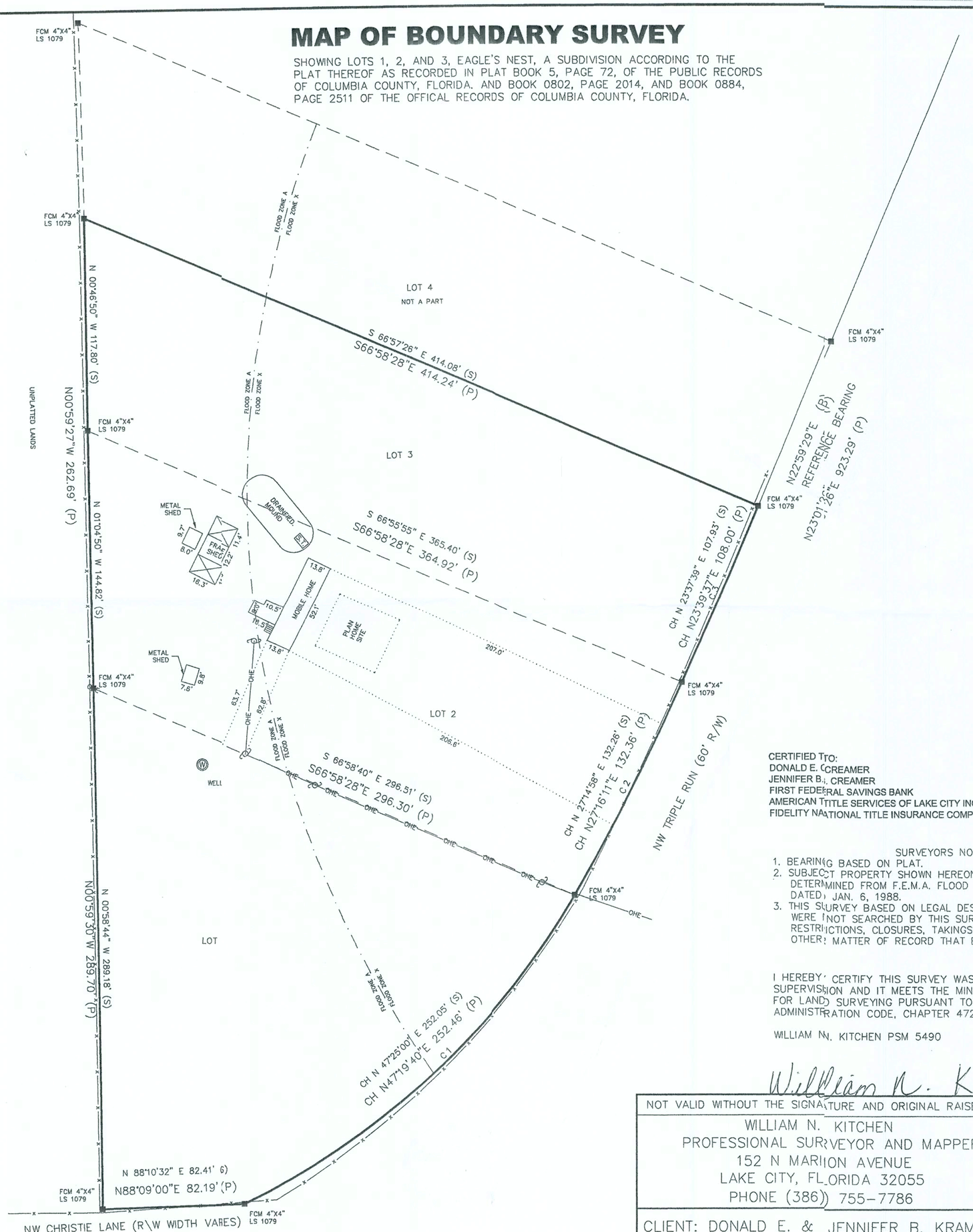
# MAP OF BOUNDARY SURVEY

SHOWING LOTS 1, 2, AND 3, EAGLE'S NEST, A SUBDIVISION ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 5, PAGE 72, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. AND BOOK 0802, PAGE 2014, AND BOOK 0884, PAGE 2511 OF THE OFFICAL RECORDS OF COLUMBIA COUNTY, FLORIDA.




| CURVE TABLE PLAT | | | | | |
|---|---|---|---|---|---|
| CURVE | DELTA | LENGTH | RADIUS | CHORD BEARING | CHORD |
| C 2 | 05°56'30" | 132.32 | 1275.97 | N27°14'58"E | 132.26 |
| C 1 | 34°06'47" | 255.81 | 429.66 | N47°25'00"E | 252.05 |
| C 3 | 01°16'19" | 107.94 | 4861.85 | N23°37'39"E | 107.93 |
| CURVE TABLE SURVEY | | | | | |
| C 1 | 34°10'11" | 256.24 | 429.66 | N47°19'40"E | 252.46 |
| C 2 | 05°56'46" | 132.42 | 1275.97 | N27°16'11"E | 132.36 |
| C 3 | 01°16'22" | 108.00 | 4861.85 | N23°39'37"E | 108.00 |

CERTIFIED TO:
DONALD E. CREAMER
JENNIFER B. CREAMER
FIRST FEDERAL SAVINGS BANK
AMERICAN TITLE SERVICES OF LAKE CITY INC.
FIDELITY NATIONAL TITLE INSURANCE COMPANY

SURVEYORS NOTES

1. BEARING BASED ON PLAT.
2. SUBJECT PROPERTY SHOWN HEREON LIES IN FLOOD ZONE X & A AS BEST DETERMINED FROM F.E.M.A. FLOOD MAPS PANEL NO. 120070 0150 B DATED JAN. 6, 1988.
3. THIS SURVEY BASED ON LEGAL DESCRIPTION FURNISHED. THE PUBLIC RECORDS, WERE NOT SEARCHED BY THIS SURVEYOR FOR EASEMENTS, TITLE, COVENANTS, RESTRICTIONS, CLOSURES, TAKINGS OR ORDINANCES, ETC., THERE COULD BE OTHER MATTER OF RECORD THAT EFFECT THIS PARCEL.

I HEREBY CERTIFY THIS SURVEY WAS DONE UNDER MY DIRECT SUPERVISION AND IT MEETS THE MINIMUM TECHNICAL STANDARDS FOR LAND SURVEYING PURSUANT TO CHAPTER 61G17-6, FLORIDA ADMINISTRATION CODE, CHAPTER 472, FLORIDA STATUES.

WILLIAM N. KITCHEN PSM 5490

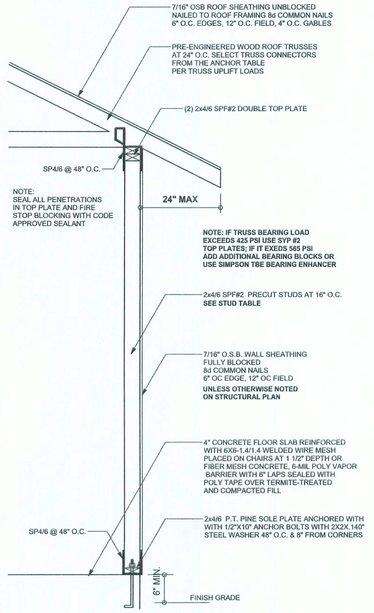
William N. Kitchen 4-20-2006

LEGEND

(D) = DEED
(P) = PLAT
(S) = SURVEY MEASUREMENT
(C) = CALCULATED MEASUREMENT
NOID = NO SURVEYORS IDENTIFICATION
LS = LAND SURVEYOR
LB = LICENSE BUSINESS
FIR = FOUND IRON ROD
FIP = FOUND IRON PIPE
FCM = FOUND CONCRETE MONUMENT
SIR = SET IRON ROD
SCM = SET CONCRETE MONUMENT
PRM = PERMANENT REFERENCE MONUMENT
C/L = CENTER LINE
R/W = RIGHT OF WAY
EOP = EDGE OF PAVEMENT
ASP = ASPHALT PAVING
CONC = CONCRETE
OHE = OVER HEAD ELECTRIC
UGE = UNDER GROUND ELECTRIC
= WOOD POWER POLE
S.T. = SEPTIC TANK
X—X = WIRE FENCE
□—□ = WOOD FENCE

NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

| | | |
|---|---|---|
| WILLIAM N. KITCHEN<br>PROFESSIONAL SURVEYOR AND MAPPER<br>152 N MARION AVENUE<br>LAKE CITY, FLORIDA 32055<br>PHONE (386) 755-7786 | DRAWN BY: WNK | FIELD BOOK : 06139 |
| | SCALE: 1" = 40' | |
| | SURVEY DATE: MARCH, 27 2006 | |
| | JOB NUMBER | SHEET |
| CLIENT: DONALD E. & JENNIFER B. KRAMER | 06139 | 1 OF 1 |



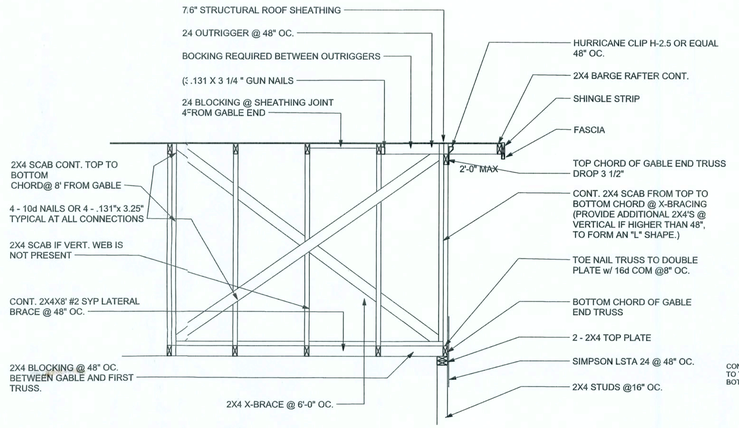

## ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|---|---|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 18'-10' STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20.0' STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

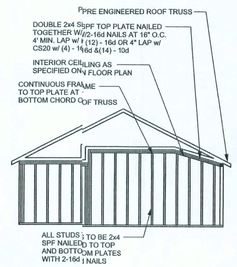



## TYFCAL GABLE END ( X-BRACING )

**ALL MEMBERS SHALL BE SYP**

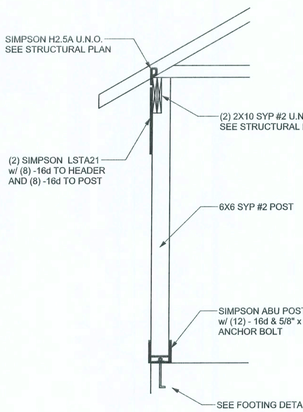
## GRADE & SPECIES TABLE

| | | Fb (psi) | E ($10^6$ psi) |
|---|---|---|---|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 1600 | 1.9 |
| PSL | PARALAM | 2900 | 2.0 |



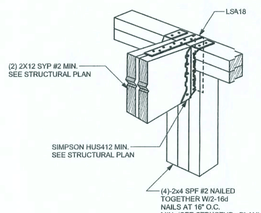

## CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



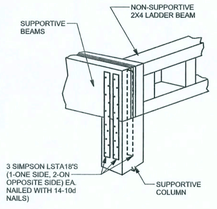

## TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



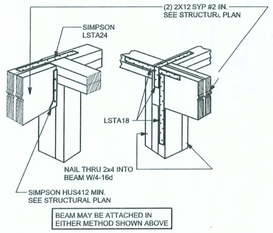

## BEAM MID-WALL CONNECTDN DETAIL

SCALE: N.T.S.



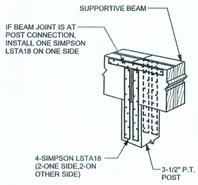

## SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



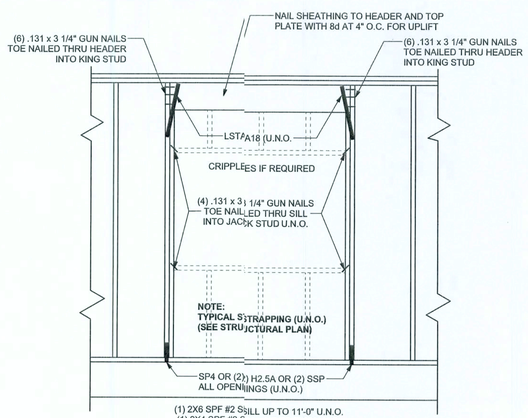

## BEAM CORNER CONNECTION. DETAIL

SCALE: N.T.S.




## SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.




## TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

## GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004, TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F'c$ = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1.4 x W1.4, FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

**ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

| THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. |
|---|
| CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. |
| VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. |

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| | ACI530.1-02 Section | Specific Requirements |
|---|---|---|
| 1.4A | Compressive strength | 8" block bearing walls F'm = 1500 psi |
| 2.1 | Mortar | ASTM C 270, Type N, UNO |
| 2.2 | Grout | ASTM C 476, admixtures require approval |
| 2.3 | CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5) |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS |
| 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |

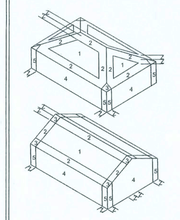
## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|---|---|---|---|---|---|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H6 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 850 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2490 | 2 - HTS24 | | | |
| < 2050 | < 1785 | LGT2 | 14 -16d | 14 -16d | |
| | | **HEAVY GIRDER TIEDOWNS*** | | | **TO FOUNDATION** |
| < 3965 | < 3330 | MGT | | 22 -10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16 -10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| | | **STUD STRAP CONNECTOR*** | | | **TO STUDS** |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3 -10d | | 4 -10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1 -10d | | 4 -10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6 -10d | | 8 -10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2 -10d | | 8 -10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| | | **STUD ANCHORS*** | **TO STUDS** | | **TO FOUNDATION** |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTTI31 | 18-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | H2A | 2-5/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18 - 16d | | 5/8" AB |
| < 1400 | < 1400 | PAHD42 | 16-16d | | |
| < 3335 | < 3335 | HPAHD22 | 16-16d | | |
| < 2200 | < 2200 | ABU44 | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABU66 | 12-16d | | 1/2" AB |
| < 2320 | < 2320 | ABU88 | 18 - 16d | | 2-5/8" AB |

## DESIGN DATA

| WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 | |
|---|---|
| (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) | |
| BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE | |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION | |
| 1.) BASIC WIND SPEED = 110 MPH | |
| 2.) WIND EXPOSURE = B | |
| 3.) WIND IMPORTANCE FACTOR = 1.0 | |
| 4.) BUILDING CATEGORY = II | |
| 5.) ROOF ANGLE = 10-45 DEGREES | |
| 6.) MEAN ROOF HEIGHT = <30 FT | |
| 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) | |
| 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) | |



| Zone | Effective Wind Area (ft2) | | | |
|---|---|---|---|---|
| | 10 | | 100 | |
| 1 | 19.9 | -21.8 | 18.1 | -18.1 |
| 2 | 19.9 | -25.5 | 18.1 | -21.8 |
| 2 Orhg | | -40.6 | | -40.6 |
| 3 | 19.9 | -25.5 | 18.1 | -21.8 |
| 3 Orhg | | -68.3 | | -42.4 |
| 4 | 21.8 | -23.6 | 18.5 | -20.4 |
| 5 | 21.8 | -29.1 | 18.5 | -22.6 |
| Doors & Windows Worst Case (Zone 5, 10 ft2) | | 21.8 | | -29.1 |
| 8x7 Garage Door | | 19.5 | | -22.9 |
| 16x7 Garage Door | | 18.5 | | -21.0 |

| DESIGN LOADS | |
|---|---|
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR =<4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) | |
| SOIL BEARING CAPACITY 1000PSF | |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |

| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Bark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.



CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to tb best of my knowledge.

LIMITATION: This design i valid for one building, at specified locatec.

MARK DISOSWAY
P.E. 53915

26 APR 06

SEAL

Danald & Jennifer
Creamer Residence

ADDRESS:
2954 NE Triph Run Rd.
Lake City, Floida 32055

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Lake City, Floida 32056
Phone: (386) ‘54 - 5419
Fax: (386) 2‹9 - 4871

PRINTED DATE:
April 26, 2006

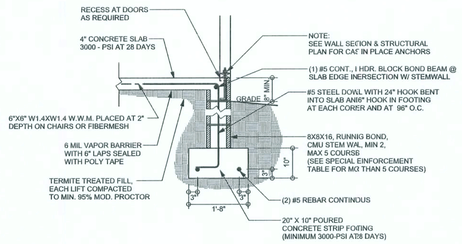
DRAWN BY: David Disosway

CHECKED BY:

FINALS DATE:
26 / Apr / 06

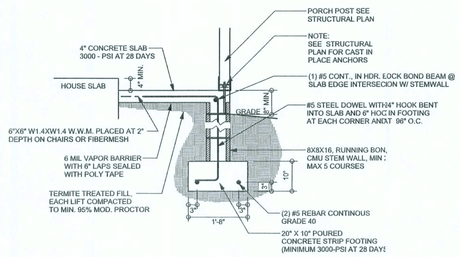
JOB NUMBER:
603304

DRAWING NIMBER
**S-1**
OF 3 SHEETS



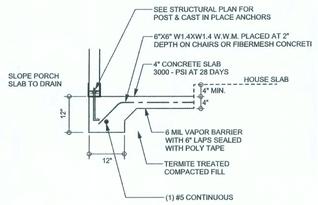

F9 S-2 STEM WALL FOOTING

SCALE: 1/2" = 1'-0"




F12 S-2 ALT. STEM WALL PORCH FOOTING

SCALE: 1/2" = 1'-0"



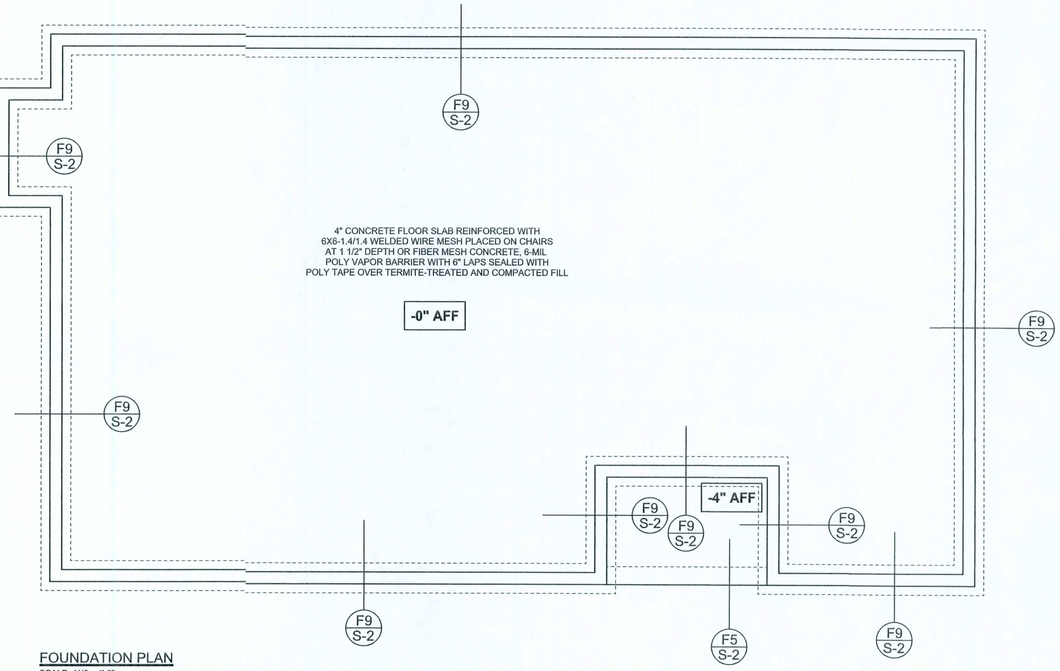

F5 S-2 PORCH FOOTING

SCALE: 1/2" = 1'-0"

## TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

| STEMWALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.) | | | VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.) | | |
|---|---|---|---|---|---|---|---|
| | | #5 | #7 | #8 | #5 | #7 | #8 |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 |
| 9.3 | 9.0 | 8 | 16 | 24 | 16 | 40 | 48 |




## FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS**

| REVISIONS | |
|---|---|
| | |
| | |
| | |

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

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CERTIFICATION: I hanby certify that I have examined this plan, andthat the applicable portions of the plan, rotting to wind engineering comply with section R31.2.1, florida building code residential 2004, t the best of my knowledge.

LIMITATION: This desin is valid for one building, at specified loation.

MARK DISOSWAY P.E. 5915

SEAL

Danald & Jennifer Creamer Residence

ADDRESS: 2954 NE Trple Run Rd. Lake City, Forida 32055

Mark Discsway P.E. P.O. Box 868 Lake City, Forida 32056 Phone: (386) 754 - 5419 Fax: (386)269 - 4871

PRINTED DATE: April 26, 2006

DRAWN BY: David Discosway

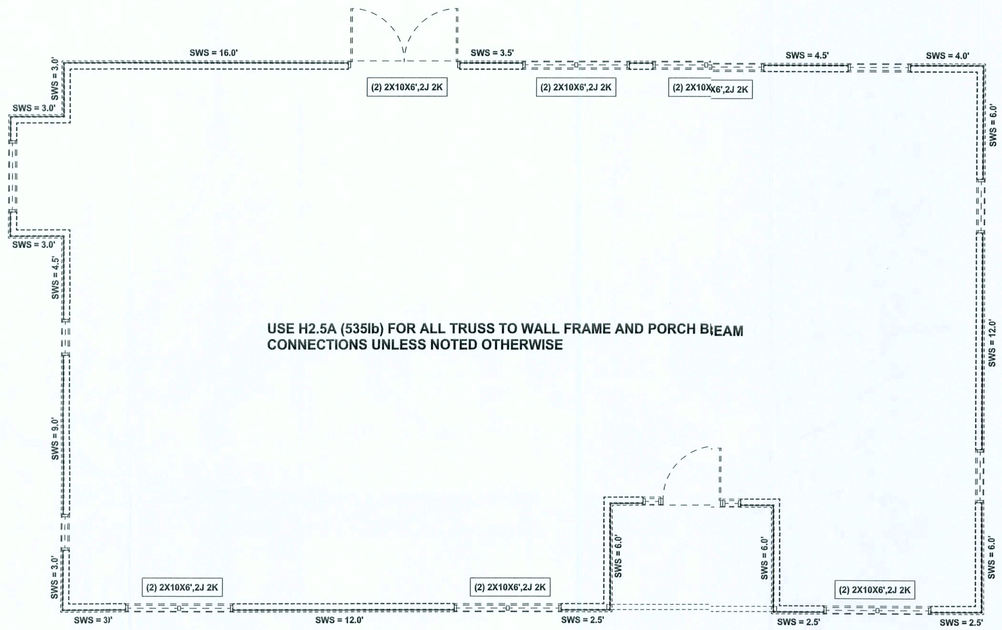
CHECKED BY:

FINALS DATE: 26 / Apr / 06

JOB NLMBER: 603304

DRAWINGNUMBER **S-2**

OF 3 SHEETS




USE H2.5A (535lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

## STRUCTURAL PLAN
SCALE: 1/4 = 1'-0"

## STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

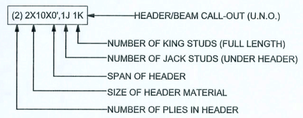
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

| | |
|---|---|
| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" o.c. EDGE, 12" O.C. FIELD (U.N.O.) |
| SWS = 0.0' | 2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" o.c. EDGE, 12" O.C. FIELD (U.N.O.) |
| IBW | 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |
| IBW | 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 |

## HEADER LEGEND




## TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|---|---|---|
| TRANSVERSE | 29.5' | 55.5' |
| LONGITUDINAL | 28.5' | 56.5' |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. A & R JOB #02031

REVISONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, PO\ 868, Lake City, FL 32056, 386-754-5\9

DIMENSIONS: Stated dimensionsupercede scaled dimensions. Referal questions to Mark Disosway, PE, for resolution. Do not proceed without clarification.

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CERTIFICATION: hereby certify that I have examined this plar, and that the applicable portions of the pla, relating to wind engineering comply with sectio R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: Thisdesign is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

26 Apr 06

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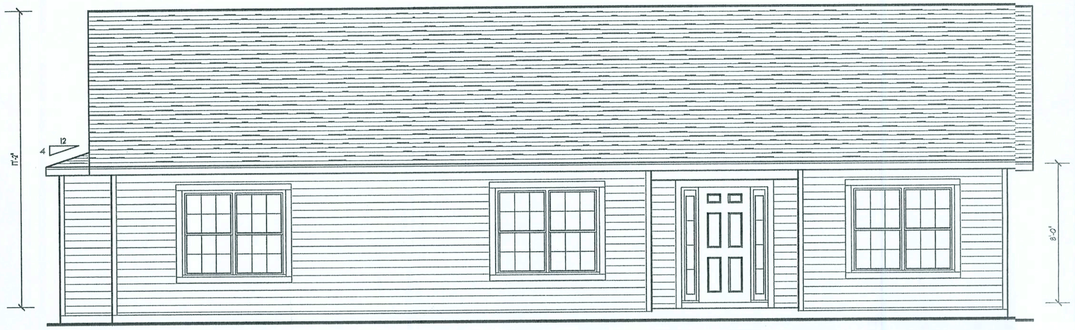
DRAWING NUMBER
**S-3**
OF 3 SHEETS




FRONT ELEVATION

SCALE: 1/4" = 1'


LEFT ELEVATION

SCALE: 1/4" = 1'


REAR ELEVATION

SCALE: 1/4" = 1'


RIGHT ELEVATION

SCALE: 1/4" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

Daniel Shaheen

March 29, 2006

D.D.S. STUDIOS
P.O. Box 213
Lake City FL. 32056
(386) 754-0181

A CUSTOM HOME DESIGNED FOR:

DONALD & JENNIFER CREAMER



OUR PLANS ARE DRAFTED FOR AVERAGE SITE CONDITIONS AND COMPLIANCE WITH APPLICABLE CODES IN LAKE CITY, FL. AT THE TIME THEY ARE DRAFTED. DUE TO VARYING STATE, LOCAL AND NATIONAL CODES RULES AND REGULATIONS, DDS STUDIOS CANNOT WARRANT COMPLIANCE WITH ALL APPLICABLE STATE LOCAL, AND NATIONAL CODES IN YOUR AREA OR WITH YOUR PARTICULAR SITE CONDITIONS. IT IS THE RESPONSIBILITY OF THE PURCHASER AND/OR BUILDER TO SEE THAT THE STRUCTURE IS BUILT IN STRICT COMPLIANCE WITH ALL GOVERNING MUNICIPAL, CODES (CITY, COUNTY, STATE, AND FEDERAL). IF YOUR CITY OR STATE REQUIRES AN ENGINEER'S STAMP, YOU WILL NEED TO HAVE THAT DONE LOCALLY BY A QUALIFIED ARCHITECT OR ENGINEER.

EXTERIOR ELEVATIONS

SHEET NUMBER
1 of 2

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

## ROOF PLAN NOTES

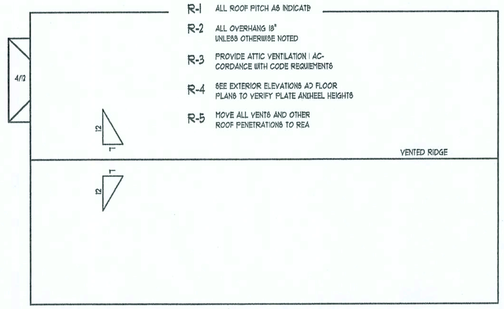
R-1 ALL ROOF PITCH AS INDICATED

R-2 ALL OVERHANG 18" UNLESS OTHERWISE NOTED

R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIEMENTS

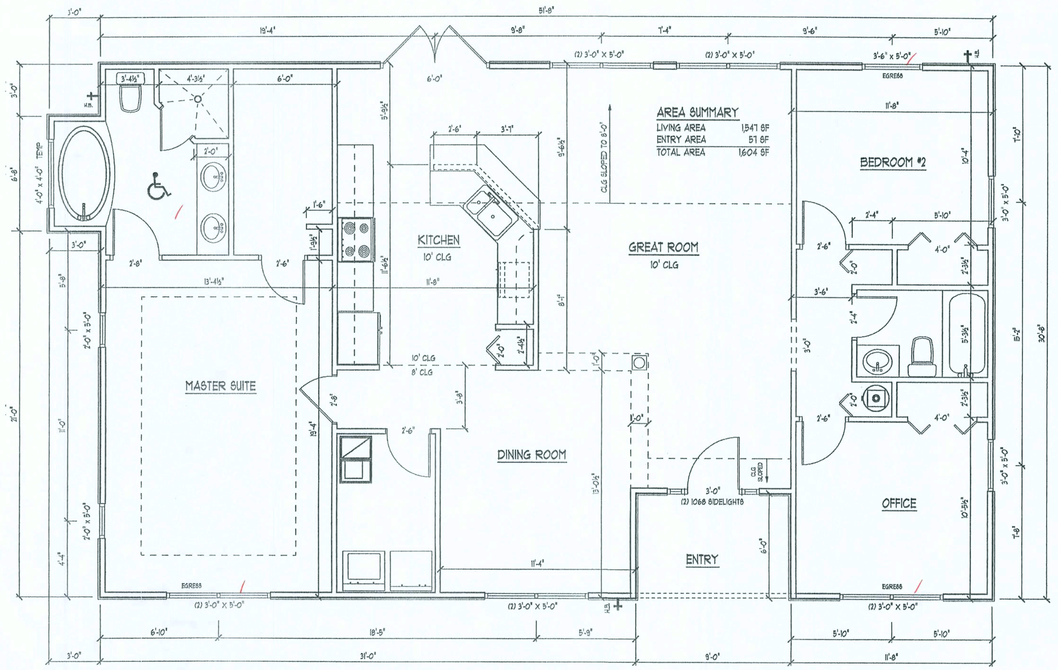
R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HEEL HEIGHTS

R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR



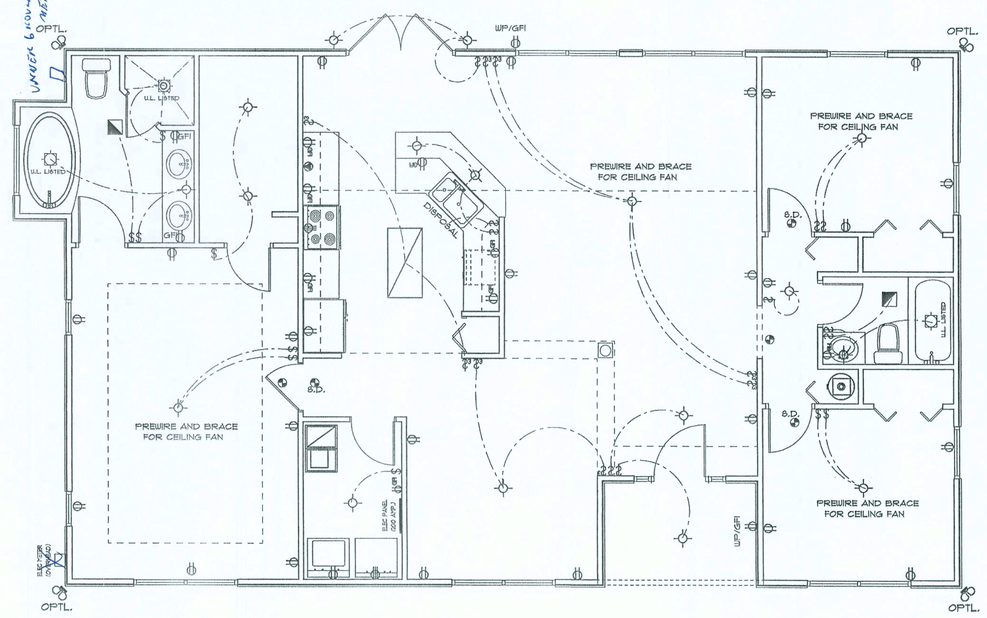

## ROOF PLAN

SCALE: 1/8" = 1'




## FLOOR PLAN SCALE: 1/4" = 1'

NOTE: ALL CEILINGS 8'-0" UNLESS OTHERWISE NOTED




NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS, DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

## ELECTRICAL PLAN

SCALE: 1/4" = 1'

## ELECTRICAL PLAN NOTES

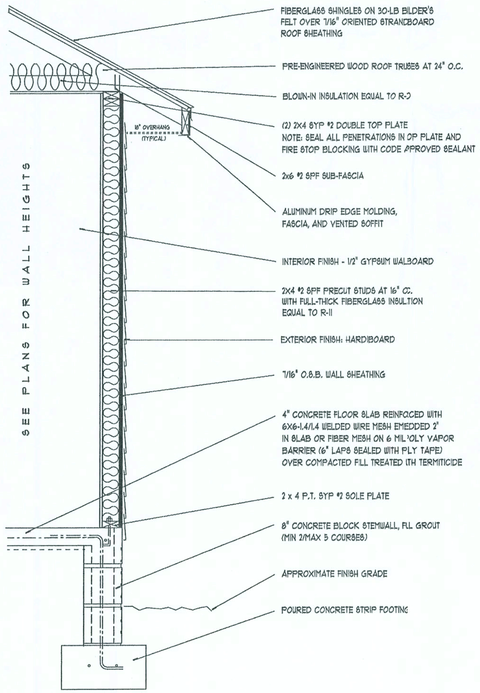
E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.

E-2 NOTE: ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTIVATE ALL ALARMS IF ANY ONE UNIT IS ACTUATED.

E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.

E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI. (ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL SERVICE PROVIDED BY
CLAY ELECTRIC COOPERATIVE




## TYPICAL WALL SECTION

SCALE: 3/4" = 1'0"

REFER TO STRUCTURAL PAGE FOR STRUCTURAL SPECIFICATIONS

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRICEDENCE OVER SCALED DIMENSIONS

March 29, 2006

D.D.S. Studios
D.D.S. STUDIOS
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FLOOR PLAN
ELEC PLAN
ROOF PLAN
TYPICAL WALL SECTION

SHEET NUMBER
2 of 2

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

Daniel Shaheen