73

# ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UWF487-Z0122153651

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

| | |
|---|---|
| Truss Fabricator: | Anderson Truss Company |
| Job Identification: | 13-166--Erkinger Home Builders Miller Residence -- Lot #29 |
| Truss Count: | 25 |
| Model Code: | Florida Building Code 2010 |
| Truss Criteria: | FBC2010Res/TPI-2007(STD) |
| Engineering Software: | Alpine Software,Version 12.03. |
| Structural Engineer of Record: | The identity of the structural EOR did not exist as of |
| Address: | the seal date per section 61G15-31.003(5a) of the FAC |
| Minimum Design Loads: | Roof - 37.0 PSF @ 1.25 Duration |
| | Floor - N/A |
| | Wind - 130 MPH ASCE 7-10 -Closed |

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-14015EC1-GBLLETIN-PB16010-14030EC1-

| # | Ref | Description | Drawing# | Date |
|---|---|---|---|---|
| 1 | 12788-A1 | 30' Stepdown | 13141005 | 05/21/13 |
| 2 | 12789-A2 | 30' Stepdown | 13141021 | 05/21/13 |
| 3 | 12790--A3 | 36' Special | 13141010 | 05/21/13 |
| 4 | 12791--A4 | 36' Special | 13142001 | 05/22/13 |
| 5 | 12792--A5 | 44' Special | 13141013 | 05/21/13 |
| 6 | 12793-A6 | 44' Special G | 13142002 | 05/22/13 |
| 7 | 12794-A7 | 30' Stepdown | 13141019 | 05/21/13 |
| 8 | 12795-A8 | 38' Stepdown | 13141004 | 05/21/13 |
| 9 | 12796-A9 | 38' Stepdown | 13141016 | 05/21/13 |
| 10 | 12797-A10 | 38' Stepdown | 13142003 | 05/22/13 |
| 11 | 12798-A11 | 38' Stepdown | 13141017 | 05/21/13 |
| 12 | 12799--A12 | 4' Flat | 13141020 | 05/21/13 |
| 13 | 12800-A13 | 20'3"8 Speci | 13141015 | 05/21/13 |
| 14 | 12801-A14 | 14'3"8 Comm | 13141006 | 05/21/13 |
| 15 | 12802-A15 | 4'6" Flat Gi | 13141011 | 05/21/13 |
| 16 | 12803-A16 | 4'6" Flat Gi | 13141014 | 05/21/13 |
| 17 | 12804-A17 | 4'6" Flat Gi | 13141009 | 05/21/13 |
| 18 | 12805--A18 | 44' Gable | 13141007 | 05/21/13 |
| 19 | 12806--A19 | 38' Gable | 13141002 | 05/21/13 |
| 20 | 12807-A20 | 20'4" Common | 13141022 | 05/21/13 |
| 21 | 12808-A21 | 20'4" Common | 13141003 | 05/21/13 |
| 22 | 12809--A22 | 5' Common | 13141018 | 05/21/13 |
| 23 | 12810-PBA1 | 8'0"1 Comm | 13141008 | 05/21/13 |
| 24 | 12811-PBA2 | 8'0"1 Comm | 13141012 | 05/21/13 |
| 25 | 12812-PBA3 | 8'0"1 Gable | 13141001 | 05/21/13 |

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A1 30' Stepdown Hip )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B2 2x4 SP M-30:
Webs 2x4 SP_#3__12A
Filler 2x10 SP_#2_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

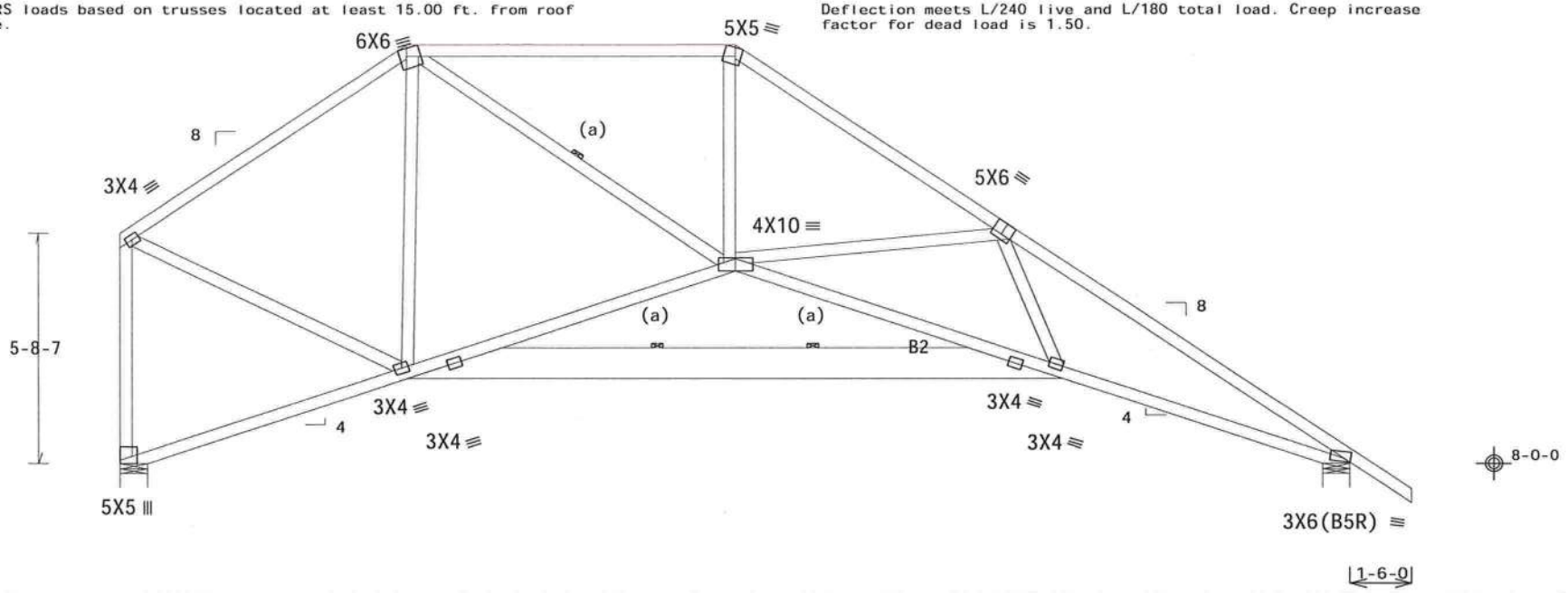
Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.13" due to live load and 0.18" due to dead load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




7-0-0 | 8-0-1 | 15-11-15 | 7-0-0
7-0-0 | 15-0-0 | 15-0-0
15-0-0
30-0-0 Over 2 Supports

R=1162 U=103 W=8" (8" min.)
RL=210/-250

R=1267 U=115 W=8" (8" min.)

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

PLT TYP. Wave | Design Crit: FBC2010Res/TPI-2007(STD) | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY: 4 | FL/-/3/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
**IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web members shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12788 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141005 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296661 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A2 30' Stepdown Hip )

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B2 2x4 SP M-30:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

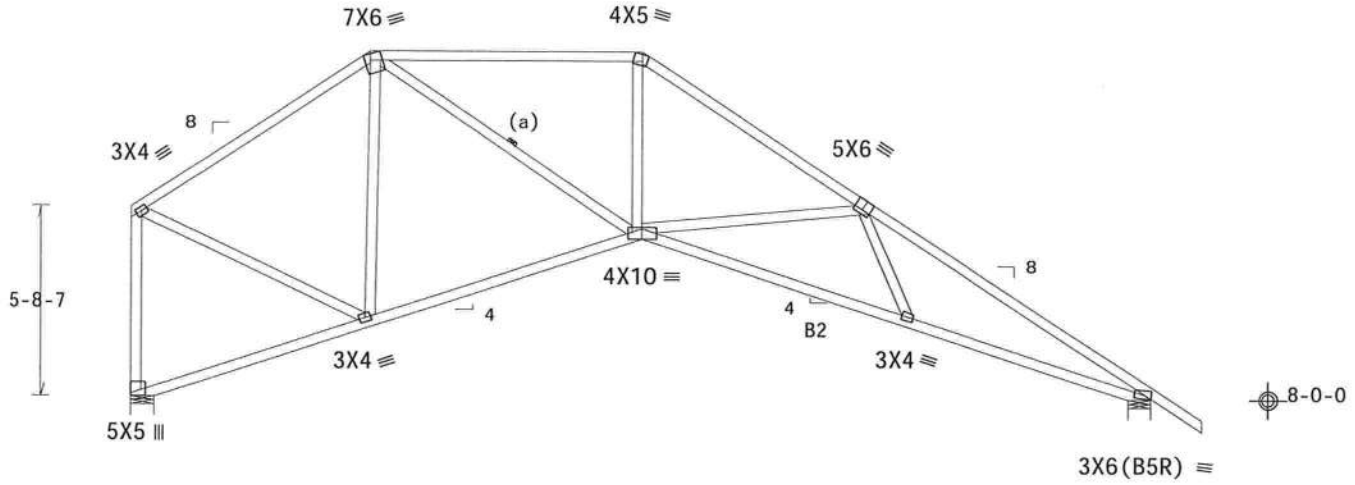
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

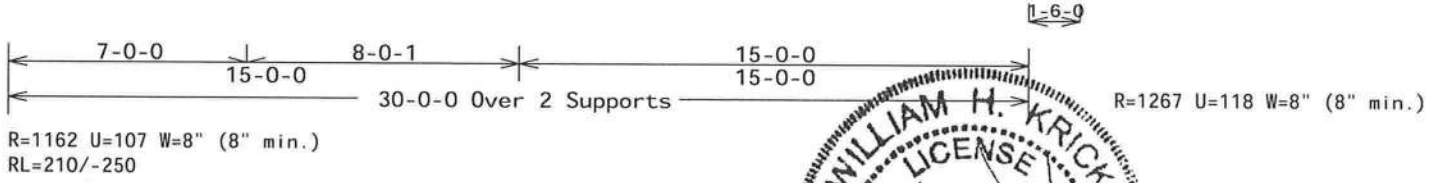
Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.15" due to live load and 0.20" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.




PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0326.14 QTY 4

FL/-/3/-/-/R/-

Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12789 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141021 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296481 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | | JREF- 1UWF487_Z01 |

( 13-166--Erkinger Home Builders  Miller Residence -- Lot #29 - A3 36' Special )

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP 2850f-2.3E :T2, T3 2x4 SP M-30:
Bot chord 2x4 SP 2850f-2.3E :B3 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A :W5, W16 2x4 SP_#1_12A:
:W13 2x4 SP 2850f-2.3E:
Filler 2x10 SP_#2_12A
:Stack Chord SC1 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

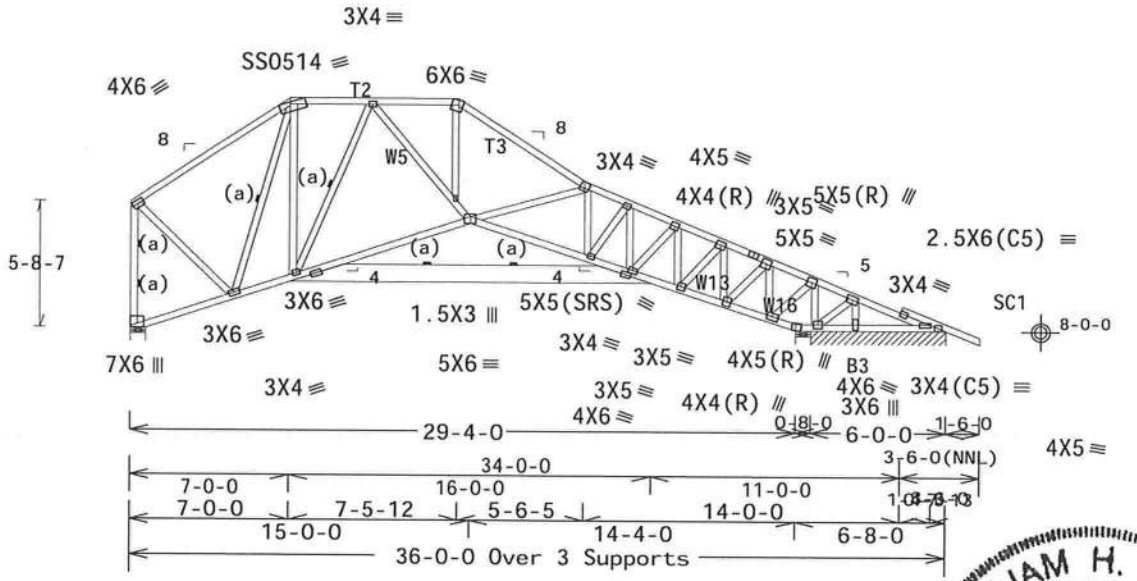
Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Shim all supports to solid bearing.




Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

12.03.04.03.06.14

QTY 1

FL/-/3/-/-/R/-

Scale =.125"/Ft.

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05/22/2013

ALPINE

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12790 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141010 |
| HC-ENG | JB/WPF |
| SEQN- | 296993 |
| JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A4 36' Special)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B3 2x6 SP_#2_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

| | | | | | |
|---|---|---|---|---|---|
| TC- From | 57 plf at | 0.00 to | 57 plf at | 7.00 |
| TC- From | 55 plf at | 7.00 to | 55 plf at | 15.00 |
| TC- From | 57 plf at | 15.00 to | 57 plf at | 20.14 |
| TC- From | 55 plf at | 20.14 to | 55 plf at | 37.50 |
| BC- From | 20 plf at | 0.00 to | 20 plf at | 6.00 |
| BC- From | 40 plf at | 6.00 to | 40 plf at | 16.00 |
| BC- From | 20 plf at | 16.00 to | 20 plf at | 36.00 |
| BC- From | 4 plf at | 36.00 to | 4 plf at | 37.50 |

Wind loads and reactions based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Negative reaction(s) of -277# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

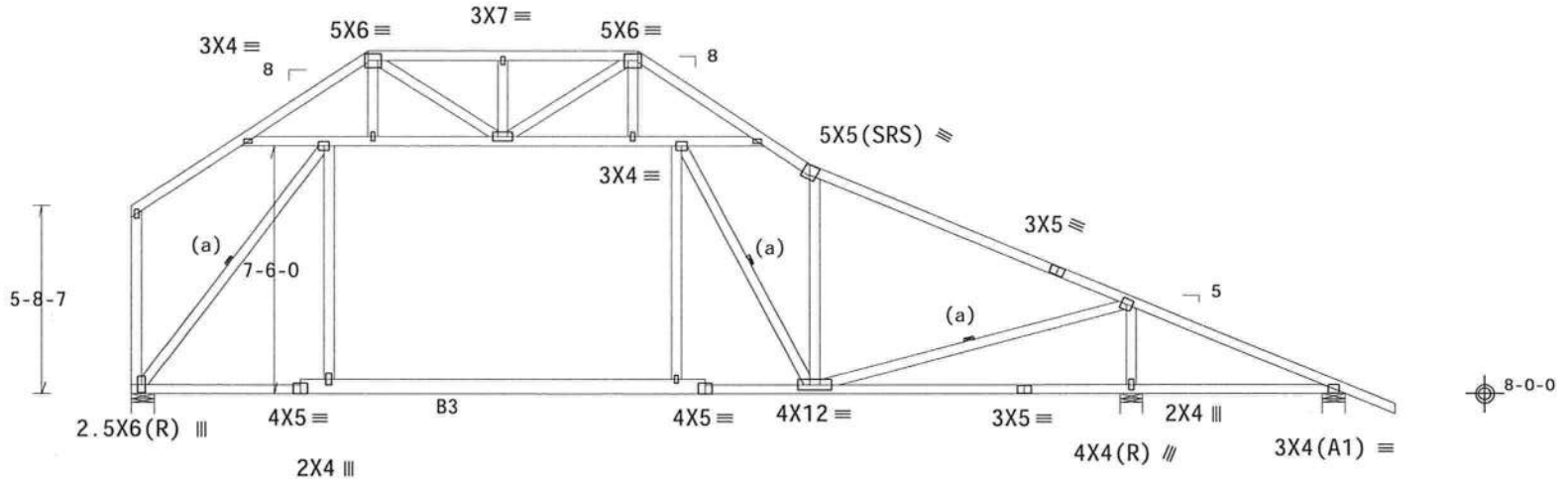
(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.




R=1157 U=117 W=8" (8" min.)
RL=173/-247

R=1961 U=285 W=8" (8" min.)
R=138/-277 U=182 W=8" (8" min.)

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY: 1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 05/22/2013

ALPINE
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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12791 |
| TC DL | 7.0 PSF | DATE | 05/22/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13142001 |
| BC LL | 0.0 PSF | HC-ENG | WHK/WHK |
| TOT.LD. | 37.0 PSF | SEQN- | 297003 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A5 44' Special )

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B2 2x4 SP 2850f-2.3E:
:B3 2x6 SP_#2_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

| | | | | | |
|---|---|---|---|---|---|
| TC - From | 62 PLF at | -1.50 to | 62 PLF at | 0.00 |
| TC - From | 57 PLF at | 0.00 to | 57 PLF at | 15.00 |
| TC - From | 55 PLF at | 15.00 to | 55 PLF at | 23.00 |
| TC - From | 57 PLF at | 23.00 to | 57 PLF at | 28.14 |
| TC - From | 55 PLF at | 28.14 to | 55 PLF at | 44.00 |
| TC - From | 59 PLF at | 44.00 to | 59 PLF at | 45.50 |
| BC - From | 20 PLF at | 0.00 to | 20 PLF at | 5.67 |
| BC - From | 60 PLF at | 5.67 to | 60 PLF at | 8.17 |
| BC - From | 20 PLF at | 8.17 to | 20 PLF at | 14.17 |
| BC - From | 60 PLF at | 14.17 to | 60 PLF at | 24.00 |
| BC - From | 20 PLF at | 24.00 to | 20 PLF at | 44.00 |

Negative reaction(s) of -294# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

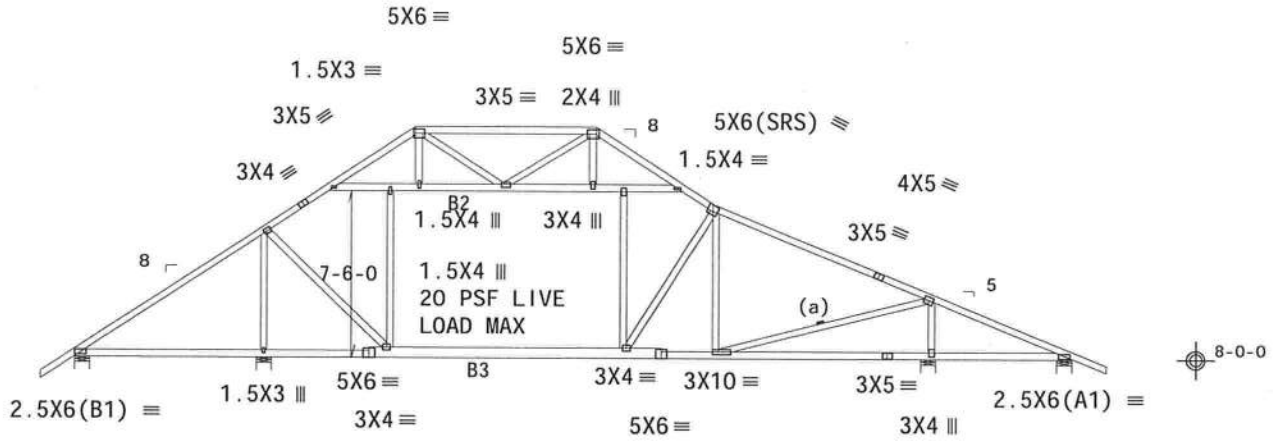
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

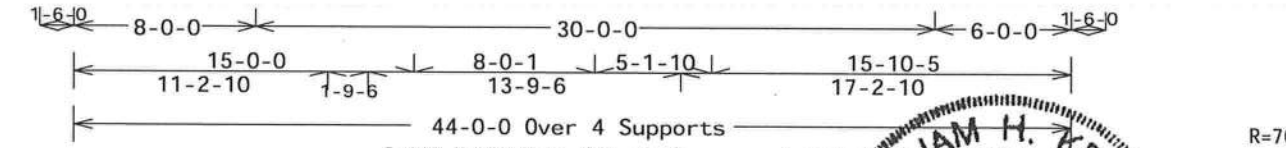
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.




R=1062 U=226 W=8" (8" min.)
RL=307/-346

R=893 U=256 W=8" (8" min.)

R=2165 U=434 W=8" (8" min.)

R=70/-295 U=148 W=8" (8" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12.03.04.0316.14

QTY 24

FL/-/3/-/-/R/-

Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12792 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141013 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296675 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

(13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A6 44' Special Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B1 2x4 SP 2850f-2.3E:
:B3, B4 2x6 SP_#2_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

| | | | | | |
|---|---|---|---|---|---|
| TC - From | 100 PLF at | -1.50 to | 100 PLF at | 0.00 |
| TC - From | 92 PLF at | 0.00 to | 92 PLF at | 15.00 |
| TC - From | 90 PLF at | 15.00 to | 90 PLF at | 23.00 |
| TC - From | 92 PLF at | 23.00 to | 92 PLF at | 28.14 |
| TC - From | 90 PLF at | 28.14 to | 90 PLF at | 44.00 |
| TC - From | 97 PLF at | 44.00 to | 97 PLF at | 45.50 |
| BC - From | 33 PLF at | 0.00 to | 33 PLF at | 5.67 |
| BC - From | 98 PLF at | 5.67 to | 98 PLF at | 8.17 |
| BC - From | 33 PLF at | 8.17 to | 33 PLF at | 14.17 |
| BC - From | 98 PLF at | 14.17 to | 98 PLF at | 24.00 |
| BC - From | 33 PLF at | 24.00 to | 33 PLF at | 44.00 |
| BC - | 272 LB Conc. Load at | 4.06 | | |
| BC - | 652 LB Conc. Load at | 23.65 | | |

## 2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -845# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

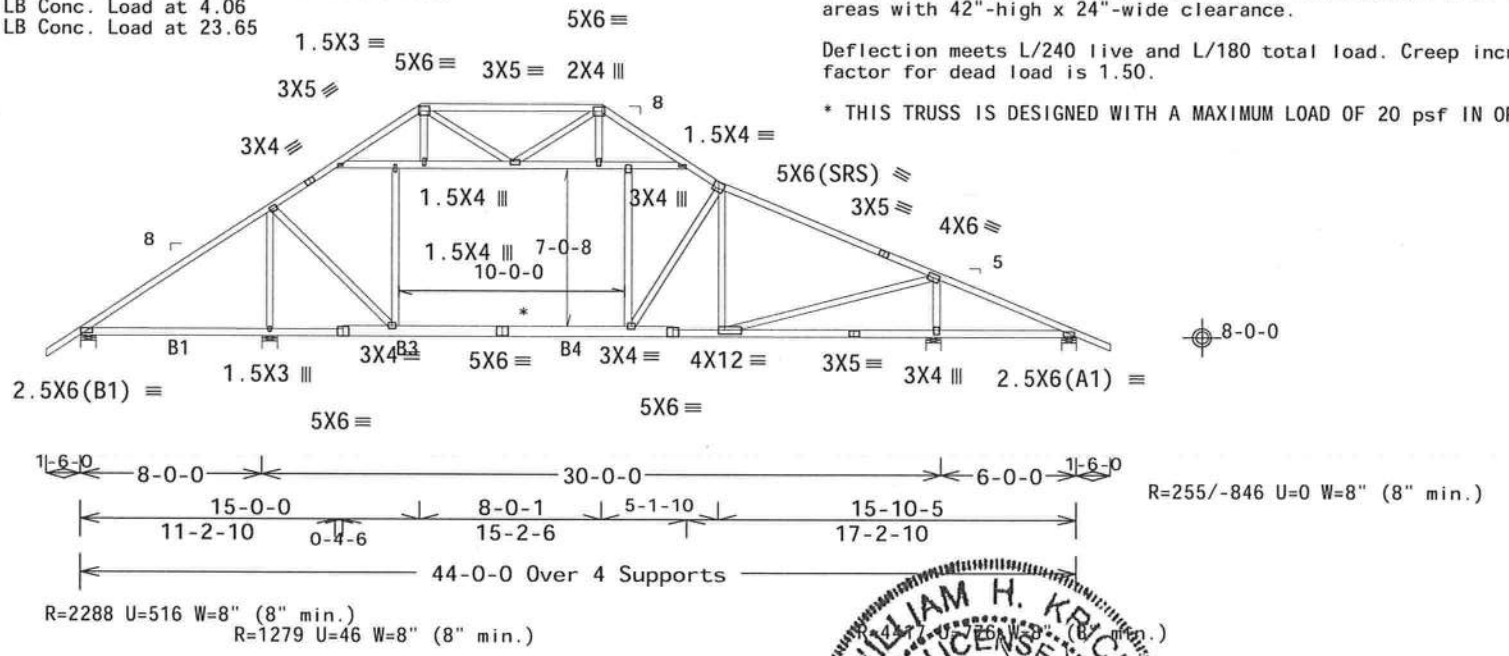
Wind loads and reactions based on MWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

* THIS TRUSS IS DESIGNED WITH A MAXIMUM LOAD OF 20 psf IN OPENING.




Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326 14 QTY:2 FL/-/3/-/-/R/- Scale =.125"/Ft.

WILLIAM H. KRUCK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12793 |
| TC DL | 7.0 PSF | DATE | 05/22/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13142002 |
| BC LL | 0.0 PSF | | HC-ENG WHK/WHK |
| TOT.LD. | 37.0 PSF | SEQN- | 297008 |
| DUR.FAC. | 1.25 | | |
| SPACING | SEE ABOVE | | JREF- 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A7 30' Stepdown Hip )

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

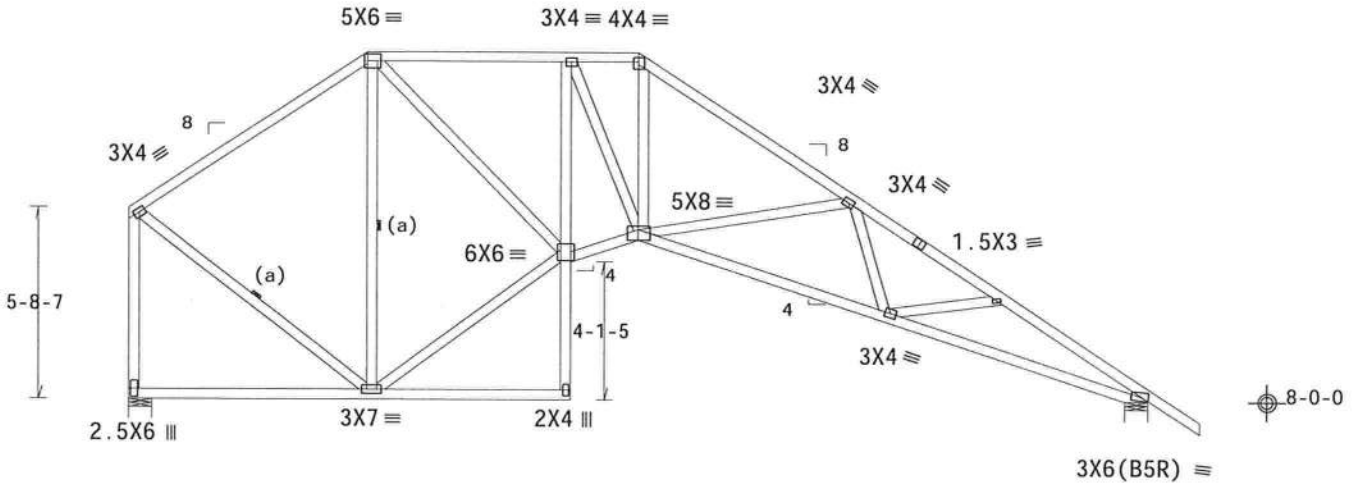
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

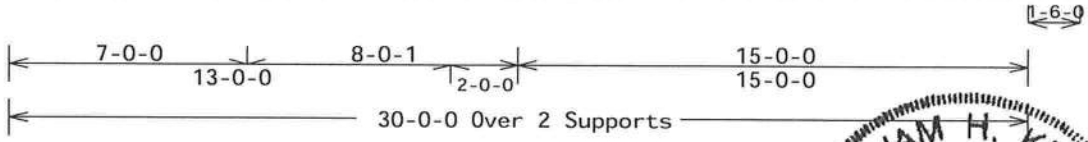
Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.16" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.




R=1264 U=118 W=8" (8" min.)

R=1151 U=110 W=8" (8" min.)
RL=237/-277

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

12.03.04.0326.14 QTY 1

FL/-/3/-/-/R/-

Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12794 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141019 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296397 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A8 38' Stepdown Hip )

Top chord 2x4 SP_#1_12A :T1 2x4 SP 2850f-2.3E:
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W1, W2, W8 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.12" due to live load and 0.16" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

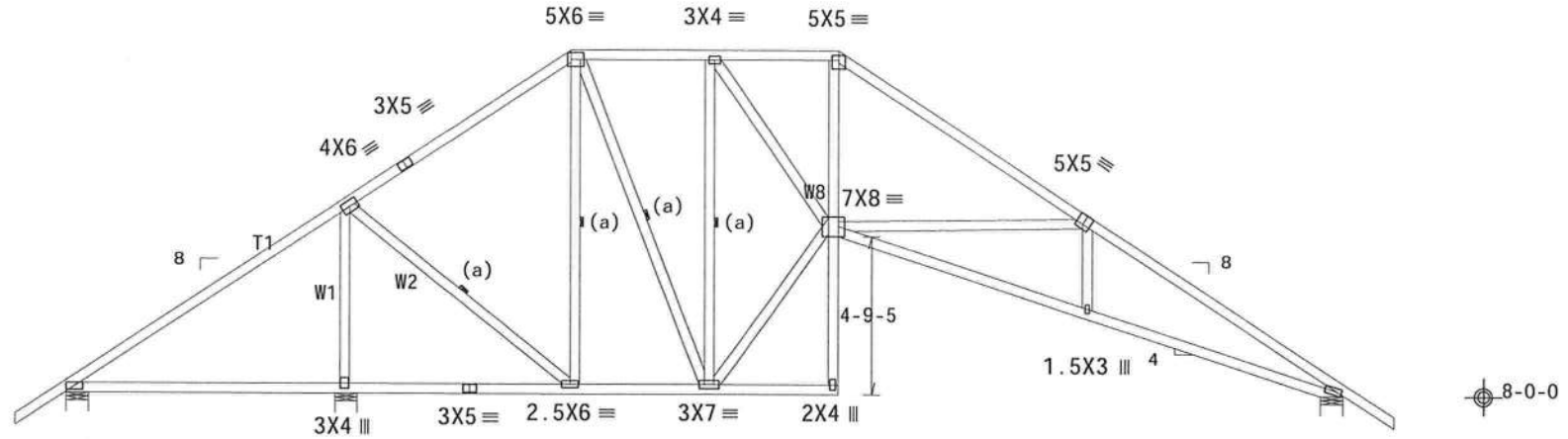
*** Negative reaction(s) of -571# MAX. (See below) from a non-wind load case requires uplift connection.

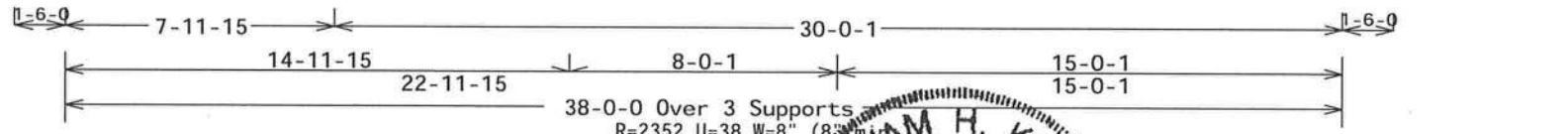
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




*** R=124/-571 U=185 W=7.936" (7.936" min.)
RL=378/-378

R=2352 U=38 W=8" (8" min.)

R=1057 U=109 W=8" (8" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12.03.04.0826.14

QTY: 1

FL/-/3/-/-/R/-

Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12795 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141004 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296477 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | | JREF- 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A9 38' Stepdown Hip )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B3 2x6 SP_#2_12A:
Webs 2x4 SP_#3__12A :W9 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

| | | | | | | |
|---|---|---|---|---|---|---|
| TC - From | 62 PLF | at | -1.50 | to | 62 PLF | at 0.00 |
| TC - From | 57 PLF | at | 0.00 | to | 57 PLF | at 38.00 |
| TC - From | 62 PLF | at | 38.00 | to | 62 PLF | at 39.50 |
| BC - From | 20 PLF | at | 0.00 | to | 20 PLF | at 5.67 |
| BC - From | 60 PLF | at | 5.67 | to | 60 PLF | at 8.17 |
| BC - From | 20 PLF | at | 8.17 | to | 20 PLF | at 14.17 |
| BC - From | 60 PLF | at | 14.17 | to | 60 PLF | at 23.92 |
| BC - From | 20 PLF | at | 23.92 | to | 20 PLF | at 29.92 |
| BC - From | 60 PLF | at | 29.92 | to | 60 PLF | at 32.42 |
| BC - From | 20 PLF | at | 32.42 | to | 20 PLF | at 38.00 |

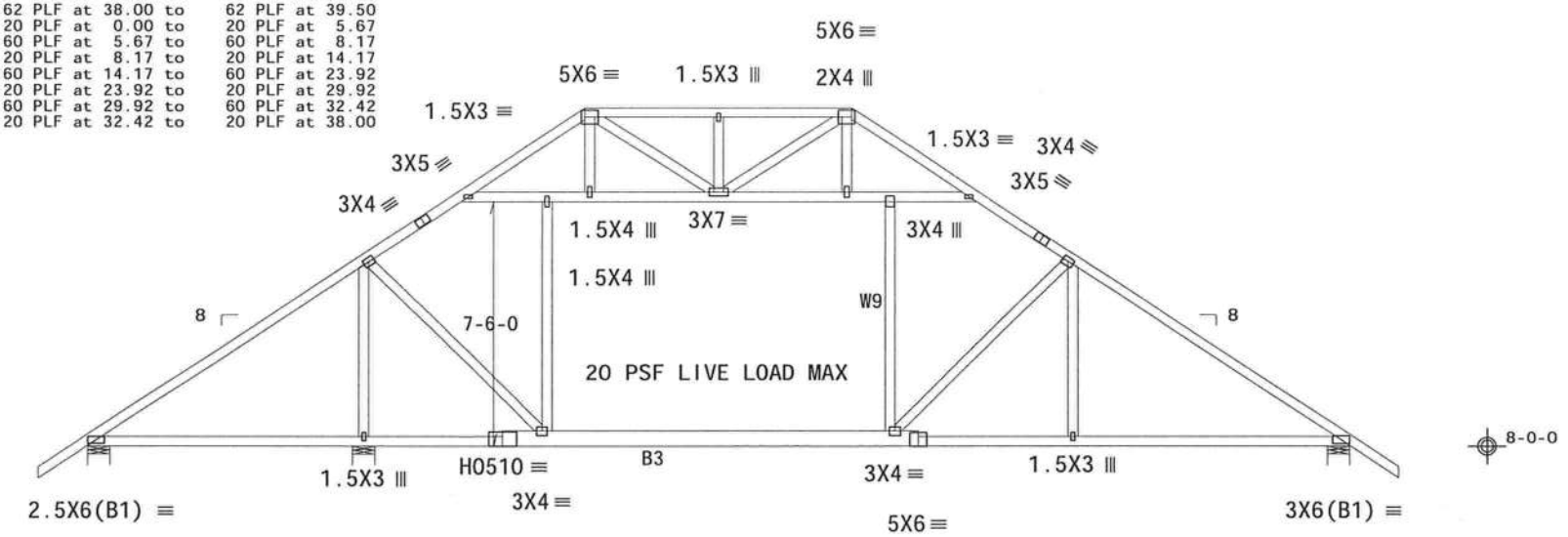
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

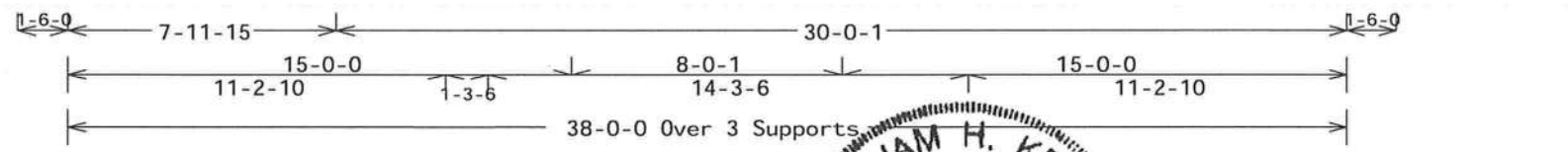
Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.




R=1406 U=176 W=7.936" (7.936" min.)
RL=346/-346

R=562 Rw=659 U=377 W=8" (8" min.)

R=1726 U=147 W=8" (8" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY: 3 FL/-/3/-/-/R/- Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12796 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141016 |
| HC-ENG | JB/WPF |
| SEQN- | 296616 |
| | |
| JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A10 38' Stepdown Hip Girder)

Top chord 2x4 SP_#1_12A :T5 2x4 SP M-30:
Bot chord 2x4 SP 2850f-2.3E :B3, B4 2x6 SP_#2_12A:
:B5 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 100 PLF at -1.50 to 100 PLF at 0.00
TC - From 92 PLF at 0.00 to 92 PLF at 38.00
TC - From 100 PLF at 38.00 to 100 PLF at 39.50
BC - From 33 PLF at 0.00 to 33 PLF at 5.67
BC - From 98 PLF at 5.67 to 98 PLF at 8.17
BC - From 33 PLF at 8.17 to 33 PLF at 14.17
BC - From 98 PLF at 14.17 to 98 PLF at 24.00
BC - From 33 PLF at 24.00 to 33 PLF at 38.00
BC - 272 LB Conc. Load at 4.06
BC - 681 LB Conc. Load at 23.65

# 2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

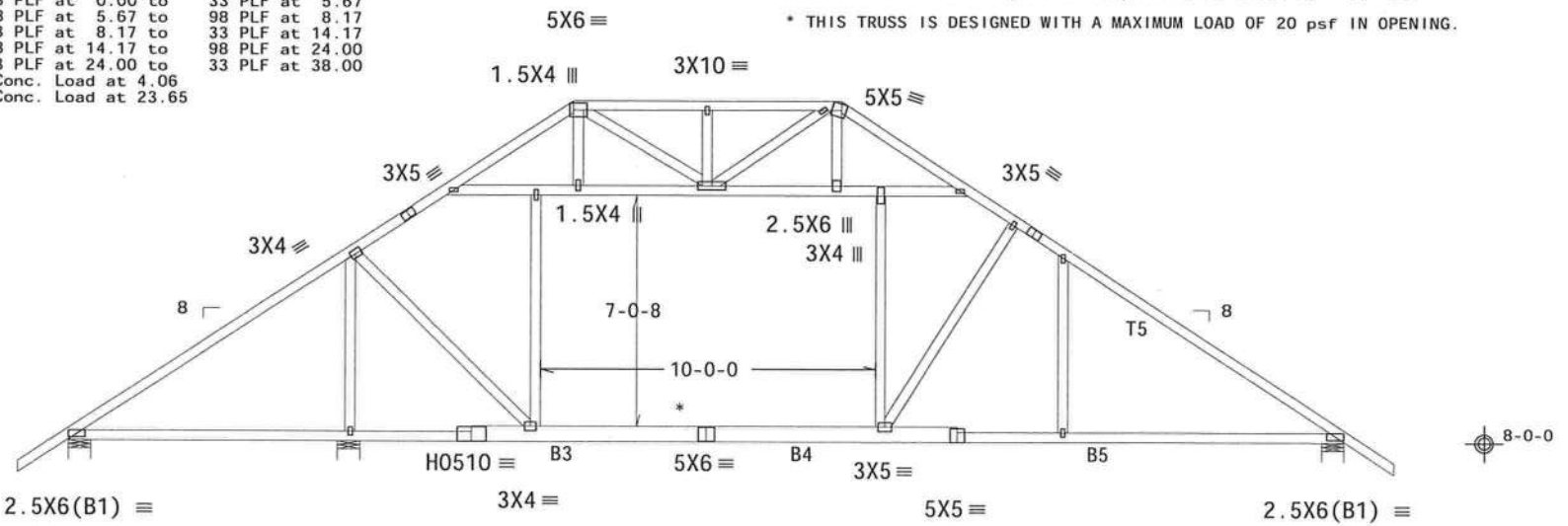
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

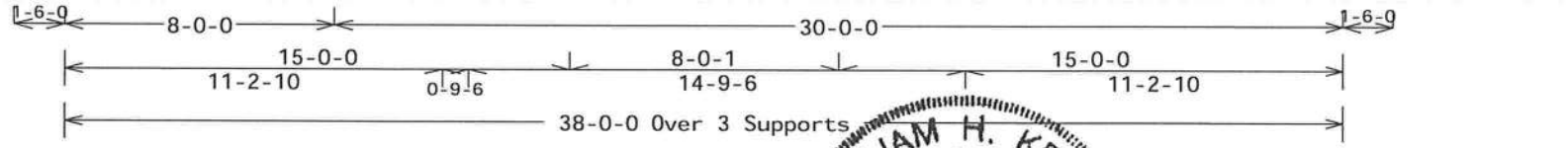
Wind loads and reactions based on MWFRS.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

* THIS TRUSS IS DESIGNED WITH A MAXIMUM LOAD OF 20 psf IN OPENING.




R=2937 U=551 W=8" (8" min.)
RL=346/-346
R=920 U=319 W=8" (8" min.)
R=3175 U=512 W=8" (8" min.)

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY: 2 FL/-/3/-/-/R/- Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12797 |
| TC DL | 7.0 PSF | DATE | 05/22/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13142003 |
| BC LL | 0.0 PSF | | HC-ENG WHK/WHK |
| TOT.LD. | 37.0 PSF | SEQN- | 297013 |
| DUR.FAC. | 1.25 | | |
| SPACING | SEE ABOVE | | JREF- 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A11 38' Stepdown Hip )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

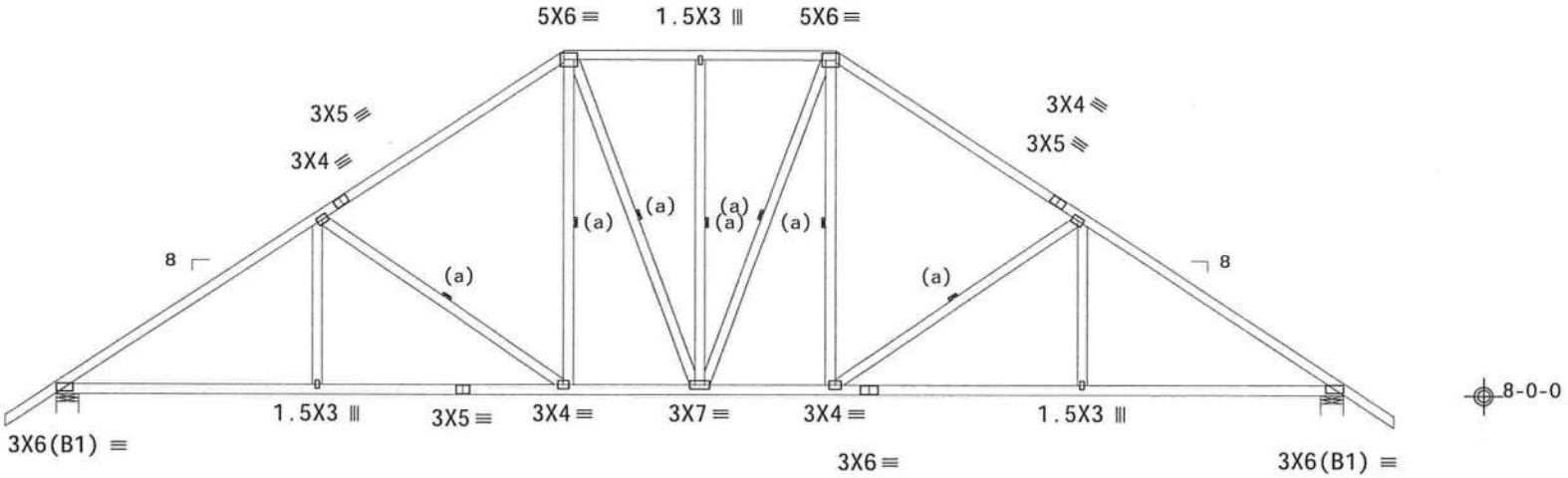
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

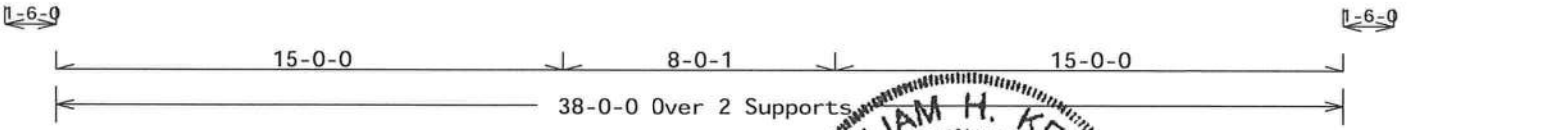
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.




R=1635 U=356 W=8" (8" min.)
RL=346/-346

R=1636 U=356 W=8" (8" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY 24 | FL/-/3/-/-/R/- | Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12798 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141017 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294747 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | | JREF- 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders  Miller Residence -- Lot #29 - A12 4' Flat )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(H1) = (J) Hanger not calculated (1)2x4 SP_#1_12A supporting member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

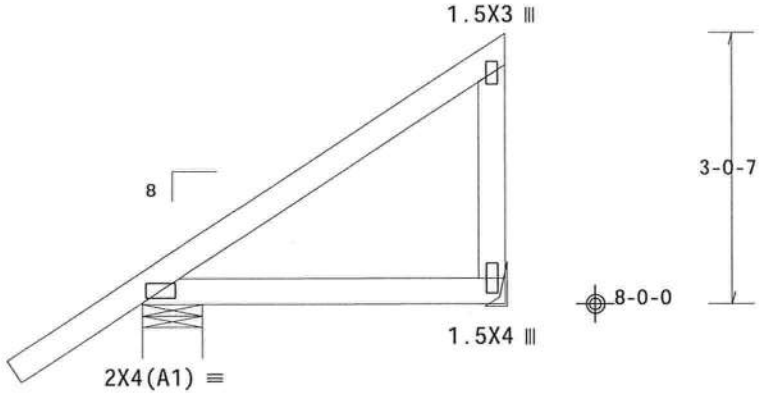
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.




R=128 U=50
H=H1

1-6-0

4-0-0 Over 2 Supports

R=266 U=46 W=8" (8" min.)
RL=104/-57

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave  FT/RT=10%(0%)/0(0)  12.03.04.0326.14  QTY: 4  FL/-/3/-/-/R/-  Scale =.5"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12799 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141020 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294763 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A13 20'3"8 Special )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

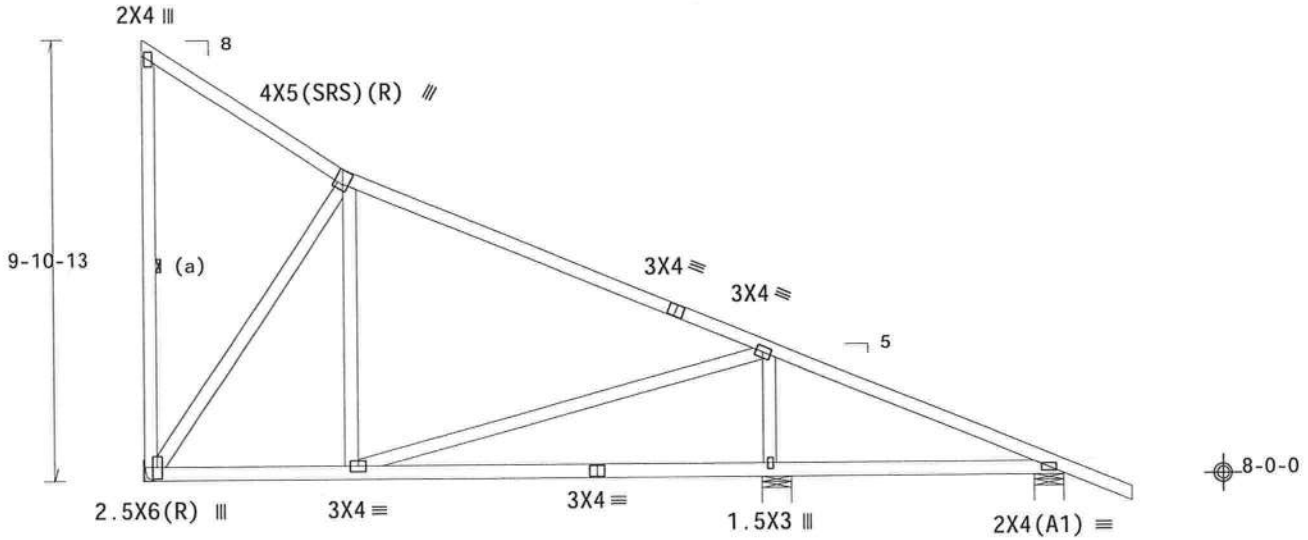
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

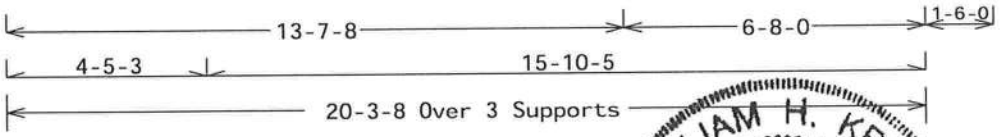
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(H1) = (J) Hanger not calculated (1)2x4 SP_#1_12A supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.




R=508 U=200
RL=65/-290
H=H1

R=341 U=64 W=8" (8" min.)

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

12.03.04.0326.14 QTY 2

FL/-/3/-/-/R/-

Scale =.25"/Ft.


ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12800 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141015 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294760 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A14 14'3"8 Common )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(H1) = (J) Hanger not calculated (1)2x4 SP 2850f-2.3E supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

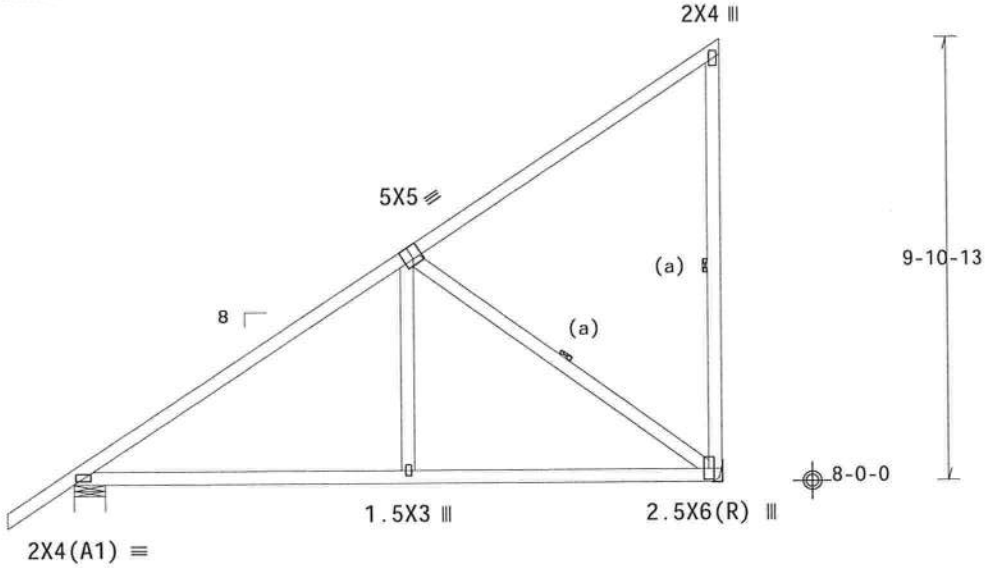
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




R=537 U=112
H=H1

1-6-0

14-3-8 Over 2 Supports

R=653 U=8 W=8" (8" min.)
RL=200/-117

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

12.03.04.0386.14 QTY 2

FL/-/3/-/-/R/-

Scale =.25"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12801 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141006 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294728 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A15 4'6" Flat Girder )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 54 plf at 0.00 to 54 plf at 4.50
BC- From 10 plf at 0.00 to 10 plf at 4.50
BC- 507.98 lb Conc. Load at 1.81, 2.69

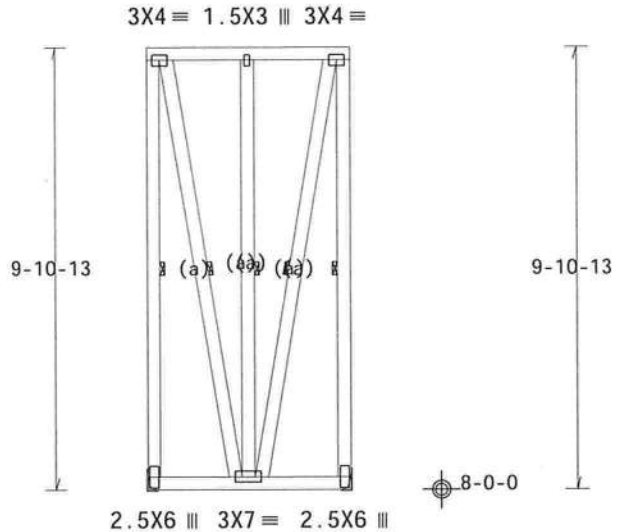
130 mph wind, 17.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.




4-6-0 Over 2 Supports

R=652 U=261
R=652 U=261

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

12.03.04.0126.14 QTY: 1

FL/-/3/-/-/R/-

Scale =.25"/Ft.

WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12802 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141011 |
| HC-ENG | JB/WPF |
| SEQN- | 294770 |
| | |
| JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders  Miller Residence -- Lot #29 - A16 4'6" Flat Girder )

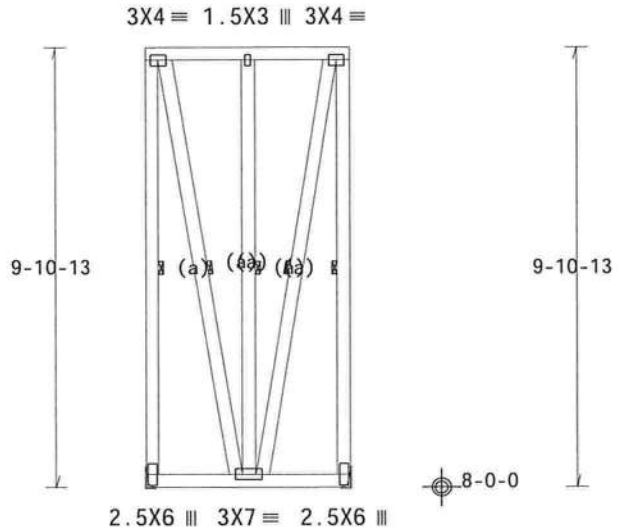
Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.




Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 54 plf at 0.00 to 54 plf at 4.50
BC- From 10 plf at 0.00 to 10 plf at 4.50
BC- 537.47 lb Conc. Load at 1.81, 2.69

130 mph wind, 17.90 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

(H1) = (J) Hanger not calculated (2)2x4 SP_#1_12A supporting member.
(H2) = (J) Hanger not calculated (2)2x4 SP_#1_12A supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

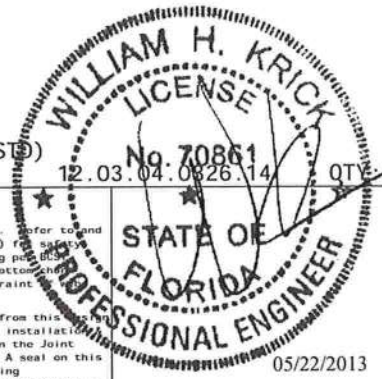
Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

4-6-0 Over 2 Supports

R=681 U=174
H=H2

R=681 U=174
H=H1




Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0826.14 QTY: 1 FL/-/3/-/-/R/-

Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12803 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141014 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294740 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A17 4'6" Flat Girder )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

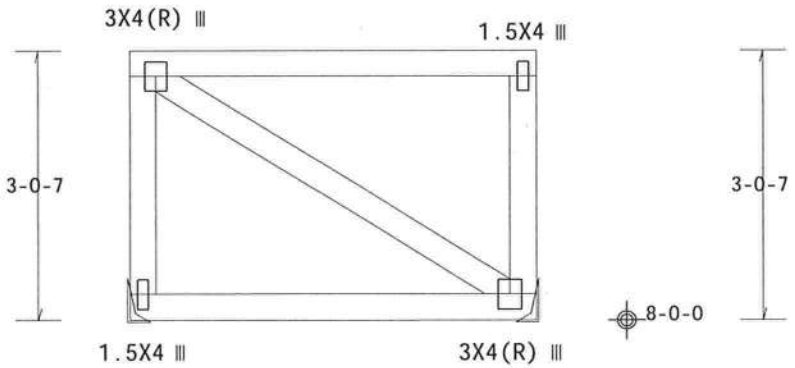
The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 54 plf at 0.00 to 54 plf at 4.50
BC- From 10 plf at 0.00 to 10 plf at 4.50
BC- 128.43 lb Conc. Load at 1.81, 2.69

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.




4-6-0 Over 2 Supports

R=272 U=102 R=272 U=102

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

PLT TYP. Wave | Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) | 12.03.04.0386.14 | QTY 2 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12804 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141009 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294766 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A18 44' Gable )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x6 SP_#2_12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

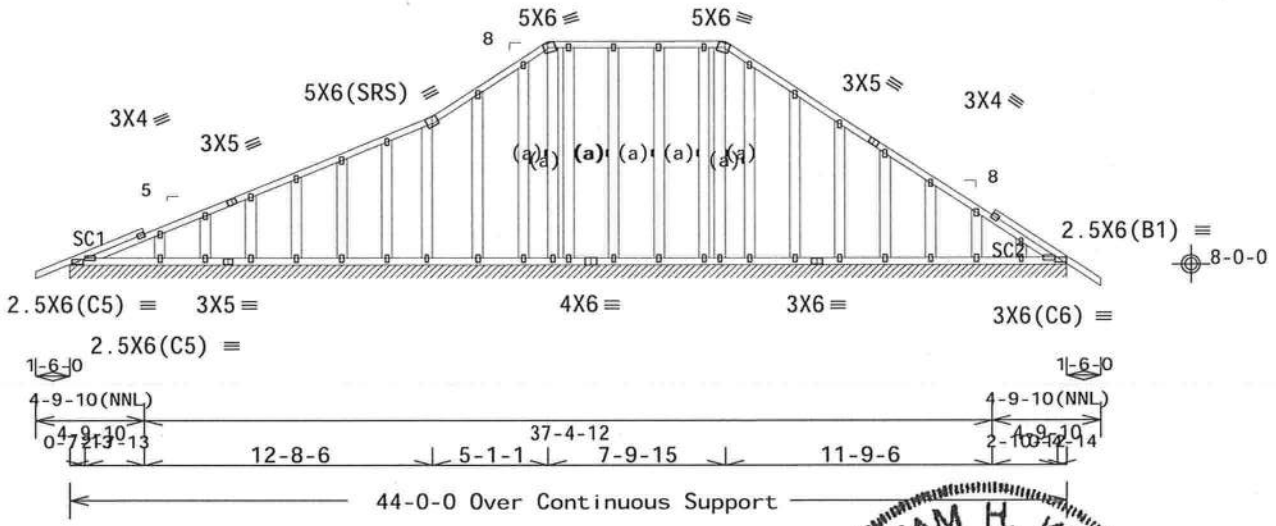
Wind loads and reactions based on MWFRS with additional C&C member design.

Truss spaced at 24.0" OC designed to support 3-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.




R=282 PLF U=79 PLF W=44-0-0
RL=28/-25 PLF

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY: 1 FL/-/3/-/-/R/- Scale =.125"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12805 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141007 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294842 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A19 38' Gable )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x6 SP_#2_12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

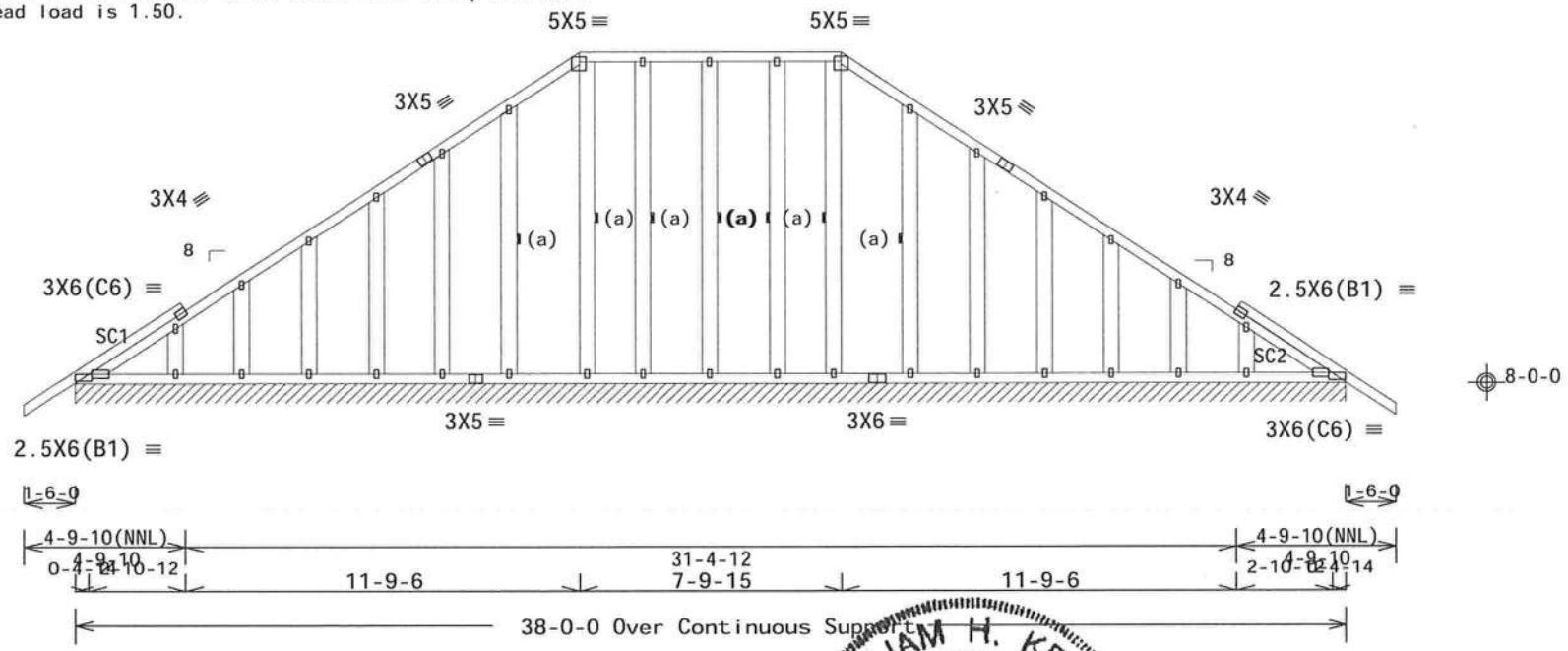
Wind loads and reactions based on MWFRS with additional C&C member design.

Truss spaced at 24.0" OC designed to support 3-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.




R=292 PLF U=79 PLF W=38-0-0
RL=32/-32 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12806 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141002 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294752 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A20 20'4" Common )

Top chord 2x4 SP_#1_12A
Bot chord 2x10 SP_#2_12A
Webs 2x10 SP_#2_12A :W2 2x4 SP_#3__12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

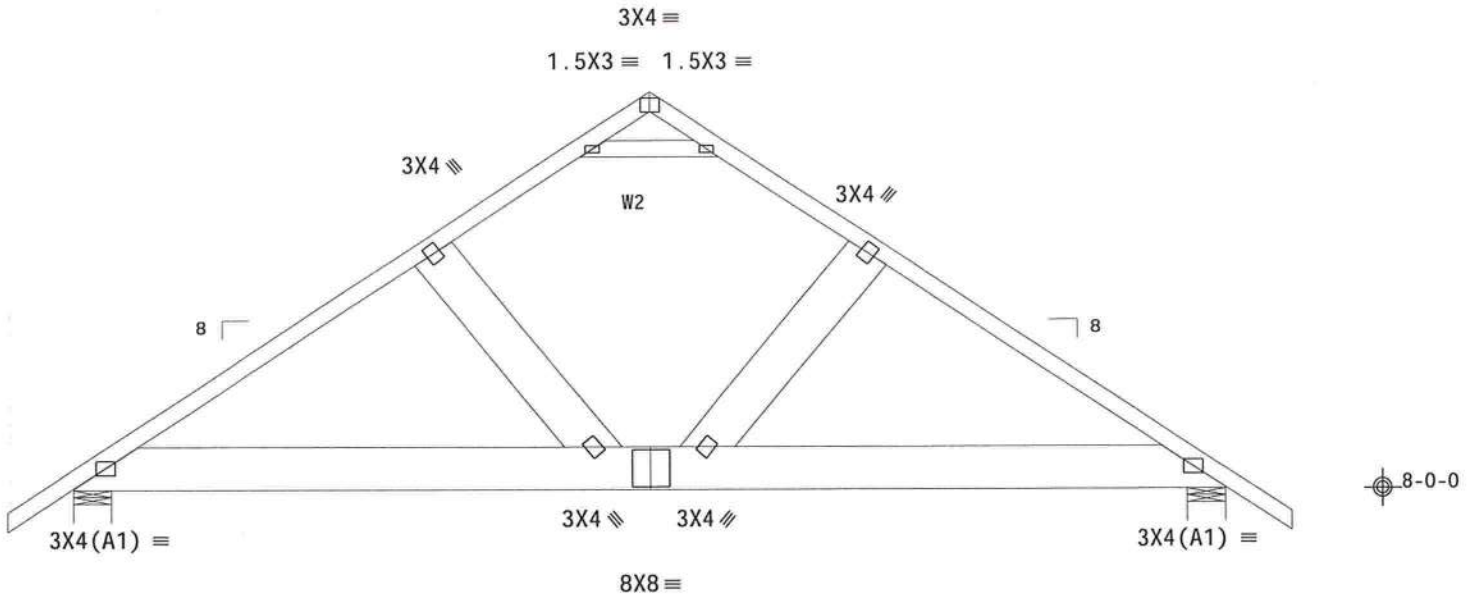
Calculated horizontal deflection is 0.07" due to live load and 0.20" due to dead load.

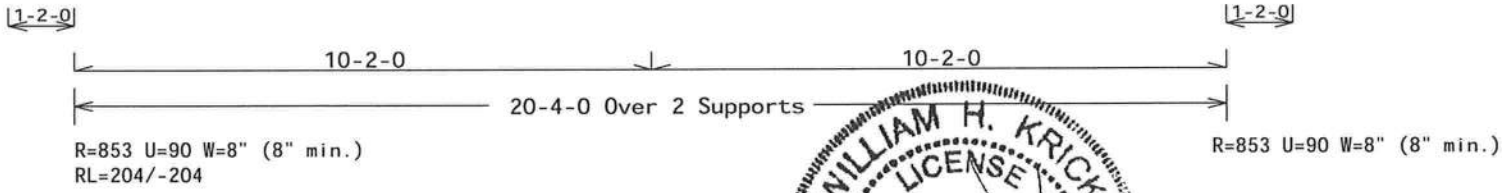
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.00

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12.03.04.0826.14 QTY: 4

FL/-/3/-/-/R/-

Scale =.3125"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12807 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141022 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 296785 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A21 20'4" Common )

Top chord 2x4 SP_#1_12A
Bot chord 2x10 SP_#2_12A
Webs 2x10 SP_#2_12A :W2 2x4 SP_#3__12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

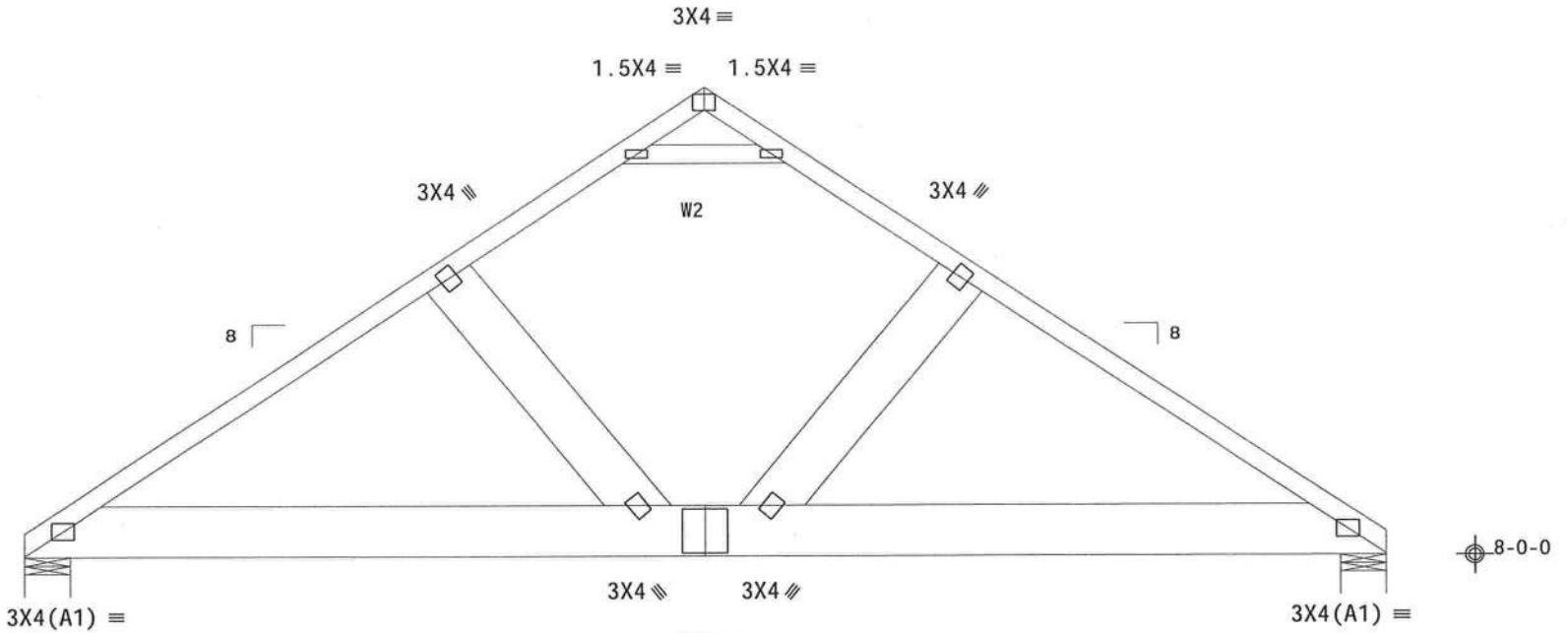
Calculated horizontal deflection is 0.12" due to live load and 0.16" due to dead load.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN_CLEAR bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.00

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




10-2-0 | 10-2-0

20-4-0 Over 2 Supports

R=781 U=70 W=8" (8" min.)
RL=182/-182

R=781 U=70 W=8" (8" min.)

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

Design Crit: FBC2010Res/TPI-2007(STD) 12.03.04.0326.14 QTY:1

PLT TYP. Wave FT/RT=10%(0%)/0(0) FL/-/3/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12808 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141003 |
| HC-ENG | JB/WPF |
| SEQN- | 296793 |
| | |
| JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A22 5' Common )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

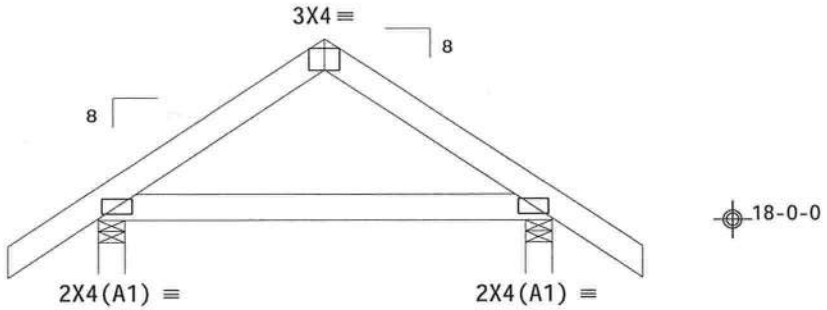
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

130 mph wind, 18.87 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




R=254 U=26 W=3.471" (3.471" min.)
RL=51/-70

R=254 U=48 W=3.5" (3.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12.03.04.0326.14

QTY: 14

FL/-/3/-/-/R/-

Scale =.5"/Ft.

WILLIAM W. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12809 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141018 |
| HC-ENG | JB/WPF |
| SEQN- | 13559 |
| | |
| JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - PBA1 8'0"1 Common )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

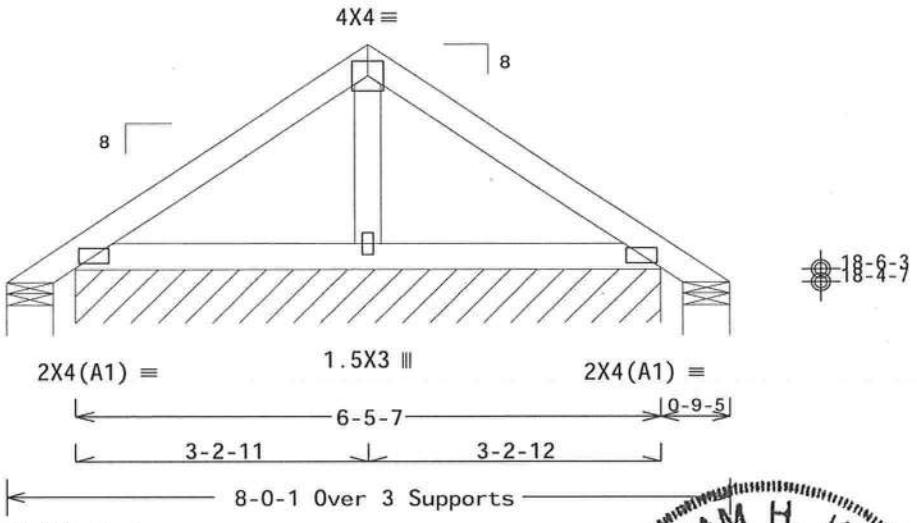
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 57 plf at 0.00 to 57 plf at 4.00
TC- From 57 plf at 4.00 to 57 plf at 8.00
BC- From 4 plf at 0.00 to 4 plf at 8.00

130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




R=2 Rw=61 U=69 W=6.31" (6.31" min.)
RL=72/-72

R=76 PLF U=41 PLF W=6-5-7

R=2 Rw=121 U=20 W=6.31" (6.31" min.)

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

12.03.04.0326 14 QTY:21 FL/-/3/-/-/R/-

Scale =.5"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12810 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141008 |
| BC LL | 0.0 PSF | HC-ENG | JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294925 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - PBA2 8'0"1 Common Girder )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 57 plf at 0.00 to 57 plf at 4.00
TC- From 57 plf at 4.00 to 57 plf at 8.00
BC- From 4 plf at 0.00 to 4 plf at 8.00

Wind loads and reactions based on MWFRS with additional C&C member design.

Refer to DWG PB160100212 for piggyback details.

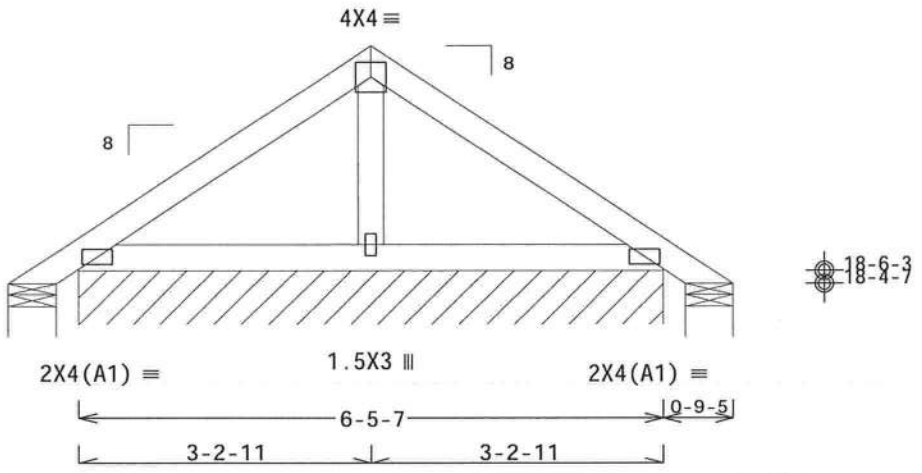
## 2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0826.14 | QTY: 4 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 12811 |
| TC DL | 7.0 PSF | DATE | 05/21/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13141012 |
| BC LL | 0.0 PSF | | HC-ENG JB/WPF |
| TOT.LD. | 37.0 PSF | SEQN- | 294927 |
| DUR.FAC. | 1.25 | | |
| SPACING | 24.0" | JREF- | 1UWF487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

( 13-166--Erkinger Home Builders  Miller Residence -- Lot #29 - PBA3 8'0"1 Gable )

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

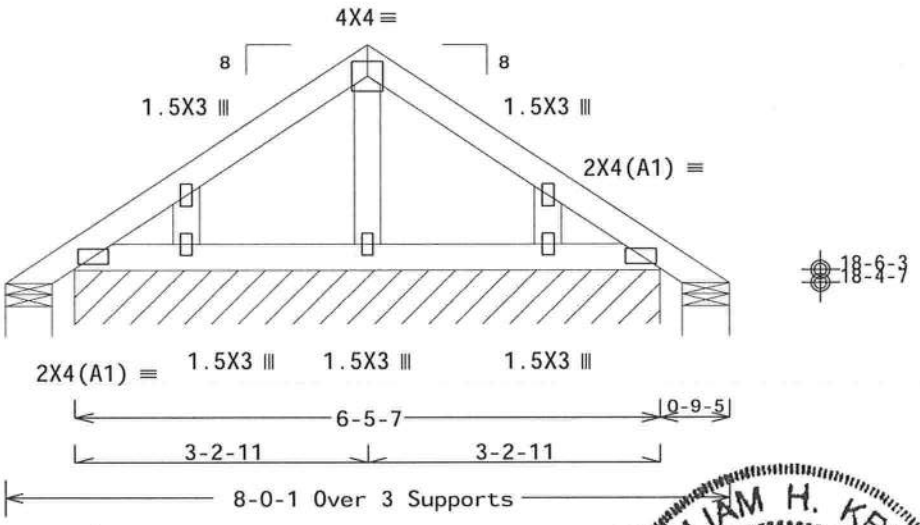
Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 57 plf at 0.00 to 57 plf at 4.00
TC- From 57 plf at 4.00 to 57 plf at 8.00
BC- From 4 plf at 0.00 to 4 plf at 8.00

130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

See DWGS A14030ENC100212 & GBLLETIN0212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.




R=18 Rw=42 U=38 W=6.31" (6.31" min.)
RL=72/-72

R=65 PLF U=31 PLF W=6-5-7

R=18 (-11" W=6.31" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

12.03.04.0386.14

QTY 2

FL/-/3/-/-/R/-

Scale =.5"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/22/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 12812 |
| DATE | 05/21/13 |
| DRW | HCUSR487 13141001 |
| HC-ENG | JB/WPF |
| SEQN- | 294926 |
| | |
| JREF- | 1UWF487_ZO1 |

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

## NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

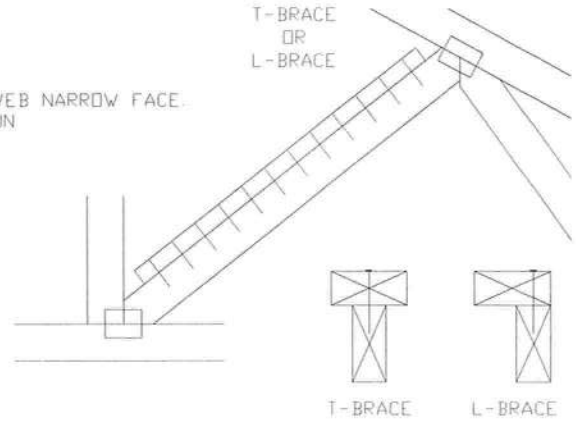
| WEB MEMBER SIZE | SPECIFIED CLB BRACING | ALTERNATIVE BRACING T OR L-BRACE | ALTERNATIVE BRACING SCAB BRACE |
|---|---|---|---|
| 2X3 OR 2X4 | 1 ROW | 2X4 | 1-2X4 |
| 2X3 OR 2X4 | 2 ROWS | 2X6 | 2-2X4 |
| 2X6 | 1 ROW | 2X4 | 1-2X6 |
| 2X6 | 2 ROWS | 2X6 | 2-2X4(*) |
| 2X8 | 1 ROW | 2X6 | 1-2X8 |
| 2X8 | 2 ROWS | 2X6 | 2-2X6(*) |

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

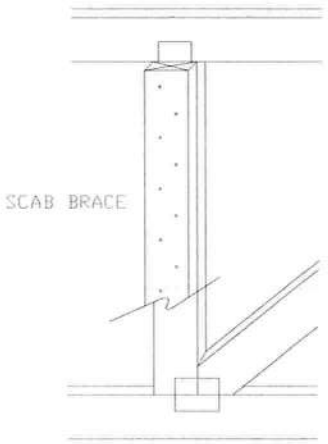
## T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH




## SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH




WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05/22/2013

Building Components Group Inc.

Earth City, MO 63045

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**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

May 22 '13

| | | |
|---|---|---|
| TC LL | PSF | REF CLB SUBST. |
| TC DL | PSF | DATE 1/1/09 |
| BC DL | PSF | DRWG BRCLBSUB0109 |
| BC LL | PSF | |
| TOT. L.D. | PSF | |
| DUR. FAC. | | |
| SPACING | | |

# Gable Stud Reinforcement Detail

## ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

| 2x4 Gable Vertical Spacing | Species | Brace Grade | No Braces | (1) 1x4 "L" Brace * | | (1) 2x4 "L" Brace * | | (2) 2x4 "L" Brace ** | | (1) 2x6 "L" Brace * | | (2) 2x6 "L" Brace ** | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 24" o.c. | SPF HF | #1 / #2 | 4' 3" | 7' 3" | 7' 7" | 8' 7" | 8' 11" | 10' 3" | 10' 8" | 13' 6" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 1" | 6' 7" | 7' 1" | 8' 6" | 8' 10" | 10' 1" | 10' 6" | 13' 4" | 13' 10" | 14' 0" | 14' 0" |
| | | Stud | 4' 1" | 7' 2" | 7' 5" | 8' 6" | 8' 10" | 10' 1" | 10' 6" | 13' 4" | 13' 10" | 14' 0" | 14' 0" |
| | | Standard | 4' 1" | 6' 11" | 7' 5" | 8' 6" | 8' 10" | 10' 1" | 10' 6" | 13' 4" | 13' 10" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 4' 4" | 7' 4" | 7' 7" | 8' 8" | 9' 0" | 10' 3" | 10' 8" | 13' 7" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 4' 3" | 7' 3" | 7' 7" | 8' 7" | 8' 11" | 10' 3" | 10' 8" | 13' 6" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 1" | 5' 11" | 6' 4" | 7' 11" | 8' 5" | 10' 1" | 10' 6" | 12' 5" | 13' 3" | 14' 0" | 14' 0" |
| | | Stud | 4' 1" | 5' 11" | 6' 4" | 7' 11" | 8' 5" | 10' 1" | 10' 6" | 12' 5" | 13' 3" | 14' 0" | 14' 0" |
| | | Standard | 3' 11" | 5' 2" | 5' 5" | 6' 10" | 7' 4" | 9' 3" | 9' 11" | 10' 9" | 11' 6" | 14' 0" | 14' 0" |
| 16" o.c. | SPF HF | #1 / #2 | 4' 11" | 8' 4" | 8' 8" | 9' 10" | 10' 3" | 11' 8" | 12' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 8" | 8' 1" | 8' 8" | 9' 8" | 10' 1" | 11' 7" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 8" | 8' 2" | 8' 6" | 9' 8" | 10' 1" | 11' 7" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 8" | 8' 2" | 8' 6" | 9' 8" | 10' 1" | 11' 7" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 5' 0" | 8' 5" | 8' 8" | 9' 11" | 10' 3" | 11' 9" | 12' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 4' 11" | 8' 4" | 8' 8" | 9' 10" | 10' 3" | 11' 8" | 12' 2" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 8" | 7' 3" | 7' 9" | 9' 8" | 10' 1" | 11' 7" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 8" | 7' 3" | 7' 9" | 9' 8" | 10' 1" | 11' 7" | 12' 1" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 8" | 6' 3" | 6' 8" | 8' 5" | 8' 11" | 11' 4" | 12' 1" | 13' 2" | 14' 0" | 14' 0" | 14' 0" |
| 12" o.c. | SPF HF | #1 / #2 | 5' 5" | 9' 2" | 9' 6" | 10' 10" | 11' 3" | 11' 8" | 13' 5" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 5' 1" | 9' 0" | 9' 4" | 10' 8" | 11' 1" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 5' 1" | 9' 0" | 9' 4" | 10' 8" | 11' 1" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 5' 1" | 9' 0" | 9' 4" | 10' 8" | 11' 1" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 5' 6" | 9' 3" | 9' 7" | 10' 11" | 11' 4" | 12' 11" | 13' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 5' 5" | 9' 2" | 9' 6" | 10' 10" | 11' 3" | 12' 11" | 13' 5" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 5' 1" | 8' 4" | 8' 11" | 10' 8" | 11' 1" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 5' 1" | 8' 4" | 8' 11" | 10' 8" | 11' 1" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 5' 1" | 7' 3" | 7' 9" | 9' 8" | 10' 4" | 12' 9" | 13' 3" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |

Bracing Group Species and Grades:

Group A:

| Spruce-Pine-Fir | | Hem-Fir | |
|---|---|---|---|
| #1 / #2 | Standard | #2 | Stud |
| #3 | Stud | #3 | Standard |

| Douglas Fir-Larch | Southern Pine*** |
|---|---|
| #3 | #3 |
| Stud | Stud |
| Standard | Standard |

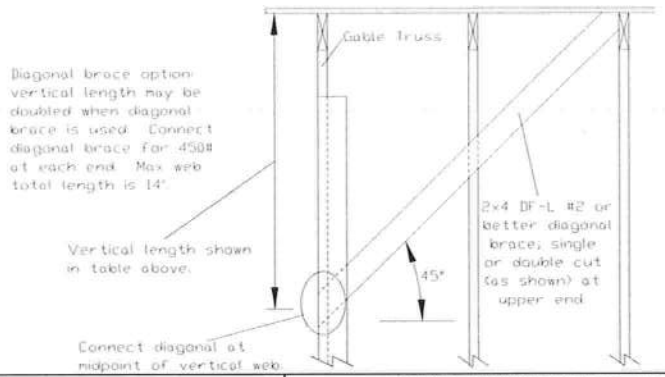
Group B:

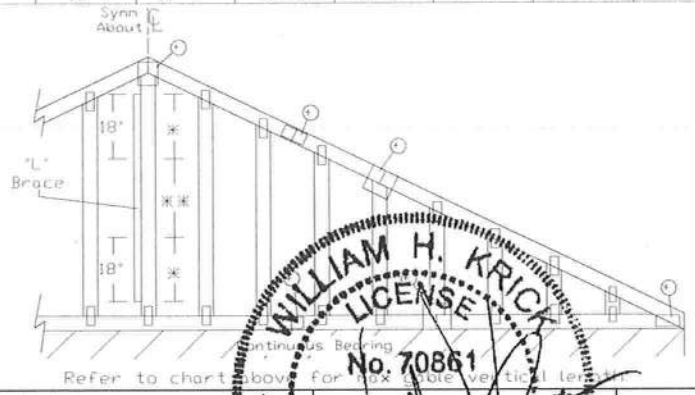
| Hem-Fir |
|---|
| #1 & Btr |
| #1 |

| Douglas Fir-Larch | Southern Pine*** |
|---|---|
| #1 | #1 |
| #2 | #2 |

1x4 Braces shall be SRB (Stress-Rated Board)

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.




## Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

| Vertical Length | No Splice |
|---|---|
| Less than 4' 0" | 1X4 or 2X3 |
| Greater than 4' 0", but less than 11' 6" | 2.5X4 |
| Greather than 11' 6" | 3X4 |

* Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

May 22 '13

ITW Building Components Group Inc.

Earth City, MO 63045

**WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
**IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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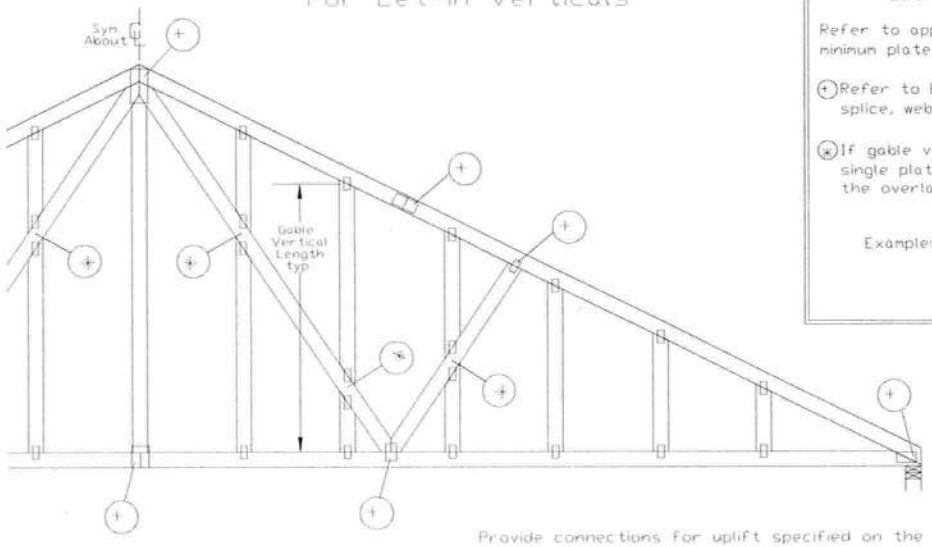
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-10-GAB14015
DATE 2/14/12
DRWG A1401SENC100212

# Gable Detail
## For Let-in Verticals




### Gable Truss Plate Sizes

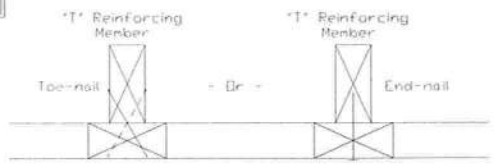
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(⊗) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 / 2X4 — 2X8

### "T" Reinforcement Attachment Detail




To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

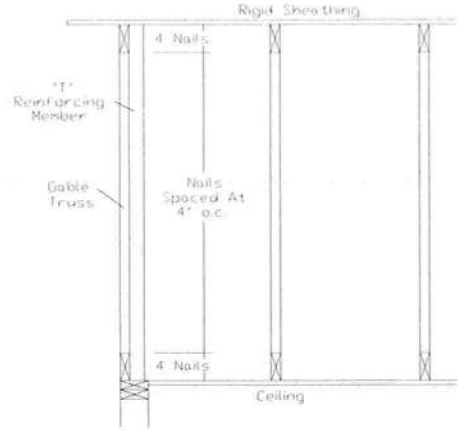
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

### Web Length Increase w/ "T" Brace

| "T" Reinf. Mbr. Size | "T" Increase |
|---|---|
| 2x4 | 30 % |
| 2x6 | 20 % |

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"




Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with:

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109, A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109, A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, A16015ENC100212, A18015ENC100212, A20015ENC100212, A20015PE100212, A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212, A18030ENC100212, A20030ENC100212, A20030PE100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013

May 22 '13

Building Components Group Inc.

Earth City, MO 63045

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**IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

# Piggyback Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

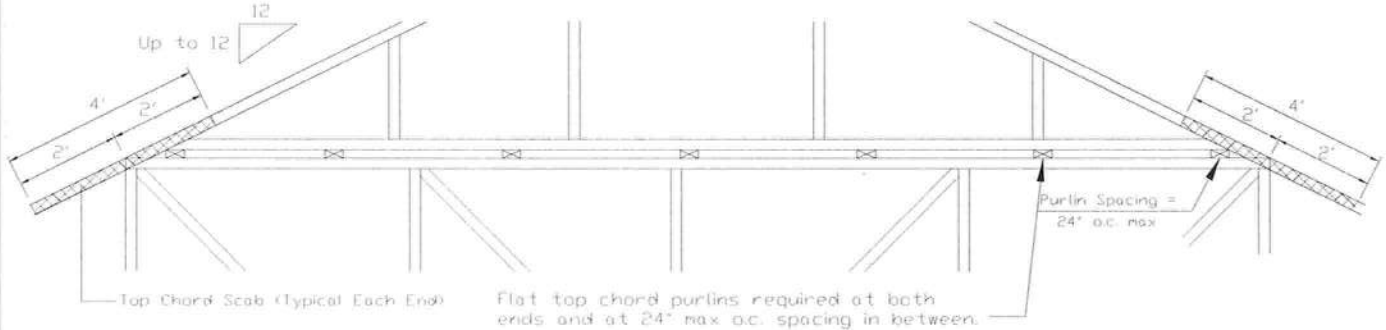
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg located anywhere in roof, Exp B, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

## Detail A : Purlin Spacing = 24" o.c. or less




Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

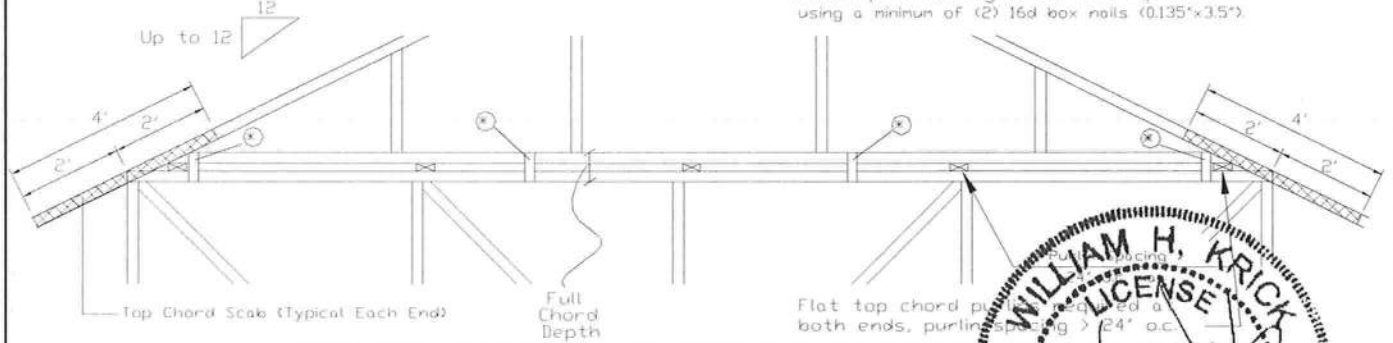
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

## Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").




Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

**Trulox**
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4" o.c. front to back faces.

**APA Rated Gusset**
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8" o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4" o.c. front to back faces.

**2x4 Vertical Scabs**
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8" o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4" o.c. front to back faces.

**28PB Wave Piggyback Plate**
One 28PB wave piggyback plate to each face @ 8" o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4" o.c. front to back faces.

ITW Building Components Group Inc.

Earth City, MO 63045

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For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05/22/2013

May 22 '13

SPACING 24.0"

| | |
|---|---|
| REF | PIGGYBACK |
| DATE | 2/14/12 |
| DRWG | PB160100212 |

# Gable Stud Reinforcement Detail

## ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

| 2x4 Gable Vertical Spacing | Species | Brace Grade | No Braces | (1) 1x4 "L" Brace * | | (1) 2x4 "L" Brace * | | (2) 2x4 "L" Brace ** | | (1) 2x6 "L" Brace * | | (2) 2x6 "L" Brace ** | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B | Group A | Group B |
| 24" o.c. | SPF HF | #1 / #2 | 4' 1" | 6' 11" | 7' 2" | 8' 2" | 8' 6" | 9' 9" | 10' 2" | 12' 10" | 13' 4" | 14' 0" | 14' 0" |
| | | #3 | 3' 10" | 6' 2" | 6' 7" | 8' 1" | 8' 5" | 9' 8" | 10' 0" | 12' 8" | 13' 2" | 14' 0" | 14' 0" |
| | | Stud | 3' 10" | 6' 10" | 7' 1" | 8' 1" | 8' 5" | 9' 8" | 10' 0" | 12' 8" | 13' 2" | 14' 0" | 14' 0" |
| | | Standard | 3' 10" | 6' 5" | 6' 10" | 8' 1" | 8' 5" | 9' 8" | 10' 0" | 12' 8" | 13' 2" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 4' 2" | 7' 0" | 7' 3" | 8' 3" | 8' 7" | 9' 9" | 10' 2" | 12' 11" | 13' 5" | 14' 0" | 14' 0" |
| | | #2 | 4' 1" | 6' 10" | 7' 2" | 8' 2" | 8' 6" | 9' 9" | 10' 2" | 12' 10" | 13' 4" | 14' 0" | 14' 0" |
| | | #3 | 3' 10" | 5' 6" | 5' 10" | 7' 4" | 7' 10" | 9' 8" | 10' 0" | 11' 6" | 12' 4" | 14' 0" | 14' 0" |
| | | Stud | 3' 10" | 5' 6" | 5' 10" | 7' 4" | 7' 10" | 9' 8" | 10' 0" | 11' 6" | 12' 4" | 14' 0" | 14' 0" |
| | | Standard | 3' 8" | 4' 9" | 5' 1" | 6' 4" | 6' 10" | 8' 7" | 9' 3" | 10' 0" | 10' 8" | 13' 7" | 14' 0" |
| 16" o.c. | SPF HF | #1 / #2 | 4' 8" | 7' 11" | 8' 3" | 9' 4" | 9' 9" | 11' 2" | 11' 7" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 5" | 7' 6" | 8' 3" | 9' 3" | 9' 7" | 11' 0" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 5" | 7' 10" | 8' 1" | 9' 3" | 9' 7" | 11' 0" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 5" | 7' 10" | 8' 1" | 9' 3" | 9' 7" | 11' 0" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 4' 9" | 8' 0" | 8' 3" | 9' 5" | 9' 9" | 11' 2" | 11' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 4' 8" | 7' 11" | 8' 3" | 9' 4" | 9' 9" | 11' 2" | 11' 7" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 5" | 6' 9" | 7' 2" | 9' 0" | 9' 7" | 11' 0" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 5" | 6' 9" | 7' 2" | 9' 0" | 9' 7" | 11' 0" | 11' 6" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 5" | 5' 10" | 6' 3" | 7' 9" | 8' 4" | 10' 6" | 11' 3" | 12' 3" | 13' 1" | 14' 0" | 14' 0" |
| 12" o.c. | SPF HF | #1 / #2 | 5' 2" | 8' 9" | 9' 1" | 10' 4" | 10' 9" | 11' 2" | 12' 9" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 10" | 8' 7" | 8' 11" | 10' 2" | 10' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 10" | 8' 7" | 8' 11" | 10' 2" | 10' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 10" | 8' 7" | 8' 11" | 10' 2" | 10' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | SP DFL | #1 | 5' 3" | 8' 9" | 9' 1" | 10' 4" | 10' 9" | 12' 4" | 12' 10" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #2 | 5' 2" | 8' 9" | 9' 1" | 10' 4" | 10' 9" | 12' 3" | 12' 9" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | #3 | 4' 10" | 7' 9" | 8' 3" | 10' 2" | 10' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Stud | 4' 10" | 7' 9" | 8' 3" | 10' 2" | 10' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |
| | | Standard | 4' 10" | 6' 9" | 7' 2" | 9' 0" | 9' 7" | 12' 2" | 12' 8" | 14' 0" | 14' 0" | 14' 0" | 14' 0" |

Bracing Group Species and Grades:

Group A:

| Spruce-Pine-Fir | |
|---|---|
| #1 / #2 | Standard |
| #3 | Stud |

| Hem-Fir | |
|---|---|
| #2 | Stud |
| #3 | Standard |

| Douglas Fir-Larch |
|---|
| #3 |
| Stud |
| Standard |

| Southern Pine*** |
|---|
| #3 |
| Stud |
| Standard |

Group B:

| Hem-Fir |
|---|
| #1 & Btr |
| #1 |

| Douglas Fir-Larch |
|---|
| #1 |
| #2 |

| Southern Pine*** |
|---|
| #1 |
| #2 |

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

## Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

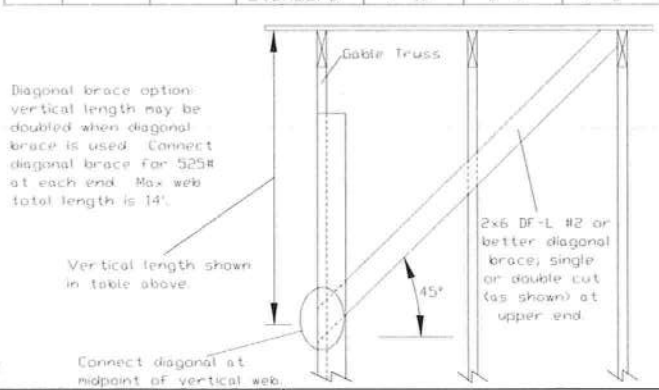
**For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

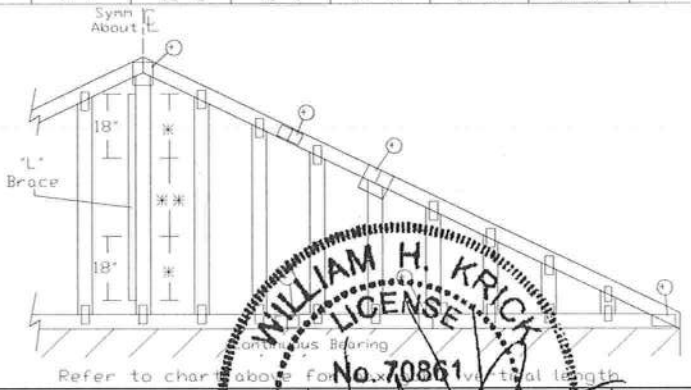
"L" bracing must be a minimun of 80% of web member length.

| Gable Vertical Plate Sizes | |
|---|---|
| Vertical Length | No Splice |
| Less than 4' 0" | 1.5X4 |
| Greater than 4' 0", but less than 11' 6" | 2.5X4 |
| Greather than 11' 6" | 3.5X4 |

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.




WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/22/2013
May 22 '13

ITW Building Components Group Inc.
Earth City, MO 63045

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For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

MAX. TOT. L.D. 60 PSF
MAX. SPACING 24.0"

REF ASCE7-10-GAB14030
DATE 2/14/12
DRWG A14030ENC100212

73

PERMIT# 31109

# ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UYC487-Z0130114605

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/30/2013

| | |
|---|---|
| Truss Fabricator: | Anderson Truss Company |
| Job Identification: | 13-166B--Erkinger Home Builders Miller Residence -- Indian Ridge |
| Truss Count: | 2 |
| Model Code: | Florida Building Code 2010 |
| Truss Criteria: | FBC2010Res/TPI-2007(STD) |
| Engineering Software: | Alpine Software,Version 12.03. |
| Structural Engineer of Record: | The identity of the structural EOR did not exist as of |
| Address: | the seal date per section 61G15-31.003(5a) of the FAC |
| Minimum Design Loads: | Roof - 37.0 PSF @ 1.25 Duration |
| | Floor - N/A |
| | Wind - 130 MPH ASCE 7-10 -Closed |

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-160TL-

| # | Ref | Description | Drawing# | Date |
|---|---|---|---|---|
| 1 | 67623-RA7 | 30' Stepdown | 13211006 | 07/30/13 |
| 2 | 67624-RA8 | 38' Stepdown | 13211053 | 07/30/13 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-166B--Erkinger Home Builders Miller Residence -- Indian Ridge S/D - RA7 30' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B2 2x4 SP 2850f-2.3E:
Webs 2x4 SP_#3__12A :W3, W6 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Left end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Shim all supports to solid bearing.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.29" due to live load and 0.40" due to dead load.

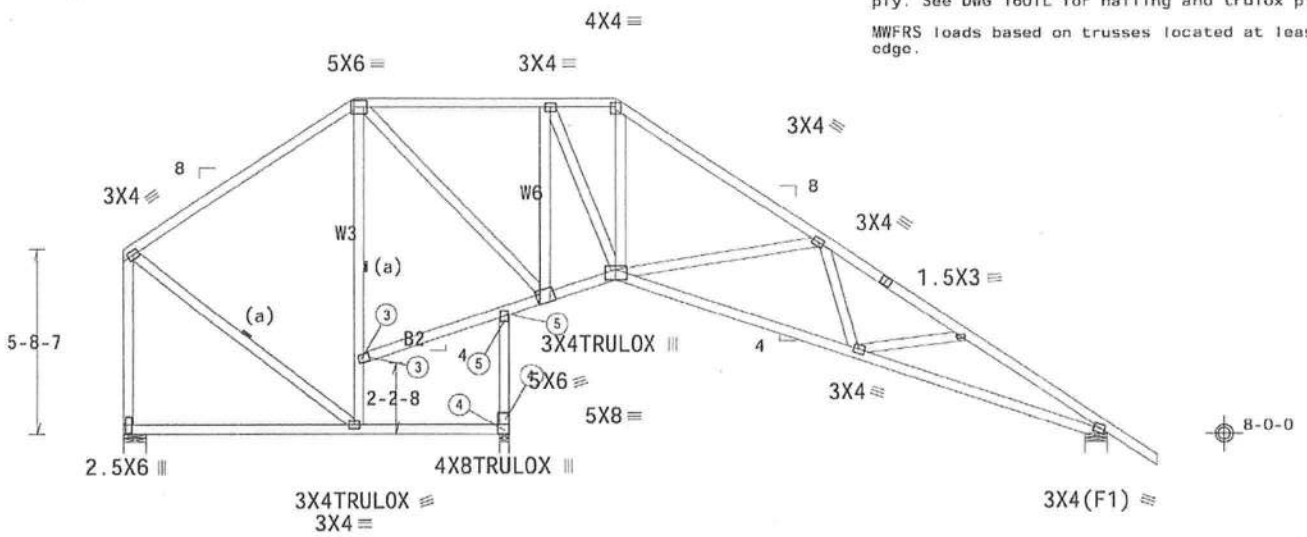
(a) Continuous lateral bracing equally spaced on member.

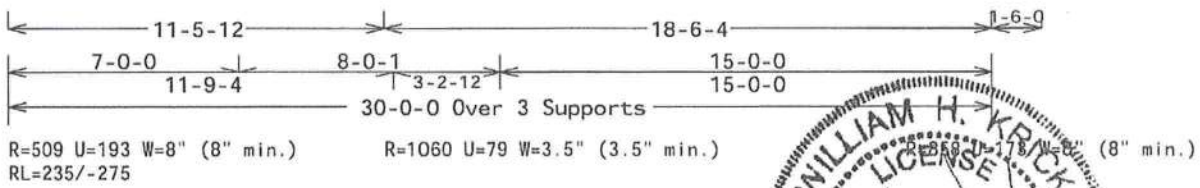
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 160TL for nailing and trulox plate requirements.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.




R=509 U=193 W=8" (8" min.)
RL=235/-275

R=1060 U=79 W=3.5" (3.5" min.)

R=858 U=178 W=8" (8" min.)

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/30/2013

Design Crit: FBC2010Res/TPI-2007(STD)
PLT TYP. Wave,Trulox
FT/RT=10%(0%)/0(0)
12.03.04.0326.14 QTY 1
FL/-/3/-/-/R/-
Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
**IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

| | | | |
|---|---|---|---|
| TC LL | 20.0 PSF | REF | R487-- 67623 |
| TC DL | 7.0 PSF | DATE | 07/30/13 |
| BC DL | 10.0 PSF | DRW | HCUSR487 13211006 |
| BC LL | 0.0 PSF | HC-ENG | WHK/WHK |
| TOT.LD. | 37.0 PSF | SEQN- | 312178 |
| DUR.FAC. | 1.25 | FROM | JMW |
| SPACING | 24.0" | JREF- | 1UYC487_Z01 |

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-166B--Erkinger Home Builders Miller Residence -- Indian Ridge S/D - RA8 38' Stepdown Hip)

Top chord 2x4 SP_#1_12A :T1 2x4 SP 2850f-2.3E:
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W1, W2, W8 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 160TL for nailing and trulox plate requirements.

Negative reaction(s) of -1298# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

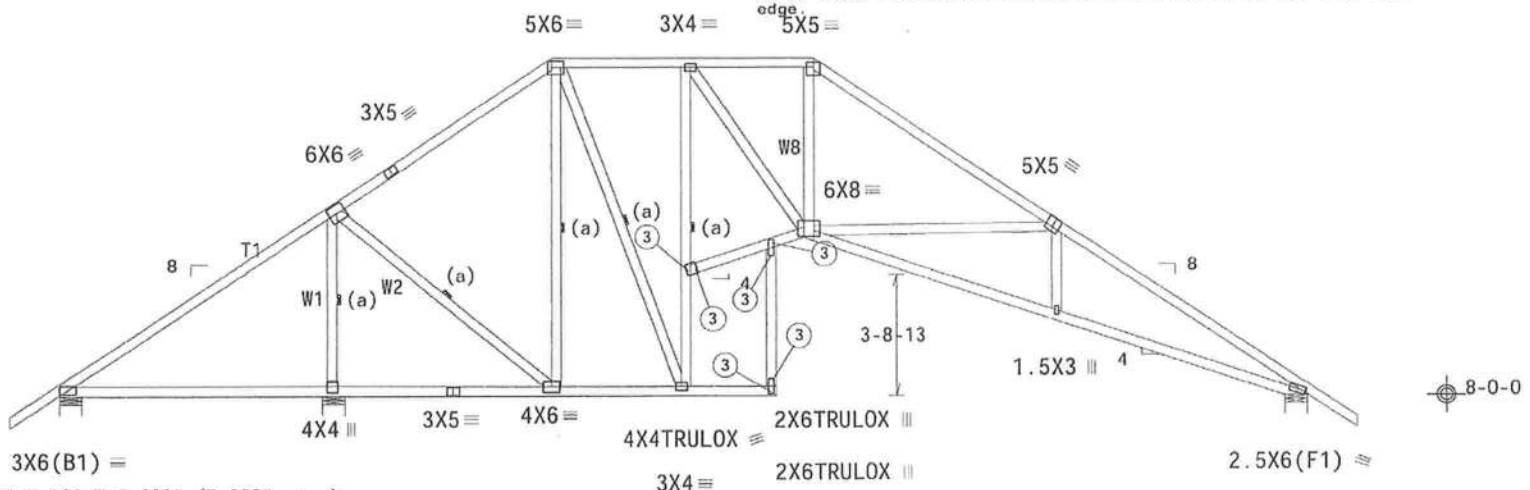
Calculated horizontal deflection is 0.20" due to live load and 0.28" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.




R=-1299 Rw=43 U=663 W=7.936" (7.936" min.)
RL=376/-376

R=846 U=153 W=8" (8" min.)

R=3340 U=37 W=8" (8" min.)

1-6-0 | 7-11-15 | 30-0-1 | 1-6-0

14-11-15 | 8-0-1 | 15-0-1
21-10-7 | 0-10-0 | 15-3-9

38-0-0 Over 3 Supports

PLT TYP. Wave,Trulox

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04.0326 14 QTY: 1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/30/2013

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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| | |
|---|---|
| TC LL | 20.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 37.0 PSF |
| DUR.FAC. | 1.25 |
| SPACING | 24.0" |

| | |
|---|---|
| REF | R487-- 67624 |
| DATE | 07/30/13 |
| DRW | HCUSR487 13211053 |
| HC-ENG | WHK/WHK |
| SEQN- | 312169 |
| FROM | JMW |
| JREF- | 1UYC487_Z01 |

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

## NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING. RE-PURCHASE WITH APPROPRIATE BRACING.

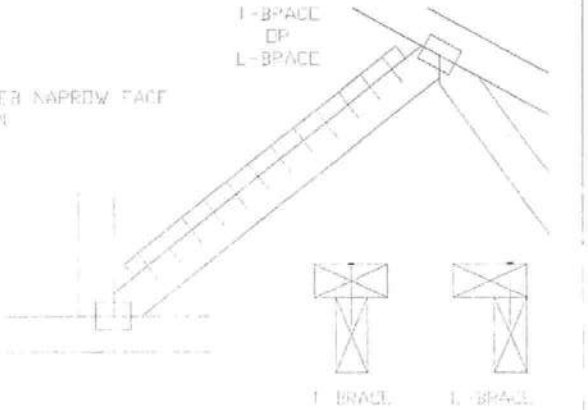
| WEB MEMBER SIZE | SPECIFIED CLB BRACING | ALTERNATIVE BRACING T OR L-BRACE | SCAB BRACE |
|---|---|---|---|
| 2X5 OR 2X4 | 1 ROW | 2X4 | 1-2X4 |
| 2X3 OR 2X4 | 2 ROWS | 2X6 | 2-2X4 |
| 2X6 | 1 ROW | 2X4 | 1-2X6 |
| 2X6 | 2 ROWS | 2X6 | 2-2X4(p) |
| 2X8 | 1 ROW | 2X6 | 1-2X8 |
| 2X8 | 2 ROWS | 2X6 | 2-2X6(p) |

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(p) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

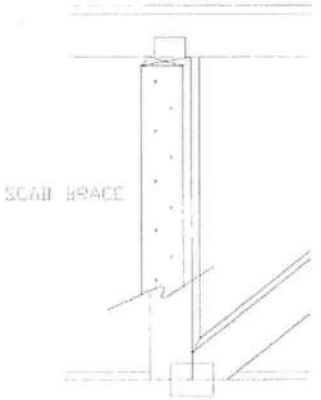
### T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE
ATTACH WITH 10d BOX OR GUN
(0.128"x 3" MIN) NAILS
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH




### SCAB BRACING:

APPLY SCABS TO WIDE FACE OF WEB
NO MORE THAN (1) SCAB PER FACE
ATTACH WITH 10d BOX OR GUN
(0.128"x 3" MIN) NAILS
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH




WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
07/30/2013
Jul 30 '13

Building Components Group Inc.

Earth City, MO 63045

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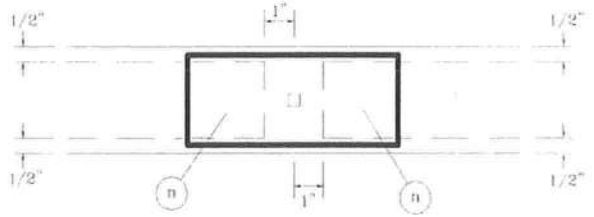
**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any failure to build the trusses in conformance with TPI, for fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA ASTM A653 A570 grade 33/40/50 (2/3/4H) galv steel. Apply plates to each face of truss, positioned as shown and on Joint Detail.
A seal on the drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG www.itwbcg.com, TPI www.tpinst.org, WTCA www.woodtruss.com, BCSI www.bcsifocus.com

| | | |
|---|---|---|
| TC LL | PSF | REF CLB SUBST |
| TC DL | PSF | DATE 1/1/09 |
| BC LL | PSF | DRWG BRCLBSUB0109 |
| BC DL | PSF | |
| TOT. L.D. | PSF | |
| JOB REF. | | |
| SPACING | | |

# TRULOX INFORMATION DETAIL

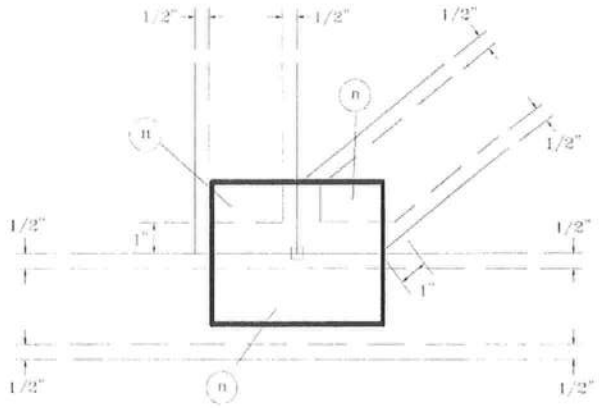
## TYPICAL OFF PANEL SPLICE



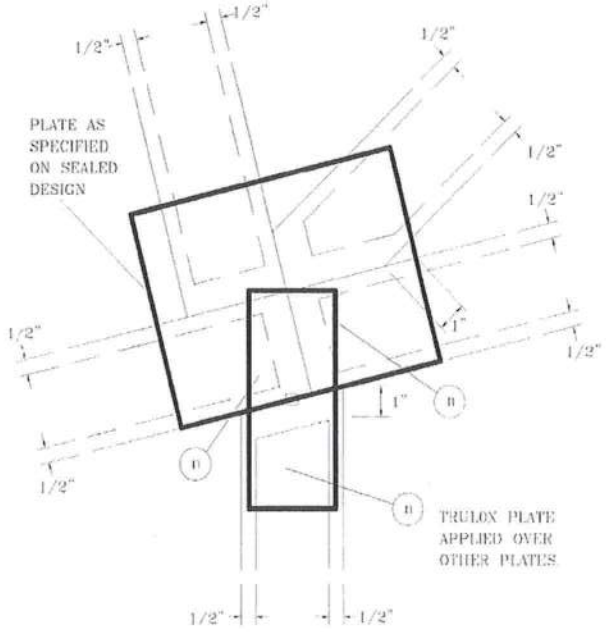

DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

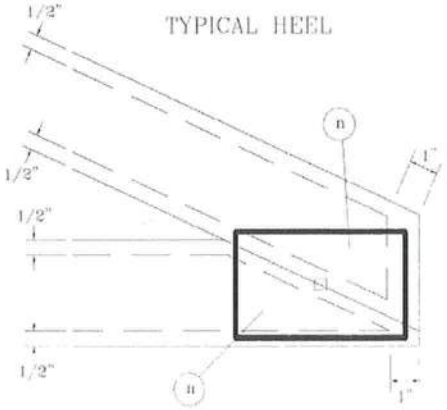
## TYPICAL PANEL POINT WITHOUT SPLICE



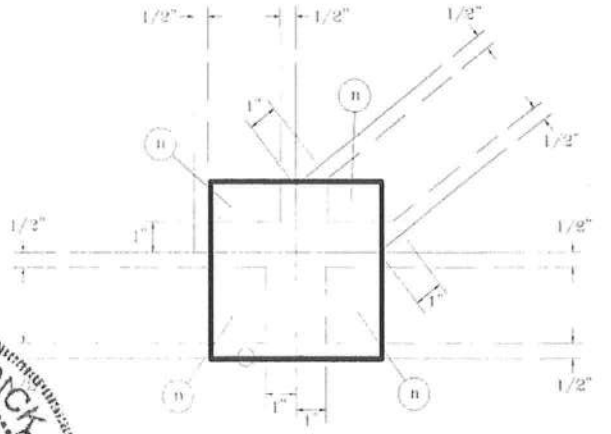

## TYPICAL FILLER




## TYPICAL HEEL




## TYPICAL PANEL POINT SPLICE




ITW
Building Components Group Inc.

Earth City, MO 63045

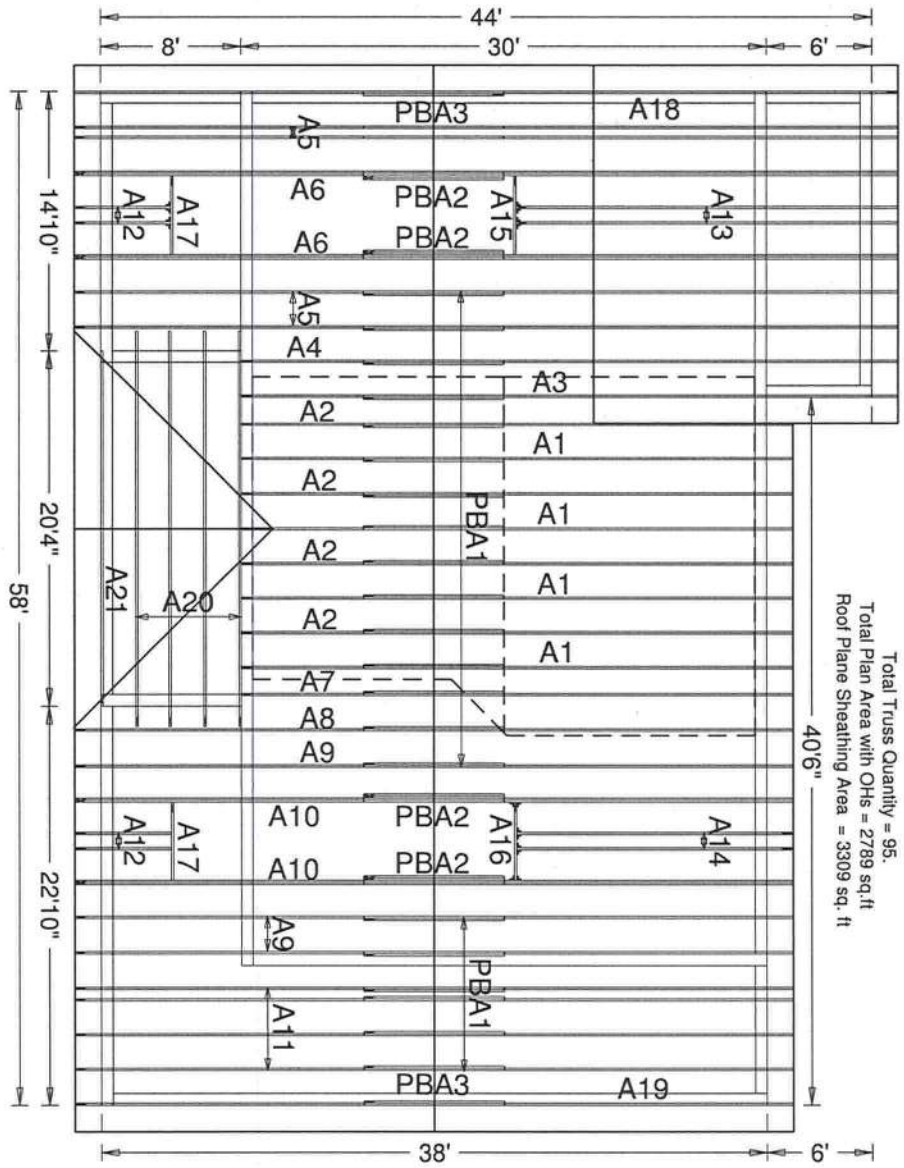
NOTES:
(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
○ LOCATES PLATE CORNER OR FLUSH EDGE.
□ LOCATES PLATE CENTER.

WILLIAM H. KRICK
LICENSE
No. 78861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
07/30/2013
Jul 30 '13

TRULOX PLATING

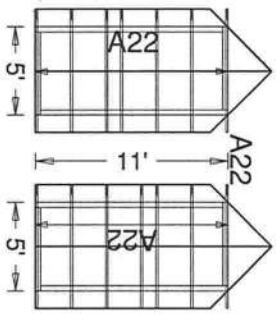
160
TL

PAGE 1 OF 1
DATE 4/1/09




# Miller Residence Alternate

Total Truss Quantity = 95.
Total Plan Area with OHs = 2789 sq.ft
Roof Plane Sheathing Area = 3309 sq. ft





PAGE NO: 1 OF 1

JOB NO: 13-166

JOB DESCRIPTION:: Erkinger Home Builders
/: Miller Residence
ADDRESS:: Lot #29
Indian Ridge S/D
Columbia County
JOB #: 13-166
DESIGNER: Jon Williams
SALESMAN: Curt V Burlingame

PlanName: Kieth and Tammy Miller
Created : 05-20-2013

ANDERSON TRUSS COMPANY
"QUALITY PRE-ENGINEERED WOOD TRUSSES"