DATE 02/29/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026807

APPLICANT MIKE MCCRAY PHONE 365-1750

ADDRESS 712 SE RACE TRACK RD LAKE CITY FL 32025

OWNER MIKE MCCRAY PHONE 386-365-1750

ADDRESS 712 SE RACE TRACK RD LAKE CITY FL 32025

CONTRACTOR OWNER BUILDER PHONE

LOCATION OF PROPERTY 441 SOUTH, L RACE TRACK RD, 3RD DRIVE ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 187200.00

HEATED FLOOR AREA 2112.00 TOTAL AREA 3744.00 HEIGHT 22.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 21-4S-17-08633-002 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 5.49

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Mike McCray

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	08-0114	BK	JH	N

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD

Check # or Cash 1016

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 940.00 CERTIFICATION FEE $ 18.72 SURCHARGE FEE $ 18.72

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 1052.44

INSPECTORS OFFICE L. Hall CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

clc# 5472

For Office Use Only Application # 0801-130 Date Received 1/25 By JW Permit # 26807

Zoning Official CJS Date 2/19/08 Flood Zone X FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE River N/A Plans Examiner OKJH Date 1-31-08

Comments

NOC EH Deed or PA Site Plan State Road Info Parent Parcel #

Dev Permit # In Floodway Letter of Authorization from Contractor

Unincorporated area Incorporated area Town of Fort White Town of Fort White Compliance letter

Septic Permit No. 08-0114 Fax 386-755-1220

Name Authorized Person Signing Permit Mike McCray Phone 386-755-8221

Address 712 SE Race Track Road, L.C, Fl 32025

Owners Name Mike McCray Phone 386 365-1750

911 Address 712 SE Race Track Rd., L.C, Fl 32025

Contractors Name Owner Phone 386 365-1750

Address 712 SE Race Track Rd.

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Mike Todd / Nicholas Paul Geisler

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progress Energy

21-4S-17

Property ID Number R08633-002 Estimated Cost of Construction 185,000

Subdivision Name Lot Block Unit Phase

Driving Directions 41/441 south approx 5 miles to L on Race Track Rd. Third Drive on Right

Number of Existing Dwellings on Property 1

Construction of MFH

Total Acreage 5.49 Lot Size

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive) Total Building Height 22'

Actual Distance of Structure from Property Lines - Front 385/323 Side 37 Side 41 Rear 900

SITE PLAN ATTACHED

Number of Stories 1 Heated Floor Area 2112 Total Floor Area ~~2820~~ 3744 Roof Pitch 8/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.) Revised 11-30-07

Left spoke to Steve Williams 386. 867-0419

2/19/08

755-4387

755-1220 fax:

Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Michael T McCoy
Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20___

Personally known_____ **or Produced Identification**_____

SEAL:

State of Florida Notary Signature (For the Contractor)

**** STRUCTURAL FRAMING INPUT DATA ****

*** Roof Structural Data ***

Member number 1
Normal gable type house truss--supported by exterior walls only
Span length out to out of supports = 32 feet
Roof cross slope = 8 /12
Truss spacing = 24 inches
Overhang = 2 feet

Member number 2
Normal gable type house truss--supported by exterior walls only
Span length out to out of supports = 44 feet
Roof cross slope = 8 /12
Truss spacing = 24 inches
Overhang = 2 feet

Member number 3
Normal gable type house truss--supported by exterior walls only
Span length out to out of supports = 24 feet
Roof cross slope = 8 /12
Truss spacing = 24 inches
Overhang = 2 feet

*** Wall Structural Data ***

Spacing of wall studs = 16 inches
Total number of plates = 3
Wall stud number 1 is 9 feet high out to out of plates

COEFFICIENTS AND PRESSURES FOR UPLIFT
Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient
Wind velocity is 110 mph
Mean roof height is 14.73395 feet
Velocity pressure is 24.7 psf
Use factor is 1.0
Roof cross slope is 8 on 12, which equals 33.69007 degrees to horizontal
End zone width is 6.4 feet

	Coefficient	Design Pressure (psf)
End zone		
Windward wall (1E)	-.75	-18.53
Windward roof (2E)	-1.4	-34.59
Leeward roof (3E)	-.8	-19.76
Leeward wall (4E)	-.75	-18.53
Overhang	-1.5	-37.06
Interior zone		
Windward wall (1)	-.7	-17.3
Windward roof (2)	-1	-24.7
Leeward roof (3)	-.65	-16.06
Leeward wall (4)	-.7	-17.3
Overhang	-1.5	-37.06

COEFFICIENTS AND PRESSURES FOR LATERAL FORCES
Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient
Wind velocity is 110 mph
Mean roof height is 14.73395 feet
Velocity pressure is 24.7 psf
Use factor is 1.0
Roof cross slope is 8 on 12, which equals 33.69007 degrees to horizontal
End zone width is 6.4 feet

	Coefficient	Design Pressure (psf)
End zone		
Windward wall (1E)	1	24.7
Windward roof (2E)	.5	12.35
Leeward roof (3E)	-.4	-9.88
Leeward wall (4E)	-.35	-8.65
Overhang	-1.5	-37.06
Interior zone		
Windward wall (1)	.85	20.99
Windward roof (2)	.45	11.11
Leeward roof (3)	-.3	-7.42
Leeward wall (4)	-.25	-6.18
Overhang	-1.5	-37.06

ROOF LOADING--Roof Number 1 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF LOADING--Roof Number 2 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF LOADING--Roof Number 3 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF LOADING--Roof Number 4 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF LOADING--Roof Number 5 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF LOADING--Roof Number 6 (pounds per square foot)

Roof cross slope = 8 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
Rigid isocyanurate insulation 5.5 in. average depth	= .935
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.267708
Total roof unit weight on slope	= 7.062708
Cosine of roof cross slope	= .8320503
Roof unit weight on horizontal	= 8.488318
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 11.78832

Roof Unit Dead Load = 12 psf
Roof dead load supported generally by wall = 200.1131 plf

ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS
All values are in pounds

Roof member number 1 --Span 32 feet, Slope 8 /12, interior zone---- 400
Roof member number 2 --Span 32 feet, Slope 8 /12, end zone--------- 400
Roof member number 3 --Span 44 feet, Slope 8 /12, interior zone---- 541
Roof member number 4 --Span 44 feet, Slope 8 /12, end zone--------- 541
Roof member number 5 --Span 24 feet, Slope 8 /12, interior zone---- 305
Roof member number 6 --Span 24 feet, Slope 8 /12, end zone--------- 305

EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 9 ft. out to out plates

3--2 in. x 4 in. plates	= 2.865625
2 in. x 4 in. studs at 16 in. spacing	= 6.179004
R-13 Insulation	= 2.15625
--------- in. Mineral board siding	= 11.45833
1/2 in. Gypsum board--Total 1 layer---	= 18
Total	= 40.65921

Exterior Wall Unit Dead Load = 41 plf

S U M M A R Y O F H U R R I C A N E A N C H O R A N A L Y S I S

All values of forces are in pounds. Resistances have been increased for wind.
End zone width = 6.4 feet
Code: C = Compliance N = Non-compliance

Southeastern Metal hurricane anchors

Member 1 --Gable roof--Span 32 feet, at 24 inches oc--in interior zone:
Uplift = 966 Dead = 400 Net = 566 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

Member 2 --Gable roof--Span 32 feet, at 24 inches oc--in end zone:
Uplift = 1259 Dead = 400 Net = 859 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

Member 3 --Gable roof--Span 44 feet, at 24 inches oc--in interior zone:
Uplift = 1247 Dead = 541 Net = 706 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

Member 4 --Gable roof--Span 44 feet, at 24 inches oc--in end zone:
Uplift = 1673 Dead = 541 Net = 1132 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

Member 5 --Gable roof--Span 24 feet, at 24 inches oc--in interior zone:
Uplift = 780 Dead = 305 Net = 475 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

Member 6 --Gable roof--Span 24 feet, at 24 inches oc--in end zone:
Uplift = 984 Dead = 305 Net = 679 Model Special, Resistance = 1205 C
Model H16--all nails per mfr.--data supplied by operator, not EDA

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 2.75 inches
Fasteners in panel interior are 8d nails spaced at 6 inches

Total lateral wind force on building = 38658 pounds
Total force transferred through diaphragm to shearwalls = 28475 pounds
Total length of shearwalls = 64 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 62.4 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 444 plf
Allowable diaphragm force per unit length of shearwall = 456 plf

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*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 2.75 inches
Fasteners in panel interior are 8d nails spaced at 6 inches

Total lateral wind force on building = 10774 pounds
Total force transferred through diaphragm to shearwalls = 5387 pounds
Total length of shearwalls = 180 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 11.5 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 29 plf
Allowable diaphragm force per unit length of shearwall = 456 plf

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*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL ****

Interior zone (area Ri)
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 2.75 inches
Fasteners along int. trusses are 8d nails spaced at 6 inches
Total outward wind force on sheathing = 797 pounds
Total withdrawal resistance of 65 nails = 4936 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

Edge strip (area Si) width = 3.2 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 2.75 inches
Fasteners along int. trusses are 8d nails spaced at 6 inches
Total outward wind force on sheathing = 1202 pounds
Total withdrawal resistance of 65 nails = 4936 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

End zone (areas Se and C) width = 6.4 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end truss are 8d nails spaced at 2.75 inches
Fasteners along end wall are 8d nails spaced at 2.75 inches
Fasteners along int. trusses are 8d nails spaced at 6 inches
Total outward wind force on sheathing = 1093 pounds
Total withdrawal resistance of 65 nails = 4936 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 1 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 8.625 feet--located in interior zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.
Moment of inertia = 5.359375 in.^4
Section Modulus = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 367 pounds
Stud moment = 395 ft-lb.

Stresses:
Stud bending vert : Actual = 1550 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 48 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 64 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .704567
Sheathing bending hor: Actual = 177 psi Allowable = 222 psi(adjusted)
Deflections:
Stud : Actual = .4413 in. Allowable = .575 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 2 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 8.625 feet--located in end zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.
Moment of inertia = 5.359375 in.^4
Section Modulus = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 422 pounds
Stud moment = 454 ft-lb.

Stresses:
Stud bending vert : Actual = 1782 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 56 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 64 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .8006333
Sheathing bending hor: Actual = 204 psi Allowable = 222 psi(adjusted)
Deflections:
Stud : Actual = .5074 in. Allowable = .575 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ALLOWABLE STRESS PROPERTIES ****

Base stresses (psi):
Wood:
   Bending   = 875
   Tension   = 425
   Shear   = 70
   Elastic modulus = 1400000

Adjustment factors for wood:
   Duration (Du)   = 1.6
   Wet service (Wt)   = 1
   Temperature (Tm)   = 1
   Stability (St)   = 1
   Size (Sz)   = 1.5
   Volume (Vm)   = 1
   Flat use (Fu)   = 1
   Repetitive (Rp)   = 1.15
   Curvature (Cu)   = 1
   Form (Fm)   = 1
   Shear stress (Sh) = 1

Allowable stresses (psi):
Wood:
   Bending = 2415 (Base x Du x Wt x Tm x St x Sz x Vm x Fu x Rp x Cu x Fm)
   Tension = 1020 (Base x Du x Wt x Tm x Sz)
   Shear   = 112 (Base x Du x Wt x Tm x Sh)
   Elastic modulus = 2240000 (Base x Wt x Tm)

Sheathing:
   Bending   = 222 (Base x 1.33)
   Elastic modulus = 61904.76 (Base)

T R A N S V E R S E D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 5 inches

Total lateral force on building = 38658 pounds
Force applied at top of walls = 28475 pounds
Total dragstrut length = 64 feet
Shear per unit dragstrut length = 444 pounds per linear foot

Actual shear on each nail = 185 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

L O N G I T U D I N A L D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 5 inches

Total lateral force on building = 10774 pounds
Force applied at top of walls = 5387 pounds
Total dragstrut length = 180 feet
Shear per unit dragstrut length = 29 pounds per linear foot

Actual shear on each nail = 12 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

**** T R A N S V E R S E S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 2.75 inch centers
Fasteners: 8d common nails spaced in interior at 6 inch centers

Total lateral force on building = 38658 pounds
Force applied at top of walls = 28475 pounds
Accumulated total shearwall length = 64 feet

Actual unit shear on shearwalls = 444 pounds per linear foot
Allowable unit shear on shearwalls = 468 pounds per linear foot

Shearwall satisfies Code requirements.

**** L O N G I T U D I N A L S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 2.75 inch centers
Fasteners: 8d common nails spaced in interior at 6 inch centers

Total lateral force on building = 10774 pounds
Force applied at top of walls = 5387 pounds
Accumulated total shearwall length = 180 feet

Actual unit shear on shearwalls = 29 pounds per linear foot
Allowable unit shear on shearwalls = 468 pounds per linear foot

Shearwall satisfies Code requirements.

*** ANALYSIS OF OUTWARD FORCES ON WALL SHEATHING ***

Wall number 1 : Total outward wind force on sheathing = 1101 pounds
: Total withdrawal resistance of 146 nails = 8146 pounds

Wall number 2 : Total outward wind force on sheathing = 1266 pounds
: Total withdrawal resistance of 146 nails = 8146 pounds

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 2.75 inch spacing

Total uniform wind uplift in first story at top of wall level = 456 plf
Uniform dead loads per linear foot:
Roof = 200.1131 plf

Total = 200.1131 plf
Total uniform dead load in first story at top of wall level = 200 plf
Net wind uplift in first story at top of wall level = 256 plf

Total uplift force on each nail = 58 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 2.75 inch spacing

Total uniform wind uplift in first story at floor level = 456 plf
Uniform dead loads per linear foot:
Roof = 200.1131 plf
Wall = 40.65921 plf

Total = 240.7723 plf
Total uniform dead load in first story at floor level = 240 plf
Net wind uplift in first story at floor level = 216 plf

Total uplift force on each nail = 49 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF FOUNDATION ANCHORAGE ****

Anchor bolts are 1/2 inch A307, with 2 inch round washer at 48 inch centers.

Total uniform wind uplift on foundation = 456 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 200.1131 plf
Wall = 40.65921 plf

Total = 240.7723 plf
Total uniform dead load times 2/3 = 160 pounds per linear foot
Net uplift force on foundation = 296 pounds per linear foot

Total uplift force on each anchor bolt = 1184 pounds
Safe tension value of each anchor bolt = 1634 pounds (increased by 1/3)
Bolt safe tension value is governed by washer failure

*** Summary of Analysis ***
Foundation anchorage satisfies all Code requirements.

**** ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS ****

Hold-down is 1/2 inch anchor bolt with 3 inch square plate washer, each wall
Normal anchor bolt spacing = 48 inches
Distance from corner to hold-down device = 6.5 inches
Distance from corner to first interior anchor bolt = 48 inches
Net uplift force on foundation = 296 pounds per linear foot
Tributary distance to corner device = 2.270833 feet
Net uplift on corner hold-down device = 672 pounds

Uplift tension due to shearwall action in a transverse shearwall segment:
Distance from corner to hold-down device = 6.5 inches
Distance from corner to first interior anchor bolt = 48 inches
Total shear from shearwall segment = 1334 pounds
Height of wall = 9 feet
Uniform dead load times 2/3 = 27 pounds per linear foot
Shearwall moment at bottom of wall = 12013 foot-pounds
Additional tension at corner device = 5965 pounds
Total uplift tension on corner hold-down devices = 6637 pounds
Allowable tension on corner hold-down devices = 7572 pounds

*** Summary of Analysis ***
Corner hold-down device COMPLIES with Code requirements.

**** ANALYSIS OF FOUNDATION ****

Stemwall is 8 inch concrete masonry, filled with grout, 24 inches high
Footing is 20 inches wide by 10 inches deep
Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 456 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 200.1131 plf
Wall = 40.65921 plf

Total = 240.7723 plf
Total uniform dead load times 2/3 = 160 pounds per linear foot
Net uplift force at top of foundation = 296 pounds per linear foot
Weight of stemwall footing earth x 2/3 = 325 pounds per linear foot
Net uplift at bottom of footing = 0 pounds per linear foot

*** Summary of Analysis ***
Foundation is stable.

**** ANALYSIS OF REINFORCING STEEL ****

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 456 pounds per linear feet
Uniform dead loads in pounds per linear foot:
Roof = 200.1131 plf
Wall = 40.65921 plf

Total = 240.7723 plf
Total uniform dead load times 2/3 = 160 pounds per linear foot
Net uplift force on foundation = 296 pounds per linear foot
Weight of concrete block stemwall x 2/3 = 122 pounds per linear foot
Net uplift at top of footing = 174 pounds per linear foot

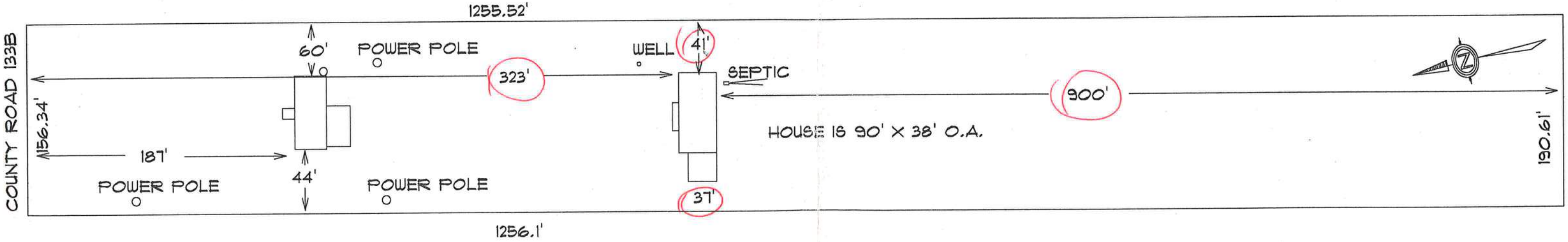
Total uplift force on each re-bar = 1044 pounds
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)

*** Summary of Analysis ***
Reinforcing steel satisfies all Code requirements.

**** SUMMARY OF REINFORCING DATA ****

Foundation wall data:
Wall is composed of 8 inch concrete masonry, fully grouted.
Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers
Minimum required lap splice for Number 5 bar is 25 inches.
Minimum required clearance for Number 5 bar is 1.5 inches.
Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing.

Footing data:
Footing is continuous, 20 inches wide by 10 inches deep.
Footing concrete is 2500 psi
Footing reinforcing is Grade 40 steel, 2--#( ) longitudinal.
Minimum required splice length = 25 inches
Reinforcing steel shall have cover as follows:
    Top-------6 inches
    Sides-----3 inches
    Bottom----3 inches


1255.52'
COUNTY ROAD 133B
156.34'
60'
POWER POLE
323'
WELL
41'
SEPTIC
900'
N
187'
44'
POWER POLE
POWER POLE
37'
HOUSE IS 90' X 38' O.A.
190.61'
1256.1'

THIS INSTRUMENT PREPARED BY:
Karen Brown
Tri-County Title Services of Lake City, Inc.
229 North Hernando Street
Lake City, Florida 32055

OR 679 PG 164

OFFICIAL RECORDS

RECORD AND RETURN TO:
Tri-County Title Services of Lake City, Inc.
229 North Hernando Street
Lake City, Florida 32055

RE PARCEL ID #: **R08633-002**
BUYER'S TIN: **263-61-3227**

94-16608

FOR RECORDER

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL
1994 DEC 28 PM 4:06
RECORDED AND VERIFIED
P. DeWITT CASON
CLERK OF COURTS
COLUMBIA COUNTY, FLORIDA
BY Marl Dixon D.C.

WARRANTY DEED

THIS WARRANTY DEED made this **28th day of December, 1994** by **Sandra J. Sieg, a single person,** hereinafter called Grantor, and whose address is **Rt. 6, Box 392-C, Lake City, FL 32025** to **Michael Torey McCray,** hereinafter called Grantee and whose address is **511 S. Marion Street, Lake City, Florida 32055.**

(Wherever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations.)

WITNESSETH:

THAT the Grantor, for and in consideration of the sum of Ten and NO/100 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee, all that certain land situate, lying and being in **Columbia** County, **Florida,** viz:

That part of the W 1/2 of the E 1/2 of the SE 1/4, Section 21, Township 4 South, Range 17 East, Columbia County, Florida described as follows: Commence at the Southeast corner of Section 21, Township 4 South, Range 17 East and run thence S 88 deg. 29'22" W along the South line of said Section 21, 991.13 feet, thence N 1 deg. 25'15" W, 1337.47 feet to the POINT OF BEGINNING, thence continue N 1 deg. 25'15" W, 1255.52 feet to the South right-of-way line of County Road No. C-133-B, thence S 88 deg. 21'40" W along said South right-of-way line, 156.34 feet, thence S 0 deg. 08'34" W, 1256.11 feet, thence N 88 deg. 21'40" E, 190.61 feet to the POINT OF BEGINNING. Columbia County, Florida.

DOCUMENTARY STAMP $ 210.00
INTANGIBLE TAX $ 0
P. DeWITT CASON, CLERK OF COURTS, COLUMBIA COUNTY
BY Marl Dixon D.C.

SUBJECT TO taxes accruing subsequent to December 31, **1994.**

SUBJECT TO covenants, restrictions and easements of record, if any; however, this reference thereto shall not operate to reimpose same.

TOGETHER with all the tenements, hereditaments and appurtenances thereunto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

AND the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances.

IN WITNESS WHEREOF, the said Grantor has signed and sealed these presents the day and year first above written.

BK 0799 PG 1645

OFFICIAL RECORDS

Signed, sealed and delivered in our presence:

Witness Signature

DAN GHERNA
Witness Printed Signature

Witness Signature

KAREN BROWN
Witness Printed Signature

Sandra J. Sieg

STATE OF **FLORIDA**
COUNTY OF **COLUMBIA**

The foregoing instrument was acknowledged before me this **28th day of December, 1994** by **Sandra J. Sieg, a single person** who produced DRIVERS LICENSE as identification and who did not take an oath.

Notary Public, State and County Aforesaid

Notary Signature

Notary Printed Signature

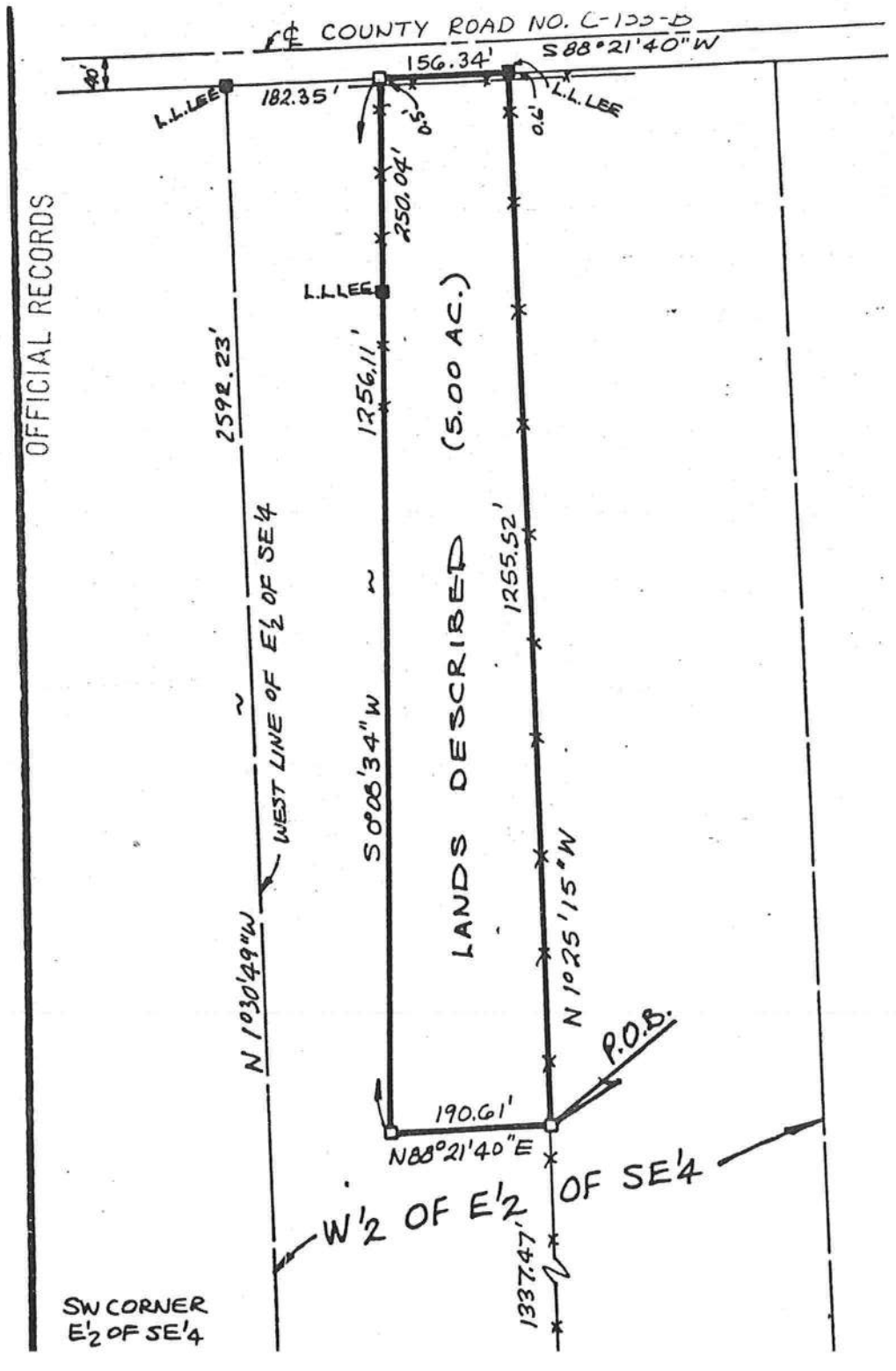
KAREN BROWN
MY COMMISSION # CC343577 EXPIRES
March 30, 1995
BONDED THRU TROY FAIN INSURANCE, INC.

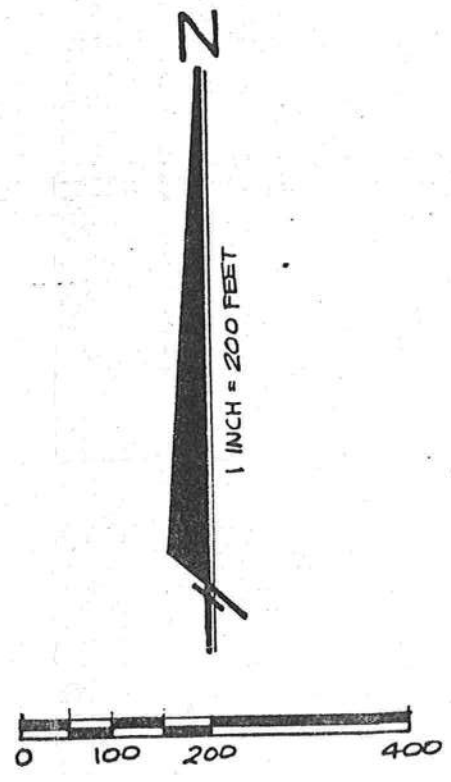
(Title or Rank)

(Serial No., if any)

BK 0799 PG 1646

OFFICIAL RECORDS





THIS ORIGINAL IS OF POOR LEGIBILITY

LEGEND

- ■ 4"x4" CONCRETE MONUMENT FOUND AS NOTED
- □ 4"x4" CONCRETE MONUMENT SET (PLS 4303)
- —x— FIELD FENCE

NOTES

1. BEARINGS PROJECTED FROM CENTERLINE OF COUNTY ROAD NO. C-133-B AS PER PLAT BY L.L.LEE DATED 10-5-82.

CERTIFIED TO:

S. MARLENE AND MICHAEL TOREY McCRAY
TRI-COUNTY TITLE SERVICES OF LAKE CITY, INC.
T.A. TITLE INSURANCE CO.

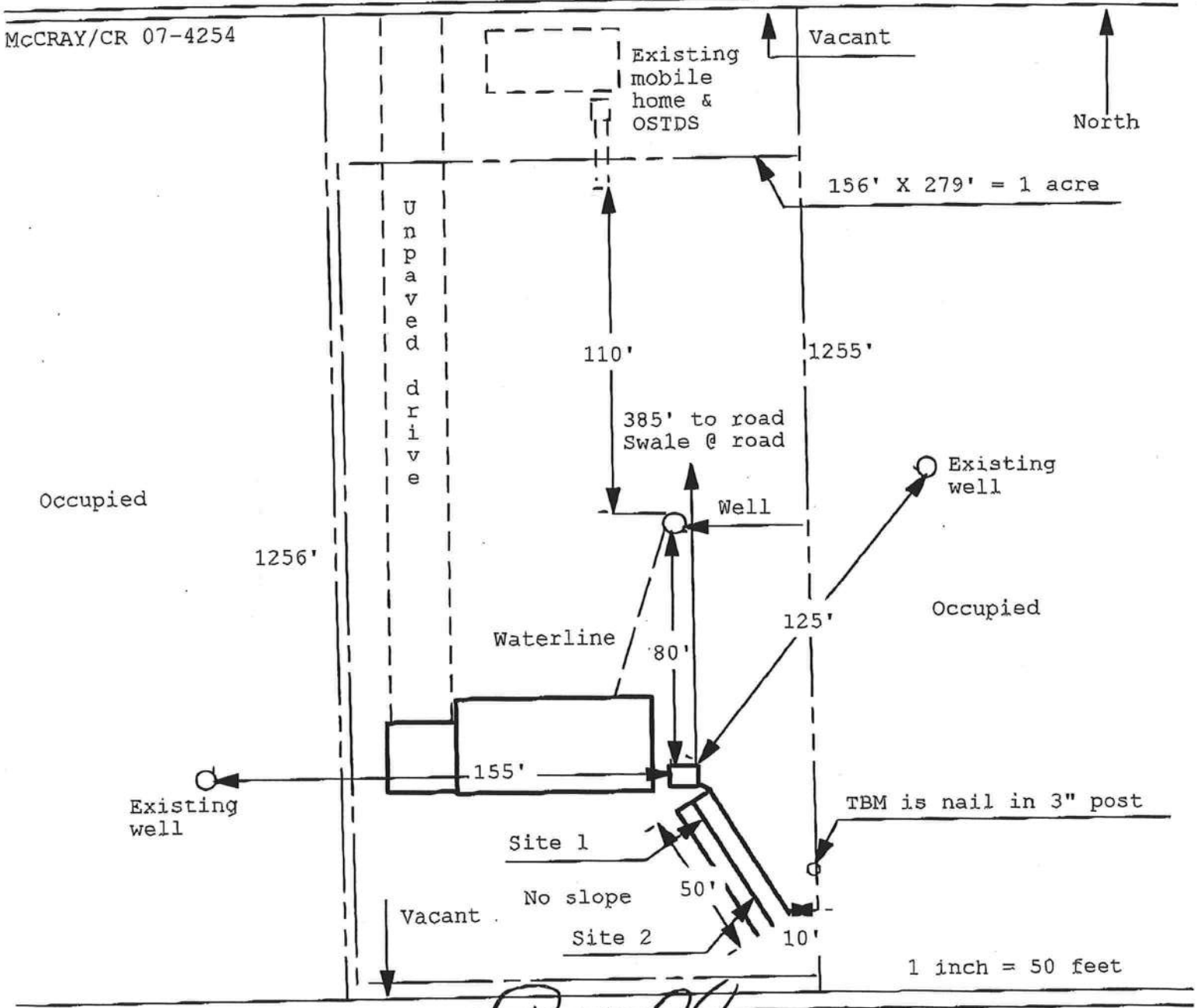
SE CORNER
SEC. 21, T4-S, R17-E

THIS IS NOT A SURVEY. ATTACHED HERETO TO COMPLY WITH AMENDMENT TO F.S. 28.222

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 08-0114-N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd Date 1/28/08

Plan Approved ✓ Not Approved ____ Date 1/28/08

By Mary Columbia CPHU

Notes: ____

Columbia County Property Appraiser

DB Last Updated: 1/15/2008

2008 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

Parcel: 21-4S-17-08633-002 HX

Search Result: 1 of 1

Owner & Property Info

Owner's Name	MCCRAY MICHAEL TOREY		
Site Address			
Mailing Address	C/O 365 S MARION AVE STE 102 LAKE CITY, FL 32025		
Use Desc. (code)	MOBILE HOM (000200)		
Neighborhood	21417.00	**Tax District**	2
UD Codes	MKTA02	**Market Area**	02
Total Land Area	5.000 ACRES		
Description	COMM SE COR OF SEC, RUN W 991.13 FT, N 1337.47 FT FOR POB, CONT N 1255.52 FT TO S R/W C-133-B, RUN W ALONG R/W 156.34 FT, S 1256.11 FT, E 190.61 FT TO POB. ORB 799-1644		

GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (2)	$54,619.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$18,185.00
XFOB Value	cnt: (1)	$400.00
Total Appraised Value		$73,204.00

Just Value		$73,204.00
Class Value		$0.00
Assessed Value		$42,427.00
Exempt Value	(code: HX)	$25,000.00
Total Taxable Value		$17,427.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
12/28/1994	799/1644	WD	I	Q		$30,000.00
6/1/1993	775/2272	WD	V	U	03	$53,600.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1985	WD or PLY (08)	1440	2216	$18,185.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0294	SHED WOOD/	1993	$400.00	1.000	10 x 16 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000200	MBL HM (MKT)	5.000 AC	1.00/1.00/1.00/1.00	$10,523.83	$52,619.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	$2,000.00	$2,000.00

FOR RECORDER

THIS INSTRUMENT PREPARED BY
Karen Brown
1st County Title Services of Lake City, Inc
229 North Hernando Street
Lake City, Florida 32055

RECORD AND RETURN TO
1st County Title Services of Lake City, Inc
229 North Hernando Street
Lake City, Florida 32055

94 1660F

WARRANTY DEED

THIS WARRANTY DEED made this **28th day of December, 1994** by **Sandra J. Sieg, a single person**, hereinafter called Grantor, and whose address is **Rt. 6, Box 392-C, Lake City, FL 32025** to **Michael Torey McElroy**, hereinafter called Grantee, and whose address is **511 S. Marion Street, Lake City, Florida 32055**.

Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations.

WITNESSETH:

THAT the Grantor, for and in consideration of the sum of Ten and NO/100 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee, all that certain land situate, lying and being in **Columbia** County, **Florida**, viz:

That part of the W 1/2 of the E 1/2 of the SE 1/4, Section 21, Township 4 South, Range 17 East, Columbia County, Florida described as follows: Commence at the Southeast corner of Section 21, Township 4 South, Range 17 East and run thence S 88 deg. 29'22" W along the South line of said Section 21, 991.13 feet, thence N 1 deg. 25'15" W, 1307.47 feet to the POINT OF BEGINNING, thence continue N 1 deg. 25'15" W, 1255.52 feet to the South right-of-way line of County Road No. C-133-B, thence S 88 deg. 21'40" W along said South right-of-way line, 156.34 feet, thence S 0 deg. 08'34" W, 1256.11 feet, thence S 88 deg. 21'40" E, 190.61 feet to the POINT OF BEGINNING. Columbia County, Florida.

21600

SUBJECT TO taxes accruing subsequent to December 31, 1994

SUBJECT TO covenants, restrictions and easements of record, if any, however, this reference thereto shall not reimpose same.

TOGETHER with all the tenements, hereditaments and appurtenances thereunto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same in fee simple forever.

AND the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances.

IN WITNESS WHEREOF, the said Grantor has signed and sealed these presents the day and year first above written.

Signed, Sealed and delivered in our presence:

Witness Signature

Witness Printed Signature

Witness Signature

KAREN BROWN

Witness Printed Signature

Sandra J. Sieg

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 28th day of December, 19__ by Sandra J. Sieg, a single person who produced _____ as identification and who did not take an oath.

Notary Public, State and County Aforesaid

Notary Signature

(Title or Rank)

Notary Printed Signature KAREN BROWN NOTARY PUBLIC STATE OF FLORIDA

(Serial No., if any)

LEGEND

- ■ 5/8" x 4" CONCRETE MONUMENT FOUND AS NOTED
- ○ 4" x 4" CONCRETE MONUMENT SET (TAS 4808)
- —+— FIELD FENCE

NOTES

1. BEARING'S PROJECTED FROM CENTERLINE OF COUNTY ROAD, NO CURVE B AS PER PLAT BY L L LEE DATED NOV. 82

CERTIFIED TO:

S MARLEAS AND MICHAEL TOBEY MACRAY

TRI-COUNTY TITLE SERVICES OF LAKE CITY, INC.

T.A TITLE INSURANCE CO.

THIS ORIGINAL IS OF POOR LEGIBILITY

COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave.. Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

(✓) Single Family Dwelling ( ) Two-Family Residence ( ) Farm Outbuilding
( ) Other_____ ( ) Addition, Alteration, Modification or other Improvement

I Michael McCray , have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Michael T. McCray
Owner Builder Signature Date

FLORIDA NOTARY
The above signer is personally known to me or produced identification personally known
Notary Signature Teresa Pierce Date 1-23-07

NOTARY PUBLIC STATE OF FLORIDA
TERESA N. PIERCE
MY COMMISSION # DD474390
EXPIRES: Sept. 21, 2009
(407) 398-0153 Florida Notary Service.com

FOR BUILDING DEPARTMENT USE ONLY
I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date_____ Building Official/Representative_____

wrightsoft

Project Summary
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

Project Information

For: Mike McCray
Lake City, Fl

Notes:

Design Information

Weather: Jacksonville, Cecil Field NAS, FL, US

Winter Design Conditions

Outside db	34	°F
Inside db	70	°F
Design TD	36	°F

Summer Design Conditions

Outside db	95	°F
Inside db	75	°F
Design TD	20	°F
Daily range	M	
Relative humidity	50	%
Moisture difference	40	gr/lb

Heating Summary

Structure	27581	Btuh
Ducts	11394	Btuh
Central vent (60 cfm)	2360	Btuh
Humidification	0	Btuh
Piping	0	Btuh
Equipment load	41335	Btuh

Sensible Cooling Equipment Load Sizing

Structure	24560	Btuh
Ducts	14979	Btuh
Central vent (60 cfm)	1311	Btuh
Blower	0	Btuh
Use manufacturer's data	n	
Rate/swing multiplier	1.00	
Equipment sensible load	40850	Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	2149	2149
Volume (ft³)	18878	18878
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	101	50

Latent Cooling Equipment Load Sizing

Structure	1976	Btuh
Ducts	2745	Btuh
Central vent (60 cfm)	1634	Btuh
Equipment latent load	6355	Btuh
Equipment total load	47205	Btuh
Req. total capacity at 0.70 SHR	4.9	ton

Heating Equipment Summary

Make	
Trade	
Model	

Efficiency	100	EFF
Heating input	0	Btuh
Heating output	41335	Btuh
Temperature rise	20	°F
Actual air flow	1883	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.00	in H2O
Space thermostat		

Cooling Equipment Summary

Make	Trane
Trade	XL19i
Cond	2TTZ9060B1
Coil	2TXCD061BC3+*DD120R9V5

Efficiency	16	SEER
Sensible cooling	39550	Btuh
Latent cooling	16950	Btuh
Total cooling	56500	Btuh
Actual air flow	1883	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.00	in H2O
Load sensible heat ratio	0.87	

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

wrightsoft

Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

					Entire House 237.0 ft				bed2 30.0 ft			
1	Room name											
2	Exposed wall				8.8 ft	d			9.0 ft	heat/cool		
3	Ceiling height								15.0 x 15.0 ft			
4	Room dimensions											
5	Room area				2149.0 ft²				225.0 ft²			

	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12E-3bw	0.059	n	2.12	0.92	279	258	548	237	135	135	287	124
.	└D	11D0	0.390	n	14.04	12.28	21	21	295	258	0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	638	548	1164	504	0	0	0	0
.	├G	1A-c1ob	1.080	e	38.88	93.55	69	0	2683	6455	0	0	0	0
11	└D	11D0	0.390	e	14.04	12.28	21	21	295	258	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	493	460	977	423	0	0	0	0
	└G	1A-c1ob	1.080	s	38.88	42.72	33	0	1283	1410	0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	646	550	1168	506	135	105	223	97
	├G	1A-c1ob	1.080	w	38.88	93.55	54	0	2100	5052	30	0	1166	2806
	└D	11D0	0.390	w	14.04	12.28	42	42	590	516	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	2149	2149	1702	3546	225	225	178	371
	F	22C-5tph	1.266	-	45.58	0.00	2149	237	10802	0	225	30	1367	0

6	c) AED excursion						0					768
	Envelope loss/gain					23605	19166			3222		4167
12	a) Infiltration					3975	1104			522		145
	b) Room ventilation					0	0			0		0
13	Internal gains:	Occupants @	230	3			690	1				230
		Appliances @	1200	3			3600	0				0
	Subtotal (lines 6 to 13)					27581	24560			3744		4542
	Less external load					0	0			0		0
	Less transfer					0	0			0		0
	Redistribution					0	0			0		0
14	Subtotal					27581	24560			3744		4542
15	Duct loads			41%	61%	11394	14979	41%	61%	1547		2770
	Total room load					38974	39539			5290		7311
	Air required (cfm)					1883	1883			256		348

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AED Assessment
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

Project Information

For: Mike McCray
Lake City, Fl

Design Conditions

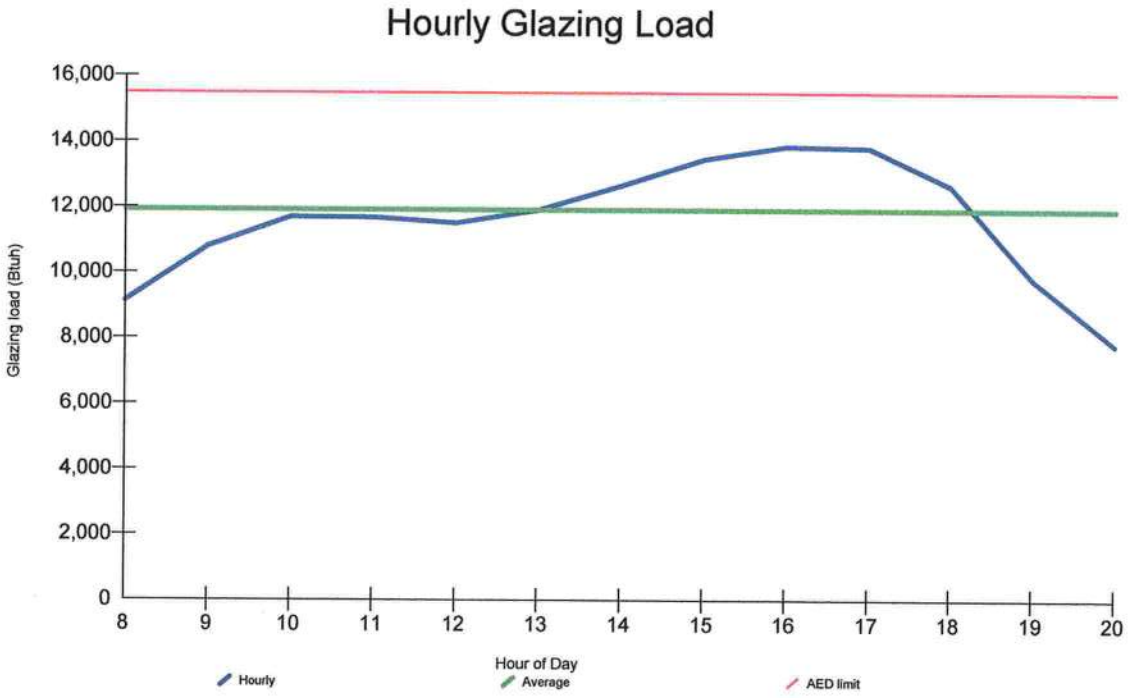
Location:
Jacksonville, Cecil Field NAS, FL, US
Elevation: 82 ft
Latitude: 30 °N

Outdoor:	Heating	Cooling
Dry bulb (°F)	34	95
Daily range (°F)	-	20 (M)
Wet bulb (°F)	-	76
Wind speed (mph)	15.0	7.5

Indoor:	Heating	Cooling
Indoor temperature (°F)	70	75
Design TD (°F)	36	20
Relative humidity (%)	30	50
Moisture difference (gr/lb)	9.7	40.3

Infiltration:

Test for Adequate Exposure Diversity



Maximum hourly glazing load exceeds average by 16.4%.

House has adequate exposure diversity (AED), based on AED limit of 30%.

AED excursion: 0 Btuh

wrightsoft

Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

					Master Bath 26.0 ft 8.0 ft heat/cool 11.0 x 15.0 ft 165.0 ft²				Room7 25.0 ft 8.0 ft heat/cool 11.0 x 14.0 ft 154.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
1	Room name													
2	Exposed wall													
3	Ceiling height													
4	Room dimensions													
5	Room area													
6	W	12E-3bw	0.059	n	2.12	0.92	0	0	0	0	0	0	0	0
.	└D	11D0	0.390	n	14.04	12.28	0	0	0	0	0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0	88	58	123	53
.	├G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0	30	0	1166	2806
11	└D	11D0	0.390	e	14.04	12.28	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	120	117	249	108	112	82	174	75
	└G	1A-c1ob	1.080	s	38.88	42.72	3	0	117	128	30	0	1166	1282
	W	12E-3bw	0.059	w	2.12	0.92	88	72	153	66	0	0	0	0
	├G	1A-c1ob	1.080	w	38.88	93.55	16	0	622	1497	0	0	0	0
	└D	11D0	0.390	w	14.04	12.28	0	0	0	0	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	165	165	131	272	154	154	122	254
	F	22C-5tph	1.266	-	45.58	0.00	165	26	1185	0	154	25	1139	0
6	c) AED excursion									355				-435
	Envelope loss/gain								2456	2426			3892	4036
12	a) Infiltration								402	112			387	107
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances @ 1200					0			0	0			0
	Subtotal (lines 6 to 13)								2858	2538			4278	4144
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								2858	2538			4278	4144
15	Duct loads						41%	61%	1181	1548	41%	61%	1767	2527
	Total room load								4039	4086			6046	6671
	Air required (cfm)								195	195			292	318

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wrightsoft

Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

1	Room name						Bath					Great Room			
2	Exposed wall						8.0 ft					122.0 ft			
3	Ceiling height						9.0 ft				heat/cool	9.0 ft			heat/cool
4	Room dimensions						8.0 x 15.0 ft					1.0 x 1341.0 ft			
5	Room area						120.0 ft²					1341.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)			Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool		Gross	N/P/S	Heat	Cool
6	W	12E-3bw	0.059	n	2.12	0.92	0	0	0	0		0	0	0	0
.	└D	11D0	0.390	n	14.04	12.28	0	0	0	0		0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0		486	435	924	400
.	├G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0		30	0	1166	2806
11	└D	11D0	0.390	e	14.04	12.28	0	0	0	0		21	21	295	258
	W	12E-3bw	0.059	s	2.12	0.92	0	0	0	0		261	261	554	240
	└G	1A-c1ob	1.080	s	38.88	42.72	0	0	0	0		0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	72	64	136	59		351	309	656	284
	├G	1A-c1ob	1.080	w	38.88	93.55	8	0	311	748		0	0	0	0
	└D	11D0	0.390	w	14.04	12.28	0	0	0	0		42	42	590	516
	C	16A-44ad	0.022	-	0.79	1.65	120	120	95	198		1341	1341	1062	2213
	F	22C-5tph	1.266	-	45.58	0.00	120	8	365	0		1341	122	5560	0
6	c) AED excursion									201					-815
	Envelope loss/gain								907	1206				10808	5903
12	a) Infiltration								139	39				2123	590
	b) Room ventilation								0	0				0	0
13	Internal gains:	Occupants @ 230					0			0		2			460
		Appliances @ 1200					0			0		3			3600
	Subtotal (lines 6 to 13)								1046	1245				12931	10553
	Less external load								0	0				0	0
	Less transfer								0	0				0	0
	Redistribution								0	0				0	0
14	Subtotal								1046	1245				12931	10553
15	Duct loads						41%	61%	432	759		41%	61%	5342	6436
	Total room load								1478	2005				18273	16989
	Air required (cfm)								71	95				883	809

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wrightsoft

Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

							Room2 4.0 ft 8.0 ft heat/cool 8.0 x 4.0 ft 32.0 ft²				Laundry 22.0 ft 8.0 ft heat/cool 8.0 x 14.0 ft 112.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12E-3bw	0.059	n	2.12	0.92	32	32	68	29	112	91	193	84
.	└D	11D0	0.390	n	14.04	12.28	0	0	0	0	21	21	295	258
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0	64	55	117	51
.	├G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0	9	0	350	842
11	└D	11D0	0.390	e	14.04	12.28	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	0	0	0	0	0	0	0	0
	└G	1A-c1ob	1.080	s	38.88	42.72	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	0	0	0	0	0	0	0	0
	├G	1A-c1ob	1.080	w	38.88	93.55	0	0	0	0	0	0	0	0
	└D	11D0	0.390	w	14.04	12.28	0	0	0	0	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	32	32	25	53	112	112	89	185
	F	22C-5tph	1.266	-	45.58	0.00	32	4	182	0	112	22	1003	0
6	c) AED excursion									-9				-64
	Envelope loss/gain								276	73			2046	1355
12	a) Infiltration								62	17			340	95
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances @ 1200					0			0	0			0
	Subtotal (lines 6 to 13)								337	90			2387	1449
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								337	90			2387	1449
15	Duct loads						41%	61%	139	55	41%	61%	986	884
	Total room load								477	145			3372	2333
	Air required (cfm)								23	7			163	111

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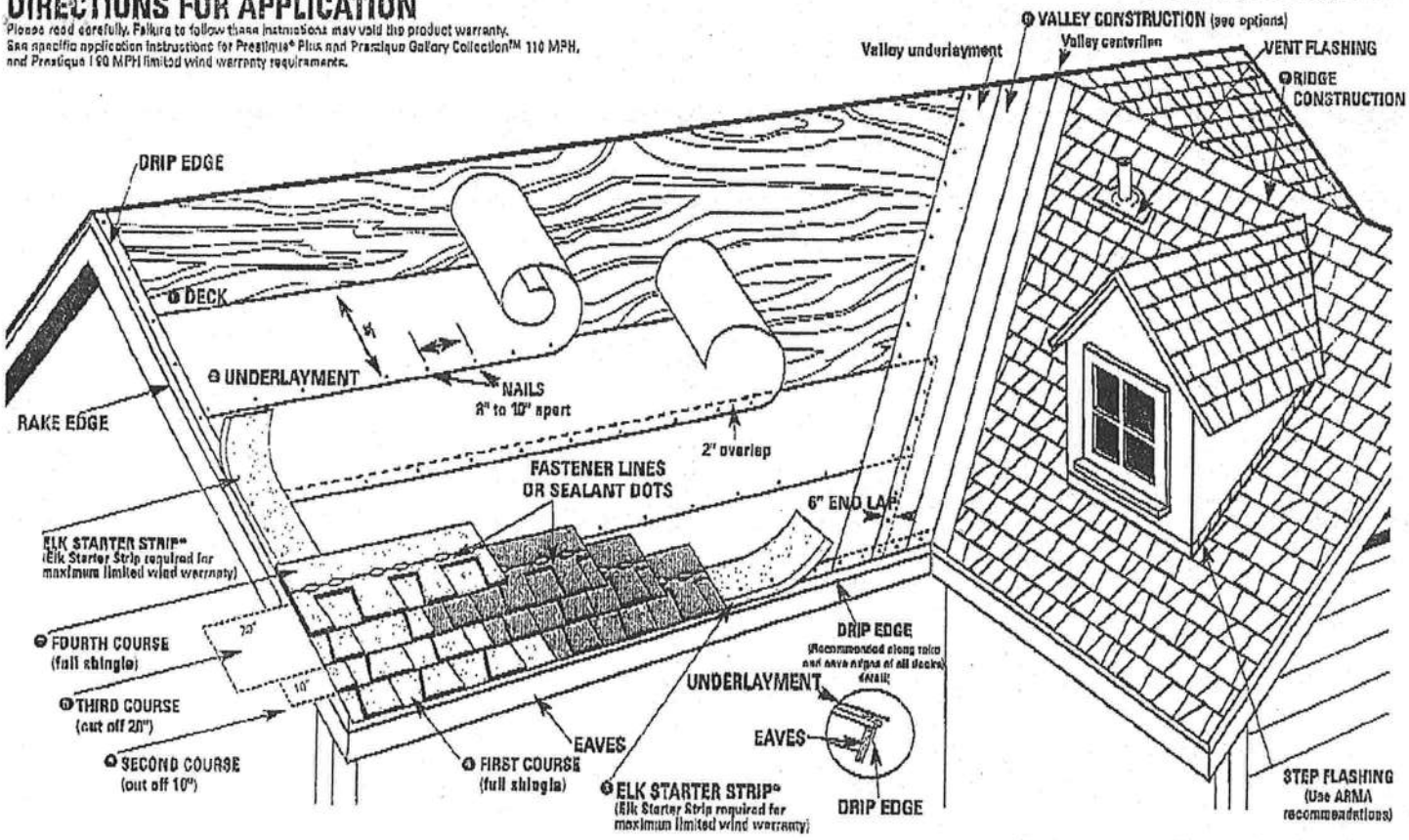
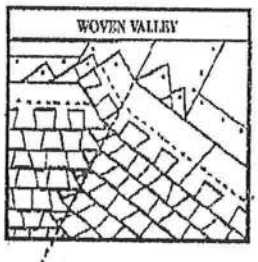
TUSCALOOSA, AL

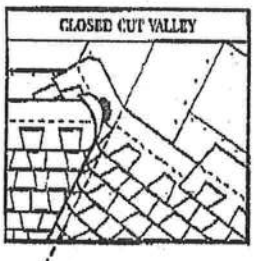
DIRECTIONS FOR APPLICATION

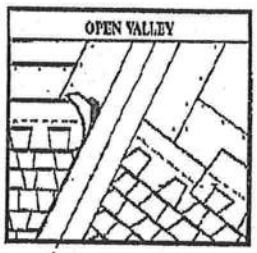
Please read carefully. Failure to follow these instructions may void the product warranty.
See specific application instructions for Prestique® Plus and Prestique Gallery Collection™ 110 MPH, and Prestique 130 MPH limited wind warranty requirements.



VALLEY CONSTRUCTION OPTION (California Open and California Closed are also acceptable) NOTE: For complete ARMA valley installation details, see ARMA Residential Asphalt Roofing Manual.







DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and

❸ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof.

❹ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying 18" metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❺ RIDGE CONSTRUCTION

For ridge construction use Class "A" Seal-A-Ridge® with formula *FLX*™ (See ridge package for installation instructions.)

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Always nail or staple through the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots.

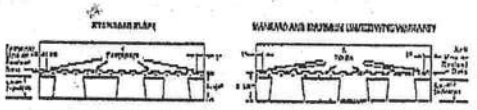
NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less.

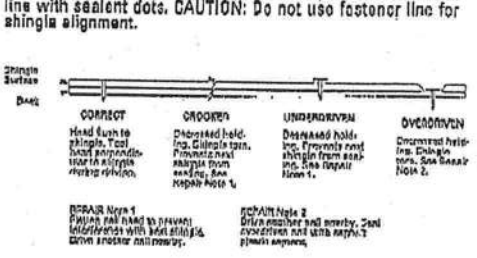
MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For



HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry

ARTER SHINGLE COURSE

AN ELK STARTER STRIP OR A STRIP SHINGLE INVERTED ITH THE HEADLAP APPLIED AT THE EAVE EDGE. With at least 4" trimmed from the end of the first shingle, start at the rake edge overhanging the eave 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❶ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

❺ SECOND COURSE

Start at the rake with the shingle having 10" trimmed off and continue across roof with full shingles.

❻ THIRD COURSE

Start at the rake with the shingle having 20" trimmed off and continue across roof with full shingles.

the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 80 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK

www.elkcorp.com

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8500
TYPE: PVC Single Hung Window

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.architest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48976.02
Test Date:	02/25/04
Report Date:	02/26/04
Expiration Date:	02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.architest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1)		
	@ 1.57 psf (25 mph)	0.21 cfm/ft^2	0.30 cfm/ft^2

Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
Optional Performance:			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

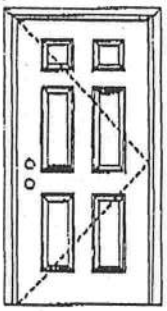
DAJ/jb
02-48976.02

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website (www.ultdemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

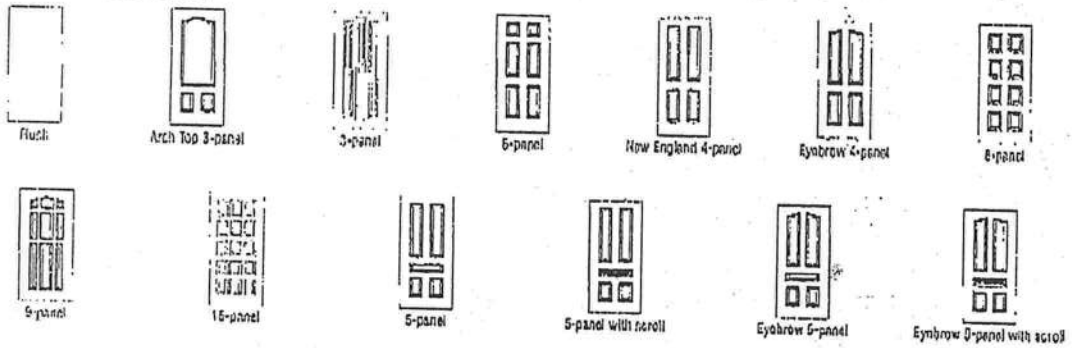
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, pictures and product details subject to change without notice.

PREMDOOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Bathy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-C01 provides additional information - available from the ITS/WH website (www.intertek.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002

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PREMDOR Millennium Premium Quality Doors

Exclusively from Masonite

Masonite International Corporation


AMTROL INC.
WEL-FLO®
Pre-pressurized
Water System Tanks
• Proven Diaphragm Design
• Tough Gloss Finish
• Sizes from 14 to 119 Gallons
• Outstanding Value
Wel-Flo
Wel-Flo
Wel-Flo
Wel-Flo

1. Air Valve. Allows air to be put into tank. Must be at or 2 psi below cut-in pressure with tank empty.
2. Pressure switch. Sets cut-in and cut-off pressure for pump.
3. Pressure gauge. Shows actual pressure in tank
4. Hose bibb. May be used to drain tank or for watering purposes
5. Pressure release valve. Safety device to prevent explosion of tank.

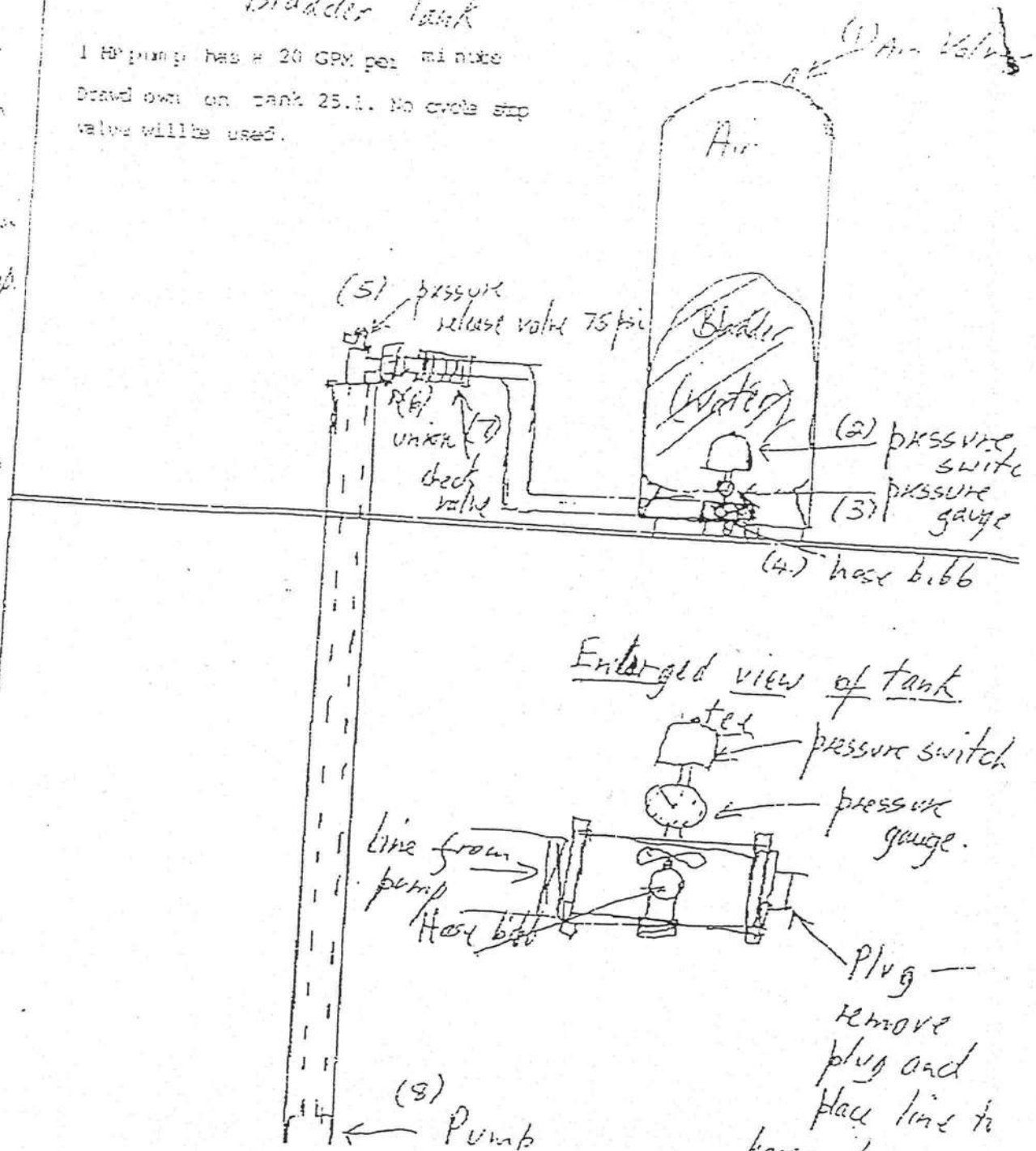
Union - Used to separate tank from well

Check valve. Prevents water from running back down well.

Pump - Pushes water up from well into tank.

Bladder Tank

1 HP pump has a 20 GPM per minute Drawdown on tank 25.1. No cycle stop valve will be used.



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Project Summary
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

Project Information

For: Mike McCray
Lake City, Fl

Notes:

Design Information

Weather: Jacksonville, Cecil Field NAS, FL, US

Winter Design Conditions

Outside db	34 °F
Inside db	70 °F
Design TD	36 °F

Summer Design Conditions

Outside db	95 °F
Inside db	75 °F
Design TD	20 °F
Daily range	M
Relative humidity	50 %
Moisture difference	40 gr/lb

Heating Summary

Structure	27581 Btuh
Ducts	11394 Btuh
Central vent (60 cfm)	2360 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	41335 Btuh

Sensible Cooling Equipment Load Sizing

Structure	24560 Btuh
Ducts	14979 Btuh
Central vent (60 cfm)	1311 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	1.00
Equipment sensible load	40850 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	2149	2149
Volume (ft³)	18878	18878
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	101	50

Latent Cooling Equipment Load Sizing

Structure	1976 Btuh
Ducts	2745 Btuh
Central vent (60 cfm)	1634 Btuh
Equipment latent load	6355 Btuh
Equipment total load	47205 Btuh
Req. total capacity at 0.70 SHR	4.9 ton

Heating Equipment Summary

Make	
Trade	
Model	
Efficiency	100 EFF
Heating input	0 Btuh
Heating output	41335 Btuh
Temperature rise	20 °F
Actual air flow	1883 cfm
Air flow factor	0.048 cfm/Btuh
Static pressure	0.00 in H2O
Space thermostat	

Cooling Equipment Summary

Make	Trane
Trade	XL19i
Cond	2TTZ9060B1
Coil	2TXCD061BC3+*DD120R9V5
Efficiency	16 SEER
Sensible cooling	39550 Btuh
Latent cooling	16950 Btuh
Total cooling	56500 Btuh
Actual air flow	1883 cfm
Air flow factor	0.048 cfm/Btuh
Static pressure	0.00 in H2O
Load sensible heat ratio	0.87

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Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

							Entire House 237.0 ft 8.8 ft d 2149.0 ft²				bed2 30.0 ft 9.0 ft heat/cool 15.0 x 15.0 ft 225.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
1	Room name													
2	Exposed wall													
3	Ceiling height													
4	Room dimensions													
5	Room area													
6	W	12E-3bw	0.059	n	2.12	0.92	279	258	548	237	135	135	287	124
.	D	11D0	0.390	n	14.04	12.28	21	21	295	258	0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	638	548	1164	504	0	0	0	0
.	G	1A-c1ob	1.080	e	38.88	93.55	69	0	2683	6455	0	0	0	0
11	D	11D0	0.390	e	14.04	12.28	21	21	295	258	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	493	460	977	423	0	0	0	0
	G	1A-c1ob	1.080	s	38.88	42.72	33	0	1283	1410	0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	646	550	1168	506	135	105	223	97
	G	1A-c1ob	1.080	w	38.88	93.55	54	0	2100	5052	30	0	1166	2806
	D	11D0	0.390	w	14.04	12.28	42	42	590	516	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	2149	2149	1702	3546	225	225	178	371
	F	22C-5tph	1.266	-	45.58	0.00	2149	237	10802	0	225	30	1367	0
6	c) AED excursion									0				768
	Envelope loss/gain								23605	19166			3222	4167
12	a) Infiltration								3975	1104			522	145
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					3			690	1			230
		Appliances @ 1200					3			3600	0			0
	Subtotal (lines 6 to 13)								27581	24560			3744	4542
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								27581	24560			3744	4542
15	Duct loads						41%	61%	11394	14979	41%	61%	1547	2770
	Total room load								38974	39539			5290	7311
	Air required (cfm)								1883	1883			256	348

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AED Assessment
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

Project Information

For: Mike McCray
Lake City, Fl

Design Conditions

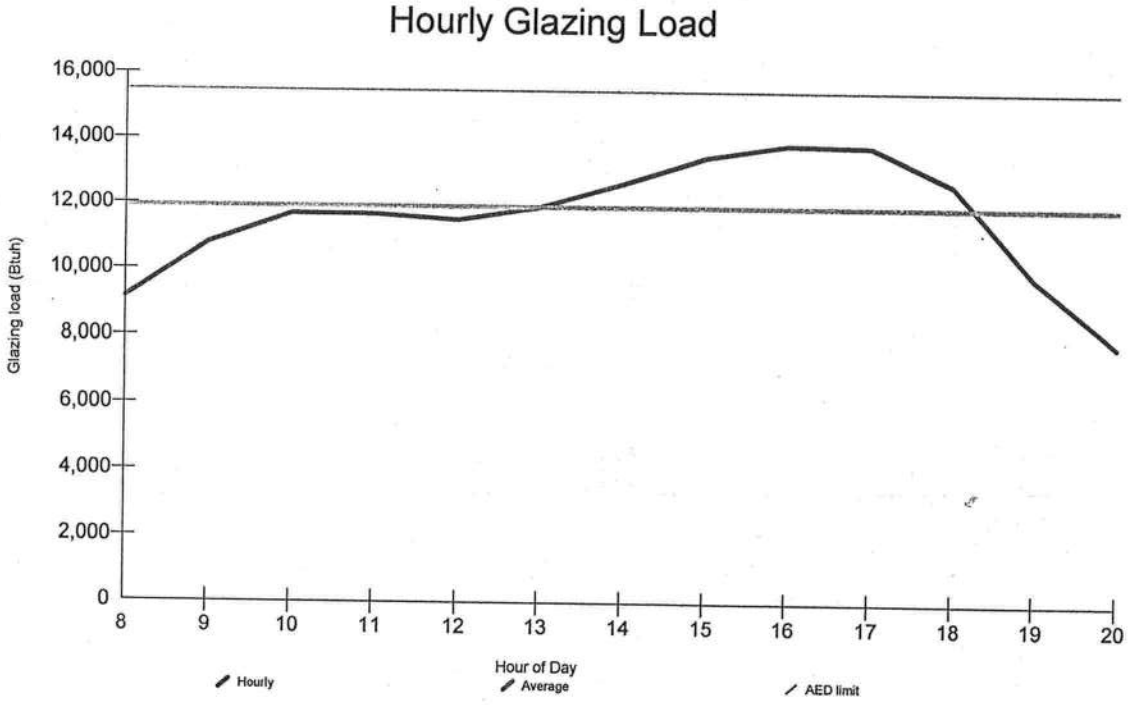
Location:
Jacksonville, Cecil Field NAS, FL, US
Elevation: 82 ft
Latitude: 30°N

Outdoor:	Heating	Cooling
Dry bulb (°F)	34	95
Daily range (°F)	-	20 (M)
Wet bulb (°F)	-	76
Wind speed (mph)	15.0	7.5

Indoor:	Heating	Cooling
Indoor temperature (°F)	70	75
Design TD (°F)	36	20
Relative humidity (%)	30	50
Moisture difference (gr/lb)	9.7	40.3

Infiltration:

Test for Adequate Exposure Diversity



Maximum hourly glazing load exceeds average by 16.4%.

House has adequate exposure diversity (AED), based on AED limit of 30%.

AED excursion: 0 Btuh

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Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

							Master Bath 26.0 ft 8.0 ft heat/cool 11.0 x 15.0 ft 165.0 ft²				Room7 25.0 ft 8.0 ft heat/cool 11.0 x 14.0 ft 154.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
1	Room name													
2	Exposed wall													
3	Ceiling height													
4	Room dimensions													
5	Room area													
6	W	12E-3bw	0.059	n	2.12	0.92	0	0	0	0	0	0	0	0
.	D	11D0	0.390	n	14.04	12.28	0	0	0	0	0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0	88	58	123	53
.	G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0	30	0	1166	2806
11	D	11D0	0.390	e	14.04	12.28	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	120	117	249	108	112	82	174	75
	G	1A-c1ob	1.080	s	38.88	42.72	3	0	117	128	30	0	1166	1282
	W	12E-3bw	0.059	w	2.12	0.92	88	72	153	66	0	0	0	0
	G	1A-c1ob	1.080	w	38.88	93.55	16	0	622	1497	0	0	0	0
	D	11D0	0.390	w	14.04	12.28	0	0	0	0	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	165	165	131	272	154	154	122	254
	F	22C-5tph	1.266	-	45.58	0.00	165	26	1185	0	154	25	1139	0
6	c) AED excursion									355				-435
	Envelope loss/gain								2456	2426			3892	4036
12	a) Infiltration								402	112			387	107
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances @ 1200					0			0	0			0
	Subtotal (lines 6 to 13)								2858	2538			4278	4144
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								2858	2538			4278	4144
15	Duct loads						41%	61%	1181	1548	41%	61%	1767	2527
	Total room load								4039	4086			6046	6671
	Air required (cfm)								195	195			292	318

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Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

							Bath 8.0 ft 9.0 ft heat/cool 8.0 x 15.0 ft 120.0 ft²				Great Room 122.0 ft 9.0 ft heat/cool 1.0 x 1341.0 ft 1341.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
1	Room name													
2	Exposed wall													
3	Ceiling height													
4	Room dimensions													
5	Room area													
6	W	12E-3bw	0.059	n	2.12	0.92	0	0	0	0	0	0	0	0
.	D	11D0	0.390	n	14.04	12.28	0	0	0	0	0	0	0	0
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0	486	435	924	400
.	G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0	30	0	1166	2806
11	D	11D0	0.390	e	14.04	12.28	0	0	0	0	21	21	295	258
	W	12E-3bw	0.059	s	2.12	0.92	0	0	0	0	261	261	554	240
	G	1A-c1ob	1.080	s	38.88	42.72	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	72	64	136	59	351	309	656	284
	G	1A-c1ob	1.080	w	38.88	93.55	8	0	311	748	0	0	0	0
	D	11D0	0.390	w	14.04	12.28	0	0	0	0	42	42	590	516
	C	16A-44ad	0.022	-	0.79	1.65	120	120	95	198	1341	1341	1062	2213
	F	22C-5tph	1.266	-	45.58	0.00	120	8	365	0	1341	122	5560	0
6	c) AED excursion									201				-815
	Envelope loss/gain								907	1206			10808	5903
12	a) Infiltration								139	39			2123	590
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	2			460
		Appliances @ 1200					0			0	3			3600
	Subtotal (lines 6 to 13)								1046	1245			12931	10553
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								1046	1245			12931	10553
15	Duct loads						41%	61%	432	759	41%	61%	5342	6436
	Total room load								1478	2005			18273	16989
	Air required (cfm)								71	95			883	809

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Right-J Worksheet
Entire House
A.E.C. HEAT AND AIR, INC

Job: McCray
Date: Jan 07, 2008
By: Sara Bucchi

1	Room name						Room2				Laundry			
2	Exposed wall						4.0 ft				22.0 ft			
3	Ceiling height						8.0 ft		heat/cool		8.0 ft		heat/cool	
4	Room dimensions						8.0 x 4.0 ft				8.0 x 14.0 ft			
5	Room area						32.0 ft²				112.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12E-3bw	0.059	n	2.12	0.92	32	32	68	29	112	91	193	84
.	└D	11D0	0.390	n	14.04	12.28	0	0	0	0	21	21	295	258
.	W	12E-3bw	0.059	e	2.12	0.92	0	0	0	0	64	55	117	51
.	└G	1A-c1ob	1.080	e	38.88	93.55	0	0	0	0	9	0	350	842
11	└D	11D0	0.390	e	14.04	12.28	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	s	2.12	0.92	0	0	0	0	0	0	0	0
	└G	1A-c1ob	1.080	s	38.88	42.72	0	0	0	0	0	0	0	0
	W	12E-3bw	0.059	w	2.12	0.92	0	0	0	0	0	0	0	0
	└G	1A-c1ob	1.080	w	38.88	93.55	0	0	0	0	0	0	0	0
	└D	11D0	0.390	w	14.04	12.28	0	0	0	0	0	0	0	0
	C	16A-44ad	0.022	-	0.79	1.65	32	32	25	53	112	112	89	185
	F	22C-5tph	1.266	-	45.58	0.00	32	4	182	0	112	22	1003	0
6	c) AED excursion									-9				-64
	Envelope loss/gain								276	73			2046	1355
12	a) Infiltration								62	17			340	95
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances @ 1200					0			0	0			0
	Subtotal (lines 6 to 13)								337	90			2387	1449
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								337	90			2387	1449
15	Duct loads						41%	61%	139	55	41%	61%	986	884
	Total room load								477	145			3372	2333
	Air required (cfm)								23	7			163	111

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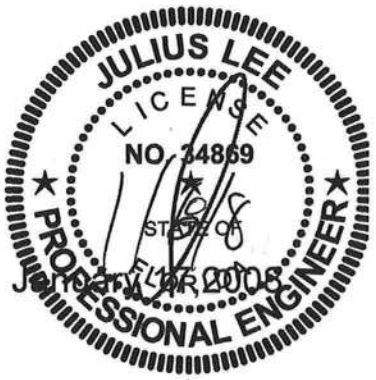
Builders FirstSource



Project Information for: L162006

...	...
Address:	712 Southeast Race Track
...	Lake City, FL
County:	Columbia
Truss Count:	19
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): 55.0	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date
Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Seal Date
1	J1927381	PB1	1/17/08
2	J1927382	PB1A	1/17/08
3	J1927383	PB1AG	1/17/08
4	J1927384	PB1G	1/17/08
5	J1927385	T01	1/17/08
6	J1927386	T01G	1/17/08
7	J1927387	T02	1/17/08
8	J1927388	T02A	1/17/08
9	J1927389	T02B	1/17/08
10	J1927390	T04	1/17/08
11	J1927391	T04A	1/17/08
12	J1927392	T04G	1/17/08
13	J1927393	T05G	1/17/08
14	J1927394	T06	1/17/08
15	J1927395	T06G	1/17/08
16	J1927396	T07	1/17/08
17	J1927397	T07G	1/17/08
18	J1927381A	PB1_ALT	1/17/08
19	J1927382A	PB1A_ALT	1/17/08

Job	Truss	Truss Type	Qty	Ply	
L162006	PB1	PIGGYBACK	9	1	J1927381
					Job Reference (optional)

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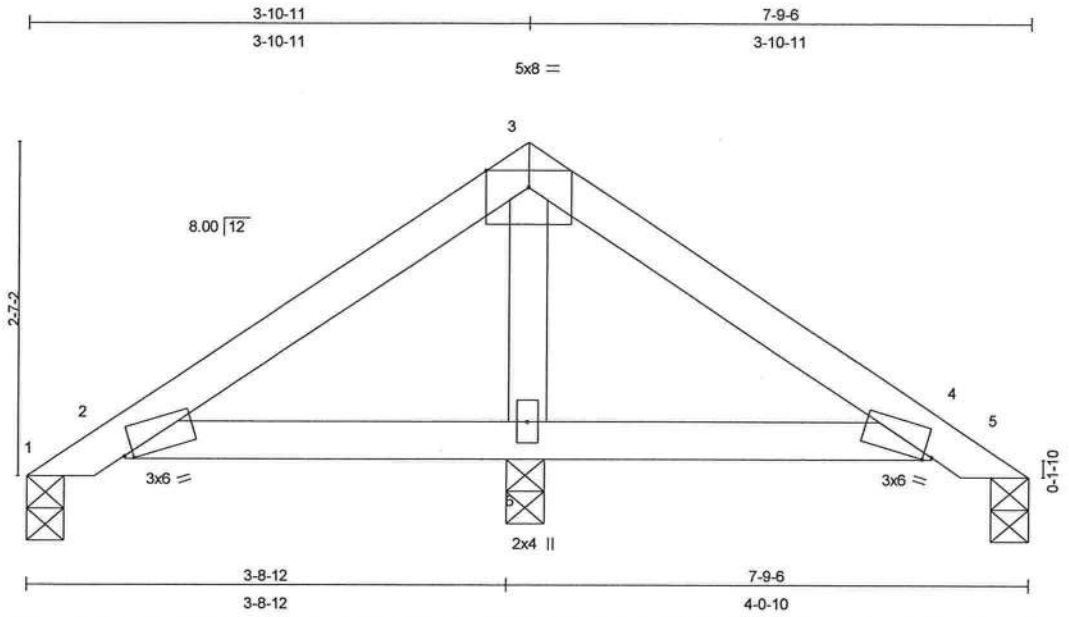

Plate Offsets (X,Y): [2:0-0-13,0-0-6], [4:0-0-13,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	-0.01	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=38/0-3-8, 5=38/0-3-8, 6=404/0-3-8
Max Horz 1=-69(load case 4)
Max Uplift 1=-11(load case 11), 5=-27(load case 4), 6=-89(load case 6)
Max Grav 1=64(load case 10), 5=64(load case 11), 6=404(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-60/64, 2-3=-74/179, 3-4=-74/179, 4-5=-30/19
BOT CHORD 2-6=-102/122, 4-6=-102/122
WEBS 3-6=-335/220

JOINT STRESS INDEX
2 = 0.28, 3 = 0.22, 4 = 0.28 and 6 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Job	Truss	Truss Type	Qty	Ply	J1927381
L162006	PB1	PIGGYBACK	9	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 27 lb uplift at joint 5 and 89 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	
L162006	PB1A	PIGGYBACK 4x6 =	23	1	J1927382 Job Reference (optional)

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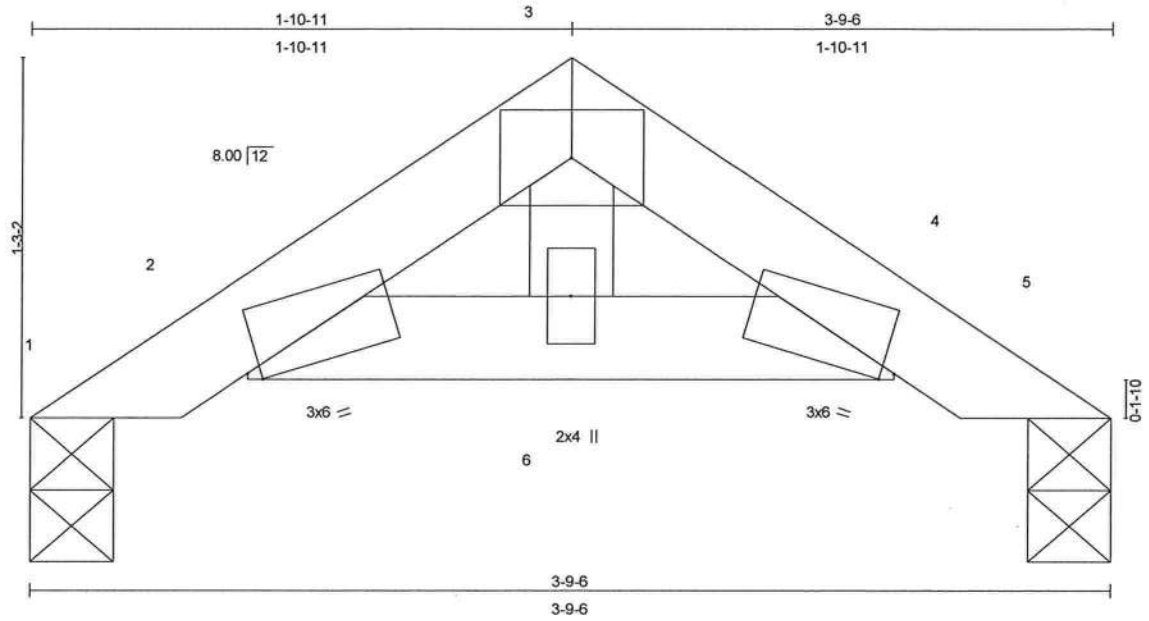

Plate Offsets (X,Y): [2:0-0-9,Edge], [4:0-0-9,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.00	6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=113/0-3-8, 5=113/0-3-8
Max Horz 1=-32(load case 4)
Max Uplift 1=-23(load case 6), 5=-23(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-55/37, 2-3=-177/95, 3-4=-177/95, 4-5=-55/37
BOT CHORD 2-6=-46/147, 4-6=-46/147
WEBS 3-6=-31/76

JOINT STRESS INDEX
2 = 0.21, 3 = 0.06, 4 = 0.21 and 6 = 0.06

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Job	Truss	Truss Type	Qty	Ply	J1927382
L162006	PB1A	PIGGYBACK	23	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 23 lb uplift at joint 5.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	
L162006	PB1AG	PIGGYBACK 4x6 =	2	1	J1927383 Job Reference (optional)

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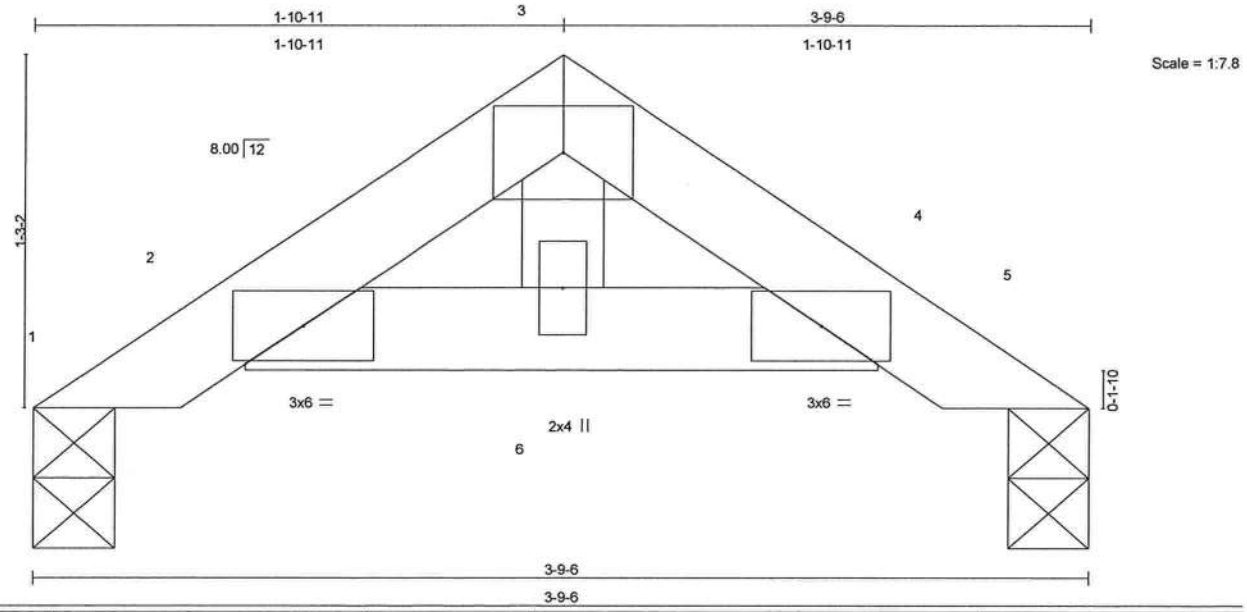

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=113/0-3-8, 5=113/0-3-8
Max Horz 1=-40(load case 4)
Max Uplift 1=-46(load case 6), 5=-46(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-55/39, 2-3=-177/95, 3-4=-177/95, 4-5=-55/37
BOT CHORD 2-6=-46/147, 4-6=-46/147
WEBS 3-6=-31/76

JOINT STRESS INDEX
2 = 0.22, 3 = 0.05, 4 = 0.22 and 6 = 0.05

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	J1927383
L162006	PB1AG	PIGGYBACK	2	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 46 lb uplift at joint 5.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	J1927384
L162006	PB1G	GABLE	1	1	Job Reference (optional)

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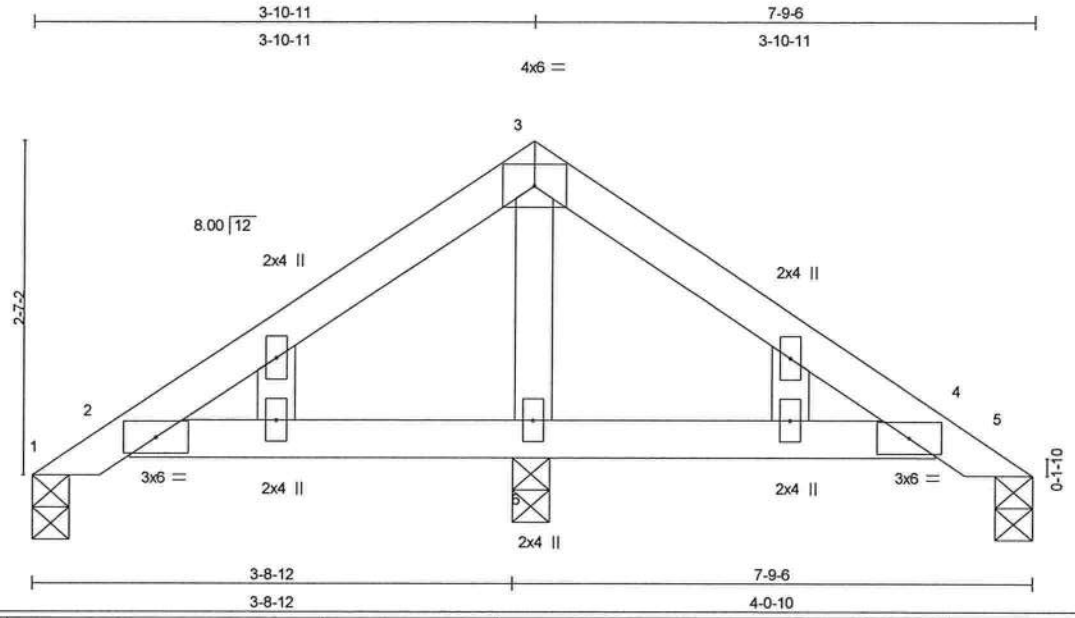

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.02	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=91/0-3-8, 5=91/0-3-8, 6=950/0-3-8
Max Horz 1=-87(load case 4)
Max Uplift 1=-38(load case 7), 5=-53(load case 7), 6=-445(load case 6)
Max Grav 1=117(load case 10), 5=117(load case 11), 6=950(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-71/81, 2-3=-199/429, 3-4=-199/429, 4-5=-54/26
BOT CHORD	2-6=-237/197, 4-6=-237/197
WEBS	3-6=-828/495

JOINT STRESS INDEX

2 = 0.64, 3 = 0.68, 4 = 0.64, 6 = 0.30, 7 = 0.00, 8 = 0.00, 9 = 0.00 and 10 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Job	Truss	Truss Type	Qty	Ply	J1927384
L162006	PB1G	GABLE	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 53 lb uplift at joint 5 and 445 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-153(F=-87), 2-3=-141(F=-87), 3-4=-141(F=-87), 4-5=-153(F=-87), 2-4=-10

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Job	Truss	Truss Type	Qty	Ply	J1927385
L162006	T01	FINK	11	1	Job Reference (optional)

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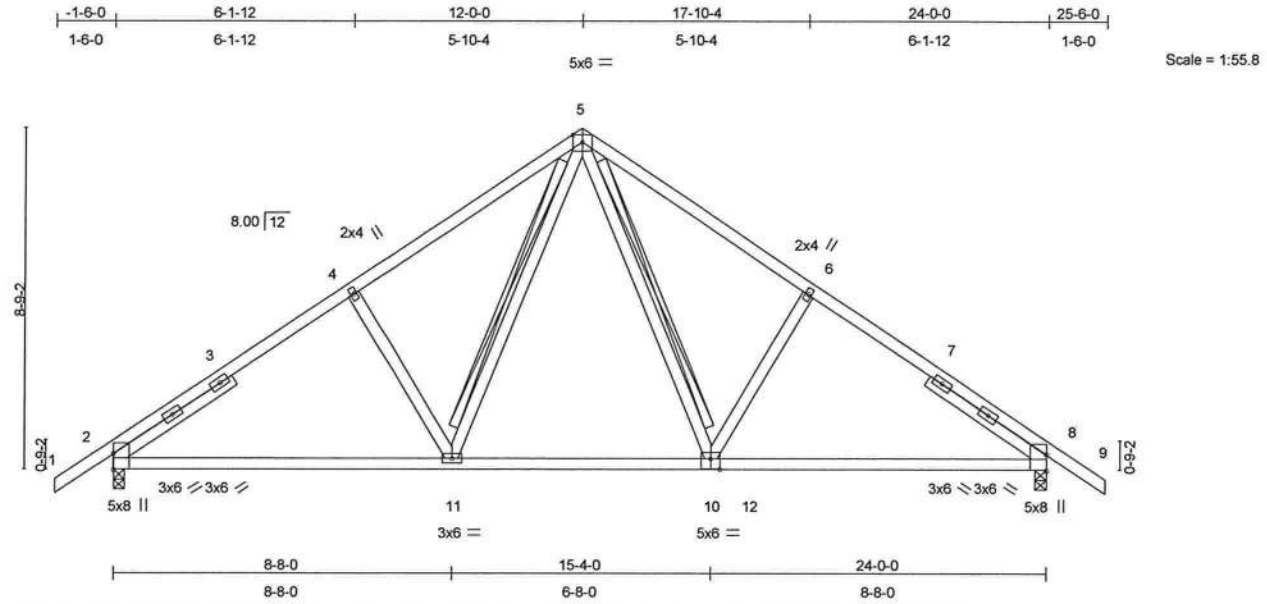

Plate Offsets (X,Y): [10:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.31	10-11	>932	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.26	10-11	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.43	Horz(TL)	-0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.1D
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 4 SYP No.3 3-8-4, Right 2 X 4 SYP No.3 3-8-4

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-3-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-9-6 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-11, 5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1056/0-3-8, 8=1062/0-3-8
Max Horz 2=233(load case 5)
Max Uplift 2=-664(load case 6), 8=-668(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/28, 2-3=-1431/1626, 3-4=-1331/1655, 4-5=-1249/1697, 5-6=-1259/1708, 6-7=-1340/1667, 7-8=-1440/1637, 8-9=0/28
BOT CHORD	2-11=-1197/1072, 10-11=-769/791, 10-12=-1207/1080, 8-12=-1207/1080
WEBS	4-11=-221/217, 5-11=-871/523, 5-10=-897/544, 6-10=-221/217

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JOINT STRESS INDEX

2 = 0.60, 2 = 0.29, 2 = 0.29, 3 = 0.00, 4 = 0.33, 5 = 0.57, 6 = 0.33, 7 = 0.00, 8 = 0.60, 8 = 0.29, 8 = 0.29, 10 = 0.78 and 11 = 0.47

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Job	Truss	Truss Type	Qty	Ply	J1927385
L162006	T01	FINK	11	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 664 lb uplift at joint 2 and 668 lb uplift at joint 8.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-9=-54, 2-11=-10, 11-12=-70, 8-12=-10

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Job	Truss	Truss Type	Qty	Ply	J1927386
L162006	T01G	GABLE	1	1	Job Reference (optional)

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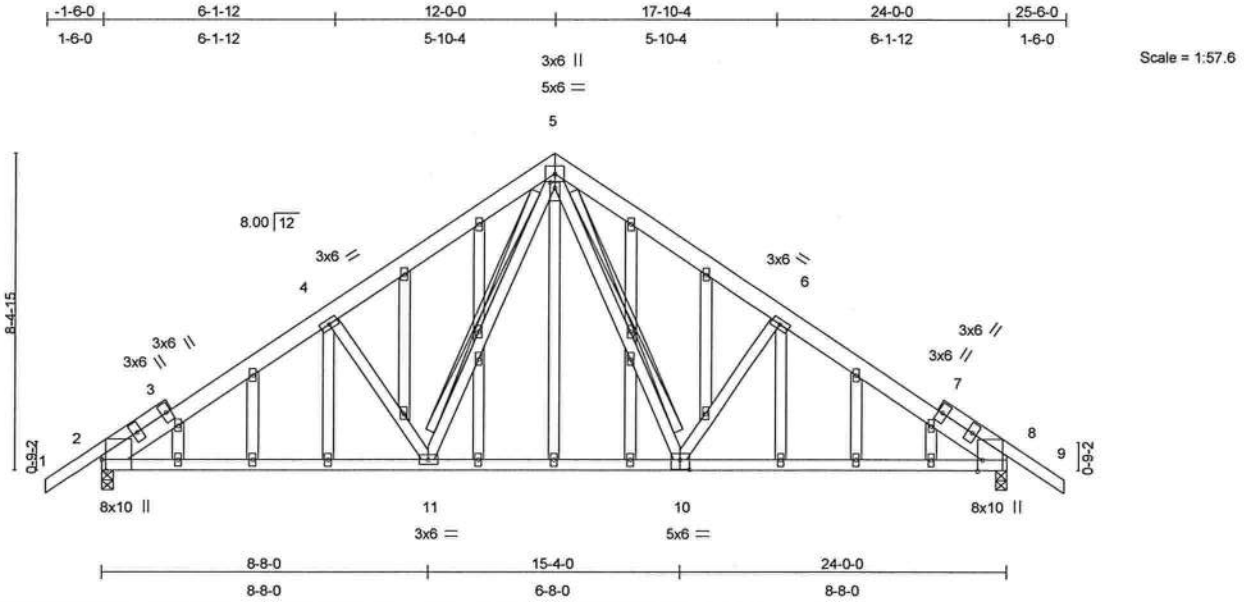

Plate Offsets (X,Y): [2:0-3-8,Edge], [5:0-1-12,0-1-8], [8:0-3-8,Edge], [10:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.28	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.18	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.37	Horz(TL)	-0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.2, 7-9 2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.1D
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-1-11 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-11, 5-10 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1295/0-3-8, 8=1295/0-3-8
Max Horz 2=-280(load case 4)
Max Uplift 2=-1103(load case 6), 8=-1103(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-17/46, 2-3=-1740/2010, 3-4=-1614/1981, 4-5=-1451/1912, 5-6=-1451/1912, 6-7=-1614/1981, 7-8=-1740/2010, 8-9=-17/46
BOT CHORD	2-11=-1548/1343, 10-11=-908/895, 8-10=-1548/1343
WEBS	4-11=-476/551, 5-11=-872/515, 5-10=-872/515, 6-10=-476/551

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JOINT STRESS INDEX

2 = 0.57, 3 = 0.00, 3 = 0.33, 3 = 0.47, 3 = 0.47, 4 = 0.37, 5 = 0.79, 5 = 0.26, 6 = 0.37, 7 = 0.00, 7 = 0.33, 7 = 0.47, 7 = 0.47, 8 = 0.57, 10 = 0.63, 11 = 0.46, 12 = 0.33, 13 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.33, 29 = 0.33 and 30 = 0.33

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Job	Truss	Truss Type	Qty	Ply	J1927386
L162006	T01G	GABLE	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1103 lb uplift at joint 2 and 1103 lb uplift at joint 8.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-87, 5-9=-87, 2-8=-10

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Job	Truss	Truss Type	Qty	Ply	J1927387
L162006	T02	PIGGYBACK ATTIC	13	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 16 12:53:22 2008 Page 1

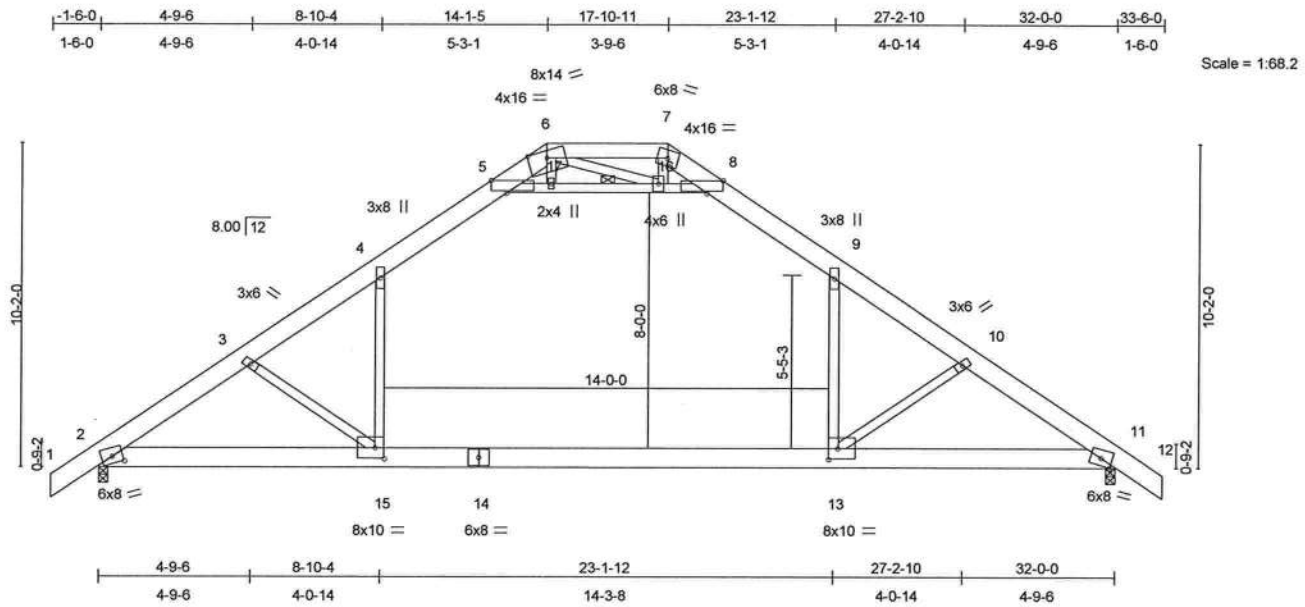

Plate Offsets (X,Y): [2:0-4-0,0-2-12], [5:Edge,0-4-11], [8:Edge,0-4-11], [11:0-4-0,0-2-12], [13:0-3-8,0-4-0], [15:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.65	13-15	>584	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-1.07	13-15	>357	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 272 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D *Except* 6-7 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-8

REACTIONS (lb/size) 2=1950/0-3-8, 11=1950/0-3-8
Max Horz 2=264(load case 5)
Max Uplift 2=-88(load case 6), 11=-88(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-3292/362, 3-4=-2938/294, 4-5=-2028/383, 5-6=0/768, 7-8=0/962, 8-9=-2029/383, 9-10=-2936/293, 10-11=-3288/361, 11-12=0/43, 6-7=0/1418
BOT CHORD 2-15=-165/2629, 14-15=0/2132, 13-14=0/2132, 11-13=-165/2626
WEBS 4-15=0/1409, 9-13=0/1402, 5-17=-3360/366, 16-17=-3310/363, 8-16=-3604/402, 3-15=-685/253, 10-13=-683/252, 6-16=-438/197, 6-17=-12/305, 7-16=-25/393

JOINT STRESS INDEX
2 = 0.74, 3 = 0.15, 4 = 0.42, 5 = 0.73, 6 = 0.32, 7 = 0.31, 8 = 0.79, 9 = 0.42, 10 = 0.15, 11 = 0.74, 13 = 0.28, 14 = 0.75, 15 = 0.28, 16 = 0.36 and 17 = 0.33

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NOTES
1) Unbalanced roof live loads have been considered for this design.

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	J1927387
L162006	T02	PIGGYBACK ATTIC	13	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-17, 16-17, 8-16; Wall dead load (5.0psf) on member(s).4-15, 9-13
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 2 and 88 lb uplift at joint 11.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	J1927388
L162006	T02A	PIGGYBACK ATTIC	4	3	Job Reference (optional)

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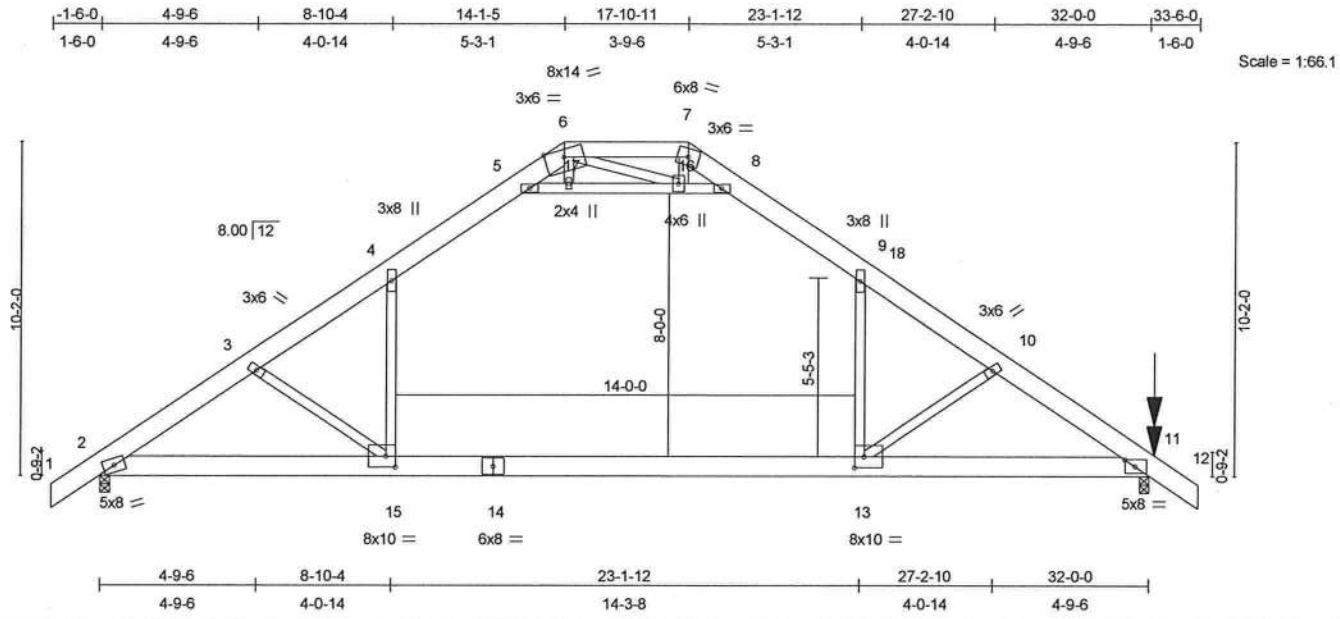

Plate Offsets (X,Y): [11:0-3-12,Edge], [13:0-3-8,0-4-0], [15:0-3-8,0-4-0]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.48	13-15	>793	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.78	13-15	>487	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.62	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 817 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* 6-7 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 6, 7

REACTIONS (lb/size) 2=4318/0-3-8, 11=7066/0-3-8
Max Horz 2=528(load case 4)
Max Uplift 2=-193(load case 5), 11=-312(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/86, 2-3=-7465/0, 3-4=-6878/0, 4-5=-4956/145, 5-6=0/2192, 7-8=0/2461, 8-9=-4688/135, 9-18=-7101/0, 10-18=-7704/0, 10-11=-9560/0, 11-12=0/86, 6-7=0/3397
BOT CHORD 2-15=-40/5889, 14-15=0/5184, 13-14=0/5184, 11-13=0/7792
WEBS 4-15=0/3003, 9-13=0/4362, 5-17=-8771/54, 16-17=-8639/57, 8-16=-8679/39, 3-15=-1085/352, 10-13=-3224/507, 6-16=-527/450, 6-17=0/802, 7-16=-49/704

JOINT STRESS INDEX
2 = 0.83, 3 = 0.16, 4 = 0.31, 5 = 0.97, 6 = 0.28, 7 = 0.25, 8 = 0.96, 9 = 0.45, 10 = 0.24, 11 = 0.84, 13 = 0.29, 14 = 0.44, 15 = 0.20, 16 = 0.37 and 17 = 0.34

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	J1927388
L162006	T02A	PIGGYBACK ATTIC	4	3	Job Reference (optional)

NOTES

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-17, 16-17, 8-16; Wall dead load (5.0psf) on member(s).4-15, 9-13
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2 and 312 lb uplift at joint 11.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-15=-20, 13-15=-220, 11-13=-220(F=-200), 1-4=-108, 4-5=-128, 5-6=-108, 7-8=-108, 8-9=-128, 9-18=-108, 11-12=-108, 6-7=-108, 5-8=-20
   Drag: 4-15=-20, 9-13=-20
   Concentrated Loads (lb)
   Vert: 11=-369
   Trapezoidal Loads (plf)
   Vert: 18=-244-to-11=-325
9) Attic Floor: Lumber Increase=1.00, Plate Increase=1.00
   Uniform Loads (plf)
   Vert: 2-15=-20, 13-15=-220, 11-13=-220(F=-200), 1-4=-28, 4-5=-48, 5-6=-28, 7-8=-28, 8-9=-48, 11-12=-28, 6-7=-28, 5-8=-20
   Drag: 4-15=-20, 9-13=-20
   Concentrated Loads (lb)
   Vert: 11=-197
   Trapezoidal Loads (plf)
   Vert: 9=-28-to-18=-31, 18=-52-to-11=-130
10) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-15=-20, 13-15=-220, 11-13=-220(F=-200), 1-4=-108, 4-5=-128, 5-6=-108, 7-8=-28, 8-9=-48, 9-18=-28, 11-12=-28, 6-7=-108, 5-8=-20
   Drag: 4-15=-20, 9-13=-20
   Concentrated Loads (lb)
   Vert: 11=-250
   Trapezoidal Loads (plf)
   Vert: 18=-85-to-11=-166
11) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-15=-20, 13-15=-220, 11-13=-220(F=-200), 1-4=-28, 4-5=-48, 5-6=-28, 7-8=-108, 8-9=-128, 9-18=-108, 11-12=-108, 6-7=-108, 5-8=-20
   Drag: 4-15=-20, 9-13=-20
   Concentrated Loads (lb)
   Vert: 11=-250
   Trapezoidal Loads (plf)
   Vert: 18=-165-to-11=-246

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Job	Truss	Truss Type	Qty	Ply	J1927389
L162006	T02B	PIGGYBACK ATTIC	1	2	Job Reference (optional)

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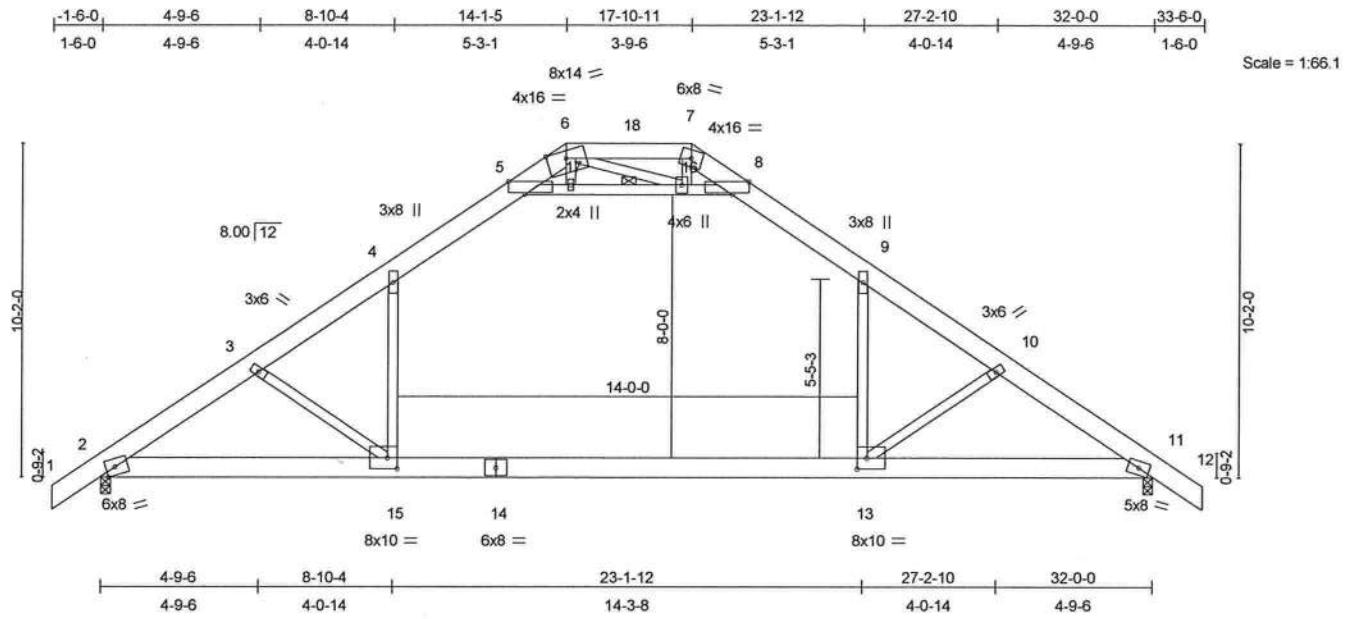

Plate Offsets (X,Y): [2:0-3-4,Edge], [5:Edge,0-4-11], [8:Edge,0-4-11], [13:0-3-8,0-4-0], [15:0-3-8,0-4-0]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.66	13-15	>573	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-1.10	13-15	>347	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.06	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 545 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* 6-7 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-8
JOINTS 1 Brace at Jt(s): 6, 7

REACTIONS (lb/size) 2=4244/0-3-8, 11=4934/0-3-8
Max Horz 2=528(load case 4)
Max Uplift 2=-271(load case 5), 11=-461(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/86, 2-3=-7260/54, 3-4=-6618/49, 4-5=-4737/297, 5-6=0/1589, 7-8=0/1797, 8-9=-4817/273, 9-10=-6945/100, 10-11=-8107/249, 11-12=0/86, 6-18=0/2664, 7-18=0/2664
BOT CHORD 2-15=-191/5759, 14-15=0/4922, 13-14=0/4922, 11-13=-77/6468
WEBS 4-15=0/2928, 9-13=0/2998, 5-17=-7704/255, 16-17=-7587/255, 8-16=-7676/116, 3-15=-1186/316, 10-13=-2060/627, 6-16=-688/489, 6-17=0/711, 7-16=0/644

JOINT STRESS INDEX
2 = 0.81, 3 = 0.16, 4 = 0.45, 5 = 0.87, 6 = 0.36, 7 = 0.30, 8 = 0.87, 9 = 0.46, 10 = 0.23, 11 = 0.84, 13 = 0.30, 14 = 0.71, 15 = 0.29, 16 = 0.37 and 17 = 0.34

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

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Job	Truss	Truss Type	Qty	Ply	J1927389
L162006	T02B	PIGGYBACK ATTIC	1	2	Job Reference (optional)

NOTES

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-17, 16-17, 8-16; Wall dead load (5.0psf) on member(s).4-15, 9-13
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2 and 461 lb uplift at joint 11.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-15=-20, 13-15=-220, 11-13=-20, 1-4=-108, 4-5=-128, 5-6=-108, 7-8=-195(F=-87), 8-9=-215(F=-87), 9-11=-195(F=-87), 11-12=-108, 6-18=-108, 7-18=-195(F=-87), 5-8=-20
   Drag: 4-15=-20, 9-13=-20

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Job	Truss	Truss Type	Qty	Ply	J1927390
L162006	T04	ATTIC	8	1	Job Reference (optional)

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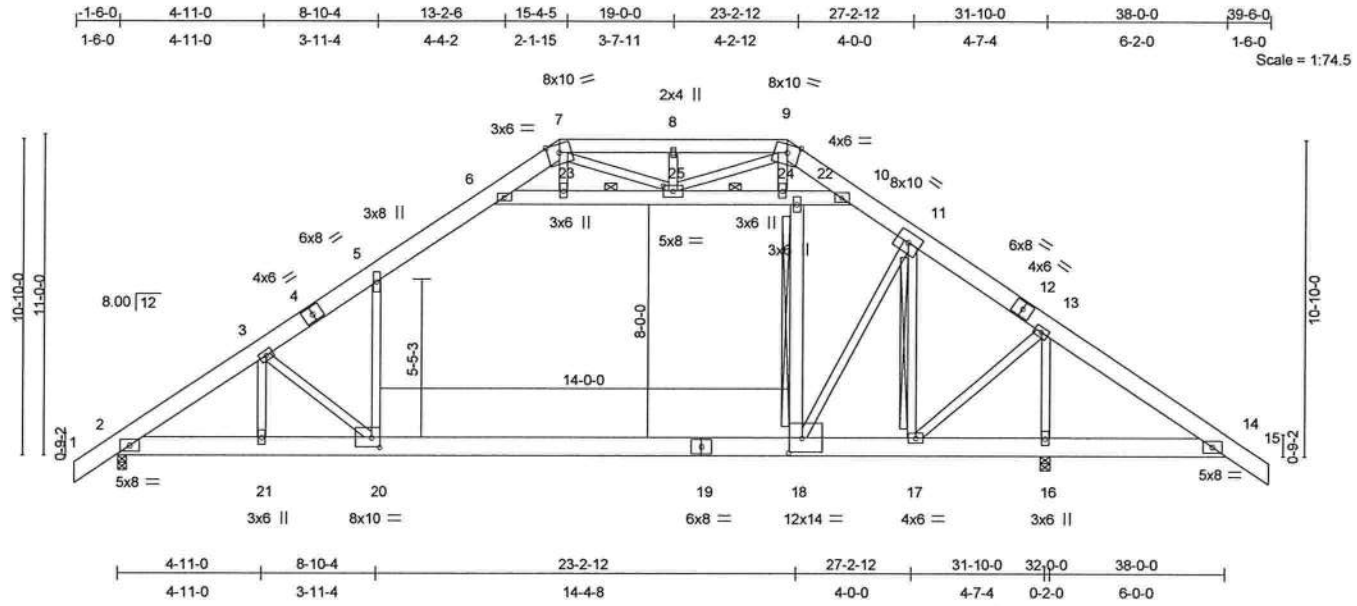

Plate Offsets (X,Y): [18:0-5-8,0-6-0], [20:0-3-8,0-4-0], [25:0-4-0,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.52	18-20	>737	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.86	18-20	>441	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.03	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 379 lb	

LUMBER

TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
7-9 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
6-10 2 X 6 SYP No.1D, 18-22 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins, except
2-0-0 oc purlins (10-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 Rows at 1/3 pts 6-10
T-Brace: 2 X 4 SYP No.3 - 11-17, 18-22
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1909/0-3-8, 16=2431/0-4-0
Max Horz 2=285(load case 5)
Max Uplift 2=-76(load case 6), 16=-313(load case 7)

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Job	Truss	Truss Type	Qty	Ply	J1927390
L162006	T04	ATTIC	8	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/43, 2-3=-2929/290, 3-4=-2758/134, 4-5=-2652/142, 5-6=-1920/248, 6-7=-39/325, 9-10=-465/585, 10-11=-1726/100, 11-12=-1004/6, 12-13=-1101/0, 13-14=-479/429, 14-15=0/42, 7-8=-144/739, 8-9=-144/739

BOT CHORD 2-21=-113/2392, 20-21=-114/2390, 19-20=0/1941, 18-19=0/1941, 17-18=0/745, 16-17=-271/526, 14-16=-260/523

WEBS 6-23=-2268/197, 23-25=-2257/199, 24-25=-2732/787, 22-24=-2720/751, 10-22=-2721/752, 5-20=0/1299, 11-18=-365/2467, 11-17=-1748/203, 13-17=-236/1358, 3-20=-603/351, 3-21=-327/98, 13-16=-2152/685, 18-22=-494/450, 7-23=0/294, 9-24=0/390, 8-25=-327/151, 7-25=-460/251, 9-25=-415/63

JOINT STRESS INDEX

2 = 0.58, 3 = 0.31, 4 = 0.67, 5 = 0.39, 6 = 0.81, 7 = 0.20, 8 = 0.33, 9 = 0.20, 10 = 0.72, 11 = 0.55, 12 = 0.12, 13 = 0.64, 14 = 0.17, 16 = 0.34, 17 = 0.56, 18 = 0.33, 19 = 0.32, 20 = 0.27, 21 = 0.15, 22 = 0.17, 23 = 0.15, 24 = 0.15 and 25 = 0.38

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 5-6, 6-23, 23-25, 24-25, 22-24, 10-22; Wall dead load (5.0psf) on member(s).5-20, 18-22
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 2 and 313 lb uplift at joint 16.

LOAD CASE(S) Standard

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January 17,2008

Job	Truss	Truss Type	Qty	Ply	J1927391
L162006	T04A	ATTIC	1	2	Job Reference (optional)

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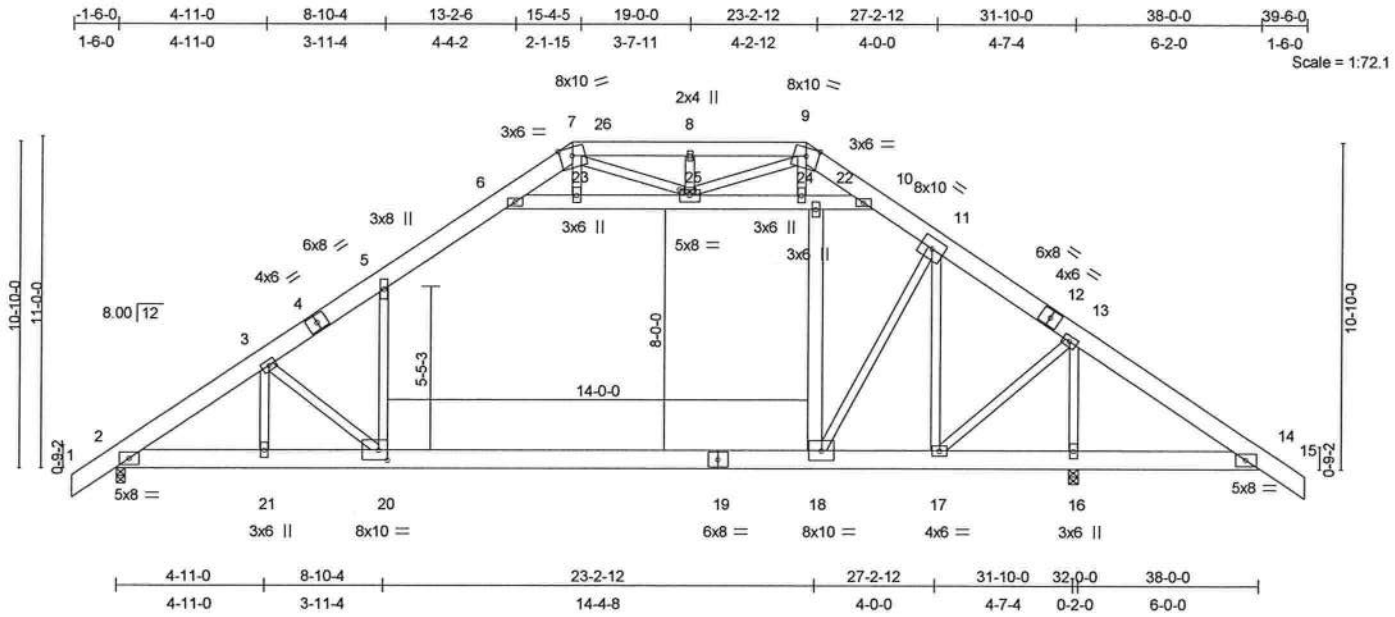

Plate Offsets (X,Y): [20:0-3-8,0-4-0], [25:0-4-0,0-2-4]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.51	18-20	>740	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.86	18-20	>444	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.78	Horz(TL)	0.04	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 758 lb	

LUMBER

TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* 7-9 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except* 6-10 2 X 6 SYP No.1D, 18-22 2 X 6 SYP No.1D

BRACING

TOP CHORD 2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-10
JOINTS 1 Brace at Jt(s): 9, 7

REACTIONS (lb/size) 2=4162/0-3-8, 16=5898/0-4-0
Max Horz 2=569(load case 4)
Max Uplift 2=-248(load case 5), 16=-912(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/86, 2-3=-6429/92, 3-4=-6140/0, 4-5=-5928/0, 5-6=-4460/171, 6-7=-655/862, 9-10=-1014/1231, 10-11=-4252/72, 11-12=-2570/131, 12-13=-2963/144, 13-14=-646/816, 14-15=0/84, 7-26=-420/1520, 8-26=-420/1520, 8-9=-420/1520
BOT CHORD 2-21=-396/5249, 20-21=-397/5245, 19-20=-127/4408, 18-19=-127/4408, 17-18=0/2040, 16-17=-505/730, 14-16=-479/726
WEBS 6-23=-4600/75, 23-25=-4576/80, 24-25=-5239/809, 22-24=-5214/744, 10-22=-5216/746, 5-20=0/2604, 11-18=-436/4887, 11-17=-3962/59, 13-17=-210/3410, 3-20=-1126/414, 3-21=-707/177, 13-16=-5302/767, 18-22=-986/745, 7-23=0/644, 9-24=0/719, 8-25=-928/822, 7-25=-779/307, 9-25=-902/321

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JOINT STRESS INDEX

2 = 0.64, 3 = 0.32, 4 = 0.69, 5 = 0.40, 6 = 0.85, 7 = 0.21, 8 = 0.34, 9 = 0.21, 10 = 0.95, 11 = 0.56, 12 = 0.13, 13 = 0.83, 14 = 0.18, 16 = 0.43, 17 = 0.72, 18 = 0.55, 19 = 0.38, 20 = 0.28, 21 = 0.16, 22 = 0.15, 23 = 0.16, 24 = 0.16 and 25 = 0.39

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Job	Truss	Truss Type	Qty	Ply	J1927391
L162006	T04A	ATTIC	1	2	Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 5-6, 6-23, 23-25, 24-25, 22-24, 10-22; Wall dead load (5.0psf) on member(s).5-20, 18-22
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 2 and 912 lb uplift at joint 16.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-20=-20, 18-20=-220, 14-18=-20, 1-5=-108, 5-6=-128, 6-7=-108, 9-13=-195(F=-87), 13-15=-108, 6-10=-20, 7-26=-108, 9-26=-195(F=-87)
   Drag: 5-20=-20, 18-22=-20

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Job	Truss	Truss Type	Qty	Ply	J1927392
L162006	T04G	ATTIC	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 17 08:21:45 2008 Page 1

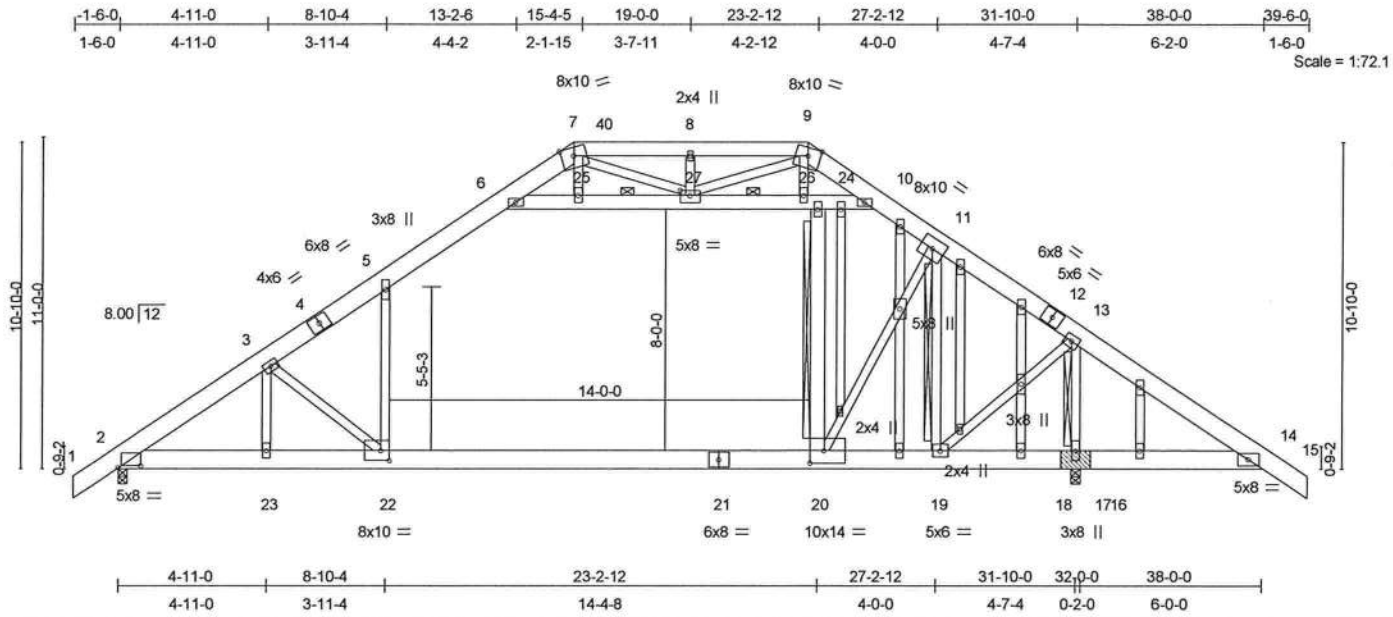

Plate Offsets (X,Y): [2:0-9-2,0-0-15], [20:0-5-8,0-5-0], [22:0-3-8,0-4-0], [27:0-4-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.52	20-22	>731	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.87	20-22	>437	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.04	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 422 lb	

LUMBER

TOP CHORD	2 X 8 SYP 2400F 2.0E *Except* 7-9 2 X 6 SYP No.1D
BOT CHORD	2 X 8 SYP 2400F 2.0E
WEBS	2 X 4 SYP No.3 *Except* 6-10 2 X 6 SYP No.1D, 20-24 2 X 6 SYP No.1D
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-0-2 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2 Rows at 1/3 pts 6-10
	T-Brace: 2 X 4 SYP No.3 - 11-19, 13-17, 20-24

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2197/0-3-8, 17=3981/0-4-11 (0-4-0 + bearing block)
Max Horz 2=362(load case 5)
Max Uplift 2=-441(load case 6), 17=-1401(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/43, 2-3=-3410/538, 3-4=-3273/398, 4-5=-3167/406, 5-6=-2436/513, 6-7=-556/778, 9-10=-686/985, 10-11=-2360/416, 11-12=-1330/275, 12-13=-1626/340, 13-14=-706/929, 14-15=-22/88, 7-40=-538/1281, 8-40=-538/1281, 8-9=-538/1281
BOT CHORD	2-23=-539/2784, 22-23=-540/2783, 21-22=-379/2375, 20-21=-379/2375, 19-20=-131/1074, 18-19=-599/695, 17-18=-599/695, 16-17=-581/687, 14-16=-581/687
WEBS	6-25=-2275/267, 25-27=-2264/269, 26-27=-2668/772, 24-26=-2642/735, 10-24=-2643/736, 5-22=0/1297, 11-20=-513/2685, 11-19=-2380/527, 13-19=-692/2247, 3-22=-549/355, 3-23=-359/87, 13-17=-3616/1436, 20-24=-701/613, 7-25=-17/335, 9-26=-155/528, 8-27=-604/347, 7-27=-632/294, 9-27=-741/379

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Job	Truss	Truss Type	Qty	Ply	J1927392
L162006	T04G	ATTIC	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.67, 3 = 0.32, 4 = 0.69, 5 = 0.40, 6 = 0.84, 7 = 0.26, 8 = 0.34, 9 = 0.26, 10 = 0.95, 11 = 0.61, 12 = 0.21, 13 = 0.83, 14 = 0.40, 16 = 0.00, 16 = 0.00, 17 = 0.59, 17 = 0.00, 18 = 0.00, 18 = 0.00, 19 = 0.73, 20 = 0.41, 21 = 0.41, 22 = 0.27, 23 = 0.16, 24 = 0.23, 25 = 0.16, 26 = 0.19, 27 = 0.47, 28 = 0.16, 29 = 0.16, 30 = 0.16, 31 = 0.16, 32 = 0.35, 33 = 0.34, 34 = 0.16, 35 = 0.16, 36 = 0.16, 37 = 0.42, 38 = 0.34 and 39 = 0.16

NOTES

1) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 17 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All plates are 3x6 MT20 unless otherwise indicated.
8) Gable studs spaced at 2-0-0 oc.
9) Ceiling dead load (5.0 psf) on member(s). 5-6, 6-25, 25-27, 26-27, 24-26, 10-24; Wall dead load (5.0psf) on member(s).5-22, 20-24
10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 20-22
11) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 2 and 1401 lb uplift at joint 17.
13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-22=-10, 20-22=-110, 14-20=-10, 1-5=-54, 5-6=-64, 6-7=-54, 9-13=-141(F=-87), 13-15=-114(F=-60), 6-10=-10, 7-40=-54, 9-40=-141(F=-87)
   Drag: 5-22=-10, 20-24=-10

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Job	Truss	Truss Type	Qty	Ply	J1927393
L162006	T05G	PIGGYBACK ATTIC	2	1	Job Reference (optional)

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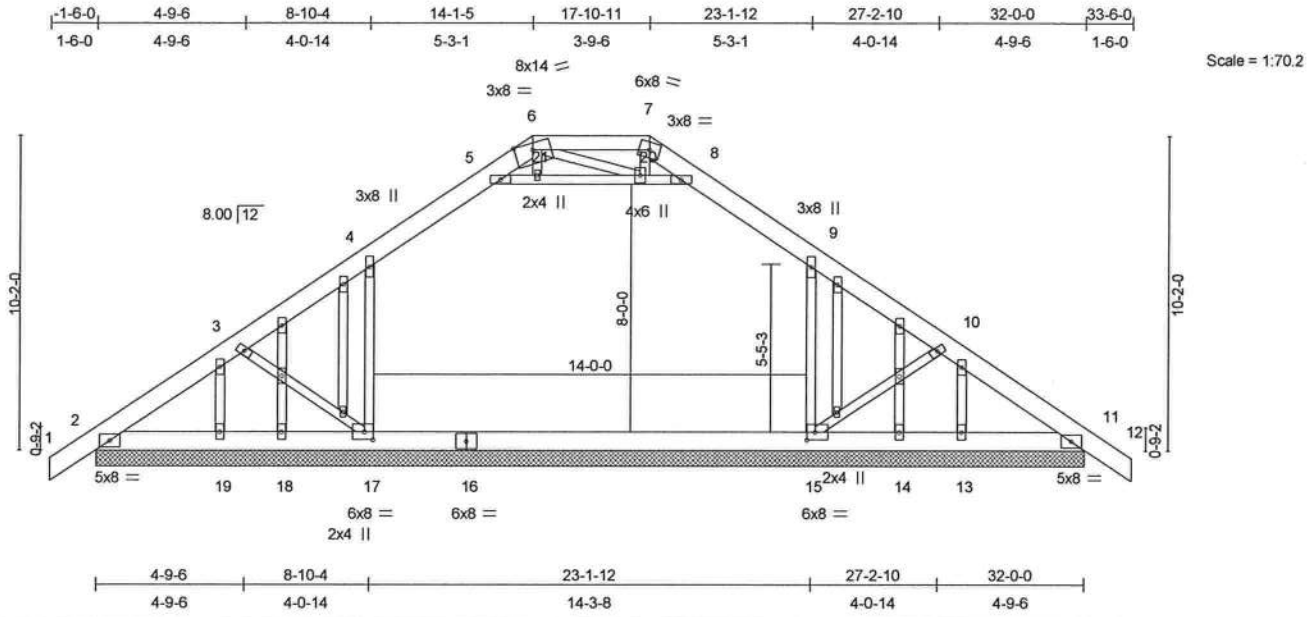

Plate Offsets (X,Y): [15:0-3-2,0-3-0], [17:0-3-5,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.00	12	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	0.00	12	n/r	90		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 302 lb	

LUMBER

TOP CHORD	2 X 8 SYP No.1D *Except* 6-7 2 X 6 SYP No.1D
BOT CHORD	2 X 8 SYP 2400F 2.0E
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1450/32-0-0, 17=1278/32-0-0, 15=1280/32-0-0, 11=1449/32-0-0, 19=136/32-0-0, 18=-106/32-0-0, 14=-106/32-0-0, 13=136/32-0-0
Max Horz 2=337(load case 5)
Max Uplift 2=-617(load case 6), 17=-560(load case 6), 15=-554(load case 7), 11=-625(load case 7), 19=-9(load case 5), 18=-236(load case 2), 14=-236(load case 2), 13=-4(load case 4)
Max Grav 2=1450(load case 1), 17=1305(load case 11), 15=1308(load case 12), 11=1449(load case 1), 19=220(load case 2), 13=220(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-35/109, 2-3=-1809/816, 3-4=-1299/655, 4-5=-1408/665, 5-6=-987/536, 7-8=-946/507, 8-9=-1409/665, 9-10=-1298/631, 10-11=-1807/787, 11-12=-35/109, 6-7=-814/477
BOT CHORD	2-19=-725/1325, 18-19=-725/1325, 17-18=-725/1325, 16-17=-489/972, 15-16=-489/972, 14-15=-582/1323, 13-14=-582/1323, 11-13=-582/1323
WEBS	4-17=-893/510, 9-15=-898/494, 5-21=-122/187, 20-21=-115/186, 8-20=-171/203, 3-17=-430/350, 10-15=-428/354, 6-20=-126/126, 6-21=0/45, 7-20=-25/77

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JOINT STRESS INDEX

2 = 0.47, 3 = 0.16, 4 = 0.27, 5 = 0.12, 6 = 0.14, 7 = 0.19, 8 = 0.12, 9 = 0.27, 10 = 0.16, 11 = 0.47, 13 = 0.16, 14 = 0.16, 15 = 0.16, 16 = 0.17, 17 = 0.15, 18 = 0.16, 19 = 0.16, 20 = 0.37, 21 = 0.34, 22 = 0.16, 23 = 0.65, 24 = 0.16, 25 = 0.34, 26 = 0.16, 27 = 0.34, 28 = 0.16, 29 = 0.16, 30 = 0.65 and 31 = 0.16

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Job	Truss	Truss Type	Qty	Ply	J1927393
L162006	T05G	PIGGYBACK ATTIC	2	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-21, 20-21, 8-20; Wall dead load (5.0psf) on member(s).4-17, 9-15
10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 617 lb uplift at joint 2, 560 lb uplift at joint 17 , 554 lb uplift at joint 15, 625 lb uplift at joint 11, 9 lb uplift at joint 19, 236 lb uplift at joint 18, 236 lb uplift at joint 14 and 4 lb uplift at joint 13.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-11=-10, 1-4=-141(F=-87), 4-5=-151(F=-87), 5-6=-141(F=-87), 7-8=-141(F=-87), 8-9=-151(F=-87), 9-12=-141(F=-87), 6-7=-141(F=-87), 5-8=-10
   Drag: 4-17=-10, 9-15=-10

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Job	Truss	Truss Type	Qty	Ply	J1927394
L162006	T06	MONO TRUSS	1	1	Job Reference (optional)

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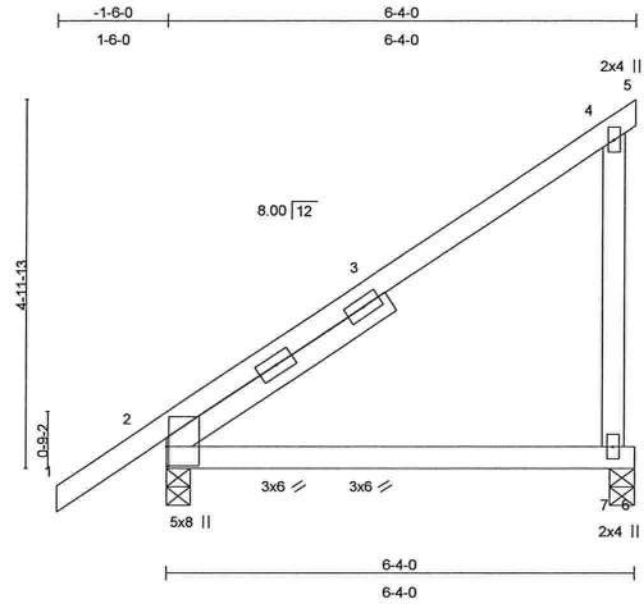

Plate Offsets (X,Y): [2:0-3-2,0-0-6] 0-1-12

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.19	2-7	>375	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.11	2-7	>670	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-7-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=284/0-4-0, 7=190/0-4-0
Max Horz 2=250(load case 6)
Max Uplift 2=-186(load case 6), 7=-249(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/28, 2-3=-137/14, 3-4=-123/66, 4-5=-2/0
BOT CHORD 2-7=0/0, 6-7=0/0
WEBS 4-7=-157/193

JOINT STRESS INDEX
2 = 0.10, 2 = 0.05, 2 = 0.05, 3 = 0.00, 4 = 0.10 and 7 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	J1927394
L162006	T06	MONO TRUSS	1	1	Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 2 and 249 lb uplift at joint 7.

LOAD CASE(S) Standard

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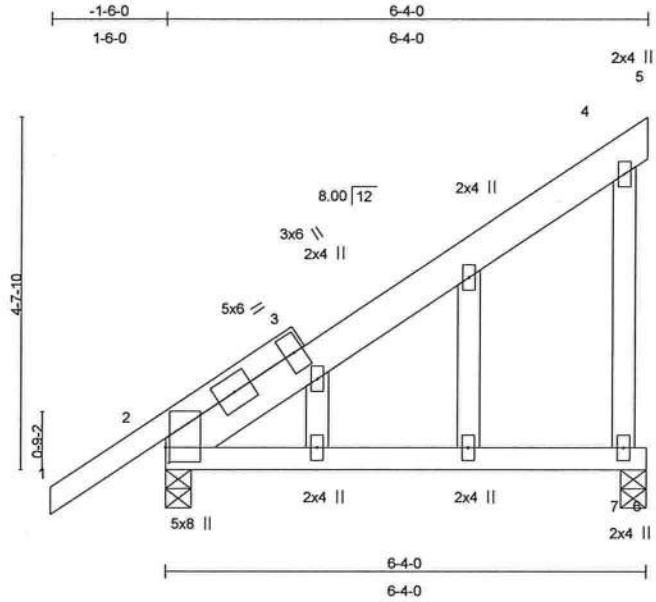
Job	Truss	Truss Type	Qty	Ply	J1927395
L162006	T06G	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 16 12:53:31 2008 Page 1



Scale = 1:28.6

Plate Offsets (X,Y): [2:0-2-5,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.19	2-7	>375	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.11	2-7	>670	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=439/0-4-0, 7=294/0-4-0
Max Horz 2=229(load case 6)
Max Uplift 2=-306(load case 6), 7=-304(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16/46, 2-3=-154/15, 3-4=-150/108, 4-5=-8/5
BOT CHORD 2-7=0/0, 6-7=0/0
WEBS 4-7=-261/296

JOINT STRESS INDEX
2 = 0.67, 3 = 0.35, 3 = 0.00, 3 = 0.00, 3 = 0.66, 4 = 0.15, 7 = 0.16, 8 = 0.00, 9 = 0.00 and 10 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

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January 17,2008

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Job	Truss	Truss Type	Qty	Ply	J1927395
L162006	T06G	MONO TRUSS	1	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Gable studs spaced at 2-0-0 oc.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 2 and 304 lb uplift at joint 7.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-87(F=-33), 4-5=-47(F=-33), 2-6=-10

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January 17,2008

Job	Truss	Truss Type	Qty	Ply	J1927396
L162006	T07	COMMON	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 16 12:53:32 2008 Page 1

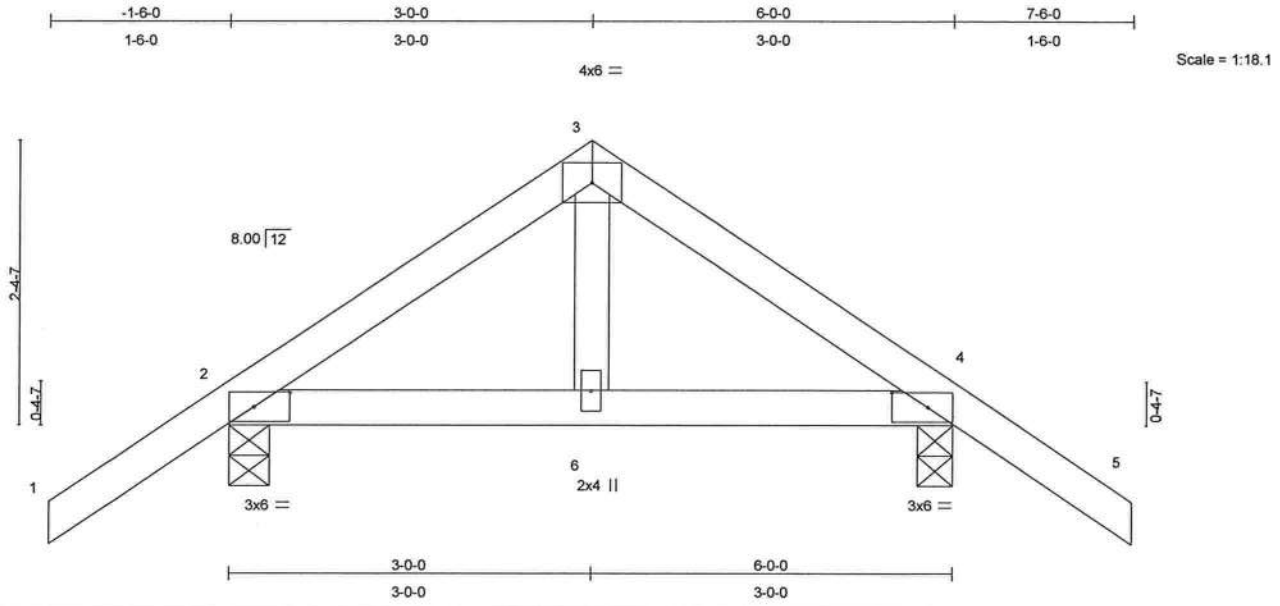

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.00	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=272/0-4-0, 4=271/0-3-8
Max Horz 2=-76(load case 4)
Max Uplift 2=-181(load case 6), 4=-180(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-176/42, 3-4=-175/41, 4-5=0/44
BOT CHORD 2-6=0/112, 4-6=0/112
WEBS 3-6=0/104

JOINT STRESS INDEX
2 = 0.15, 3 = 0.05, 4 = 0.15 and 6 = 0.07

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	J1927396
L162006	T07	COMMON	8	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 180 lb uplift at joint 4.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	J1927397
L162006	T07G	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Jan 16 12:53:33 2008 Page 1

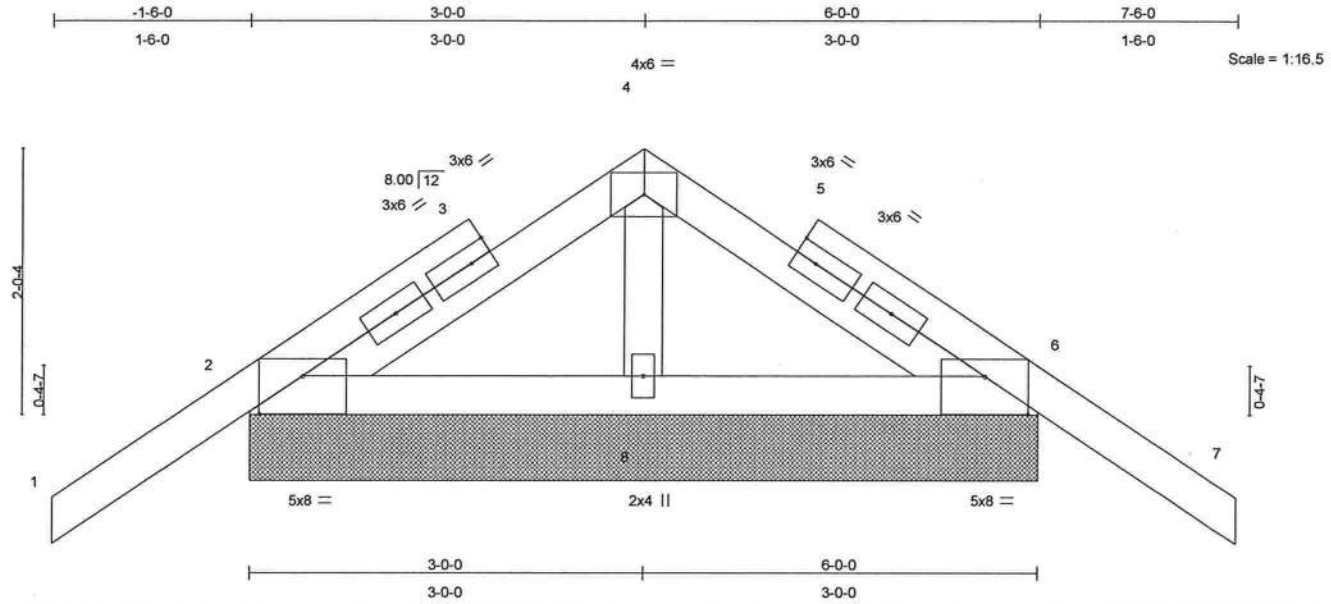

Plate Offsets (X,Y): [2:0-4-0,0-3-7], [6:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	7	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=281/6-0-0, 6=281/6-0-0, 8=281/6-0-0
Max Horz 2=-64(load case 4)
Max Uplift 2=-196(load case 6), 6=-208(load case 7), 8=-72(load case 6)
Max Grav 2=287(load case 10), 6=287(load case 11), 8=281(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5/70, 2-3=-37/40, 3-4=-21/88, 4-5=-7/88, 5-6=-37/31, 6-7=-5/70
BOT CHORD 2-8=-42/101, 6-8=-42/101
WEBS 4-8=-243/94

JOINT STRESS INDEX
2 = 0.55, 3 = 0.00, 3 = 0.23, 3 = 0.23, 4 = 0.06, 5 = 0.00, 5 = 0.23, 5 = 0.23, 6 = 0.55 and 8 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

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Job	Truss	Truss Type	Qty	Ply	J1927397
L162006	T07G	COMMON	2	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 208 lb uplift at joint 6 and 72 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-10

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Job	Truss	Truss Type	Qty	Ply	J1927381A
L162006	PB1_ALT	PIGGYBACK	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 17 08:44:19 2008 Page 1

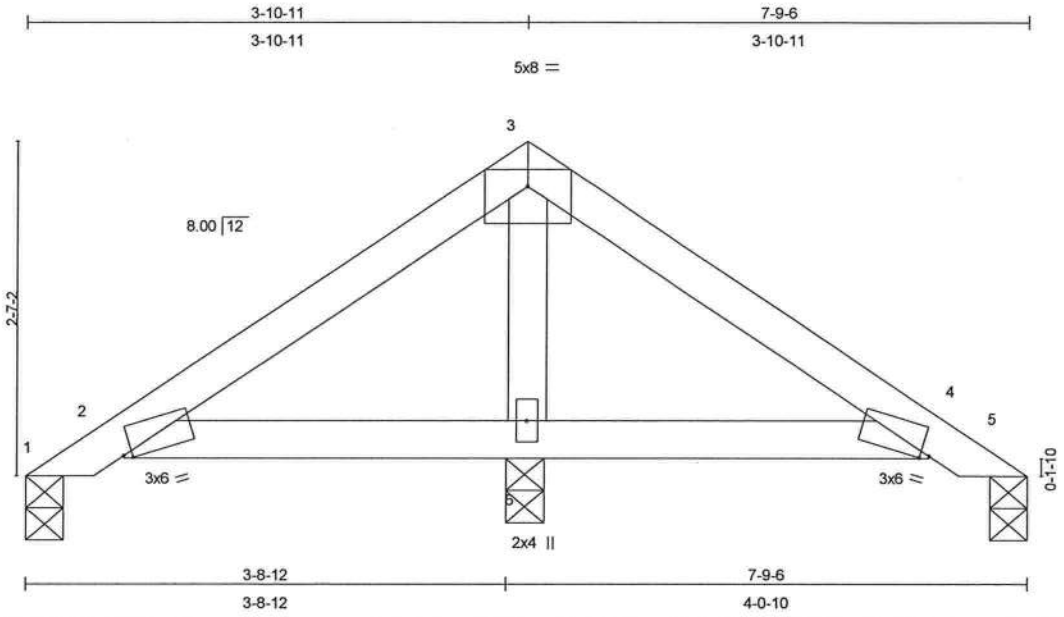

Plate Offsets (X,Y): [2:0-0-13,0-0-6], [4:0-0-13,0-0-6]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.02	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.03	2-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.20	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
(Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 3

REACTIONS (lb/size) 1=130/0-3-8, 5=130/0-3-8, 6=1355/0-3-8
Max Horz 1=-139(load case 4)
Max Uplift 1=-25(load case 7), 5=-65(load case 4), 6=-329(load case 6)
Max Grav 1=180(load case 10), 5=180(load case 11), 6=1355(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-116/128, 2-3=-242/607, 3-4=-242/607, 4-5=-84/37
BOT CHORD 2-6=-338/302, 4-6=-338/302
WEBS 3-6=-1162/675

JOINT STRESS INDEX
2 = 0.90, 3 = 0.77, 4 = 0.90 and 6 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	J1927381A
L162006	PB1_ALT	PIGGYBACK	9	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 65 lb uplift at joint 5 and 329 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-218(F=-87), 2-3=-195(F=-87), 3-4=-195(F=-87), 4-5=-218(F=-87), 2-4=-20

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Job	Truss	Truss Type	Qty	Ply	J1927382A
L162006	PB1A_ALT	PIGGYBACK 4x6 =	23	1	Job Reference (optional)

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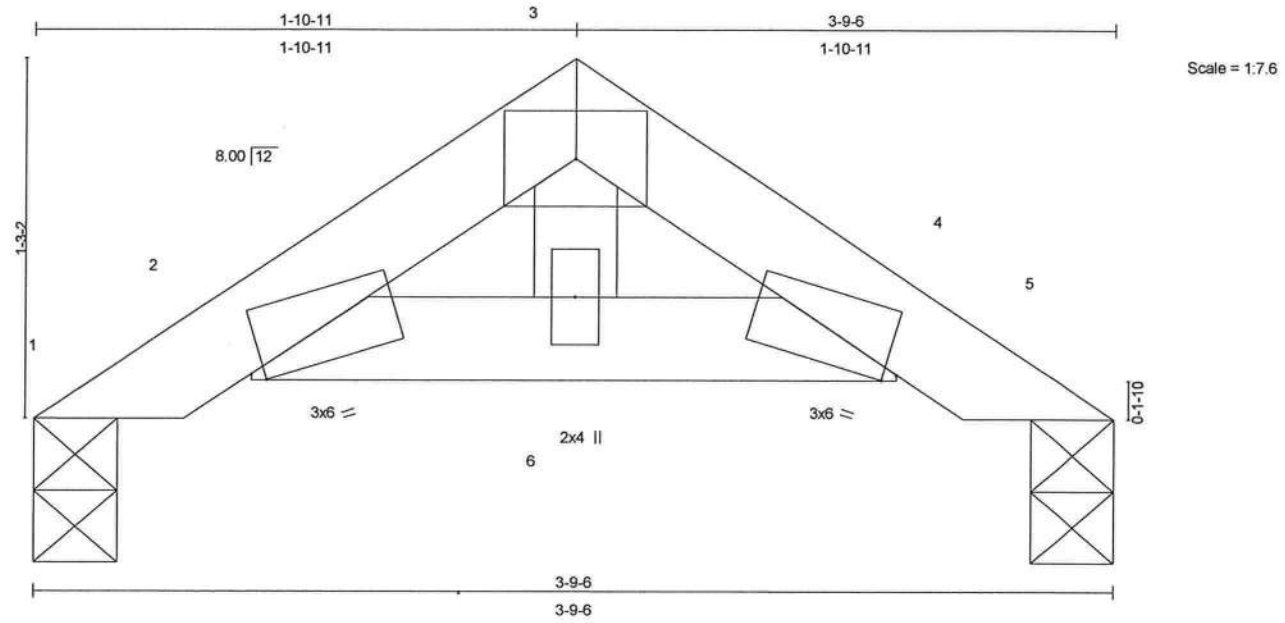

Plate Offsets (X,Y): [2:0-0-9,Edge], [4:0-0-9,Edge]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.01	6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.02	6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 3

REACTIONS (lb/size) 1=377/0-3-8, 5=377/0-3-8
Max Horz 1=-65(load case 4)
Max Uplift 1=-88(load case 6), 5=-88(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-184/107, 2-3=-600/316, 3-4=-600/316, 4-5=-184/107
BOT CHORD 2-6=-206/494, 4-6=-206/494
WEBS 3-6=-113/241

JOINT STRESS INDEX
2 = 0.73, 3 = 0.16, 4 = 0.73 and 6 = 0.18

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	J1927382A
L162006	PB1A_ALT	PIGGYBACK	23	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 1 and 88 lb uplift at joint 5.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-218(F=-87), 2-3=-195(F=-87), 3-4=-195(F=-87), 4-5=-218(F=-87), 2-4=-20

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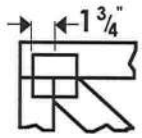
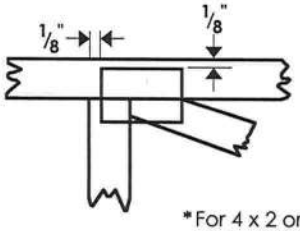
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

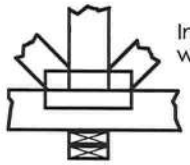
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

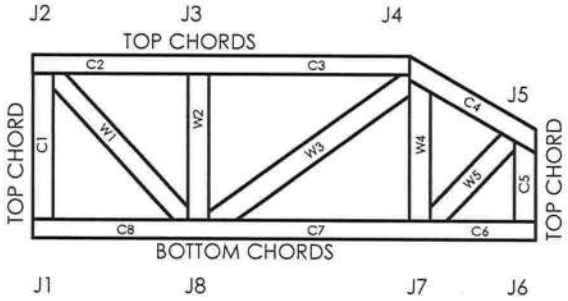
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

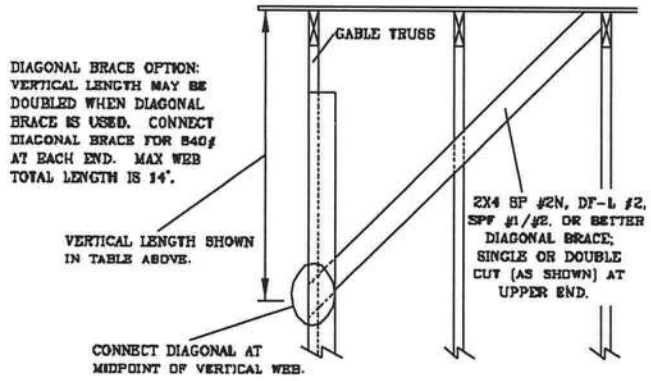
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

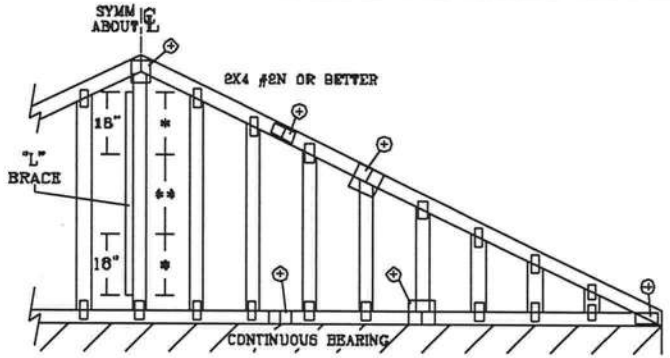
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34859
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 8"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 5"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 5"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

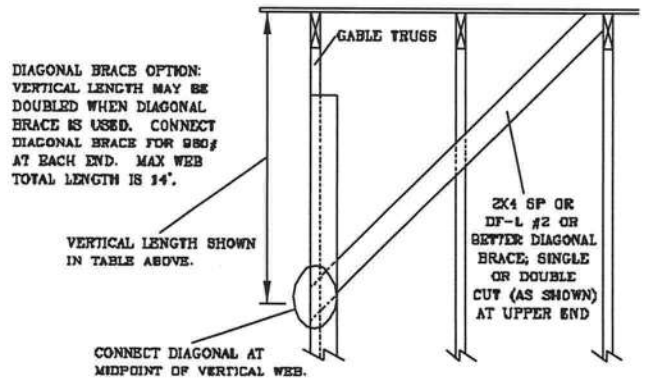
SOUTHERN PINE
#3
STUD
STANDARD

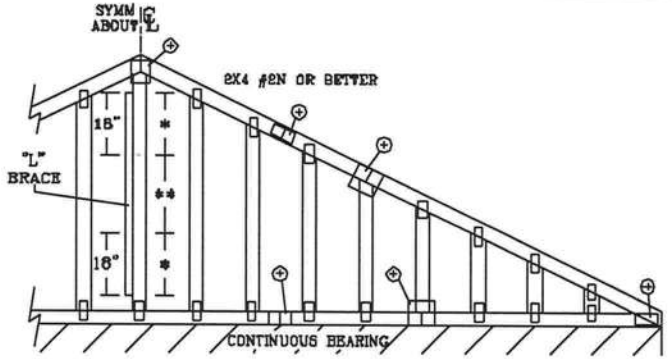
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

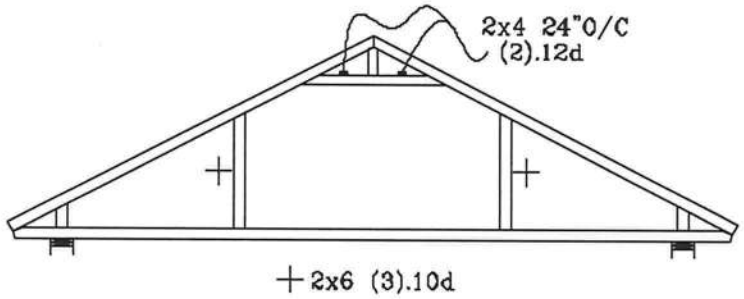
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

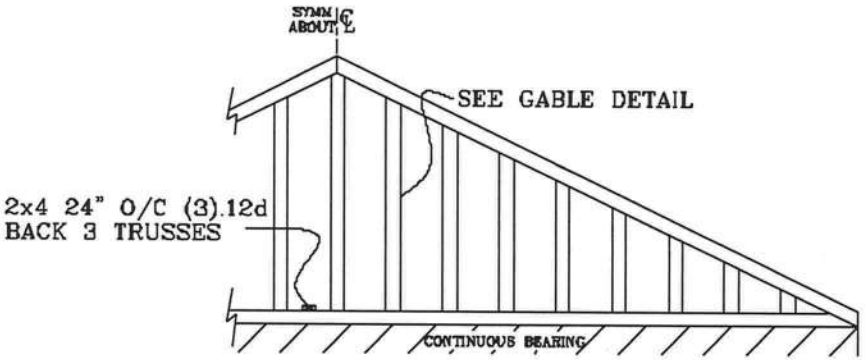
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 90' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

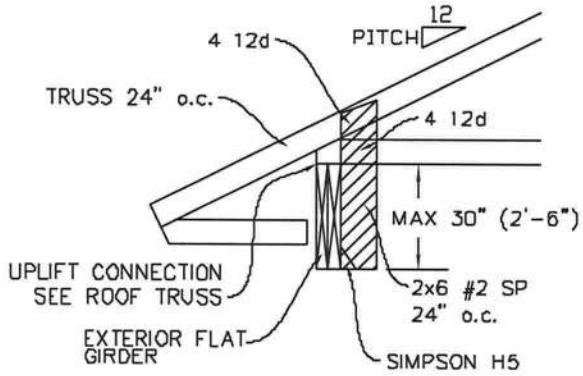


GABLE END TRUSS DETAIL

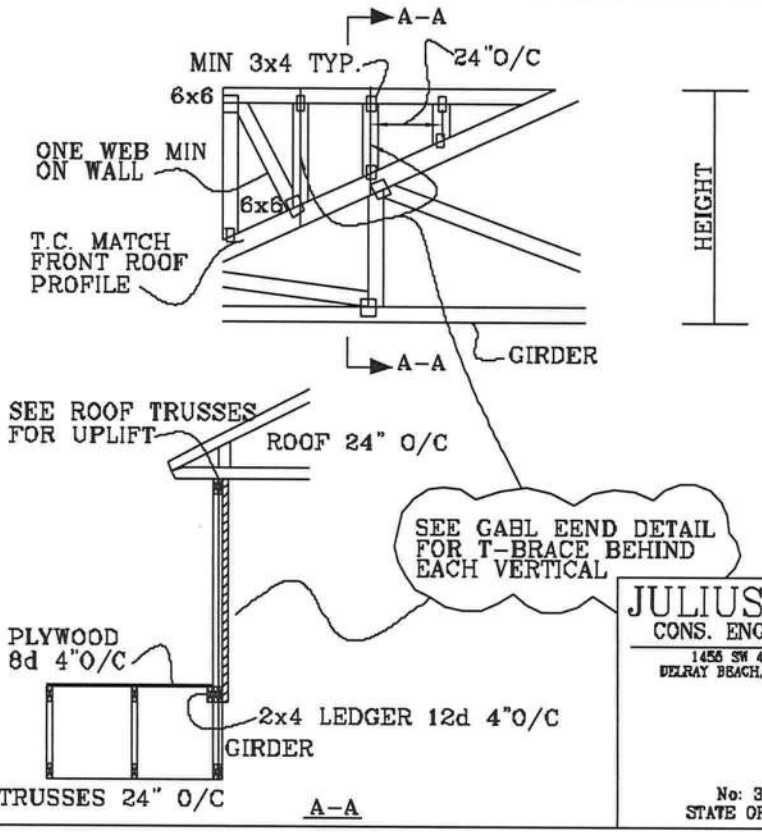


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

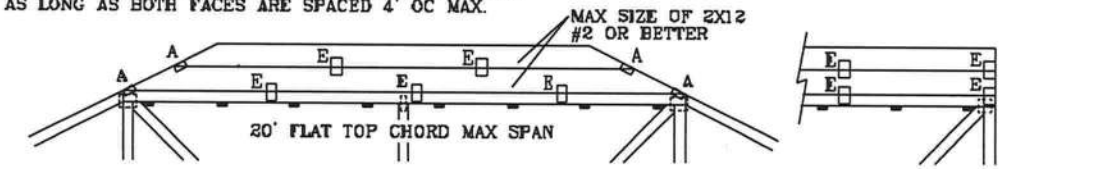
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

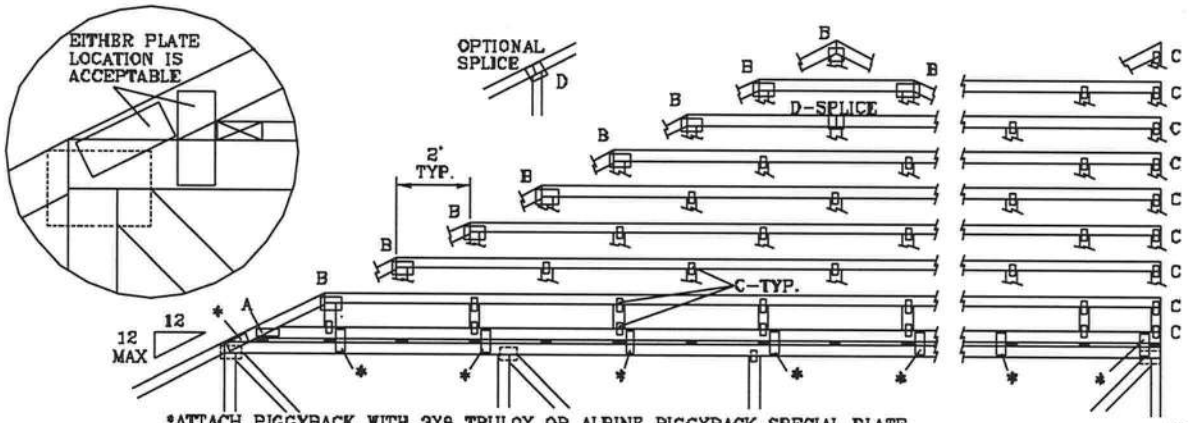
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

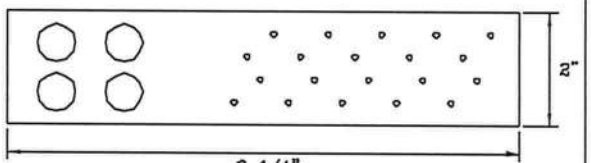
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 537199 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

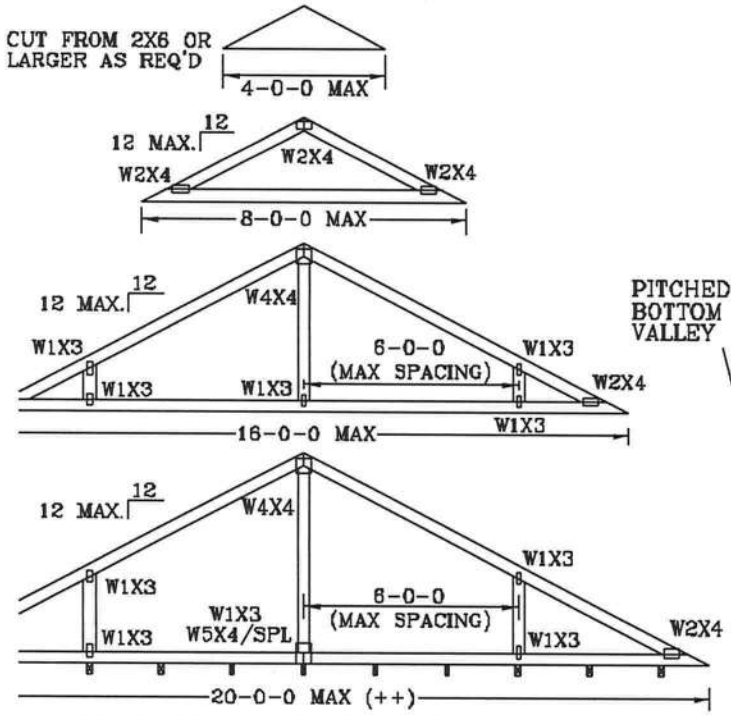
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

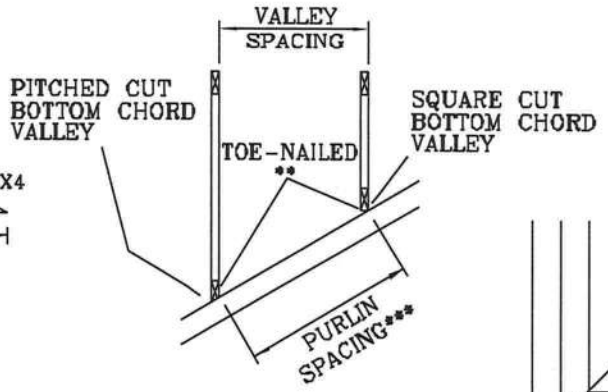
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

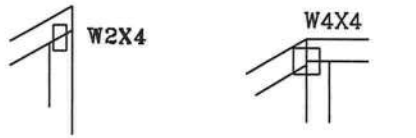
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

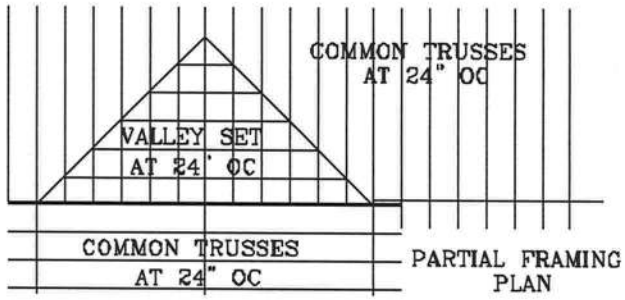
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

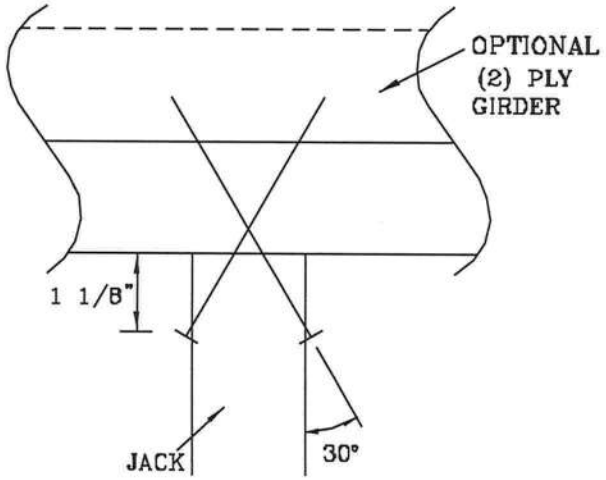
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

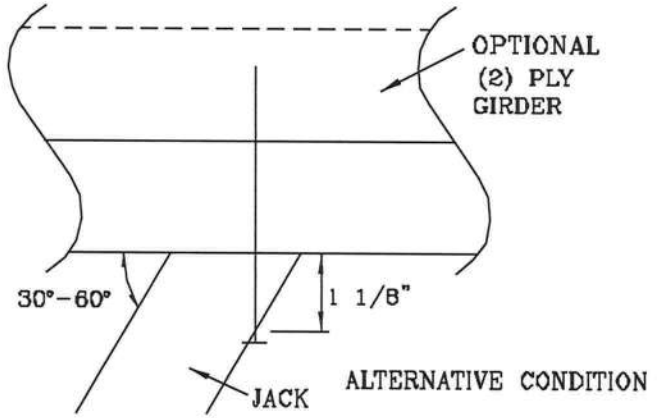
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

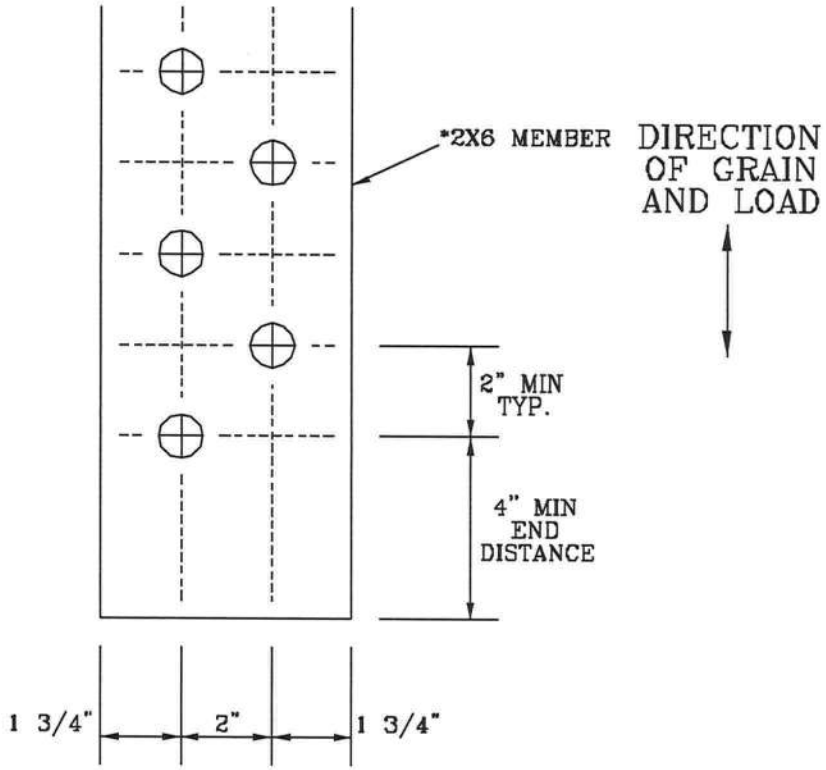
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

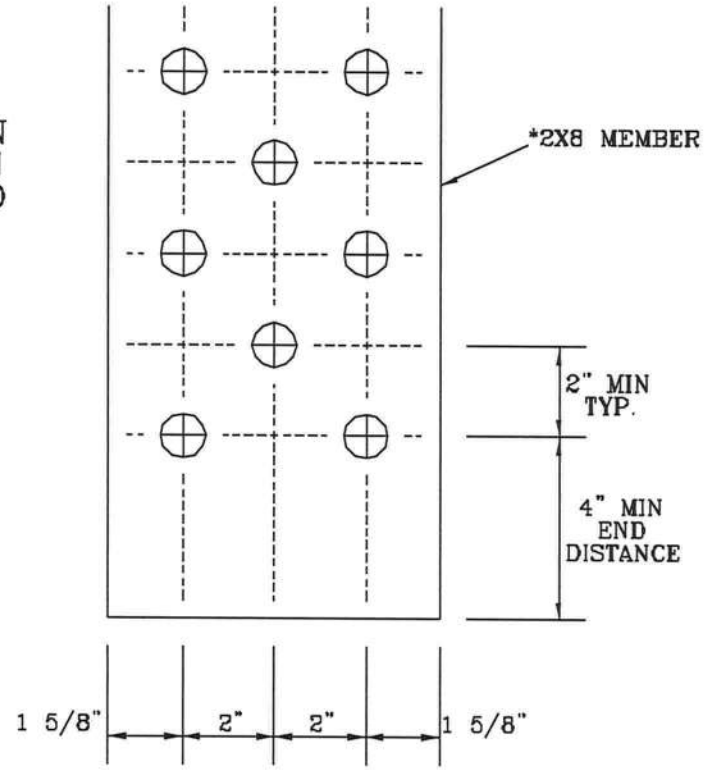
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

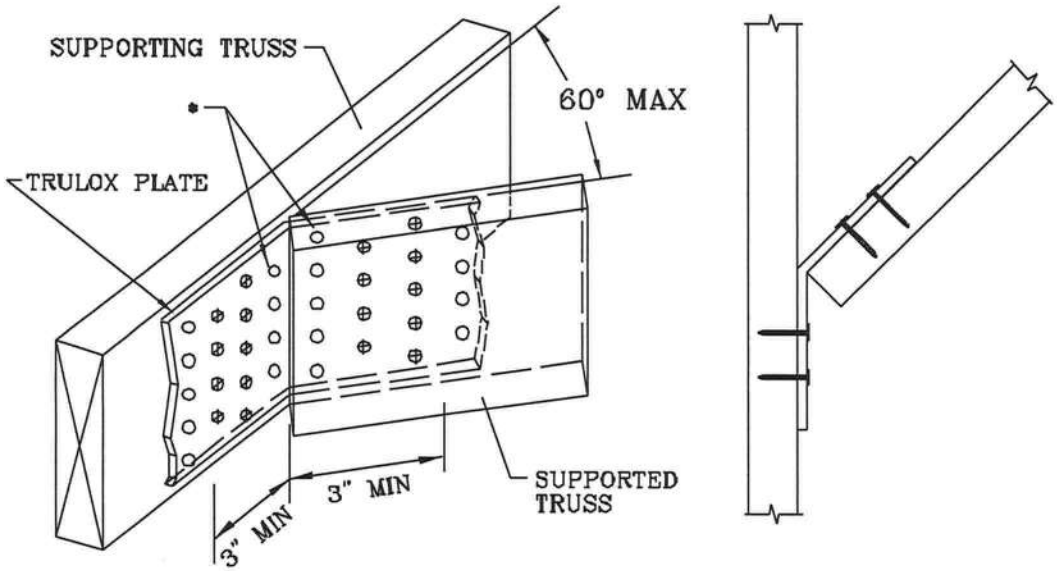
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

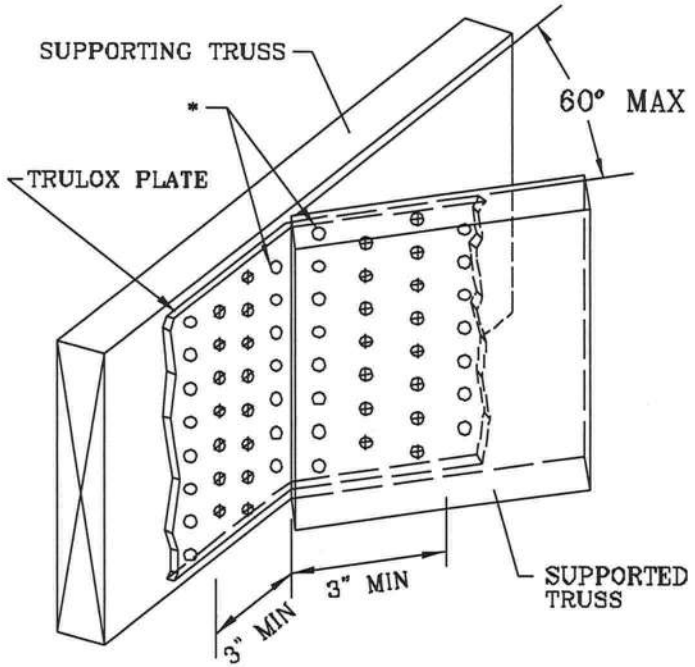
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#



MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

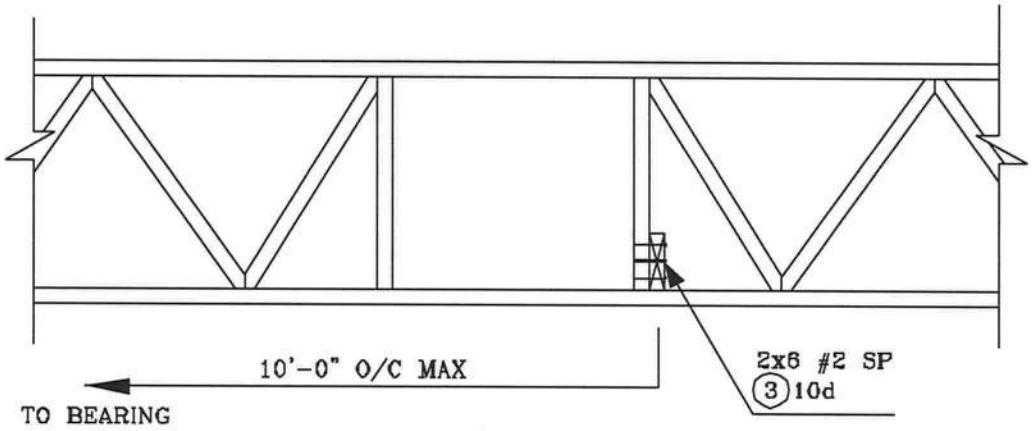
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2351

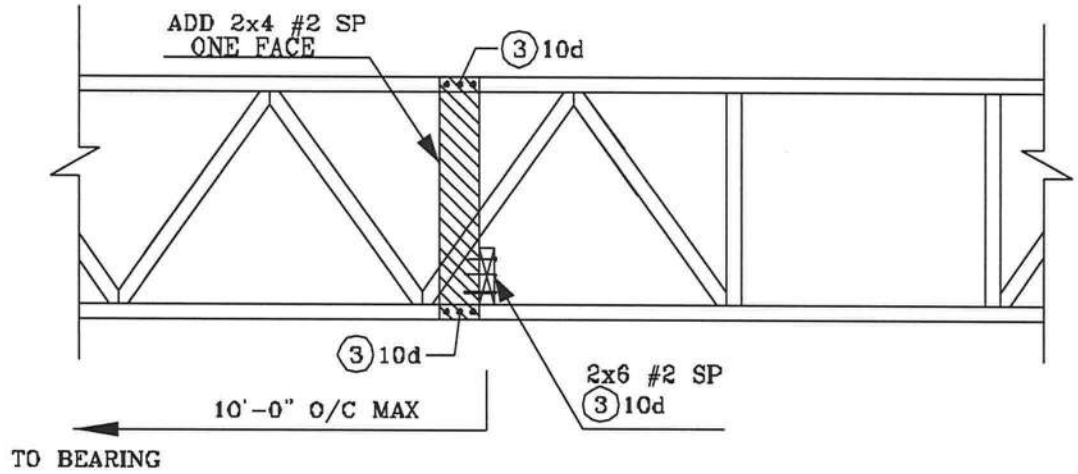
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS

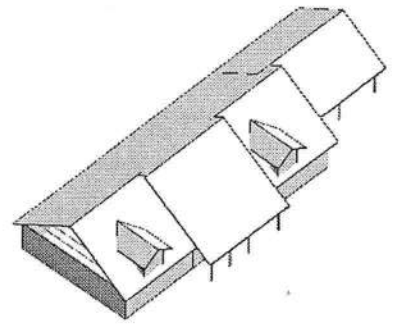


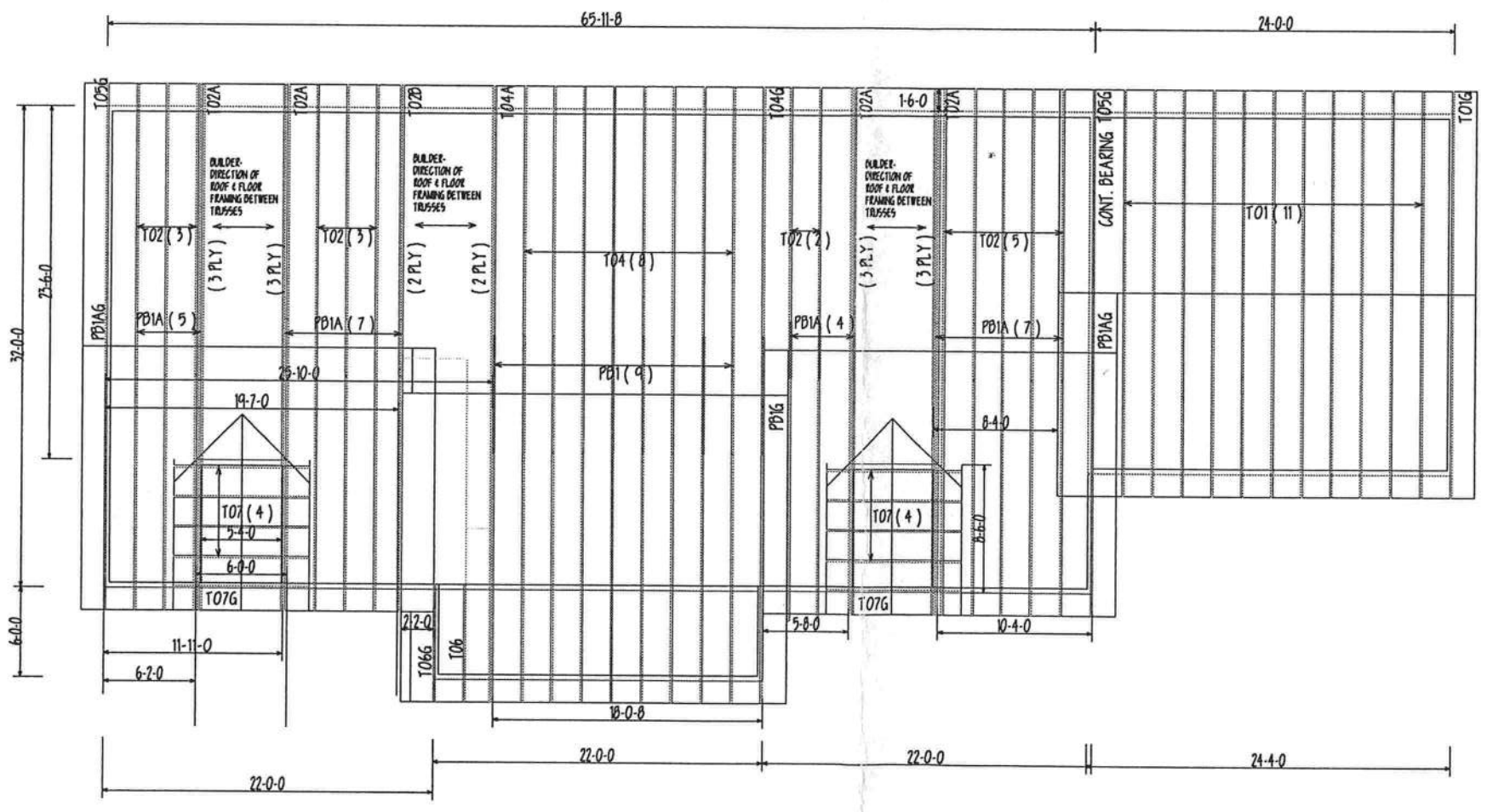
ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA





8/12
PITCH
18"
O.H.

BEARING HEIGHT SCHEDULE

	8'0"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: OWNER BUILDER		
LEGAL ADDRESS: MCCRAY RESIDENCE		
MODEL: CUSTOM		REVISION: 1-14-08
		SCALE: NTS
DATE: 5/9/06	DRAWN BY: JOE	JOB #: L162006

TABLE 6B-1 **MINIMUM REQUIREMENTS** **Climate Zones 1 2 3**

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED
		A	B	C	D	E	
GLASS	Max.%of glass to Floor Area	15%	15%	20%	20%	25%	7 %
	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☑ DT: ☐
	Overhang	1'4"	2'	2'	2'	2'	____ FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ____ ADJ: R = ____ COM: R = ____
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD FRAME WALLS R-11					EXT: R = 13 ADJ: R = ____ COM: R = ____
CEILINGS		R-30	R-30	R-30	R-30	R-30	UNDER ATTIC: R = 22 COMMON: R = ____
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)					
FLOORS	Slab-On-Grade	R-0					R = 0
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R = ____
	Raised Concrete	R-7					R = ____
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R = 6 COND. ☑
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	SEER = 16
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	COP= 7.5
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					AFUE = ____
HOT WATER SYSTEM	Electric Resistance**	EF .88	NOT ALLOWED (SEE BELOW)	EF .91	NOT ALLOWED (SEE BELOW)	EF .91	EF = .88
	Gas & Oil **	MINIMUM EF OF .54				NATURAL GAS ONLY (SEE BELOW)	EF = ____
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☑ EF= ____ HRU: ☐ SOLAR: ☐ EF= ____

* Single package units minimum SEER=9.7, HSPF = 6.6.

** Minimum efficiencies for gas and electric hot water systems apply to to 40 gallon water heaters. Refer to Table 6-12 for minimum Code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multi-story house.

Wall, Ceiling and Floor Insulation Values: The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multifamily building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a State-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTAC's equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed", an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options". See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .88 or greater, or natural gas systems with EF .54 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	✓
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-01 — **Residential Component Prescriptive Method B** — **NORTH 1 2 3**

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME: McCray	**BUILDER:** Owner		
AND ADDRESS: 712 SE Racetrack Rd Lake City FL 32025	**PERMITTING OFFICE:** Columbia Co.	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☐	
OWNER: Mr. & Mrs Mike McCray	**PERMIT NO.:** ☐☐☐☐☐☐☐☐	**JURISDICTION NO.:** ☐☐☐☐☐☐	

GENERAL DIRECTIONS

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" fromTable 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-F)**	1. A	
2. **New construction or addition**	2. NCW	
3. **Single family detached or Multifamily attached**	3. Sgl Fam.	
4. **If Multifamily—No. of units covered by this submission**	4.	
5. **Is this a worst case?** (yes / no)	5. no	
6. **Conditioned floor area** (sq. ft.)	6. 2112	
7. **Predominant eave overhang** (ft.)	7. 2'	
8. **Glass type and area :**	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. / 163 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. / ____ sq. ft.	
9. **Percentage of glass to floor area**	9. 7 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab on grade (R-value)	10a. R= 0 ____ lin. ft.	
b. Wood, raised (R-value)	10b. R= ____ ____ sq. ft.	
c. Wood, common (R-value)	10c. R= ____ ____ sq. ft.	
d. Concrete, raised (R-value)	10d. R= ____ ____ sq. ft.	
e. Concrete, common (R-value)	10e. R= ____ ____ sq. ft.	
11. **Wall type, area and insulation:**		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R= ____ ____ sq. ft.	
2. Wood frame (Insulation R-value)	11a-2 R= 13 (60) sq. ft.	
b. Adjacent: 1. Masonry (Insulation R-value)	11b-1 R= ____ ____ sq. ft.	
2. Wood frame (Insulation R-value)	11b-2 R= ____ ____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation R-value)	12a. R= 22 2534 sq. ft.	
b. Single assembly (Insulation R-value)	12b. R= ____ ____ sq. ft.	
13. **Air Distribution System: Duct insulation, location**	13. R= 6 Attic	
Test report (attach if required)		
14. **Cooling system**	14a. Type: Central	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 16	
	14c. Capacity: 35 kBtu	
15. **Heating system:**	15a. Type: Heat Pump	
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE:	
	15c. Capacity: 41331	
16. **Hot water system:**	16a. Type: Elect.	
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: .88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____ **DATE:** 1-25-08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER AGENT: _____ **DATE:** _____

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____