Product Approval

USER: Public User

Search Criteria			
Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	Therma-
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validated
FL1170-R1 History	Revision	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL1185-R1 History	Revision	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL5262	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL5265	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL5891	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6
FL6993	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6
FL7186	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6

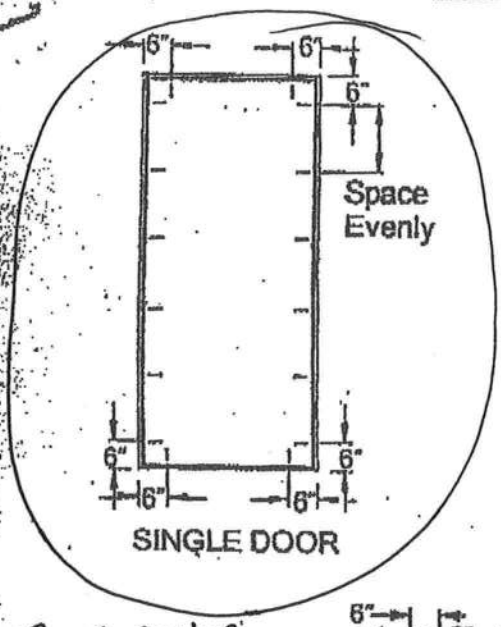
All Approved

FL7347	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6
FL7586	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6
FL7627	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6
FL7630	New	Therma-Tru Corporation **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	L.F. Schmid P.E. (813) 926-6

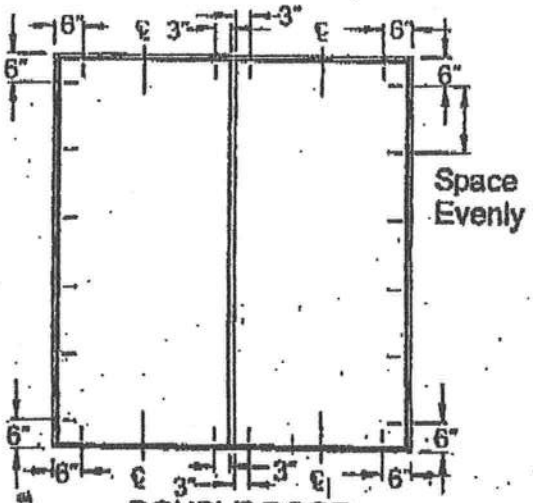
DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

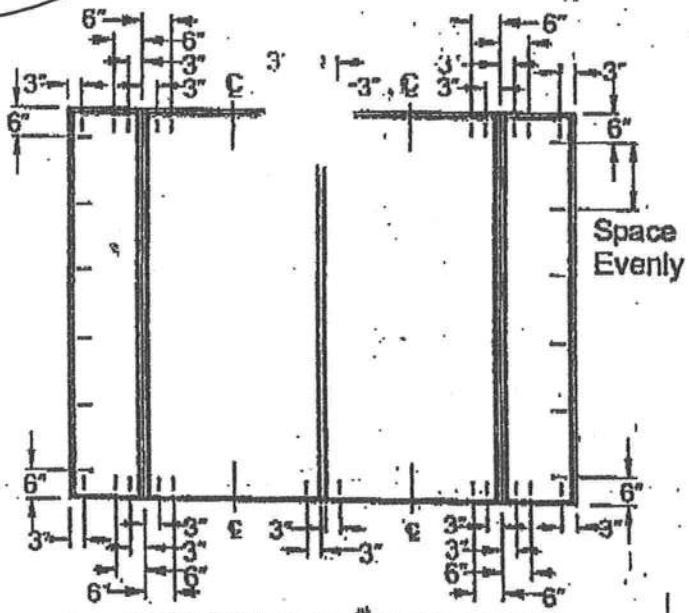


SINGLE DOOR

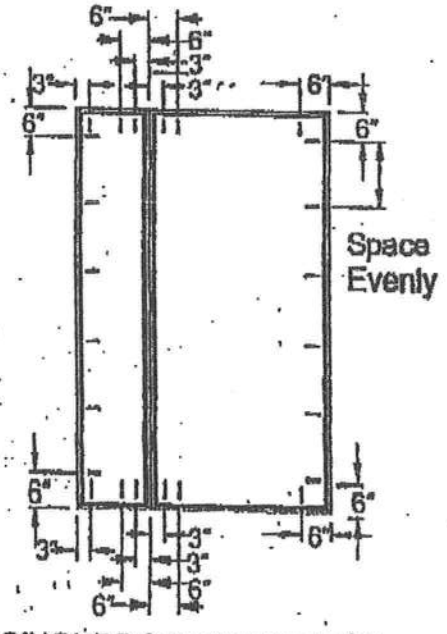


DOUBLE DOOR

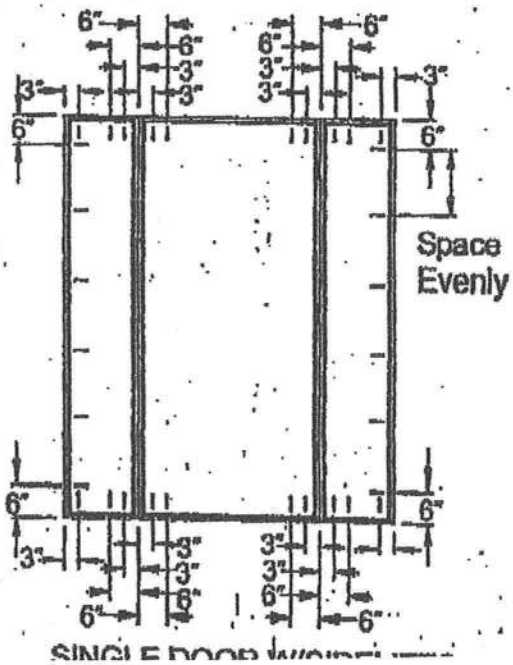
Door Anchor Systems



DOUBLE DOOR W/SIDELITES



SINGLE DOOR W/SIDELITE



SINGLE DOOR W/SIDE

PRODUCT APPROVAL

Product Type Detail

User: Public User - Not Associated with Organization -

Need Help ?

Application #:	FL1956-R1
Date Submitted:	06/09/2005
Code Version:	2004
Product Manufacturer:	TAMKO Roofing Products, Inc.
Address/Phone/email:	PO Box 1404 Joplin, MO 64802 (800) 641-4691
Technical Representative:	Frederick J. O'Connor
Technical Representative Address/Phone/email:	PO Box 1404 Joplin, MO 64802 (800) 641-4691 fred_oconnor@tamko.com
Category:	Roofing
Subcategory:	Asphalt Shingles
Evaluation Method:	Certification Mark or Listing
Referenced Standards from the Florida Building Code:	Section: Standard: ASTM D 3462 Year: 2001
Certification Agency:	Underwriters Laboratories Inc.
Quality Assurance Entity:	
Validation Entity:	
Authorized Signature:	Frederick O'Connor fred_oconnor@tamko.com
Evaluation/Test Reports Uploaded:	
Installation Documents Uploaded:	PTID_1956_R1_J_Tamko_Ict_061705.pdf

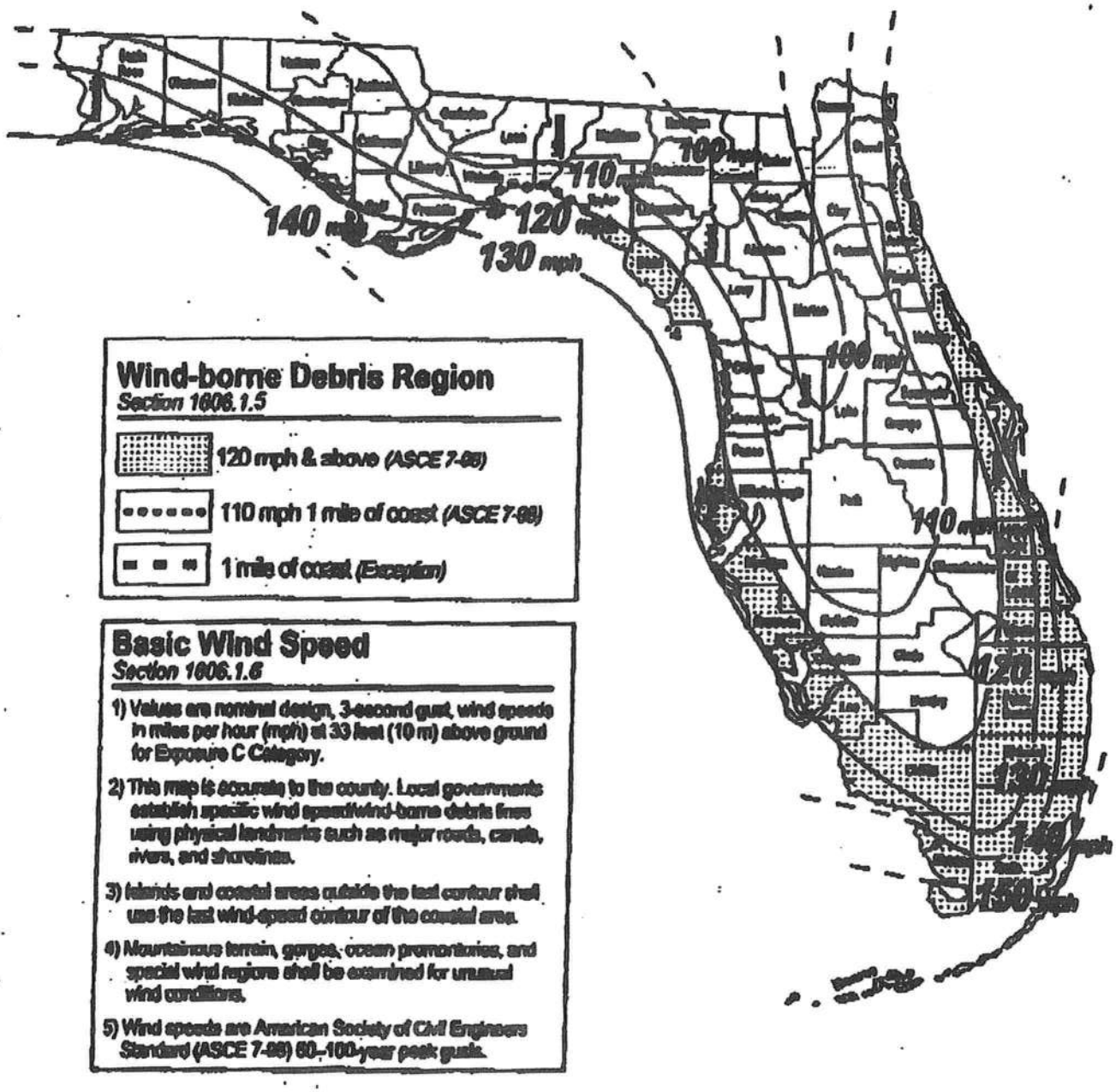
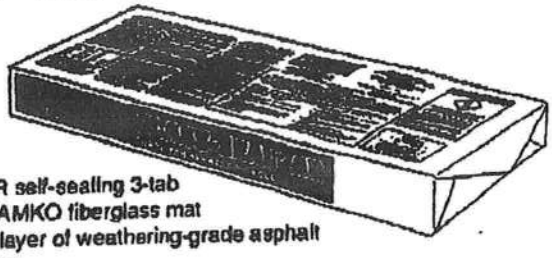


FIGURE 1606
STATE OF FLORIDA
WIND-BORNE DEBRIS REGION & BASIC WIND SPEED

TAMKO Elite Glass-Seal® AR

PRODUCT DATA

THREE-TAB ASPHALT SHINGLES



Manufactured in Tuscaloosa, AL

TAMKO ELITE GLASS-SEAL® AR self-sealing 3-tab shingles are made with a tough TAMKO fiberglass mat coated on both sides with a thick layer of weathering-grade asphalt and surfaced with ceramic granules.

USES

For application to roof decks with inclines of not less than 2 inches per foot. For slopes between 2 inches and 4 inches per foot, refer to wrapper instructions.

ANALYSIS

- 25-year limited warranty, 3-year FULL START, limited transferability, winds up to 60 MPH.
- Superior fire resistance compared to organic shingles.
- 10-year Algae-Relief (AR) limited warranty that provides for cleaning of discoloration caused by certain algae growth that may occur in areas with high humidity.

CERTIFICATIONS

UL Class A Fire Rating
UL Wind Resistant
ASTM D 3018, Type I
Miami Dade County Florida NOA 02-0130-03
Expiration Date: 04/11/07

ASTM D 3161, Type I (modified to 110 mph)
ASTM D 3462
ASTM E 108, Class A
TAS 100-95 Wind and Wind Driven Rain

Fed. Spec.: Exceeds SS-S-001534, Class A, Type I

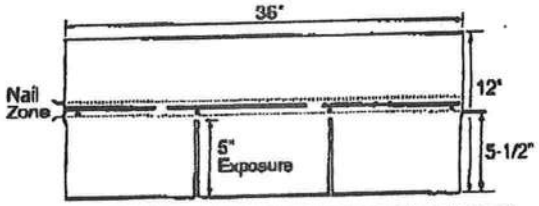
COLORS

- Glacier White
- Grey Blend
- Rustic Hickory
- Pastel Red
- Shadow Grey
- Rustic Black
- Weathered Wood
- Olde English Pewter
- Pastel Green
- Empire Green Blend
- Driftwood
- Rustic Cedar
- Oxford Grey
- Pastel Brown
- Tile Red Blend
- Desert Sand

PRODUCT DATA*

Shingle size	12" X 36"
Exposure	5"
Shingles per square	80
Bundles per square	3

*All values stated as nominal



CAUTION: The National Institute for Occupational Safety and Health (NIOSH) has concluded that fumes of heated asphalt are a potential occupational carcinogen. Do not heat or burn this product.

TAMKO ROOFING PRODUCTS

TAMKO® and ELITE GLASS-SEAL® are registered trademarks of TAMKO Roofing Products, Inc.

Visit our Web Site at www.tamko.com

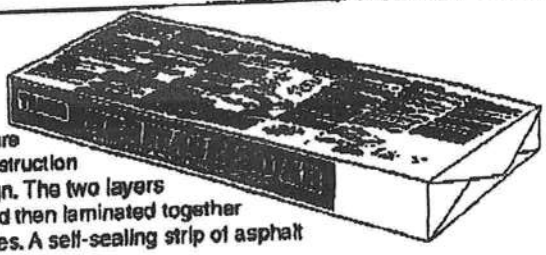
V32003

Central District	220 West 4th St., Joplin, MO	64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD	21701	800-368-2055
Southeast District	2300 35th St., Tuscaloosa, AL	35401	800-228-2656
Southwest District	7910 S. Central Exp., Dallas, TX	75216	800-443-1634
Western District	5900 East 43rd Ave., Denver, CO	80216	800-530-8668

HERITAGE 30 AR®

PRODUCT DATA

LAMINATED ASPHALT SHINGLES



Manufactured in Tuscaloosa, AL.

HERITAGE 30 AR® shingles feature a double-layer fiberglass mat construction with a random-cut sawtooth design. The two layers of mat are coated with asphalt and then laminated together and surfaced with ceramic granules. A self-sealing strip of asphalt helps provide added wind resistance.

USES

For application to roof decks with inclines of not less than 2 inches per foot. For slopes between 2 inches and 4 inches per foot, refer to wrapper instructions.

ADVANTAGES

- 30-year limited warranty, 5-year FULL START, limited transferability, winds up to 70 MPH.
- Superior fire resistance compared to organic shingles.
- Rustic beauty of wood shakes.
- Shadowtone feature adds depth and dimensional appearance.
- 10-year Algae-Relief (AR) limited warranty that provides for cleaning of discoloration caused by certain algae growth that may occur in areas with high humidity.

CERTIFICATIONS

UL Class A Fire Rating	ASTM D 3018, Type I
UL Wind Resistant	ASTM E 108, Class A
	ASTM D 3161, Type I

Fed. Spec.: Exceeds SS-S-001534, Class A, Type I

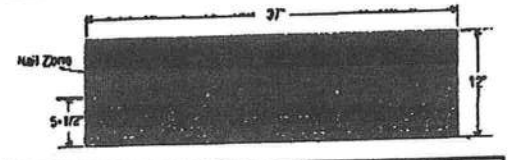
COLORS

Classic Heritage Colors:

- Weathered Wood
- Rustic Cedar
- Oxford Grey
- Rustic Hickory
- Shadow Grey
- Rustic Black

PRODUCT DATA*

Shingle size	12" X 37"
Exposure	5"
Shingles per square	78
Bundles per square	3



*All values stated as nominal

CAUTION: The National Institute for Occupational Safety and Health (NIOSH) has concluded that fumes of heated asphalt are a potential occupational carcinogen. Do not heat or burn this product.

TAMKO ROOFING PRODUCTS

TAMKO® and HERITAGE® are registered trademarks of TAMKO Roofing Products, Inc.

Visit our Web Site at www.tamko.com

052003

Central District	220 West 4th St., Joplin, MO	64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD	21701	800-388-2055
Southeast District	2300 35th St., Tuscaloosa, AL	35401	800-228-2656
Southwest District	7910 S. Central Exp., Dallas, TX	75216	800-443-1834
Western District	5900 East 43rd Ave., Denver, CO	80216	800-530-8868

Product Approval Method: Method 1 Option A

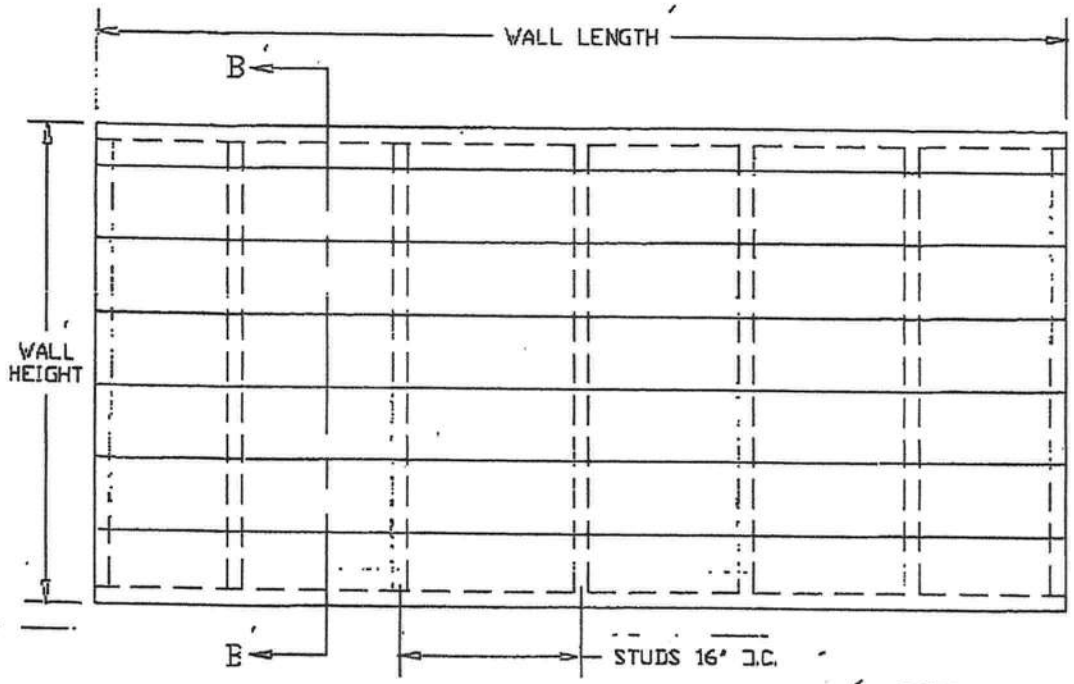
Application Status: Approved
Date Validated: 06/20/2005
Date Approved: 06/29/2005
Date Certified to the 2004 Code:

Page: Go

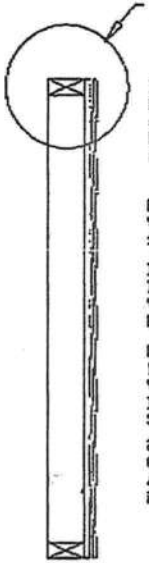
App/Seq #	Product Model # or Name	Model Description	Limits of Use
1956.1	Elite Glass-Seal AR	A heavy weight 3 tab asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.2	Glass-Seal AR	A 3 tab asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.3	Heritage 30 AR	A heavy weight dimensional asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.4	Heritage 40 AR	A heavy weight dimensional asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.5	Heritage 50 AR	A heavy weight dimensional asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.6	Heritage Declaration	A heavy weight triple laminate asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.
1956.7	Heritage XL	A heavy weight dimensional asphalt shingle.	Asphalt shingles shall be used only on roof slopes of 2:12 or greater. Not approved for use in HVHZ.

Next

FL # 889.5



SECTION B-B



DESCRIPTION
Hardiplank & Cenplank siding material is a non asbestos fiber cement product tested in accordance with ASTM C-1185 and meeting the requirements of the Florida Building Code.

PLANK DIMENSIONS

Width	Length	Thickness
59 1/2"	12 & 14'	5/16"

DESIGN PRESSURE RATING -

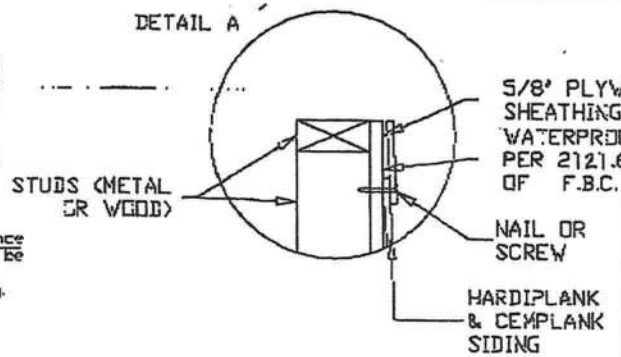
Installation	Design Pressure
Wood frame	-92 PSF
Metal frame	-92 PSF

NOTES
1) ALL INSTALLATION SHALL BE DONE IN CONFORMANCE WITH THIS NOTICE OF ACCEPTANCE, THE MANUFACTURER'S INSTALLATION RECOMMENDATIONS, AND THE APPLICABLE SECTIONS OF THE FLORIDA BUILDING CODE.

2) STUDS OF METAL OR WOOD WHERE HARDIPLANK & CENPLANK WILL BE INSTALLED SHALL BE DESIGNED BY AN ENGINEER OR ARCHITECT PER THE F.B.C. AND THE REQUIREMENTS OF THIS N.O.A.

HARDIPLANK & CEMPLANK SIDING INSTALLATION DETAILS
The planks are applied horizontally commencing from the bottom course of a wall with 1 1/4" wide laps at top of the plank. The optional PVC cover molding 1 5/8" wide is applied to the bottom plate under the bottom plank course. The vertical joints must be over framing members. Optional PVC butt joints inserts are used for on-stud jointing. The planks are to be installed over 5/8" (5 ply) APA rated plywood supported by a minimum of 2"x4" wood studs or 20 ga. x 3 5/8" x 1 3/8" steel studs spaced a maximum of 16" o.c. The siding shall be fastened through over lapping planks with 8d x 2 1/2" long galvanized box nails over wood studs or with #9 x 2 1/4" long x 0.315" corrosion resistance H.D. ribbed buple screws over steel studs. The fasteners shall be placed in the over-lapping area 8" oc. vertically and 16" o.c. horizontally into the studs through the 5/8" plywood sheathing. A distance of 3/4" from the edges shall always be observed.

5/8" PLYWOOD SHEATHING SHALL BE ATTACHED TO THE STUDS IN ACCORDANCE TO FLORIDA BUILDING CODE, WITH ANOTHER SET OF NAILS OR SCREWS AS UNDERLINED ABOVE.



REVISION BLOCK
REV # / DATE

PRODUCT RENEWED
as complying with the Florida
Approval No. 02-0418.04
Expiration Date 05/01/2012
By: Miami-Dade Product Control Division

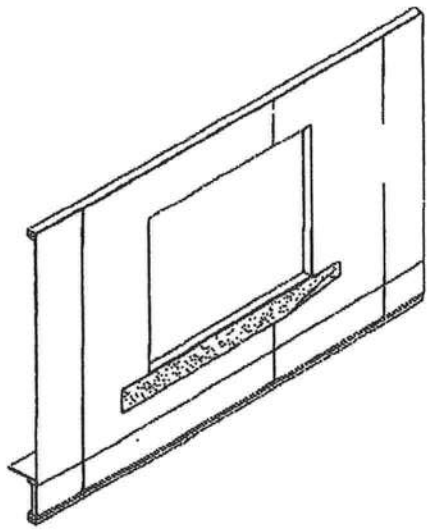
PRODUCT REVISED
as complying with the Florida
Approval No. 02-0729.02
Effective Date 05/01/07
By: Miami-Dade Product Control Division

JAMES HARDIE BUILDING PRODUCTS - USA
RESEARCH & DEVELOPMENT CENTER
10901 ELM AVENUE
FONTANA, CA 92337
909-356-6300
FAX: 909-427-6634

DATE: 04/02/2004
DRAWING NO.: HPLK-4X8
SHEET NO.: 2/3
TITLE: HARDIPLANK® & CEMPLANK® INSTALLATION DETAILS
SCALE: NTS
DRAWN BY: C DIERCKS
APPROVED BY:
CHECKED BY:
ENG NO.:

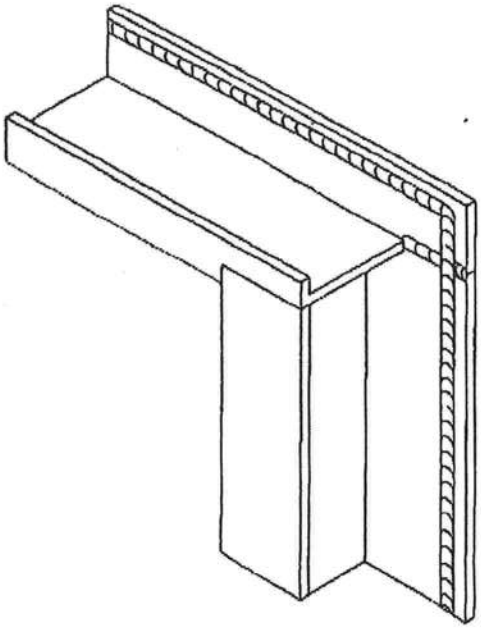
ALUMINUM MOUNTING FLANGE (NAILING FIN) WINDOW INSTALLATION INSTRUCTIONS

ROUGH OPENING PREPARATION



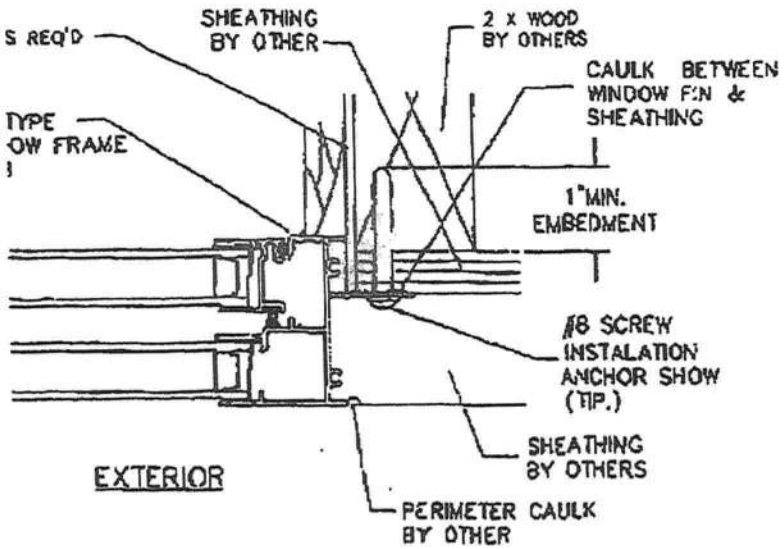
1. Rough opening should be sized one-half inch (½") larger than the window size in width and height. Provide a minimum of one-quarter inch (¼") clearance at the top (head) and one-eighth inch (⅛") clearance on each side (jamb) of the unit. Improperly sized rough openings will not allow the window to function as was intended.
2. Sheathing and house wrap should be installed around the perimeter of the rough opening before window installation or the attachment of flashing. Flashing materials must meet a minimum water resistance of 24 hours in accordance with ASTM-D779 and be a minimum of nine inches (9") wide. GA recommends the use of appropriate self-adhering flashing. All notations on flashing placement and installation are based on the use of appropriate nine inch (9") self-adhering flashing.
3. Attach a strip of flashing material horizontally along the bottom face (sill) of the rough opening. Sill flashing length should be equal to the rough opening width plus two times the flashing width. The sill flashing should be long enough to extend nine inches (9") beyond both jambs of the rough opening.
4. Regarding building sequence, the roof should be installed before windows are installed. This will prevent any problems from header deflection. If windows are installed before the roof loads have been applied and the header deflects, serious problems such as stress cracks and window frame deflection may occur which will void the window warranty. If windows must be installed before the roof loads have been applied, then the size of the headers must be large enough to limit the amount of deflection.

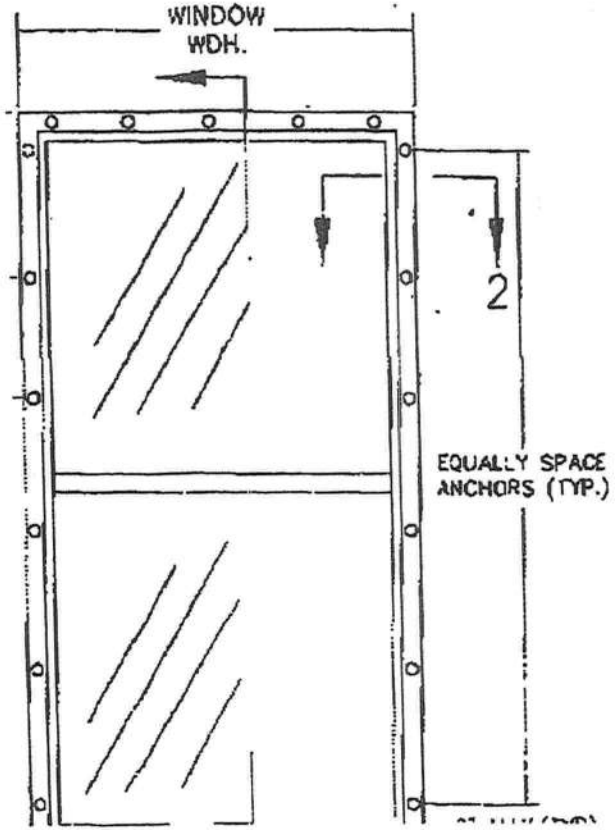
WINDOW PREPARATION



5. The operable sash of the window must remain closed and locked during the entire installation process.
6. Apply a three-eights inch (⅜") nominal continuous bead of sealant to the interior perimeter of the mounting flange (nailing fin) and along the seams of all joints at the corners. It is important to properly select and apply sealant. Refer to ASTM standards C1193 and C1299 for information on sealant selection and use. If in doubt as to compatibility, use a silicone or polyurethane sealant rated to ASTM C920 Class 25.

	8175.1	HR - Aluminum	1180 XO	72 x 54 DSB	+40/-40
	8175.2	HR - Aluminum	1480 XO	72 x 54 DSB	+40/-40
	8175.3	HR - Aluminum	1580/1590 XO	72 x 54 IG DSB	+40/-40
	8175.4	HR - Aluminum	1580/1590 XOX	108 X 48 IG SSB/DSB/SSB	+40/-40
	8175.5	HR - Aluminum	1980/1990 XO	72 x 54 IG DSB	+40/-40
	8175.6	HR - Aluminum	1980/1990 XOX	108 X 48 IG SSB/DSB/SSB	+40/-40
	8177.1	SH - Vinyl	7100	48 x 72 IG DSB	+40/-40
	8177.2	SH - Vinyl	7100 Twinned	72 x 72 IG SSB	+40/-40
	8359.1	SH - Aluminum	1100/1900/1970 Twinned	88 x 72 DSB Anld	+40/-40
X	8359.2	SH - Aluminum	5000/5300	48 x 96 DSB Anld IG	+20/-20
	8359.3	SH - Aluminum	5000/5300	44 x 84 DSB Anld IG	+40/-40
	8359.4	SH - Aluminum	5000/5300	52 x 72 DSB Anld IG	+40/-40
X	8359.5	SH - Aluminum	5000/5300 Twinned	72 x 72 DSB Anld IG	+40/-40
	8359.6	SH - Vinyl	7000/7100	48 x 72 DSB Anld IG	+30/-30
	8359.7	SH - Aluminum	1100/1900/1970/1450/1550/1570	53 x 72 DSB Anld & IG	+40/-40
	8359.8	SH - Aluminum	1100/1900/1970/1450/1550/1570	44 x 84 Anld 3/16 & IG	+40/-50
	8359.9	SH - Aluminum	1100/1900/1970/1450/1550/1570	44 x 84 Temp 1/8 & IG	+40/-60
	8359.10	SH - Aluminum	1550/1570/1900/1970	53 x 72 DSB Anld IG	+40/-45
	8359.11	SH - Aluminum	1550/1570/1900/1970	48 x 72 DSB Anld IG	+40/-50
	8359.12	SH - Vinyl	7000/7100	48 x 72 DSB Anld IG	+40/-40
	8359.13	SH - Vinyl	7000/7100 Twinned	72 x 72 DSB/SSB Anld IG	+40/-40
	8359.14	SH - Vinyl	7000/7100 Twinned	72 x 72 SSB/SSB Anld IG	+40/-40
	8704.1	SH - Aluminum	1450 Twinned	73 x 71 DSB Anld	+40/-40
	8704.2	SH - Aluminum	1550/1570 Tripled	106 3/8 x 72 DSB Anld IG	+40/-40
	8705.1	Casement - Vinyl	7500	32 x 72 DSB Anld	+50/-50
	8705.2	Casement - Vinyl	7500	36 x 60 DSB Anld	+50/-50
	8905.1	Mulls - Horizontal	1442/5348		
	8905.2	Mulls - Vertical	1442/1448/5348/5548		
	8905.3	Mulls - Horizontal	1445/1447/5347		
	8904.4	Mulls - Horizontal	1445/1447/5347/5547 Twin		
	8905.5	Mulls - Vertical	1445/5347/5547		
	8905.6	Mulls - Horizontal	1445-2/1447-2/5347-2		
	8905.7	Mulls - Horizontal	1445-2/1447-2/5347-2/5547-2 Twin		
	8905.8	Mulls - Vertical	1445-2/5347-2/5547-2		

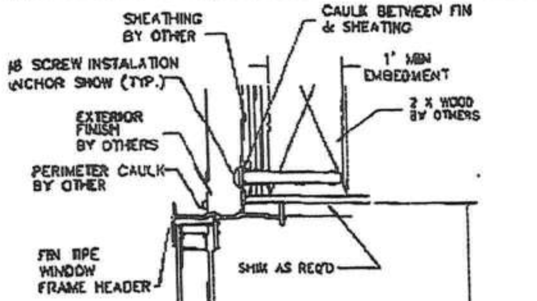


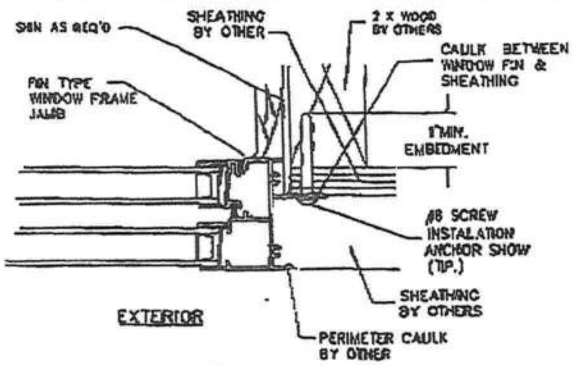


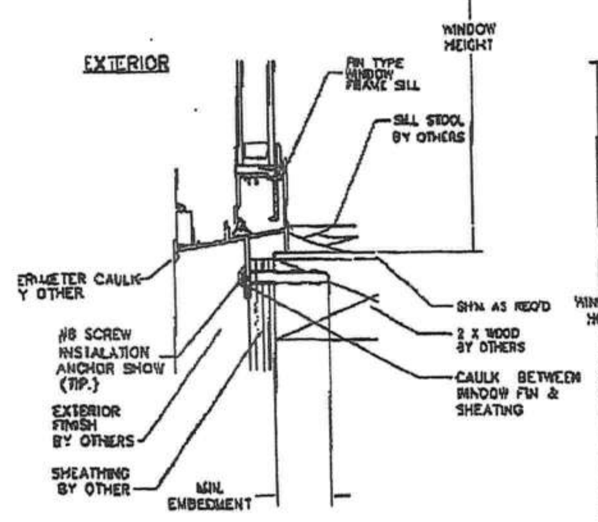
SINGLE HUNG WINDOW COMPARATIVE ANALYSIS & ANCHOR CHART FOR GENERAL ALUMINUM SERIES 5000/5300

WINDOW SIZE (INCHES)	DOUBLE GLAZED INSULATED UNIT WITH SSB(3/32") OUTBOARD & INBOARD LITE	DOUBLE GLAZED INSULATED UNIT WITH OSB(7/16") OUTBOARD & INBOARD LITE	NO. OF #8 SCREWS REQUIRED IN FRAME 1" MIN. EMBEDMENT			NO. OF 0.113 DIA. NAILS REQUIRED IN FRAME 1 1/2" MIN. EMBEDMENT		
WIDTH/ HEIGHT	DESIGN PRESSURE CAPACITY (PSF)	DESIGN PRESSURE CAPACITY (PSF)	HEAD	SILL	AT EACH JAMB	HEAD	SILL	AT EACH JAMB
20 X 30	-80	-	2	2	2	2	2	3
20 X 40	-80	-	2	2	2	2	2	4
20 X 48	-80	-	2	2	2	2	2	5
20 X 60	-80	-	2	2	2	2	2	5
20 X 60	-80	-	2	2	3	2	2	7
20 X 72	-78	-	2	2	3	2	2	7
20 X 84	-66	-	2	2	3	2	2	7
30 X 30	-80	-	2	2	2	2	2	4
30 X 40	-80	-	2	2	2	3	3	5
30 X 48	-80	-	2	2	3	3	3	7
30 X 60	-80	-	2	2	3	3	3	7
30 X 60	-80	-	2	2	3	3	3	6
30 X 72	-68	-	2	2	3	2	2	8
30 X 84	-62	-	2	2	3	2	2	8
36 X 30	-80	-	2	2	2	3	3	5
36 X 40	-71	-	2	2	2	4	2	6
36 X 48	-80	-	2	2	3	4	4	7
36 X 60	-67	-	2	2	3	4	4	7
36 X 60	-68	-	2	2	3	3	3	8
36 X 72	-63	-	2	2	3	3	3	8
36 X 84	-60	-	2	2	3	2	2	8
40 X 30	-74	-	2	2	2	3	3	5
40 X 40	-61	-	2	2	2	4	4	5
40 X 48	-80	-	2	2	3	4	4	7
40 X 50	-68	-	2	2	3	4	4	7
40 X 60	-62	-	2	2	3	4	4	7
40 X 72	-58	-	2	2	3	3	3	8
40 X 84	-45	-45	2	2	4	4	4	10
44 X 30	-78	-	2	2	2	4	4	5
44 X 40	-63	-	2	2	2	3	3	5
44 X 48	-62	-	2	2	2	4	4	6
44 X 50	-61	-	2	2	2	4	4	7
44 X 60	-49	-	2	2	3	4	4	7
44 X 72	-45	-	2	2	3	4	4	8
44 X 84	-40	-40	2	2	3	4	4	9
48 X 30	-87	-	2	2	2	4	4	5
48 X 40	-52	-	2	2	2	3	3	5
48 X 48	-51	-	2	2	2	4	4	5
48 X 50	-50	-	2	2	2	4	4	8
48 X 60	-46	-	2	2	3	4	4	7
48 X 72	-43	-45	2	2	3	4	4	9
52 X 30	-65	-	2	2	2	5	4	5
52 X 40	-53	-	2	2	2	3	3	5
52 X 48	-53	-	2	2	2	4	4	5
52 X 50	-50	-	2	2	3	4	4	8
52 X 60	-41	-	2	2	3	4	4	7
52 X 72	-40	-40	2	2	3	4	4	8

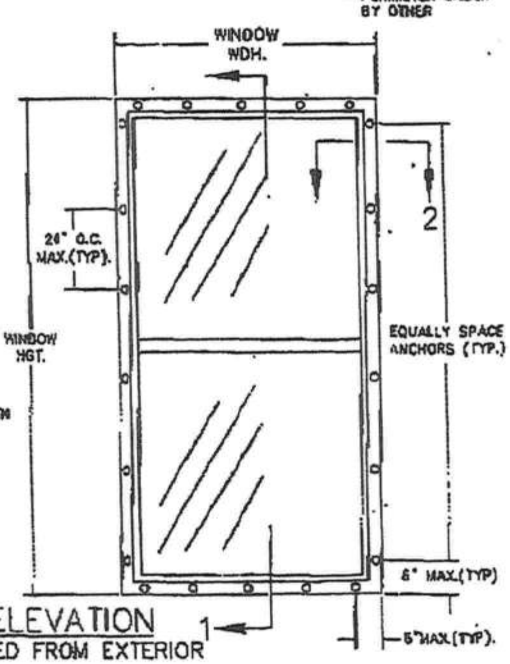
YLA JOB #08-053-K
YOOSEF LAVI, P.E.
CONSULTING ENGINEER
10300 N.Central Expressway
Suite # 322
DALLAS, TEXAS 75231
T: (214)340-0048 F:(214)340-0067







SECTION 1



ELEVATION 1
VIEWED FROM EXTERIOR

GENERAL NOTES:

) SHIM IF REQ'D FOR LEVELING.
) WINDOW FRAME MATERIAL ALLOY 6063.
) CAULK BEHIND WINDOW FLANGE AT HEAD SILL & JAMBS.
) CAULK FOR PERIMETER SEAL AROUND EXTERIOR OF WINDOW FIN.
) DESIGN PRESSURES SHOWN ARE BASED ON MULLION CAPACITY ONLY. DESIGN PRESSURES FOR GLASS THICKNESS MUST BE DETERMINED PER ASTM-E1300 GLASS CHART AND THE SMALLER VALUE MUST BE USED FOR DESIGN.
) WHERE WATER RESISTANCE TEST REQUIREMENT OF 15% OF DESIGN LOAD APPLIES, POSITIVE DESIGN LOADS WILL BE LIMITED TO 40.0 PSF DUE TO WATER TEST PRESSURE OF 6.0 PSF ACHIEVED ON TEST.
) INSTALLATION: FIN FRAME WINDOW IN WOOD FRAME OPENING WITH TWO BY WOOD STUDS USING .090"Ø NAILS OR SCREWS OF SUFFICIENT LENGTH TO PROVIDE MIN.1" EMBEDMENT IN TO SUBSTRATE.
) IF EXACT WINDOW SIZE IS NOT LISTED IN ANCHOR CHART, USE ANCHOR QUANTITY LISTED WITH NEXT LARGER SIZE.

SINGLE HUNG WINDOW COMPARATIVE ANALYSIS & ANCHOR CHART FOR GENERAL ALUMINUM SERIES 5000/5300

WINDOW SIZE (INCHES)	[illegible]	[illegible]	[illegible]			[illegible]		
WIDTH/ HEIGHT	DESIGN PRESSURE CAPACITY (PSF)	DESIGN PRESSURE CAPACITY (PSF)	HEAD	SILL	AT EACH JAMB	HEAD	SILL	AT EACH JAMB
30 X 30	-60	-	2	2	1	2	2	3
30 X 40	-60	-	2	2	2	2	2	4
30 X 45	-60	-	2	2	2	2	2	3
30 X 60	-60	-	2	2	1	2	2	3
30 X 69	-60	-	1	2	1	2	2	7
30 X 72	-70	-	2	2	3	2	1	7
30 X 84	-55	-	2	2	3	2	2	7
30 X 30	-60	-	2	2	1	2	2	4
30 X 40	-80	-	2	2	2	3	3	5
30 X 48	-60	-	2	2	3	3	3	7
30 X 50	-80	-	2	2	3	3	3	7
30 X 60	-60	-	2	2	3	3	3	4
30 X 72	-60	-	2	2	3	2	2	4
30 X 84	-52	-	2	2	3	2	1	8
38 X 30	-80	-	2	2	2	3	3	5
38 X 40	-75	-	2	2	2	4	3	5
38 X 48	-60	-	2	2	3	4	4	7
38 X 50	-60	-	2	2	3	4	4	7
38 X 60	-58	-	2	2	3	3	3	6
38 X 72	-53	-	2	2	3	3	3	5
38 X 84	-50	-	2	2	3	3	2	4
40 X 30	-74	-	2	2	2	3	3	5
40 X 40	-61	-	2	2	2	4	4	5
40 X 48	-60	-	2	2	3	4	4	7
40 X 50	-60	-	2	2	3	4	4	7
40 X 60	-52	-	2	2	3	4	4	7
40 X 72	-50	-	2	2	3	3	3	8
40 X 84	-43	-40	2	2	4	4	4	10
44 X 30	-70	-	2	2	2	4	4	5
44 X 40	-53	-	2	2	2	3	3	5
44 X 48	-62	-	2	2	3	4	4	6
44 X 50	-51	-	2	2	2	4	4	7
44 X 60	-10	-	2	2	3	4	4	7
44 X 72	-45	-	2	2	3	4	4	8
44 X 84	-40	-40	2	2	3	4	4	9
48 X 30	-67	-	2	2	2	4	4	5
48 X 40	-52	-	2	2	2	3	3	5
48 X 48	-58	-	2	2	2	4	4	5
48 X 50	-50	-	2	2	2	4	4	6
48 X 60	-46	-	2	2	3	4	4	7
48 X 72	-43	-45	2	2	3	4	4	7
52 X 30	-63	-	2	2	2	4	4	3
52 X 40	-53	-	2	2	2	3	3	4
52 X 48	-53	-	2	2	2	4	4	5
52 X 50	-60	-	2	2	3	4	4	5
52 X 60	-41	-	2	2	3	4	4	7
52 X 72	-40	-40	2	2	3	4	4	6

YLA JOB #08-053-K

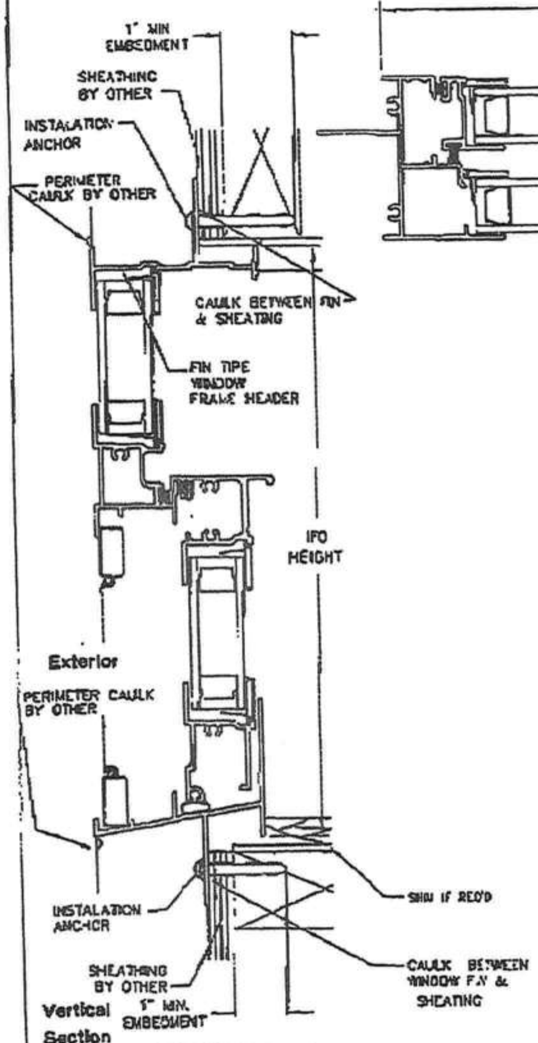
YOOSEF LAVI, P.E.
CONSULTING ENGINEER
10303 N.Central Expressway
Suite # 322
DALLAS, TEXAS 75231
P: (214)340-0040 F:(214)340-0057
FLORIDA LICENSE # 42371

YOOSEF LAVI LICENSE No. 42371 STATE OF FLORIDA PROFESSIONAL ENGINEER FEB 15 2006

GENERAL ALUMINUM Company of Texas, LP
1001 W. Crosby Rd. • Carrollton, TX 75006 • P.O. Box 819022 • Dallas, TX

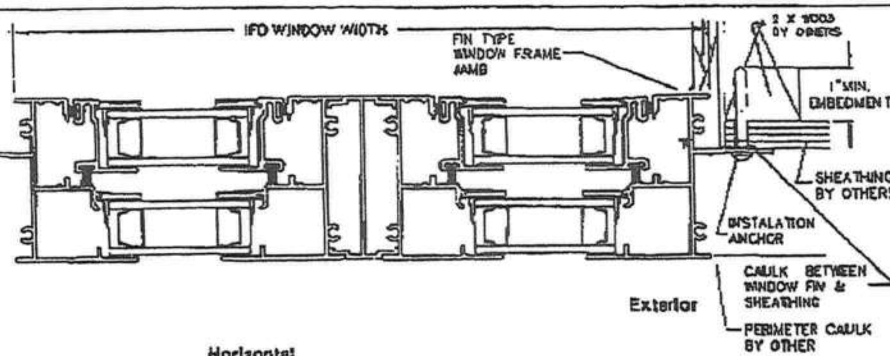
ASSEMBLY DRAWING
SERIES 5300 SINGLE HUNG WINDOW

DRN.	CHKD	SCALE	
AR	IP	N/A	
ALLOY	TEMPER	DATE 02/13/06	B DWG. NO. INSTALATION



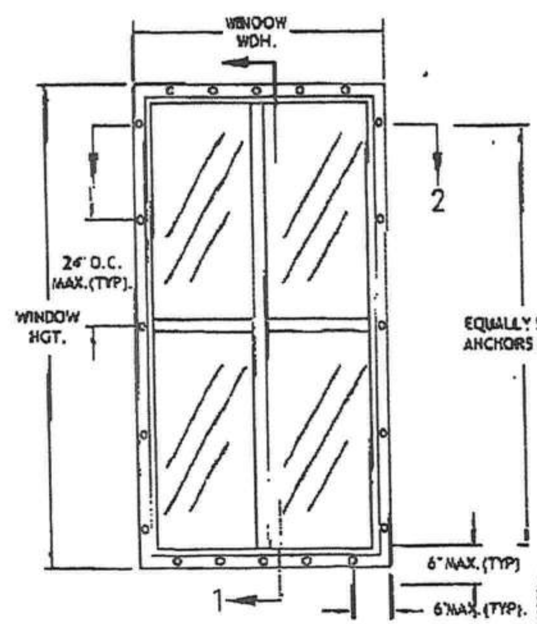
Vertical Section

SECTION 1



Horizontal Section

SECTION 2



ELEVATION
VIEWED FROM EXTERIOR

MINIMUM [illegible] SINGLE MULLION (T.L.) COMPARATIVE ANALYSIS & ANCHOR CHART FOR GENERAL ALUMINUM SERIES 5000/5300 [illegible]								
WINDOW SIZE (INCHES)	[illegible]	[illegible]	NUMBER OF [illegible] SCREWS REQUIRED IN FRAME 1" [illegible]			NUMBER OF [illegible] REQUIRED IN FRAME 1-1/2" [illegible]		
WIDTH/ HEIGHT	DESIGN PRESSURE CAPACITY (PSF)	DESIGN PRESSURE CAPACITY (PSF)	HEAD	SILL	AT EACH JAMB	HEAD	SILL	AT EACH JAMB
36 X 36	-80	-	2	2	2	6	8	4
36 X 44	-80	-	2	3	2	7	7	5
36 X 48	-80	-	3	3	2	7	9	3
36 X 52	-80	-	3	3	2	8	8	5
36 X 60	-80	-	3	3	2	3	8	6
36 X 72	-68	-	3	3	3	8	8	7
36 X 84	-	-41	3	3	2	9	9	7
48 X 36	-80	-	3	3	2	6	8	4
48 X 44	-83	-	3	3	2	7	9	5
48 X 48	-83	-	4	4	2	10	10	6
48 X 52	-80	-	4	4	3	10	10	7
48 X 60	-67	-	6	4	3	10	10	3
48 X 72	-54	-	3	5	3	9	9	3
48 X 84	-	-45	3	5	4	3	8	3
54 X 36	-73	-	3	3	2	3	8	4
54 X 44	-80	-	3	3	2	13	16	5
54 X 48	-81	-	3	3	2	13	15	6
54 X 52	-80	-	4	4	3	13	19	7
54 X 60	-80	-	4	4	3	[illegible]	[illegible]	3
54 X 72	-47	-	6	4	3	16	12	7
54 X 84	-	-47	4	4	3	10	18	3
60 X 36	-83	-	3	3	2	9	3	4
60 X 44	-82	-	3	3	2	8	3	5
60 X 48	-50	-	4	4	2	10	13	5
60 X 52	-56	-	4	4	2	10	13	6
60 X 60	-37	-	4	4	3	11	[illegible]	7
60 X 72	-46	-	4	4	3	10	[illegible]	7
60 X 84	-	-60	4	5	3	10	[illegible]	7
66 X 36	-65	-	3	3	2	4	8	4
66 X 44	-53	-	3	3	2	3	8	4
66 X 48	-51	-	3	3	3	3	8	5
66 X 52	-49	-	3	3	3	12	10	6
66 X 60	-48	-	4	4	2	12	10	6
66 X 72	-42	-	4	4	3	12	16	3
66 X 84	-	-37	4	4	3	10	10	7
72 X 36	-63	-	3	3	2	3	8	4
72 X 44	-57	-	3	3	3	3	8	4
72 X 48	-63	-	3	3	2	8	3	4
72 X 52	-63	-	3	3	2	8	8	5
72 X 60	-41	-	4	4	3	10	13	3
72 X 72	-50	-	4	4	3	11	11	7
72 X 84	-	-35	4	4	3	11	11	7

GENERAL NOTES:

1) SHIM IF REQ'D FOR LEVELING.
2) WINDOW FRAME MATERIAL ALLOY 6063.
3) CAULK BEHIND WINDOW FLANGE AT HEAD SILL & JAMBS.
4) CAULK FOR PERIMETER SEAL AROUND EXTERIOR OF WINDOW FIN.
5) DESIGN PRESSURES SHOWN ARE BASED ON MULLION CAPACITY ONLY. DESIGN PRESSURES FOR GLASS THICKNESS MUST BE DETERMINED PER ASTM-E1300 GLASS CHART AND THE SMALLER VALUE MUST BE USED FOR DESIGN
6) WHERE WATER RESISTANCE TEST REQUIREMENT OF 15% OF DESIGN LOAD APPLIES, POSITIVE DESIGN LOADS WILL BE LIMITED TO 40.0 PSF DUE TO WATER TEST PRESSURE OF 6.0 PSF ACHIEVED ON TEST.
7) INSTALLATION: FIN FRAME WINDOW IN WOOD FRAME OPENING WITH TWO BY WOOD STUDS USING .080's NAILS OR SCREWS OF SUFFICIENT LENGTH TO PROVIDE MIN.1" EMBEDMENT IN TO SUBSTRATE.
8) IF EXACT WINDOW SIZE IS NOT LISTED IN ANCHOR CHART, USE ANCHOR QUANTITY LISTED WITH NEXT LARGER SIZE.

TLA JOB #08-033-F
YOOSEF LAW, P.C.
CONSULTING ENGINEER
[illegible] N Central Expressway
Suite # 322
DALLAS, TEXAS 75231
T. (214)340-0040 F:(214)340-0067
FLORIDA LICENSE # 42371

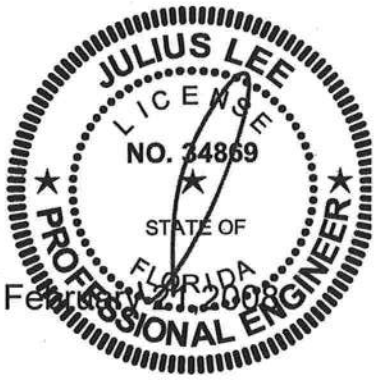
YOOSEF LAW ... LICENSE ... STATE OF ... FEB 26 2008 ... PROFESSIONAL ENGINEER

GENERAL ALUMINUM
Company of Texas, LP
1901 W. Crosby Rd. - Carrollton, TX 75006 - P.O. Box 819022 - Dallas, TX 75381

ASSEMBLY DRAWING
SERIES 5000/5300 (MULL TWIN) CONTINUOUS HEAD & SILL SINGLE HU[illegible]

DWN BY	CHK BY	SCALE	
FA/JB	AR	N/A	
REV		DATE	
-		02/13/08	B INSTALLATION 5000/5300 [illegible]

Builders FirstSource



Project Information for: L266212

Address :	662 SW CR 241
.	LULU, FL
County:	COLUMBIA
Truss Count:	7
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
RYE CONSTRUCTION COMPANY INC Florida License No. CGC1511121
Address: 3817 NW 28TH TER, GAINSVILLE, FL

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1938467	T01	2/21/08
2	J1938468	T01G	2/21/08
3	J1938469	T02	2/21/08
4	J1938470	T02G	2/21/08
5	J1938471	T03	2/21/08
6	J1938472	T03A	2/21/08
7	J1938473	T03G	2/21/08

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T01	SPECIAL	31	1	J1938467
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:25:57 2008 Page 1

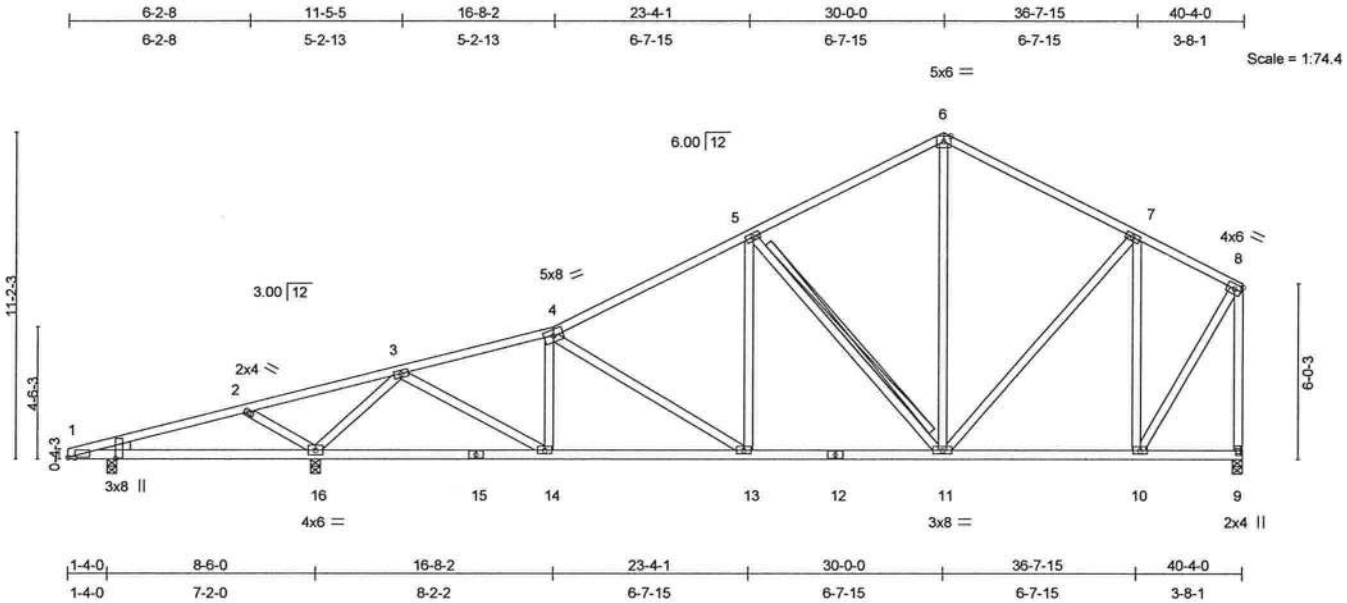


Plate Offsets (X,Y): [1:0-2-13,0-0-1], [1:0-0-5,1-7-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	0.27	1-16	>370	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.15	1-16	>659	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 247 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 16=1528/0-4-0, 9=962/0-4-0, 1=71/0-4-0
Max Horz 1=246(load case 6)
Max Uplift 16=-534(load case 6), 9=-186(load case 6), 1=-125(load case 4)
Max Grav 16=1528(load case 1), 9=962(load case 1), 1=84(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-307/384, 2-3=-605/780, 3-4=-1271/618, 4-5=-1170/657, 5-6=-772/548, 6-7=-769/544, 7-8=-495/305, 8-9=-947/560
BOT CHORD 1-16=-325/25, 15-16=-312/454, 14-15=-312/454, 13-14=-722/1215, 12-13=-575/977, 11-12=-575/977, 10-11=-238/420, 9-10=-3/5
WEBS 2-16=-475/450, 3-16=-1662/1011, 3-14=-460/862, 4-14=-364/310, 4-13=-293/174, 5-13=-65/304, 5-11=-564/407, 6-11=-186/328, 7-11=-102/316, 7-10=-629/422, 8-10=-450/796

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
					J1938467
L266212	T01	SPECIAL	31	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.40, 1 = 0.19, 2 = 0.33, 3 = 0.58, 4 = 0.52, 5 = 0.41, 6 = 0.59, 7 = 0.41, 8 = 0.44, 9 = 0.34, 10 = 0.62, 11 = 0.56, 12 = 0.35 , 13 = 0.34, 14 = 0.48, 15 = 0.22 and 16 = 0.35

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 16, 186 lb uplift at joint 9 and 125 lb uplift at joint 1.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T01G	GABLE	2	1	J1938468
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:25:59 2008 Page 1

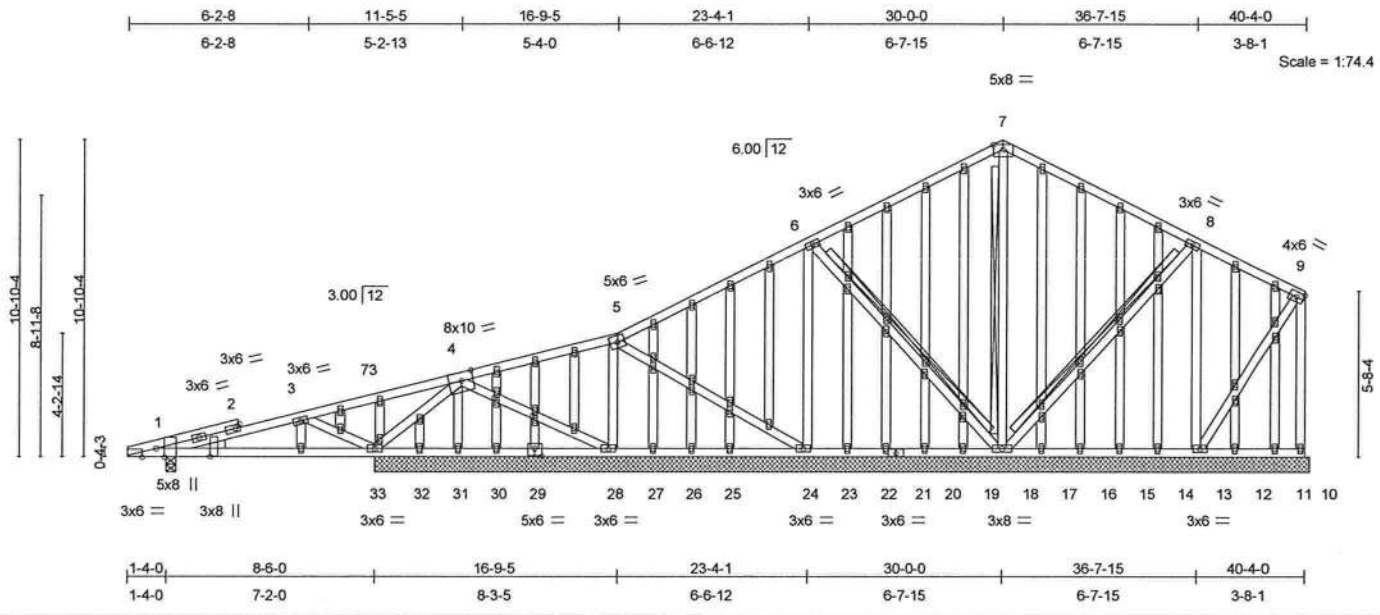


Plate Offsets (X,Y): [1:0-5-12,Edge], [1:0-3-4,1-10-4], [1:0-3-8,Edge], [29:0-3-0,0-3-0], [52:0-1-12,0-1-0], [54:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.11	1-33	>822	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.09	1-33	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.28	Horz(TL)	-0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 417 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-18, 7-18, 8-18
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 33=749/32-0-0, 28=416/32-0-0, 24=293/32-0-0, 18=488/32-0-0, 13=219/32-0-0, 10=114/32-0-0, 19=13/32-0-0, 20=13/32-0-0, 22=14/32-0-0, 23=9/32-0-0, 25=23/32-0-0, 26=12/32-0-0, 27=3/32-0-0, 30=32/32-0-0, 31=20/32-0-0, 32=-45/32-0-0, 17=13/32-0-0, 16=13/32-0-0, 15=13/32-0-0, 14=13/32-0-0, 12=14/32-0-0, 11=7/32-0-0, 1=241/0-4-0
Max Horz 1=311(load case 6)
Max Uplift 33=-640(load case 4), 28=-241(load case 6), 24=-207(load case 6), 18=-289(load case 6), 13=-167(load case 7), 10=-24(load case 6), 31=-30(load case 4), 32=-89(load case 2), 1=-199(load case 4)
Max Grav 33=751(load case 10), 28=416(load case 1), 24=306(load case 10), 18=488(load case 1), 13=231(load case 11), 10=115(load case 11), 19=40(load case 2), 20=39(load case 2), 22=43(load case 2), 23=27(load case 2), 25=68(load case 2), 26=37(load case 2), 27=9(load case 2), 30=99(load case 2), 31=46(load case 2), 32=114(load case 4), 17=40(load case 2), 16=40(load case 2), 15=40(load case 2), 14=40(load case 2),

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T01G	GABLE	2	1	J1938468 Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-251/95, 2-3=-212/85, 3-73=-476/382, 4-73=-486/421, 4-5=-153/46, 5-6=-120/44, 6-7=-77/104, 7-8=-75/98, 8-9=-48/31, 9-10=-111/33

BOT CHORD 1-33=-302/224, 32-33=-90/35, 31-32=-90/35, 30-31=-90/35, 29-30=-90/35, 28-29=-90/35, 27-28=-39/58, 26-27=-39/58, 25-26=-39/58, 24-25=-39/58, 23-24=-33/63, 22-23=-33/63, 21-22=-33/63, 20-21=-33/63, 19-20=-33/63, 18-19=-33/63, 17-18=0/33, 16-17=0/33, 15-16=0/33, 14-15=0/33, 13-14=0/33, 12-13=-1/2, 11-12=-1/2, 10-11=-1/2

WEBS 3-33=-665/666, 4-33=-544/439, 4-28=-47/77, 5-28=-360/271, 5-24=0/41, 6-24=-303/236, 6-18=-73/127, 7-18=-381/215, 8-18=-60/129, 8-13=-269/178, 9-13=-1/60

JOINT STRESS INDEX

1 = 0.22, 1 = 0.19, 1 = 0.17, 2 = 0.00, 2 = 0.26, 2 = 0.26, 3 = 0.50, 4 = 0.22, 4 = 0.00, 5 = 0.72, 6 = 0.40, 7 = 0.61, 8 = 0.40, 9 = 0.30, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.42, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.56, 19 = 0.33, 20 = 0.33, 21 = 0.15, 22 = 0.33, 23 = 0.33, 24 = 0.34, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.34, 29 = 0.19, 30 = 0.33, 31 = 0.33, 32 = 0.33, 33 = 0.34, 33 = 0.33, 34 = 0.33, 35 = 0.33, 35 = 0.33, 36 = 0.33, 37 = 0.33, 37 = 0.33, 38 = 0.33, 39 = 0.33, 39 = 0.33, 40 = 0.33, 41 = 0.33, 41 = 0.33, 42 = 0.33, 43 = 0.33, 44 = 0.33, 45 = 0.33, 45 = 0.33, 46 = 0.33, 46 = 0.33, 47 = 0.33, 48 = 0.33, 48 = 0.33, 49 = 0.33, 50 = 0.33, 51 = 0.33, 52 = 0.39, 52 = 0.33, 53 = 0.33, 54 = 0.39, 54 = 0.33, 55 = 0.33, 56 = 0.33, 57 = 0.33, 58 = 0.33, 59 = 0.33, 60 = 0.33, 61 = 0.33, 62 = 0.33, 62 = 0.33, 63 = 0.33, 64 = 0.33, 64 = 0.33, 65 = 0.33, 66 = 0.33, 66 = 0.33, 67 = 0.33, 68 = 0.33, 68 = 0.33, 69 = 0.33, 70 = 0.33, 70 = 0.33, 71 = 0.33, 72 = 0.33 and 72 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 640 lb uplift at joint 33, 241 lb uplift at joint 28, 207 lb uplift at joint 24, 289 lb uplift at joint 18, 167 lb uplift at joint 13, 24 lb uplift at joint 10, 30 lb uplift at joint 31, 89 lb uplift at joint 32 and 199 lb uplift at joint 1.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Gable truss supports 16" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-73=-79(F=-25), 5-73=-54, 5-7=-54, 7-9=-54, 1-10=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Ray Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T02	MONO TRUSS	19	1	J1938469
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:26:00 2008 Page 1

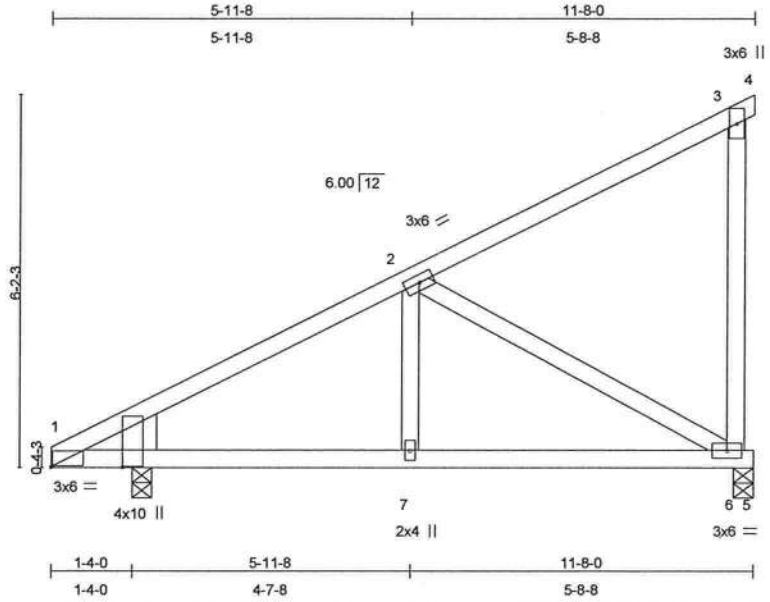


Plate Offsets (X,Y): [1:0-0-4,Edge], [1:0-0-0,1-2-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.12	1-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.06	1-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.30	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 62 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-3 oc bracing.

REACTIONS (lb/size) 6=366/0-4-0, 1=359/0-4-0
Max Horz 1=187(load case 6)
Max Uplift 6=-292(load case 6), 1=-191(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-520/571, 2-3=-91/39, 3-4=-2/0, 3-6=-122/130
BOT CHORD 1-7=-752/405, 6-7=-752/405, 5-6=0/0
WEBS 2-7=-363/194, 2-6=-447/830

JOINT STRESS INDEX
1 = 0.51, 1 = 0.09, 2 = 0.39, 3 = 0.27, 6 = 0.55 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T02	MONO TRUSS	19	1	J1938469 Job Reference (optional)

NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 6 and 191 lb uplift at joint 1.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T02G	GABLE	1	1	J1938470
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:26:02 2008 Page 1

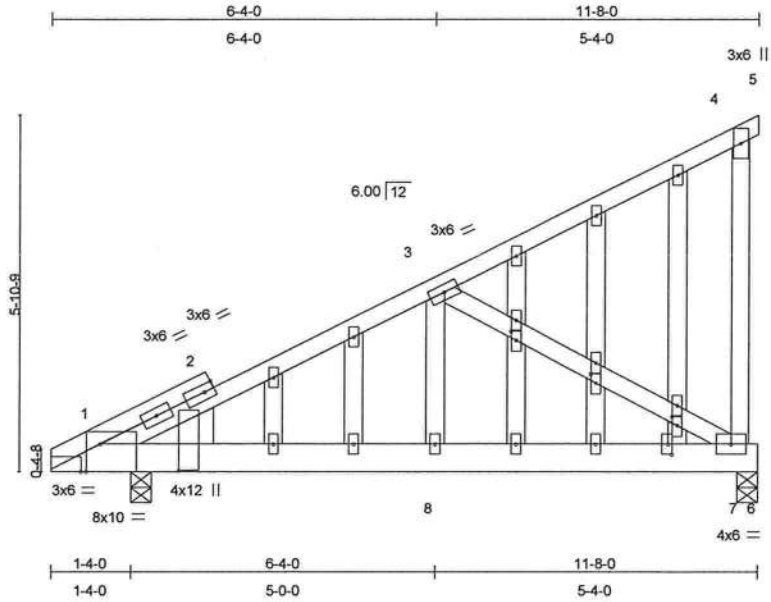


Plate Offsets (X,Y): [1:0-2-13,Edge], [1:0-5-4,1-3-8], [1:0-3-14,Edge], [10:0-2-0,0-0-12], [11:0-1-12,0-1-0], [14:0-1-12,0-1-0], [17:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.07	1-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.04	1-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-1 oc bracing.

REACTIONS (lb/size) 1=476/0-4-0, 7=491/0-4-0
Max Horz 1=241(load case 6)
Max Uplift 1=-362(load case 6), 7=-498(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-687/836, 2-3=-607/808, 3-4=-125/53, 4-5=-5/4, 4-7=-173/219
BOT CHORD 1-8=-949/542, 7-8=-955/542, 6-7=0/0
WEBS 3-8=-365/194, 3-7=-597/1057

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1300 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
1 = 0.38, 1 = 0.22, 1 = 0.00, 2 = 0.00, 2 = 0.47, 2 = 0.47, 3 = 0.50, 4 = 0.37, 7 = 0.35, 8 = 0.14, 9 = 0.00, 10 = 0.00, 11 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00 and 21 = 0.00

Continued on page 2

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
					J1938470
L266212	T02G	GABLE	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 1-4-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 1 and 498 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Gable truss supports 16" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-79(F=-25), 4-5=-39(F=-25), 1-6=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T03	COMMON	6	1	J1938471
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:26:03 2008 Page 1

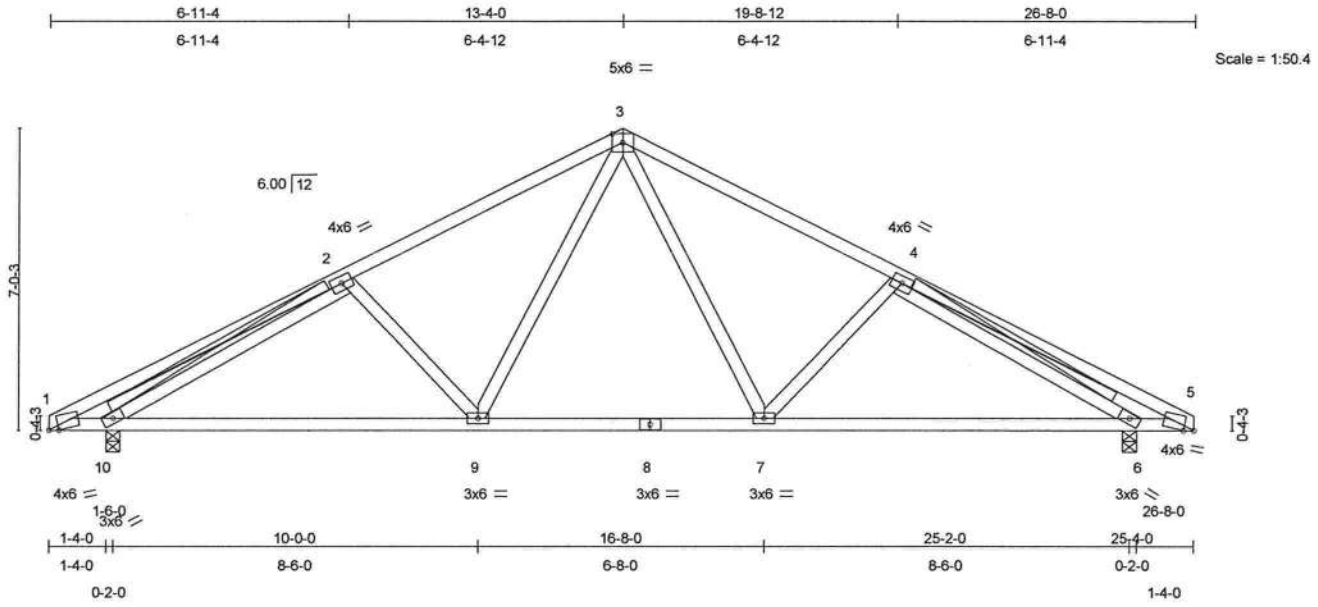


Plate Offsets (X,Y): [1:0-2-13,0-0-11], [5:0-2-13,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.33	7-9	>883	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.27	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.79	Horz(TL)	-0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 138 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-10, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 10=1020/0-4-0, 6=1020/0-4-0
Max Horz 10=86(load case 5)
Max Uplift 10=-636(load case 6), 6=-636(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-307/506, 2-3=-1360/1915, 3-4=-1360/1915, 4-5=-307/506
BOT CHORD 1-10=-349/252, 9-10=-1577/1280, 8-9=-1090/949, 7-8=-1090/949, 6-7=-1577/1280, 5-6=-349/252
WEBS 2-9=-234/196, 3-9=-794/472, 3-7=-794/472, 4-7=-234/196, 2-10=-1384/1443, 4-6=-1384/1443

JOINT STRESS INDEX
1 = 0.89, 2 = 0.39, 3 = 0.62, 4 = 0.39, 5 = 0.89, 6 = 0.47, 7 = 0.44, 8 = 0.69, 9 = 0.44 and 10 = 0.47

Julius Lee
Truss Design Engineer
Florida PE No. 31898
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T03	COMMON	6	1	J1938471 Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 636 lb uplift at joint 10 and 636 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-5=-54, 1-9=-10, 7-9=-60(F=-50), 5-7=-10

Julius Lee
Truss Design Engineer
Florida PE# No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T03A	COMMON	5	1	J1938472
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:26:04 2008 Page 1

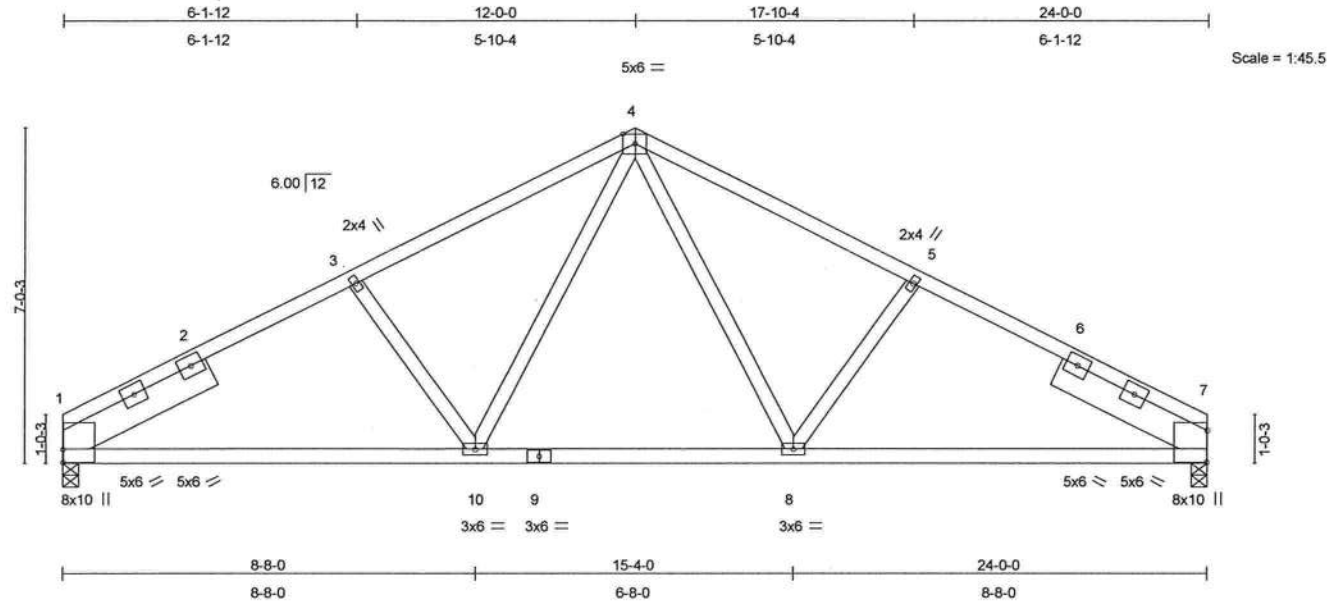


Plate Offsets (X,Y): [1:0-3-4,0-0-2], [7:0-8-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.35	8-10	>822	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.27	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.79	Horz(TL)	-0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.1D
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 8 SYP No.1D 3-6-7, Right 2 X 8 SYP No.1D 3-6-7

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-7-5 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-1-9 oc bracing.

REACTIONS (lb/size) 1=935/0-4-0, 7=935/0-4-0
Max Horz 1=86(load case 5)
Max Uplift 1=-578(load case 6), 7=-578(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-1517/1908, 2-3=-1423/1939, 3-4=-1312/1940, 4-5=-1312/1940, 5-6=-1423/1939, 6-7=-1517/1908
BOT CHORD	1-10=-1558/1220, 9-10=-1121/949, 8-9=-1121/949, 7-8=-1558/1220
WEBS	3-10=-189/148, 4-10=-792/458, 4-8=-792/458, 5-8=-189/148

JOINT STRESS INDEX

1 = 0.54, 1 = 0.34, 1 = 0.34, 2 = 0.00, 3 = 0.33, 4 = 0.70, 5 = 0.33, 6 = 0.00, 7 = 0.54, 7 = 0.34, 7 = 0.34, 8 = 0.44, 9 = 0.45 and 10 = 0.44

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 21,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T03A	COMMON	5	1	J1938472 Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 578 lb uplift at joint 1 and 578 lb uplift at joint 7.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 1-10=-10, 8-10=-60(F=-50), 7-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Ray Blvd
Boynton Beach, FL 33435

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
L266212	T03G	GABLE	1	1	J1938473
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Feb 21 15:26:06 2008 Page 1

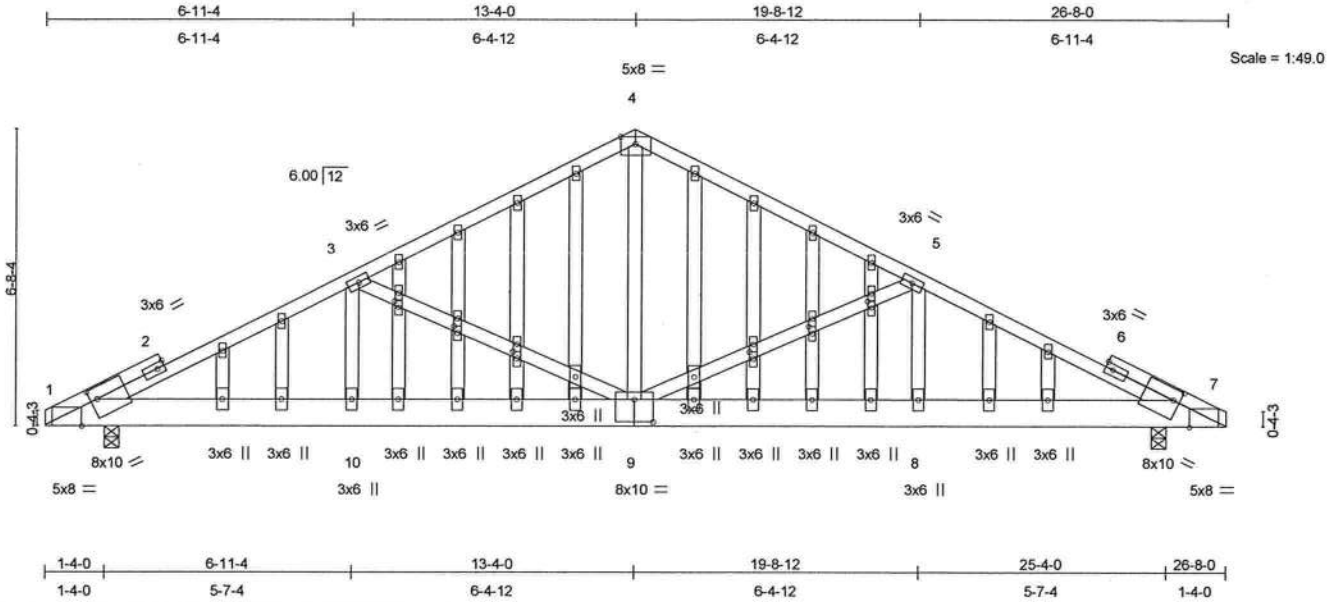


Plate Offsets (X,Y): [1:0-1-12,0-3-0], [1:0-4-5,Edge], [7:0-1-12,0-3-0], [7:0-4-5,Edge], [9:0-5-0,0-6-0], [14:0-1-12,0-1-0], [17:0-1-12,0-1-0], [20:0-1-12,0-1-0], [32:0-1-12,0-1-0], [35:0-1-12,0-1-0], [38:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.28	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.21	9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.87	Horz(TL)	-0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 224 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-13 oc bracing.

REACTIONS (lb/size) 1=1172/0-4-0, 7=1172/0-4-0
Max Horz 1=90(load case 5)
Max Uplift 1=-984(load case 6), 7=-984(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2346/3049, 2-3=-2286/3048, 3-4=-1553/2058, 4-5=-1553/2058, 5-6=-2286/3048, 6-7=-2346/3049
BOT CHORD 1-10=-2677/2045, 9-10=-2677/2045, 8-9=-2677/2045, 7-8=-2677/2045
WEBS 3-10=-418/259, 3-9=-842/1226, 4-9=-1344/799, 5-9=-842/1226, 5-8=-418/259

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Crandall Bay Blvd.
Boynton Beach, FL 33435

JOINT STRESS INDEX
1 = 0.85, 1 = 0.09, 2 = 0.00, 2 = 0.00, 2 = 0.92, 3 = 0.54, 4 = 0.73, 5 = 0.54, 6 = 0.00, 6 = 0.92, 6 = 0.00, 7 = 0.85, 7 = 0.09, 8 = 0.15, 9 = 0.39, 10 = 0.15, 11 = 0.49, 11 = 0.00, 12 = 0.33, 13 = 0.15, 14 = 0.39, 14 = 0.33, 15 = 0.33, 16 = 0.15, 17 = 0.39, 17 = 0.33, 18 = 0.33, 19 = 0.15, 20 = 0.39, 20 = 0.33, 21 = 0.33, 22 = 0.15, 23 = 0.15, 24 = 0.33, 25 = 0.15, 26 = 0.33, 27 = 0.33, 28 = 0.15, 29 = 0.49, 29 = 0.00, 30 = 0.33, 31 = 0.15, 32 = 0.39, 32 = 0.33, 33 = 0.33, 34 = 0.15, 35 = 0.39, 35 = 0.33, 36 = 0.33, 37 = 0.15, 38 = 0.39, 38 = 0.33, 39 = 0.15, 40 = 0.33, 41 = 0.15 and 42 = 0.33

Continued on page 2

February 21,2008

Job	Truss	Truss Type	Qty	Ply	STEVE COODY
					J1938473
L266212	T03G	GABLE	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 984 lb uplift at joint 1 and 984 lb uplift at joint 7.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Gable truss supports 16" max. rake gable overhang.

LOAD CASE(S) Standard

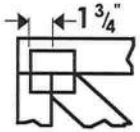
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-79(F=-25), 4-7=-79(F=-25), 1-7=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

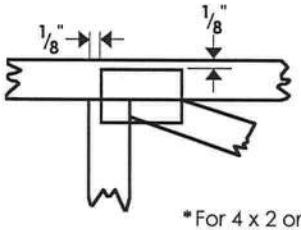
February 21,2008

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

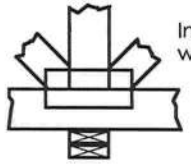
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

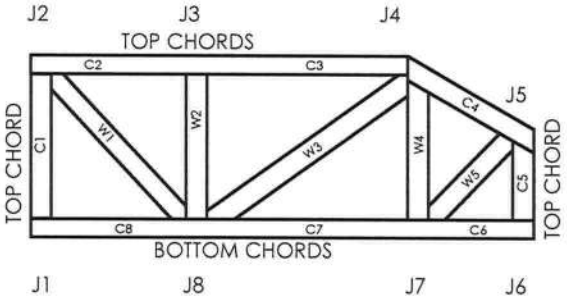
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

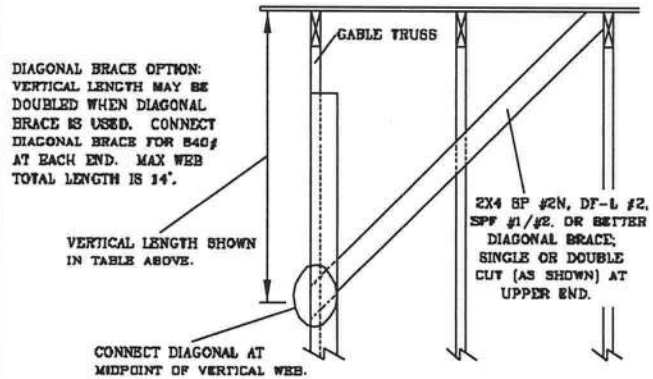
SOUTHERN PINE
#3
STUD
STANDARD

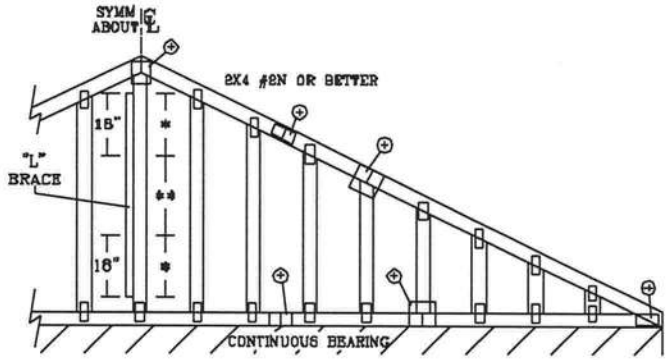
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	8' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

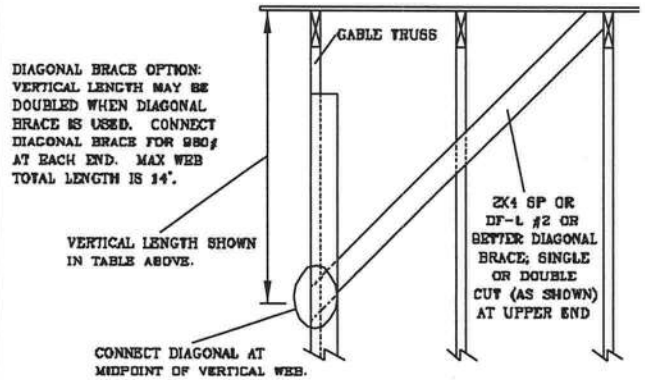
SOUTHERN PINE
#3
STUD
STANDARD

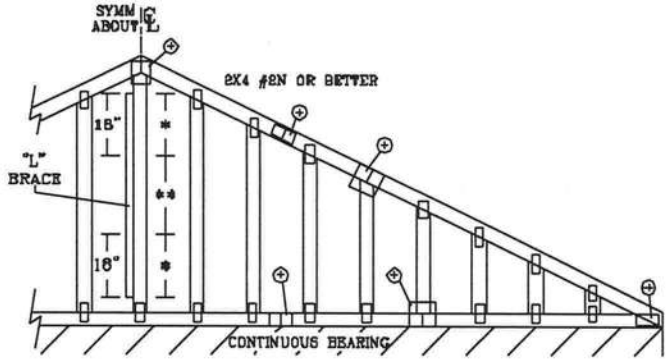
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

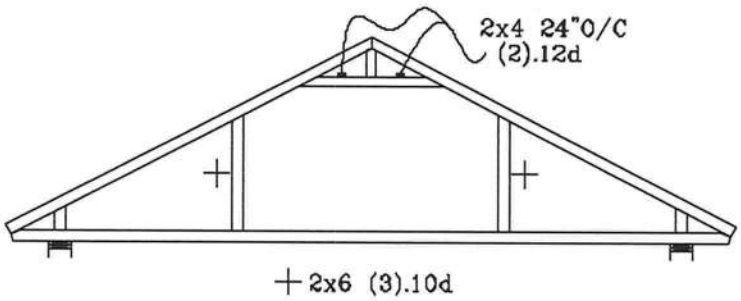
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

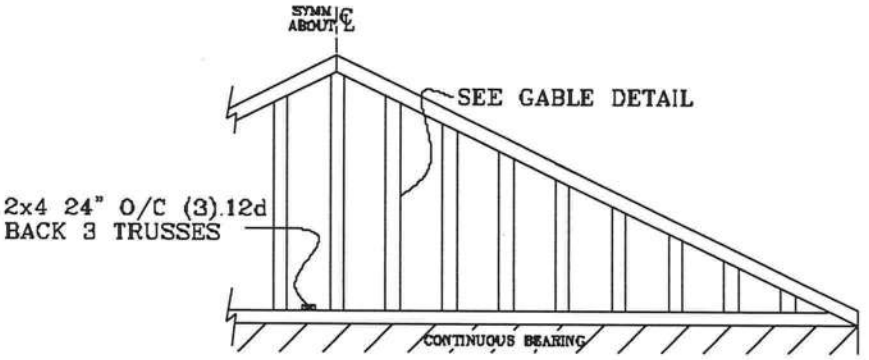
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 30' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

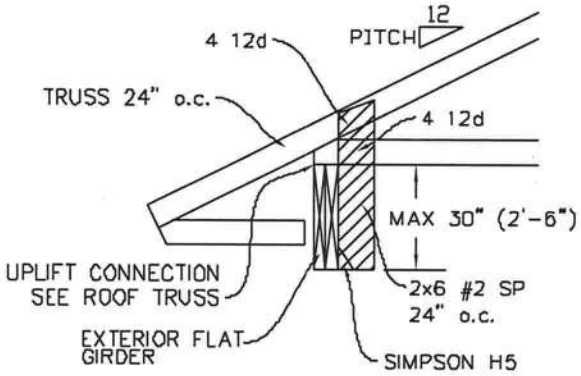


GABLE END TRUSS DETAIL

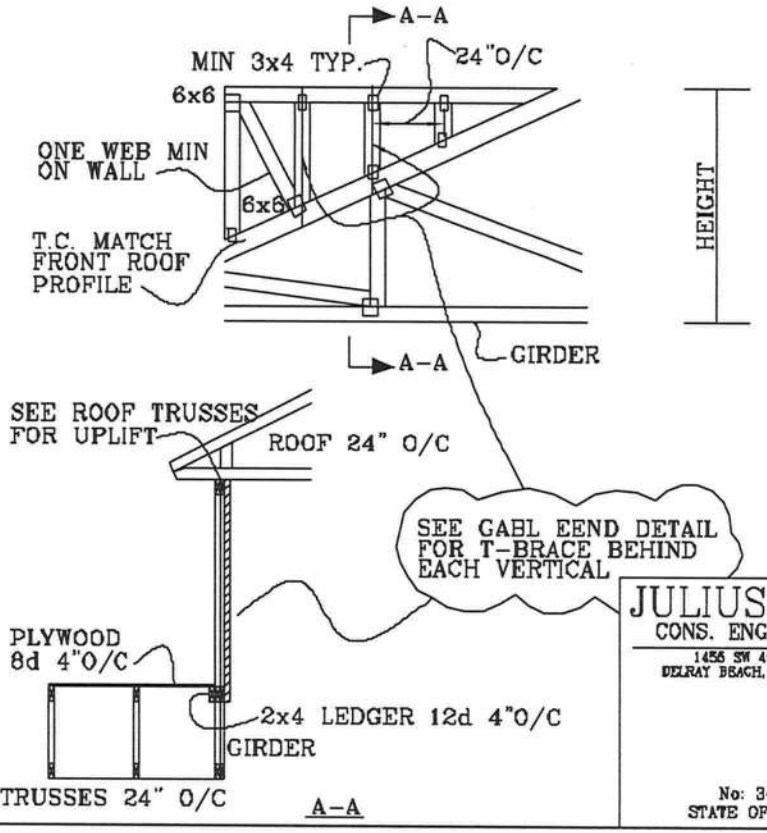


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

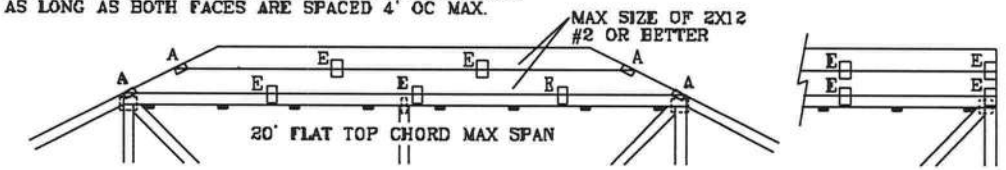
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

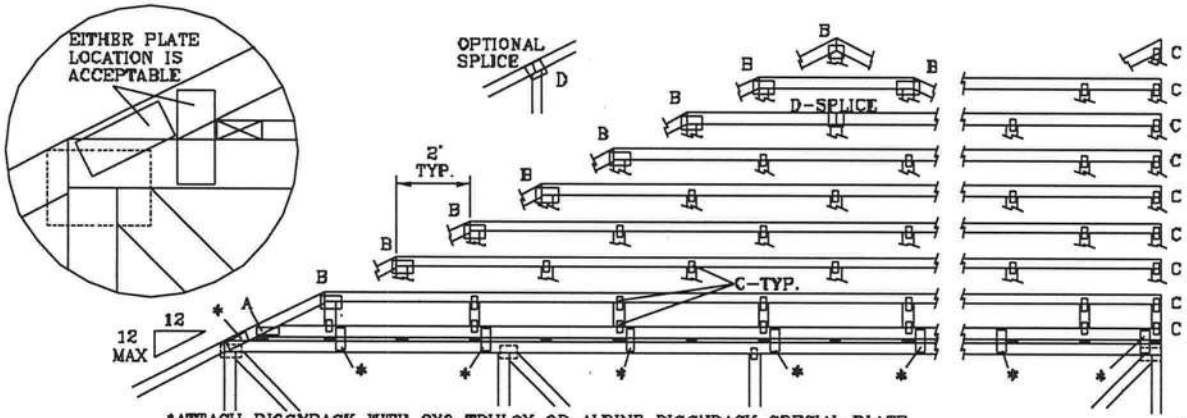
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

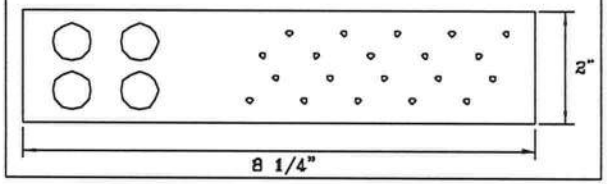
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X8	5X8	5X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X8
E	4X8 OR 3X8 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

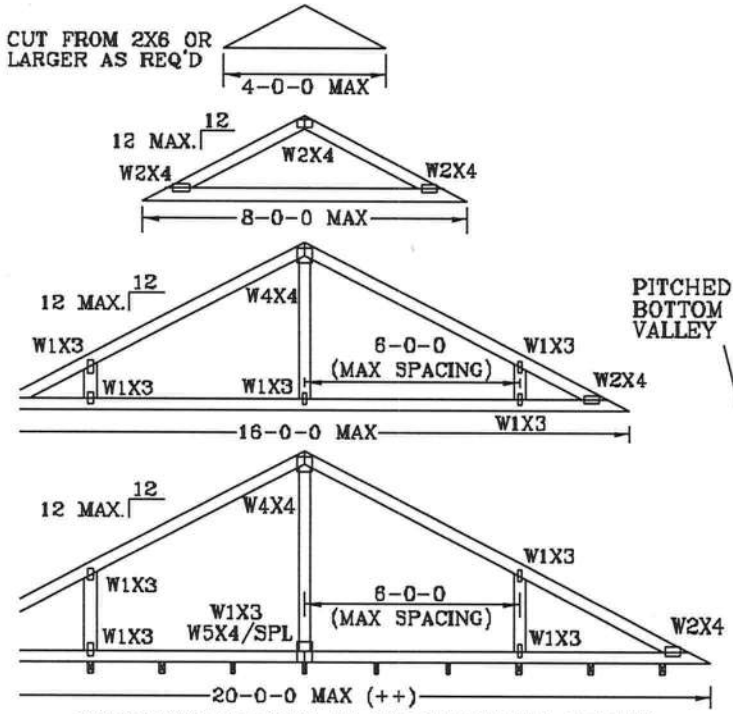
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

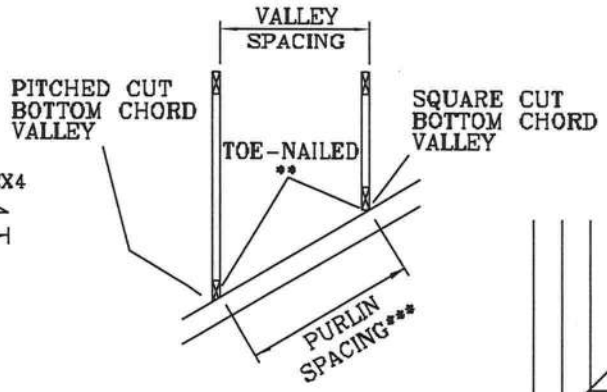
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

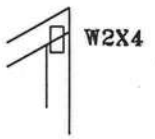
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

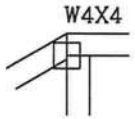


SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

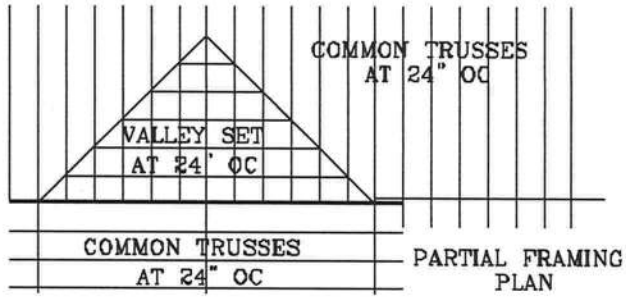




OPTIONAL STUB END DETAIL



OPTIONAL HIP JOINT DETAIL



PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

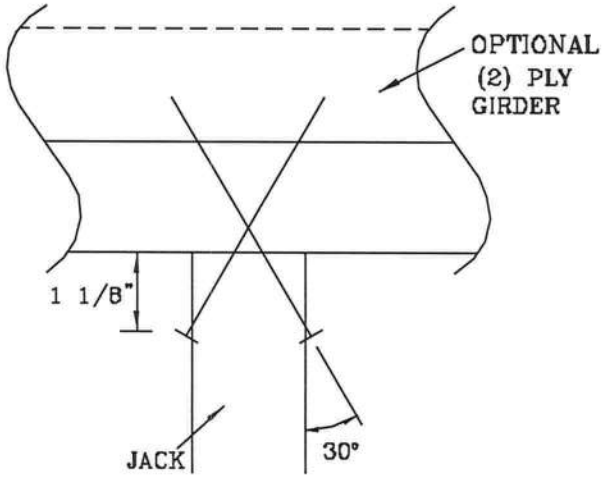
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

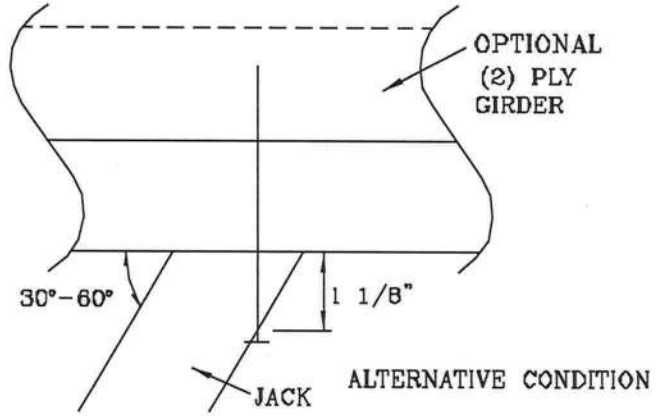
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATIONS, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

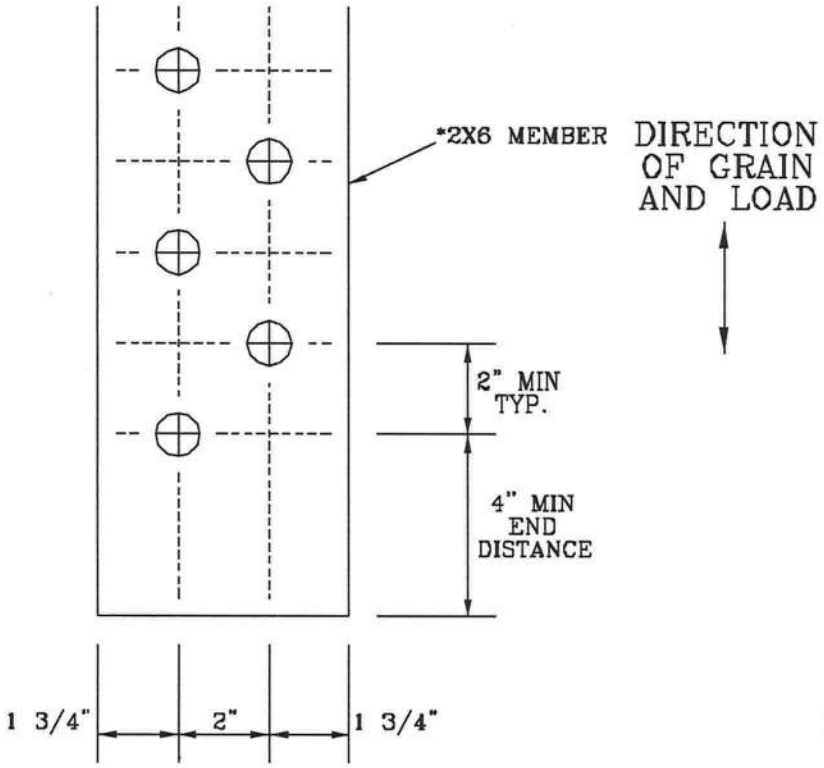
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

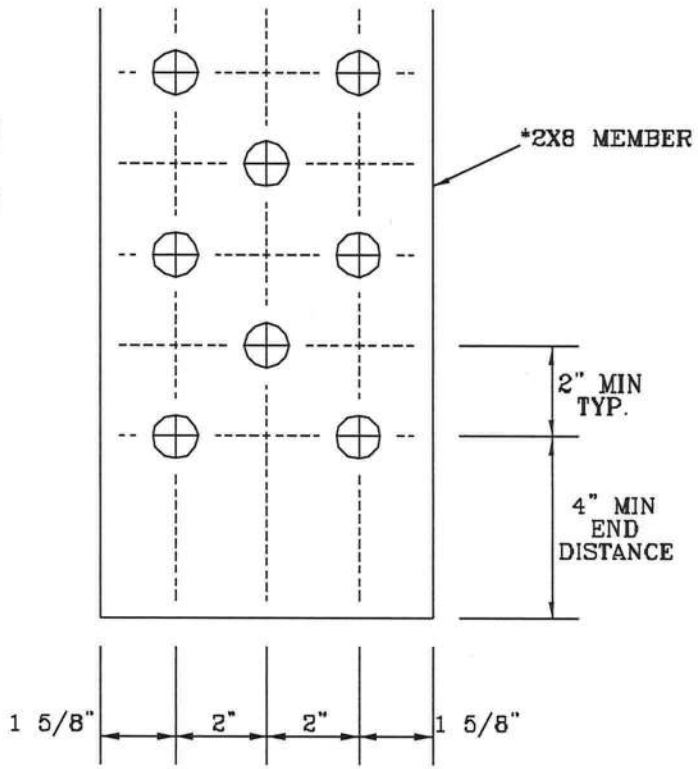
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

TRULOX CONNECTION DETAIL

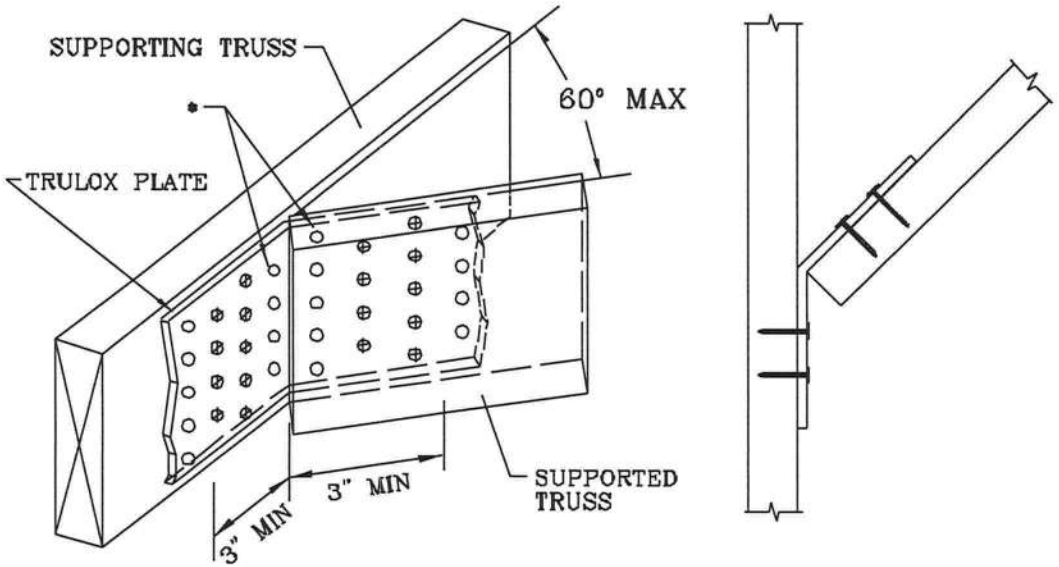
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

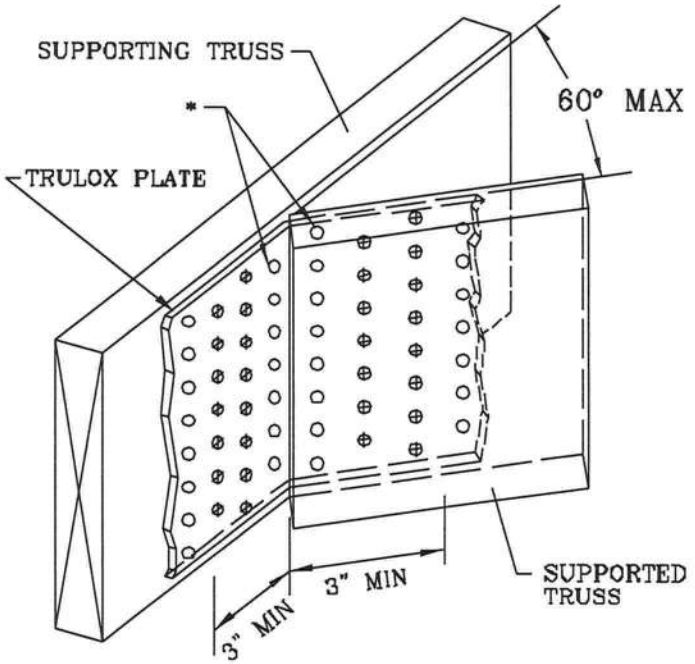
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#



MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

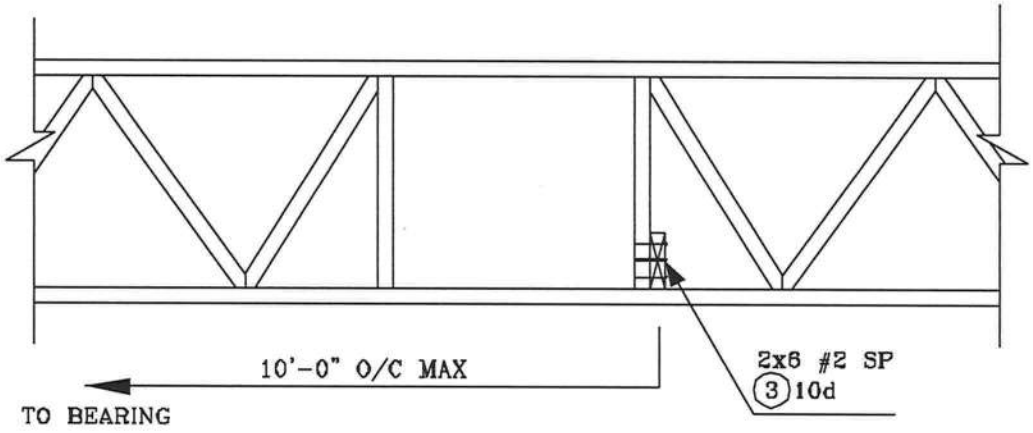
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 537193 AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 537193 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

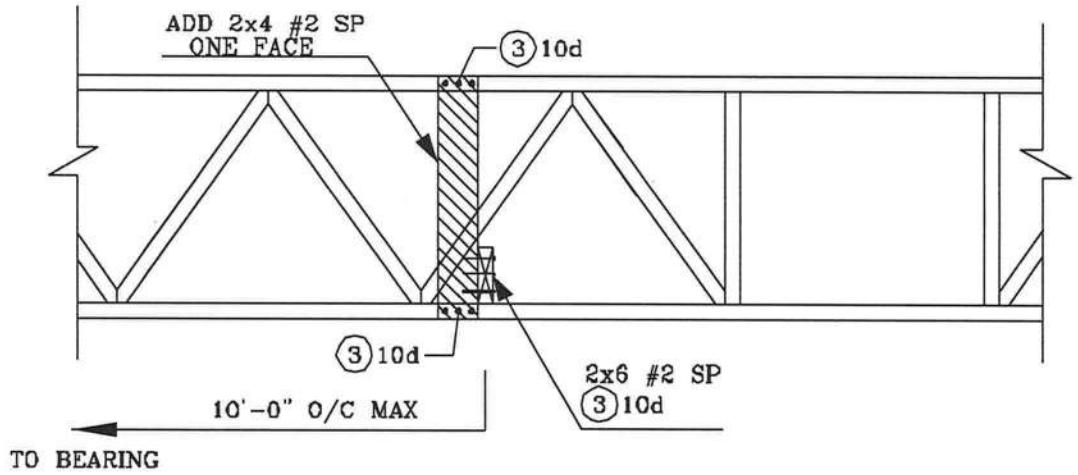
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS

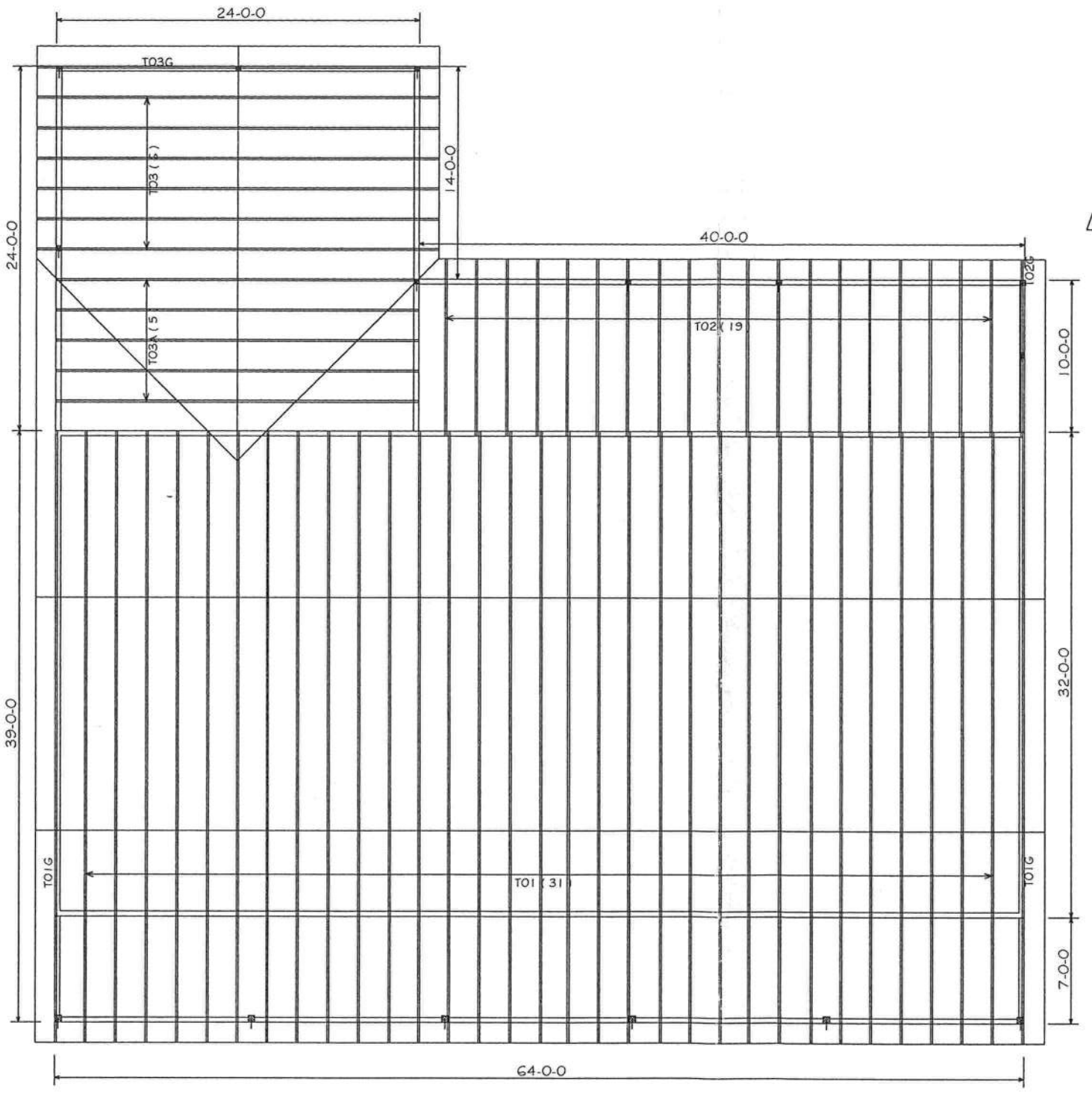


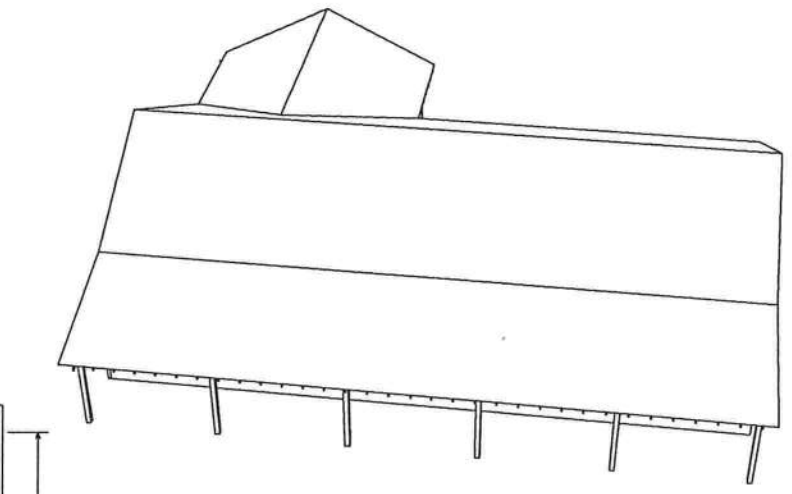
ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA





TRUSS INFORMATION:

ROOF PITCH: 3-6/12

CEILING: ALL FLAT (PER PLANS)

CANTILEVER: 1' 4"

HANGER LIST:
NONE

VALLEY:
FRAMED BY OTHERS

NOTE: ALL BEARING WALLS PER PLANS

BEARING HEIGHT SCHEDULE

8' 0"

NOTES:

1) REFER TO HIB-91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 386-755-6894 FAX: 386-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: N/A		
LEGAL ADDRESS: LAKE BUTLER, FL		
MODEL: STEVE COODY		REVISION: SCALE: NTS
DATE: 1/25/08	DRAWN BY: JP	JOB #: L266212

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-4S-18-10550-001 **Building permit No.** 000026962

Use Classification SFD,UTILITY **Fire:** 0.00

Permit Holder RYE CONSTRUCTION CO **Waste:**

Owner of Building STEVE & BARBARA COODY **Total:** 0.00

Location: 662 SE CR 241, LULU, FL

Date: 11/14/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. # 26962

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 321 N.W. Cole Terrace, Suite 107 City Lake City State FL Zip 32055
Company Business License No. JB100478 Company Phone No. 386-755-3611 • 352-494-5751
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: R & E. Construction (cell # 352-258-8673) Company Phone No. 352-378-3006

Section 3: Property Information (Steve & Barbara Coody) - 758-7639

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 662 S.E. CR 241 Lulu Fla. 32061.

Type of Construction (More than one box may be checked) ☐ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside Inside Type of Fill

Section 4: Treatment Information

Date(s) of Treatment(s) 6/3/08
Brand Name of Product(s) Used Bifen
EPA Registration No. 53883-189
Approximate Final Mix Solution % .06.
Approximate Size of Treatment Area: Sq. ft. 3,472 Linear ft. Linear ft. of Masonry Voids
Approximate Total Gallons of Solution Applied 350 gals
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☑ Yes ☐ No upon Completion

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Wes Certification No. (if required by State law)

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 6/3/08

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Cal-Tech Testing, Inc. FOUNDATIONS • ASPHALT • SOILS • CONCRETE • ENGINEERING • GEOTECHNICAL • ENVIRONMENTAL

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254
2230 Greensboro Hwy., Quincy, FL 32351

Lake City • (386) 755-3633
Fax • (386) 752-5456

Jacksonville • (904) 381-8901
Fax • (904) 381-8902

Quincy • (850) 442-3495
Fax • (850) 442-4008

JOB NO.: 08-261
DATE TESTED: 05-06-08

26962

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ (D-2922) Nuclear _____ (D-2937) Drive Cylinder _____ Other

PROJECT: COODY STEVE ~~[illegible]~~ RESIDENCE

CLIENT: WILSON'S SITE PREP

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): _____ SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: S. OSTEEN

MODIFIED (ASTM D-1557): ✓ STANDARD (ASTM D-698): _____

TEST NO.	TEST LOCATION	TEST: ✓DEPTH ___ELEV. ___LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
1B	CENTER OF W FOOTER	12"	1	107.8	101.8	5.9	96.0
2B	CENTER OF S FOOTER	12"	1	108.2	100.8	7.3	95.1
3B	CENTER OF E FOOTER	12"	1	108.0	100.8	7.1	95.1
4B	CENTER OF N FOOTER	12"	1	109.5	100.8	8.6	95.1

REMARKS: _____

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
1		106.0	11.6

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.