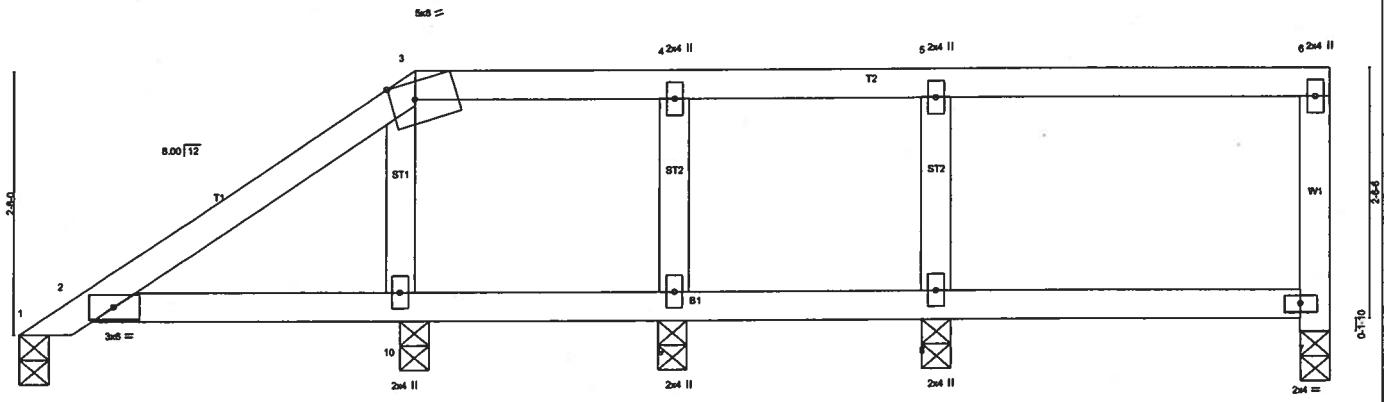
4-0-0 | 13-3-8
4-0-0 | 9-3-8

Scale = 1:22.3



4-0-0 | 9-3-8 | 13-1-12
4-0-0 | 5-3-8 | 3-10-4

Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=127/0-3-8, 7=153/0-3-8, 9=158/0-3-8, 10=332/0-3-8, 8=311/0-3-8
Max Horz 1=120(load case 5)
Max Uplift7=-78(load case 3), 9=-116(load case 3), 10=-174(load case 5), 8=-133(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-102/3, 2-3=-84/44, 3-4=-29/19, 4-5=-29/19, 5-6=-29/19, 6-7=-98/85
BOT CHORD 2-10=-18/16, 9-10=-19/29, 8-9=-19/29, 7-8=-19/29
WEBS 4-9=-104/123, 3-10=-193/185, 5-8=-200/171

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 7, 116 lb uplift at joint 9, 174 lb uplift at joint 10 and 133 lb uplift at joint 8.
5) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

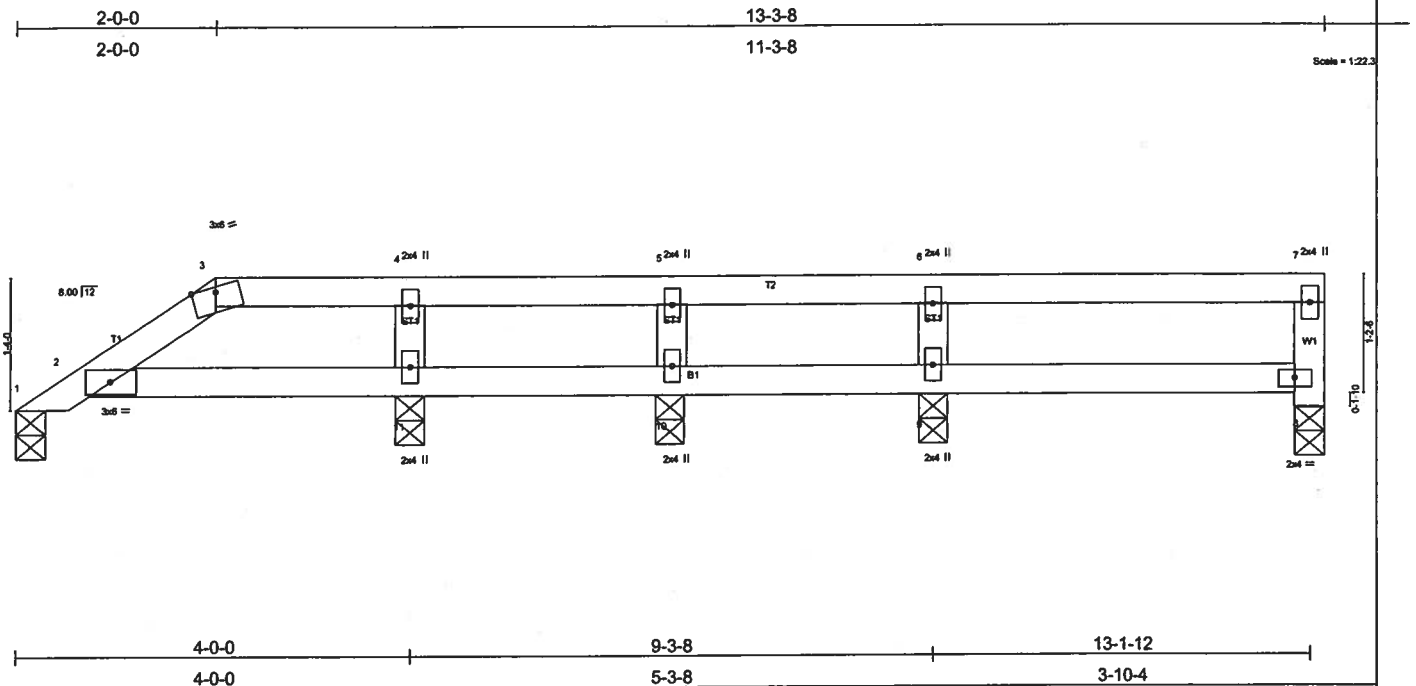
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051421

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB04	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:54:51 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=149/0-3-8, 8=161/0-3-8, 10=169/0-3-8, 11=304/0-3-8, 9=298/0-3-8
Max Horz 1=58(load case 5)
Max Uplift1=-42(load case 5), 8=-75(load case 3), 10=-96(load case 3), 11=-107(load case 4), 9=-140(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-76/9, 2-3=-126/53, 3-4=-98/55, 4-5=-98/55, 5-6=-98/55, 6-7=-98/55, 7-8=-103/89
BOT CHORD 2-11=-55/98, 10-11=-55/98, 9-10=-55/98, 8-9=-55/98
WEBS 5-10=-116/109, 4-11=-174/148, 6-9=-191/162

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 75 lb uplift at joint 8, 96 lb uplift at joint 10, 107 lb uplift at joint 11 and 140 lb uplift at joint 9.
5) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051422

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB05	PIGGYBACK	8	1	Job Reference (optional)

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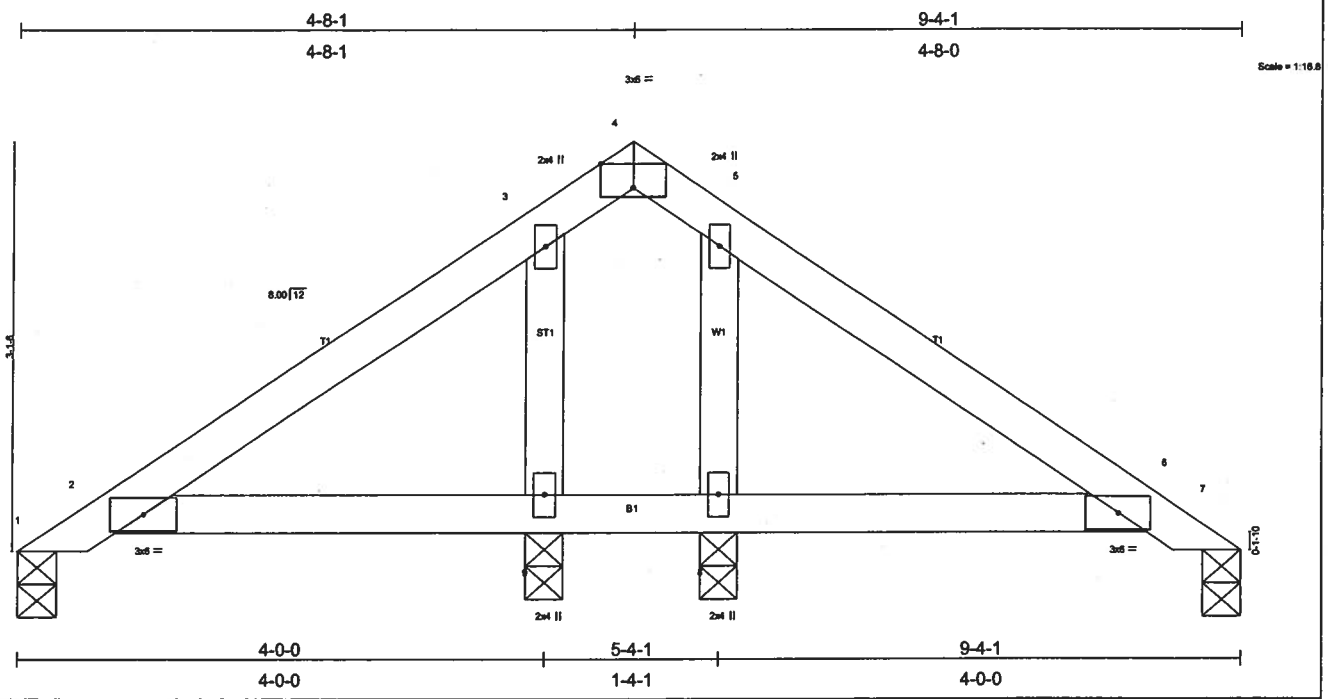


Plate Offsets (X,Y): [4:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=31/0-3-8, 9=338/0-3-8, 8=338/0-3-8, 7=31/0-3-8
Max Horz 1=105(load case 4)
Max Uplift1=-10(load case 3), 9=-176(load case 5), 8=-160(load case 6), 7=-17(load case 3)
Max Grav 1=53(load case 9), 9=355(load case 9), 8=355(load case 10), 7=53(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-101/99, 2-3=-133/260, 3-4=-27/139, 4-5=-33/139, 5-6=-133/260, 6-7=-26/14
BOT CHORD 2-9=-167/175, 8-9=-167/175, 6-8=-167/175
WEBS 3-9=-253/168, 5-8=-253/163

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1, 176 lb uplift at joint 9, 160 lb uplift at joint 8 and 17 lb uplift at joint 7.
5) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

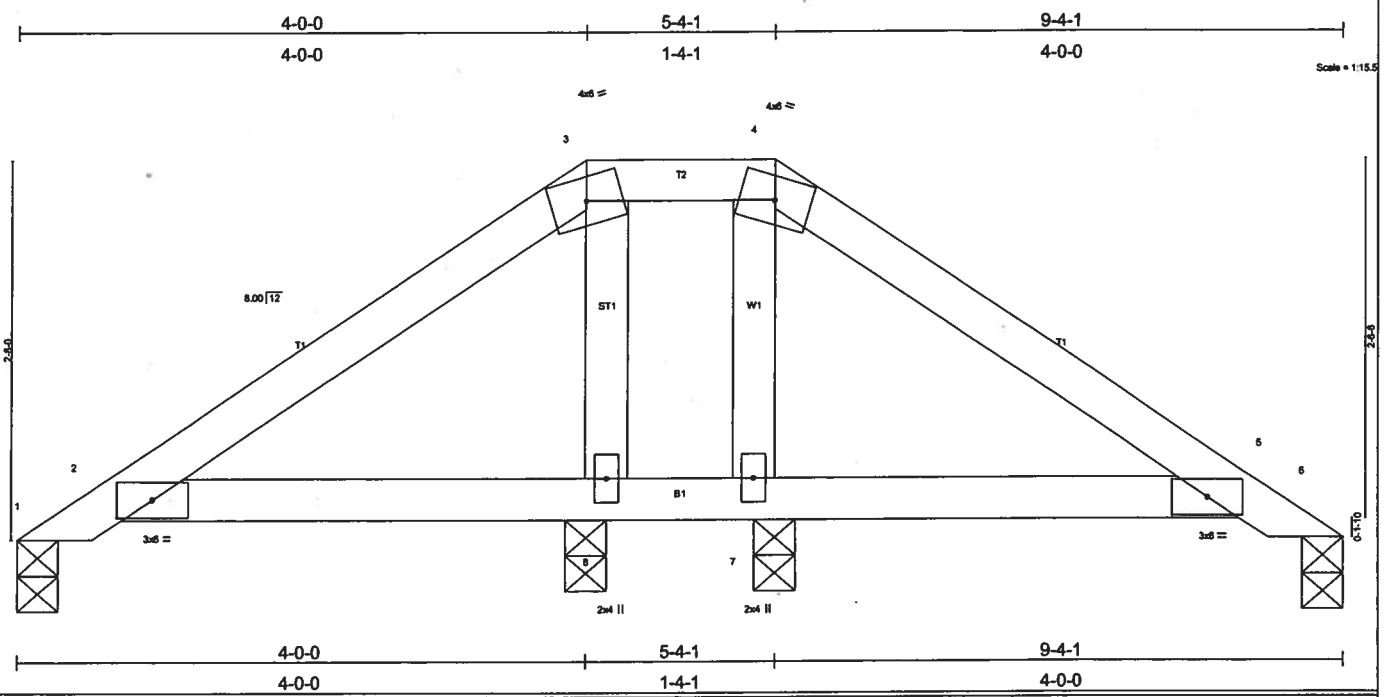
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051423

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB06	HIP PIGGYBACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:58:41 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=28/0-3-8, 8=341/0-3-8, 7=341/0-3-8, 6=28/0-3-8
Max Horz 1=90(load case 4)
Max Uplift1=-13(load case 10), 8=-164(load case 4), 7=-142(load case 6), 6=-31(load case 3)
Max Grav 1=53(load case 9), 8=369(load case 9), 7=369(load case 10), 6=53(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-81/85, 2-3=-98/271, 3-4=-30/195, 4-5=-98/271, 5-6=-26/21
BOT CHORD 2-8=-178/145, 7-8=-195/155, 5-7=-178/145
WEBS 3-8=-261/156, 4-7=-261/156

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 164 lb uplift at joint 8, 142 lb uplift at joint 7 and 31 lb uplift at joint 6.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

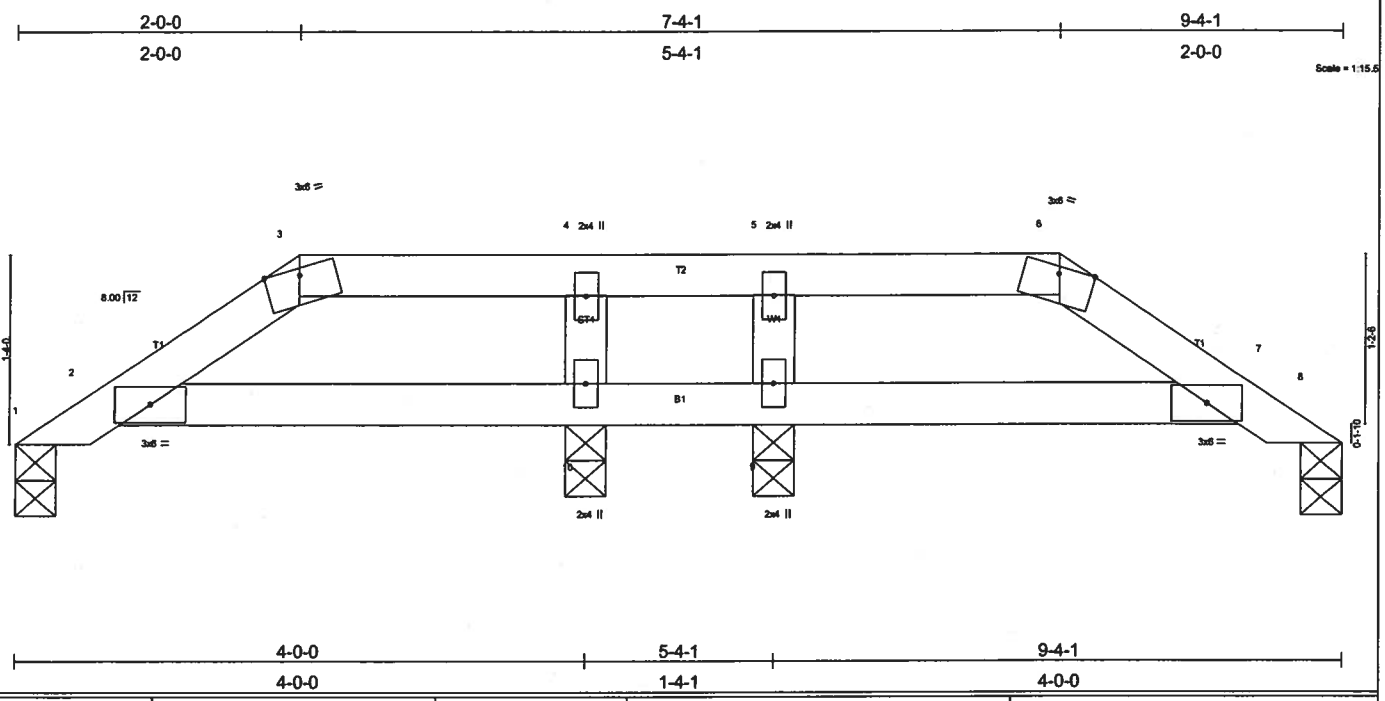
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051424

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	PB07	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:59:17 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.01	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/0-3-8, 10=205/0-3-8, 9=205/0-3-8, 8=165/0-3-8
Max Horz 1=44(load case 4)
Max Uplift1=-59(load case 5), 10=-120(load case 4), 9=-114(load case 3), 8=-61(load case 6)
Max Grav 1=165(load case 1), 10=231(load case 9), 9=231(load case 10), 8=165(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-84/52, 2-3=-174/131, 3-4=-143/131, 4-5=-143/131, 5-6=-143/131, 6-7=-174/131, 7-8=-84/52
BOT CHORD 2-10=-72/143, 9-10=-72/143, 7-9=-72/143
WEBS 4-10=-129/110, 5-9=-129/108

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 1, 120 lb uplift at joint 10, 114 lb uplift at joint 9 and 61 lb uplift at joint 8.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

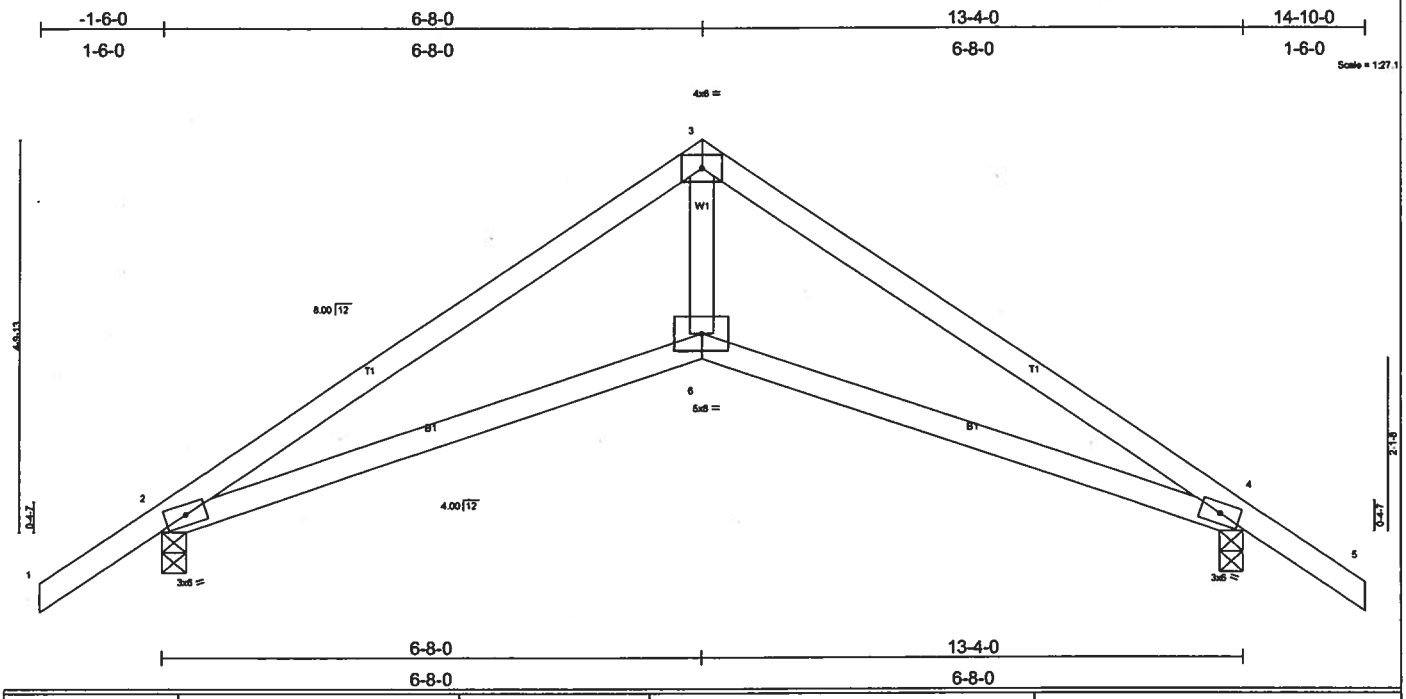
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051425

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T01	SCISSOR	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:12 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.09	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.14	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.08	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 54 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=637/0-3-8, 4=637/0-3-8
Max Horz 2=-162(load case 3)
Max Uplift2=-271(load case 5), 4=-271(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-1160/239, 3-4=-1160/270, 4-5=0/43
BOT CHORD 2-6=-136/966, 4-6=-134/966
WEBS 3-6=-61/828

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2 and 271 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051426

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T01G	SCISSOR	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:13 2005 Page 1

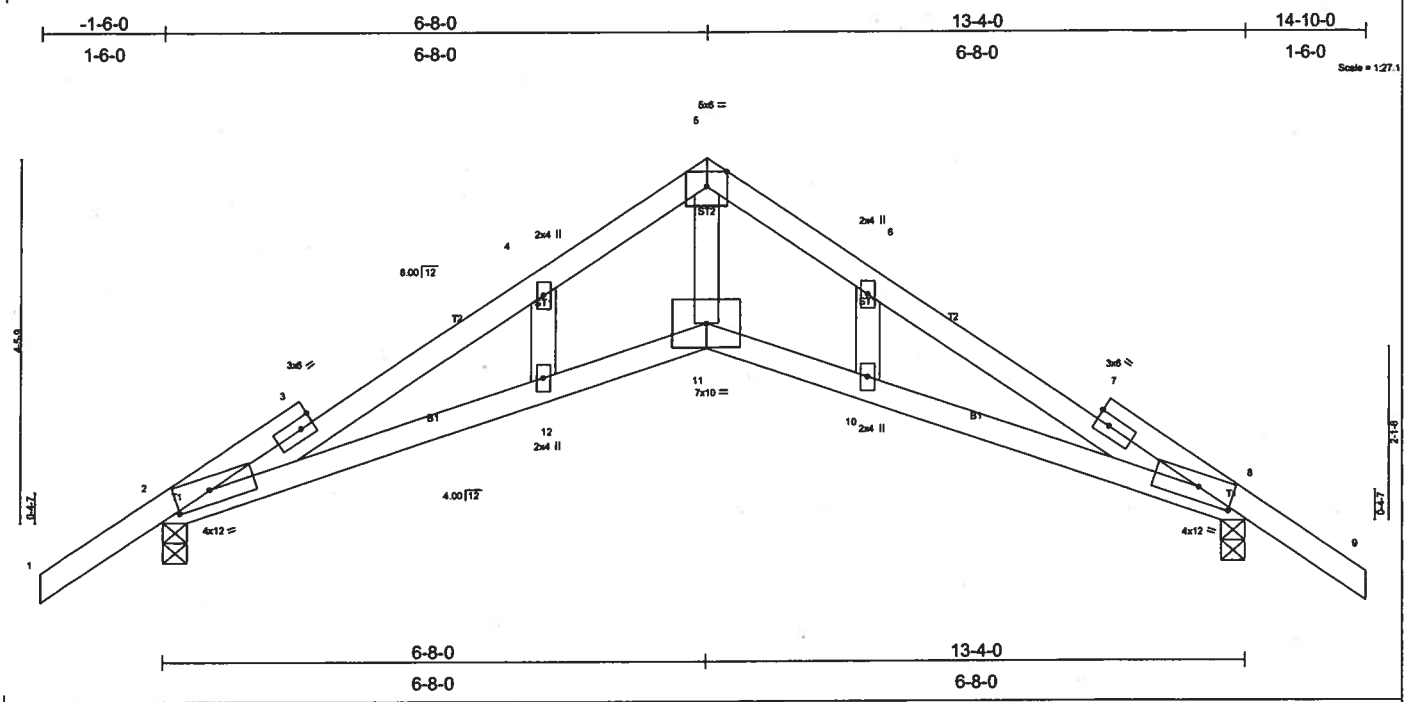


Plate Offsets (X,Y): [2:0-5-4,0-2-0], [8:0-5-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.20	2-12	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.32	2-12	>496	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.59	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 61 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-3 oc bracing.

REACTIONS (lb/size) 2=1127/0-3-8, 8=1127/0-3-8
Max Horz 2=-150(load case 3)
Max Uplift2=-457(load case 5), 8=-457(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/91, 2-3=-2258/580, 3-4=-2142/566, 4-5=-2117/650, 5-6=-2117/666, 6-7=-2142/604, 7-8=-2258/618, 8-9=-11/91
BOT CHORD 2-12=-457/1885, 11-12=-505/1980, 10-11=-498/1980, 8-10=-461/1885
WEBS 5-11=-537/1829, 4-12=-228/179, 6-10=-228/177

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable studs spaced at 2-0-0 oc.
5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 2 and 457 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-11=-30, 8-11=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051427

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T02	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:13 2005 Page 1

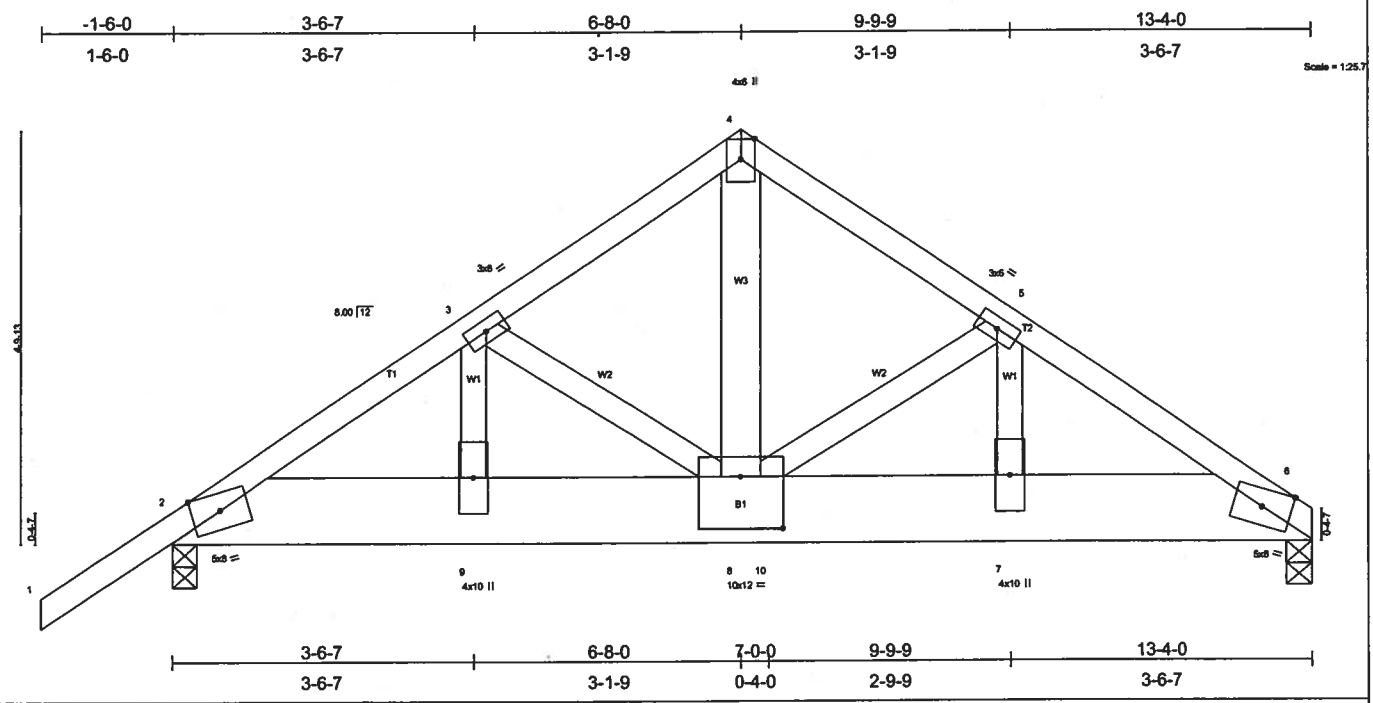


Plate Offsets (X,Y): [8:0-6-0,0-7-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.10	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.31	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=5180/0-3-8, 2=3016/0-3-8
Max Horz 2=175(load case 3)
Max Uplift6=-1965(load case 5), 2=-1186(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-4705/1728, 3-4=-5056/1943, 4-5=-5053/1931, 5-6=-6921/2608
BOT CHORD 2-9=-1432/3914, 8-9=-1432/3914, 8-10=-2140/5794, 7-10=-2140/5794, 6-7=-2140/5794
WEBS 3-9=-609/311, 3-8=-220/427, 4-8=-2027/5352, 5-8=-1967/834, 5-7=-699/1912

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 10 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1965 lb uplift at joint 6 and 1186 lb uplift at joint 2.
6) Girder carries tie-in span(s): 33-2-8 from 7-0-0 to 13-4-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2995 lb down and 1131 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-10=-30, 6-10=-679(F=-649)
Concentrated Loads (lb)
Vert: 10=-2995(F)

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051428

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T03	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:14 2005 Page 1

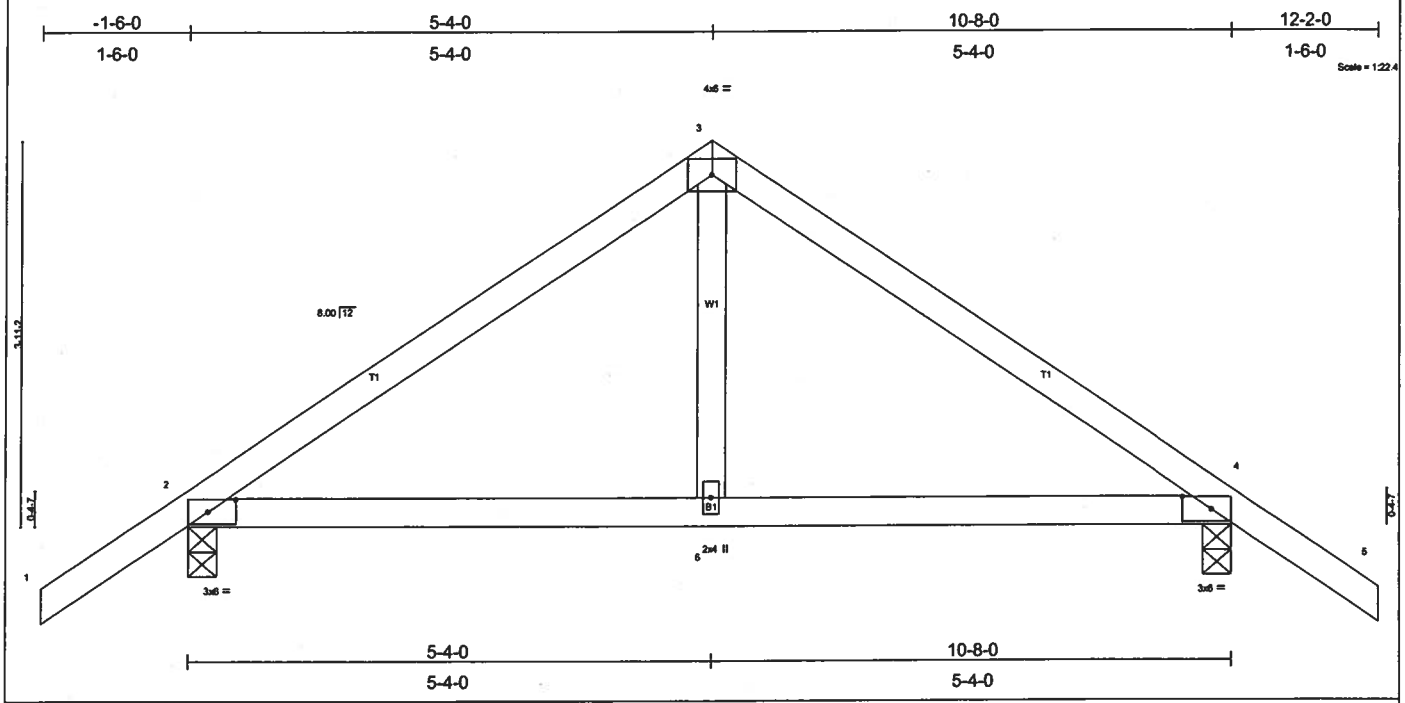


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.05	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	0.04	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=525/0-3-8, 4=525/0-3-8
Max Horz 2=130(load case 4)
Max Uplift2=-367(load case 5), 4=-367(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-508/493, 3-4=-508/493, 4-5=0/44
BOT CHORD 2-6=-254/359, 4-6=-254/359
WEBS 3-6=-325/189

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 367 lb uplift at joint 2 and 367 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051429

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T03G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:15 2005 Page 1

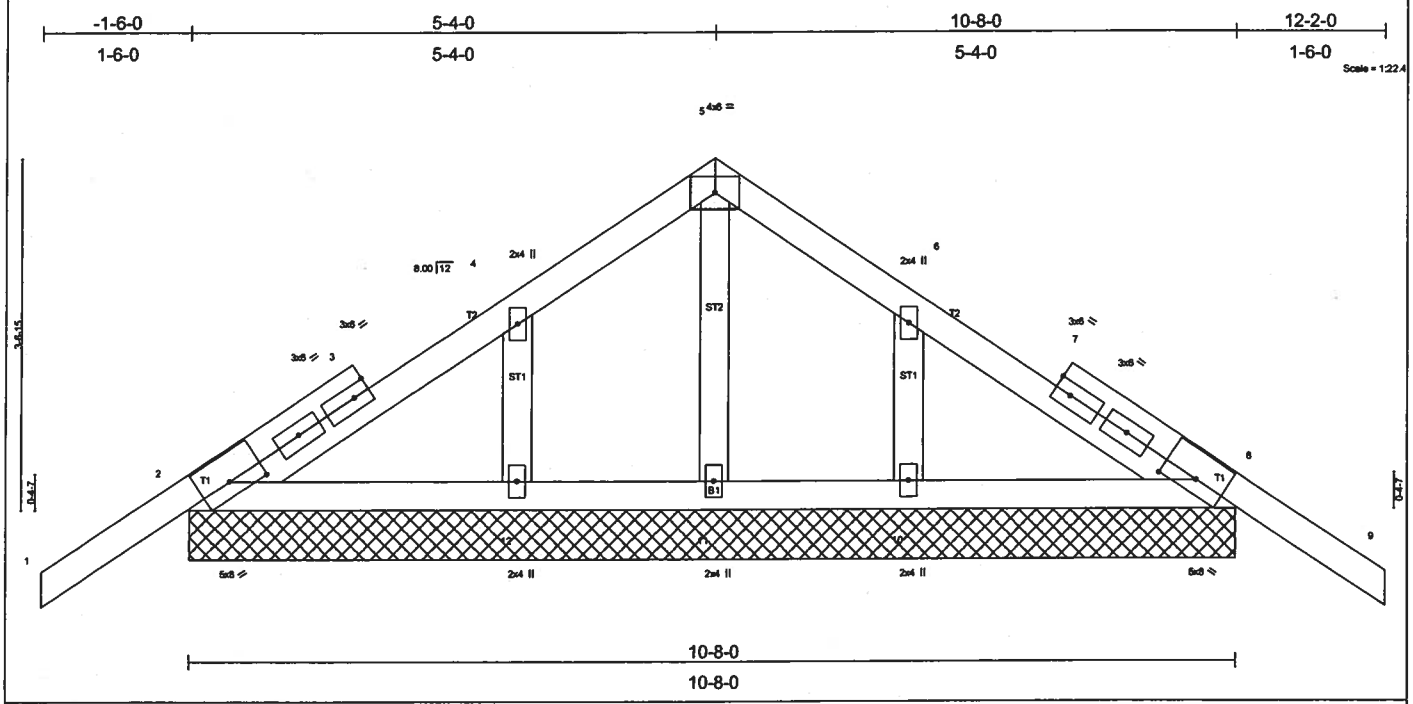


Plate Offsets (X,Y): [2:0-4-5,0-1-12], [8:0-4-5,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.01	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=408/10-8-0, 8=408/10-8-0, 11=246/10-8-0, 12=408/10-8-0, 10=408/10-8-0
Max Horz 2=118(load case 4)
Max Uplift2=-212(load case 5), 8=-227(load case 6), 11=-29(load case 5), 12=-172(load case 5), 10=-175(load case 6)
Max Grav 2=409(load case 9), 8=409(load case 10), 11=246(load case 1), 12=411(load case 9), 10=411(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/91, 2-3=-64/76, 3-4=-69/124, 4-5=-54/100, 5-6=-54/100, 6-7=-27/124, 7-8=-69/33, 8-9=-12/91
BOT CHORD 2-12=-23/119, 11-12=-23/119, 10-11=-23/119, 8-10=-23/119
WEBS 5-11=-212/38, 4-12=-315/192, 6-10=-315/193

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 2, 227 lb uplift at joint 8, 29 lb uplift at joint 11, 172 lb uplift at joint 12 and 175 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

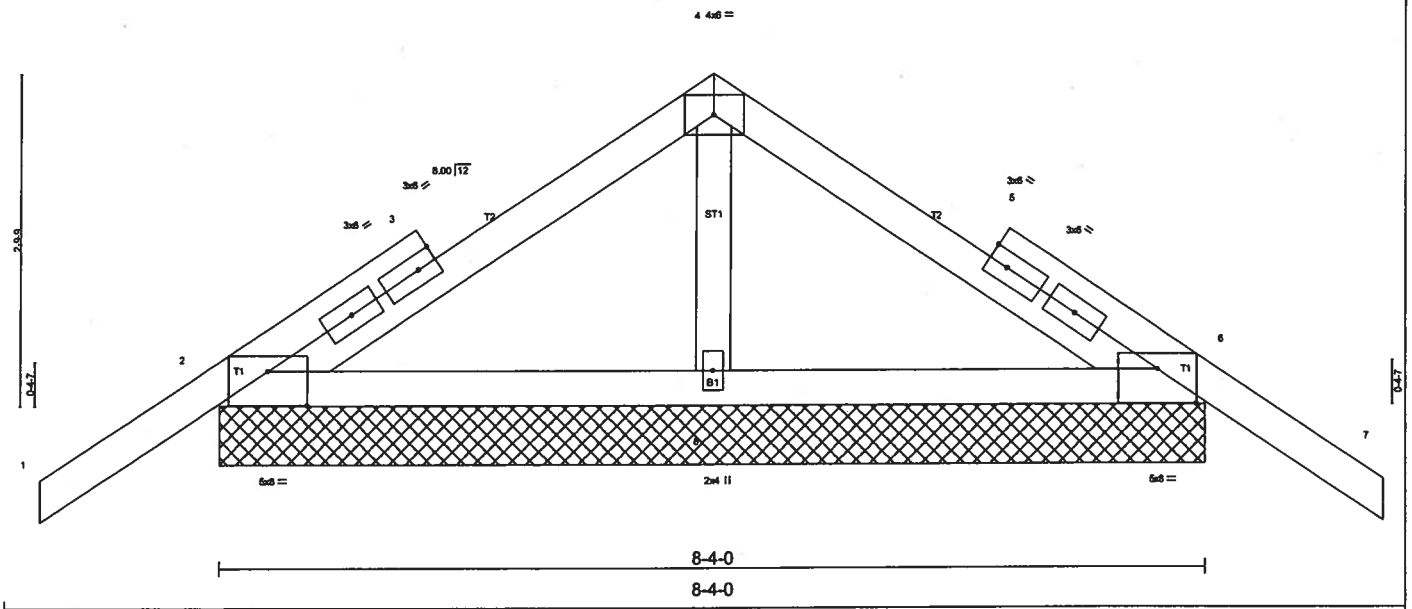
Dwg.#1130051430

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T04G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:16 2005 Page 1

-1-6-0 | 4-2-0 | 8-4-0 | 9-10-0
1-6-0 | 4-2-0 | 4-2-0 | 1-6-0

Scale = 1:18.6



8-4-0
8-4-0

Plate Offsets (X,Y): [2:0-4-0,0-3-7], [6:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.00	6-7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.00	6-7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.11	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 41 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=394/8-4-0, 6=394/8-4-0, 8=754/8-4-0
Max Horz 2=-91(load case 3)
Max Uplift2=-217(load case 5), 6=-230(load case 6), 8=-231(load case 5)
Max Grav 2=409(load case 9), 6=409(load case 10), 8=754(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-13/91, 2-3=-41/100, 3-4=-60/242, 4-5=-46/242, 5-6=-29/100, 6-7=-13/91
BOT CHORD 2-8=-91/136, 6-8=-91/136
WEBS 4-8=-597/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2, 230 lb uplift at joint 6 and 231 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051431

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T05	COMMON	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:16 2005 Page 1

-1-6-0 | 5-6-7 | 10-8-0 | 15-9-9 | 21-4-0 | 22-10-0
1-6-0 | 5-6-7 | 5-1-9 | 5-1-9 | 5-6-7 | 1-6-0
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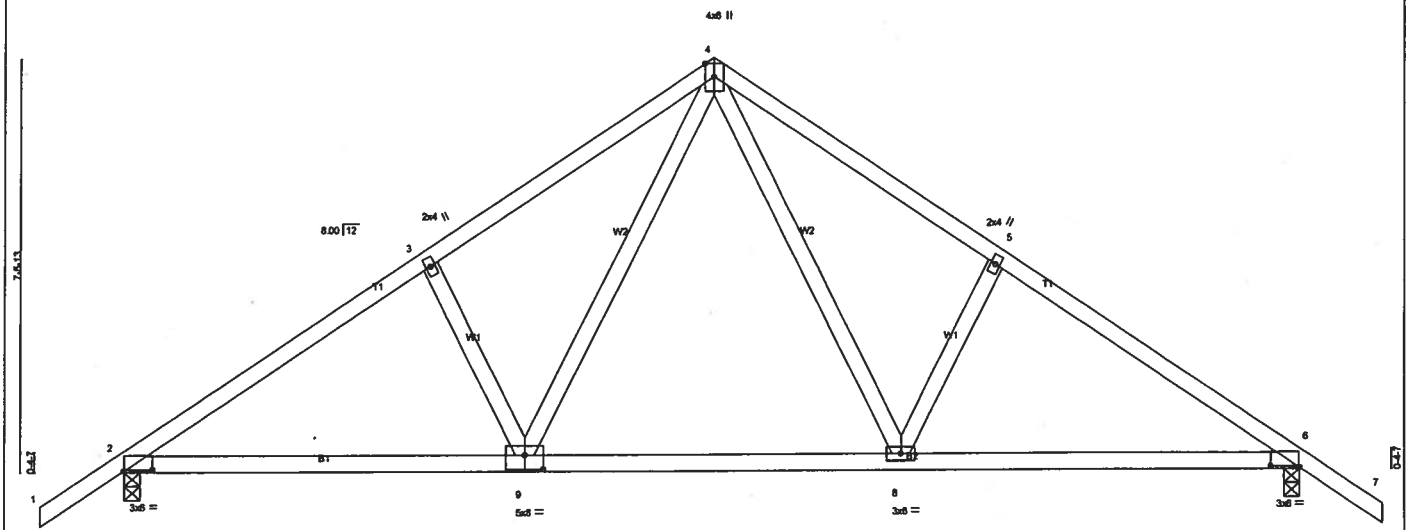


Plate Offsets (X,Y): [2:0-6-3,0-0-6], [6:0-6-3,0-0-6], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.17	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.28	8-9	>918	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 110 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1144/0-3-8, 6=1144/0-3-8
Max Horz 2=-254(load case 3)
Max Uplift2=-440(load case 5), 6=-440(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1566/534, 3-4=-1435/592, 4-5=-1435/592, 5-6=-1566/534, 6-7=0/44
BOT CHORD 2-9=-386/1224, 8-9=-155/836, 6-8=-303/1224
WEBS 3-9=-235/251, 4-9=-285/694, 4-8=-285/694, 5-8=-235/251

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 2 and 440 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-9=-30, 8-9=-80(F=-50), 6-8=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051432

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T05A	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:17 2005 Page 1

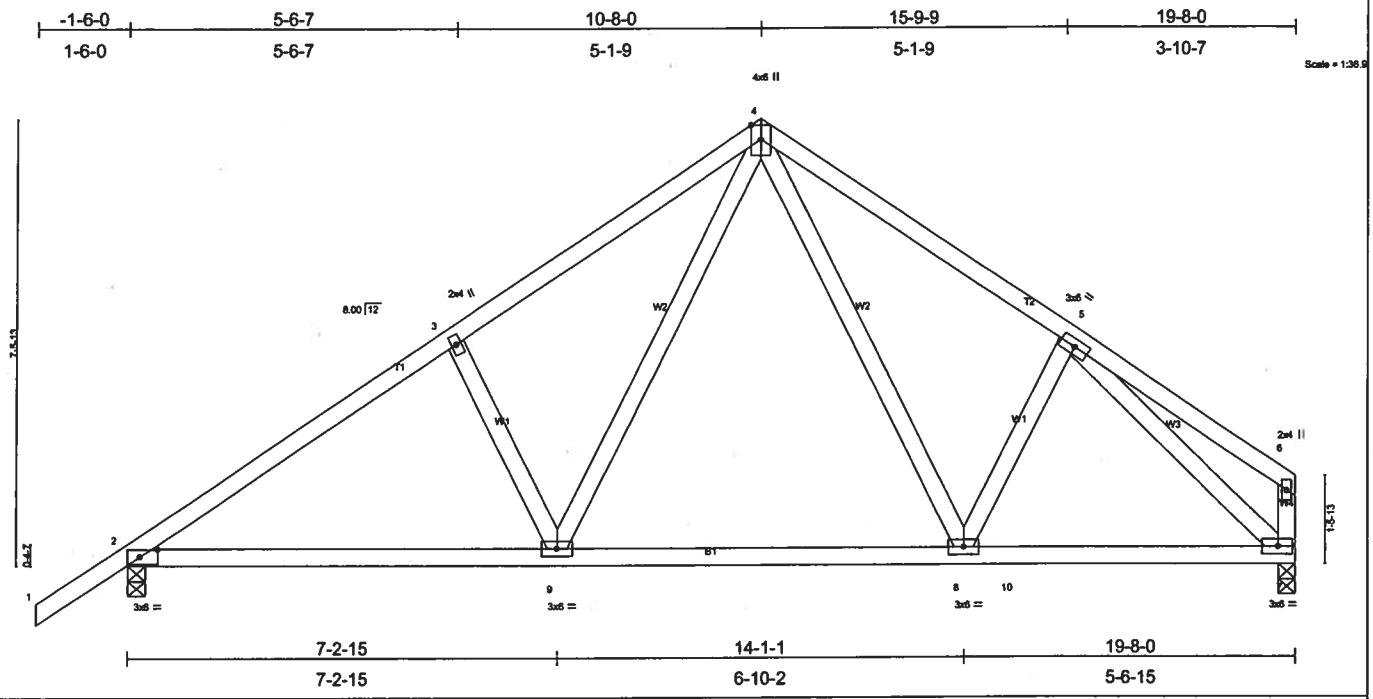


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.16	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.26	8-9	>889	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.55	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 110 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-7-14 oc bracing.

REACTIONS (lb/size) 2=1073/0-3-8, 7=1025/0-3-8
Max Horz 2=266(load case 4)
Max Uplift2=-420(load case 5), 7=-330(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1443/495, 3-4=-1312/553, 4-5=-1174/513, 5-6=-147/86, 6-7=-149/98
BOT CHORD 2-9=-421/1123, 8-9=-190/733, 8-10=-294/898, 7-10=-294/898
WEBS 3-9=-235/250, 4-9=-283/698, 4-8=-214/472, 5-8=-88/158, 5-7=-1166/392

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint 2 and 330 lb uplift at joint 7.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 9-10=-80(F=-50), 7-10=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051433

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T05G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:18 2005 Page 1

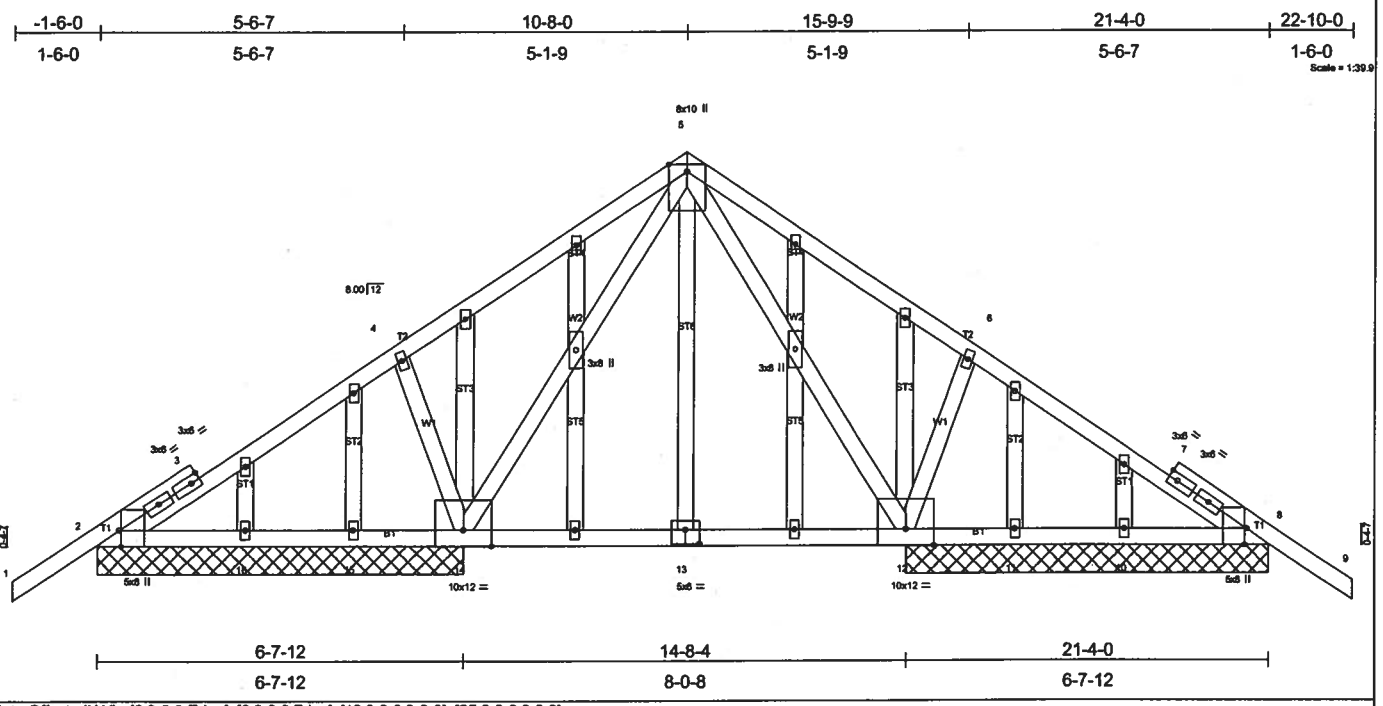


Plate Offsets (X,Y): [2:0-3-8,Edge], [8:0-3-8,Edge], [13:0-3-0,0-3-0], [25:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.06	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.11	12-14	>867	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 12-14.

REACTIONS (lb/size) 2=412/6-7-12, 8=412/6-7-12, 14=1175/6-7-12, 15=-88/6-7-12, 16=208/6-7-12, 12=1175/6-7-12, 11=-88/6-7-12, 10=208/6-7-12
Max Horz 2=-241(load case 3)
Max Uplift2=-187(load case 5), 8=-208(load case 6), 14=-464(load case 5), 15=-88(load case 1), 16=-38(load case 6), 12=-442(load case 6), 11=-88(load case 1), 10=-36(load case 5)
Max Grav 2=422(load case 9), 8=422(load case 10), 14=1175(load case 1), 15=10(load case 6), 16=208(load case 1), 12=1175(load case 1), 11=9(load case 5), 10=208(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/91, 2-3=-129/102, 3-4=-134/230, 4-5=-86/425, 5-6=-56/425, 6-7=-87/230, 7-8=-105/26, 8-9=-11/91
BOT CHORD 2-16=-64/224, 15-16=-64/224, 14-15=-64/224, 13-14=-40/229, 12-13=-40/229, 11-12=-50/190, 10-11=-50/190, 8-10=-50/190
WEBS 4-14=-537/366, 5-14=-507/188, 5-12=-507/191, 6-12=-537/368

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 208 lb uplift at joint 8, 464 lb uplift at joint 14, 88 lb uplift at joint 15, 38 lb uplift at joint 16, 442 lb uplift at joint 12, 88 lb uplift at joint 11 and 36 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

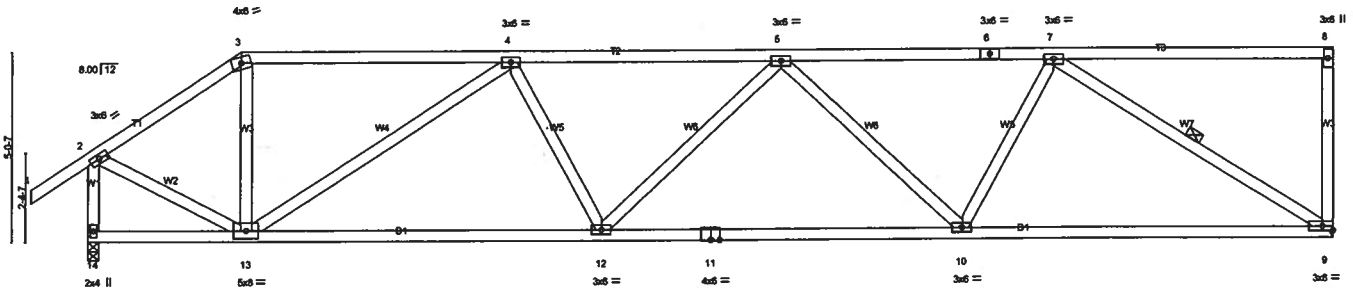
Dwg.#1130051434

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T06	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:19 2005 Page 1

-1-6-0	4-0-0	11-2-12	18-5-8	25-8-4	33-2-8
1-6-0	4-0-0	7-2-12	7-2-12	7-2-12	7-6-4

Scale = 1:58.6



4-0-0	13-7-11	23-3-5	33-2-8
4-0-0	9-7-11	9-7-11	9-11-3

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.27	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.43	10-12	>908	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 369 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=2995/Mechanical, 14=3111/0-3-8
Max Horz 14=240(load case 4)
Max Uplift9=-1407(load case 2), 14=-1239(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-2825/1189, 3-4=-2370/1019, 4-5=-5091/2320, 5-6=-4412/2023, 6-7=-4412/2023, 7-8=-175/69, 8-9=-415/261, 2-14=-2932/1165
BOT CHORD 13-14=-209/41, 12-13=-2208/4732, 11-12=-2442/5173, 10-11=-2442/5173, 9-10=-1764/3705
WEBS 3-13=-380/974, 4-13=-2844/1431, 4-12=-249/794, 5-12=-114/171, 5-10=-1067/588, 7-10=-573/1562, 7-9=-4198/2016, 2-13=-1189/2663

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1407 lb uplift at joint 9 and 1239 lb uplift at joint 14.
7) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 4-0-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-8=-117(F=-63), 13-14=-132(F=-102), 9-13=-65(F=-35)

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

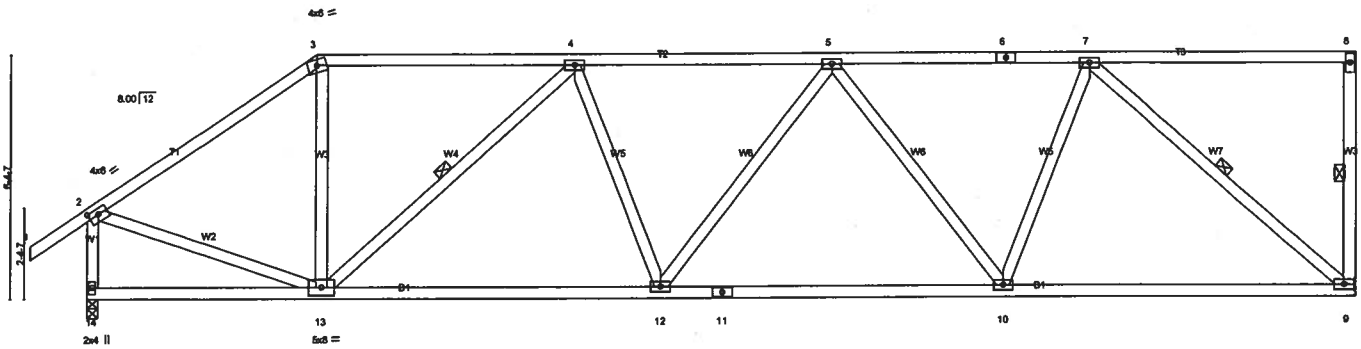
Dwg.#1130051435

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T07	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:19 2005 Page 1

-1-6-0	6-0-0	12-8-12	19-5-8	26-2-4	33-2-8
1-6-0	6-0-0	6-8-12	6-8-12	6-8-12	7-0-4

Scale = 1:57.6



6-0-0	14-11-11	23-11-5	33-2-8
6-0-0	8-11-11	8-11-11	9-3-3

Plate Offsets (X,Y): [2:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.18	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.30	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 199 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-8 oc bracing.
WEBS 1 Row at midpt 8-9, 4-13, 7-9

REACTIONS (lb/size) 9=1380/Mechanical, 14=1474/0-3-8
Max Horz 14=302(load case 5)
Max Uplift9=-623(load case 3), 14=-433(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-1466/577, 3-4=-1159/556, 4-5=-1867/778, 5-6=-1520/645, 6-7=-1520/645, 7-8=-49/14, 8-9=-175/141, 2-14=-1398/604
BOT CHORD 13-14=-300/64, 12-13=-793/1788, 11-12=-815/1825, 10-11=-815/1825, 9-10=-583/1265
WEBS 3-13=-140/422, 4-13=-856/512, 4-12=-13/229, 5-12=-4/81, 5-10=-514/304, 7-10=-195/738, 7-9=-1622/759, 2-13=-475/1157

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 3x6 MT20 unless otherwise indicated.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 9 and 433 lb uplift at joint 14.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051436

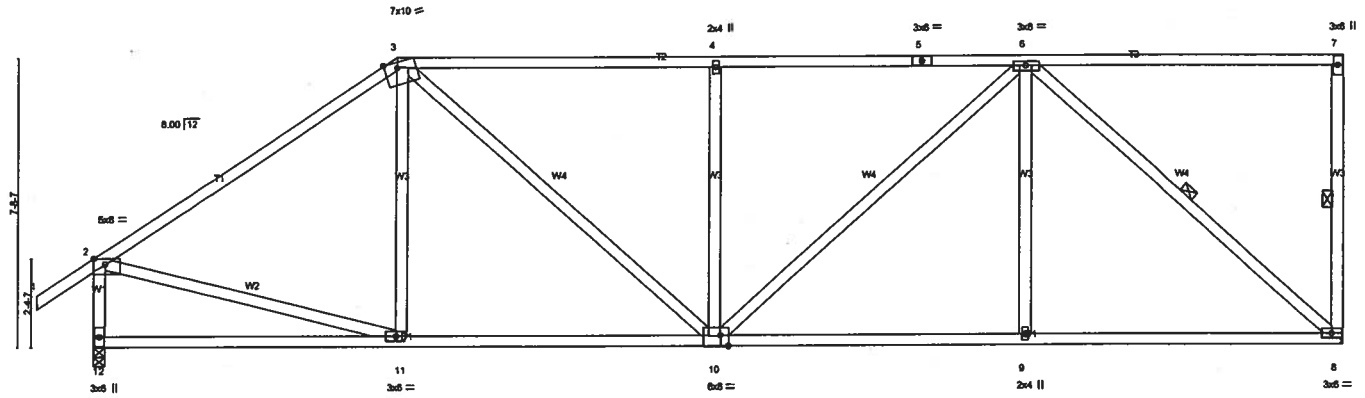
Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T08	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:20 2005 Page 1

-1-6-0 | 8-0-0 | 16-5-7 | 24-9-1 | 33-2-8

1-6-0 | 8-0-0 | 8-5-7 | 8-3-11 | 8-5-7

Scale = 1:58.6



8-0-0 | 16-5-7 | 24-9-1 | 33-2-8

8-0-0 | 8-5-7 | 8-3-11 | 8-5-7

Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,Edge], [10:0-2-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.13	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.22	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 209 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-10 oc bracing.
WEBS 1 Row at midpt 7-8, 6-8

REACTIONS (lb/size) 8=1380/Mechanical, 12=1474/0-3-8
Max Horz 12=363(load case 5)
Max Uplift8=-598(load case 3), 12=-443(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-1529/580, 3-4=-1593/698, 4-5=-1593/698, 5-6=-1593/698, 6-7=-43/21, 7-8=-206/172, 2-12=-1355/593
BOT CHORD 11-12=-415/150, 10-11=-581/1176, 9-10=-518/1225, 8-9=-518/1225
WEBS 3-11=-29/170, 3-10=-390/554, 4-10=-445/410, 6-10=-242/493, 6-9=0/255, 6-8=-1584/671, 2-11=-466/1064

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 598 lb uplift at joint 8 and 443 lb uplift at joint 12.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

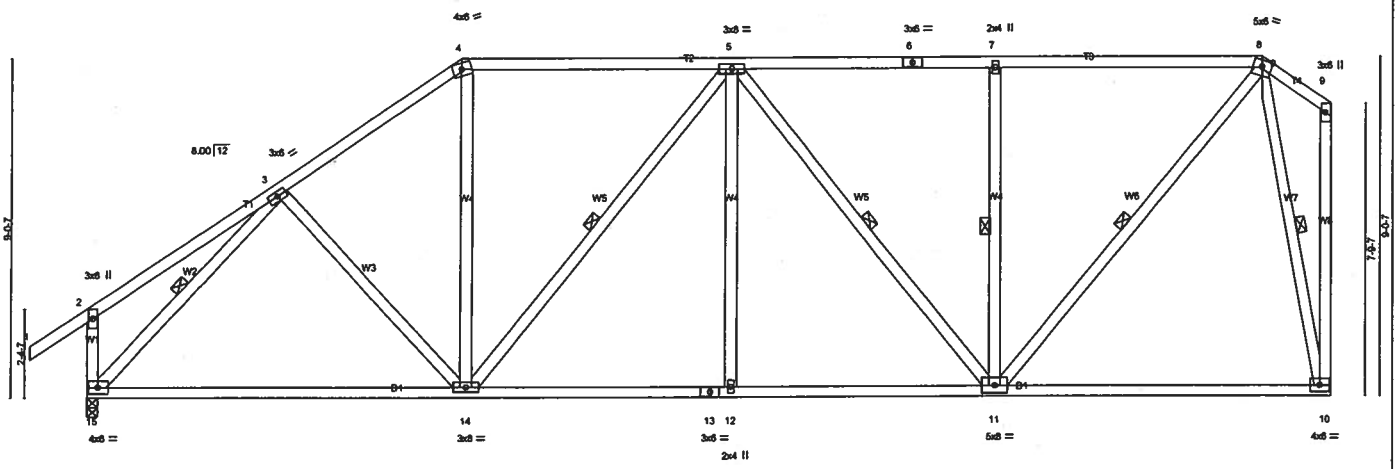
Dwg.#1130051437

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:21 2005 Page 1

-1-6-0	5-1-12	10-0-0	17-1-15	24-2-1	31-4-0	33-2-8
1-6-0	5-1-12	4-10-4	7-1-15	7-0-3	7-1-15	1-10-8

Scale = 1:58.6



10-0-0	17-1-15	24-2-1	31-4-0	33-2-8
10-0-0	7-1-15	7-0-3	7-1-15	1-10-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.88	Vert(LL)	-0.19	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.32	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-7 oc bracing.
WEBS 1 Row at midpt 5-14, 5-11, 7-11, 8-11, 3-15, 8-10

REACTIONS (lb/size) 15=1474/0-3-8, 10=1380/Mechanical
Max Horz 15=387(load case 5)
Max Uplift15=-457(load case 5), 10=-561(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-267/210, 3-4=-1423/608, 4-5=-1139/575, 5-6=-1072/496, 6-7=-1072/496, 7-8=-1072/496, 8-9=-62/35, 2-15=-342/295, 9-10=-105/37
BOT CHORD 14-15=-600/1037, 13-14=-637/1330, 12-13=-637/1330, 11-12=-637/1330, 10-11=-132/272
WEBS 3-14=-183/239, 4-14=-100/409, 5-14=-389/307, 5-12=0/176, 5-11=-415/219, 7-11=-392/331, 8-11=-580/1264, 3-15=-1320/431, 8-10=-1281/703

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 15 and 561 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051438

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T10	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:22 2005 Page 1

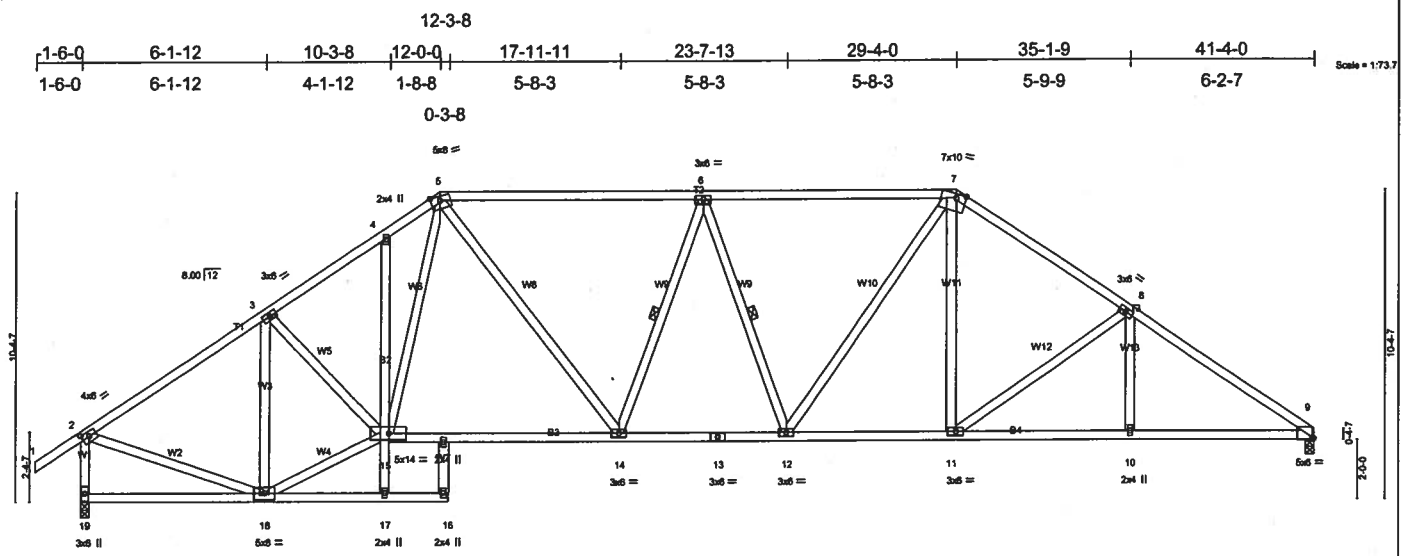


Plate Offsets (X,Y): [2:0-2-14,0-2-0], [7:0-4-0,Edge], [9:0-0-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.23	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.38	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 279 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 6-12

REACTIONS (lb/size) 9=1739/0-3-8, 19=1861/0-3-8
Max Horz 19=362(load case 4)
Max Uplift9=-487(load case 6), 19=-570(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-1900/722, 3-4=-2475/979, 4-5=-2336/990, 5-6=-2200/969, 6-7=-2172/969, 7-8=-2314/950, 8-9=-2770/1007, 2-19=-1763/728
BOT CHORD 18-19=-359/313, 17-18=-54/0, 16-17=0/0, 15-17=0/122, 4-15=-187/140, 14-15=-750/1888, 13-14=-841/2271, 12-13=-841/2271, 11-12=-524/1866, 10-11=-730/2210, 9-10=-730/2210
WEBS 3-18=-1134/440, 3-15=-198/705, 5-14=-337/608, 6-14=-288/333, 6-12=-380/341, 7-12=-353/623, 7-11=-135/420, 8-11=-433/305, 8-10=0/212, 2-18=-389/1492, 15-18=-677/1733, 5-15=-235/484

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 487 lb uplift at joint 9 and 570 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

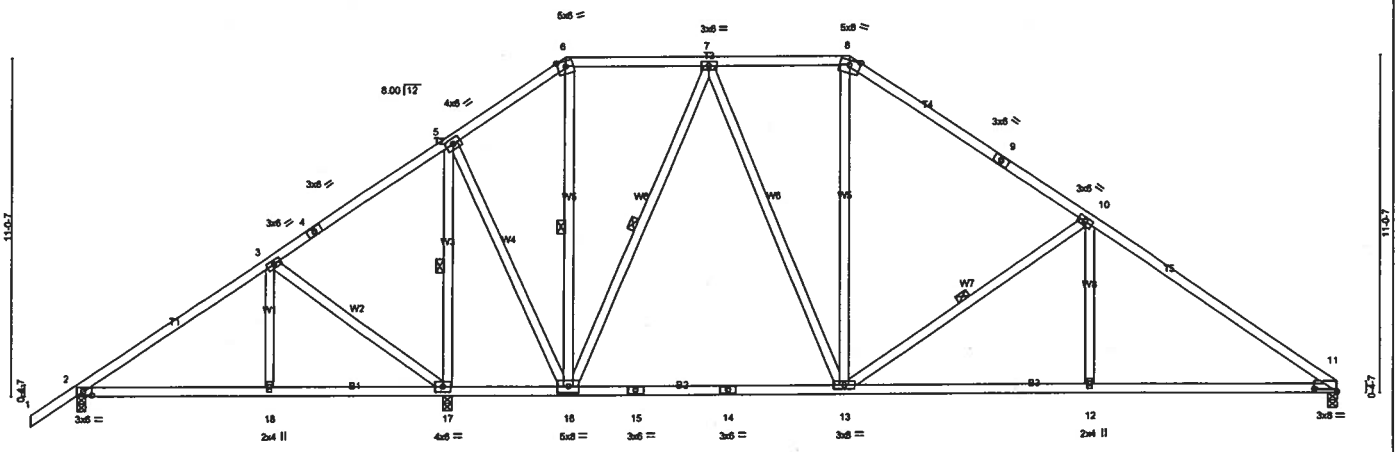
Dwg.#1130051439

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T11	HIP	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:47 2005 Page 1

1-6-0	6-3-5	12-1-12	16-0-0	20-8-0	25-4-0	33-1-9	41-4-0
1-6-0	6-3-5	5-10-7	3-10-4	4-8-0	4-8-0	7-9-9	8-2-7

Scale = 1:72.1



6-3-5	12-1-12	16-0-0	25-4-0	33-1-9	41-4-0
6-3-5	5-10-7	3-10-4	9-4-1	7-9-9	8-2-7

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [11:0-8-7,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.20	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.32	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 263 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 16-17.
WEBS 1 Row at midpt 5-17, 6-16, 7-16, 10-13

REACTIONS (lb/size) 2=438/0-3-8, 17=1946/0-3-8, 11=1153/0-3-8
Max Horz 2=394(load case 4)
Max Uplift2=-308(load case 5), 17=-801(load case 5), 11=-368(load case 6)
Max Grav 2=481(load case 9), 17=1946(load case 1), 11=1153(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-390/231, 3-4=-159/209, 4-5=-144/341, 5-6=-372/264, 6-7=-256/263, 7-8=-751/469, 8-9=-901/456, 9-10=-1028/423, 10-11=-1667/553
BOT CHORD 2-18=-352/252, 17-18=-352/252, 16-17=-217/347, 15-16=-169/551, 14-15=-169/551, 13-14=-169/551, 12-13=-330/1290, 11-12=-330/1290
WEBS 3-18=-240/212, 3-17=-502/529, 5-17=-1540/617, 5-16=-297/1076, 6-16=-54/100, 7-16=-762/343, 7-13=-220/516, 8-13=-7/200, 10-13=-659/433, 10-12=0/277

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2, 801 lb uplift at joint 17 and 368 lb uplift at joint 11.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

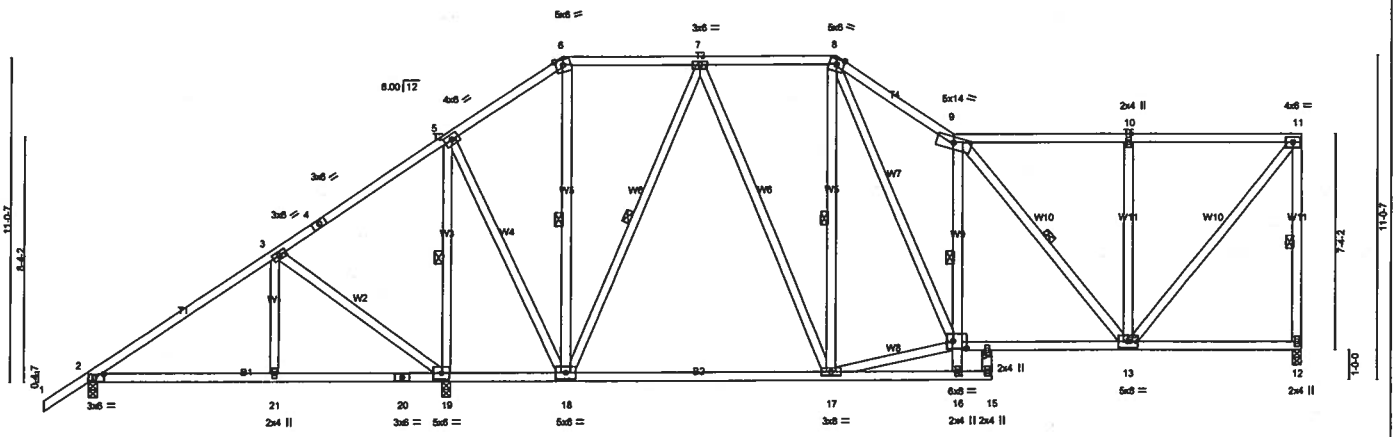
Dwg.#1130051440

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:23 2005 Page 1

1-6-0	6-3-5	12-1-12	16-0-0	20-8-0	25-4-0	29-4-8	30-8-0	35-4-4	41-4-0
1-6-0	6-3-5	5-10-7	3-10-4	4-8-0	4-8-0	4-0-8	1-3-8	4-8-4	5-11-12

Scale = 1:74.9



6-3-5	12-1-12	16-0-0	25-4-0	29-4-8	30-8-0	35-4-4	41-4-0
6-3-5	5-10-7	3-10-4	9-4-1	4-0-8	1-3-8	4-8-4	5-11-12

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [14:0-5-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.17	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.29	17-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 319 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 18-19
9-11-5 oc bracing: 13-14.
WEBS 1 Row at midpt 11-12, 5-19, 6-18, 7-18, 8-17, 9-16, 9-13

REACTIONS (lb/size) 12=1172/0-3-8, 2=431/0-3-8, 19=1968/0-3-8
Max Horz 2=470(load case 5)
Max Uplift12=-435(load case 4), 2=-242(load case 5), 19=-856(load case 5)
Max Grav 12=1172(load case 10), 2=478(load case 9), 19=1968(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-384/81, 3-4=-249/220, 4-5=-234/352, 5-6=-371/138, 6-7=-255/156, 7-8=-752/351, 8-9=-1465/569, 9-10=-808/285, 10-11=-807/285, 11-12=-1091/444
BOT CHORD 2-21=-317/247, 20-21=-317/247, 19-20=-317/247, 18-19=-227/121, 17-18=-215/559, 16-17=-10/24, 15-16=0/0, 13-14=-401/1158, 12-13=-9/20
WEBS 3-21=-239/213, 3-19=-502/530, 5-19=-1563/701, 5-18=-365/1093, 6-18=-51/96, 7-18=-788/352, 7-17=-152/498, 8-17=-398/239, 14-17=-235/755, 14-16=0/64, 9-14=-626/357, 9-13=-541/178, 10-13=-350/312, 11-13=-435/1235, 8-14=-425/1032

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 12, 242 lb uplift at joint 2 and 856 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

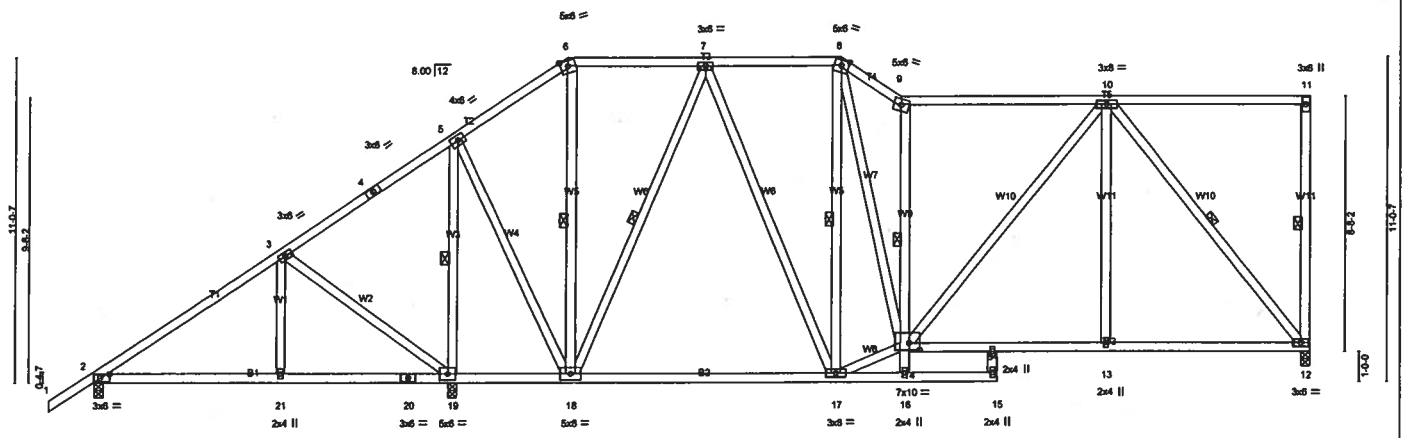
Dwg.#1130051441

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:24 2005 Page 1

1-6-0	8-2-7	16-0-0	20-8-0	25-4-0	27-4-8	30-8-0	34-4-4	41-4-0
1-6-0	8-2-7	7-9-9	4-8-0	4-8-0	2-0-8	3-3-8	3-8-4	6-11-12

Scale = 1:74.9



6-3-5	12-1-12	16-0-0	25-4-0	27-4-8	30-8-0	34-4-4	41-4-0
6-3-5	5-10-7	3-10-4	9-4-1	2-0-8	3-3-8	3-8-4	6-11-12

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [14:0-4-8,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.23	15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.39	15	>903	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 329 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 18-19.
WEBS 1 Row at midpt 11-12, 5-19, 6-18, 7-18, 8-17, 9-16, 10-12

REACTIONS (lb/size) 12=1208/0-3-8, 2=443/0-3-8, 19=1975/0-3-8
Max Horz 2=513(load case 5)
Max Uplift12=-492(load case 4), 2=-232(load case 5), 19=-895(load case 4)
Max Grav 12=1208(load case 10), 2=486(load case 9), 19=1975(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-399/65, 3-4=-272/199, 4-5=-257/331, 5-6=-395/114, 6-7=-275/136, 7-8=-789/319, 8-9=-1325/486, 9-10=-1041/366, 10-11=-21/10, 11-12=-160/133
BOT CHORD 2-21=-348/259, 20-21=-348/259, 19-20=-348/259, 18-19=-209/82, 17-18=-247/588, 16-17=-6/44, 15-16=0/0, 13-14=-312/800, 12-13=-312/800
WEBS 3-19=-501/530, 5-19=-1569/700, 5-18=-369/1098, 6-18=-41/86, 7-18=-810/340, 7-17=-131/518, 8-17=-593/253, 14-17=-297/812, 14-16=0/121, 9-14=-949/438, 10-14=-109/386, 10-13=0/240, 10-12=-1233/479, 8-14=-425/1177, 3-21=-239/213

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 492 lb uplift at joint 12, 232 lb uplift at joint 2 and 895 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

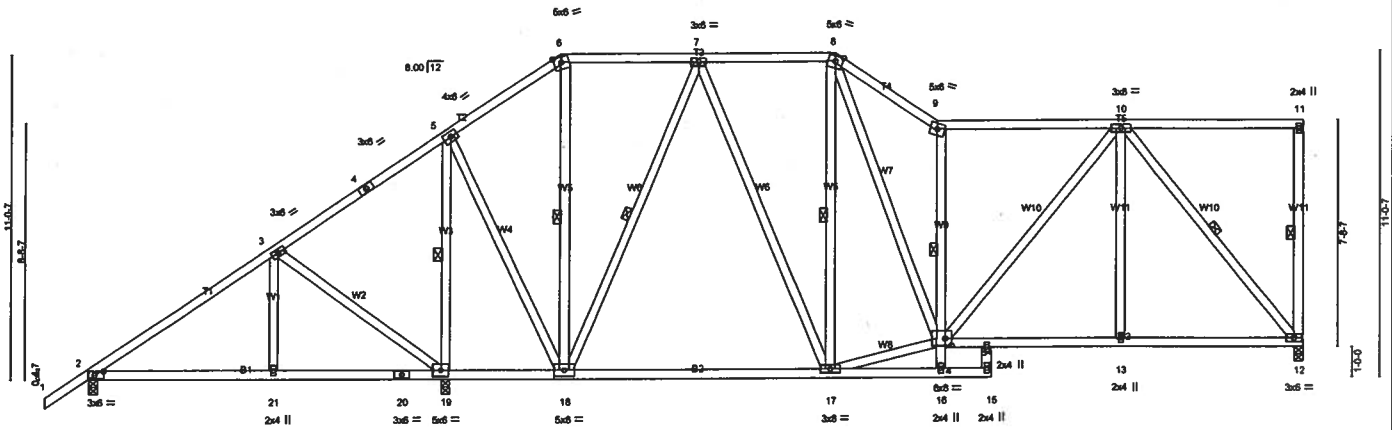
Dwg.#1130051442

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:25 2005 Page 1

1-6-0	8-2-7	16-0-0	20-8-0	25-4-0	28-10-0	30-8-0	35-1-0	41-4-0
1-6-0	8-2-7	7-9-9	4-8-0	4-8-0	3-6-0	1-10-0	4-5-0	6-3-0

Scale = 1:74.9



6-3-5	12-1-12	16-0-0	25-4-0	28-10-0	30-8-0	35-1-0	41-4-0
6-3-5	5-10-7	3-10-4	9-4-1	3-6-0	1-10-0	4-5-0	6-3-0

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [14:0-2-8,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.17	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.30	17-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 322 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 18-19.
WEBS 1 Row at midpt 11-12, 5-19, 6-18, 7-18, 8-17, 9-16, 10-12

REACTIONS (lb/size) 12=1182/0-3-8, 2=440/0-3-8, 19=1960/0-3-8
Max Horz 2=482(load case 5)
Max Uplift12=-452(load case 4), 2=-240(load case 5), 19=-855(load case 4)
Max Grav 12=1182(load case 10), 2=484(load case 9), 19=1960(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-395/77, 3-4=-254/204, 4-5=-239/337, 5-6=-383/133, 6-7=-265/151, 7-8=-763/343, 8-9=-1429/548, 9-10=-1108/379, 10-11=-20/9, 11-12=-143/118
BOT CHORD 2-21=-325/256, 20-21=-325/256, 19-20=-325/256, 18-19=-214/109, 17-18=-226/570, 16-17=-8/35, 15-16=0/0, 13-14=-294/807, 12-13=-294/807
WEBS 3-19=-501/530, 5-19=-1556/700, 5-18=-364/1087, 6-18=-46/91, 7-18=-789/349, 7-17=-146/497, 8-17=-416/235, 14-17=-245/756, 14-16=0/78, 9-14=-1011/474, 10-14=-150/480, 10-13=0/213, 10-12=-1238/448, 8-14=-417/1036, 3-21=-239/213

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 12, 240 lb uplift at joint 2 and 855 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

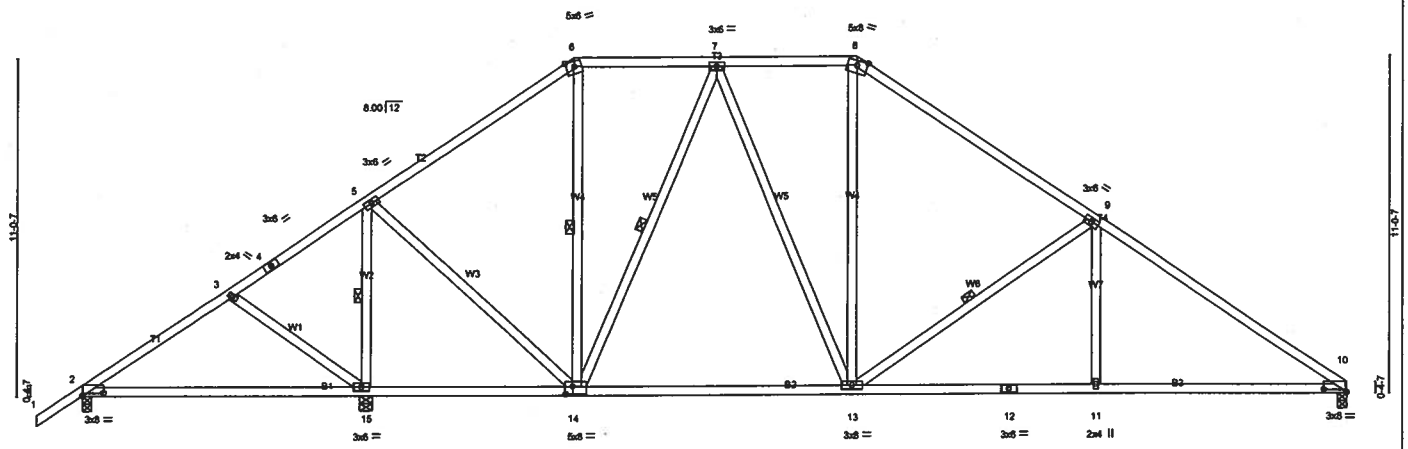
Dwg.#1130051443

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:26 2005 Page 1

1-6-0	4-10-1	9-3-4	16-0-0	20-8-0	25-4-0	33-1-9	41-4-0
1-6-0	4-10-1	4-5-3	6-8-12	4-8-0	4-8-0	7-9-9	8-2-7

Scale = 1:72.1



9-3-4	16-0-0	25-4-0	33-1-9	41-4-0
9-3-4	6-8-12	9-4-1	7-9-9	8-2-7

Plate Offsets (X,Y): [2:0-8-3,0-1-2], [10:0-8-7,0-1-2], [14:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.31	2-15	>350	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	0.25	2-15	>437	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15, 6-14, 7-14, 9-13

REACTIONS (lb/size) 10=1283/0-3-8, 2=277/0-3-8, 15=1977/0-4-15
Max Horz 2=394(load case 4)
Max Uplift10=-412(load case 6), 2=-237(load case 5), 15=-751(load case 5)
Max Grav 10=1283(load case 1), 2=330(load case 9), 15=1977(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-122/230, 3-4=-166/271, 4-5=-147/363, 5-6=-772/393, 6-7=-560/413, 7-8=-945/564, 8-9=-1259/568, 9-10=-1895/662
BOT CHORD 2-15=-248/275, 14-15=-241/283, 13-14=-231/794, 12-13=-420/1477, 11-12=-420/1477, 10-11=-420/1477
WEBS 3-15=-248/254, 5-15=-1586/622, 5-14=-263/1091, 6-14=-93/144, 7-14=-646/329, 7-13=-182/405, 8-13=-48/300, 9-13=-655/430, 9-11=0/276

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 10, 237 lb uplift at joint 2 and 751 lb uplift at joint 15.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051444

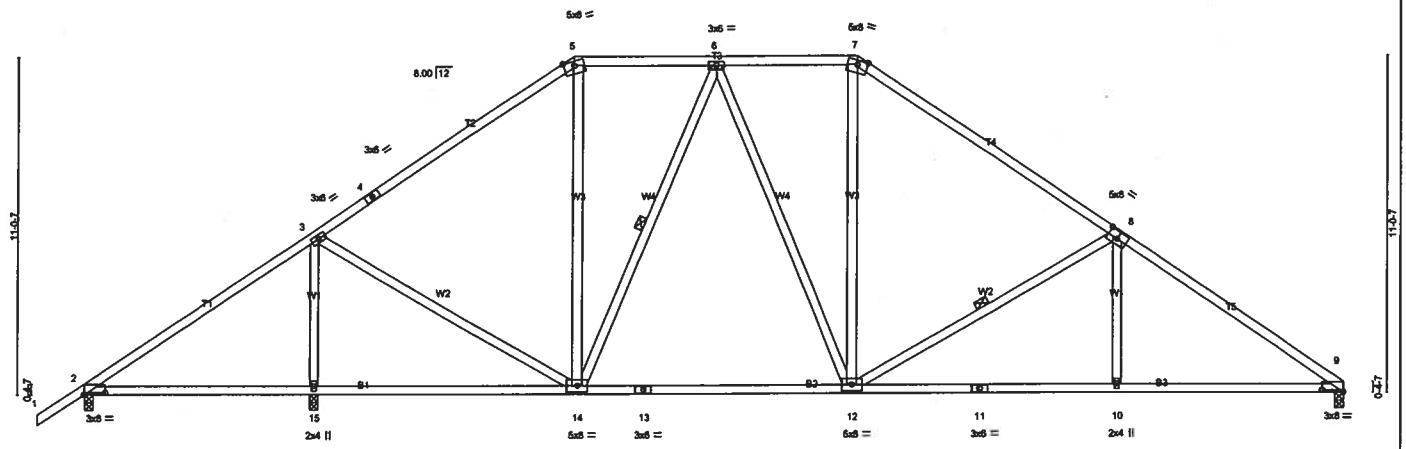
Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T16	HIP	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:27 2005 Page 1

1-6-0 | 7-5-12 | 16-0-0 | 20-8-0 | 25-4-0 | 33-10-4 | 41-4-0

1-6-0 | 7-5-12 | 8-6-4 | 4-8-0 | 4-8-0 | 8-6-4 | 7-5-12

Scale = 1:72.1



7-5-12 | 16-0-0 | 25-4-0 | 33-10-4 | 41-4-0

7-5-12 | 8-6-4 | 9-4-1 | 8-6-4 | 7-5-12

Plate Offsets (X,Y): [2:0-8-3,0-0-14], [8:0-4-0,0-3-0], [9:0-8-3,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.20	2-15	>448	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	0.17	2-15	>525	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 244 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 8-12

REACTIONS (lb/size) 9=1376/0-3-8, 2=223/0-3-8, 15=1937/0-3-8
Max Horz 2=394(load case 4)
Max Uplift9=-440(load case 6), 2=-208(load case 5), 15=-703(load case 5)
Max Grav 9=1376(load case 1), 2=271(load case 9), 15=1937(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-200/323, 3-4=-1088/464, 4-5=-956/500, 5-6=-797/518, 6-7=-1091/631, 7-8=-1440/632, 8-9=-2105/750
BOT CHORD 2-15=-202/253, 14-15=-202/253, 13-14=-274/972, 12-13=-274/972, 11-12=-509/1666, 10-11=-509/1666, 9-10=-510/1662
WEBS 3-15=-1664/728, 3-14=-224/1102, 5-14=-134/257, 6-14=-547/321, 6-12=-177/339, 7-12=-67/361, 8-12=-670/435, 8-10=0/272

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 9, 208 lb uplift at joint 2 and 703 lb uplift at joint 15.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1130051445

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:27 2005 Page 1

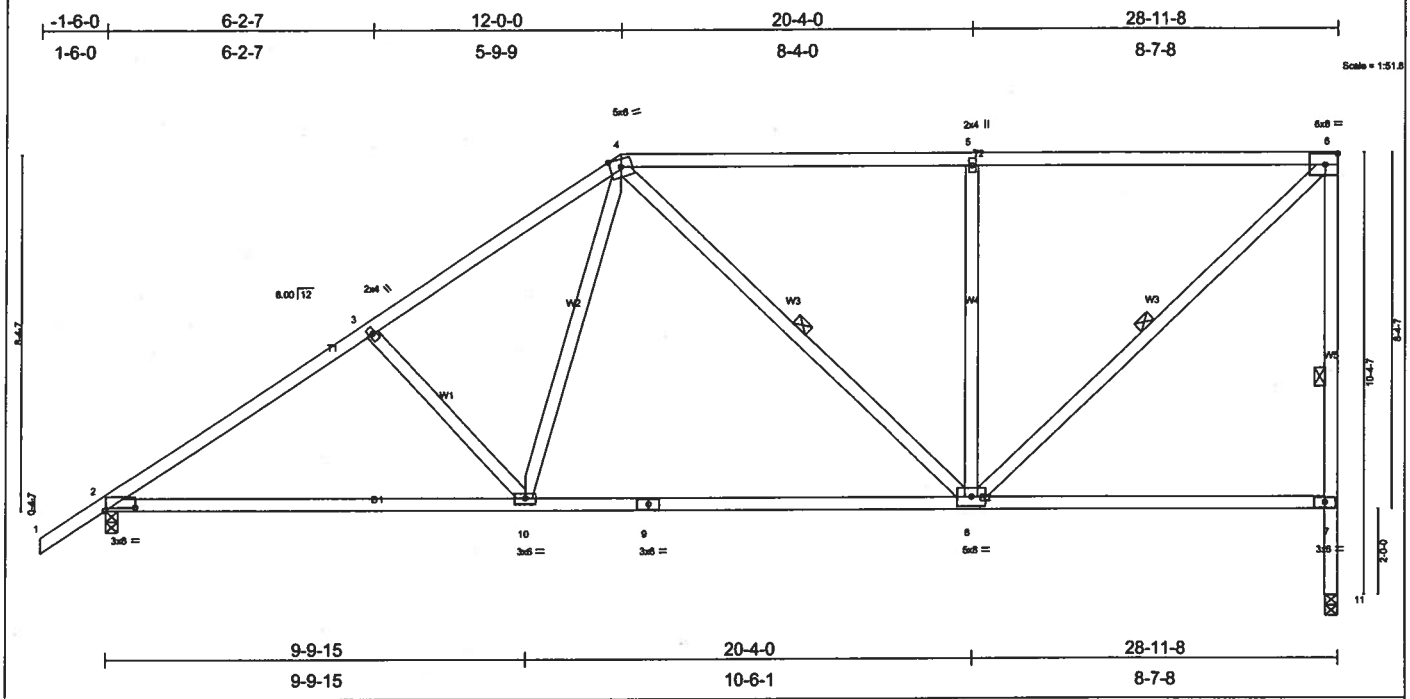


Plate Offsets (X,Y): [2:0-8-3,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.37	8-10	>934	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.11	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-11 oc bracing.
WEBS 1 Row at midpt 6-11, 4-8, 6-8

REACTIONS (lb/size) 11=1201/0-3-8, 2=1295/0-3-8
Max Horz 2=434(load case 5)
Max Uplift11=-435(load case 4), 2=-429(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1731/565, 3-4=-1531/557, 4-5=-962/411, 5-6=-962/412, 7-11=-1201/507, 6-7=-1087/523
BOT CHORD 2-10=-714/1378, 9-10=-505/1062, 8-9=-505/1062, 7-8=-18/38
WEBS 3-10=-261/292, 4-10=-163/543, 4-8=-144/172, 5-8=-492/438, 6-8=-546/1276

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 11 and 429 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#1130051446

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:28 2005 Page 1

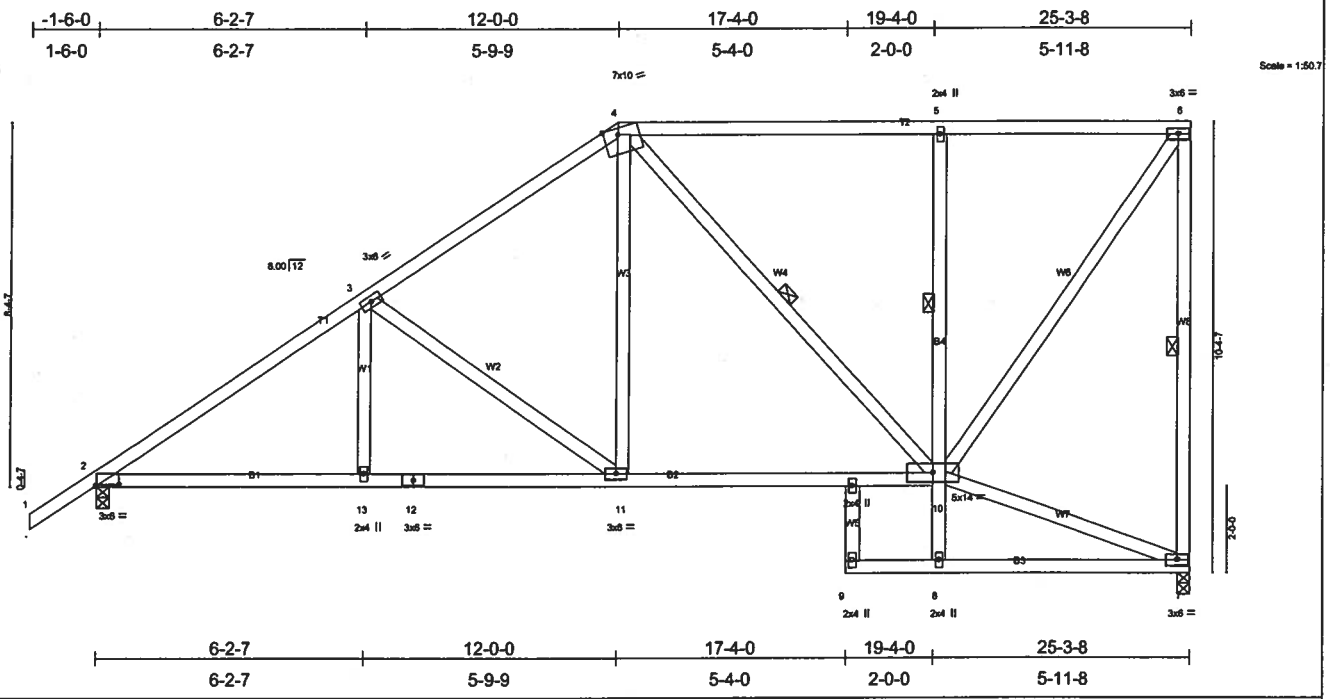


Plate Offsets (X,Y): [2:0-6-3,0-0-6], [4:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.10	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.16	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 5-10
WEBS 1 Row at midpt 6-7, 4-10

REACTIONS (lb/size) 7=1094/0-3-8, 2=1159/0-3-8
Max Horz 2=434(load case 5)
Max Uplift7=-364(load case 4), 2=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1573/434, 3-4=-1124/387, 4-5=-654/262, 5-6=-642/260, 6-7=-1027/453
BOT CHORD 2-13=-606/1223, 12-13=-606/1223, 11-12=-606/1223, 10-11=-398/874, 8-10=0/154, 5-10=-387/351, 8-9=0/0, 7-8=-25/0
WEBS 3-13=0/194, 3-11=-437/291, 4-11=-125/455, 4-10=-323/216, 7-10=0/37, 6-10=-448/1099

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 364 lb uplift at joint 7 and 381 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1130051447

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T19	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:29 2005 Page 1

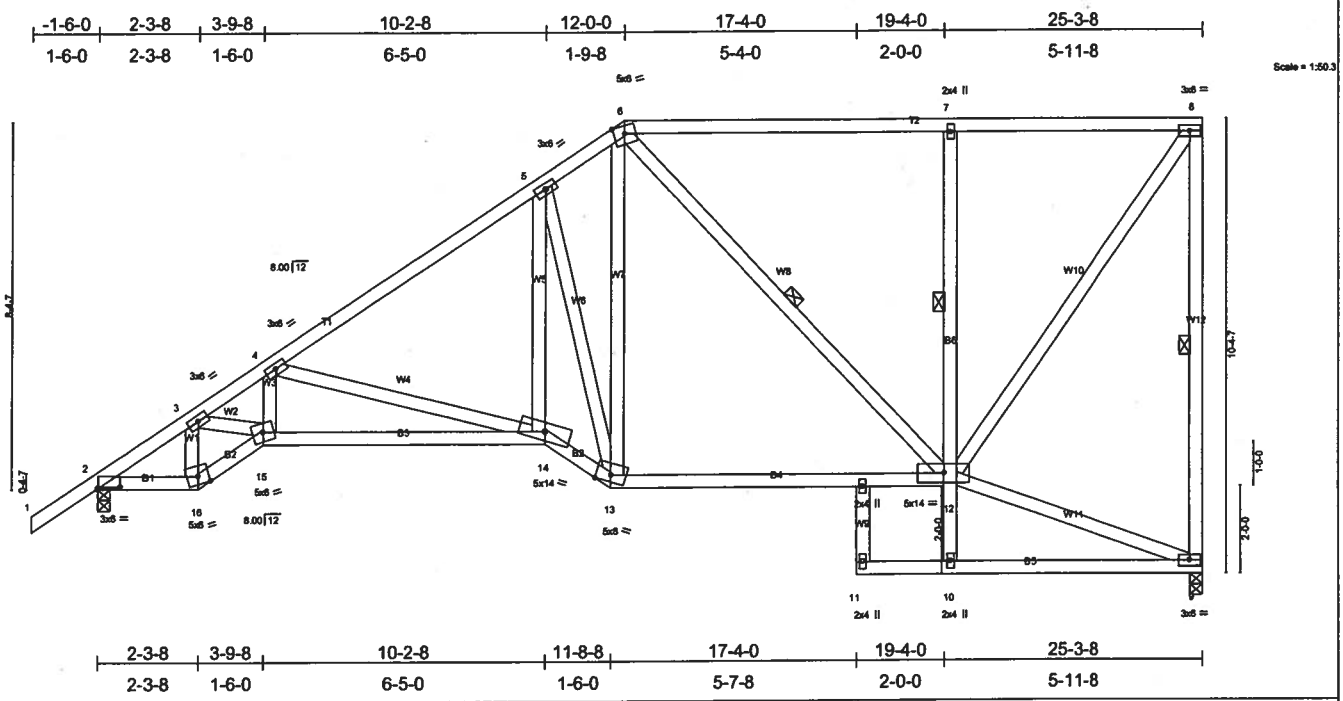


Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.16	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.26	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.14	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 7-12
WEBS 1 Row at midpt 8-9, 6-12

REACTIONS (lb/size) 9=1094/0-3-8, 2=1159/0-3-8
Max Horz 2=434(load case 5)
Max Uplift9=-364(load case 4), 2=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1605/408, 3-4=-2608/949, 4-5=-1455/488, 5-6=-1073/454, 6-7=-655/262, 7-8=-643/259, 8-9=-1027/451
BOT CHORD 2-16=-615/1240, 15-16=-684/1438, 14-15=-1078/2136, 13-14=-659/1364, 12-13=-397/873, 10-12=0/154, 7-12=-388/336, 10-11=0/0, 9-10=-27/0
WEBS 3-16=-791/391, 3-15=-538/1073, 4-15=-193/658, 4-14=-1038/567, 5-14=-478/1083, 5-13=-1129/631, 6-13=-271/542, 6-12=-321/209, 9-12=0/40, 8-12=-446/1100

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 364 lb uplift at joint 9 and 381 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#1130051448

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:29 2005 Page 1

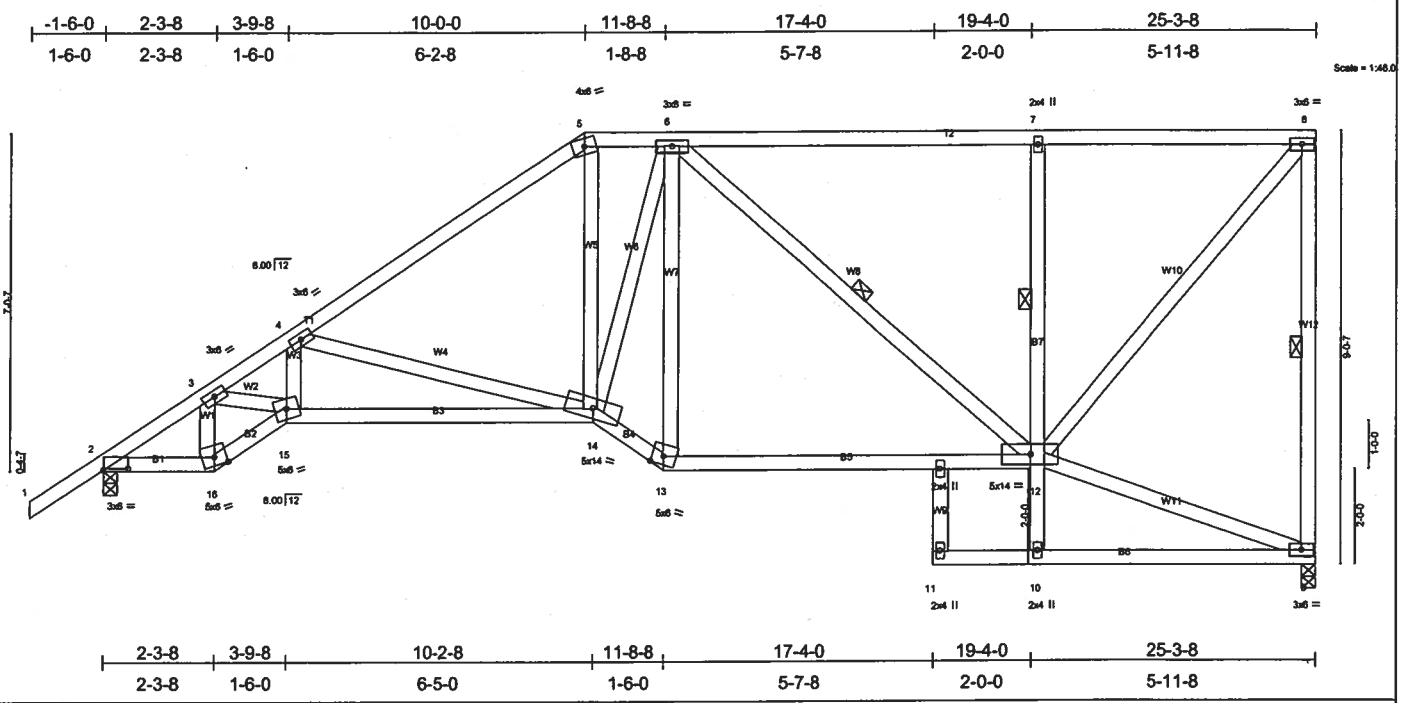


Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.14	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.23	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B7 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 7-12
WEBS 1 Row at midpt 8-9, 6-12

REACTIONS (lb/size) 9=1094/0-3-8, 2=1159/0-3-8
Max Horz 2=372(load case 5)
Max Uplift9=-372(load case 3), 2=-383(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1609/445, 3-4=-2595/958, 4-5=-1457/522, 5-6=-1162/517, 6-7=-787/304, 7-8=-769/299, 8-9=-1027/437
BOT CHORD 2-16=-588/1244, 15-16=-651/1439, 14-15=-1026/2120, 13-14=-535/1230, 12-13=-447/1042, 10-12=0/154, 7-12=-395/336, 10-11=0/0, 9-10=-25/0
WEBS 3-16=-786/371, 3-15=-510/1051, 4-15=-180/658, 4-14=-1016/532, 6-14=-318/490, 6-13=-512/325, 6-12=-340/192, 9-12=0/40, 8-12=-459/1175, 5-14=-259/562

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 9 and 383 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

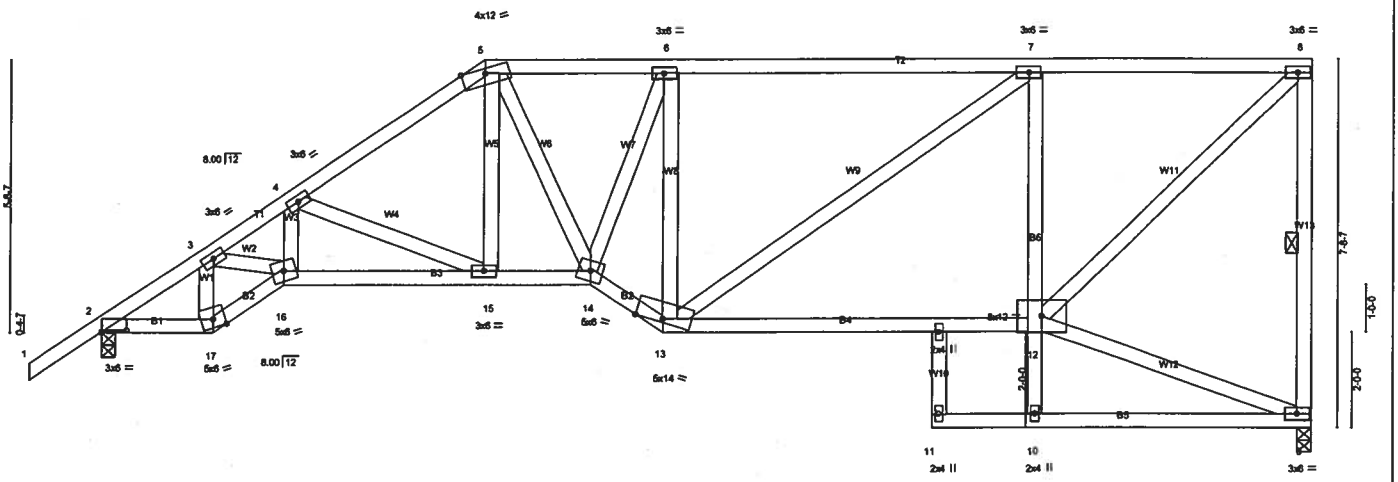
Dwg.#1130051449

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:30 2005 Page 1

-1-6-0	2-3-8	3-9-8	8-0-0	10-2-8	11-8-8	17-4-0	19-4-0	25-3-8
1-6-0	2-3-8	1-6-0	4-2-8	2-2-8	1-6-0	5-7-8	2-0-0	5-11-8

Scale = 1:46.0



2-3-8	3-9-8	8-0-0	10-2-8	11-8-8	17-4-0	19-4-0	25-3-8
2-3-8	1-6-0	4-2-8	2-2-8	1-6-0	5-7-8	2-0-0	5-11-8

Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.13	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 8-9

REACTIONS (lb/size) 9=1094/0-3-8, 2=1159/0-3-8
Max Horz 2=311(load case 5)
Max Uplift9=-414(load case 3), 2=-380(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1629/481, 3-4=-2515/950, 4-5=-1648/622, 5-6=-1457/612, 6-7=-1305/531, 7-8=-968/365, 8-9=-1025/427
BOT CHORD 2-17=-565/1264, 16-17=-630/1437, 15-16=-934/2038, 14-15=-572/1335, 13-14=-626/1473, 12-13=-373/996, 10-12=0/153, 7-12=-610/398, 10-11=0/0, 9-10=-18/0
WEBS 3-17=-744/371, 3-16=-439/936, 4-16=-198/608, 4-15=-772/394, 5-15=-136/455, 5-14=-246/269, 6-14=-269/493, 6-13=-788/487, 7-13=-193/377, 9-12=0/37, 8-12=-495/1307

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 414 lb uplift at joint 9 and 380 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#1130051450

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:31 2005 Page 1

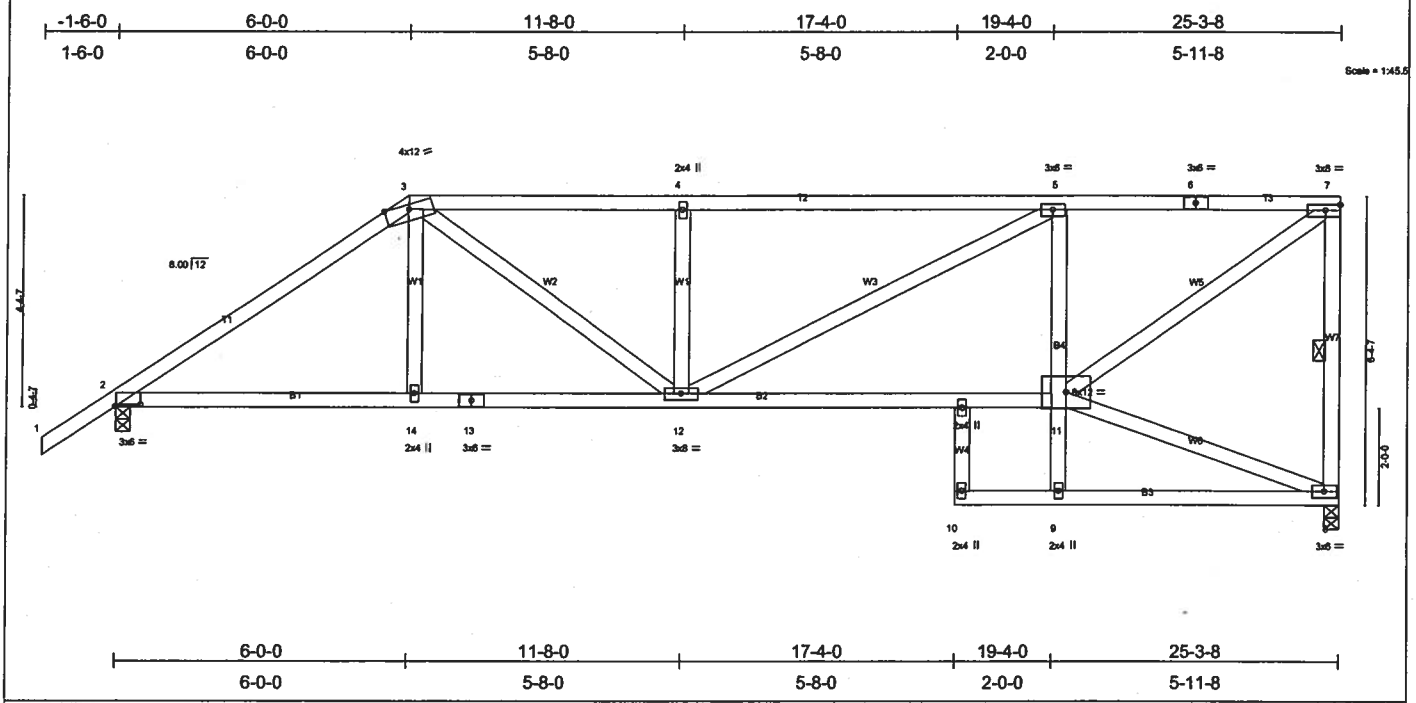


Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.15	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.25	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-8

REACTIONS (lb/size) 8=1094/0-3-8, 2=1159/0-3-8
Max Horz 2=249(load case 5)
Max Uplift8=-447(load case 3), 2=-372(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1574/532, 3-4=-1714/668, 4-5=-1714/668, 5-6=-1285/504, 6-7=-1285/504, 7-8=-1025/456
BOT CHORD 2-14=-512/1225, 13-14=-511/1232, 12-13=-511/1232, 11-12=-518/1334, 9-11=0/152, 5-11=-593/406, 9-10=0/0, 8-9=-17/0
WEBS 3-14=0/185, 3-12=-386/594, 4-12=-358/341, 5-12=-205/430, 8-11=0/40, 7-11=-606/1545

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 447 lb uplift at joint 8 and 372 lb uplift at joint 2.

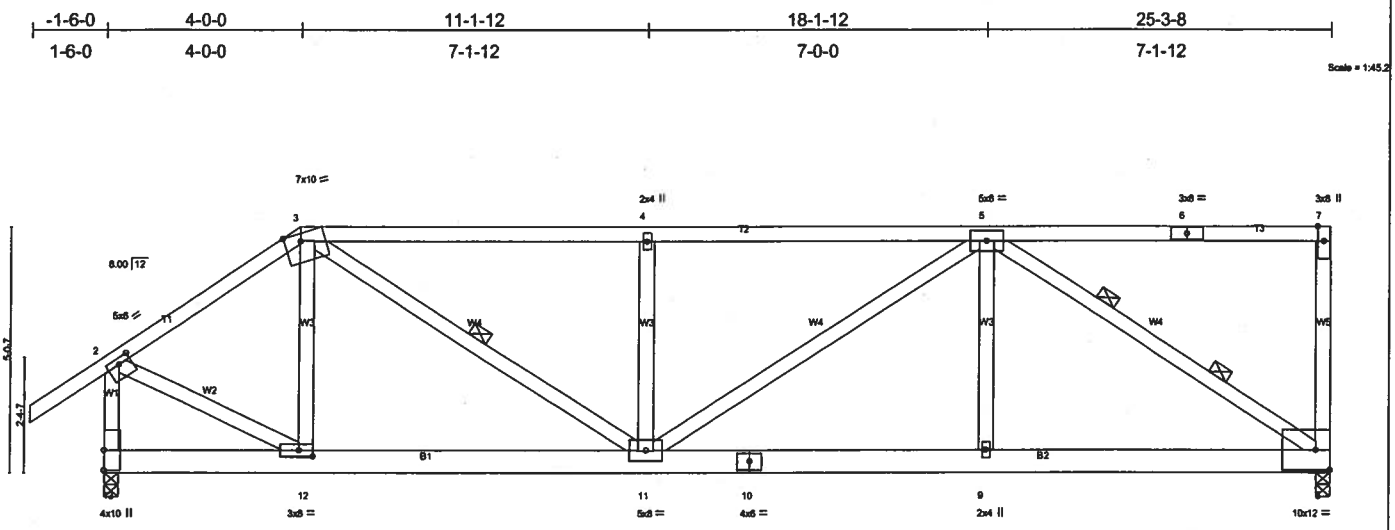
LOAD CASE(S) Standard

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1130051451

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T23	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:31 2005 Page 1



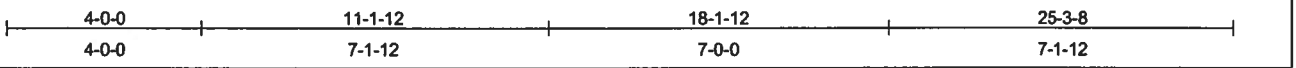


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [3:0-4-0,Edge], [12:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	0.16	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.24	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-11
2 Rows at 1/3 pts 5-8

REACTIONS (lb/size) 8=2284/0-3-8, 13=2386/0-3-8
Max Horz 13=240(load case 4)
Max Uplift8=-1413(load case 2), 13=-1191(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-2109/1138, 3-4=-3162/1916, 4-5=-3161/1917, 5-6=-122/102, 6-7=-122/102, 7-8=-390/419, 2-13=-2179/1110
BOT CHORD 12-13=-168/75, 11-12=-1007/1733, 10-11=-1607/2623, 9-10=-1607/2623, 8-9=-1607/2623
WEBS 3-12=-355/488, 3-11=-1139/1703, 4-11=-848/936, 5-11=-387/646, 5-9=0/464, 5-8=-3006/1808, 2-12=-1184/1972

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1413 lb uplift at joint 8 and 1191 lb uplift at joint 13.
4) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 4-0-0
5) Girder carries hip end with 0-0-0 right side setback, 4-0-0 left side setback, and 7-0-0 end setback.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-7=-118(F=-64), 12-13=-129(F=-99), 8-12=-65(F=-35)

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1130051452

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	T24	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:32 2005 Page 1

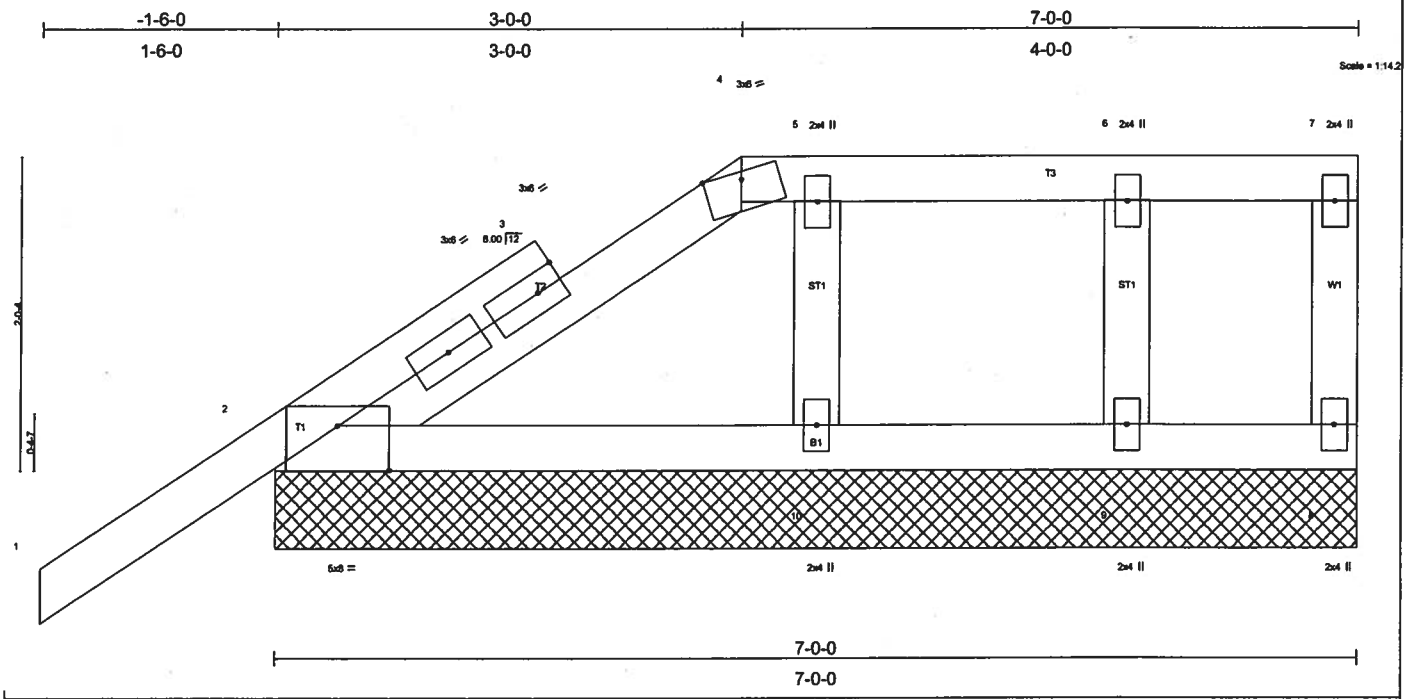


Plate Offsets (X,Y): [2:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.01	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=434/7-0-0, 8=102/7-0-0, 10=433/7-0-0, 9=189/7-0-0
Max Horz 2=141(load case 5)
Max Uplift2=-220(load case 5), 8=-37(load case 4), 10=-140(load case 4), 9=-136(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-24/91, 2-3=-110/27, 3-4=-37/42, 4-5=-14/9, 5-6=-14/9, 6-7=-14/9, 7-8=-75/59
BOT CHORD 2-10=-9/14, 9-10=-9/14, 8-9=-9/14
WEBS 5-10=-330/251, 6-9=-161/150

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2, 37 lb uplift at joint 8, 140 lb uplift at joint 10 and 136 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

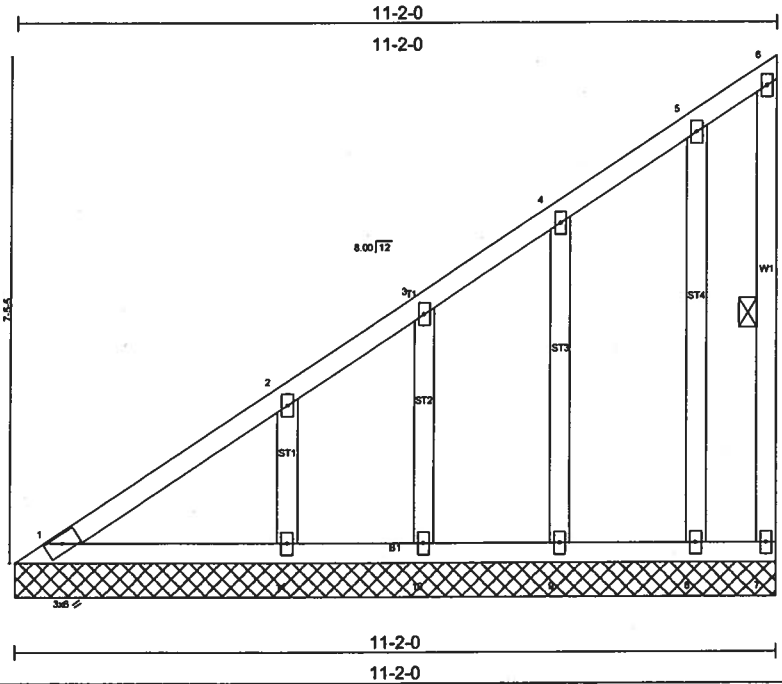
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-8=-30

NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1130051453

Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. LOT 2 ROLLING MEADOWS
L141740	V01	VALLEY	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Nov 30 08:48:33 2005 Page 1



11-2-0
11-2-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7

REACTIONS (lb/size) 1=207/11-2-0, 7=50/11-2-0, 11=516/11-2-0, 10=196/11-2-0, 9=322/11-2-0, 8=225/11-2-0
Max Horz 1=324(load case 5)
Max Uplift7=-31(load case 5), 11=-268(load case 5), 10=-103(load case 5), 9=-168(load case 5), 8=-110(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-341/118, 2-3=-215/38, 3-4=-160/69, 4-5=-77/56, 5-6=-43/21, 6-7=-39/29
BOT CHORD 1-11=-1/1, 10-11=-1/1, 9-10=-1/1, 8-9=-1/1, 7-8=-1/1
WEBS 2-11=-373/257, 3-10=-172/127, 4-9=-251/178, 5-8=-179/126

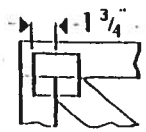
NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All plates are 2x4 MT20 unless otherwise indicated.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 7, 268 lb uplift at joint 11, 103 lb uplift at joint 10, 168 lb uplift at joint 9 and 110 lb uplift at joint 8.
4) Non Standard bearing condition. Review required.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 1-7=-30

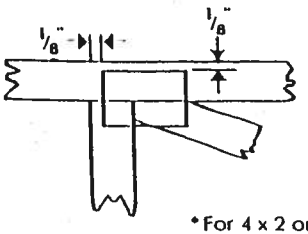
NOVEMBER 30, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

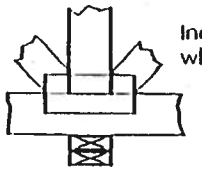
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

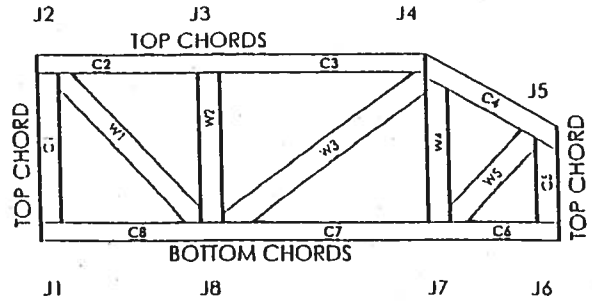
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (386) 752-1884
FAX (386) 758-7022
904 NW MAIN BLVD.
LAKE CITY, FLORIDA 32055

July 12, 2005

NOTICE TO BUILDING DEPT

For Lot 2 Rolling Meadows

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results. All wells will have a submersible pump and diaphragm tank combination that will be sufficient for each situation.

If you have any questions please feel free to call our office anytime.

Thank you,

Don Hall

Donald D. Hall
DDH/rb

Lot 25 Rolling Meadows

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☑ wood burning ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessable bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- ☑ ☐ c) Any special support required by soil analysis such as piling
- ☐ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☐ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termiticide or alternative method)
  10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☑ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☑ ☐ b) Floor joist size and spacing

☑ ☐ c) Girder size and spacing

☑ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. # 24113

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB108478 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Sparks construction Company Phone No. 623-0575

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 145 Sw Morning glory dr lake city fl 32024

Type of Construction (More than one box may be checked) ☑ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside Inside Type of Fill Sand

Section 4: Treatment Information

Date(s) of Treatment(s) 4-11-06
Brand Name of Product(s) Used Bora care
EPA Registration No. 64405-1
Approximate Final Mix Solution % .25%
Approximate Size of Treatment Area: Sq. ft. 2948 Linear ft. 0 Linear ft. of Masonry Voids 0
Approximate Total Gallons of Solution Applied 6 gals
Was treatment completed on exterior? ☑ Yes ☐ No
Service Agreement Available? ☑ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Josh Wood Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Josh Wood Date

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

4113

BAILEY BISHOP & LANE, INC.

Engineers *Surveyors* *Planners*

July 21, 2006

Mr. Anthony & Jessica Smith
145 SW Morning Glory Drive
Lake City, FL 32024

RE: Finish Flood Elevation Letter

Dear Mr. Smith:

We have performed a vertical survey on the structure located on Lot 2, Rolling Meadows, Columbia County, Florida and have determined the following:

- That subdivision plat requires that the minimum finish floor elevation be 101.00'.
- That the field located finish floor elevation is 106.3', being 5.3' above the required elevation.

Should you have any questions, please do not hesitate to give me a call.

Sincerely,

Brian Scott Daniel, PSM
Director of Surveying
BAILEY, BISHOP & LANE, INC.

P. O. Box 3717	Lake City, FL 32056-3717	Ph. (386) 752-5640	FAX (386) 755-7771
P. O. Box 814	Port St. Joe, FL 32457	Ph. (850) 227-9449	FAX (850) 227-9650

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 15-4S-16-03023-502 **Building permit No.** 000024113

Use Classification SF/UTILITY **Fire:** 17.76

Permit Holder JOSH SPARKS **Waste:** 36.75

Owner of Building SPARKS CONTRACTORS,INC. **Total:** 54.51

Location: 145 SW MORNING GLORY DR(ROLLING MEADOWS,LOT 2)

Date: 07/21/2006

Randy Jones

Building Inspector

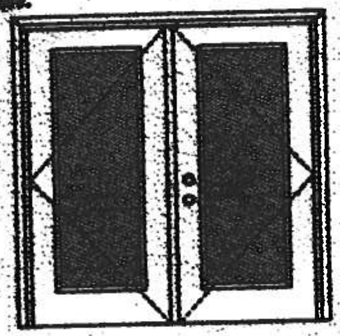
POST IN A CONSPICUOUS PLACE
(Business Places Only)

[illegible]

XX
Glazed Outswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.*

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

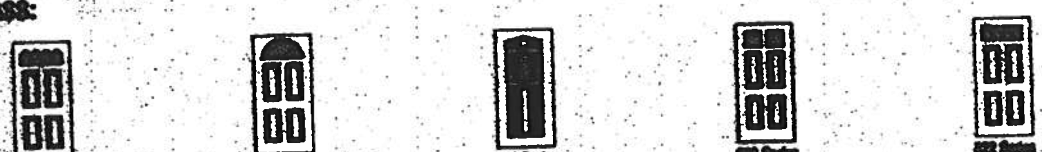
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

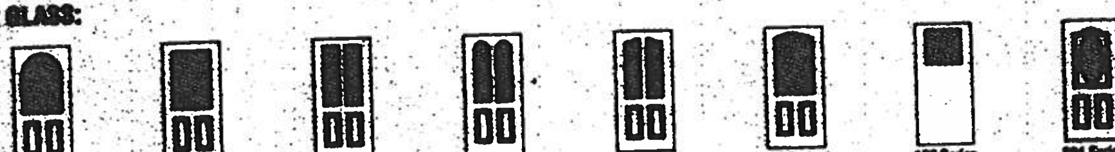
Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:



100 Series | 122, 125 Series | 135 Series | 600 Series | 522 Series

1/2 GLASS:



105 Series* | 106, 150 Series* | 120 Series* | 200 Series* | 12 Ri., 22 Ri., 24 Ri. Series* | 107 Series* | 103 Series | 304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

XX
Glazed Outswing Unit

[redacted]

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:

FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56633

Johnson

March 28, 2002

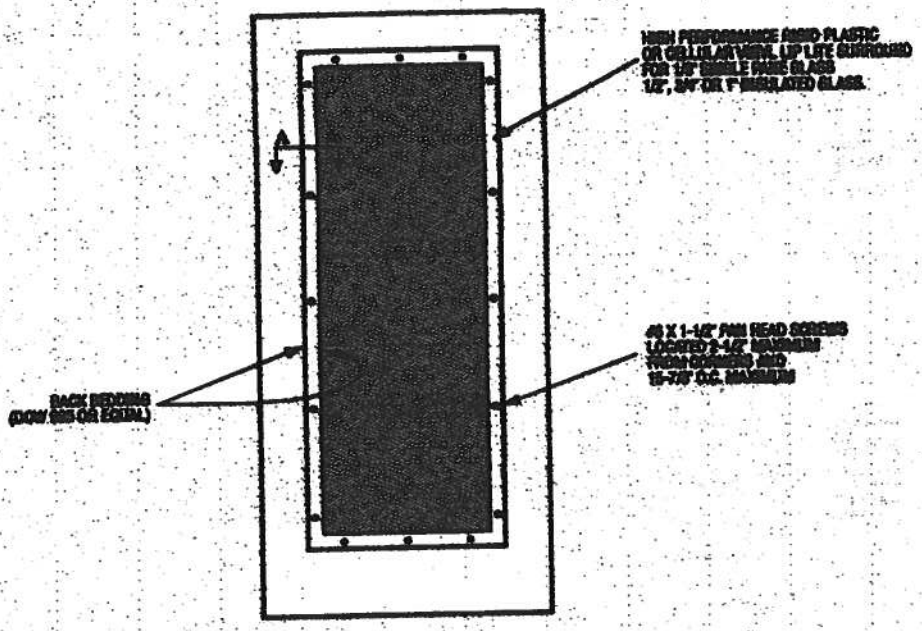
FRENDORF

Exclusively from Masonite

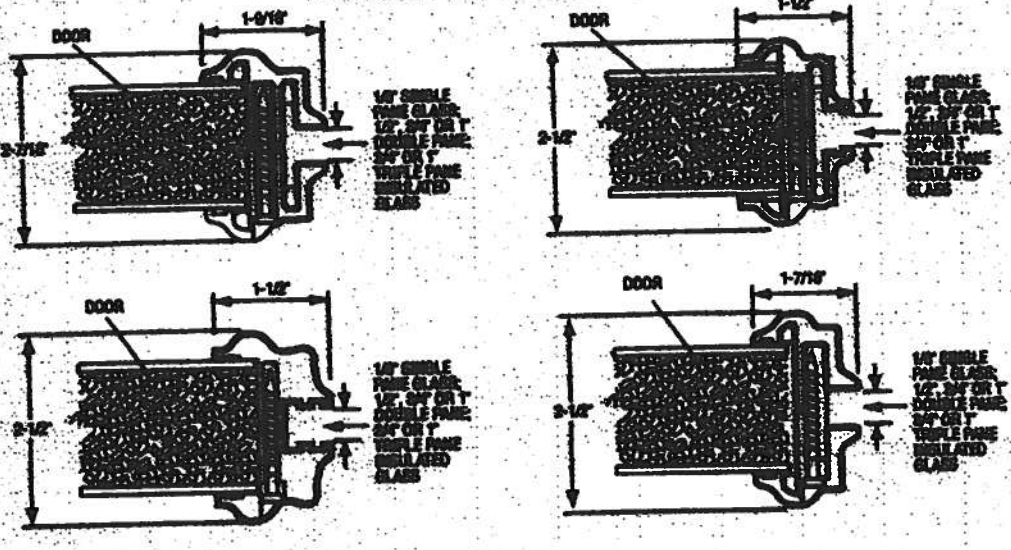
Masonite International Corporation

[redacted]

GLASS INSERT IN DOOR OR SIDELITE PANEL



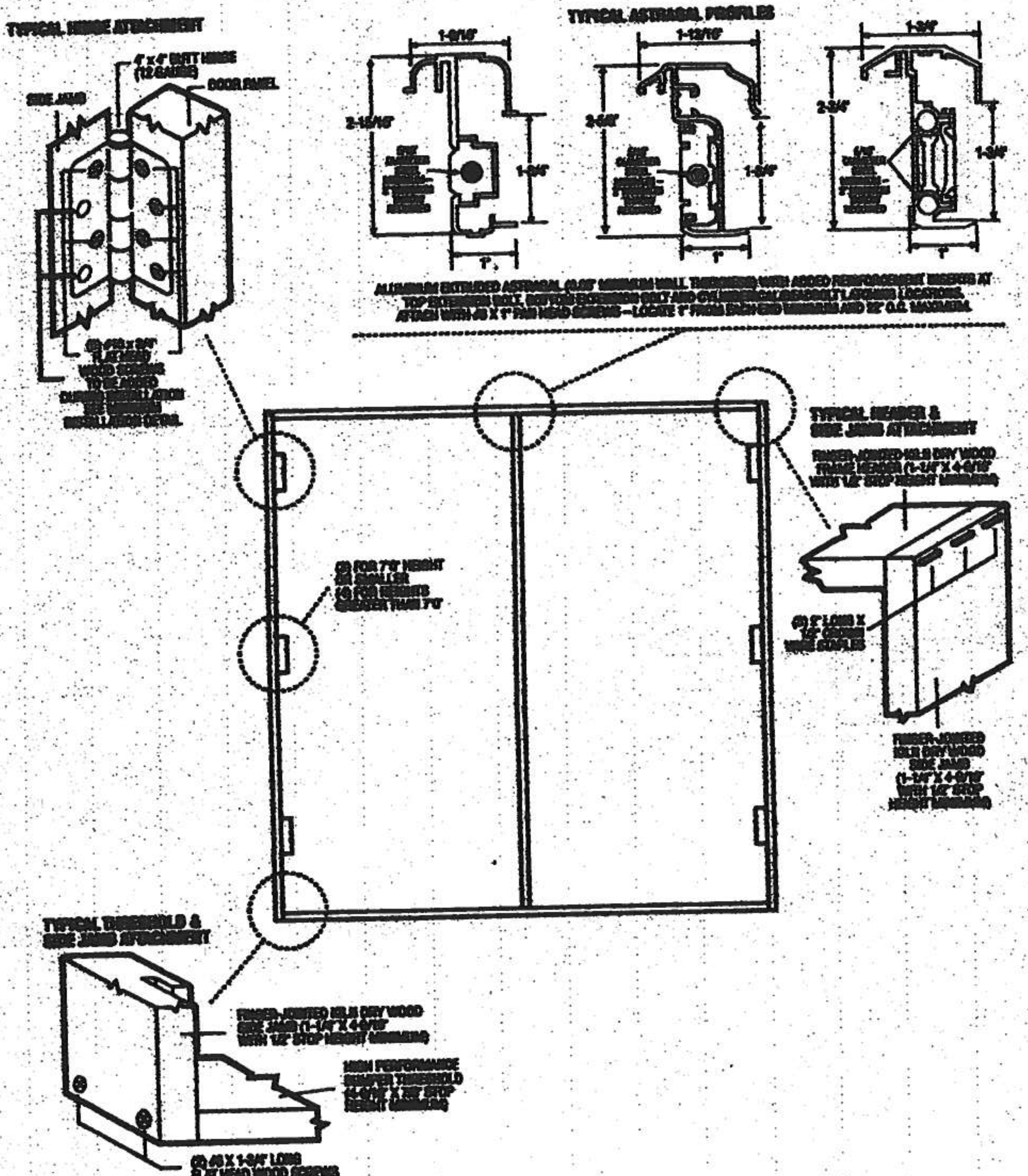
SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



XX
Unit

[redacted]

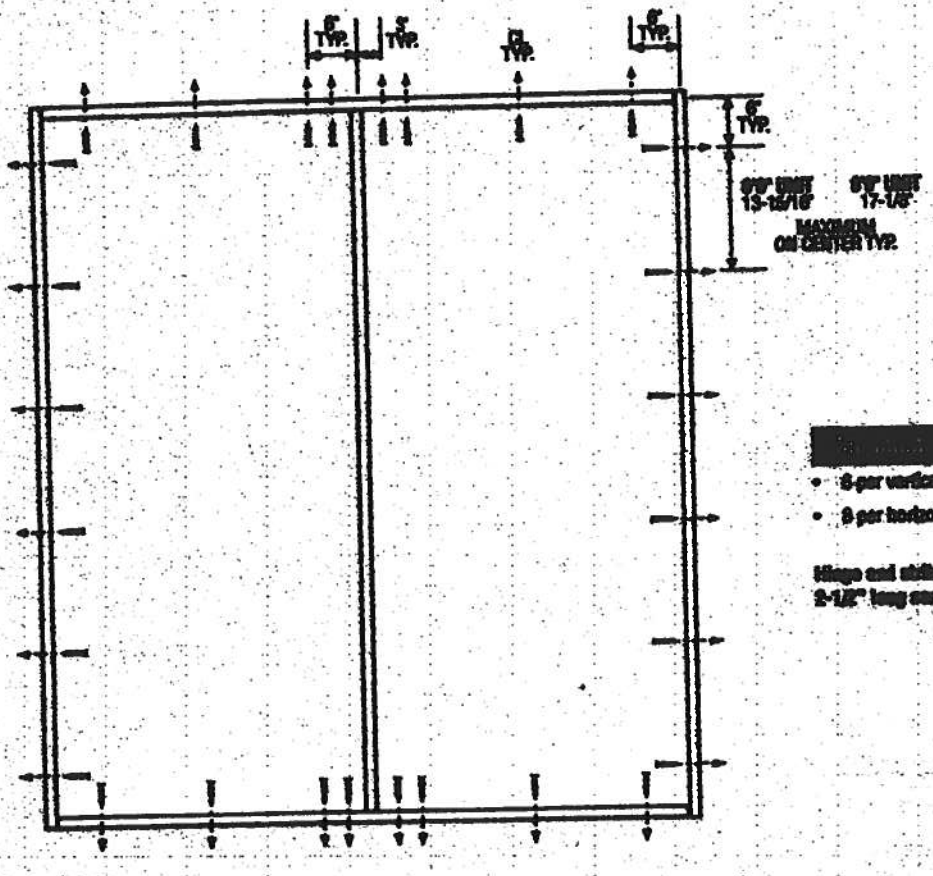
OUTSWING UNITS WITH DOUBLE DOOR



XX
Unit

[redacted]

DOUBLE DOOR



[redacted]

- 8 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

[redacted]

Exclusively from Masonite®
Masonite International Corporation

March 29, 2022
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Overview | User Registration | Organization Registration | User Authentication | Organization Search | Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Cancel Search

Result List for Organizations

Displaying 1-1 of 1

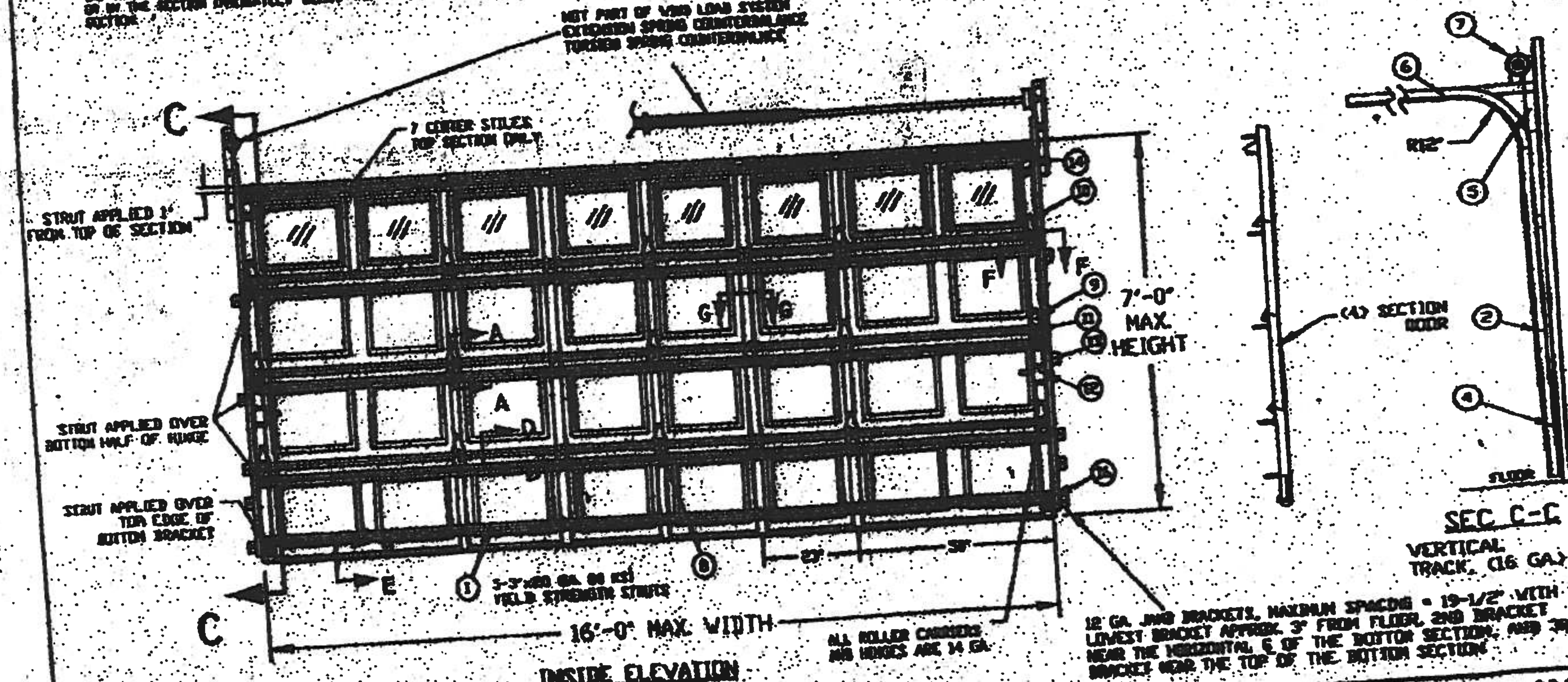
Name	City	Contact	Phone	Type	Expire	Status
General American Door	Montgomery	James Campbell	[illegible]	Product Manufacturer	01/01/2099	Approved
Org Code: PDM System ID: 3585			Site Links: [illegible]			

Displaying 1-1 of 1

REV.	DATE	BY	DESCRIPTION
A-A	11-10-00		SEE ECR 031

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURE PER ASTM E-330.
2. MAXIMUM SECTION HEIGHT= 21".
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION AS TESTED WITH 1/8" DSB GLASS OR CONVALUED OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MAXIMUM LENGTH OF ROLLER STEM IS 5" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE ALL OR [illegible] AS TESTED.
9. SHOP OR TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL SEAL PE No. 026290

LISTED
[redacted]
REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/00 00293

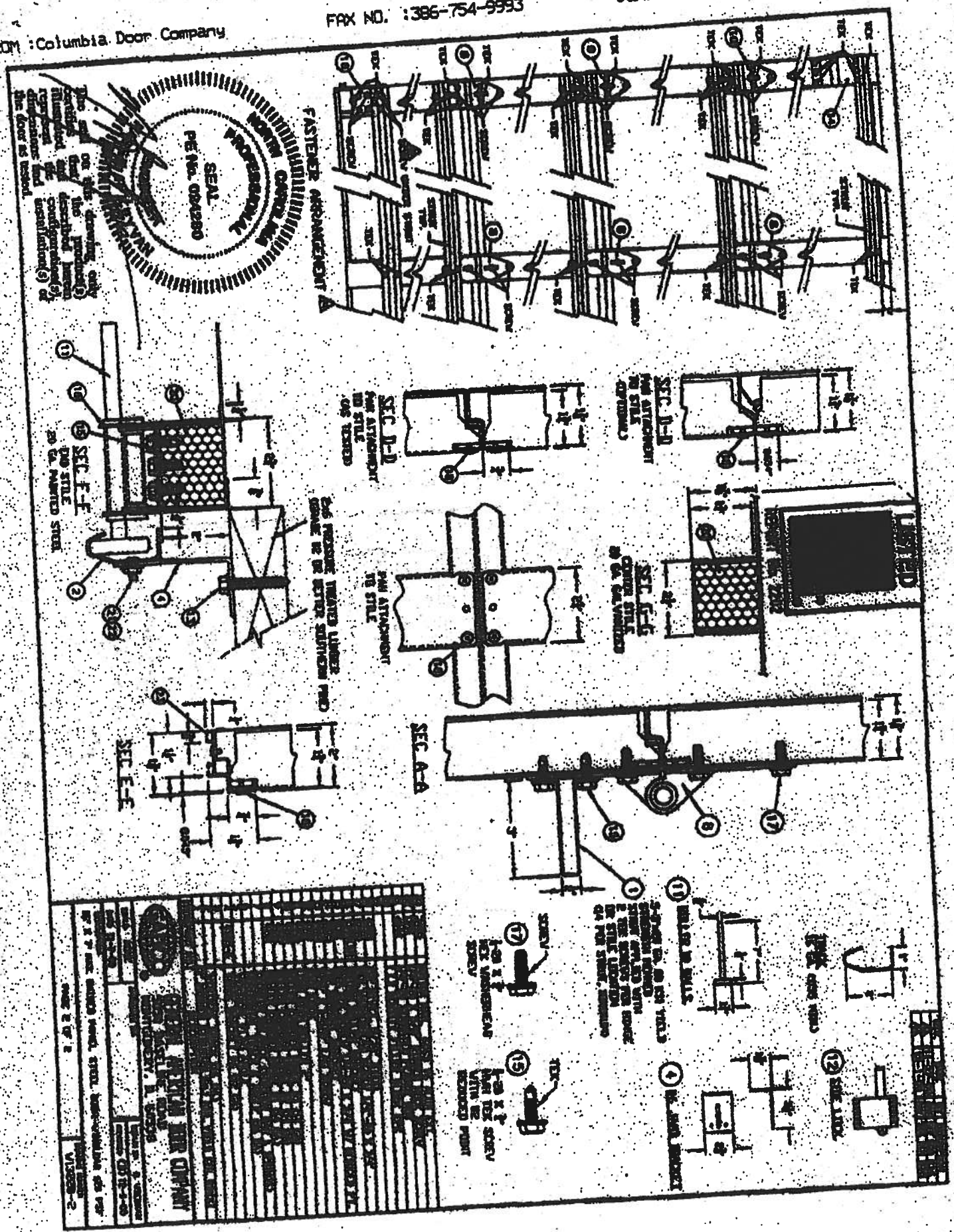
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

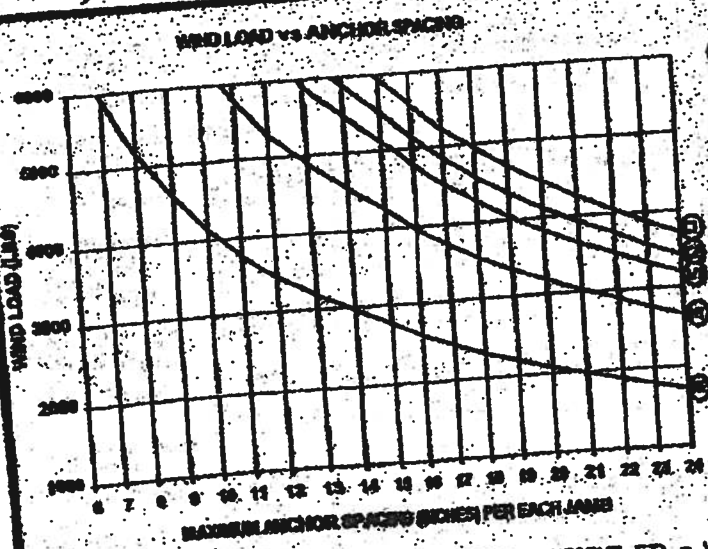
GENERAL AMERICAN DOOR COMPANY
3050 BASELINE ROAD
MONTGOMERY, IL 60538

GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.017" MIN
SERIES 7524, EXTERIOR STEEL =.024" MIN
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL STR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

DRAWN BY: B. WEIGAND
REVISION (A) 11-10-00
DATE 10-20-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR - WINDLOAD ±20 PSF
PAGE 1 OF 2
DRAWING NUMBER V13220-1


SEAL
PE No. 0042280
FASTENER ARRANGEMENT A
SEC F-F
SEC D-D
SEC H-H
SEC G-G
SEC A-A
SEC E-E



(1) CONCRETE BACKUP MULTI SCREW BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(2) CONCRETE BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(3) [illegible] BACKUP [illegible] 3/8" DIA. 1-5/8" EMBEDMENT

(4) [illegible] BACKUP [illegible] 1/4" DIA. 1-3/4" EMBEDMENT

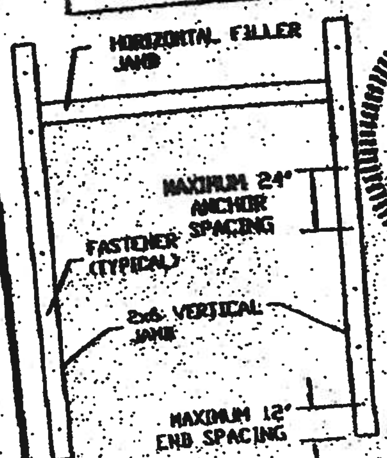
(5) WOOD STUD BACKUP LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

DESIGN (LBS) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)
LOAD $\overline{FT^2}$

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(1) USE 22" SPACING (4) USE 16" SPACING
(2) USE 21" SPACING (3) USE 10" SPACING
(5) USE 19" SPACING SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATION USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2000 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024800 ENGINEER NASER R KEYVAN

NASER R. KEYVAN P.E. No. 53777 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/8/2002

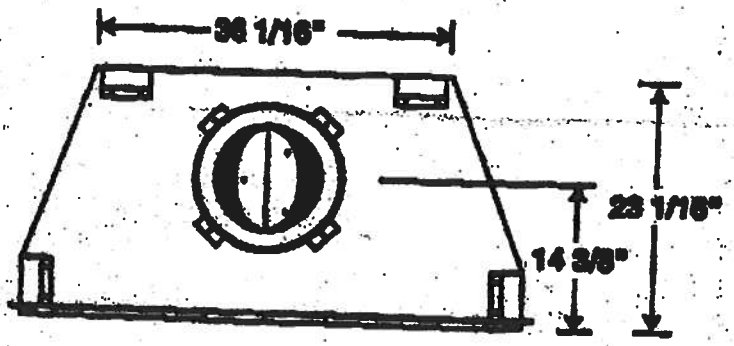
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

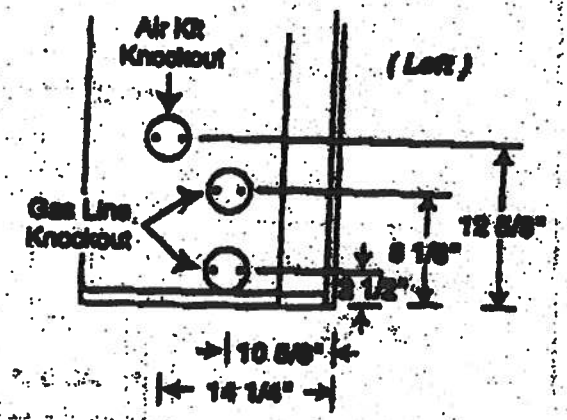
DRAWN: NBNK	SCALE: NONE
DATE: 8-30-99	REV:
TITLE: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	
DWG NO.: A10560	

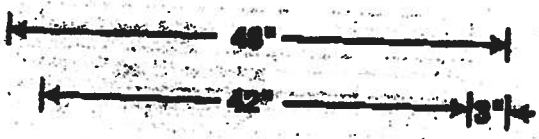
Craftsman

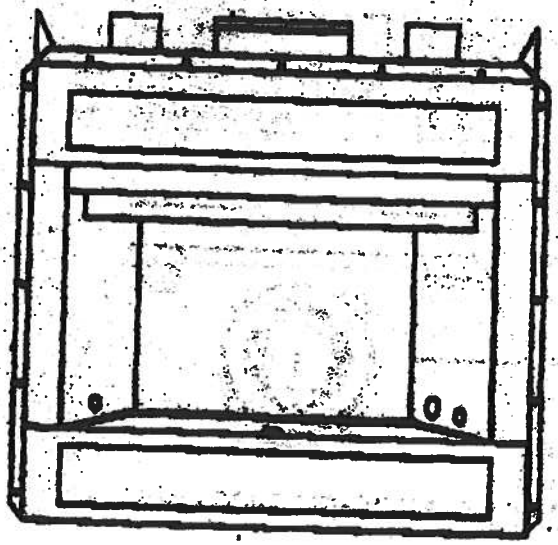
42" Woodburning Fireplace

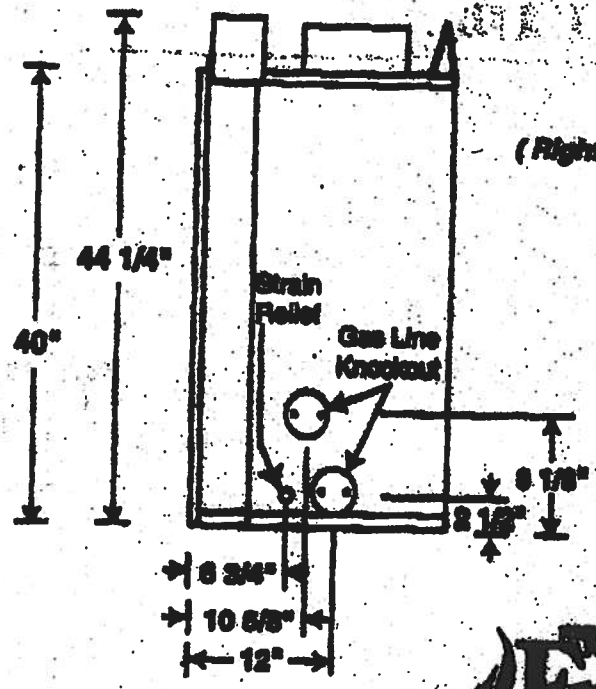
Vent Pipe Size	10"
Min. Pipe Clearance	1"
Min. System Height	14' 6"
- w/ Single Offset	14' 6"
- w/ Two Offsets	22' 0"
Max. Dist. Between Elbows	6' 0"
Max. System Height	60' 0"











TAMKO®
ROOFING PRODUCTS

FEB - 4 RECD

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

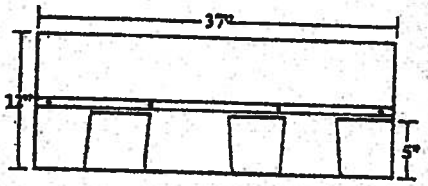
TAMKO Roofing Products, Inc.

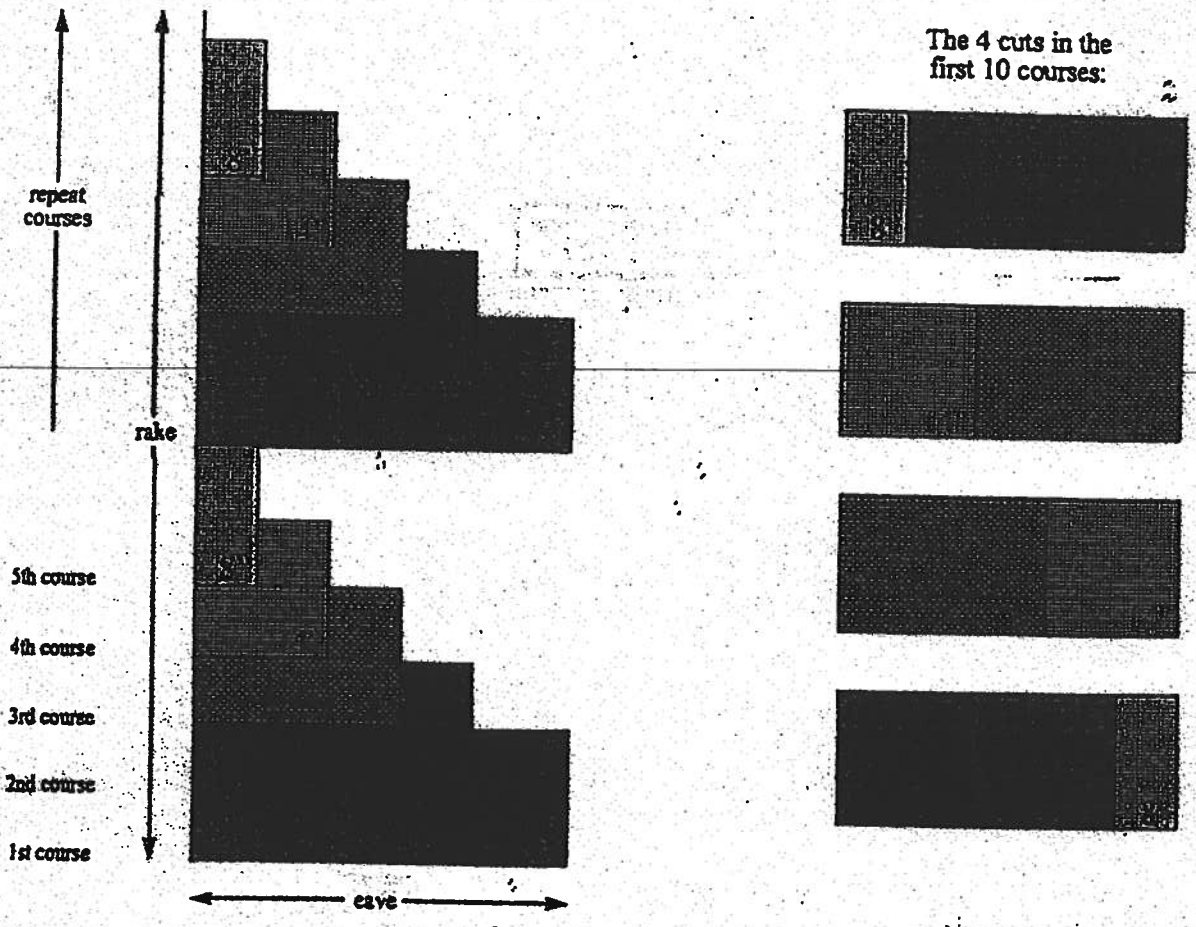
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1925

TAMKO
ROOFING PRODUCTS

Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"





The 4 cuts in the first 10 courses:

In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO ROOFING PRODUCTS

Application Instructions for

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.
THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.
IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

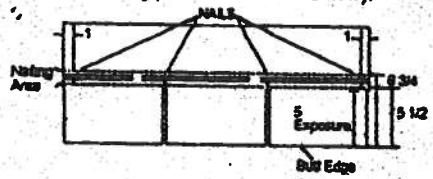
3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

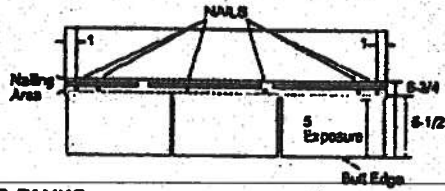
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

(CONTINUED from Pg. 2)

TAMKO ROOFING PRODUCTS

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Nansard Fastening Pattern.

5. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

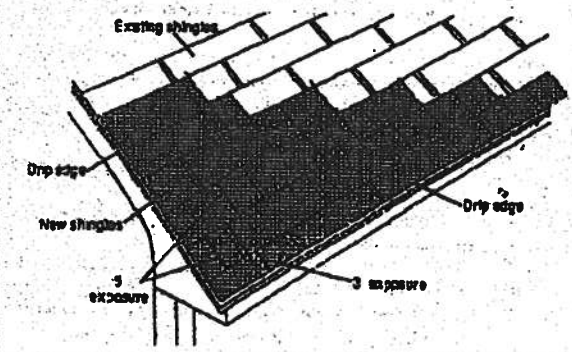
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.



6. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50 lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

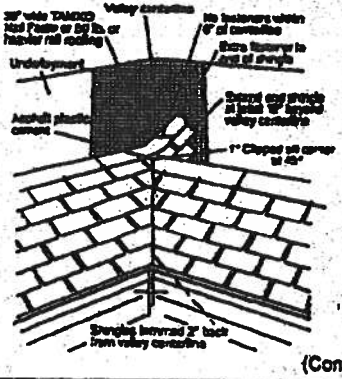
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- *CAUTION: Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

(CONTINUED from Pg. 3)

TAMKO ROOFING PRODUCTS

• Glass-Seal • Glass-Seal AR • Elite Glass-Seal® • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

10. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

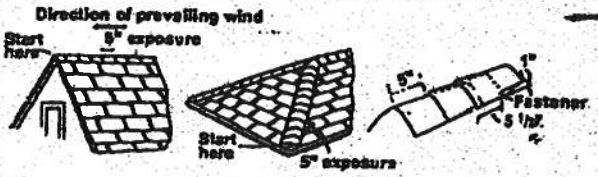
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.

Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES IN COOL WEATHER.

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.



THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE

In this paragraph "You" and "Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

I

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

ALLEN R. REEVES
SERIAL DAY
NO. 11254
STATE OF FLORIDA
PROFESSIONAL ENGINEER

1 APRIL 2002

II

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.architest.com

Allan N. Reeves
1 APRIL 2002

ALLAN N. REEVES
No. 12384
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and slit into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

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IV

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

*Exceeds L/175 for deflection, but passes all other test requirements.

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V

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

Uniform Load Structural (ASTM E 330-97)
(Measurements reported were taken on the meeting rail)
(Loads were held for 10 seconds)
@ 67.5 psf (positive) 0.05" 0.18" max.
@ 70.8 psf (negative) 0.05" 0.18" max.

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VI

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 April 2002

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