

WOOD FRAME

OF PLATE JOINT

ONE STORY WOOD FRAME

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE RUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLE

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

EBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT. WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL **ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCF IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE T REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL RACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, **EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS** FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

TO 17'-3" STUD HEIGHT

(END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.) TO 10'-1" STUD HEIGHT ΓΟ 11'-2" STUD HEIGHT TO 15'-7" STUD HEIGHT

GRADE & SPECIES TABLE					
		Fb	Е		
2x8	SP #2	925	1.4		
2x10	SP #2	800	1.4		
2x12	SP #2	750	1.4		
GLB	24F-V3 SP	2600	1.9		
LSL	TIMBERSTRAND	1700	1.7		

16x7 GARAGE DOOR

,	24F-V3 3F	2000	1.9	
-	TIMBERSTRAND	1700	1.7	
	MICROLAM	2950	2.0	
-	PARALAM	2900	2.0	
DESIGN CRITERIA & LOADS:				
ВІ	JILDING CODE			8TH EDITION
				ELORIDA BUILDING CC

LDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
DE FOR DESIGN LOADS	ASCE 7-22
NDLOADS	
SIC WIND SPEED CE 7-22, 3S GUST)	130 MPH
D EXPOSURE ILDER MUST FIELD VERIFY)	С
POGRAPHIC FACTOR ILDER MUST FIELD VERIFY)	I
K CATEGORY	II
CLOSURE CLASSIFICATION	ENCLOSED
ERNAL PRESSURE EFFICIENT	0.18
OF ANGLE	7-45 DEGREES
AN ROOF HEIGHT	30 FT
C DESIGN PRESSURES	SEE TABLE
OOR LOADING	
DMS OTHER THAN EPING ROOM	40 PSF LIVE LOAD
EPING ROOMS	30 PSF LIVE LOAD
OF LOADING	
T OR < 4:12	20 PSF LIVE LOAD
! TO < 12:12	16 PSF LIVE LOAD
2 & GREATER	12 PSF LIVE LOAD
IL BEARING CAPACITY	1500 PSF
OOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE
	·

RING CAPACITY	1500 PSF	Mark Disosway P.E.
NE	THIS BUILDING IS NOT IN THE FLOOD ZONE	163 SW Midtown Place
		Suite 103 Lake City, Florida 32025

END 4' FROM ALL

+25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd)

+42.6(Vult) -46.2(Vult) +42.6(Vult) -57(Vult)

+22.6(Vasd) -25.5(Vasd

+21.7(Vasd) -24.1(Vasd)

386.754.5419 JOB NUMBER:

> 240444 **S-1** OF 4 SHEETS

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023)

LIMITATION: This design is valid for one building, at specified location.

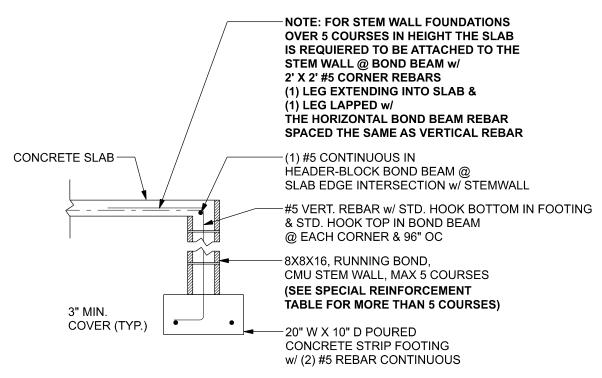
Lake City, Florida 32025 disoswaydesign@gmail.com

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

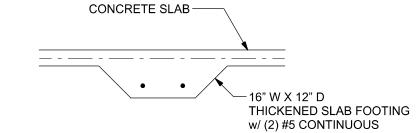
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT
SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION
FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602).
THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE
PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS
BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

THE E	THE ENGINEER IN WRITING.				
	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approval			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.			

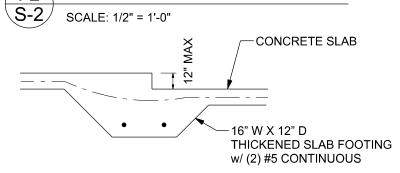


F1 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"

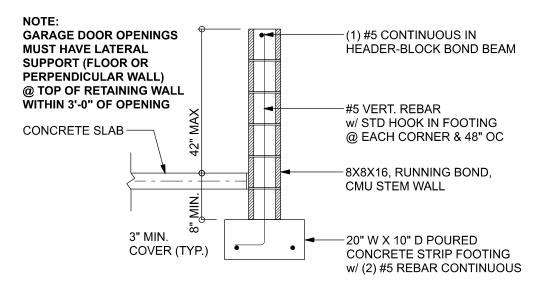
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



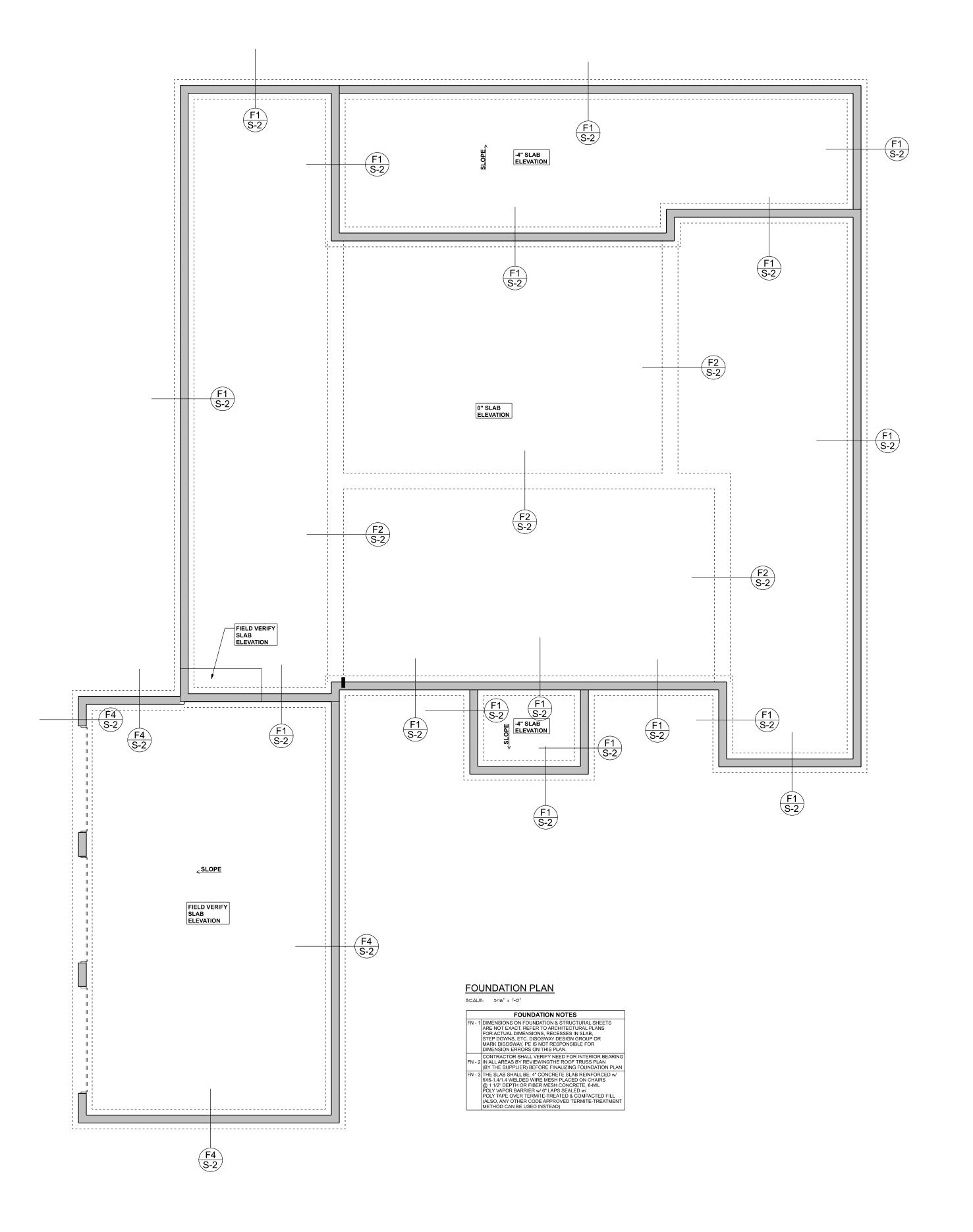
F2 INTERIOR BEARING FOOTING



F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 STEM WALL CURB FOOTING
S-2 SCALE: 1/2" = 1'-0"



Stanley & Beverly Pope Res.

PROJECT ADDRESS:
649 NW Noegel Rd., Lake City, FL 32055

FL PE 53915

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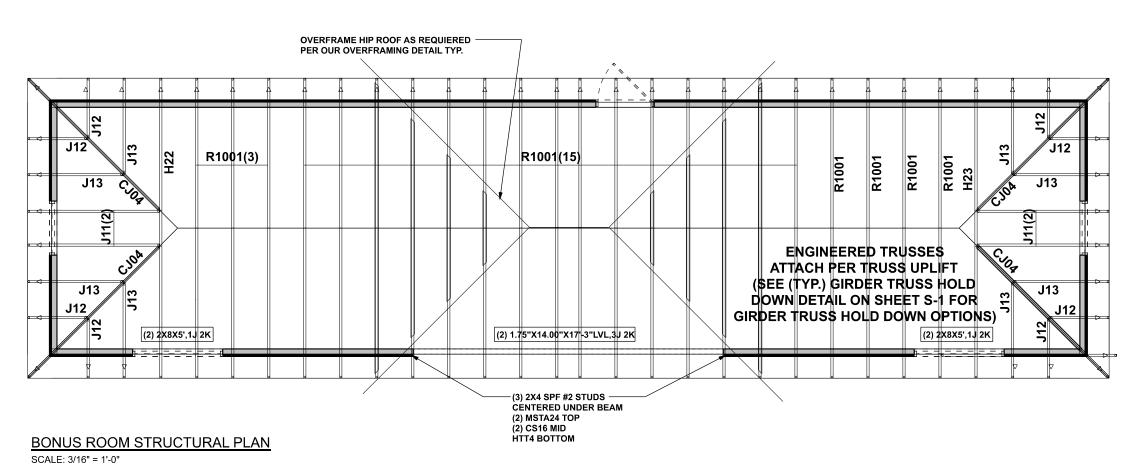
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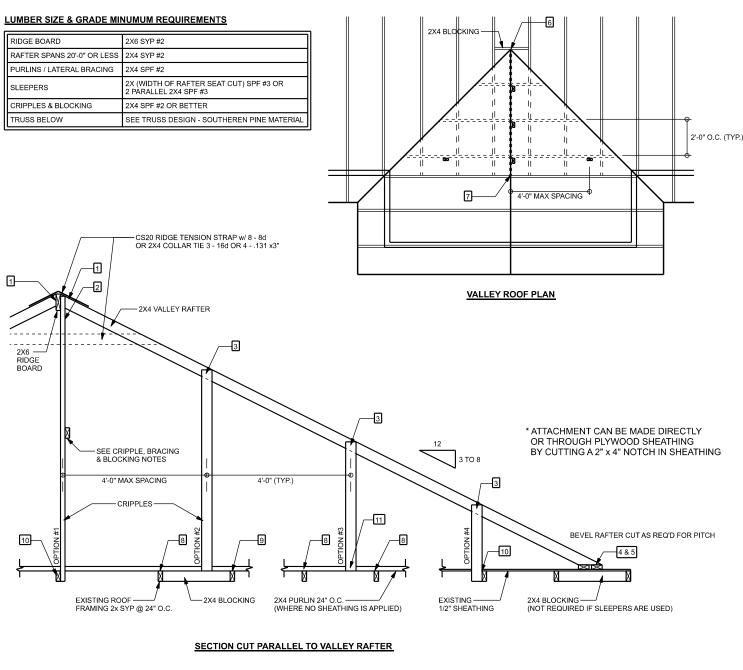
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JOB NUMBER: 240444 **S-2**OF 4 SHEETS





ROOF OVER FRAMING & BRACING DETAIL

VALLEY ROOF PLAN MEMBER LEGEND TRUSS = = = TRUSS UNDER VALLEY FRAMING

:===:: VALLEY RAFTER OR RIDGE CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

<u>NECTION REQUIREMENT NOT</u>	E٤
2V4 DAETEDS TO DIDGE	1

1	2X4 RAFTERS TO RIDGE	3 -16d OR 6131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6131 x 3" FACE NAILS

GENERAL NOTES

GENERAL NOTES

MAXIMUM RAFTER SPANS
6"-0" FOR 2X4, 9"-0" FOR 2X6 SPF #2 OR SYP #2.

MAXIMUM ROOF AREA PER SUPPORT
16ft2 IN ZONES 2 & 3, 2 4ft2 IN ZONE 1. (EXAMPLE: 4"-0" O.C. X 4"-0" SPAN = 16ft2 OR 2"-0" X 8"-0" SPAN = 16ft2)

PURLINS REQUIRED 2"-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM
OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
-SPANS (DISTANCS BETWEEN HEELS) 40"-0" OR LESS
-MAXIMUM WIND SPEED: 130 MPH
-MAXIMUM WIND SPEED: 130 MPH
-MAXIMUM MEAN ROOF HEIGHT: 30 FEET
-MAXIMUM MEAN ROOF HEIGHT: 30 FEET
-MAXIMUM MEAN ROOF HEIGHT: 30 FEET
-MAXIMUM TOTAL LOADING: 40 psf
-MEETS FBC / ASCE 7 WIND REQUIREMENTS
-EXPOSURE CATEGORY "C", I = 1.0, Kzt = 1.0

CRIEDIE E BRACING & BLOCKING NOTES

CRIPPLE, BRACING, & BLOCKING NOTES -2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W 2 - 104 NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10-0" LONG REQUIRE TWO CLB's OR BOTH FACES W "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.

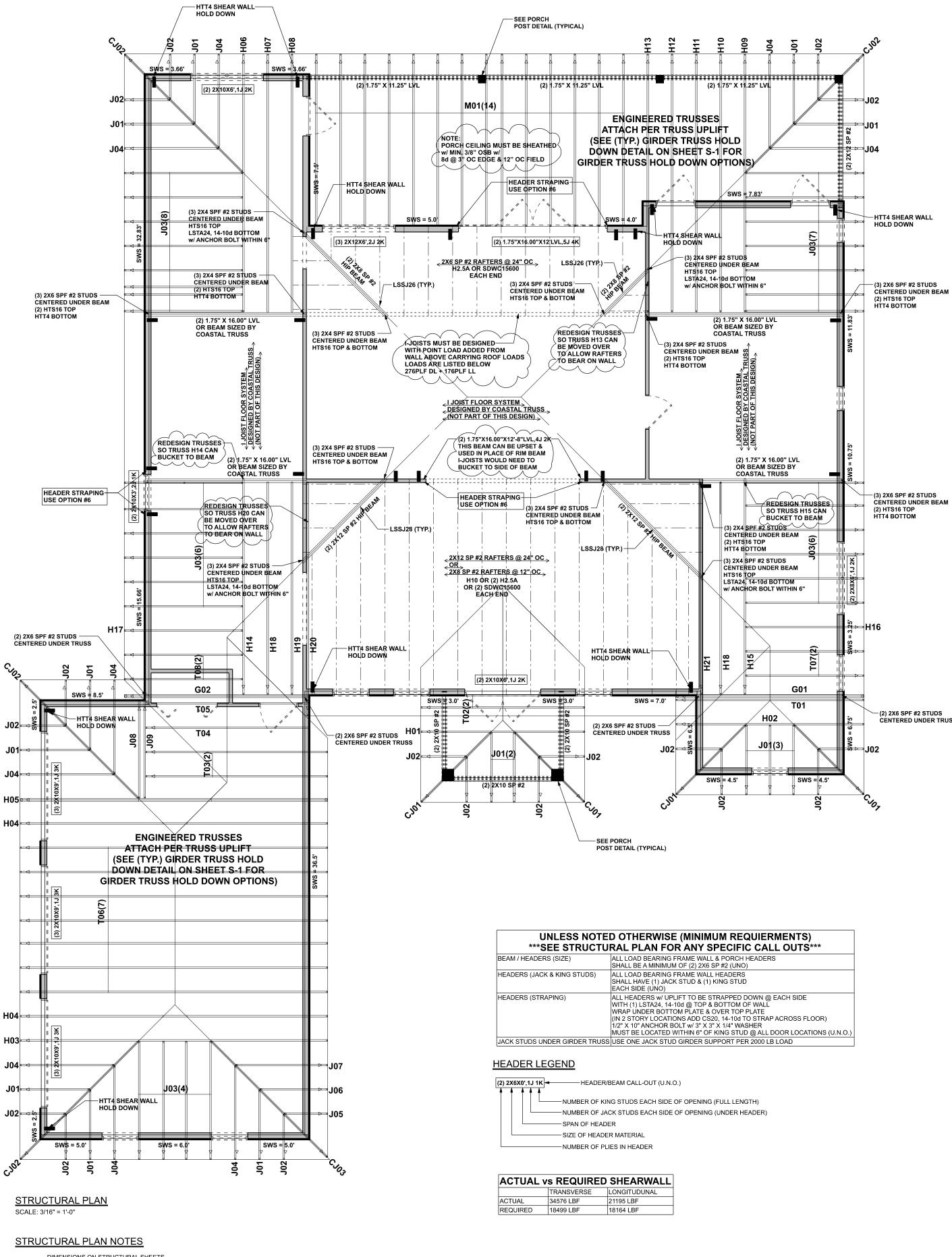
INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.

- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03. SN-2 LATERAL BRACING IS TO BE RESTRAINED. ETC. SOCI. 53, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



10C0000017E97 Mark Disosway Digital Signature DE07CA000746F [0, CN=Mark d Disosway PRINTED SIGNATURE IS NOT VALID CLICK HERE TO VALIDATE DISSIS 2024-05-02 13: 32:05 DIMENSIONS: Stated dimensions supercede scaled

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LIMITATION: This design is valid for one building, at specified location.

> Mark Disosway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

JOB NUMBER: CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING 240444 FURNISHED BY BUILDER. COASTAL TRUSS & VINYL SIDING

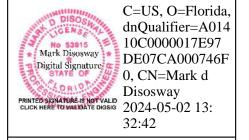
JOB #240124-03KM

S-3

OF 4 SHEETS

RAFTERS SHALL BE IN ACCORDANCE WITH RAFTER SPAN TABLES. RAFTER OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/3 RAFTER SPAN OR 24" UNO. RAKE OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/2 PURLIN LENGTH OR 24". RAKE OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/2 PURLIN LENGTH OR 24". RAKE OVERHANGS SHALL NOT EXCEED THE LESSER OF 1/2 PURLIN LENGTH OR 24". RAFTERS AND CEILING JOISTS SHALL BEARING DIRECTLY ON BEAMS, GIRDERS, LEDGERS OR BEARING WALLS OR BE SUPPORTED BY JOIST HANGERS. MINIMUM BEARING LENGTH IS 1 1/2" ON WOOD OR METAL AND 3" ON MASONRY OR CONCRETE. RAFTERS AND CEILING JOISTS HAVING A DEPTH-TO-THICKNESS RATIO EXCEEDING 5:1 SHALL HAVE LATERAL SUPPORT AT POINTS OF BEARING TO PREVENT ROTATION. RIDGE, HIP, AND VALLEY BEAMS SHALL BE INSTALLED PER FRAMING PLANS, WITH RAFTER BEARING ON BEAM OR HANGERS. CEILING JOISTS OR RAFTER TIES ARE NOT REQUIRED WHEN RIDGE BEAM IS PROVIDED. RIDGE BOARDS ARE BE PERMITTED PROVIDED. WHEN RIDGE BEAM IS SHALL BE INSTALLED PER FRAMING PLAOS SYSTEM. WHERE THE ROOF PITCH IS LESS THAN 3:12, STRUCTURAL MEMBERS THAT SUPPORT RAFTERS & CEILING JOISTS (RIDGES, HIPS, & VALLEYS) SHALL BE DESIGNED AS BEAMS. COLLAR TIES OR RIDGE TENSION STRAPS TO RESIST WIND UPLIFT SHALL BE PROVIDED. A CONTINUOUS LOAD PATH SHALL BE PROVIDED TO TRANSMIT THE UPLIFT FORCES FROM RAFTER/RIVES TIES TO THE FOUNDATION. CEILING JOISTS MUST BE NAILED TOGETHER OR STRAPPED TOGETHER TO RESIST LOAD. CATHEDRAL CEILING SWITHOUT CEILING JOIST SPAN TABLE. WHEN CEILING JOISTS MUST BE NOT SECONDE RESISTANCE TO RAFTER THRUST, LAPPED JOISTS MUST BE NAILED TOGETHER OR STRAPPED TOGETHER TO RESIST LOAD. CATHEDRAL CEILING SWITHOUT CEILING JOISTS OR RAFTER TIES SHALL HAVE RAFTERS BEARNING AT EACH SIDG OF OPPENING. OPPENINGS IN ROOF AND CEILING FRAMING SHALL BE FRAMED WITH HEADER & JOISTS / RAFTERS AT EACH SIDG OF OPPENING. OPPENINGS MORE THAN 4 FT. WIDE: HEADER TO BE SIZED BASED ON LOAD SUPPORTED AND BE ATTACHED AT A FT. WI	RAFTER SPANS - SOUTHERN PINE #2 - ROOF LIVE LOAD = 20 PSF	CEILING JOISTS SPANS FOR SOUTHERN PINE #2
ROOF SYSTEM DESIGN NOTE CONVENTIONALLY FRAMED ROOF	RAFTER SPAN TABLE (SP#2) BASED ON WFCM TABLES 3.62A-L & FOOTNOTES	CEILING JOIST SPAN TABLE (SP#2) BASED ON WFCM TABLES 3.25
16' 112 151 193 224 238 278 20 16 20 24' 148 200 256 318 278 318 20 24' 24 28' 36' 202 24' 249 321 353 388 388 388 32' 388 32' 388 388 32' 388 32' 388 32' 388 32' 388 388 388 388 32' 388 <td>ED 150 MPH 160 MPH 170 MPH REQUIRED CAPACITY OF CONNECTION</td> <td>ROOF SHEATHING SEE ROOF SHEATHING & ATTACHMENT DETAIL 2x_ SP#2 RAFTER @ 24" OC MAX. 2x_ CEILING JOIST @ 24" OC MAX. ATTACHED TO RAFTER AS PER HEEL JOINT CONNECTION DETAIL BLOCKING BETWEEN RAFTER AT BEARING TO PREVENT ROTATION (NO REQUIRED IF DEPTH-TO- THICKNESS RATIO IS 5:1 OR LESS) TOE-NAIL RAFTER TO TOP PLATE PER TABLE BELOW (FOR SHEAR LOADS) SELECT CONNECTOR TABLE PER UPLIFT LOADS (SEE RAFTER TO WALL CONNECTION TABLE) SOFFIT BY OWNERS # OF .131" x 2 1/2" TOE-NAILS REQUIRED AT RAFTER TO WALL (FOR SHEAR) DESIGN WIND SPEED (MPH)</td>	ED 150 MPH 160 MPH 170 MPH REQUIRED CAPACITY OF CONNECTION	ROOF SHEATHING SEE ROOF SHEATHING & ATTACHMENT DETAIL 2x_ SP#2 RAFTER @ 24" OC MAX. 2x_ CEILING JOIST @ 24" OC MAX. ATTACHED TO RAFTER AS PER HEEL JOINT CONNECTION DETAIL BLOCKING BETWEEN RAFTER AT BEARING TO PREVENT ROTATION (NO REQUIRED IF DEPTH-TO- THICKNESS RATIO IS 5:1 OR LESS) TOE-NAIL RAFTER TO TOP PLATE PER TABLE BELOW (FOR SHEAR LOADS) SELECT CONNECTOR TABLE PER UPLIFT LOADS (SEE RAFTER TO WALL CONNECTION TABLE) SOFFIT BY OWNERS # OF .131" x 2 1/2" TOE-NAILS REQUIRED AT RAFTER TO WALL (FOR SHEAR) DESIGN WIND SPEED (MPH)
U = CONNECTOR UPLIFT LOAD L = CONNECTOR LATERAL LOAD (PERPENDICULAR TO THE WALL) S = CONNECTOR SHEAR LOAD (PARALLEL TO THE WALL) R = L/W FOR WIND PERPENDICULAR TO THE RIDGE AND W/L FOR WIND PARALLEL TO THE RIDGE AND LATERAL LOADS CAN BE MULTIPLIED BY 0.75 (UPLIFT) AND 0.92 (LATERAL) FOR	FRAMING NOT LOCATED WITHIN 8' OF BUILDING CORNERS.	RIDGE HEIGHT RAFTER SPACING = 16" OC RAFTER SPACING = 24" OC 10' 4 4 4 4 4 4 4 5 5 5 5 6 6 15' 4 4 4 4 4 4 5 5 5 5
- UPLIFT LOADS ASSUME A REDUCED ROOF / CEILING ASSEMBLY DEAD LOAD OF 9 psf (0.6 x 1: FOR WALL OR FOUNDATION CONNECTIONS, UPLIFT VALUES CAN BE REDUCED BY 73 plf (0.6 - FOR JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER (HIP) UPLIFT CONNECTIONS (HIP) UPLIFT (HIP)	6 x 121 plf) FOR EACH WALL ABOVE.	20' 5 5 5 5 6 6 - - - - - - - -
RAFTER TO WALL CONNECTION BASED ON WFCM TABLE 3.4 - EXPOSURE C		RAFTER EAVE CONNECTION DETAIL CONVENTIONALLY FRAMED ROOF
RIDGE TENSION STRAP (SEE TABLE) RIDGE BOARD / HIP BOARD, MIN. WIDTH OF 1" AND DEPTH NOT TO BE LESS THAN THE CUT END OF THE RAFTER (DROP TO PROVIDE ROOF VENTILATION IF REQUIRED) RIDGE BOARD / HIP BOARD, CAN BE USED CEILING JOIST OR RAFTER TIES, OF FLOOR IS USED TO PREVENT REPORT TO PROVIDE ROOF VENTILATION IF REQUIRED) RIDGE BOARD / HIP BOARD RIDGE BEAM / HIP BEAM (SEE ROOF FRAMING PLAN) RIDGE BEAM / HIP BEAM (SEE ROOF FRAMING PLAN) DROP TO PROVIDE ROOF VENTILATION IF REQUIRED RIDGE BEAM / HIP BEAM MUST BE USED WHEN RAFTER NEED TO BE SUPPORTED AT EACH END (NO CEILING JOISTS OR TIES) TO RIDGE BEAM / HIP BEAM ATTACHMENT TO RIDGE BEAM PER RAFTER SPAN: UPTO 4" SPAN = (4) .131" x 3.25" TOE-NAILS UPTO 8" SPAN = LSSJ26LZ -RZ (ADJUSTABLE SLOPE/SKEW HANGER) UPTO 12" SPAN = LSSJ28LZ -RZ (ADJUSTABLE SLOPE/SKEW HANGER) UPTO 19" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 21" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER) UPTO 26" SPAN = LRU26Z (FACE MOUNT RAFTER HANGER)	ROOF PITCH = 3:12 - 5:12 ROOF PITCH = 5:12 - 12:12 ROOF PITCH = 5:12 - 12:12 ROOF PITCH = 6:12 - 12:12 ROOF PITCH = 6:13 - 13:13 3:13	RAFTER / CEILING JOIST HEEL JOINT CONNECTION 20 psf ROOF LIVE LOAD 30 psf GROUND SNOW LOAD
CONVENTIONALLY FRAMED ROOF	BASED ON WFCM, TABLE 3.6, EXP C	BASED ON WFCM TABLE 3.9 — SP#2 RAFTERS @ 24" OC MAX
		SP#2 RAFTERS @ 24" OC MAX SEE ROOF FRAMING PLAN IF RAFTER IS SPLICED AT PURLIN LAP W(5). 131" x 3.25" NAILS IN LAP OR SCAB ON ONE SIDE W(5). 131" x 3.25" NAILS IN EACH END DO NOT EXCEED 30 deg. OFF VERTICAL WITHOUT AN OPPOSING BRACE H2.5A RAFTER TO PURLIN VICE RAFTER TO PURLIN LSTA24 OR CS20 WRAP OVER TOP OF PURLIN NAILED INTO KICKER AT EACH END W (6). 131" x 2.5" NAILS RATRUN TO KICKER WHEN OPPOSING BRACE IS USED LAP AT BEARING W(5). 131" x 3.25" NAILS INTERIOR BEARING WALL/HEADER SEE STRUCTRAL LAYOUT
		TYPICAL KICKER (RAFTER BRACE) SECTION CONVENTIONALLY FRAMED ROOF

FL PE 53915 This item has been digitally signed and sealed by Mark Disosway PE on digital signature date.
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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> JOB NUMBER: 240444

S-3.1 OF 4 SHEETS