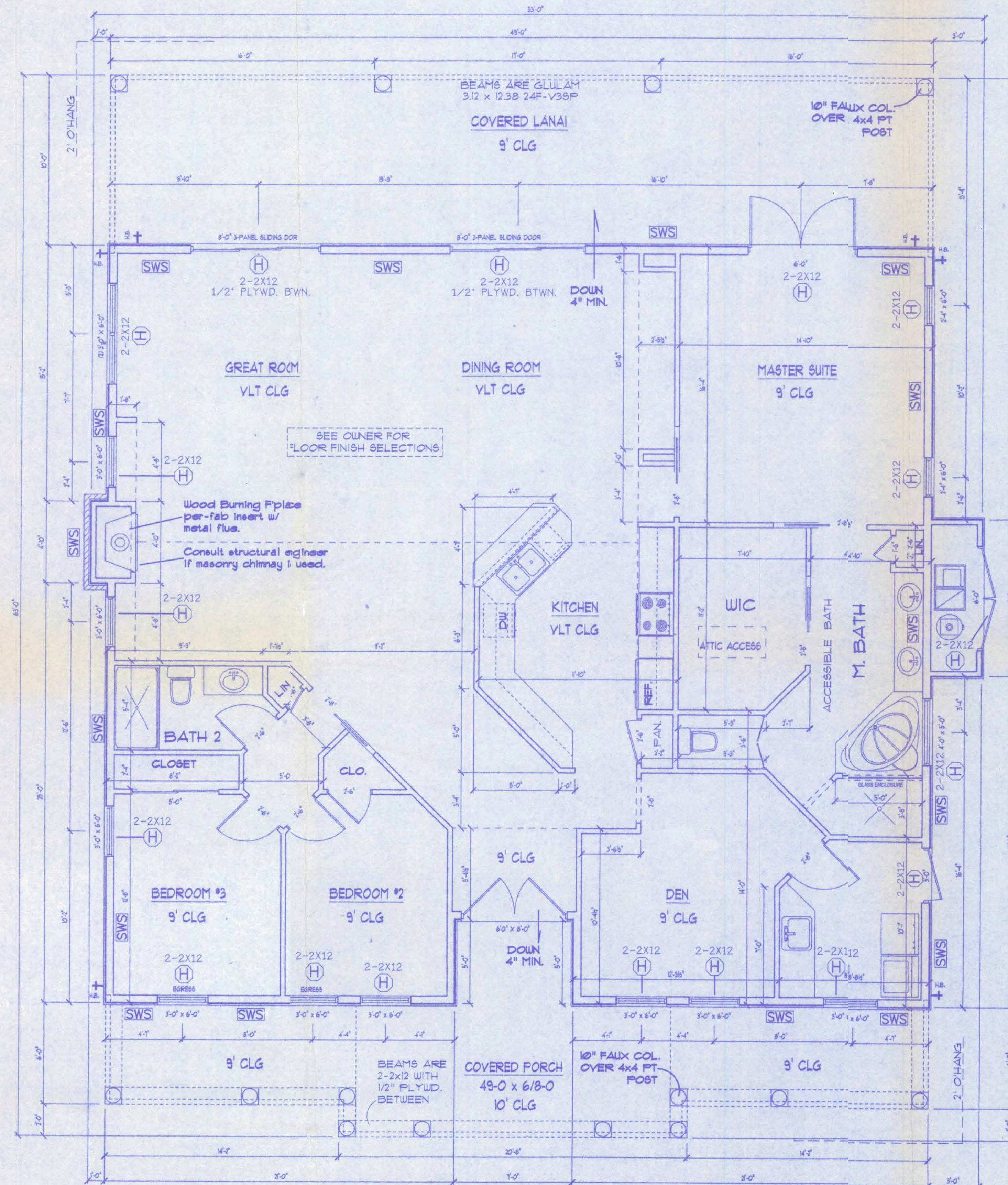


Huett Residence

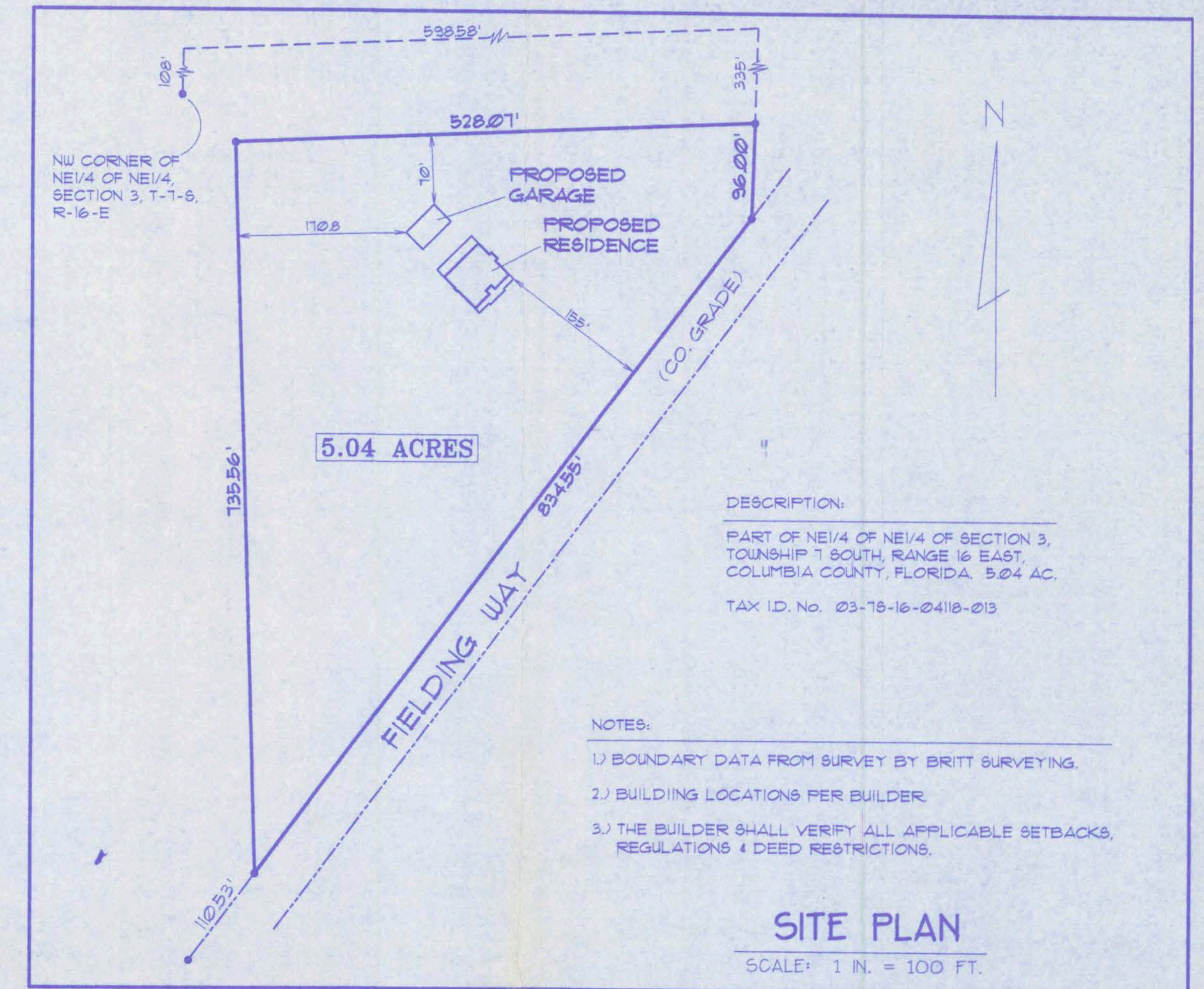


FLOOR PLAN

SCALE: 1/4" = 1'

AREA SUMMARY

MAIN FLOOR	1731 SF
LANAI AREA	153 SF
ENTRY AREA	117 SF
TOTAL AREA	2153 SF



DESCRIPTION:

PART OF NE 1/4 OF NE 1/4 OF SECTION 3,
TOWNSHIP 7 SOUTH, RANGE 16 EAST,
COLUMBIA COUNTY, FLORIDA. 5.04 AC.

TAX I.D. No. 03-18-16-04118-013

NOTES:

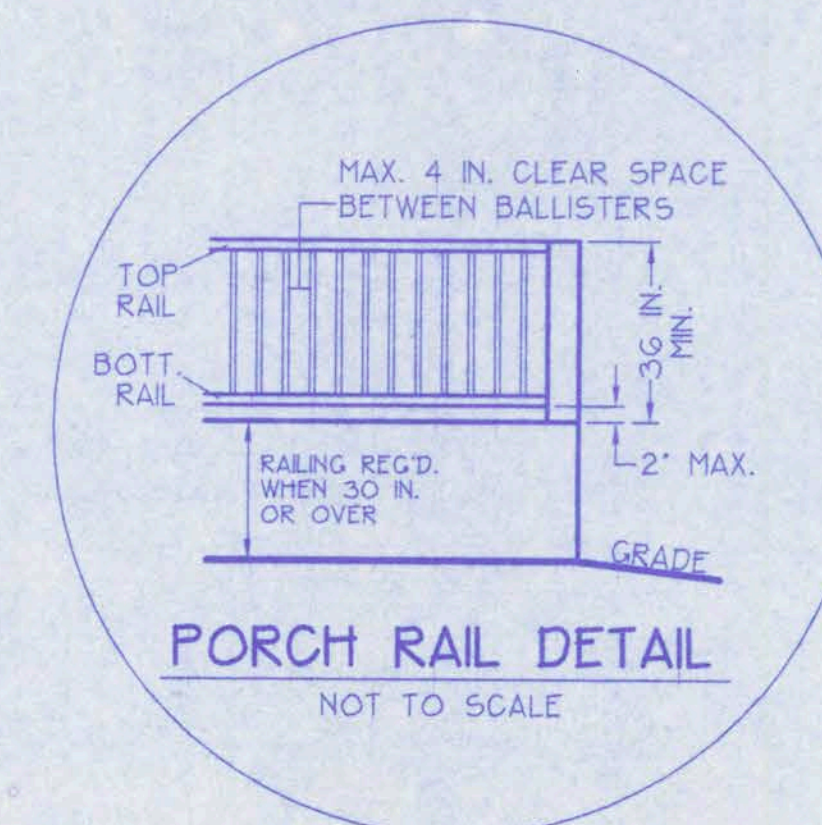
- 1) BOUNDARY DATA FROM SURVEY BY BRITT SURVEYING.
- 2) BUILDING LOCATIONS PER BUILDER.
- 3) THE BUILDER SHALL VERIFY ALL APPLICABLE SETBACKS, REGULATIONS & DEED RESTRICTIONS.

SITE PLAN

SCALE: 1 IN. = 100 FT.

2-2X12
(H) = Indicates Header Size for designated opening.

SWS = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.



PORCH RAIL DETAIL
NOT TO SCALE

Index to Sheets

SHEET 1	FLOOR PLAN - HOUSE + SITE PLAN
SHEET 2	ELEVATIONS - HOUSE + GEN. NOTES
SHEET 3	ELEVATIONS - HOUSE
SHEET 4	FLOOR PLAN - GARAGE + ELEVATIONS - GARAGE + ELECTRICAL - GARAGE
SHEET 5	FOUNDATIONS - ALL + SECTIONS / DETAILS
SHEET 6	ENGINEERING DATA - ALL + TYP. WALL SECTION
SHEET 6	ELECTRICAL - HOUSE

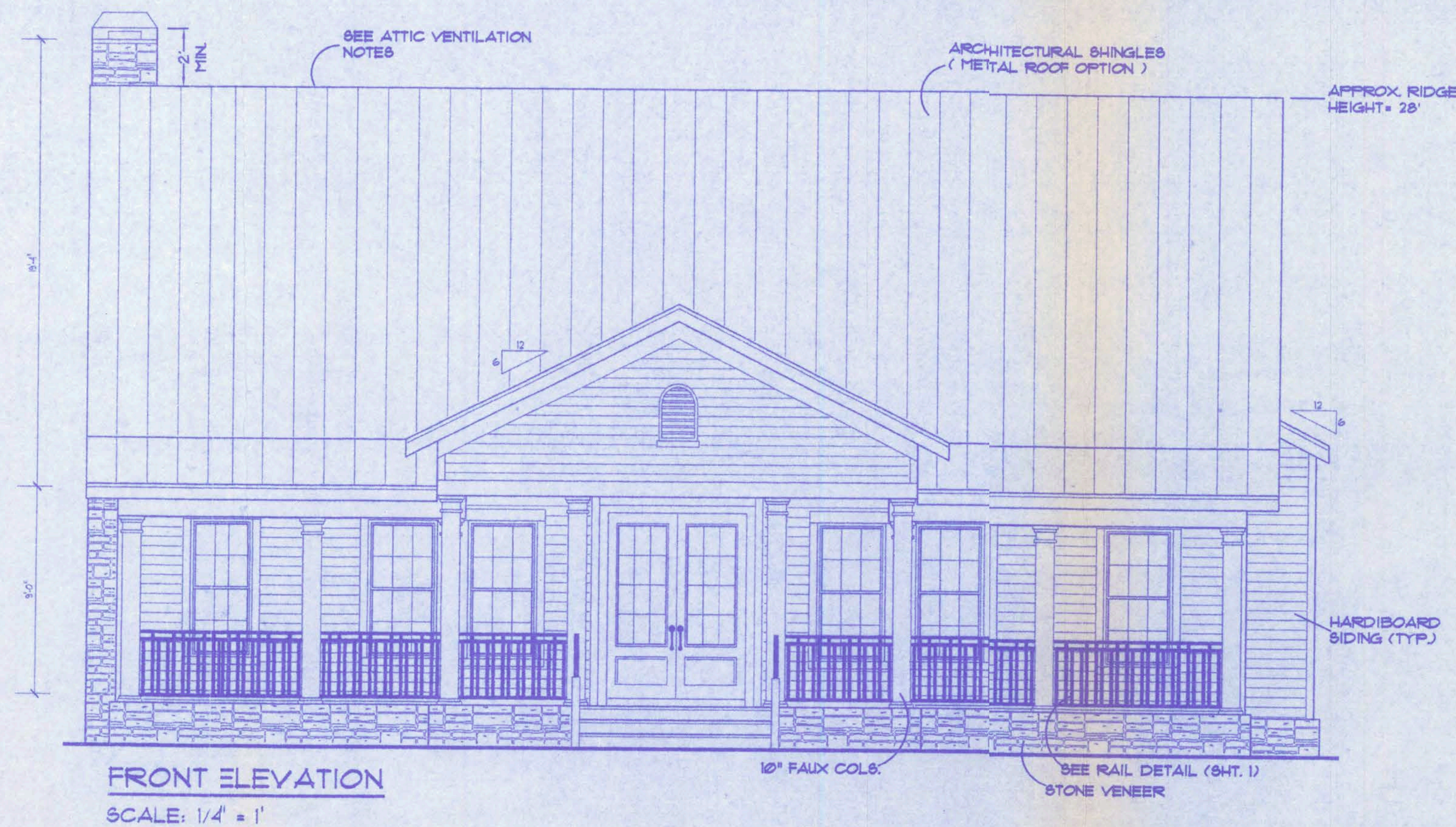
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

SIGNED: _____ DATE: ____/____/2006

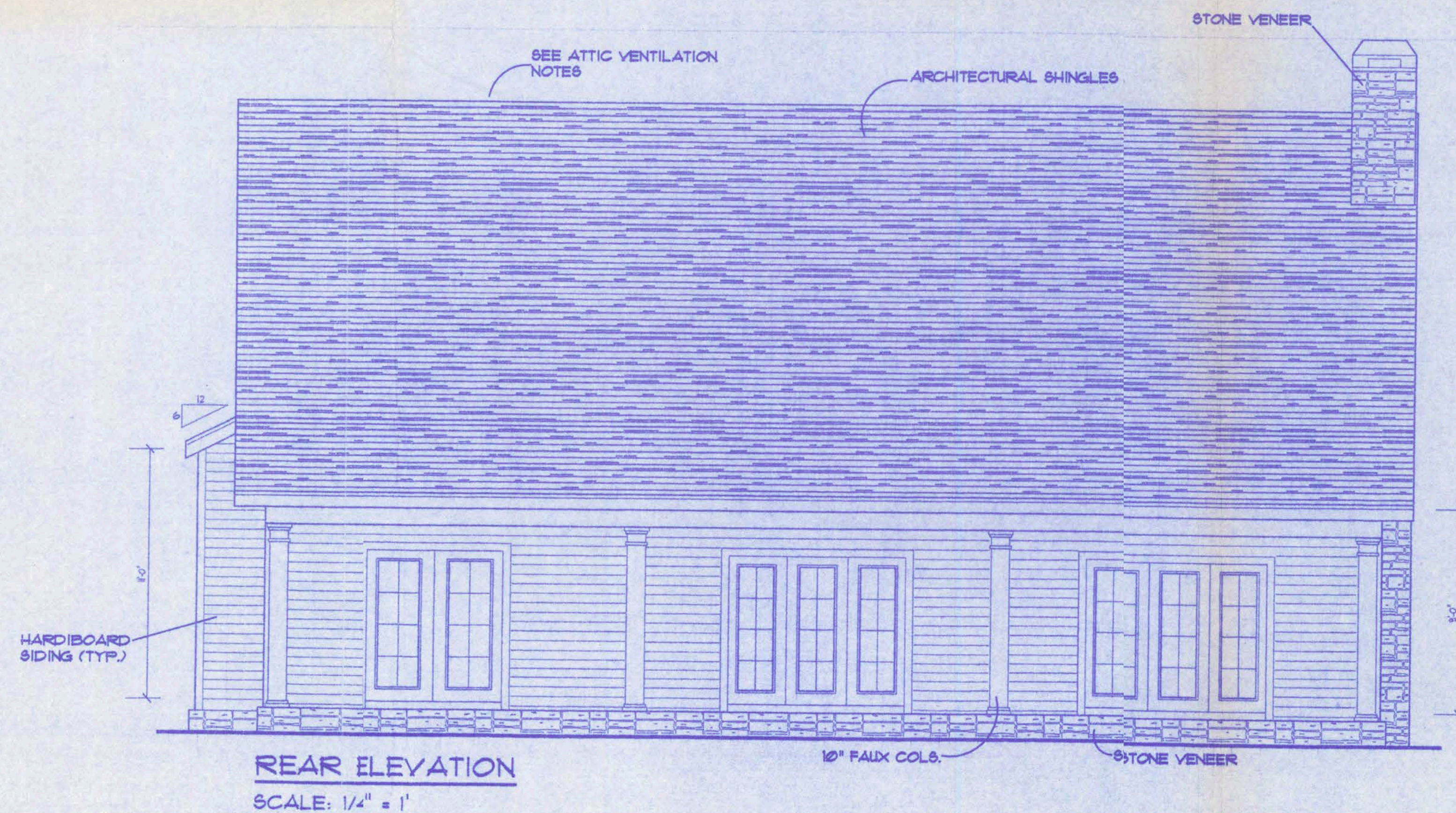
FILE: 06-001	HUETT RESIDENCE	REV:
DATE: 1/17/2006		REV:
DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: (386)497-3991	CAD FILE: 06001
CHECK: HH		SHEET: 1 OF 1

1



GENERAL NOTES

- 1.) All concrete used to be 2500 PSI strength or greater.
- 2.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 3.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 4.) Roof Truss design is the responsibility of the supplier.
- 5.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 6.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 7.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.



ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

WINDLOAD ENGINEER: Huey Hawkins, P.E. No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

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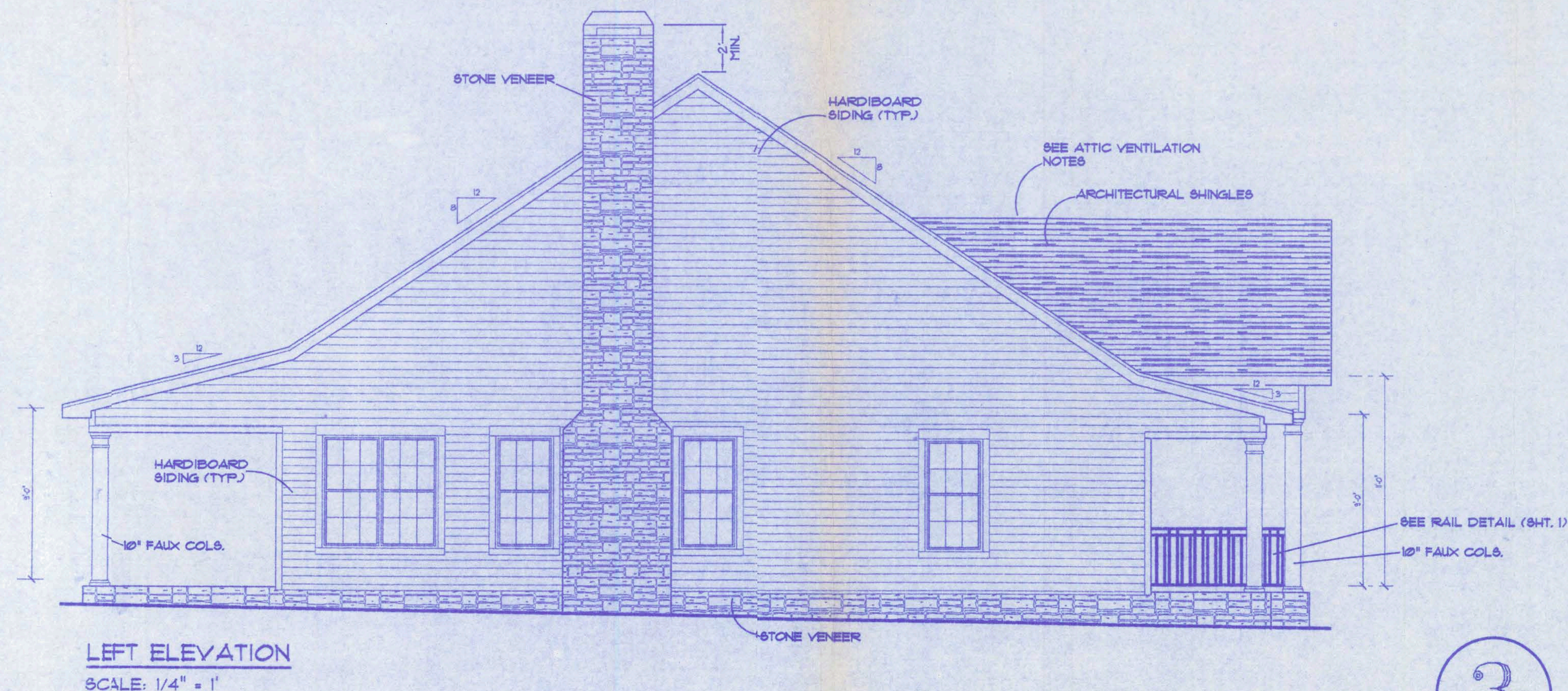
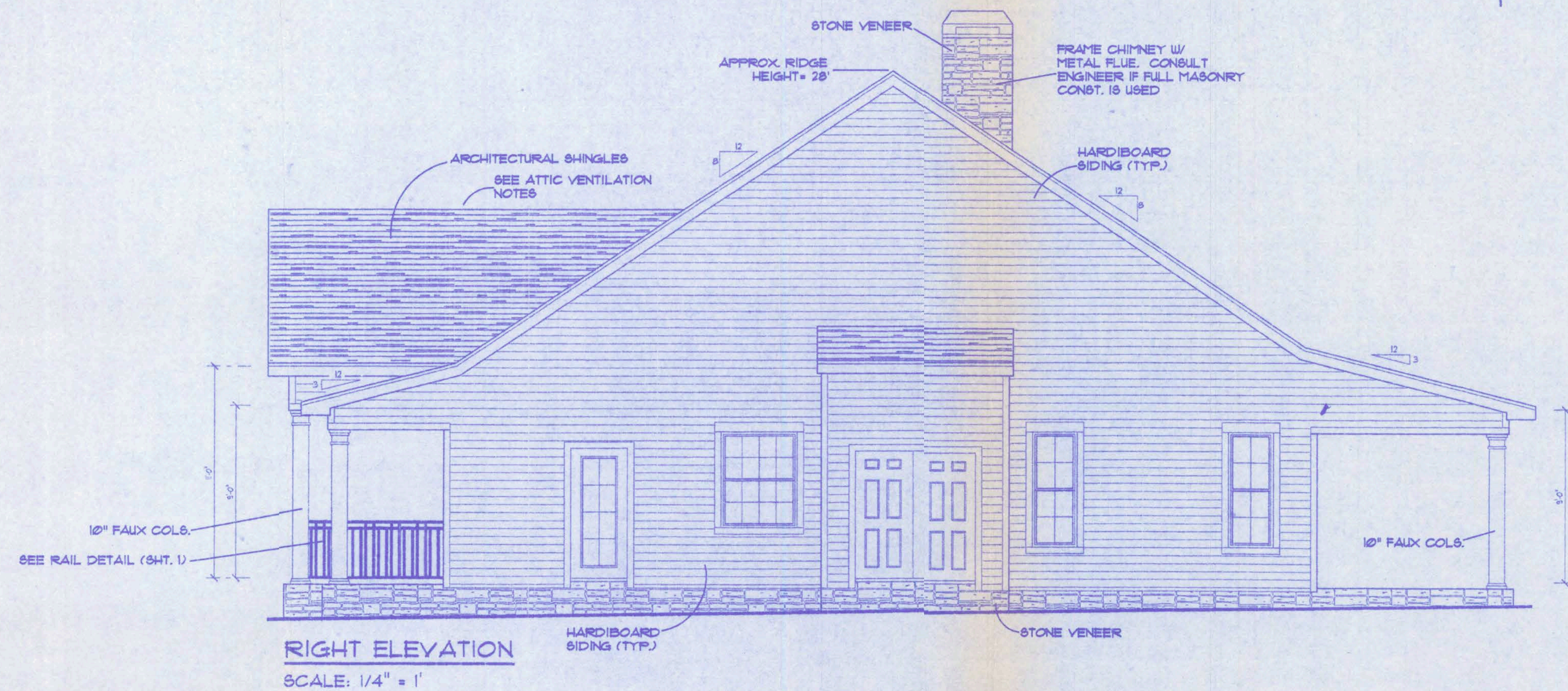
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DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: (386)497-3991	CAD FILE: 06001
CHECK: HH		SHEET: 2 OF 7

2

Huey Hawkins

2-10-06



ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

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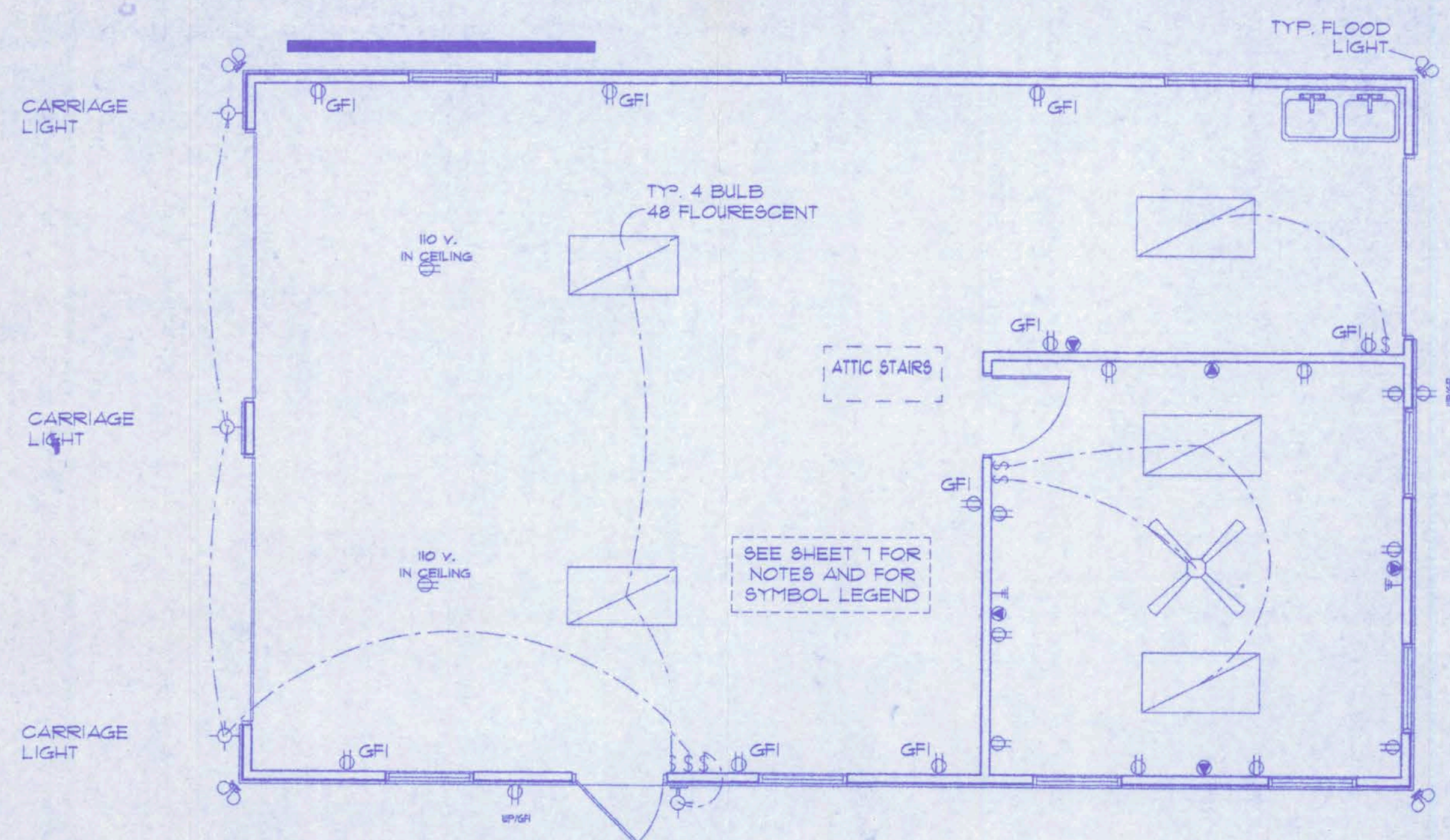
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3891

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

SIGNED: _____ DATE: ____/____/2006

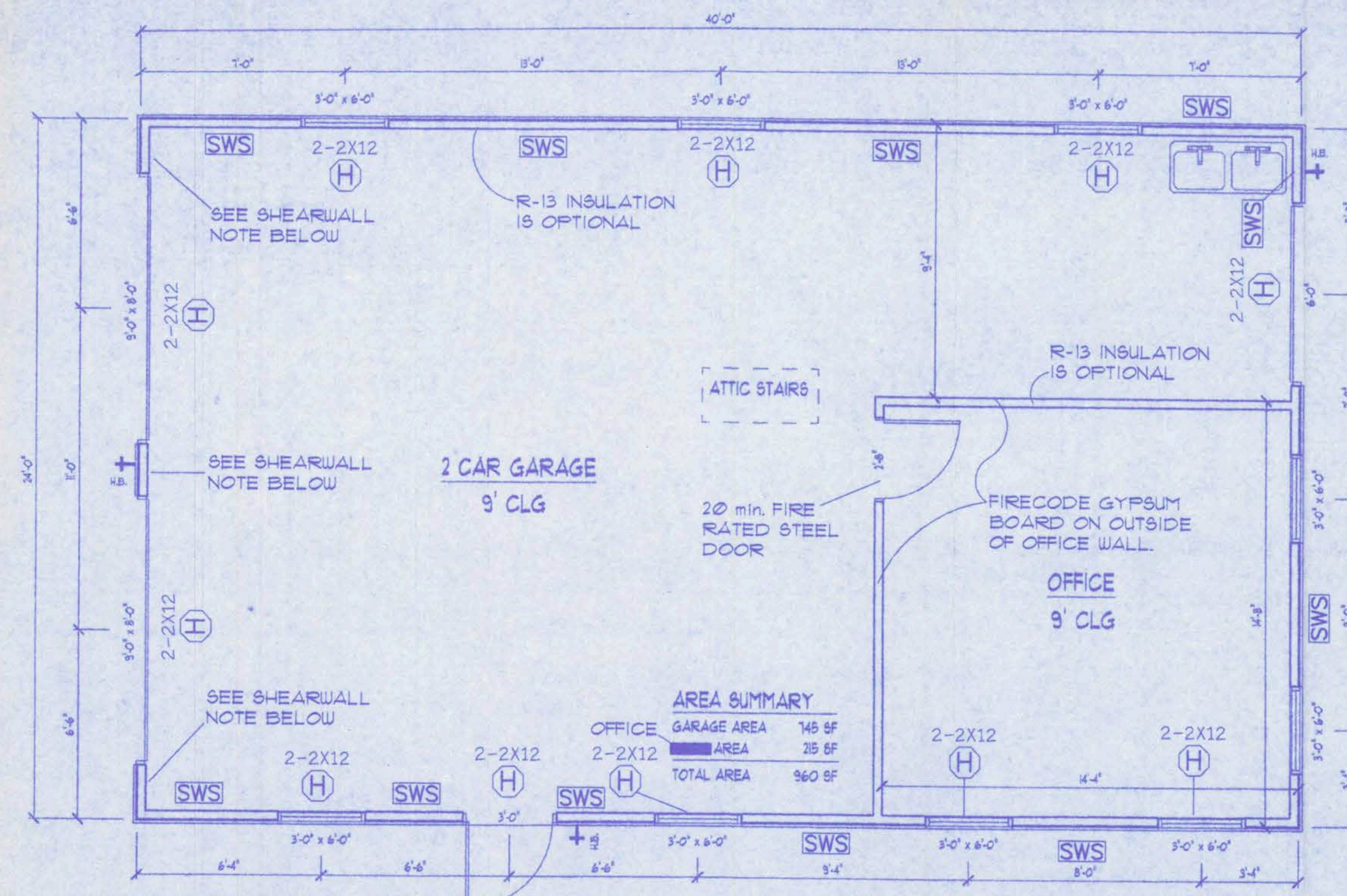
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DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: (386)497-3991	CAD FILE: 06001
CHECK: HH		SHEET: 3 OF 7

3
Huey Hawkins
2-10-06



ELECTRICAL PLAN

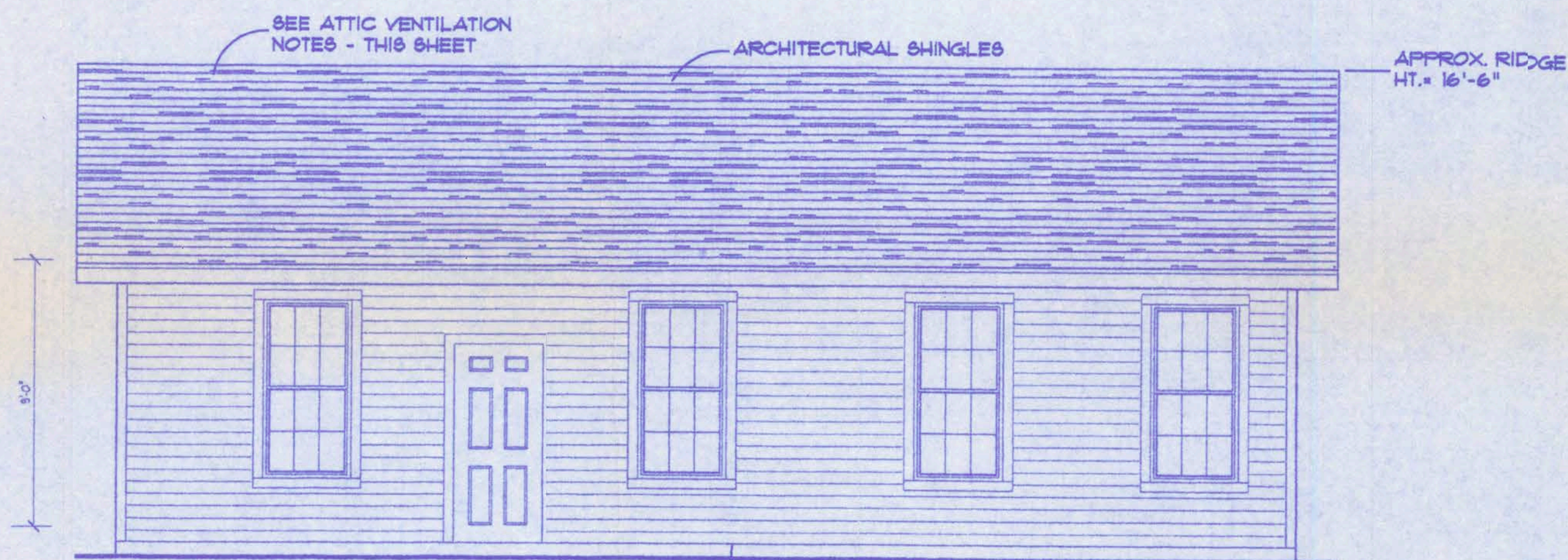
SCALE: 1/4" = 1'



FLOOR PLAN

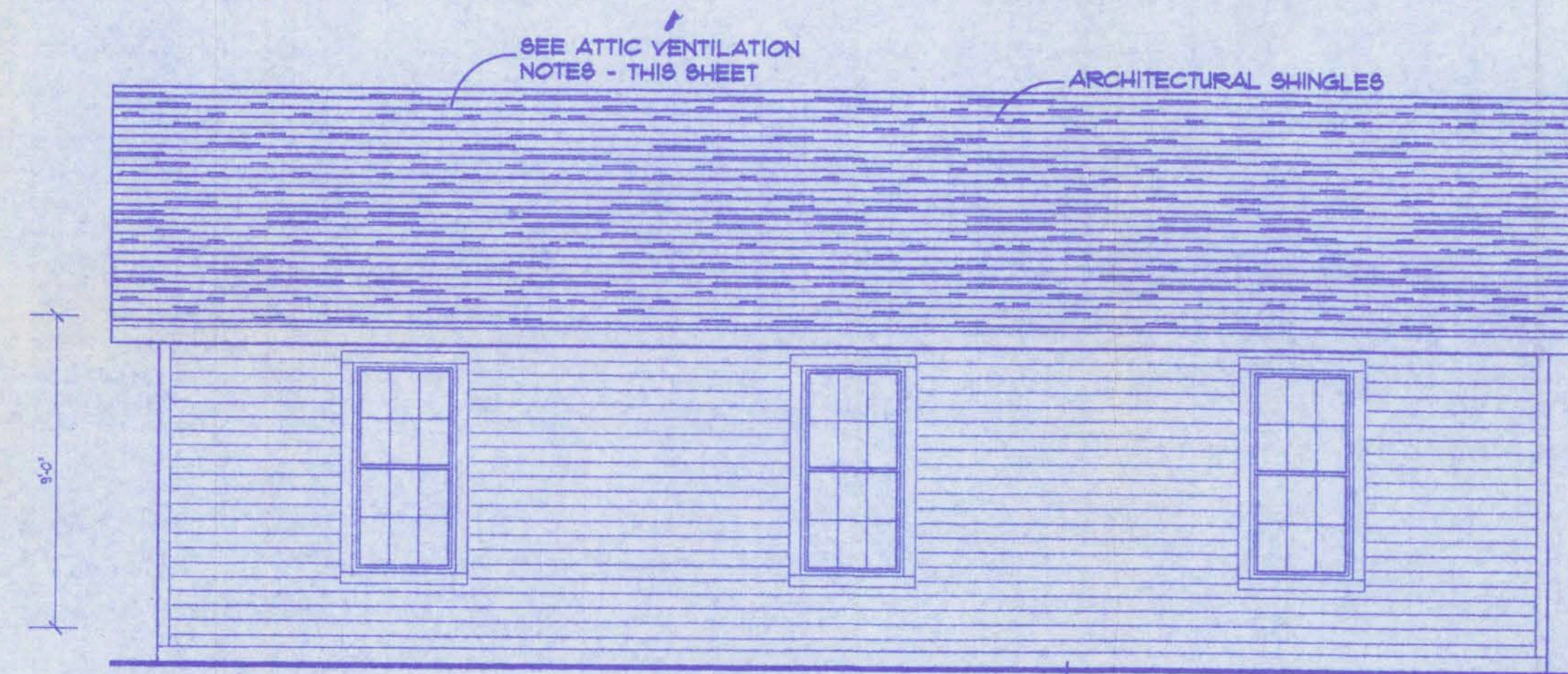
SCALE: 1/4" = 1'

SHEARWALL NOTE AT GARAGE DOORS:
SHEATH BOTH SIDES, EA. SIDE, SIMPSON SP44 W/
10-12d NAILS, 1/2" ANCHOR BOLT, FULLY BLOCKED
W/ NO JOINT WITHIN 3' OF JACK STUD INTERSECTION
WITH HEADER



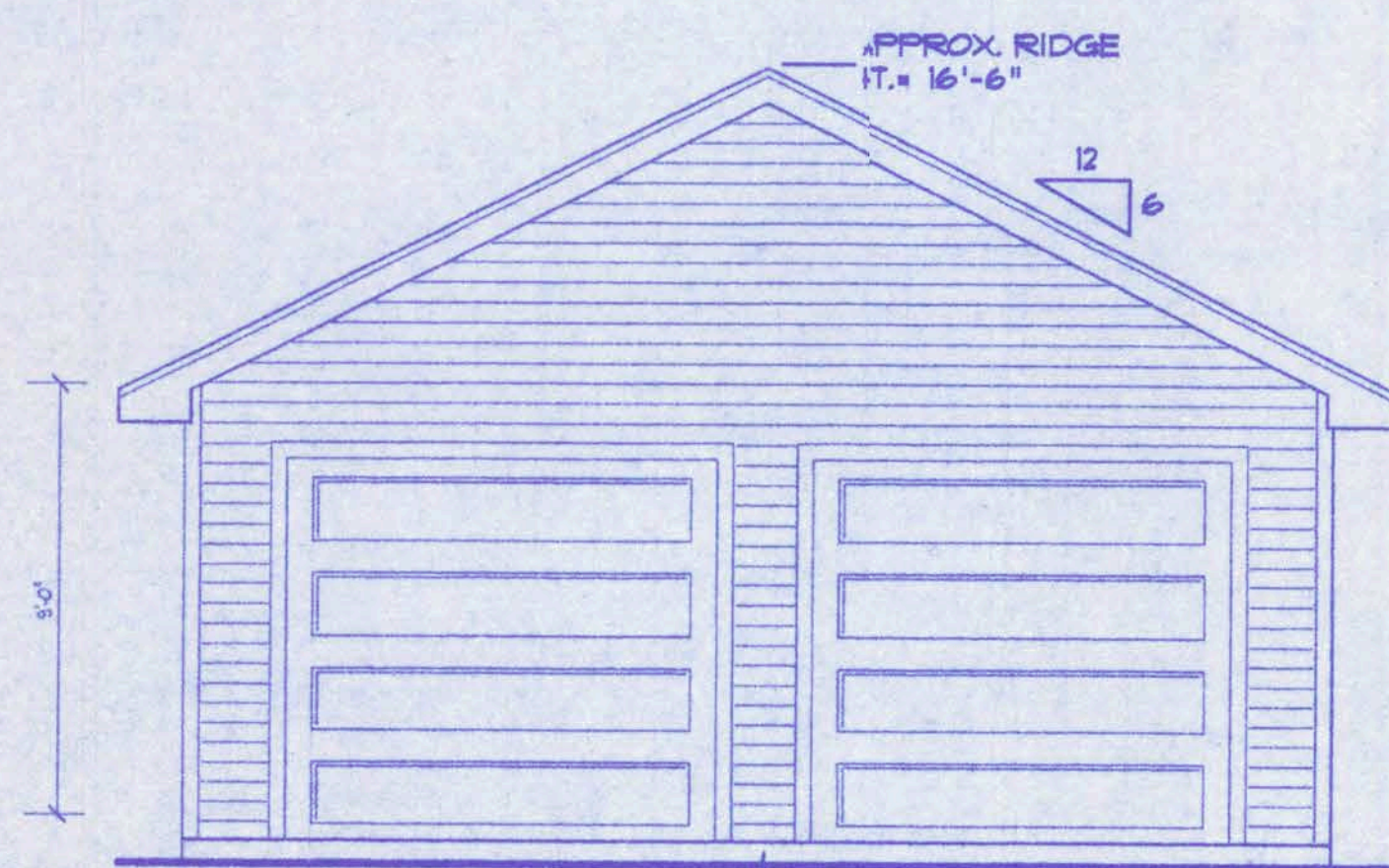
FRONT ELEVATION

SCALE: 1/4" = 1'



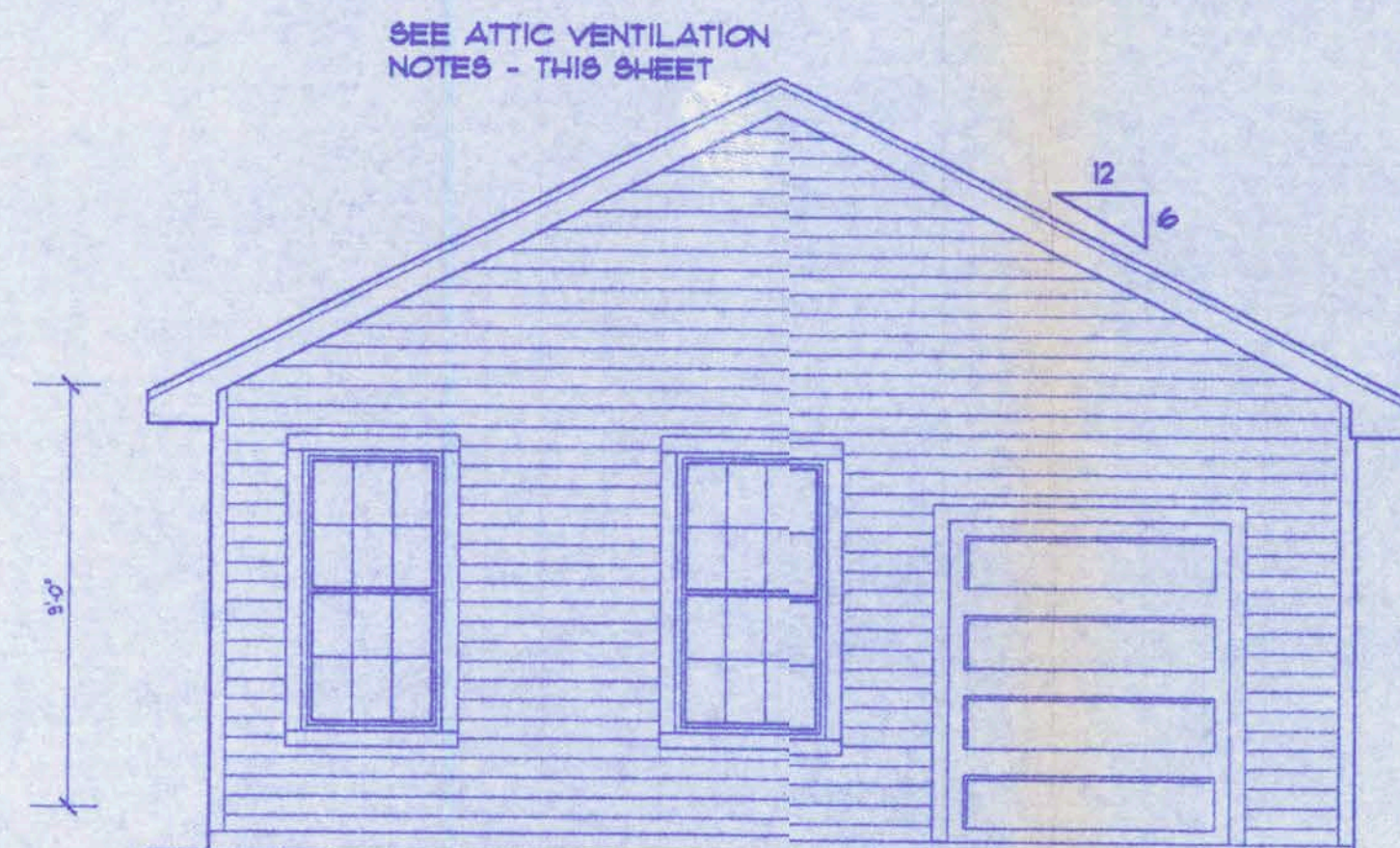
REAR ELEVATION

SCALE: 1/4" = 1'



LEFT ELEVATION

SCALE: 1/4" = 1'



RIGHT ELEVATION

SCALE: 1/4" = 1'

ATTIC VENTILATION

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FLOOR PLAN NOTES

2-2X12
(H) = Indicates Header Size for designated opening.

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WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

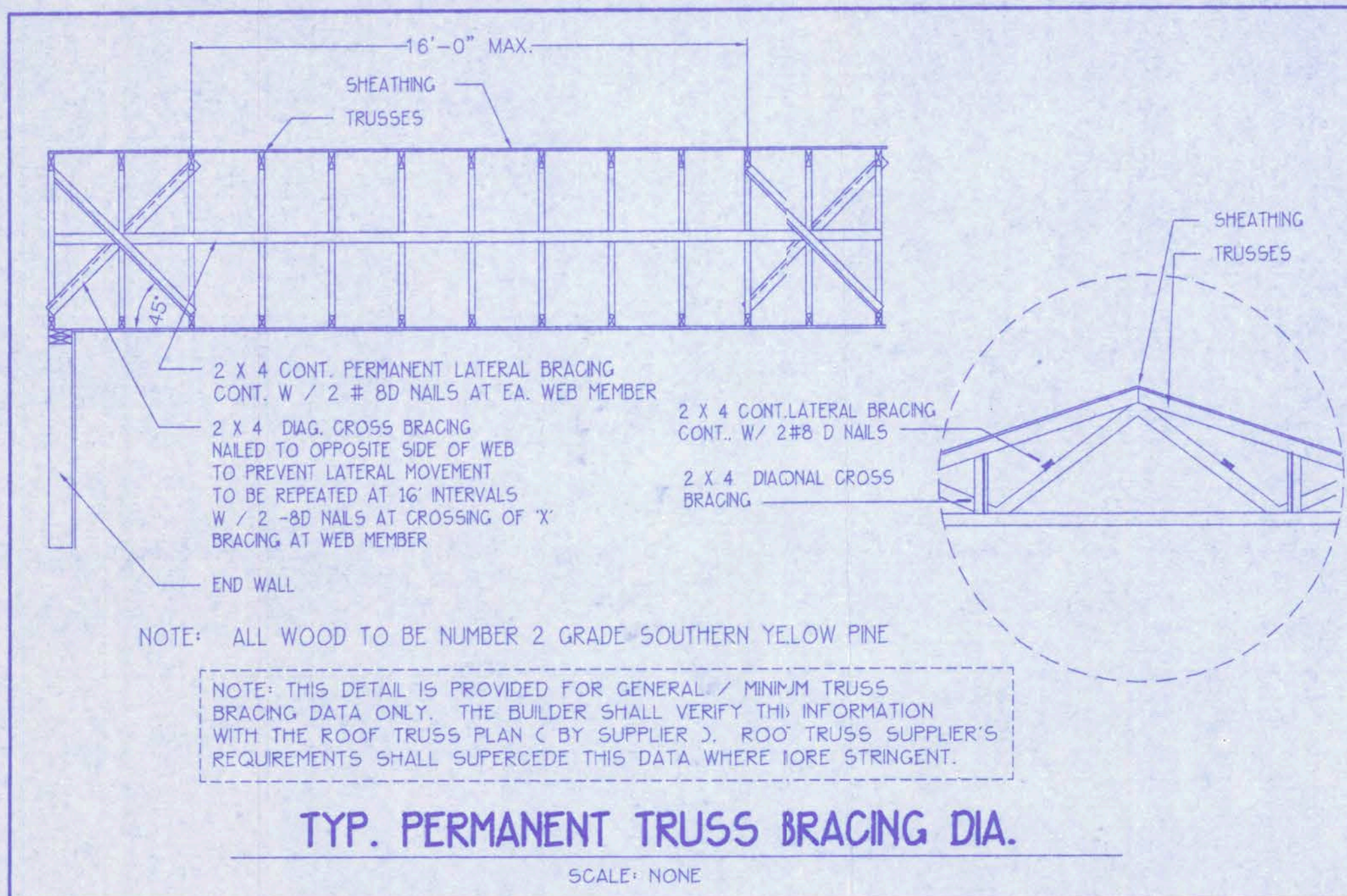
CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

SIGNED: _____ DATE: ____/____/2006

FILE: 06-001	" HUETT RESIDENCE	REV:
DATE: 1/11/2006		REV:
DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: (386)497-3991	CAD FILE: 06001
CHECK: HH		SHEET: 4 OF 7

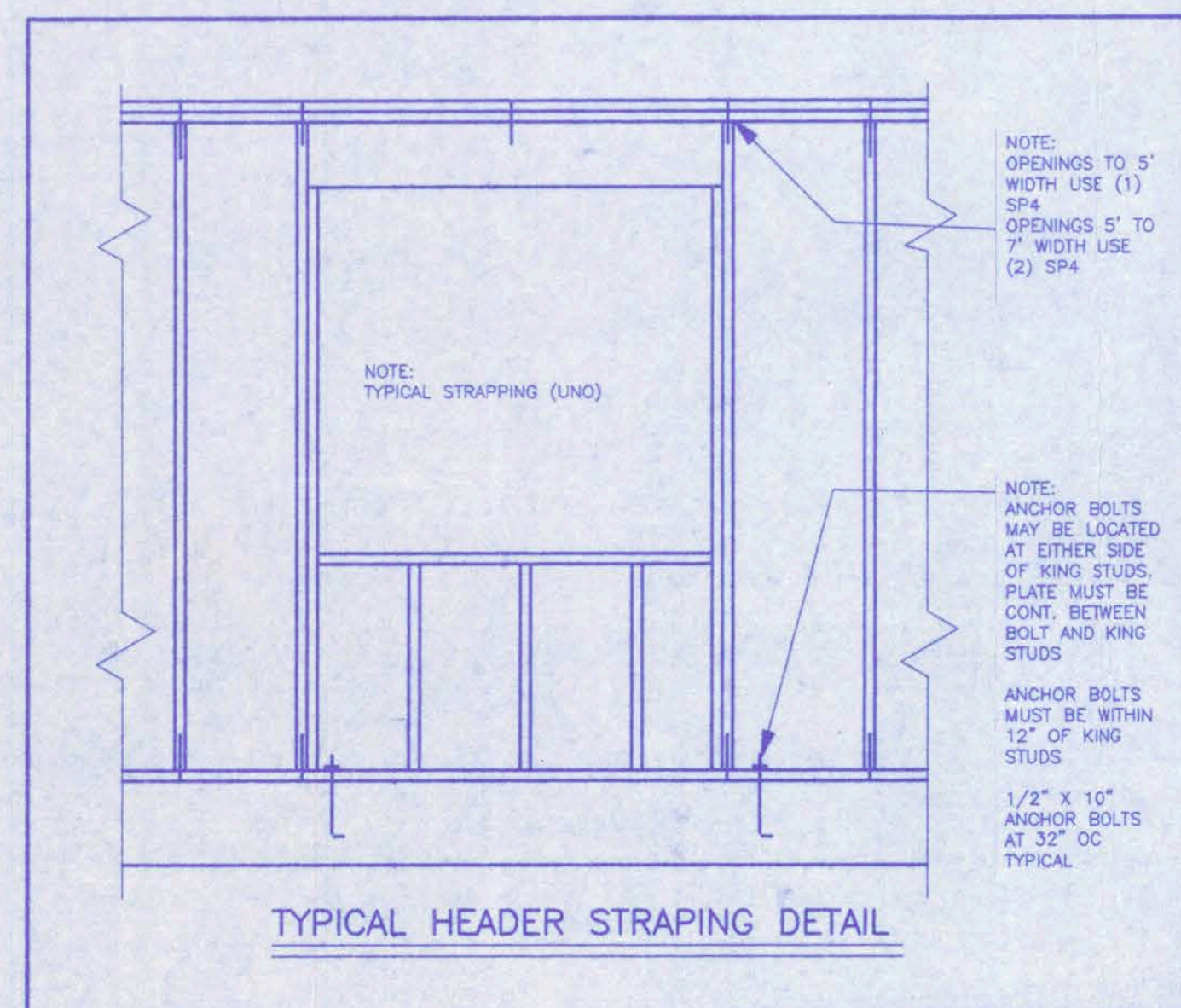


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DATE: 1/17/2006		REV:
DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: C3862497-3991	CAD FILE: 06001
CHECK: HH		SHEET: 5 OF 7



ENGINEER'S NOTES:

- This report establishes the minimum requirements for wind load stability. It is the owner/builder's responsibility to provide materials and construction techniques which comply with SBC requirements for the stated wind velocity. It is the builder's responsibility to provide a continuous load path from trusses to foundation.
- Since truss engineering was not complete at the time of this analysis, it is the builder's responsibility to select uplift connections based on truss engineering uplift and provide footings for interior bearing walls. Builder is to furnish truss engineering to wind load engineer for review of truss reactions on the building structure. Strap 2x6 rafters with min uplift connection 415lb each end; 2x8 rafters 700lb each end.
- Site analysis and preparation information is not part of this plan and is responsibility of the owner. All foundations and footings are designed for stable soil conditions with 1000 psf bearing capacity. It is the owner's / builder's responsibility to verify soil and clean fill are compacted to provide 1000 psf minimum bearing capacity or to request foundation design based on actual site conditions.
- Manufacturers and product number for connectors, anchors, and reinforcement are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads.
- Anchor bolts - A-307. Minimum embedment: 7" in concrete or reinforced bond beam; 15" in grouted CMU.
- Concrete - Minimum compressive strength, $F_c = 2500$ psi.
- Rebar - Grade 40 deformed bars, $F_y = 36$ ksi. All laps 40xDb (25" for #5) unless otherwise specified.
- Nails - All nails are common nails unless otherwise specified or accepted by SBC test reports as having equal structural values.



DESIGN DATA

WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2	
(FOR ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED AND GABLE-SHAPED ROOFS HAVING A MEAN ROOF HEIGHT NOT EXCEEDING THE LEAST HORIZONTAL DIMENSION OF THE BUILDING OR 60 FT. NOT SITED ON THE UPPER HALF OF A HILL OR ESCARPMENT 80 FT IN EXPOSURE B, 30 FT IN EXPOSURE C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED =	110 MPH
2.) WIND IMPORTANCE FACTOR =	1
3.) BUILDING CATEGORY =	II
4.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLOSED BUILDING)
5.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES:	
(DOORS & WINDOWS)	+21.8 / -29.1 PSF
GARAGE DOOR (N/A)	9x7 +19.3 / -22.2 PSF; 16x7, +18.5 / -20.9
DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
NO SNOW LOAD	
WIND LOADS ARE PER FLORIDA BUILDING CODE 2001, SECTION 1606.2	

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2001, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2001 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. SELECT UPLIFT CONNECTIONS AND PROVIDE FOOTINGS BASED ON TRUSS ENGINEERING REACTIONS. FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS.

TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQ. FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TOP CONNECTOR *	BOTTOM CONNECTOR *
< 415	H2.5 10-8d	415 NO SPECIAL CONNECTOR REQ.
< 750	H16 6-10d, 1-1/2"	750 NO SPECIAL CONNECTOR REQ.
< 905	H10 16-8d, 1-1/2"	905 NO SPECIAL CONNECTOR REQ.
< 1250	H16 10-10d, 1-1/2"	1250 SPH4 W/10-10d, 1-1/2" + 1/2" AB
< 1245	HTS20 24-10d, 1-1/2"	1245 LTT19 W/8-16d + 1/2" AB
< 2490	2-HTS20 24-10d, 1-1/2"	2490 HD2A-2.5", 5/8" AB

UPLIFT GREATER THAN 2500 LBS. REQUIRES ENGINEERING APPROVAL

* MODEL NUMBERS SHOWN ARE FOR SIMPSON CONNECTORS. THESE ARE LISTED AS EXAMPLES + NOT FOR ENDORSEMENT. OTHER MANUFACTURER'S CONNECTORS WITH SUFFICIENT LOAD CAPACITY MAY BE SUBSTITUTED. FOLLOW MANUFACT. INSTRUCTIONS.

WALL STUD TABLE

STUDS ARE SPRUCE/PINE/FIR AT 16 IN. O.C.

1 - 2 X 4	TO 10 FT. WALL HEIGHT
2 - 2 X 4	TO 12.5 FT. WALL HEIGHT
3 - 2 X 4	TO 16.5 FT. WALL HEIGHT
1 - 2 X 6	TO 16 FT. WALL HEIGHT
2 - 2 X 6	TO 21 FT. WALL HEIGHT

TOP PLATE IS 2 - 2 X 4 (OR 2 X 6) SPRUCE/PINE/FIR.
BOTTOM PLATE IS 2 X 4 (OR 2 X 6) PRESS. TREATED BEARING ON FOUNDATION WALL.

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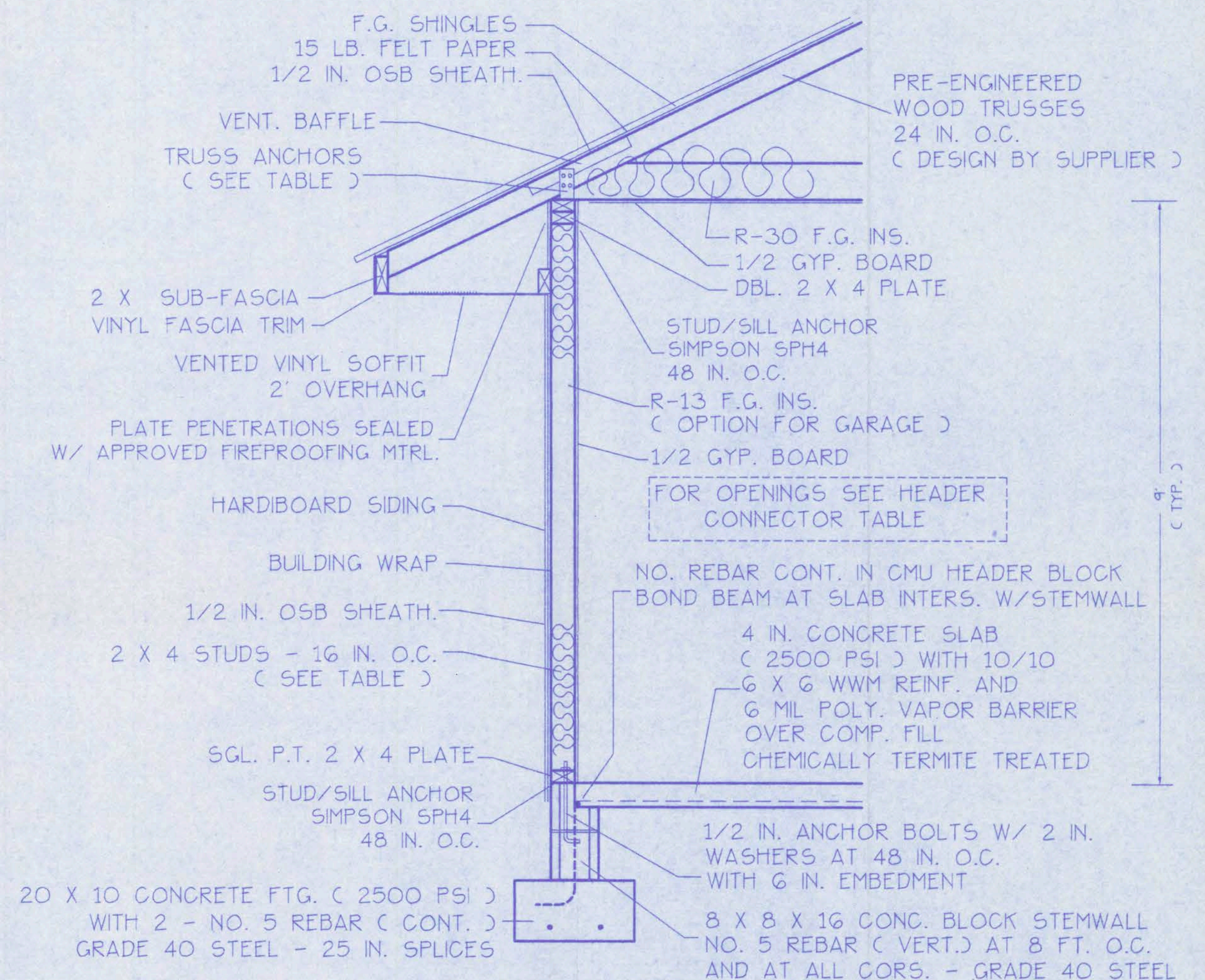
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GENERAL TRUSS NOTES:

- TRUSSES SHALL BE DESIGNED BY A LICENSED ENGINEER, AND IN ACCORDANCE WITH THE REQUIREMENTS OF THE 'NATIONAL FOREST PRODUCTS ASSOCIATION' MANUAL FOR 'STRESS RATED LUMBER AND ITS CONNECTIONS', LATEST Ed. ALONG W/ THE 'TRUSS PLATE INSTITUTE' SUGGESTED GUIDELINES FOR TEMPORARY AND PERMANENT BRACING. AND HANDLING OF TRUSSES. TRUSS SHOP DRAWINGS SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, DETS. + TRUSS TO TRUSS CONNECTIONS.
- TRUSS SHOP DRAWINGS SHALL BE SIGNED + SEALED BY THE DESIGNING ENGINEER.

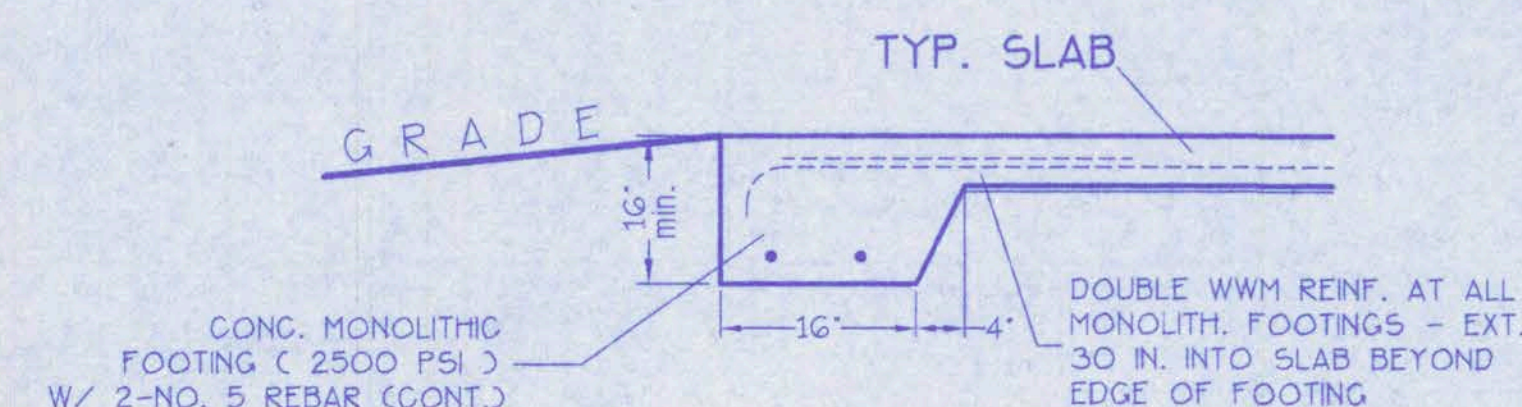
SHEATHING NAILING

WALLS - 4 IN. O.C. EDGES, 8 IN. O.C. FIELD FULLY BLOCKED. CONT. FROM TOP TO BOTTOM PLATE.
ROOF - 4 IN. O.C. EDGES, 8 IN. O.C. FIELD UNBLOCKED.



TYPICAL WALL SECTION

SCALE: 3/4 IN. = 1 FT.



GARAGE MONOLITHIC FOOTING

SCALE: 3/4 IN. = 1 FT.

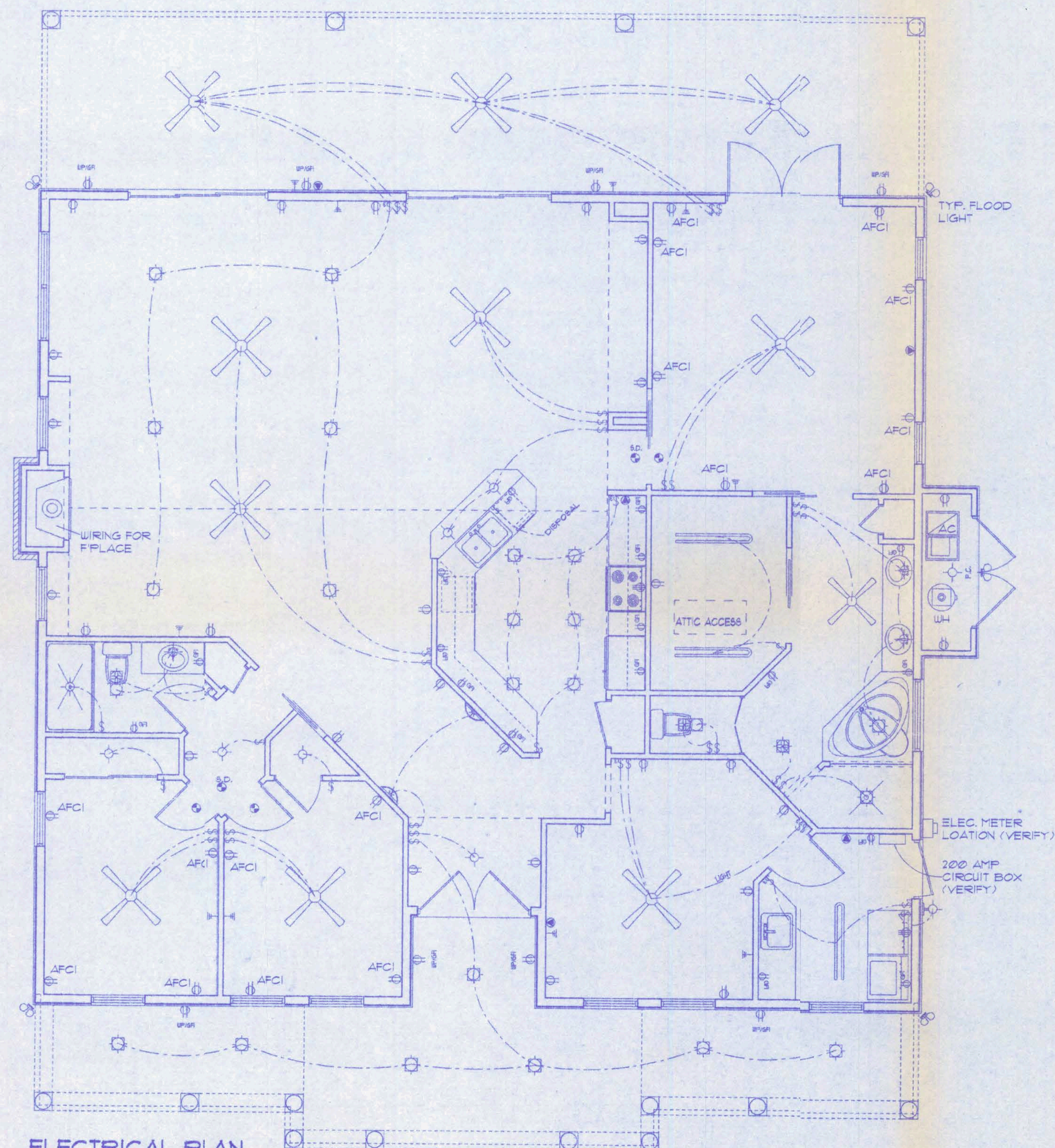
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

SIGNED: _____ DATE: ____/____/2006

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DATE: 1/17/2006		REV:
DRAWN: TAD	HUEY HAWKINS, P.E. 6855 SW Elm Church Rd. Fort White, Florida 32038 Ph: (386)497-3991	CA) FILE: 06001
CHECK: HH		SHEET: 6 OF 7

6
Huey Hawkins
2-0-06



ELECTRICAL PLAN

SCALE: 1/4" = 1'

ELECTRICAL SYMBOL LEGEND

	= FLOURESCENT LIGHTING FIXTURE.
	= CEILING LIGHT FIXTURE
	= LIGHT SWITCH.
	= THREE-WAY SWITCH.
	= 110 V. DUPLEX OUTLET.
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
	= GROUND FAULT CIRC. OUTLET
	= ARC. FAULT CIRC. OUTLET
	= 110 V. SINGLE RECEPTACLE OUTLET.
	= 220 VOLT OUTLET (4 WIRE)
	= FAN LOCATION (CEILING)
	= FAN LOCATION (EXHAUST) W/ LIGHT
	= SMOKE DETECTOR
	= PHONE JACK
	= CATV JACK

ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE 1999.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 1999.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

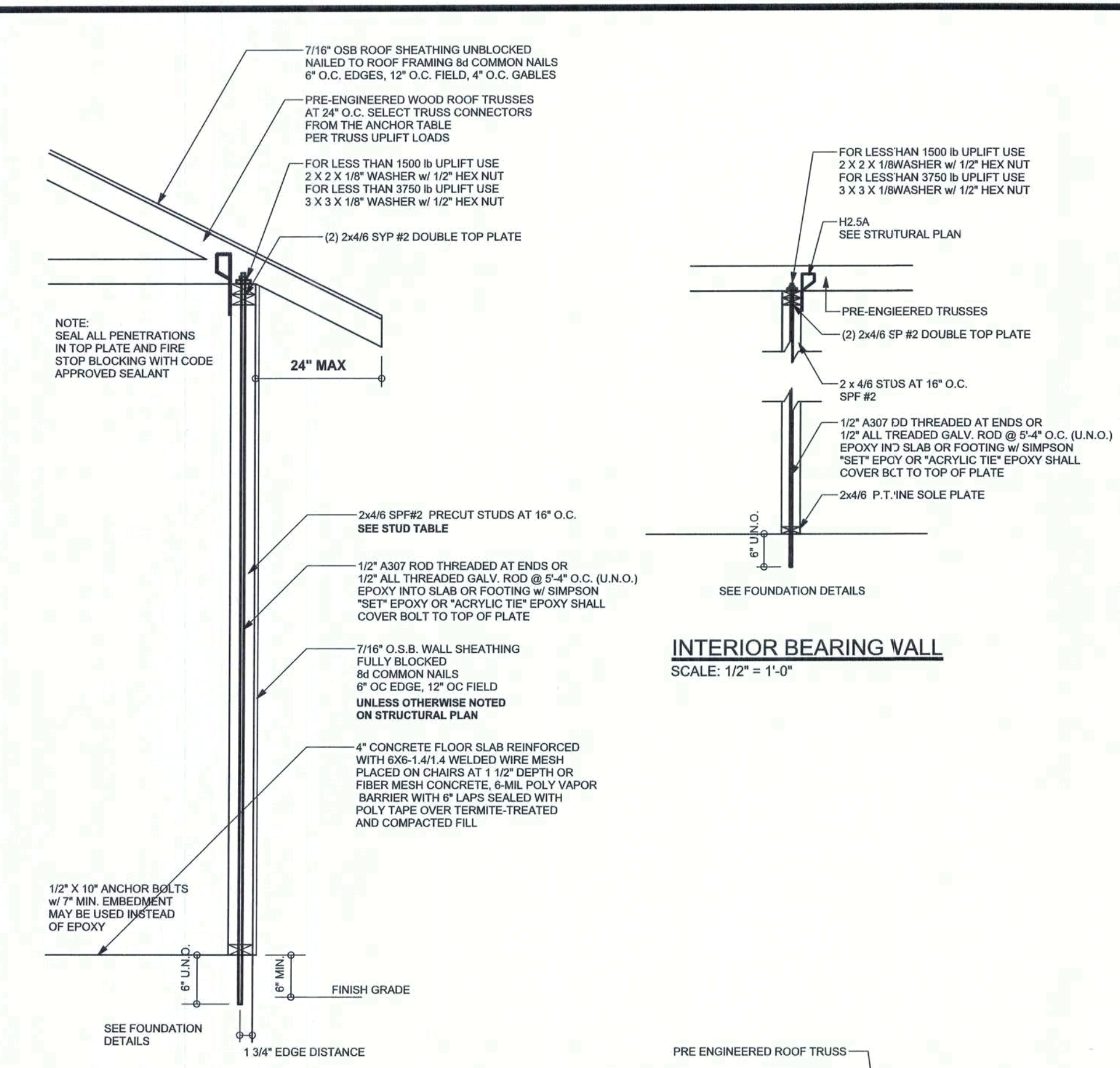
WINDLOAD ENGINEER: Huey Hawkins, PE No. 33665 - 6855 SW Elm Church Rd., Fort White, FL 32038 - 386-497-3991

CERTIFICATION: These plans and windload engineering, comply with Florida Building Code Res. 2004 Sec. R301.2.1, to the best of my knowledge.

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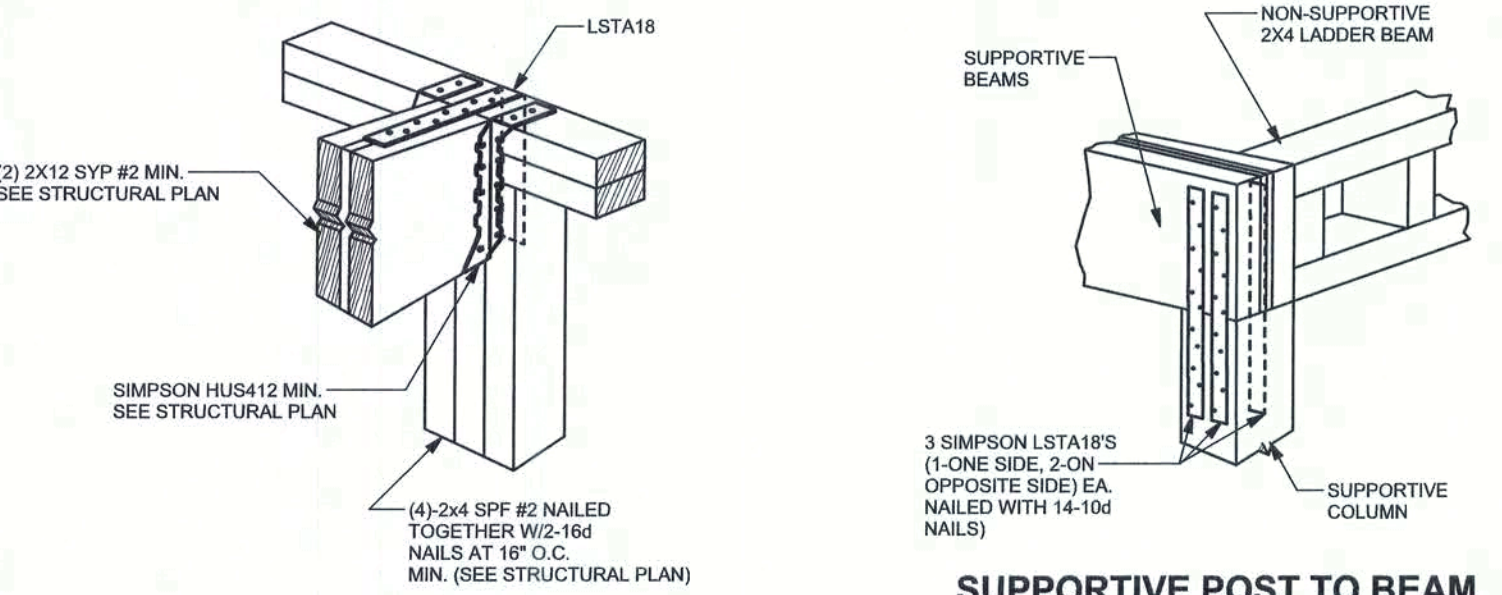


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

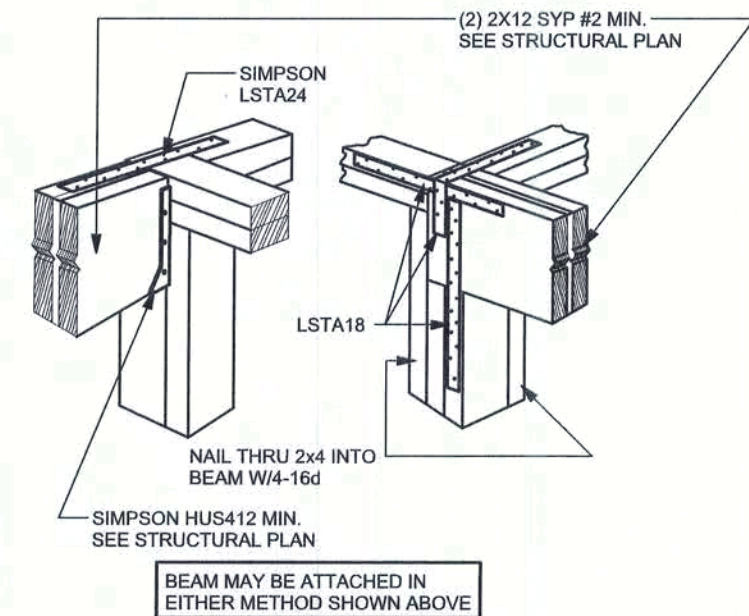
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 18" O.C. x 0.85 = 15.3" O.C.

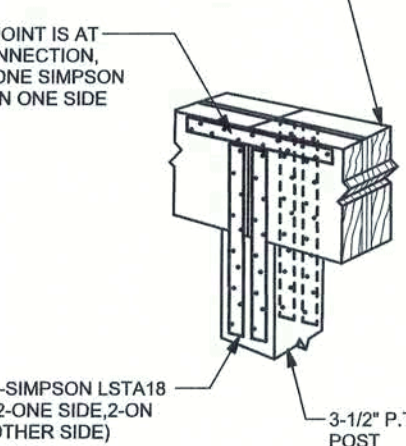


BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.

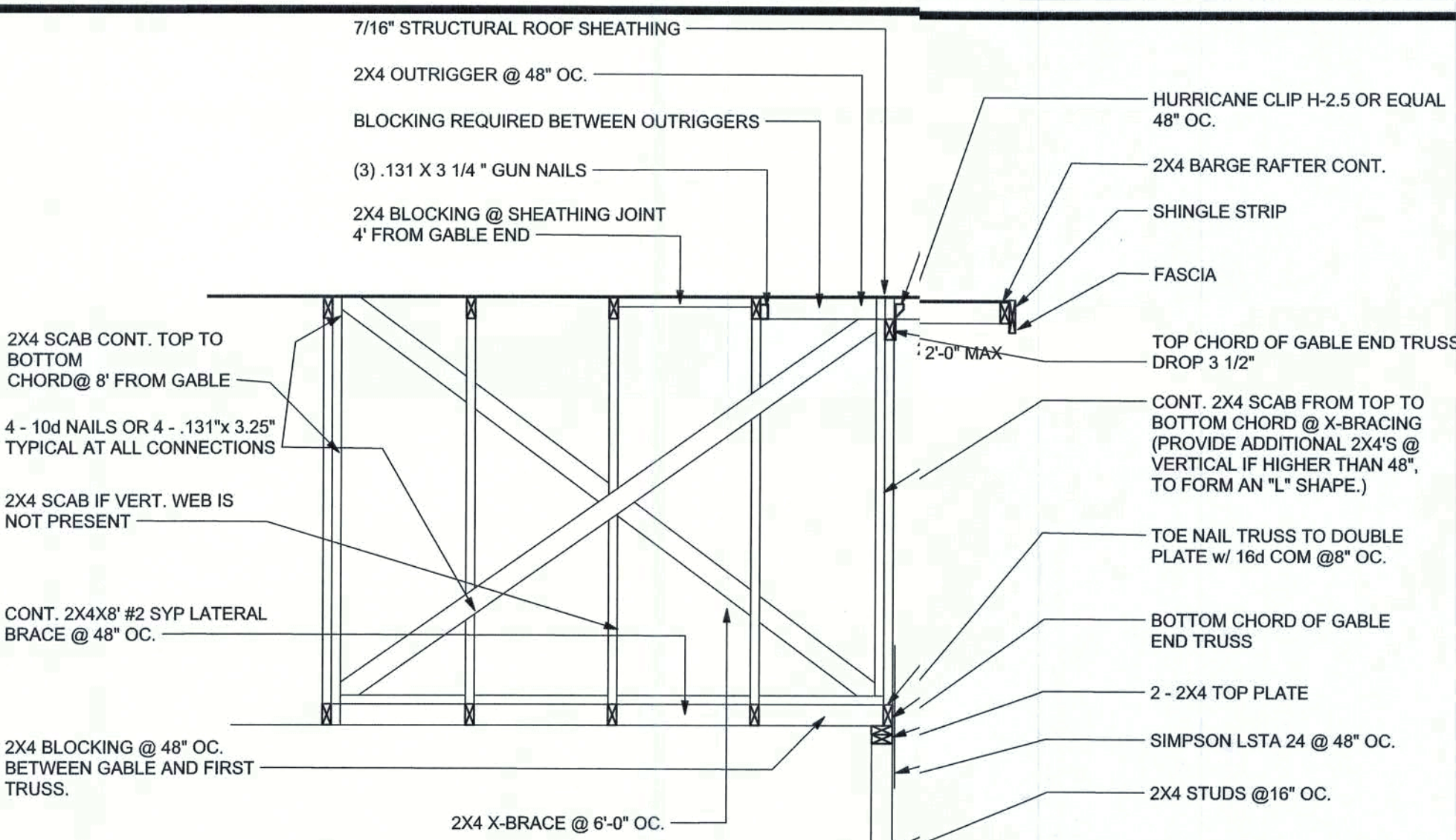


BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

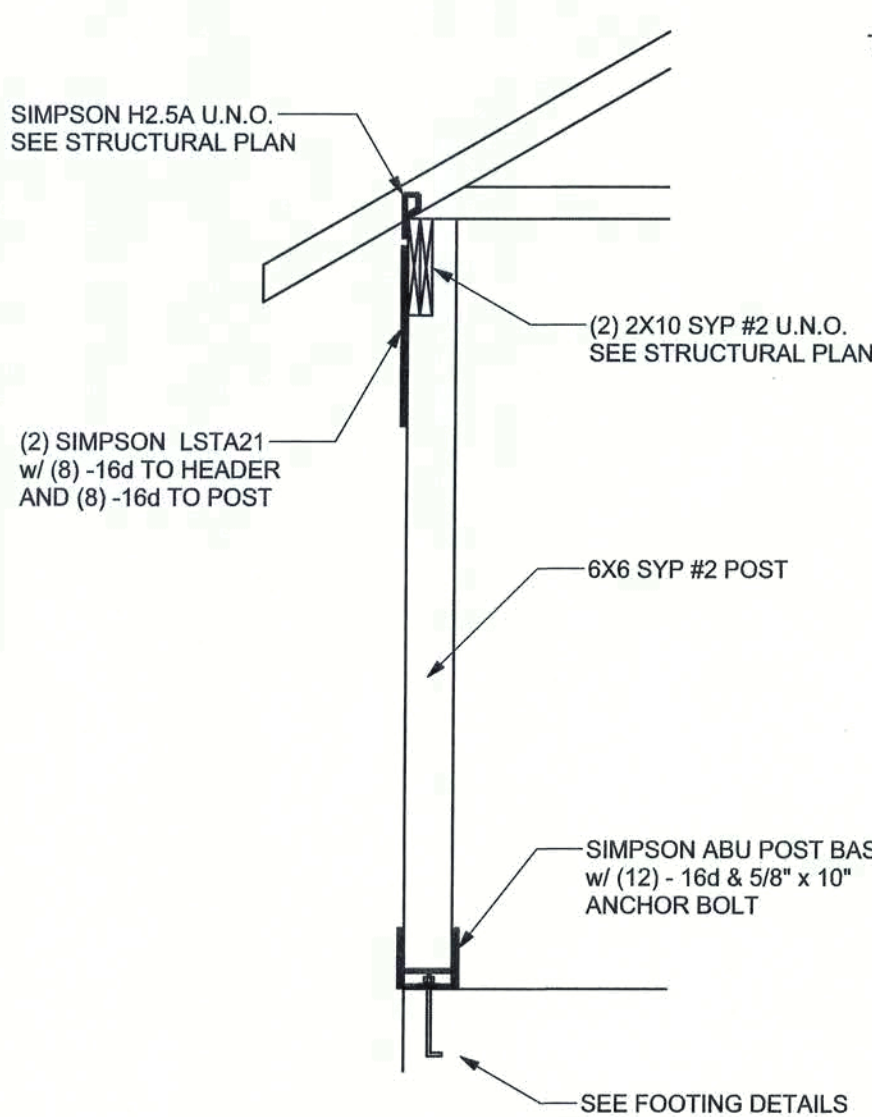
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



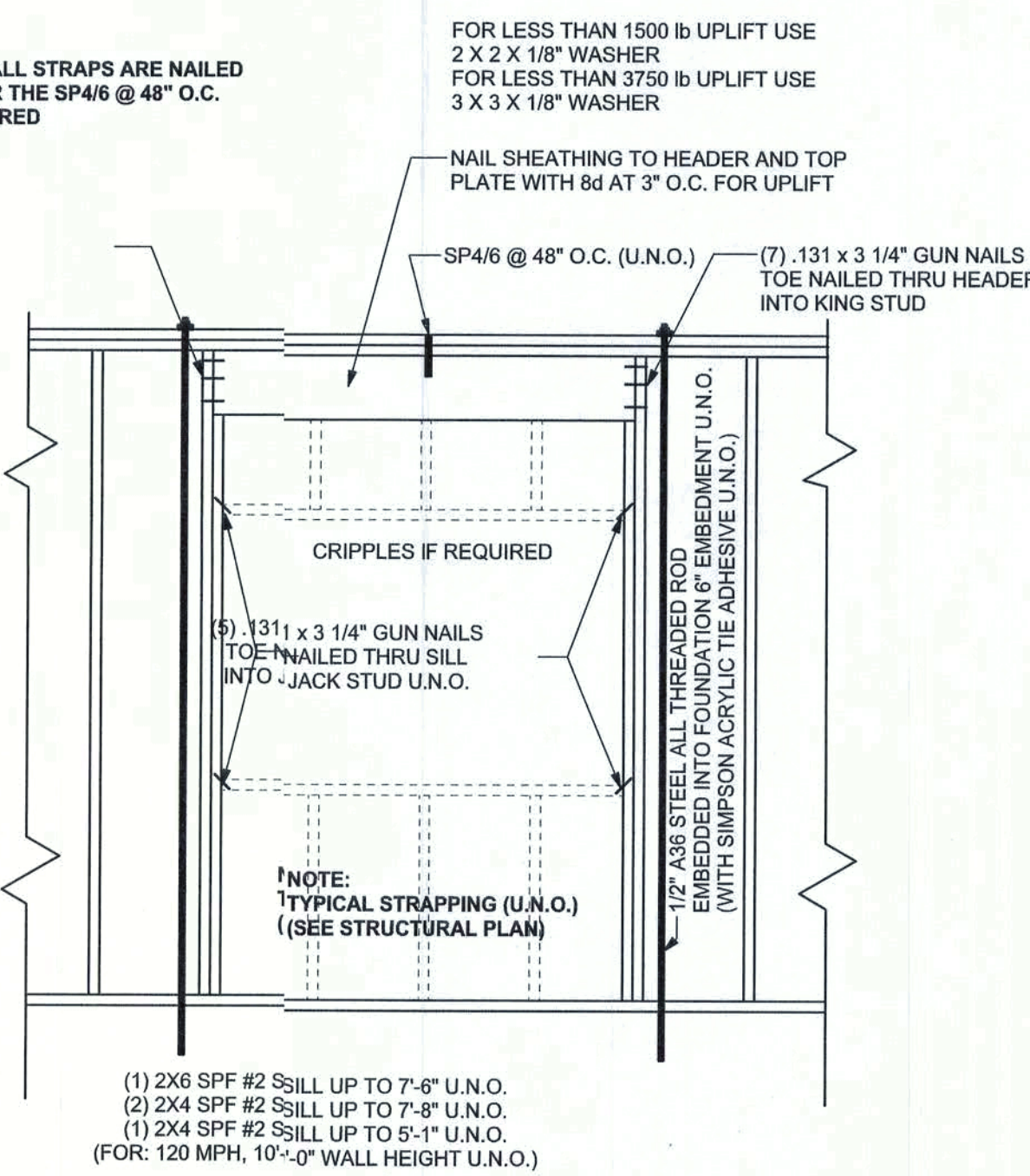
SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.



TYPICAL GABLE END (X-BRACING)
ALL MEMBERS SHALL BE SYP



TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"



TYPICAL 1 STORY HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

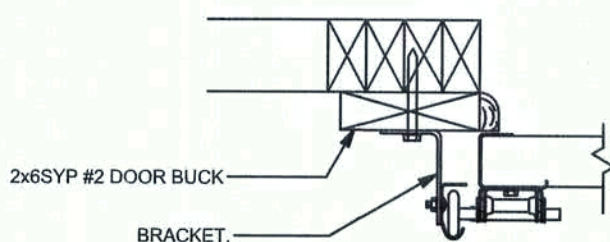
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 650	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 855	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG7		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		TO STUDS
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 865	< 760	SP4			6-10d, 1 1/2"
< 1340	< 1065	SPH4			10-10d, 1 1/2"
< 865	< 760	SP6			6-10d, 1 1/2"
< 1340	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H02A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131 x 3 1/4" ON PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF 131 x 3 1/4" GN
8' - 10'	24" O.C.	8" O.C.	8" O.C.
11' - 15'	16" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSSES-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS RESPONSIBILITY VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. FOR THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2.4kl, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOTTED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 8" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN COMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSSES-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1. BASIC WIND SPEED = 110 MPH
2. WIND EXPOSURE = B
3. WIND IMPORTANCE FACTOR = 1.0
4. BUILDING CATEGORY = II
5. ROOF ANGLE = 10-45 DEGREES
6. MEAN ROOF HEIGHT = <30 FT
7. INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (R2)			
	10	10	100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'ng	-40.6			-40.6
3	19.9	-25.5	18.1	-21.8
3 O'ng	-68.3			-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-28.1	18.5	-22.6
Worst Case (Zone 5, 10 R2)			21.8	-29.1
8x7 Garage Door	19.5			-22.9
16x7 Garage Door	18.5			-21.0

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway,
PE No.53916, POB508, Lake City, FL
32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specific location.

MARK DISCOWAY
P.E. 53916
3/26/06
SEAL

Blake Construction

Noll Residence

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FINALS DATE:
13 / Feb / 06

JOB NUMBER:
59111

DRAWING NUMBER

S-1

OF 3SHEETS