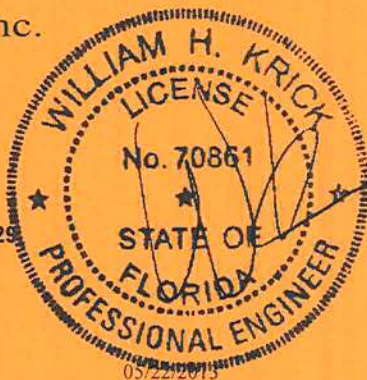


# ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844  
Florida Engineering Certificate of Authorization Number: 0 278  
Florida Certificate of Product Approval # FL1999  
Page 1 of 1 Document ID:1UWF487-Z0122153651



Truss Fabricator: **Anderson Truss Company**  
Job Identification: **13-166--Erkinger Home Builders Miller Residence -- Lot #29**  
Truss Count: **25**  
Model Code: **Florida Building Code 2010**  
Truss Criteria: **FBC2010Res/TPI-2007(STD)**  
Engineering Software: **Alpine Software, Version 12.03.**  
Structural Engineer of Record: **The identity of the structural EOR did not exist as of the seal date per section 61615-31.003(5a) of the FAC**  
Address: **Roof - 37.0 PSF @ 1.25 Duration**  
Minimum Design Loads: **Floor - N/A**  
**Wind - 130 MPH ASCE 7-10 -Closed**

## Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

William H. Krick  
-Truss Design Engineer-

1950 Marley Drive  
Haines City, FL 33844

Details: BRCLBSUB-14015EC1-GBLLETIN-PB16010-14030EC1-

#	Ref	Description	Drawing#	Date
1	12788-A1	30' Steppedown	13141005	05/21/13
2	12789-A2	30' Steppedown	13141021	05/21/13
3	12790--A3	36' Special	13141010	05/21/13
4	12791--A4	36' Special	13142001	05/22/13
5	12792--A5	44' Special	13141013	05/21/13
6	12793-A6	44' Special G	13142002	05/22/13
7	12794-A7	30' Steppedown	13141019	05/21/13
8	12795-A8	38' Steppedown	13141004	05/21/13
9	12796-A9	38' Steppedown	13141016	05/21/13
10	12797-A10	38' Steppedown	13142003	05/22/13
11	12798-A11	38' Steppedown	13141017	05/21/13
12	12799--A12	4' Flat	13141020	05/21/13
13	12800-A13	20' 3" 8 Spec	13141015	05/21/13
14	12801-A14	14' 3" 8 Comm	13141006	05/21/13
15	12802-A15	4' 6" Flat Gi	13141011	05/21/13
16	12803-A16	4' 6" Flat Gi	13141014	05/21/13
17	12804-A17	4' 6" Flat Gi	13141009	05/21/13
18	12805--A18	44' Gable	13141007	05/21/13
19	12806--A19	38' Gable	13141002	05/21/13
20	12807-A20	20' 4" Common	13141022	05/21/13
21	12808-A21	20' 4" Common	13141003	05/21/13
22	12809--A22	5' Common	13141018	05/21/13
23	12810-PBA1	8' 0" 1 Comm	13141008	05/21/13
24	12811-PBA2	8' 0" 1 Comm	13141012	05/21/13
25	12812-PBA3	8' 0" 1 Gable	13141001	05/21/13





THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webbs	2x4	SP_#3_12A
Fillier	2x10	SP_#2_12A

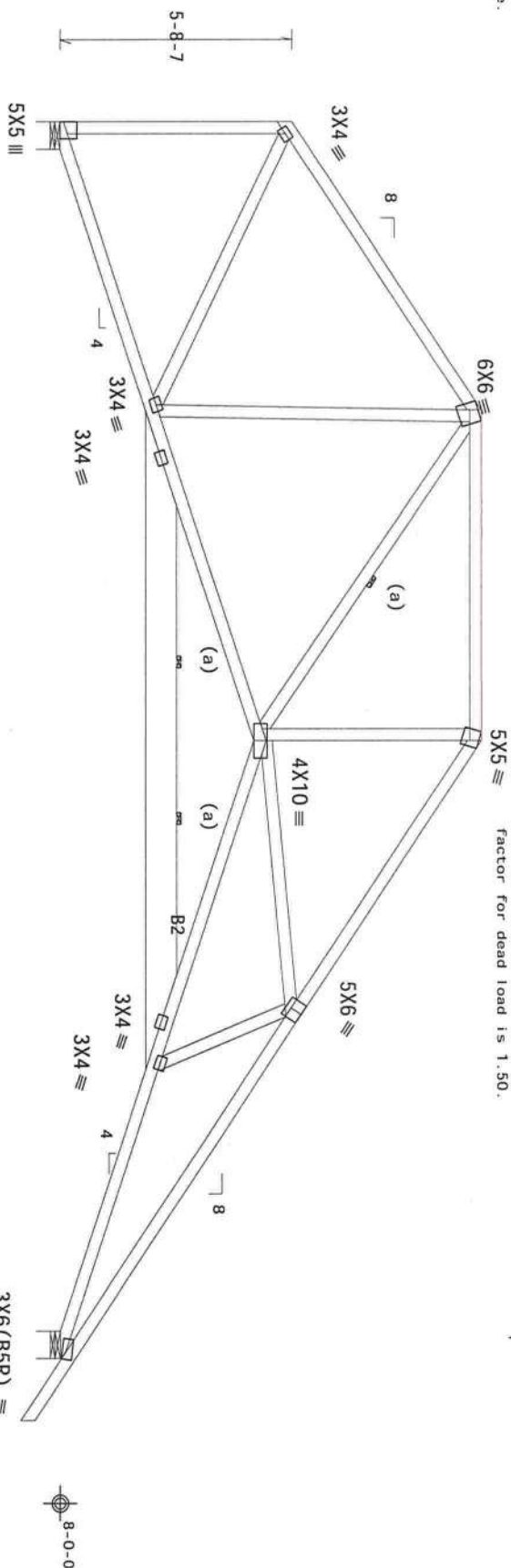
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24 OC.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, Risk CAT II, Exp C, wind TC DL=3.5 psf, wind BC DL=5.0 psf,  $G C p f (+/-)=0.18$

Wind loads and reactions based on MMFRS with additional C&C member design.

Calculated horizontal deflection is 0.13" due to live load and 0.18" due to dead load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

7-0-0 15-11-15 7-0-0  
 7-0-0 8-0-1 15-0-0  
 15-0-0  
 30-0-0 Over 2 Supports 15-0-0  
 R=1162 U=103 W=8" (8" min.)  
 RL=210/-250  
 R=1267 U=1

R=1267 U=115 W=8" (8" min.)

PLT TYP. Wave

Design Crit: FBC2010Res/TP1-2007(STP)  
FT/RT=10%(0%)/0(0)

**FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

**ITW Building Components Group Inc**

Haines City, FL 33844  
FL COA #0278

Tenants require steelwork come care in fabricating, handling, shipping, installing and bracing. To follow the latest edition of BCSI (Building Component Safety Information), by TPI and WCA, all practices prior to performing these functions. Insulators shall provide temporary bracing plan and details as shown on drawings. All bracing shall be installed before erection of structural steel. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid soffit ceiling. Location shown for permanent lateral restraint shall have bracing installed per BCSI sections 87 or 910, as applicable.

1TW Building Components Group Inc. (TBGCO) shall not be responsible for any deviation from this specification if such deviation is required to complete the project. TBGCO shall not be responsible for any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Detail Drawing. The responsibility for design, fabrication, inspection, testing, and engineering of the bracing system rests solely with the designer. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information see: This Job's general notes page: 1TW-BGCO, www.tbco.com; TPI, www.tpi.net.org; WCA, www.steelindustry.com; ICC, www.iccsafe.org

ports  
WILLIAM H. KRICK  
LICENSE  
No. 708614  
12.03.04 08:26.14  
Q17

FL/-/3/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0 PSF	REF R487-- 12788
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141005
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN- 296661
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01

Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A :B2 2x4 SP M-30:  
Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

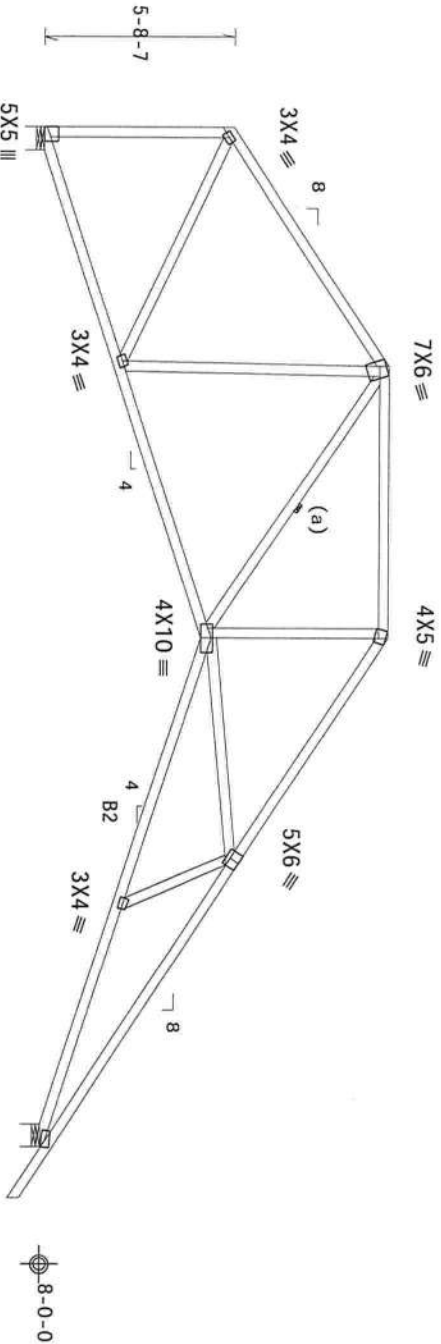
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Calculated horizontal deflection is 0.15" due to live load and 0.20" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



7-0-0 15-0-0 8-0-1 15-0-0 15-0-0  
30-0-0 Over 2 Supports  
R=1267 U=118 W=8" (8" min.)  
RL=210/-250

PLT TYP. Wave

Design Crit: FBC2010Res/TP1-2007 (STB)  
FT/RT=10%(0%)/0(0)

\*\*IMPORTANT\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET!

Trusses require extreme care in fabricating, handling, shipping, installing, and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for instructions on performing these functions. Installers shall provide temporary bracing per BCSI instructions. Trusses shall be braced in all directions. Trusses shall have a properly attached rigid collar. Trusses shall have bracing installed per BCSI sections B1, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TP1-1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 180A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering design. The responsibility of the Building Designer per ANSI/TP1-1 and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1-1. This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpiinc.org; WTCA: www.theindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844  
FL COA #0278



TC LL	20.0 PSF	REF R487-- 12789
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141021
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 296481
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24 OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, Exp C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCFI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

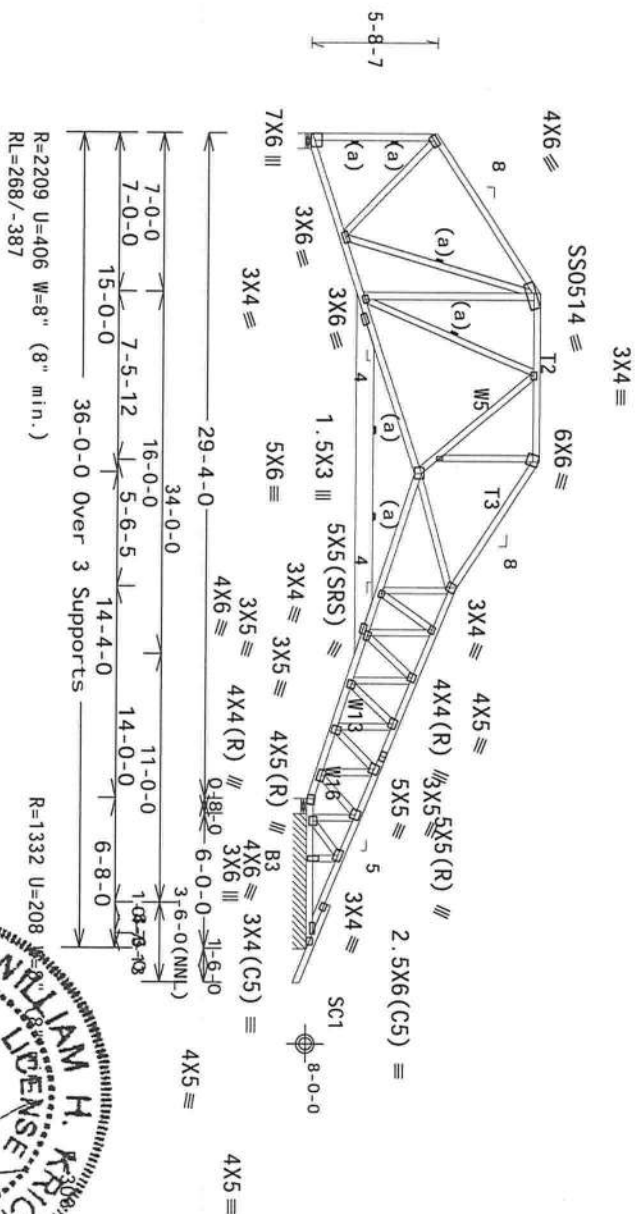
Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

See DMGS A14015EMC100212 & GBLLETT10212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Shim all supports to solid bearing.



PLT TYP. 18 Gauge HS, Wave

Design Crit: FBC2010Res/TP1-2007(STB  
FT/RT=10%(0%)/0(0)

No. 70867  
12.03.04.0386.74

FL--/3/--/B-

Scale = .125"/Ft.

WARNING - READ AND FOLLOW ALL NOTES ON THIS SHEET!  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

## ADDITIONAL INFORMATION

**ITW Building Components Group Inc**

Haines City, FL 33844  
FL COA #0278

**\*\*IMPORTANT\*\***

Truscon requires extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of DCSI Building Component Safety Information, by TPI and WCA for details on proper installation and bracing practices prior to performing these functions. Installers shall provide temporary bracing per drawings noted otherwise. Tool shed shall have properly attached structural sheathing and bottom chord bracing. All bracing shall be installed per DCSI detailing. Locations shown for permanent lateral constraint of tool shed shall be braced installed per DCSI detailing.

The Building Components Group Inc. (BTBCG) shall not be responsible for any deviation from this document. The contractor shall ensure that all components are installed as specified on the drawings. Any failure to build the truss in conformance with BTBCG specifications will be at the contractors' expense. Details, unless noted otherwise, Refer to drawings BTBD-2 for standard plate positions. A seal on this drawing or cover page lacking this drawing indicates acceptance of professional engineering approval and responsibility of the building system. The suitability and use of this design for any structure is the responsibility of the building owner.

general notes page: ITR-BDG: [www.ltdbcg.com](http://www.ltdbcg.com) TEL: [781-960-3000](tel:7819603000) FAX: [781-960-3000](tel:7819603000) E-mail: [sales@ltdbcg.com](mailto:sales@ltdbcg.com) [www.trussco.org](http://www.trussco.org)

WILLIAM H. KRICK  
LICENSE  
No. 70861  
03-04-036-14  
STATE OF FLORIDA  
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R487-- 12790
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141010
BC LL	0.0 PSF	HC-ENG JB/WMP
TOT.LD.	37.0 PSF	SEQN- 2969993
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Negative reaction(s) of -277# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf wind BC DL=5.0 psf. Gcpi (+/-)=0.18

(a) Continuous lateral bracing equally spaced on member

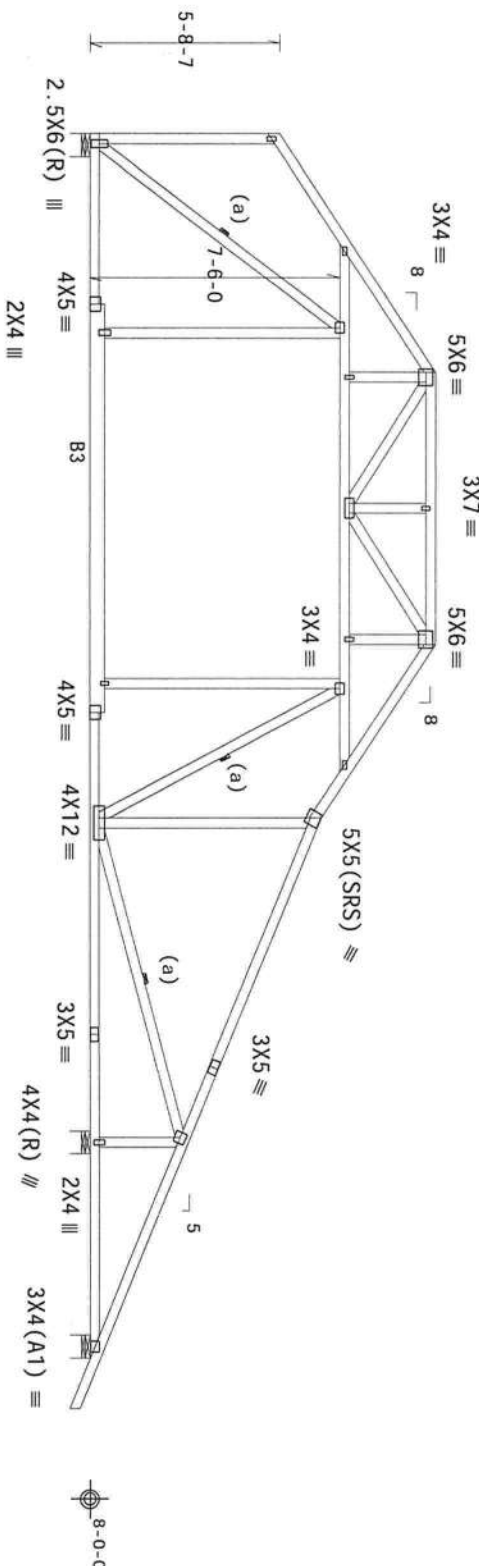
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.00 ft. from roof

edge.



RL=173/-247

Note: All Plates Are 1.5X3 Except As Shown

Design Crit: FBC2010Res/TP1-2007(STB)

PLT TYP. Wave

$$FT/RT = 10\%(0\%) / 0(0)$$

12.03.04.0326.14


FL/-/3/-/-/R/-

Scale = .1875"/Ft.

**FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

ITW Building Components Group Inc.

Haines City, FL 33844  
FL COA #0278

[illegible]

TC LL	20.0 PSF	REF	R487-- 12791
TC DL	7.0 PSF	DATE	05/22/13
BC DL	10.0 PSF	DRW	H05R487 13142001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	297003
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UWF487_Z01



Negative reaction(s) of -294# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. Gcpi(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

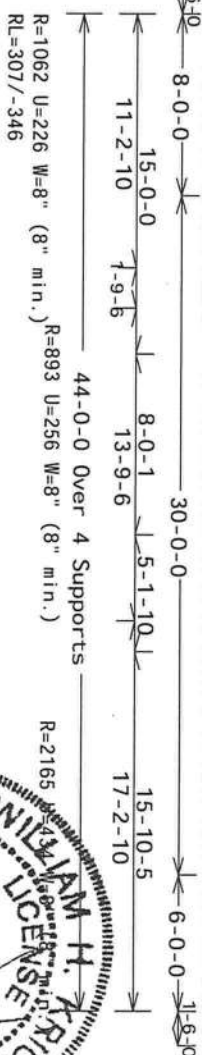
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Collar-tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.



R=70/-295 U=148 W=8" (8" min.)

Design Crit: FBC2010Res/TP1-2007(STB)  
FT/RT=10%(0%)/0(0)

12.03.04.0386.14

FL/-/3/-/-/R/-

Scale = .125"/Ft.

**\*\*IMPORTANT\*\***  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ITW Building Components Group Inc.

Haimes City, FL 33844  
FL COA #0278

[illegible]A circular seal for the State of Florida Professional Engineer. The outer ring contains the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom. The center of the seal features a five-pointed star.

05/22/2013

TC LL	20.0 PSF	REF R487-- 12792
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141013
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 296675
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SLIGHTLY DIFFER FROM THOSE USED

**2 COMPLETE TRUSSES REQUIRED**  
Nail Schedule: 0.131"x3", min. nails  
Top Chord: 1 Row @ 12.00" o.c.

Nail Schedule: 0.131"x3" min. nails  
Top Chord: 1 Row @12.00" o.c.  
Bot Chord: 1 Row @12.00" o.c.  
Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -845# MAX. (See below) from a non-wind load case requires uplift connection.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

\* THIS TRUSS IS DESIGNED WITH A MAXIMUM LOAD OF 20 PSF IN OPENING.



UNIVERSITY OF CALIFORNIA

Design Crit: FBC2010Res/TP1-2007(Std)  
FT/RT=10%(0%)/0(0)

12.03.2024 № 208614/2024 OT

FL--/3/--/--/R/

Scale = 125"/Ft.

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ITW Building Components Group Inc.

Haines City, FL 33844  
FL COA #0278

drawing or cover page listing this drawing. Indicate acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per AISI/TPI 1 Sec.2. For more information see: This job's general notes page: ITB-DCO: [www.itbdcg.com](http://www.itbdcg.com); TPI: [www.tpinatc.org](http://www.tpinatc.org); WTCA: [www.shcindustry.com](http://www.shcindustry.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

05/22/2013

TC LL	20.0 PSF	REF	R487 - - 12793
TC DL	7.0 PSF	DATE	05/22/13
BC DL	10.0 PSF	DRW	HCUSR487 13142002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN -	297008
DUR.FAC.	1.25		
SPACING	SEE ABOVE	JREF -	1UWF487 201



Top chord 2x4 SP\_#1\_12A  
 Bot chord 2x4 SP\_#1\_12A  
 Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

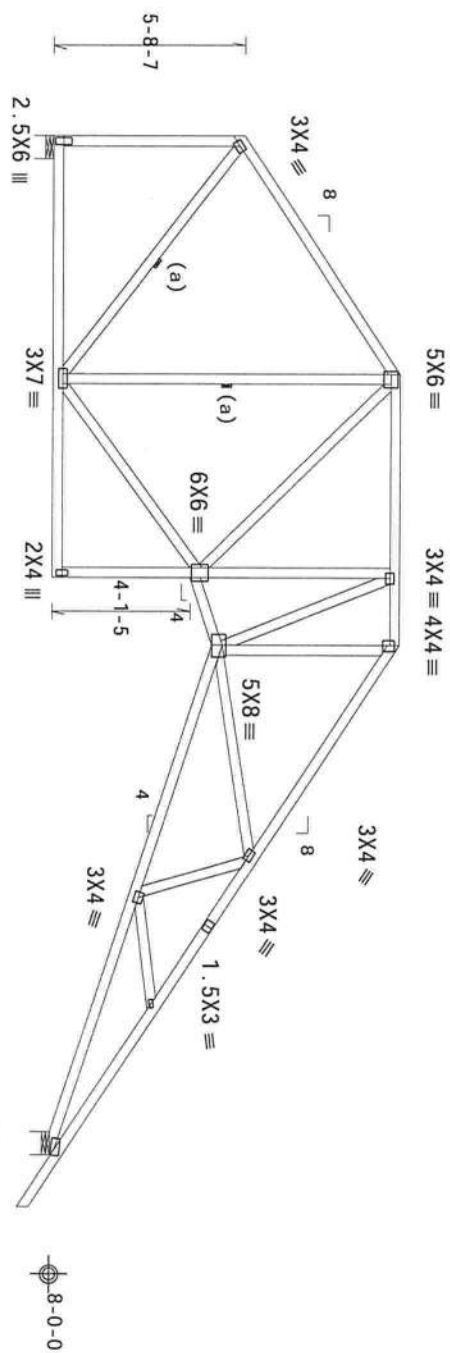
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.16" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



7-0-0 13-0-0 8-0-1 2-0-0 15-0-0 5-8-7 4-1-5 4 1.5X3 3X4 3X4 3X6(B5R) 8-0-0

30-0-0 Over 2 Supports

R=1151 U=110 W=8" (8" min.)  
 RL=237/-277

R=1264 U=118 W=8" (8" min.)

PLT TYP. Wave

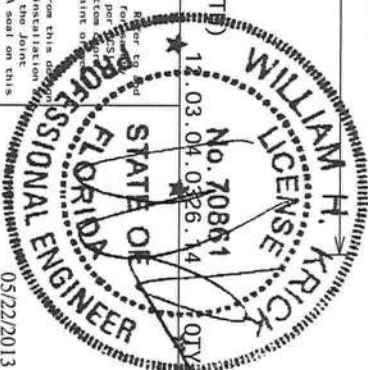
Design Crit: FBC2010Res/TP1-2007(STB) FT/RT=10%(0%)/0(0)



ITW Building Components Group Inc.  
 Gaines City, FL 33844  
 FL COA #0278

**\*\*IMPORTANT\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCS (Building Component Safety Information, by TPI and WTC) for best practices prior to performing these functions. Installers shall provide temporary bracing per BCS. Trusses noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any other design, specification, or code requirement, or for handling, shipping, installing, bracing, or any other work performed on the trusses. Apply plates to each area of truss and purlin bracing of trusses. Refer to drawings 100A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, the suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec. 2. For more information see: This Job's ITW BCG, www.tdeng.com, TPI: www.tpiinc.org, WTC: www.structure.com; ICC: www.iccsafe.org



TC LL	20.0 PSF	REF	R487--	12794
TC DL	7.0 PSF	DATE	05/21/13	
BC DL	10.0 PSF	DRW	HCSR487	13141019
BC LL	0.0 PSF	HC-ENG	JB/WPF	
TOT. LD.	37.0 PSF	SEQN-	296397	
DUR. FAC.	1.25			
SPACING	24.0"	JREF	1UWF487_Z01	

Scale = .1875"/Ft.



top chord	2x4	SP_#1_12A
Bot chord	2x4	SP_#1_12A

Webbs 2x4 SP\_#3\_12A : W1, W2, W8 2x4 SP\_#1\_12A:

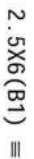
Negative reaction(s) of -571# MAX. (See below) from a non-wind load case requires uplift connection.

Wind loads and reactions based on MMFRS with additional C&C member design.

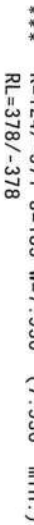
In lieu of structural panels use purlins to brace all flat TC @ 24"

factor for dead load is 1.50.

CVS  
CVS  
CVS



2.5X6(F1) ≡



R=1057 U=109 W=8" (8" min.)

Design Crit: FBC2010Res/TP1-2007(S100)  
FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0)$$

12.03.04:0826.14

QTY: 1 FL/-/3/-/-/R/-

Scale = .1875"/Ft.

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

10

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Those requiring extreme care in fabricating, handling, shipping, installing and bracing are noted as such. The contractor shall coordinate all bracing practices prior to performing those functions. Bracing shall be designed by a professional engineer or other qualified person. No bracing shall have a proprietary attached rigid collar. Locations shown for permanent lateral restraint shall have bracing installed per BECI sections B3, B7 or B10, as applicable.

ITB Building Components Group Inc. (ITBCOG) shall not be responsible for any deviation from this drawing if it results in structural failure due to inadequate bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings T60A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering design and use of this design for any structure is the responsibility of the Building Designer per ASIS/TPI 1. This job was prepared by ITBCOG. www.itbcog.com. TPI : www.tpi.net.org. WCA : www.sdcindustry.com. general notes page: ITB-GCN : www.itbcog.com. TPI : www.tpi.net.org. WCA : www.sdcindustry.com. www.itbcog.com

05/22/2013

TC LL	20.0 PSF	REF	R487 --	12795
TC DL	7.0 PSF	DATE	05/21/13	
BC DL	10.0 PSF	DRW	H05R487	13141004
BC LL	0.0 PSF	HC-ENG	JB/WPF	
TOT.LD.	37.0 PSF	SEQN-	296477	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UWF487	Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT 11, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI (+/-)=0.18

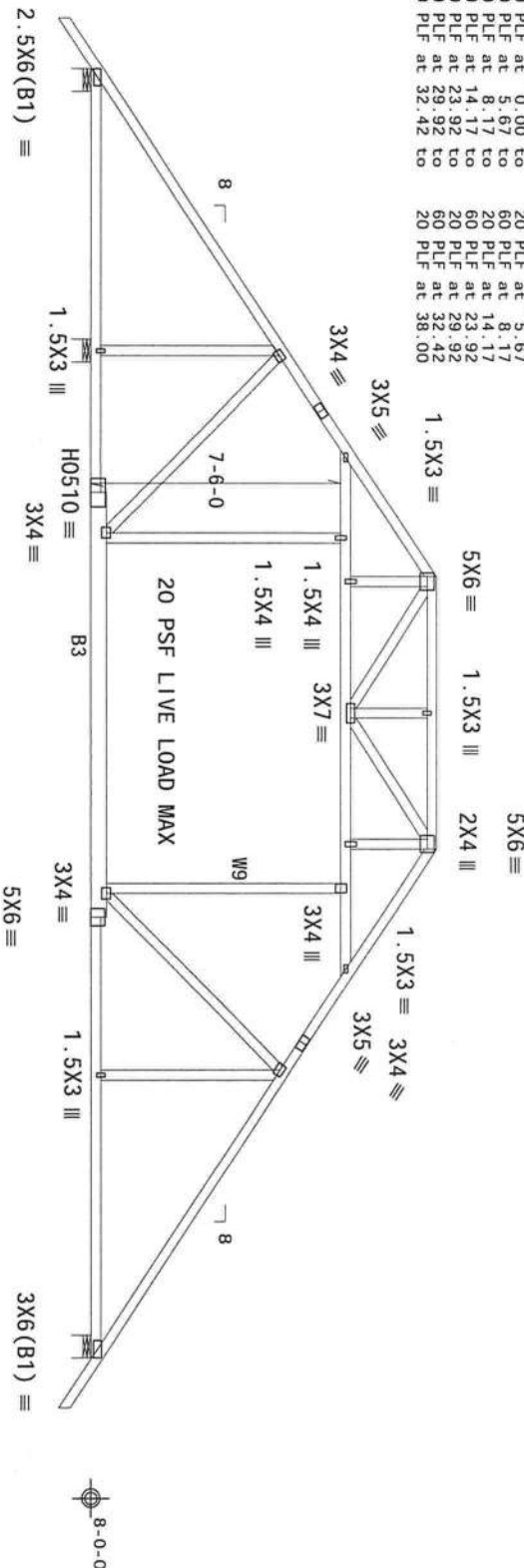
Wind loads and reactions based on MMFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50

Collar-tie braced with continuous lateral bracing at 24" OC

5X6 ≡  
2X4 ≡

 $3 \times 4 =$  $5 \times 6 =$  $3X6(B1) \equiv$ 

9

7

-15-

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---

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30-0-

1

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6-9-11



11-2-10

•

1-3-6

103

14-3-

1003

→

3

11-2-10

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Y

R=1406 U=176 W=7.936" (7.936" min.)  
RL=346/-346 R=562 Rw=659 U=377 W=8" (8" min.)

Design Crit: FBC2010Res/TP1-2007(STB)

PLT TYP. 20 Gauge HS, Wave

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

**ITW Building Components Group Inc.**

Haines City, FL 33844  
FL COA #0278

Tenues require extreme care in fabricating, handling, shipping, installing and bracing. Refer to Building Component Safety Information by TPI and WCA for detailed instructions regarding practices prior to performing those functions. Installations shall provide temporary bracing per AISI/TIP-1 Section 7.6. If all other provisions are followed, no additional lateral restraint or bracing is needed per BCSI sections D3, E7 or F10, as applicable.

The Building Components Group Inc. (TBGCO) shall not be responsible for any deviation from this design if failure to build the truss, in conformance with ANSI/TPI-1, or for handling, shipping, installation, erection, erection details, unless noted otherwise. Refer to drawings TBGR-2 for standard plate positions. A seal on this drawing or cover page indicating this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per AISI/TPI-1 Sec 2.2. For more information see: This Job # \_\_\_\_\_ general notes page; ITB-BCS: www.tbcs.org; TB-TBI: www.tbiinc.com; WTCA: www.abctindustry.com

CC - zero / iStockphoto.org

05/22/2013

FL/-3/-/-R/-		Scale = .1875"/FC
TC LL	20.0 PSF	REF R487 -- 12796
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141016
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 296616
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



Top chord 2x4 SP #1-12A : TS 2x4 SP M-30:  
Bot chord 2x4 SP #2-12A : TS 2x4 SP M-30:  
Bot chord 2x4 SP #3-12A : TS 2x4 SP M-30:  
Webs 2x4 SP #3-12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

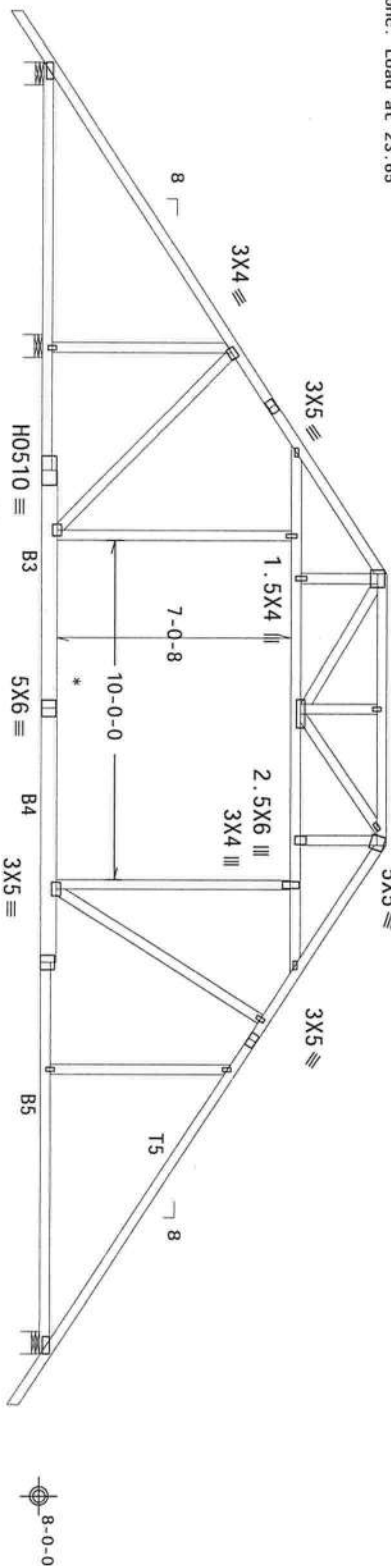
MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

### SPECIAL LOADS

----- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)  
TC - From 100 PLF at -1.50 to 100 PLF at 0.00  
TC - From 92 PLF at 0.00 to 92 PLF at 38.00  
TC - From 100 PLF at 38.00 to 100 PLF at 39.50  
BC - From 33 PLF at 0.00 to 33 PLF at 5.67  
BC - From 98 PLF at 5.67 to 98 PLF at 8.17  
BC - From 33 PLF at 8.17 to 33 PLF at 14.17  
BC - From 98 PLF at 14.17 to 98 PLF at 24.00  
BC - From 33 PLF at 24.00 to 33 PLF at 38.00  
BC - 272 LB Conc. Load at 4.06  
BC - 681 LB Conc. Load at 23.65

5X6 ≡

1.5X4 ≡ 3X10 ≡

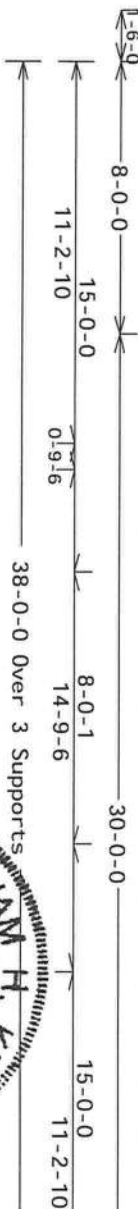


2.5X6(B1) ≡

3X4 ≡

5X5 ≡

2.5X6(B1) ≡



R=2937 U=551 W=8" (8" min.)  
R=346/-346 R=920 U=319 W=8" (8" min.)

R=3175 U=512 W=8" (8" min.)

Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2010Res/TP1-2007 (STB)  
FT/RT=10%(0%)/0(0)

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. For the latest edition of BCSI (Building Component Safety Information), by TPI and WTC, for details, unless noted otherwise. Apply plates to each face of truss and position as shown on this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's IBC, www.iccsafe.org

ALPINE

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Haines City, FL 33844

FL COA #0278

## 2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3", min. nails  
Top Chord: 1 Row @ 12.00" o.c.  
Bot Chord: 1 Row @ 12.00" o.c.  
Webs: 1 Row @ 4" o.c.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

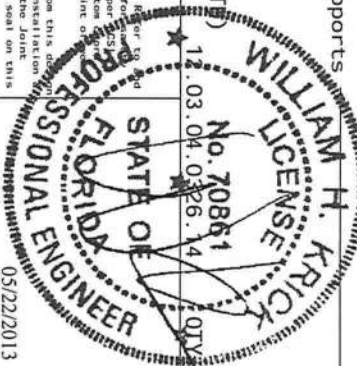
130 mph wind, 15.00 ft. mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft. from roof edge, RISK CAT 11, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI (+/-)=0.18

Wind loads and reactions based on MMFRS.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

\* THIS TRUSS IS DESIGNED WITH A MAXIMUM LOAD OF 20 psf IN OPENING.



TC LL	20.0 PSF	REF R487-- 12797
TC DL	7.0 PSF	DATE 05/22/13
BC DL	10.0 PSF	DRW HCUR487 13142003
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT. LD.	37.0 PSF	SEQN- 29/013
DUR. FAC.	1.25	
SPACING	SEE ABOVE	JREF- 1UWF487_Z01

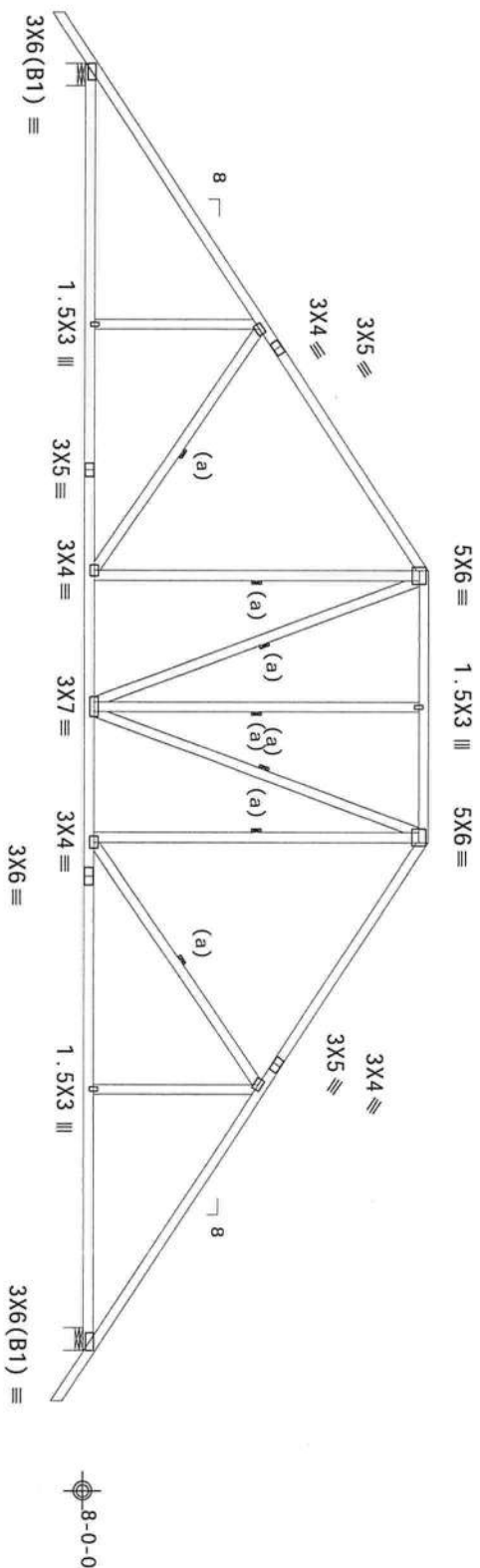
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.



$L \leq 9$   
 15-0-0  
 8-0-1  
 15-0-0  
 38-0-0 Over 2 Supports  
 $R=1635 \ U=356 \ W=8'' \ (8'' \text{ min.})$   
 $R=1636 \ U=356 \ W=8'' \ (8'' \text{ min.})$   
 $RL=346/-346$

Scale = .1875"/Ft.

Those who require more care in fabricating, handling, shipping, installing and bracing. For example, follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTC) for bracing practices noted or for performing these functions. Installers shall provide temporary bracing per manufacturer's instructions. Bracing shall be installed in accordance with the manufacturer's instructions. Bracing shall have a properly attached rigid collar. Locations shown for permanent lateral restraint of BCSI sections D3, B9 or B10, as applicable.

**ITW Building Components Group Inc.**

Haines City, FL 33844  
FL COA #0278

the responsibility of the Building Designer per ANSI/FP 1 Sec. 2. For more information see: This job's general notes page; ITB-BGC: [www.itb-bgc.com](http://www.itb-bgc.com); TPI: [www.tpinet.org](http://www.tpinet.org); WTCIA: [www.sbcindustry.com](http://www.sbcindustry.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

TC LL	20.0 PSF	REF R487-- 12798
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUR487 13141017
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294747
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A  
Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(H1) = (J) Hanger not calculated (1)2x4 SP\_#1\_12A supporting member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

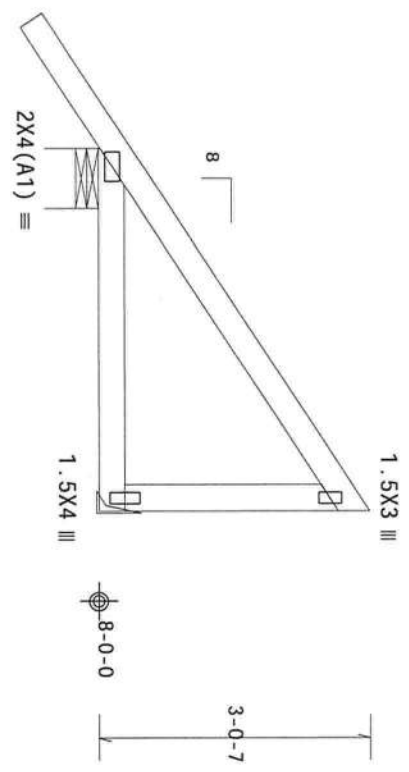
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



← 1-6-0 →

← 4-0-0 Over 2 Supports →

R=266 U=46 W=8" (8" min.)  
RL=104/-57

R=128 U=50  
H=H1

PLT TYP. Wave

Design Crit: FBC2010Res/TP1-2007(SFD)  
FT/RT=10%(0)/0(0)

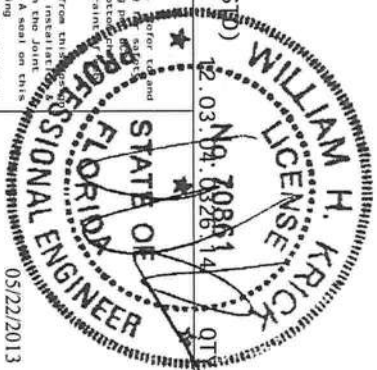
**\*\*IMPORTANT\*\*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, unloading, installing, bracing and erecting. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) practices prior to performing these functions. Installers shall provide temporary bracing and bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise. Refer to drawings 160A.2 for standard plate positions. A seal on this drawing or cover page listing this design. The suitability and use of this design for any structure is the responsibility of the Building Designer per ASCE/TP1 1 Sec 2. For more information see: This Job's drawings and page 17B BCSI, www.bcsi.org, TPI, www.tpi.org, WCA, www.wcaindustry.com.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844  
FL COA #0 278



TC LL	20.0 PSF	REF R487-- 12799
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUR487 13141020
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294763
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01

Scale =.5"/Ft.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, Exp C, wind TC DL=3.5 psf wind BC DL=5.0 psf, Gcpi (+/-)=0.18

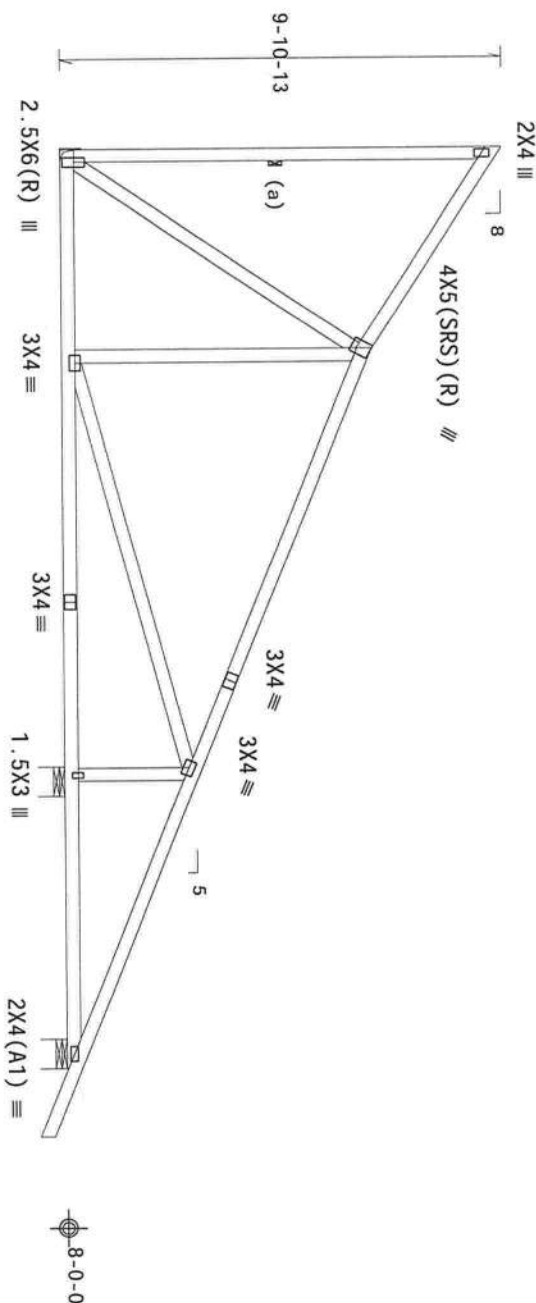
wind BC DL=5.0 psf.  $G_{Cpi}(+/-)=0.18$

Wind loads and reactions based on MWFRS with additional C&C member design.

(H1) = (J) Hanger not calculated (1)2x4 SP\_#1\_12A supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Bottom chord checked for 10.00 psf non-concurrent live load



R=341 U=64 W=8" (8" min.)

R=508 U=200  
RL=65/-290  
H=H1

PLT TYP. Wave

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
**\*\*IMPORTANT\*\*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Tenants, require extensive care in fabricating, handling, shipping, installing and bracing. For each building, the contractor shall submit a bracing plan to the building owner. To follow the latest edition of BCSP (Building Component Safety Information, by TPI and WCA) practices used or to performing these functions. Installers shall provide temporary bracing per BCSP and other codes. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ALPINE

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Haines City, FL 33844  
FL COA #0278

ICC: [www.iccnatf.org](http://www.iccnatf.org)

Design Crit: FBC2010Res/TP1-2007(STP)  
FT/RT=10%(0%)/0(0)

No. 7086  
12.03.04.0326.1

QTY 4

FL/-/3/-/-/R/-

Scale = .25"/Ft.

3

1

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Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A  
Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(H1) = (J) Hanger not calculated (1)2x4 SP 2850F-2.3E supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

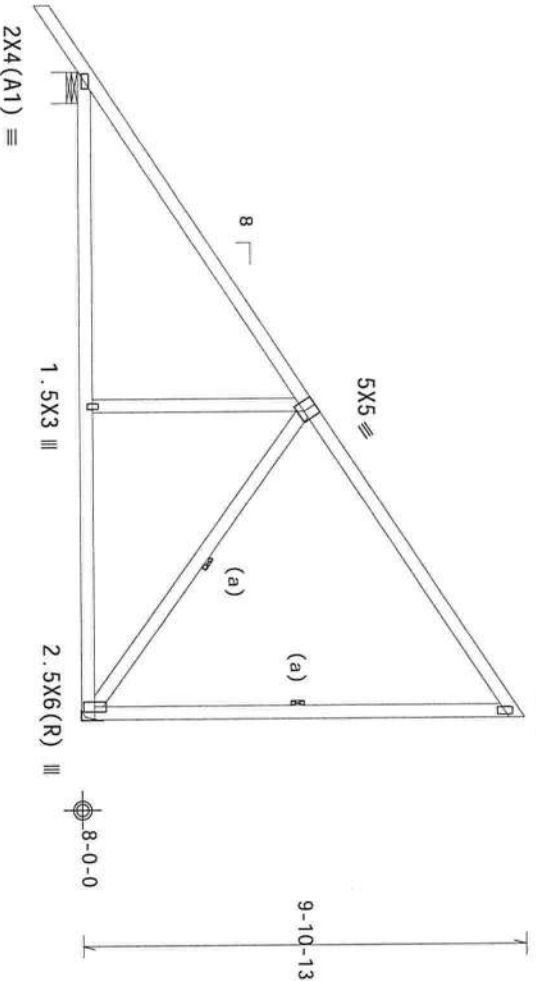
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18  
Wind loads and reactions based on MMFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2X4 III



R=537 U=112  
H=H1

14-3-8 Over 2 Supports  
R=653 U=8 W=8" (8" min.)  
RL=200/-117

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)  
FT/RT=10%(0%)/0(0)

\*\*IMPORTANT\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and WICA) for practices prior to performing those functions. Installers shall provide a written copy of the BCSI manual to the customer. Trusses shall be installed in accordance with the BCSI manual. Trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (TBCGI) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. This drawing is not to be used for any other structure. For more information see: This Job's general notes page: TPI-BCSI: www.tpiinc.com; TPI: www.tpiinc.org; WICA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE

TPI Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



FL/-/3/-/-/R/-	Scale = .25"/Ft.
TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT. LD.	37.0 PSF
DUR. FAC.	1.25
SPACING	24.0"
JREF	1UWF487_201

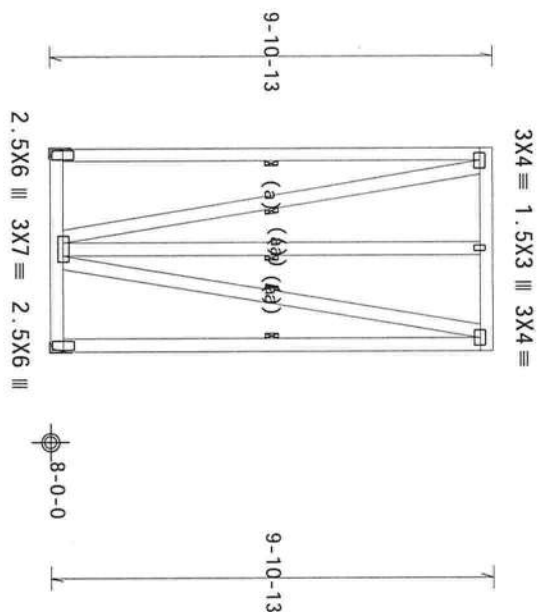
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Webs 2x4 SP\_#3\_12A

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.



4-6-0 Over 2 Supports

R=652 U=261  
R=652 U=261

PLT TYP. Wave


Design Crit: FBC2010Res/TP1-2007(ST0)  
FT/RT=10%(0%)/0(0)

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
**\*\*IMPORTANT\*\*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require struts, care in fabricating, handling, shipping, installing and bracing. For more information on the design and construction of trusses, see the following articles in the latest edition of *BCS1 (Building Component Safety Information)* by TPI and WCA: "Practices noted for performing these functions," "Installers shall provide temporary bracing per the manufacturer's instructions," "Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling," "Locations shown for permanent lateral restraint shall have bracing installed per *BCS1* sections B3, B or B10, as applicable."

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or

Apply please to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard gable positions. Drawing 160C covers the remaining gable positions. The suitability and use of this design for any structure is the responsibility of the building designer, per AISI/TPI 1 Sec. 2. For more information see: This job's general notes page: 17B-DEG; [www.ltdbng.com](http://www.ltdbng.com); TPI: [www.tpiinc.org](http://www.tpiinc.org); WTCA: [www.steelindustry.org](http://www.steelindustry.org); CCI: [www.cciinc.org](http://www.cciinc.org)



**ALPINE**

**ITW Building Components Group Inc.**  
 Haines City, FL 33844  
 FL COA #0 278

Special loads

TC	From	To	54 plf at
-----	(Lumber	Dur. Fac.=1.25 /	Plate Dur. Fac.=1.25)
-----	54 plf at	0.00	to 54 plf at 4.50

BC- From	10 pif at 0.00 to	10 pif at 4.50
10 pif at 0.00 to	10 pif at 4.50	

BC- 507.98 lb Conc. Load at 1.81, 2.69

130 mph wind, 17.90 ft mean hgt, ASCE 7-10, CLOSED bldg, not located

within 4.50 ft from roof edge,  $K_{15}$   
wind BC  $D_L=5.0$  psf,  $G C p_i (+/-)=0.18$

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

12.03.04.0126.14 QTY: 1 FL/-/3/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0 PSF	REF R487-- 12802
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141011
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294770
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1UWF487_Z01



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 SP\_#3\_\_12A

Special loads	Dur.	Fac.=1.25 /	Plate Dur.	Fac.=1.25)
-----Lumber				
TC- From	54 pif at	0.00 to	54 pif at	4.50
BC- From	10 pif at	0.00 to	10 pif at	4.50
BC- 537.47	1b Conc.	Load at	1.81,	2.69

130 mph wind, 17.90 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT 11, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

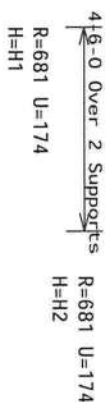
(H1) = (J) Hanger not calculated (2)2x4 SP\_#1\_12A supporting member.  
(H2) = (J) Hanger not calculated (2)2x4 SP\_#1\_12A supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets  $L/240$  live and  $L/180$  total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

Design Crit: FBC2010Res/TP1-2007(STP)  
FT/RT=10%(0%)/0(0)

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

**ITW Building Components Group Inc.**

Haines City, FL 33844  
FL COA #0278

Those requiring access care in fabric testing, handling, shipping, installing and bracing, refer to the following edition of BCSI (Building Component Safety Information, by TPI and WITA) for the practices prior to performing these functions. Insulators shall provide temporary bracing prior to the installation of the insulation. Insulators shall provide temporary bracing and bolting for the insulations noted otherwise. Top chord shall have properly attached structural sheathing and bolting. Insulators shall have a properly attached lateral bracing system. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections 83, 89 or 910, as applicable.

1TW Building Components Group Inc. (1TWBCG) shall not be responsible for any deviation from this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the Building Designer. For more information see: This Job's general notes page: TPI-BCSI: [www.1twbcg.com](http://www.1twbcg.com); TPI: [www.tpinet.org](http://www.tpinet.org); WITA: [www.abnindustry.com](http://www.abnindustry.com); CC: [www.1twbcg.org](http://www.1twbcg.org)

Seal of the State of Florida Professional Engineer  
 WILLIAM H. KRICK  
 LICENSE  
 No. 70861-14  
 12/03/04  
 EXPIRATION DATE  
 STATE OF FLORIDA  
 PROFESSIONAL ENGINEER

FL/-/3/-/-/R/-		Scale = .25"/Ft.
TC LL	20.0 PSF	REF R487-- 12803
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCURS487 13141014
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN- 294740
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01

Top chord 2x4 SP\_#1-12A  
Bot chord 2x4 SP\_#1-12A  
Webs 2x4 SP\_#3-12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

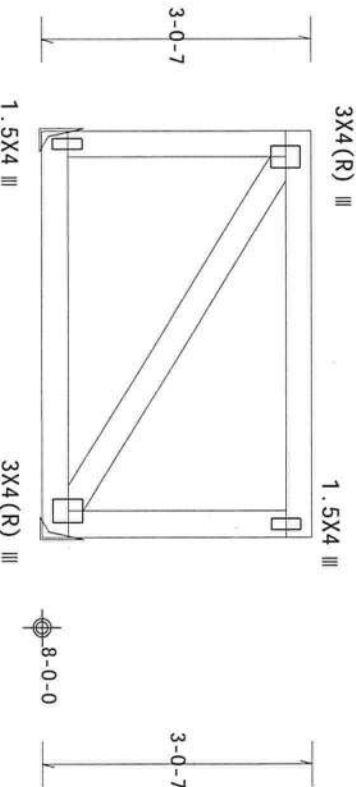
Wind loads and reactions based on MMFRS.

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

Special loads  
-----  
Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25  
TC-From 54 pif at 0.00 to 54 pif at 4.50  
BC-From 10 pif at 0.00 to 10 pif at 4.50  
BC- 128.43 lb Conc. Load at 1.81, 2.69  
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18  
Bottom chord checked for 10.00 psf non-concurrent live load.  
Truss must be installed as shown with top chord up.



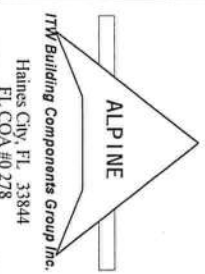
4-6-0 Over 2 Supports  
R=272 U=102  
R=272 U=102

PLT TYP. Wave

Design Cr't: FBC2010Res/TP1-2007(STB)  
FT/RT=10%(0%)/0(0)

\*\*\*WARNING\*\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Before installation, the truss manufacturer must be consulted for proper bracing and installation practices for the specific truss design. The truss manufacturer must be consulted for proper bracing and installation practices for the specific truss design. The truss manufacturer must be consulted for proper bracing and installation practices for the specific truss design.



ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TP1 1, or for handling, shipping, installation or any other failure of the truss. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see: general notes page: ITW BCG: www.itwbcg.com; TP1: www.tp1inc.org; WICA: www.decindustry.com; ICC: www.iccsafe.org



TC LL	20.0 PSF	REF	R487-- 12804
TC DL	7.0 PSF	DATE	05/21/13
BC DL	10.0 PSF	DRW	HCUSR487 13141009
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT. LD.	37.0 PSF	SEQN-	294766
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UWF487_Z01



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

130 mph win 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCP(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

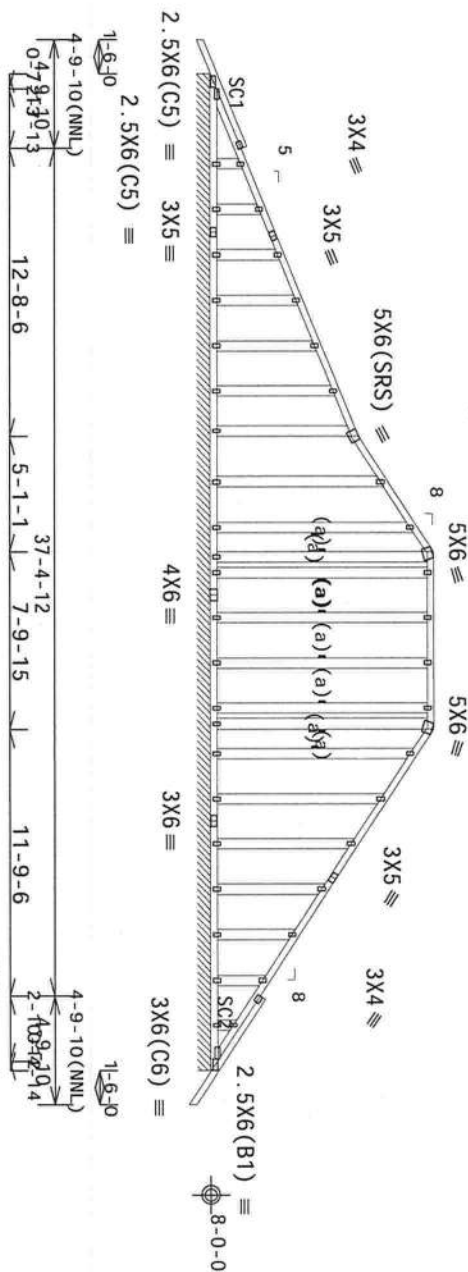
Truss spaced at 24.0" OC designed to support 3-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=282 PLF U=79 PLF W=44-0-0  
RL=28/-25 PLF

Note: All Plates Are 2X4 Except As Shown

Design Crit: FBC2010Res/TP1-2007(Std)

PLT TYP. Wave

**FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

**ITW Building Components Group Inc.**

Haines City, FL 33844  
FL COA #0278

Trusses, require someone care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) practices prior to performing these functions. Installers shall provide temporary bracing prior to erection, unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid wall section. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

11W Building Components Group, Inc. (11WBCEG) shall not be responsible for any deviation from this document by failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Drawing or cover joist listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's general notes page: 01W-BCSI: [www.bcsig.com](http://www.bcsig.com); TPI: [www.tpi.net.org](http://www.tpi.net.org); WCA: [www.abcdirect.com](http://www.abcdirect.com).

05/22/2013

FL/-3/-/-R/-	Scale = .125"/Ft.
TC LL 20.0 PSF	REF R487-- 12805
TC DL 7.0 PSF	DATE 05/21/13
BC DL 10.0 PSF	DRW HCURSR487 13141007
BC LL 0.0 PSF	HC-ENG JB/WPF
TOT.LD. 37.0 PSF	SEON- 294842
DUR.FAC. 1.25	
SPACING 24.0"	JREF- 1UWF487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

5X5 =

(a) Continuous lateral bracing equally spaced on member.

Design Crit: FBC2010Res/TP1-2007(STU)

$$FT/RT=10\%(0\%)/0(0)$$

12.03.04.0326.14


FL--/3/--/R/-

Scale = .1875"/Ft.

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS SHEET!  
**\*\*IMPORTANT\*\*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

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FL COA #0278

[illegible]

TC LL	20.0 PSF	REF R487 -- 12806
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141002
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEON- 294752
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



( 13-166--Erkinger Home Builders Miller Residence -- Lot #29 - A20 20'4" Common )

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1\_12A  
Bot chord 2x10 SP #2\_12A  
Webs 2x10 SP #2\_12A : W2 2x4 SP #3\_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

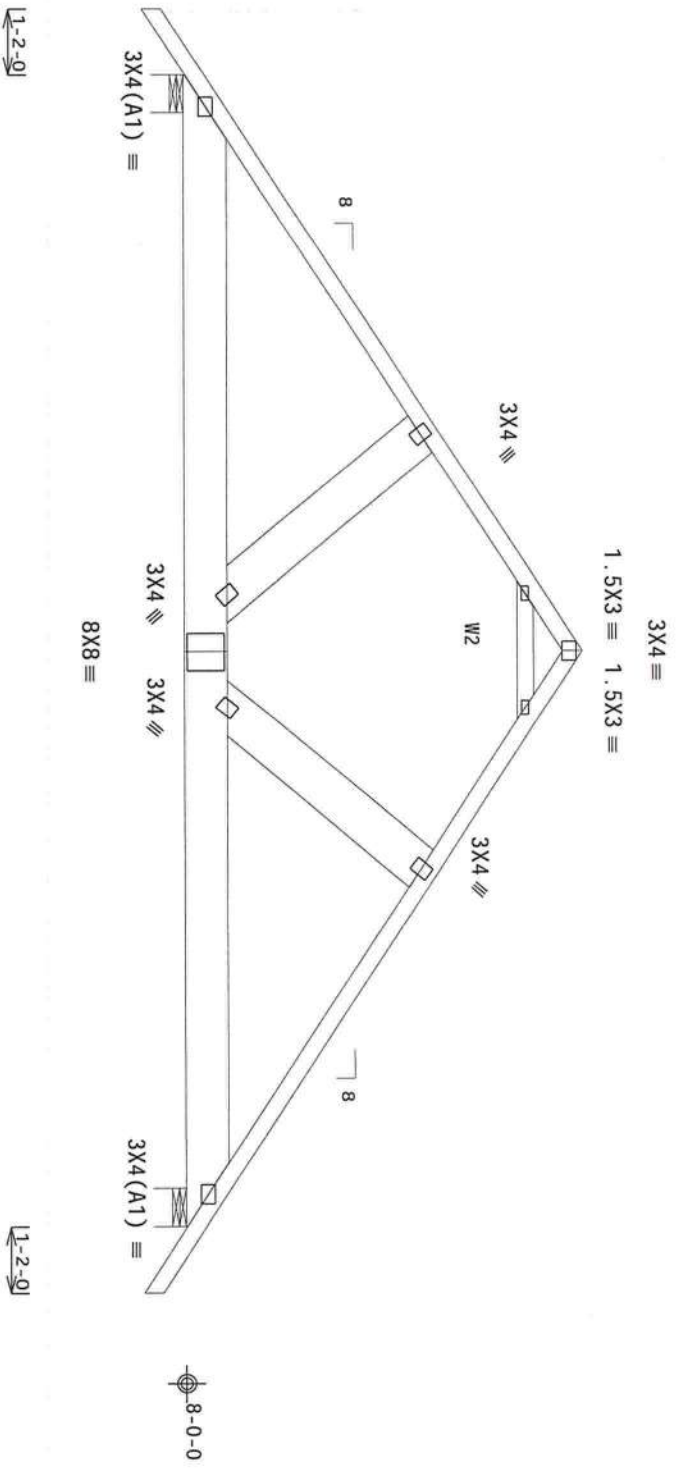
Calculated horizontal deflection is 0.07" due to live load and 0.20" due to dead load.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN CLEAR bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCFI (+/-)=0.00

Wind loads and reactions based on MMFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

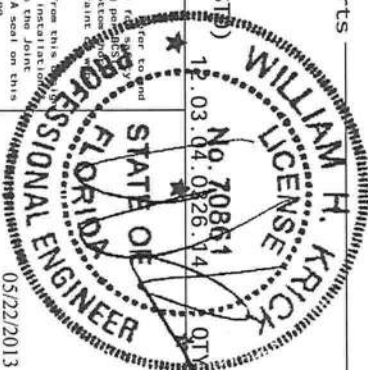
Design Crit: FBC2010Res/TP1-2007(STB)  
FT/RT=10%(0%)/0(0)

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET.  
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for all practices prior to performing these functions. Installers shall provide temporary bracing and bracing of trusses. Apply bracing to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see: ICC- www.iccsafe.org

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FL COA #0278



TC LL	20.0 PSF	REF	RA87--	12807
TC DL	7.0 PSF	DATE	05/21/13	
BC DL	10.0 PSF	DRW	HCUSR487	13141022
BC LL	0.0 PSF	HC-ENG	JB/WPF	
TOT. LD.	37.0 PSF	SEQN-	296785	
DUR. FAC.	1.25			
SPACING	24.0"	JREF-	1UWF487_Z01	

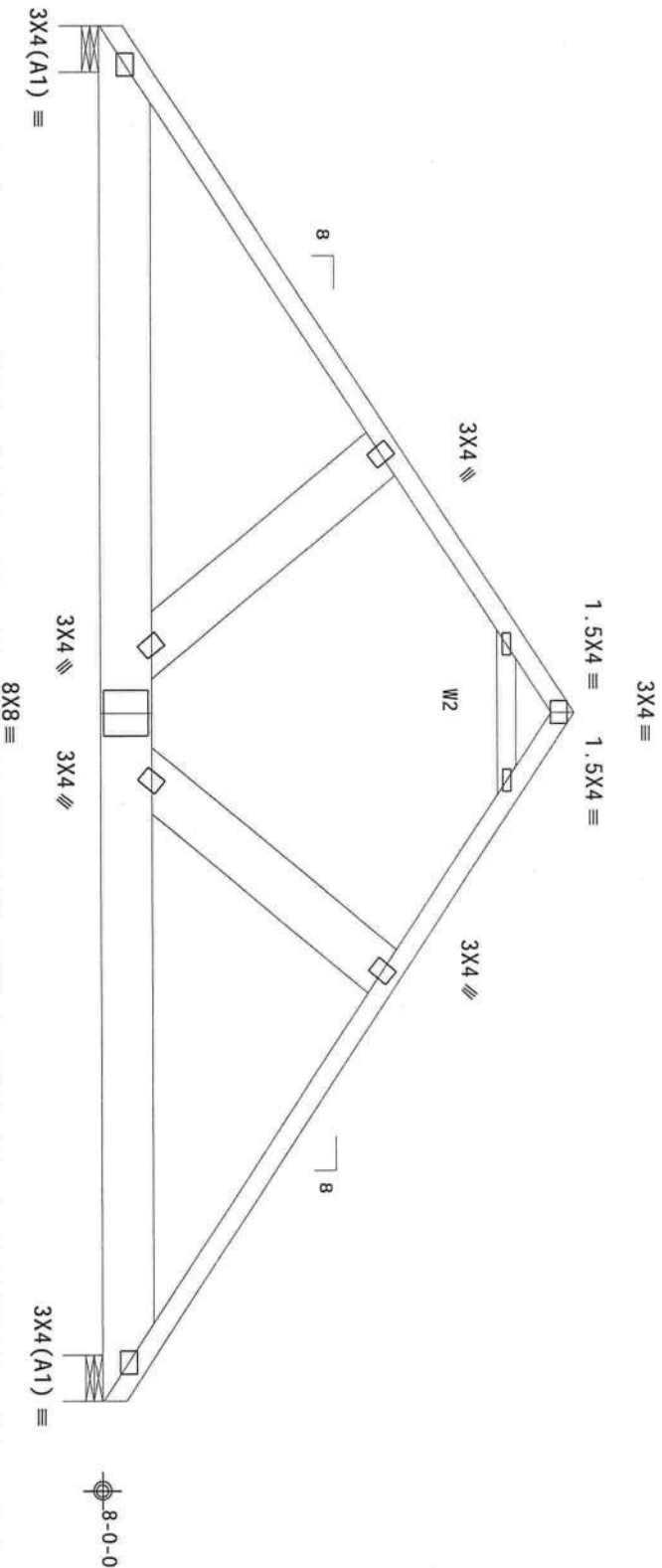
Top chord 2x4 SP\_#1\_12A  
Bot chord 2x10 SP\_#2\_12A  
Webs 2x10 SP\_#2\_12A : W2 2x4 SP\_#3\_12A:  
Lumber grades designated with "12A" use design values approved  
1/5/2012 by ALSC.  
Calculated horizontal deflection is 0.12" due to live load and 0.16"  
due to dead load.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, OPEN CLEAR bldg, Located  
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC  
DL=5.0 psf, GCPI(+/-)=0.00

Wind loads and reactions based on MWFRS with additional C&C member  
design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase  
factor for dead load is 1.50.



10'-2'-0" 20'-4'-0" Over 2 Supports 10'-2'-0" 8'-0'-0"

R=781 U=70 W=8" (8" min.)  
RL=182/-182

R=781 U=70 W=8" (8" min.)

PLT TYP. Wave

Design Crit: FBC2010Res/TP1-2007(ST)  
FT/RT=10%(0%)/0(0)

No. 70861  
12.03.04.03.06.14

FL/-/3/-/-/R/-

Scale = .375"/Ft.

\*\*\*IMPORTANT\*\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety) Information, by TPI and WTC, for detailed instructions on proper installation and bracing. Trusses must be installed in accordance with the manufacturer's instructions. Trusses must be installed in accordance with the manufacturer's instructions. Trusses must be installed in accordance with the manufacturer's instructions.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure of trusses, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information see: www.itwbcg.com; TPI: www.tprint.org; WTC: www.theindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



TC LL	20.0 PSF	REF R487--	12808
TC DL	7.0 PSF	DATE	05/21/13
BC DL	10.0 PSF	DRW	HCSR487 13141003
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT. LD.	37.0 PSF	SEQN-	296793
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1UWF487_Z01





Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A  
Webs 2x4 SP\_#3\_12A

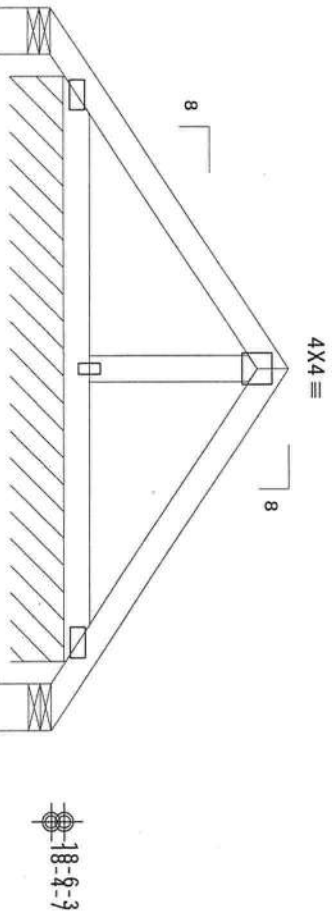
Lumber grades designated with "12A" use design values approved  
1/5/2012 by ALSC.

Wind loads and reactions based on MMFRS with additional C&C member  
design.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Refer to DWG PB160100212 for piggyback details.

Special loads  
-----Lumber  
TC- From 57 pif at 0.00 to 57 pif at 4.00  
TC- From 57 pif at 4.00 to 57 pif at 8.00  
BC- From 4 pif at 0.00 to 4 pif at 8.00  
130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, Located  
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC  
DL=2.0 psf, GCpl(+/-)=0.18  
Deflection meets L/240 live and L/180 total load. Creep increase  
Factor for dead load is 1.50.



R=2 Rw=61 U=69 W=6.31" (6.31" min.)  
R=72/-72  
R=76 PLF U=41 PLF W=6-5-7

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(SD)  
FT/RT=10%(0%)/0(0)

R=2 Rw=61 U=69 W=6.31" (6.31" min.)

\*\*\*IMPORTANT\*\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) Information, by TPI and WIDA practices prior to performing these functions. Installers shall provide temporary bracing for all trusses until they are permanently braced. Trusses shall be braced in accordance with the BCSI (Building Component Safety) Information, by TPI and WIDA. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility for the design shown. The seal of the engineer and the use of this design for any structure is the responsibility of the user. This drawing is not to be used for any other purpose. This Job's general notes page: ITW BCSI: www.bcsi.org; TPI: www.tpinet.org; WIDA: www.widaindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



TC LL	20.0 PSF	REF R487-- 12810
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141008
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294925
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A  
Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved  
1/5/2012 by ALSC.

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC- From 57 pif at 0.00 to 57 pif at 4.00  
TC- From 57 pif at 4.00 to 57 pif at 8.00  
BC- From 4 pif at 0.00 to 4 pif at 8.00

Wind loads and reactions based on MWFRS with additional C&C member design.

Refer to DWG PB160100212 for piggyback details.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" min. nails

Top Chord: 1 Row @12.00" o.c.

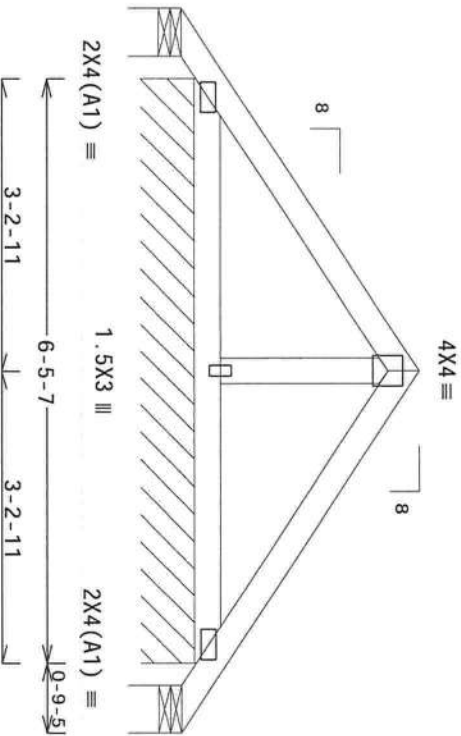
Bot Chord: 1 Row @12.00" o.c.

Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails  
in each row to avoid splitting.  
130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, not located  
within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf,  
wind BC DL=2.0 psf. Gcpl(+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase  
factor for dead load is 1.50.



R=2 R<sub>w</sub>=61 U=69 W=6.31" (6.31" min.)  
R<sub>L</sub>=72/-72

R=76 PLF U=41 PLF W=6-5-7

Design Cr-ic: FBC2010Res/TP1-2007(SUB)  
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

\*\*IMPORTANT\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and WFL) practices prior to performing these functions. Truss fabricators shall provide temporary bracing prior to erection and shall have a properly attached rigid ceiling locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

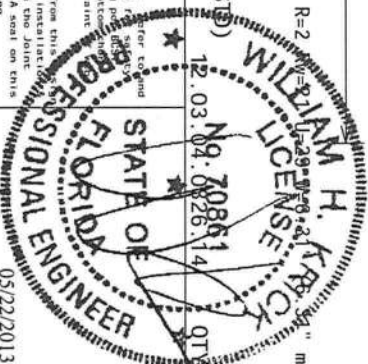
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this drawing or for any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. This drawing shall be used for the design of any structure in accordance with the ITWBCG design manual and ANSI/TPI 1. For more information on any structure in general, please refer to ITWBCG design manual and ANSI/TPI 1. This job's general notes page: ITWBCG design manual, TPI: www.tpi.org WFL: www.wflindustry.com. ITC: www.itcinfo.org

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



TC LL	20.0 PSF	REF R487-- 12811
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCURS487 13141012
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294927
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01

Top chord 2x4 SP\_#1\_12A  
Bot chord 2x4 SP\_#1\_12A  
Webs 2x4 SP\_#3\_12A

Lumber grades designated with "12A" use design values approved  
1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member  
design.

Gable end supports 8" max rake overhang.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Refer to DWG PB160100212 for piggyback details.

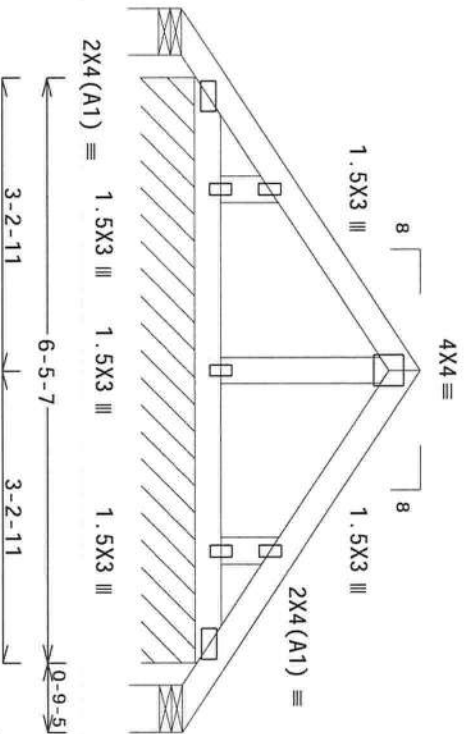
Special loads

-----Lumber  
TC-From 57 pif at 0.00 to 57 pif at 4.00  
TC-From 57 pif at 4.00 to 57 pif at 8.00  
BC-From 4 pif at 0.00 to 4 pif at 8.00

130 mph wind, 19.70 ft mean hgt, ASCE 7-10, CLOSED bldg, located  
anywhere in roof, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC  
DL=2.0 psf, GCPI(+/-)=0.18

See DWGS A14030ENC100212 & GBLLET1N0212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase  
factor for dead load is 1.50.



R=18 Rw=42 U=38 W=6.31" (6.31" min.)  
R=72/-72 R=65 PLF U=31 PLF W=6-5-7

PLT TYP. Wave

Design Crt: FBC2010Res/TPI-2007(STB)  
FT/RT=10%(0%)/0(0)

\*\*IMPORTANT\*\* READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) Information, by TPI and WTC, for details prior to performing these tasks. Trusses shall be properly braced and bracing shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this drawing or any failure of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering. The responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see: This Job's ICC: www.iccinfo.org. TPI: www.tpiinc.org. WTC: www.theindustry.com.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844  
FL COA #0 278



TC LL	20.0 PSF	REF R487-- 12812
TC DL	7.0 PSF	DATE 05/21/13
BC DL	10.0 PSF	DRW HCUSR487 13141001
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT. LD.	37.0 PSF	SEQN- 294926
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1UWF487_Z01



# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

## NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY TO SCAB TO EACH FACE OF WEB.

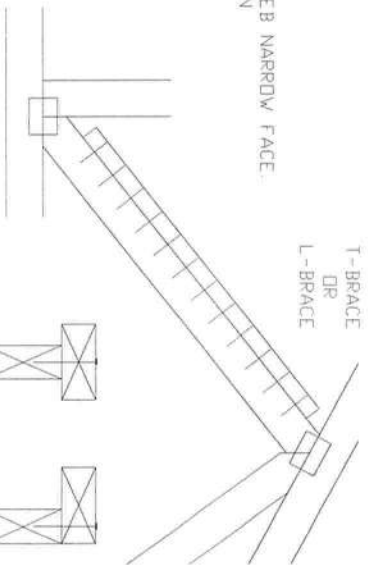


Building Components Group Inc.

Earth City, MO 63045

T-BRACING  
OR  
L-BRACING:

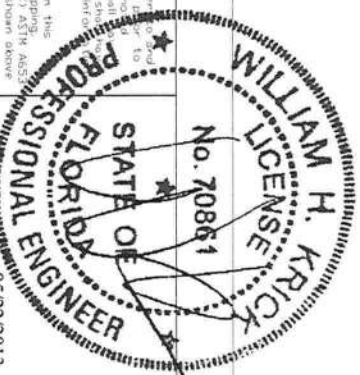
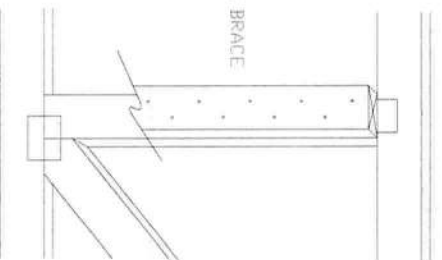
APPLY TO EITHER SIDE OF WEB NARROW FACE.  
ATTACH WITH 10d BOX OR GUN  
(0.128" x 3" MIN) NAILS.  
AT 6" O.C.  
BRACE IS A  
MINIMUM 80% OF WEB  
MEMBER LENGTH



SCAB BRACING:

APPLY SCABS TO WIDE FACE OF WEB.  
NO MORE THAN (1) SCAB PER FACE.  
ATTACH WITH 10d BOX OR GUN  
(0.128" x 3" MIN) NAILS.  
AT 6" O.C.  
BRACE IS A MINIMUM  
80% OF WEB MEMBER LENGTH

SCAB BRACE



May 22 '13

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

# Gable Stud Reinforcement Detail

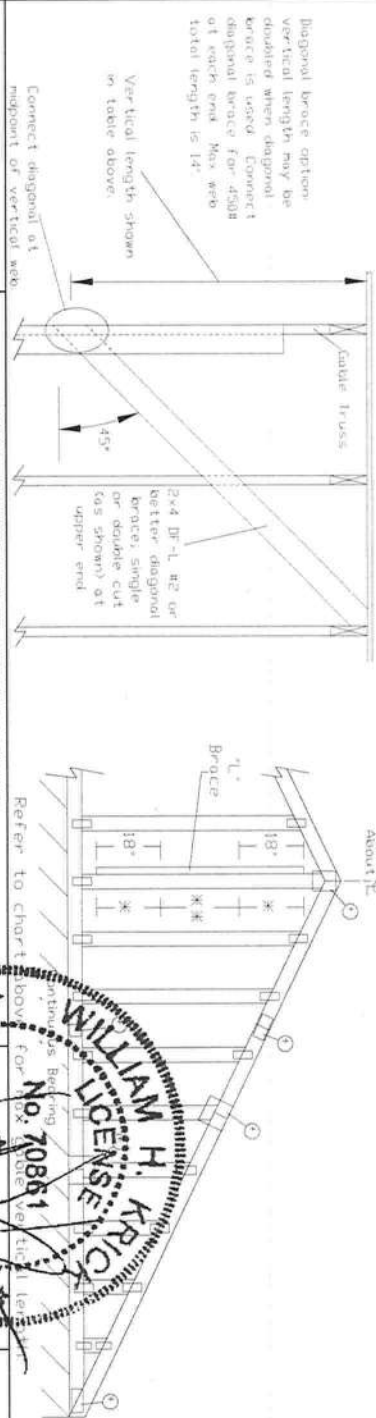
ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr. 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00  
 Dr. 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00  
 Dr. 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length																
Gable Vertical Spacing	2x4 Species	Brace	No Braces	(1) 1x4 L <sup>1</sup> Brace *				(1) 2x4 L <sup>1</sup> Brace *				(1) 2x6 L <sup>1</sup> Brace *			(2) 2x6 L <sup>1</sup> Brace *	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B			
12" o.c.	SPF	#1 / #2	4'-3"	7'-3"	7'-7"	8'-7"	8'-11"	10'-3"	10'-8"	13'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#3	4'-1"	6'-7"	7'-1"	8'-6"	8'-10"	10'-1"	10'-6"	13'-4"	13'-10"	14'-0"	14'-0"	14'-0"		
		Stud	4'-1"	7'-2"	7'-5"	8'-6"	8'-10"	10'-1"	10'-6"	13'-4"	13'-10"	14'-0"	14'-0"	14'-0"		
		Standard	4'-1"	6'-11"	7'-5"	8'-6"	8'-10"	10'-1"	10'-6"	13'-4"	13'-10"	14'-0"	14'-0"	14'-0"		
		#1	4'-4"	7'-4"	7'-7"	8'-8"	9'-0"	10'-3"	10'-8"	13'-7"	14'-0"	14'-0"	14'-0"	14'-0"		
		#2	4'-3"	7'-3"	7'-7"	8'-7"	8'-11"	10'-3"	10'-8"	13'-6"	14'-0"	14'-0"	14'-0"	14'-0"		
	DFL	Stud	4'-1"	5'-11"	6'-4"	7'-11"	8'-5"	10'-1"	10'-6"	12'-5"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	
		Standard	3'-11"	5'-2"	5'-5"	6'-10"	7'-4"	9'-3"	9'-11"	10'-9"	11'-6"	14'-0"	14'-0"	14'-0"	14'-0"	
		#1 / #2	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#3	4'-8"	8'-1"	8'-8"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Stud	4'-8"	8'-2"	8'-6"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Standard	4'-8"	8'-2"	8'-6"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
16" o.c.	SPF	#1	5'-0"	8'-5"	8'-8"	9'-11"	10'-3"	11'-9"	12'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#2	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#3	4'-8"	7'-3"	7'-9"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Stud	4'-8"	7'-3"	7'-9"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Standard	4'-8"	6'-3"	6'-8"	8'-5"	8'-11"	11'-4"	12'-1"	13'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#1 / #2	5'-5"	9'-2"	9'-6"	10'-10"	11'-3"	11'-8"	13'-5"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	HF	#3	5'-1"	9'-0"	9'-4"	10'-8"	11'-1"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Stud	5'-1"	9'-0"	9'-4"	10'-8"	11'-1"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		Standard	5'-1"	9'-0"	9'-4"	10'-8"	11'-1"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#1	5'-6"	9'-3"	9'-7"	10'-11"	11'-4"	12'-11"	13'-6"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#2	5'-5"	9'-2"	9'-6"	10'-10"	11'-3"	12'-11"	13'-5"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
		#3	5'-1"	8'-4"	8'-11"	10'-8"	11'-1"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
DFL	Stud	5'-1"	8'-4"	8'-11"	10'-8"	11'-1"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	Standard	5'-1"	7'-3"	7'-9"	9'-8"	10'-4"	12'-9"	13'-3"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	#1 / #2	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	#3	4'-8"	8'-1"	8'-8"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	Stud	4'-8"	8'-2"	8'-6"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
	Standard	4'-8"	8'-2"	8'-6"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
24" o.c.	DFL	#1 / #2	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Stud	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Standard	3'-11"	5'-2"	5'-5"	6'-10"	7'-4"	9'-3"	9'-11"	10'-9"	11'-6"	14'-0"	14'-0"	14'-0"	14'-0"	
		#1	4'-11"	8'-4"	8'-8"	9'-10"	10'-3"	11'-8"	12'-2"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		#3	4'-8"	8'-1"	8'-8"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	
		Stud	4'-8"	8'-2"	8'-6"	9'-8"	10'-1"	11'-7"	12'-1"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	

## Max Gable Vertical Length

Spacing	2x4	Brace	No Braces	(1) 1x4 L <sup>1</sup> Brace *	(1) 2x4 L <sup>1</sup> Brace *	(2) 2x4 L <sup>1</sup> Brace *	(1) 2x6 L <sup>1</sup> Brace *	(2) 2x6 L <sup>1</sup> Brace *
12" o.c.	SPF	#1 / #2	Standard	4'-3"	7'-3"	7'-7"	8'-7"	8'-11"
				4'-1"	6'-7"	7'-1"	8'-6"	8'-10"
				4'-1"	7'-2"	7'-5"	8'-6"	8'-10"
				4'-1"	6'-11"	7'-5"	8'-6"	8'-10"
16" o.c.	SP	#1	Standard	4'-4"	7'-4"	7'-7"	8'-8"	9'-0"
				4'-3"	7'-3"	7'-7"	8'-7"	8'-11"
				4'-1"	5'-11"	6'-4"	7'-11"	8'-5"
				4'-1"	5'-11"	6'-4"	7'-11"	8'-5"
24" o.c.	DFL	Standard	3'-11"	5'-2"	5'-2"	5'-5"	6'-10"	7'-4"
				4'-11"	8'-4"	8'-8"	9'-10"	10'-3"
				4'-8"	8'-2"	8'-6"	9'-8"	10'-1"
				4'-8"	8'-2"	8'-6"	9'-8"	10'-1"



Building Components Group Inc.

Earth City, MO 63045



May 22 '13

MAX. TOT. L.D. 60 PSF  
 MAX. SPACING 24'-0"

REF ASCE7-10-GAB14015  
 DATE 2/14/12  
 DRWG A14015ENC100212

Bracing Group Species and Grades:			
Group A:		Group B:	
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Standard
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Standard	#3	Standard
Group B:		Group A:	
Hem-Fir		Spruce-Pine-Fir	
#1 & Btr	#1	#1 / #2	Standard
#1	#2	#3	Standard

1x4 Braces shall be SSB (Stress-Rated Board) or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Wind Load deflection criterion is L/240

Provide uplift connections for 55 psf over continuous bearing (3 psf TC Dead Load).

Gable end supports load from 4' o.c. outboarders with 2' o.c. overhang, or 12' plywood overhang

So frame lumber design values based on the AISC January, 2012 rule

Attach L brace with 10d (0.128x3.0 min) nails.

For (1) L brace: space nails at 2' o.c.

For (2) L brace: space nails at 3' o.c.

L brace must be a minimum of 80% of web member length.

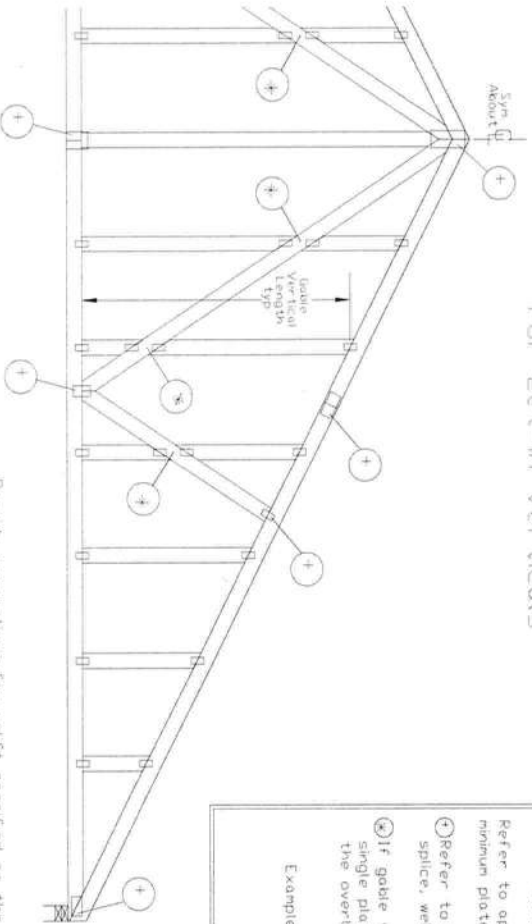
Refer to the Building Designer for conditions not addressed by this detail.

Gable Vertical Plate Sizes			
Vertical Length		No Splice	
Less than 4' 0"		1X4 or 2X3	
Greater than 4' 0", but less than 11' 6"		25X4	
Greater than 11' 6"		3X4	

\* Refer to common truss design for peak, splice, and heel plates.



# Gable Detail For Let-in Verticals



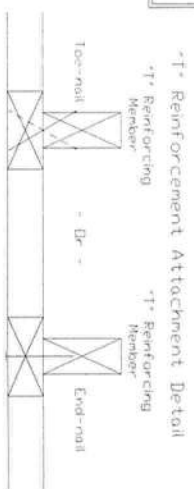
## Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



To convert from 1" to 1" reinforcing members, multiply 1" increase by length (based on appropriate ITW gable detail).

Maximum allowable 1" reinforced gable vertical length is 16' from top to bottom chord.  
1" reinforcing member material must match size, specie, and grade of the 1" reinforcing member.

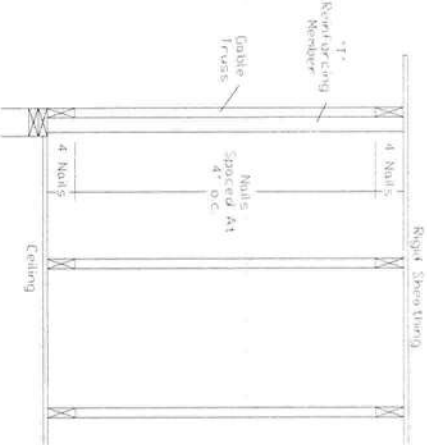
## Web Length Increase w/ 1" Brace

1" Reinf. Member Size	1" Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph  
Mean Roof Height = 30 ft, Kzt = 1.00  
Gable Vertical = 24' o.c. SP #3  
1" Reinforcing Member Size = 2x4  
1" Brace Increase (from Above) = 30% = 1.30  
⊕ 2x4 1" Brace Length = 8' 7"  
Maximum 1" Reinforced Gable Vertical Length  
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.  
Attach each 1" reinforcing member with:  
End Driven Nails:  
10d Common (0.148" x 1.75") Nails at 4' o.c. plus  
⊕ d nails in the top and bottom chords.  
Toe-nailed Nails:  
10d Common (0.148" x 3") Toe-nails at 4' o.c. plus  
⊕ d Toe-nails in the top and bottom chords.  
This detail to be used with the appropriate ITW gable detail for ASCE  
Wind Load.



See appropriate ITW gable detail for maximum vertical length.

\*\*\*WARNING\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING  
\*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS



Building Components Group Inc.

Earth City, MO 63045



REF LET-IN VERT  
DATE 2/16/12  
DRWG GBLLET1N0212

MAX. TOT. LD 60 PSF  
DUR. FAC. ANY  
MAX. SPACING 24.0"

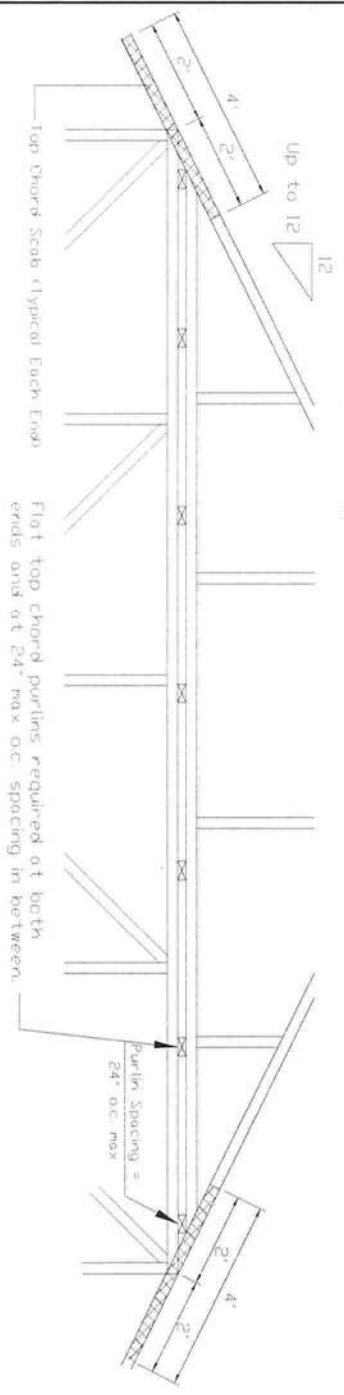
# Piggyback Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg, located anywhere in roof, Exp. C, Wind DL= 5.0 psf (min), Kzt=1.00.  
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg located anywhere in roof, Exp. D, Wind DL= 5.0 psf (min), Kzt=1.00.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

## Detail A : Purlin Spacing = 24" o.c. or less

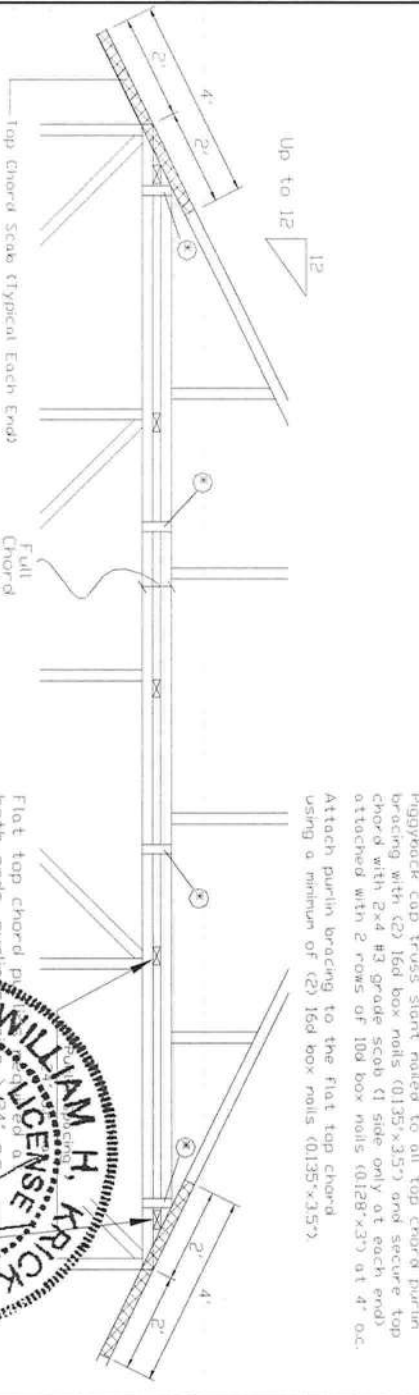


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 TruLok plate attached with (8) 0.120"x1.375" nails, (4) into cap 1C & (4) into base truss 1C or (1) 2x8PB wave piggyback plate attached to the piggyback truss 1C and attached to the base truss 1C with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

## Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

In addition, provide connection with one of the following methods:

TruLok:  
 Use 3x8 TruLok plates for 2x4 chord member, and 3x10 TruLok plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

APA Rated Gussset  
 8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gussset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

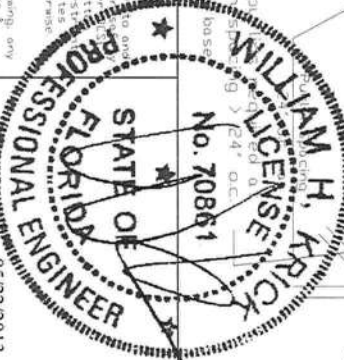
2x4 Vertical Scabs  
 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

2x8PB Wave Piggyback Plate  
 Line 2x8PB wave piggyback plate to each face @ 8' o.c. Attach length to piggyback at time of installation. Attach to supporting truss with (4) 0.120"x1.375" nails. Piggyback plates may be staggered 4' o.c. front to back faces.



Building Components Group Inc.

Earth City, MO 63045



May 22 '13

05/22/2013

SPACING	24.0"
REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212



ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C,  $K_z t = 1.00$

Dr-120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Dr-120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Dr-100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch

#3	Stud
Standard	

Southern Pine\*\*\*

#3	Stud
Standard	

Group B:

Hem-Fir	
#1 & Fir	
#1	

Douglas Fir-Larch

#1	
#2	

Southern Pine\*\*\*

#1	
#2	

1x4 Braces shall be SDB (Stress-Rated Board)

\*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC dead load).  
Gable end supports: load from 4' ft. outleaklers with 2' ft. overhang, or 12" plywood overhang.  
So Pine lumber design values based on the ALSC January, 2012 rules

Vertical Length	No Splice
Less than 4' 0"	15x4
Greater than 4' 0", but less than 11' 6"	25x4
Greater than 11' 6"	35x4

\* Refer to common truss design for peak, splice, and heel plates.

REF	ASCE7-10-GABI403
DATE	2/14/12
DRWG	A14030ENC10021

**Building Components Group Inc.**

Earth City, MO 63045

The Publishing Committee (PCC) will not be responsible for any deletion from this drawing and/or any modification to the drawing. The PCC will not be responsible for any deletion or failure to build that PCCs in conformance with ANSI Z10.1 or for handling, shipping, installation or use of this drawing. A seal on the drawing or cover page listing the drawing, addressee acceptance or withdrawal, and the date of acceptance or withdrawal is required. The use of this drawing is not intended to constitute a contract. The user of this drawing is responsible for obtaining the correct version of the drawing. For more information see this job's general instructions page and those with sites [www.ansi.org/standards/3d](http://www.ansi.org/standards/3d) and [www.ansi.org/standards/3d](http://www.ansi.org/standards/3d). For more information see this job's general instructions page and those with sites [www.ansi.org/standards/3d](http://www.ansi.org/standards/3d) and [www.ansi.org/standards/3d](http://www.ansi.org/standards/3d).

05/22/2013  
May 22 '13

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF ASCE7-10-GABI4030

DATE 2/14/12

DRAWG A14030ENC100212

100

73  
Permit# 31109

**ITW Building Components Group, Inc.**

1950 Marley Drive Haines City, FL 33844  
Florida Engineering Certificate of Authorization Number: 0 278  
Florida Certificate of Product Approval # FL1999  
Page 1 of 1 Document ID: IUYC487-Z0130114605



Truss Fabricator: **Anderson Truss Company**  
Job Identification: **13-166B--Erkinger Home Builders Miller Residence -- Indian Ridge**  
Truss Count: **2**  
Model Code: **Florida Building Code 2010**  
Truss Criteria: **FBC2010Res/TPI-2007(STD)**  
Engineering Software: **Alpine Software, Version 12.03.**  
Structural Engineer of Record: **The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC**  
Address: **Roof - 37.0 PSF @ 1.25 Duration**  
Minimum Design Loads: **Floor - N/A**  
**Wind - 130 MPH ASCE 7-10 -Closed**

**Notes:**

- Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
- The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
- As shown on attached drawings; the drawing number is preceded by: HCUSR487**

William H. Krick  
-Truss Design Engineer-

1950 Marley Drive  
Haines City, FL 33844

**Details: BRCLBSUB-160TL-**

#	Ref	Description	Drawing#	Date
1	67623-RA7	30' Stepdwn	13211006	07/30/13
2	67624-RA8	38' Stepdwn	13211053	07/30/13



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130 mph wind, 15.00 ft mean hgt, ASCE 7-10, closed bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCP (+/-)=0.18

(a) Continuous lateral bracing equally spaced on member. Calculated horizontal deflection is 0.29" due to live load and 0.40" due to dead load.

In lieu of structural panels use purlins to brace all Flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 160TL for nailing and trulox plate requirements.



(8" min.)

Design Crit: FBC2010Res/TP1-2007(STB) No. 70861  
FT/RT=10%(0%)/0(0) 12.03.04.03P6.14

FL/-/3/-/-/R/-  
Scale = .1875"/Ft.

ITW Building Components Group Inc.

Orlando FL, 32837  
FL COA #0278

[illegible]

WILLIAM H. KRACK  
No. 70861  
12.03.04  
STATE OF FLORIDA  
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R487-- 67623
TC DL	7.0 PSF	DATE	07/30/13
BC DL	10.0 PSF	DRW	HCSUR487 13211006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	312178
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1UYC487_Z01

THIS DNG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Bot chord 2x4 Sp\_#1\_12A

Webbs 2x4 SP\_#3\_12A : W1, W2, W8 2x4 SP\_#1\_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

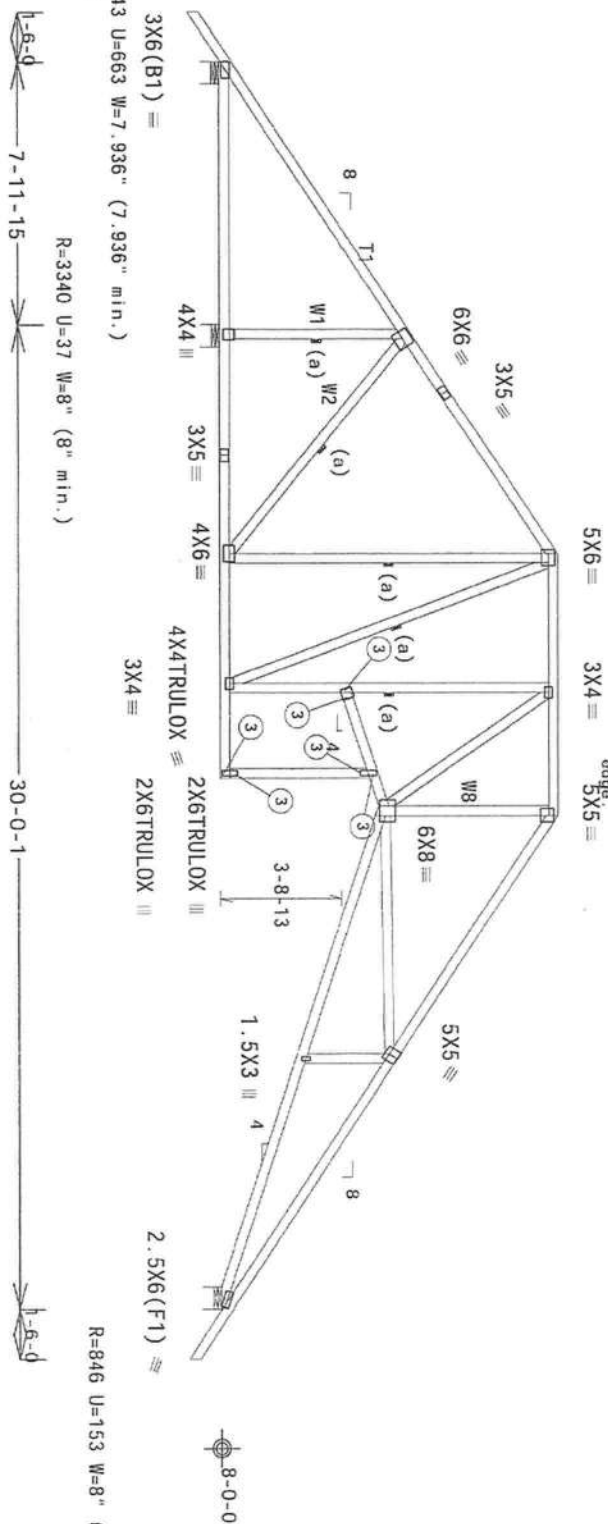
This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

(a) Continuous lateral bracing equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss.

ply. See DMG 160TL for nailing and trulox plate requirements.



R=-1299 RW=43 U=663 W=7.936" (7.936" min.)  
RL=376/-376

R=3340 U=37 W=8" (8" min.)

R=846 U=153 W=8" (8" min.)

PLT TYP. Wave, Trulox

Design Crit: FBC2010Res/TP1-2007 (STD)

FT/RT=10%(0%)/0(0)

QTY: 1 FL/-/3/-/-/R/-

Scale = .1875"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837  
FL COA #0278

[illegible]

07/30/2013

TC LL	20.0 PSF	REF	R487-- 67624
TC DL	7.0 PSF	DATE	07/30/13
BC DL	10.0 PSF	DRW	HCSR487 13211053
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	312169
DUR. FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1UYC487_Z01



THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHAIRING THE SPECIFIED CL.B SHOWN ON SINGLE PLY SEALED DESIGNS. 1:1-SPLICING THE STAR IPACING.

ALTERNATIVE BRACING SPECIFIED IN UGAP: BRACING MAY BE CONSIDERED AN ALTERNATE BRACING. SEE FIG. 9-10 FOR A FURTHER DISCUSSION.

BRACING

WEB MEMBER SIZE	SPECIFIED CLR BRACING	ALTERNATIVE BRACING T OR L-BEAM OR B-RACE
2x5 DR 2x4	1 ROW	2x4
2x3 DR 2x4	2 ROWS	2x6
2x6	1 ROW	2x4
2x6	2 ROWS	2x6
2x8	1 ROW	2x6
2x8	2 ROWS	2x6

1-BRACE, 1-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE  
 DIP NOT TO EXCEED 10% UNLESS SPECIFIED OTHERWISE IN  
 ENGINEERING SEALED DESIGN

4) CENTER SCAB ON WIDE FACE OF WEB. APPLY 1/2 STAB TO EACH FACE OF WEB.

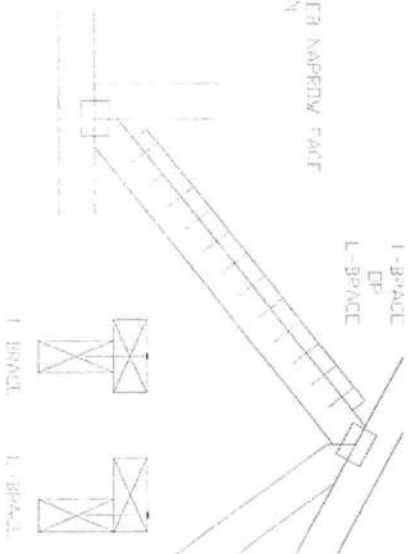


**Building Components Group Inc.**

Earth City, MO 63045

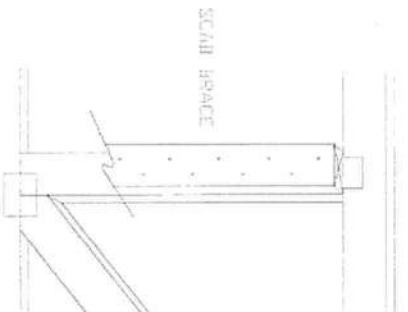
L-BRACINING  
DP  
T-BRACINING

APPLY TO OTHER SIDE OF WEB APPROPRIATE  
ATTACH WITH 100 BTX OR GUM  
1012613 3.5MM NAILS  
AT 6" O.C.  
SPACE IS A  
MINIMUM 80% JF WHITE  
MEMBER LEADERS



## SCAR PROBLEM

APPLY SCALING TO WIDE FACES OF THE  
NO MORE THAN 10 SCALING FACT  
ATTACH WITH 10% BDC TOP COR  
(0.18" x 3.3" MIN) HALL  
AT 5" DIA  
BRACE IS A MINIMUM  
80% OF WORK HEIGHT. ENDS

[illegible]

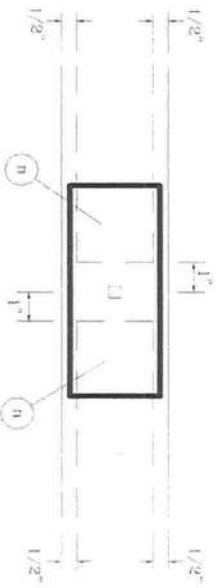
A circular professional engineer seal for the State of Florida. The outer ring contains the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom. Inside the ring, the number "12518" is printed. The seal is signed with the name "WILLIAM C. ADAMS" in cursive script across the center.

07/30/2013  
Jul 30 '13

	REF	CLB SUBST
IC LL	PSF	
IC EL	PSF	DATE 1/1/09
BC LL	PSF	DRAWG BRCL BSLT0108
BC EL	PSF	
THL L.D.	PSI	

# TRULOX INFORMATION DETAIL

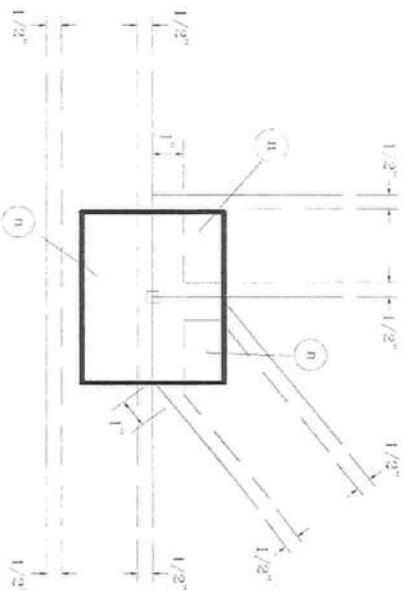
TYPICAL OFF-PANEL SPLICE



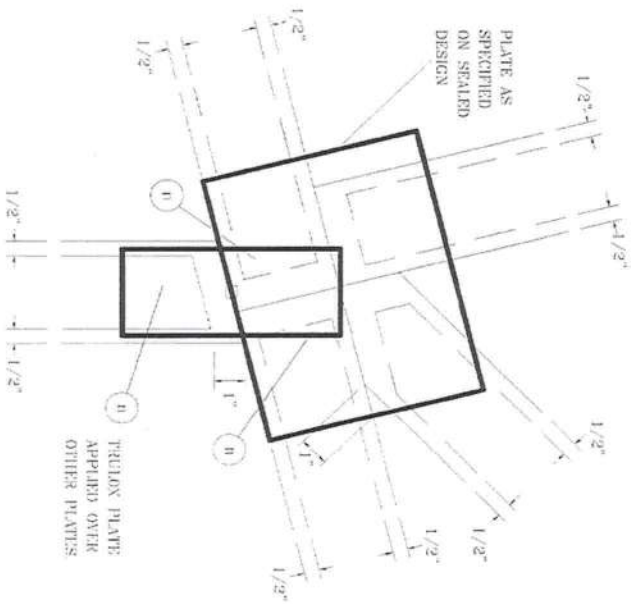
DO NOT APPLY NAILS WITHIN 1/2" OF LAMBER EDGES OR 1" OF LAMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LAMBER

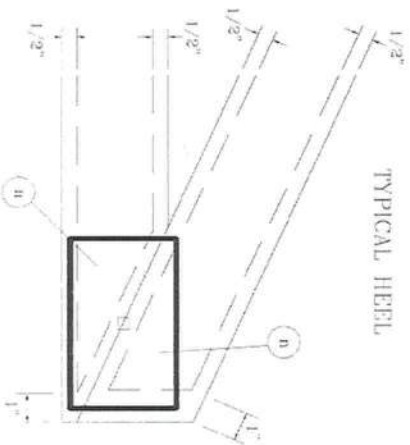
TYPICAL PANEL, POINT WITHOUT SPLICE



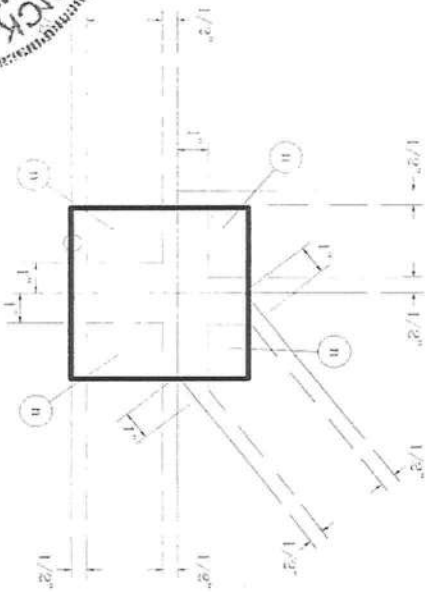
TYPICAL FILLER



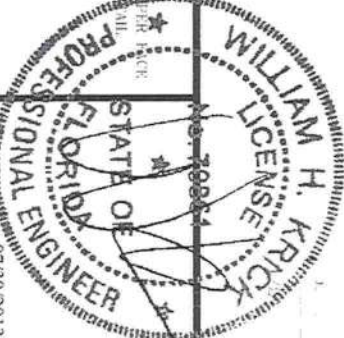
TYPICAL HEEL



TYPICAL PANEL, POINT SPLICE



NOTES:  
(a) IS THE REQUIRED NUMBER OF 0.125" X 1.375" NAILS, OR EQUAL, PER FACE PER PLATE AS SPECIFIED ON THE SEALED DESIGN REFLECTING THIS DETAIL.  
○ LOCATES PLATE CORNER OR FLASH EDGE.  
□ LOCATES PLATE CENTER.



Earth City, MO 63045

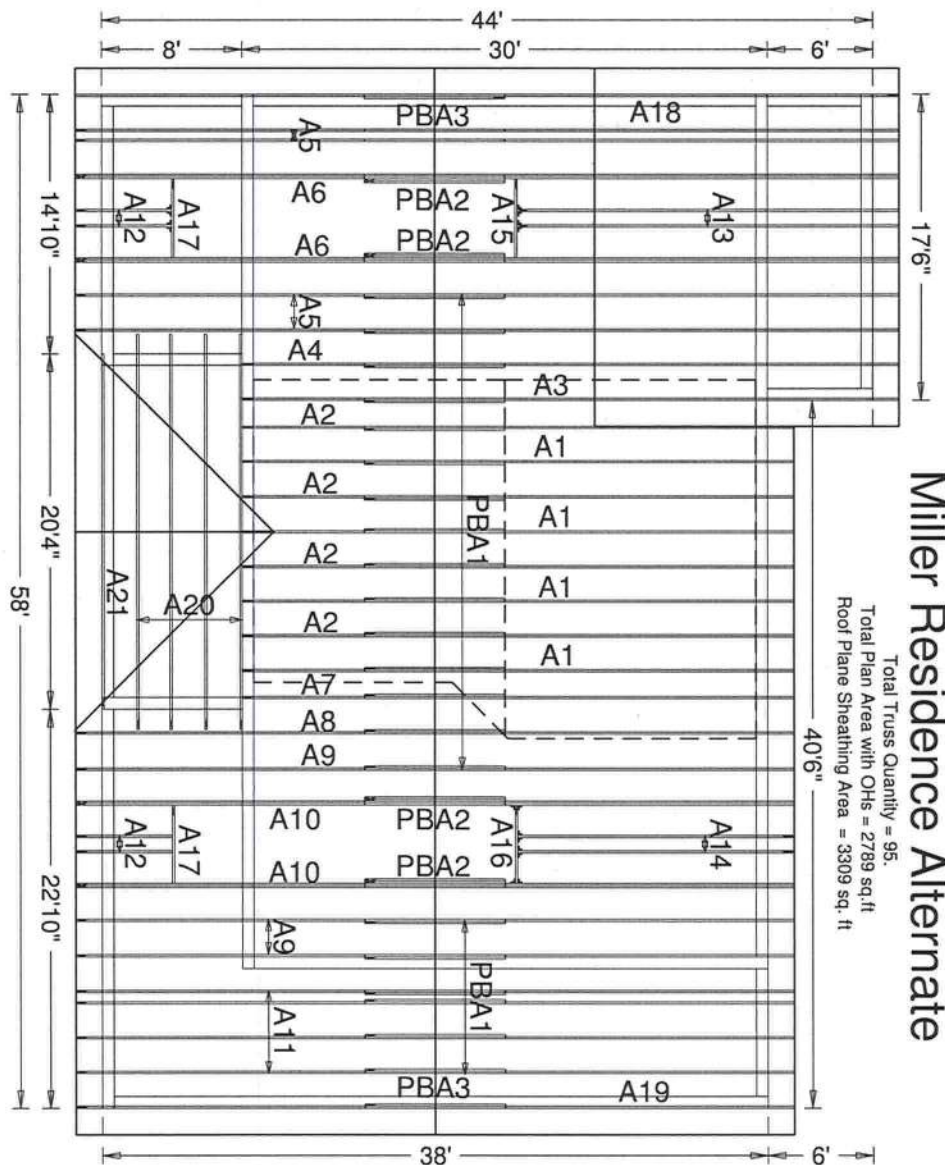
Jul 30 '13

PAGE 1 OF 1  
DATE 4/1/09

160  
TTI

TRULOX PLATING





# Miller Residence Alternate

Total Truss Quantity = 95.  
 Total Plan Area with OHs = 2789 sq. ft.  
 Roof Plane Sheathing Area = 3309 sq. ft.

