ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V1Y487-Z0109083116

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator	Anderson Truss Company
Job Identification	13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive, [illegible] 337 sw rose
Truss Count	29
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Com/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

12/09/2013

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-12030EC1-CNNAILSP-

#	Ref	Description	Drawing#	Date
1	31211	--A 20' Common	13343001	12/09/13
2	31212	--AGE 20' Gable	13343002	12/09/13
3	31213	--B 19'4" Common	13343003	12/09/13
4	31214	--BGE 19'4" Gable	13343004	12/09/13
5	31215	--C1 19' Common	13343005	12/09/13
6	31216	--C2 19' Common	13343006	12/09/13
7	31217	-C3 19' Common Gi	13343007	12/09/13
8	31218	--CGE 10' Gable	13343008	12/09/13
9	31219	--CGE1 19' Gable	13343009	12/09/13
10	31220	--CJ1 1' Jack	13343010	12/09/13
11	31221	--CJ3 3' Jack	13343011	12/09/13
12	31222	--CJ5 5' End Jack	13343012	12/09/13
13	31223	--D 31'8" Common	13343013	12/09/13
14	31224	-D1 20'4" Common	13343014	12/09/13
15	31225	--DGE 31'8" Gable	13343015	12/09/13
16	31226	-E1 27'6" Special	13343016	12/09/13
17	31227	--E2 28' Mono	13343017	12/09/13
18	31228	--E3 28' Mono	13343018	12/09/13
19	31229	--E4 28' Mono Hip	13343019	12/09/13
20	31230	--EJ7 7' End Jack	13343020	12/09/13
21	31231	-H11 27'6" Stepdo	13343021	12/09/13
22	31232	-H11A 28' Mono Hi	13343022	12/09/13
23	31233	-H13 27'6" Stepdo	13343023	12/09/13
24	31234	-H13A 28' Mono Hi	13343024	12/09/13
25	31235	-H7 27'4" Stepdow	13343025	12/09/13
26	31236	-H9 27'6" Stepdow	13343026	12/09/13
27	31237	-HJ7 9'10"13 Hip	13343027	12/09/13
28	31238	-MH7 28' Mono Hip	13343028	12/09/13
29	31239	-MH9 28' Mono Hip	13343029	12/09/13

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

COLUMBIA COUNTY BUILDING DEPARTMENT Received [illegible] PLANS

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - A 20 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

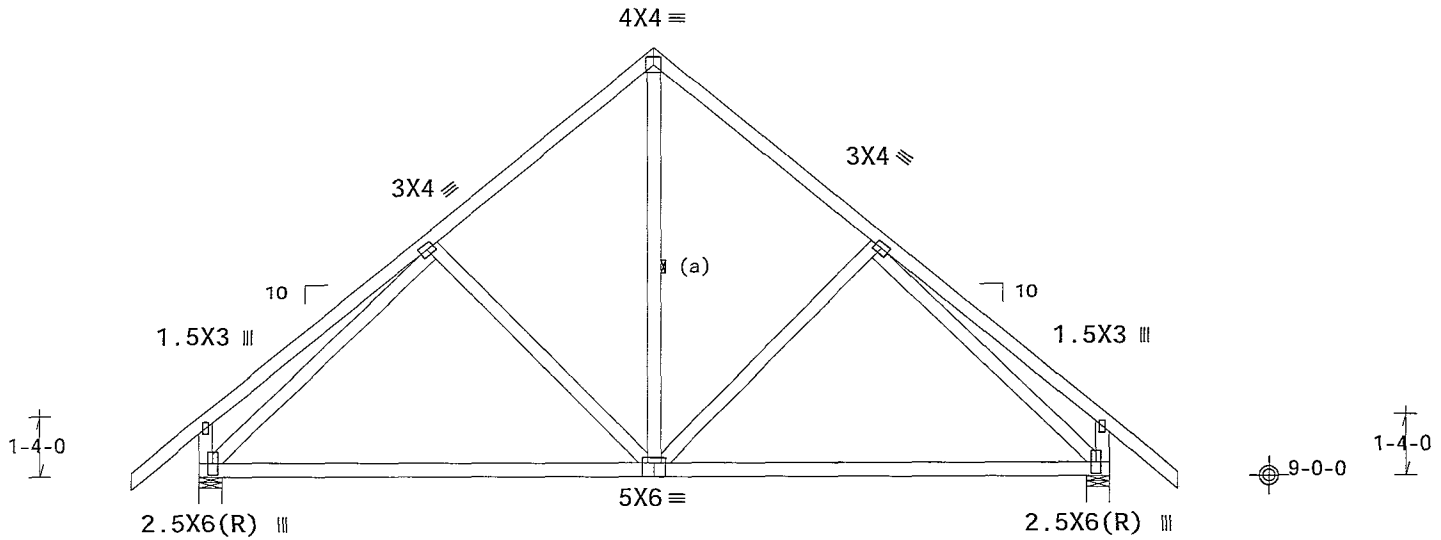
Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=877 U=23 W=6 (6 min)
RL=185/-185

R=877 U=23 W=6" (6' min)

Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:10 FL/-/4/-/-/R/- | Scale = 25"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabricat ng handling sh pp ng nstal ng and brac ng Re follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for pract ces prior to perform ng these funct ons Installers shall provide temporary bracing Unless noted otherw se top chord shall have properly attached structural sheathing and bot shall have a properly attached rig d ce l ng Locations shown for permanent lateral restr nt of webs shall have bracing nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons b e for any dev at on from th s des gn any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng shipp ng nstall on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the joint Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons draw ng or cover page list ng this draw ng indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitab l ty and use of th s des gn for any the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31211
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10 0 PSF	DRW	HCUSR9114 13343001
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	335792
DUR.FAC	1 25	FROM	JMW
SPACING	24 0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - AGE 20' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) 1x4 #3SRB SPF-S or better "L" brace 80% length of web member Attach with 8d Box or Gun (0 113"x2 5 ,min)nails @ 6" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

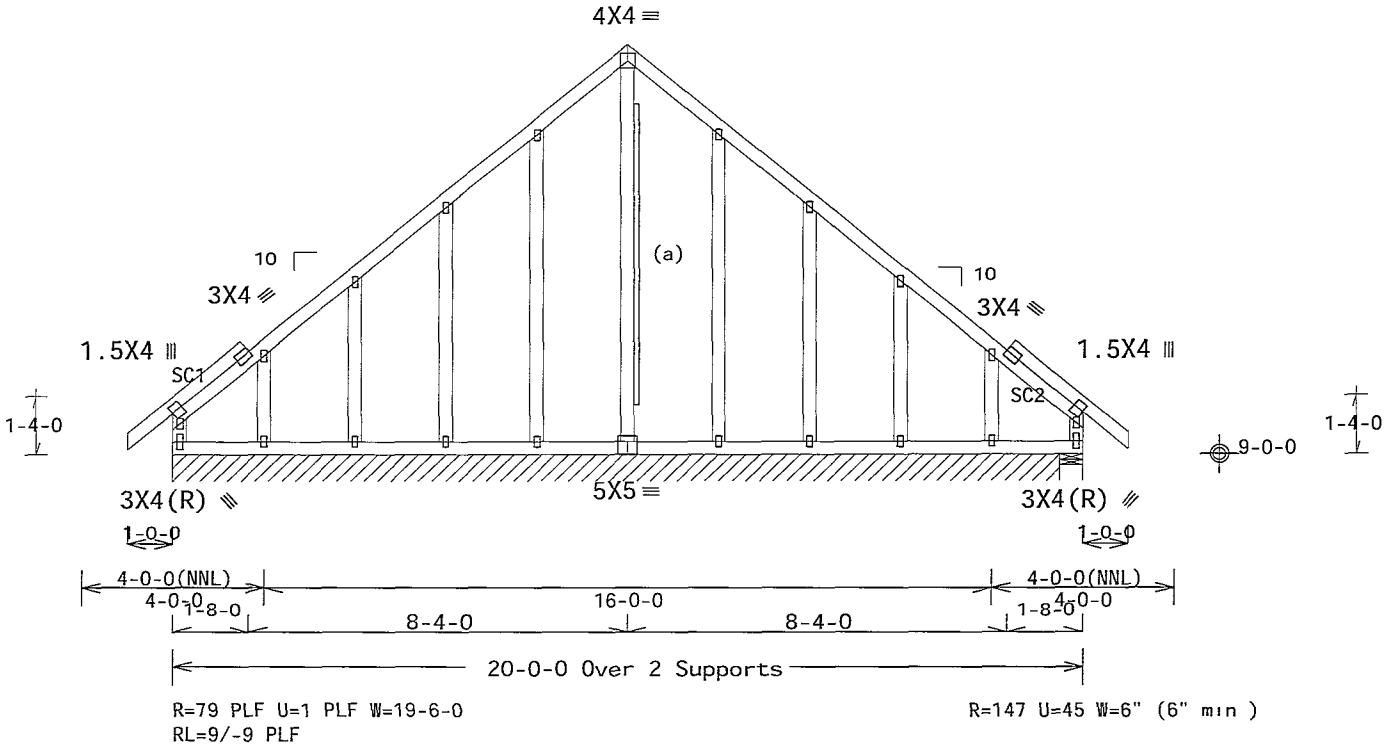
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Gable end supports 8" max rake overhang

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Fasten rated sheathing to one face of this frame



R=79 PLF U=1 PLF W=19-6-0
RL=9/-9 PLF

R=147 U=45 W=6" (6" min)

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326 13 QTY:1 FL/-/4/-/-/R/- Scale = 25"/Ft.

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Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31212
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343002
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	336034
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - B 19'4" Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

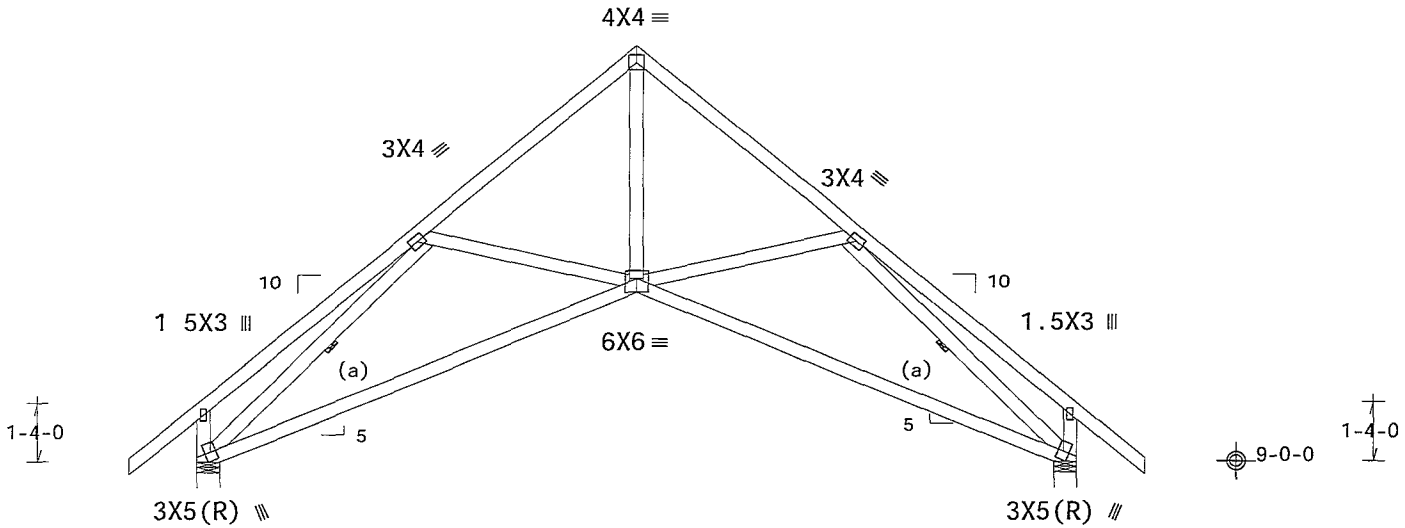
Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=867 U=18 W=6 (6 min)
RL=180/-180

R=867 U=18 W=6" (6' min)

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:10 FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstall ng and bracing Re fo ow the latest ed tion of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for practices prior to performing these functions Installers shall prov de temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locat ons shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

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12/09/2013

TC LL	20.0 PSF	REF	R9114- 31213
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	335782
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - BGE 19 4' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) 1x4 #3SRB SPF-S or better "L" brace 80% length of web member Attach with 8d Box or Gun (0 113"x2 5",min)nails @ 6" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

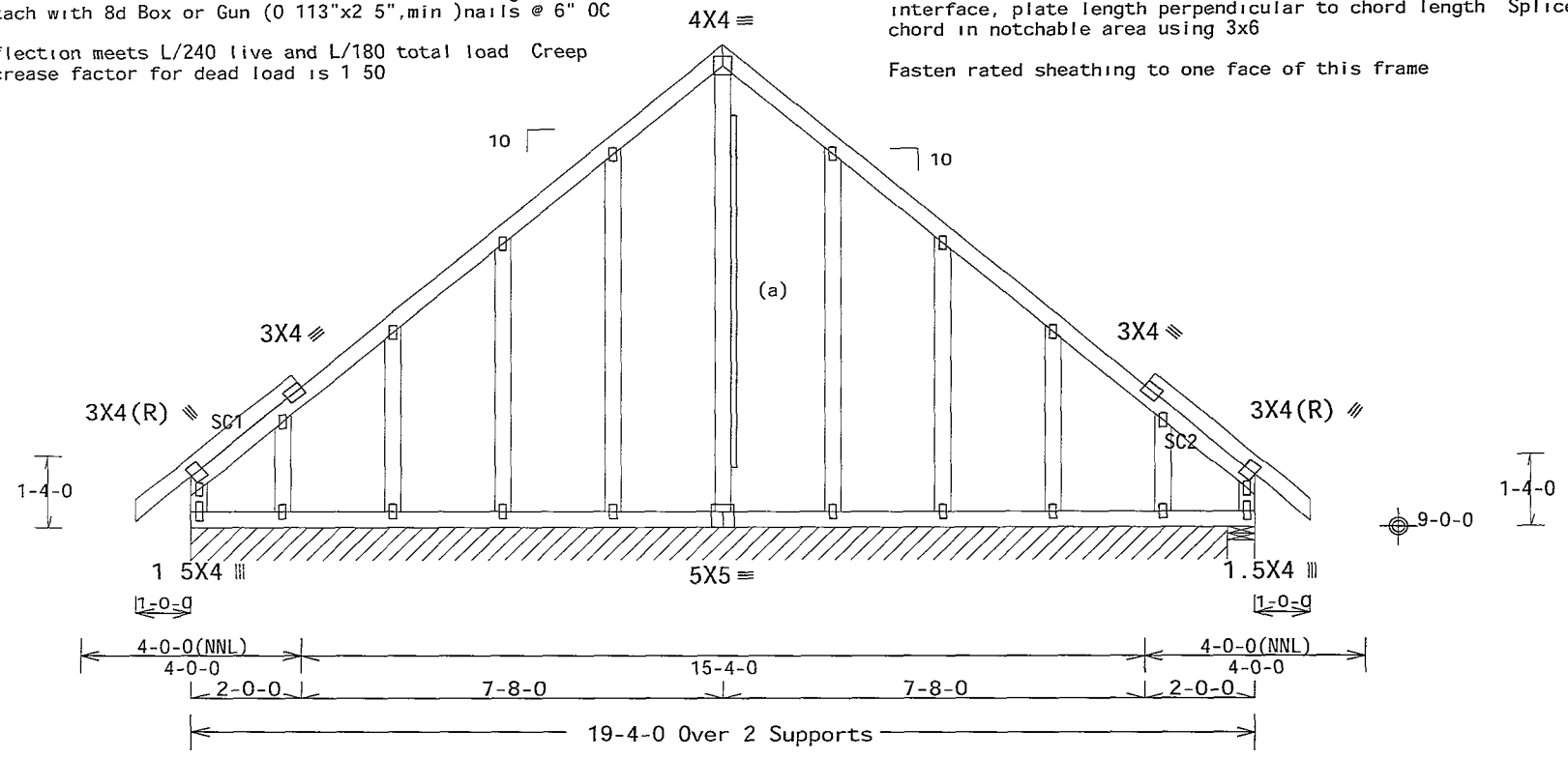
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Gable end supports 8" max rake overhang

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Fasten rated sheathing to one face of this frame



R=80 PLF U=1 PLF W=18-10-0
RL=9/-9 PLF

R=138 U=47 W=6" (6' min)

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 08 04 0326 13 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

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FL COA #0 278

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12/09/2013

TC LL	20.0 PSF	REF	R9114- 31214
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343004
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	336029
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - C1 19' Common)

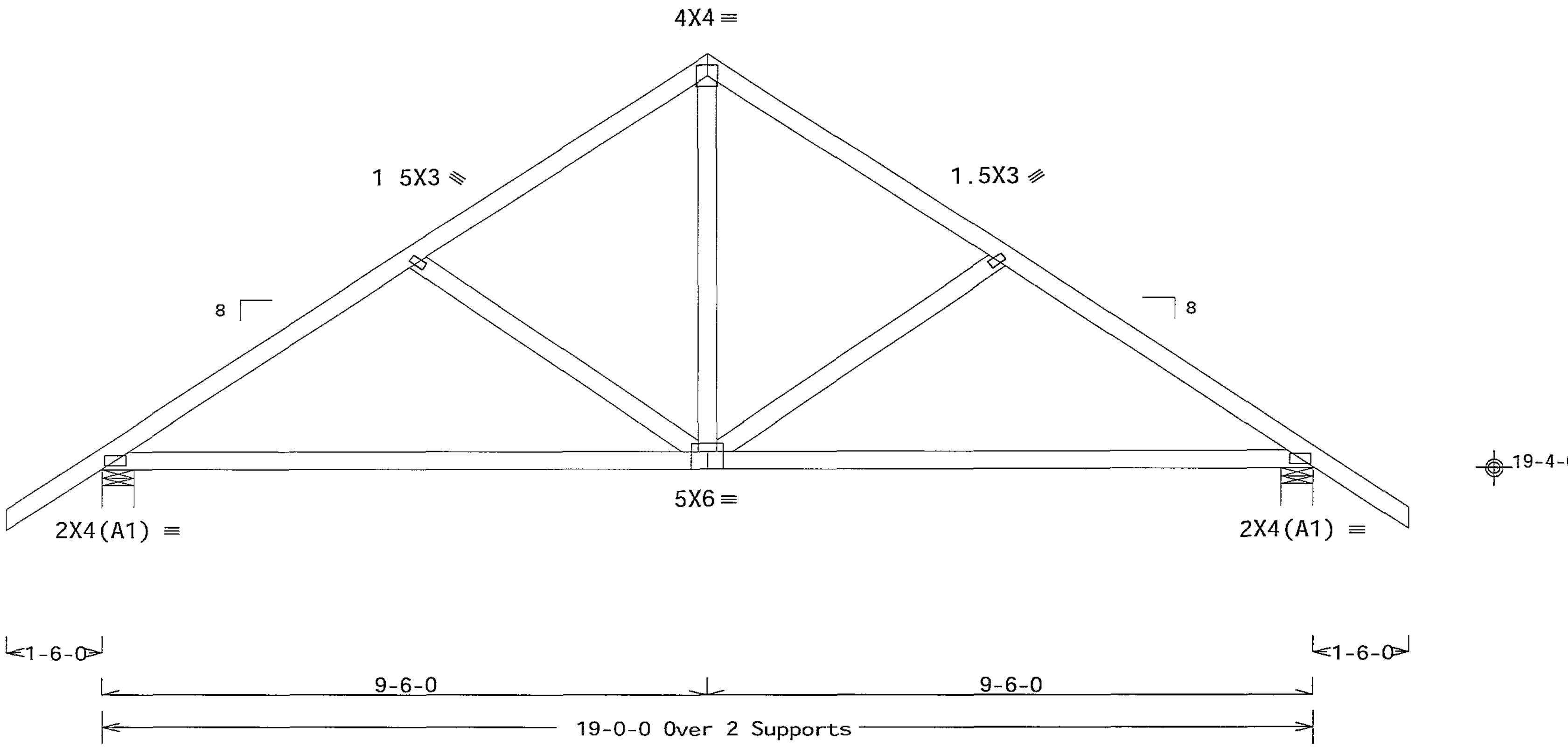
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



R=822 U=100 W=6 (6" min)
RL=143/-143

R=822 U=100 W=6" (6" min)

Design Crit FBC2010Com/TPI-2007(STD)
FT/RT=10%(0%)/0(0) 12 08 04 0326 13 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

PLT TYP Wave

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37 0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R9114- 31215
DATE	12/09/13
DRW	HCUSR9114 13343005
HC-ENG	WHK/WHK
SEQN-	336059
FROM	JMW
JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - C2 19 Common)

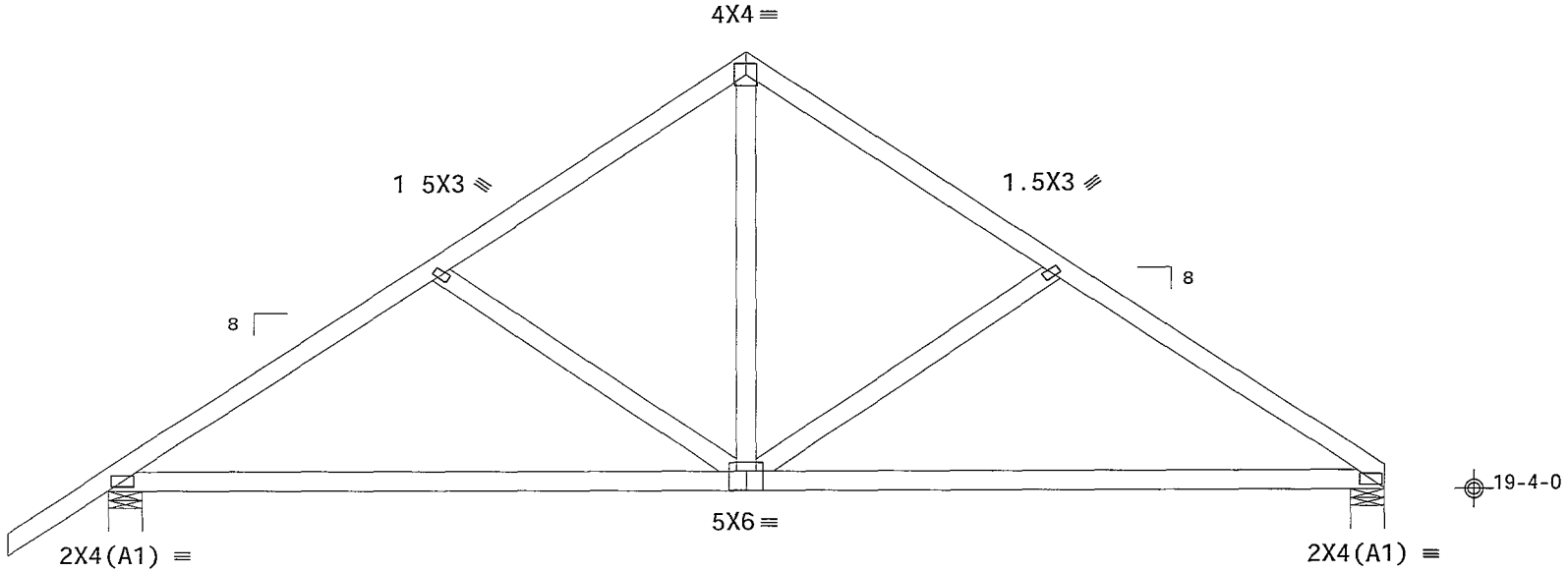
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



R=827 U=102 W=6" (6" min)
RL=121/-130

R=725 U=77 W=6" (6" min)

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 08 04 0326 13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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12/09/2013

TC LL	20.0 PSF	REF	R9114- 31216
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336049
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - C3 19 Common Girder)

Top chord 2x4 SP M-30
Bot chord 2x6 SP SS-13B
Webs 2x4 SP #3-13B W4, W5 2x4 SP #2-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 06
TC- From 28 plf at 7 06 to 28 plf at 9 50
TC- From 57 plf at 9 50 to 57 plf at 19 00
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 06
BC- From 10 plf at 7 06 to 10 plf at 10 00
BC- From 10 plf at 10 00 to 10 plf at 19 00
BC- 2317 52 lb Conc Load at 7 06
BC- 1066 96 lb Conc Load at 9 06
BC- 1071 77 lb Conc Load at 11 06,13 06,15 06,17 06

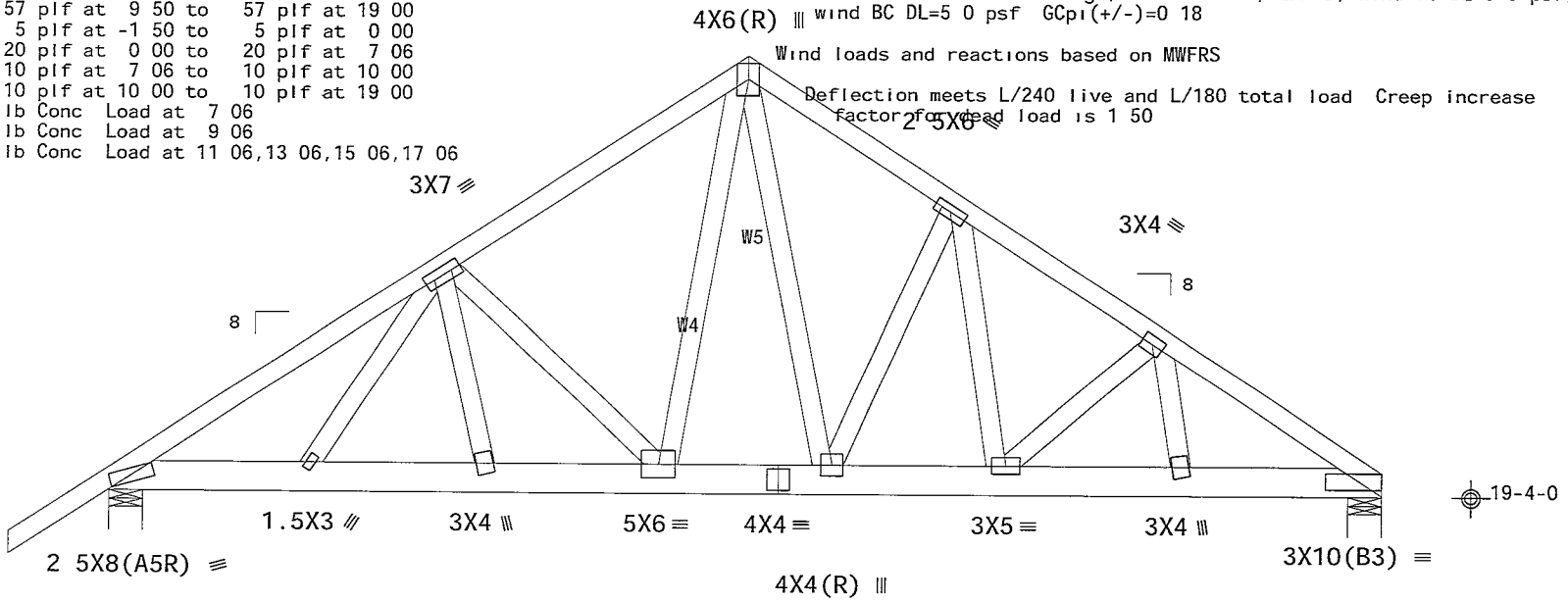
2 COMPLETE TRUSSES REQUIRED

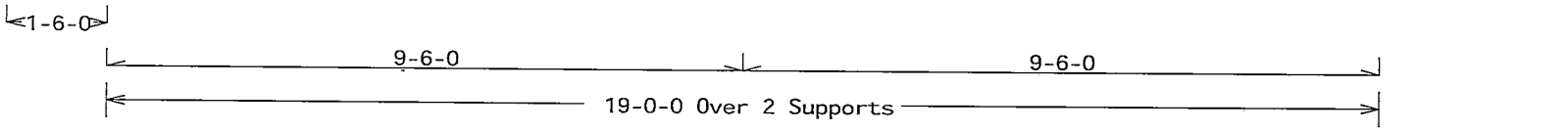
Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @ 3 75" o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=3868 U=540 W=6" (6" min)
RL=121/-130

R=5167 U=467 W=6" (6" min)

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 08 04 1032 6 13 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng shipping install ng and brac ng Refer to follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) for practices prior to performing these functions Installers shall provide temporary bracing per BCSI. Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locat ons shown for permanent lateral restrain of webs shall have bracing installed per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any fa lure to build the truss in conformance w th ANSI/TPI 1 or for handl ng sh pping install ation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this draw ng or cover page l st ng th s draw ng ind cates acceptance of professional eng neering respons bility solely for the design shown The suitab lity and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This design general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31217
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336338
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - CGE 10 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

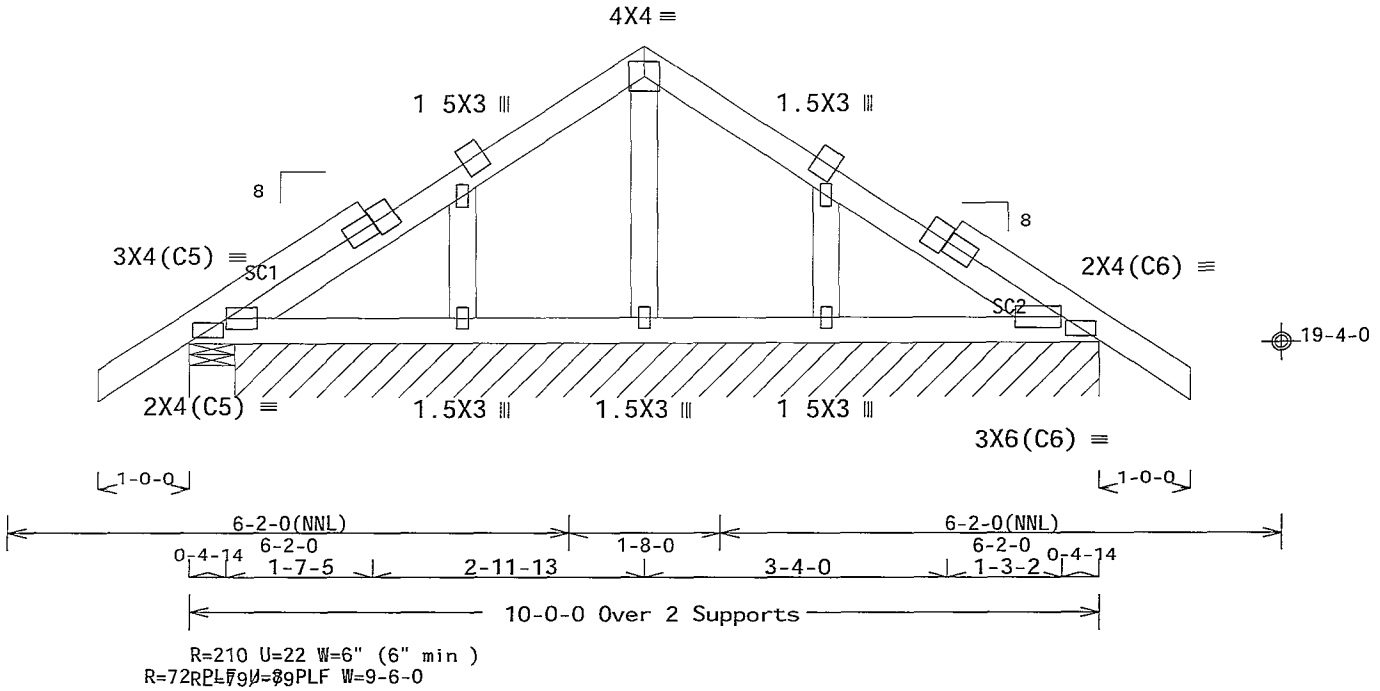
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 86 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Gable end supports 8" max rake overhang

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=210 U=22 W=6" (6" min)
R=72 PLF 90 89 PLF W=9-6-0

Note. All Plates Are 3X4 Except As Shown.

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03-04-0326.13 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care n fabricating handl ng sh pp ng nstall ng and bracing Refer to follow the latest edition of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for pract ces pr or to performing these funct ons Installers shall provide temporary brac ng pe Unless noted otherwise top chord shall have properly attached structural sheathing and bott shall have a properly attached rigid ceil ng Locations shown for permanent lateral restrai shall have bracing installed per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be responsible for any deviat on fr any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling shipp ng brac ng of trusses Apply plates to each face of truss and pos tion as shown above and on Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons drawing or cover page list ng this draw ng ndicates acceptance of professional eng neer ng responsibility solely for the design shown The suitabil ty and use of th s des gn for any the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31218
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	336019
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - CGE1 19' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface plate length perpendicular to chord length Splice top chord in notchable area using 3x6

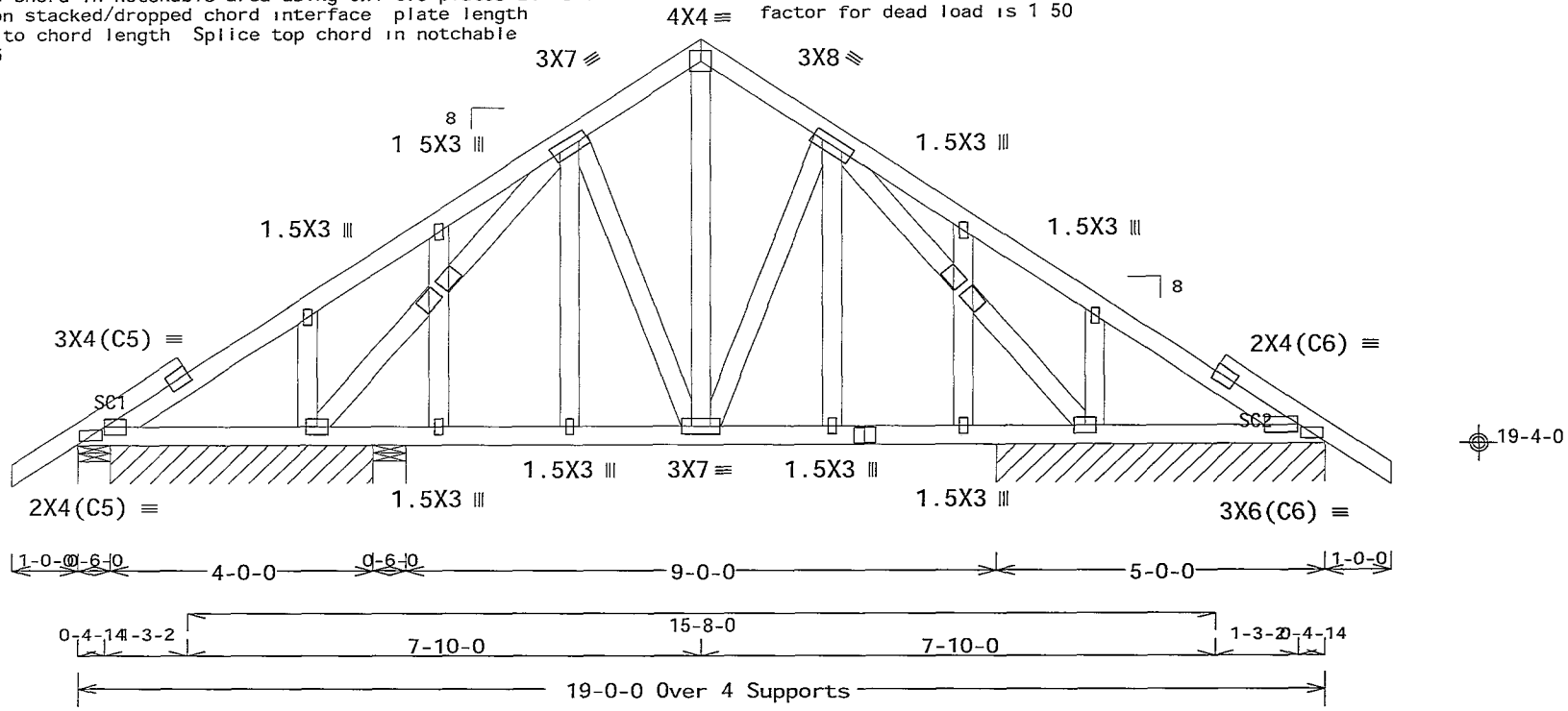
120 mph wind, 22 36 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=459 U=47 W=6" (6" min) R=303 U=72 W=6" (6' min) R=360 PLF U=55 PLF W=5-0-0
RL=2945 U=8F U=40 PLF W=4-0-0

Note. All Plates Are 3X4 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care n fabr cat ng handl ng sh pp ng nstalling and brac ng Refer to follow the latest edition of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for practices pr or to perform ng these functions Installers shall prov de temporary brac ng per Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r gid ceiling Locat ons shown for permanent latoral restraint of webs shall have brac ng nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th s drawing or cover page listing this drawing indicates acceptance of profess ona eng neer ng responsibility solely for the des gn shown The suitabil ty and use of th s des gn for any str the respons b lity of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31219
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	336014
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - CJ1 1 Jack)

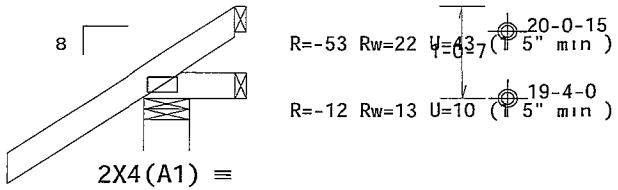
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

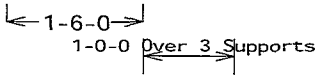
Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 19 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design





R=234 U=41 W=6' (6' min)
RL=33/-24

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03-04-0226.13 QTY:6 FL/-/4/-/-/R/- Scale = 5"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care n fabricating handling sh pp ng nstall ng and bracing Re... follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for... pract ces pr or to performing these functions Installers shall provide temporary bracing per BCSI. Unless noted otherw se top chord shall have properly attached structural sheath ng and bott... chord shall have a properly attached rig d ce l ng Locat ons shown for permanent lateral restra... of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on fr... des gn any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng shipp ng ... tallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Data is unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on th s draw ng or cover page list ng this draw ng indicates acceptance of profess onal eng neer ng respons bility solely for the design shown The suitabil ty and use of th s des gn for any st... the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31220
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	336312
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City - CJ3 3 Jack)

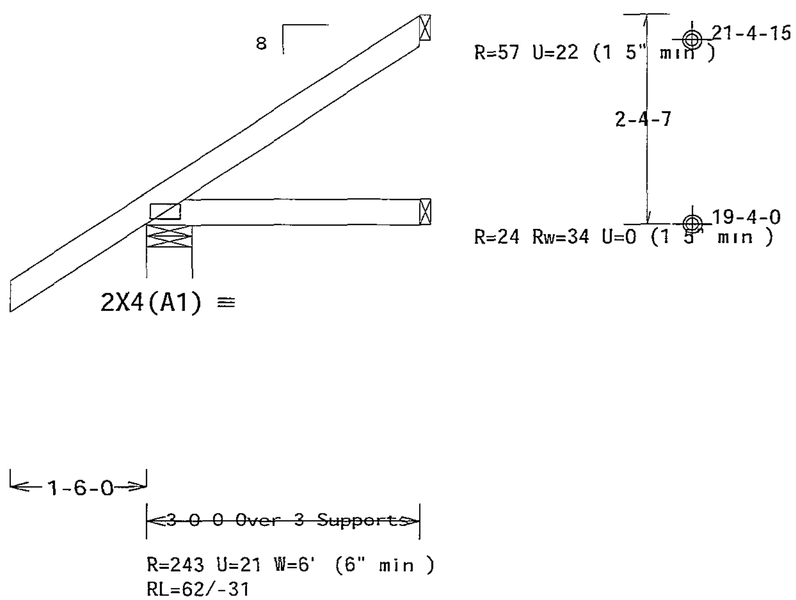
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 20 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave   FT/RT=10%(0%)/0(0)   12.02.04.0026.13   QTY:6   FL/-/4/-/-/R/-   Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31221
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	336321
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City - CJ5 5 End Jack)

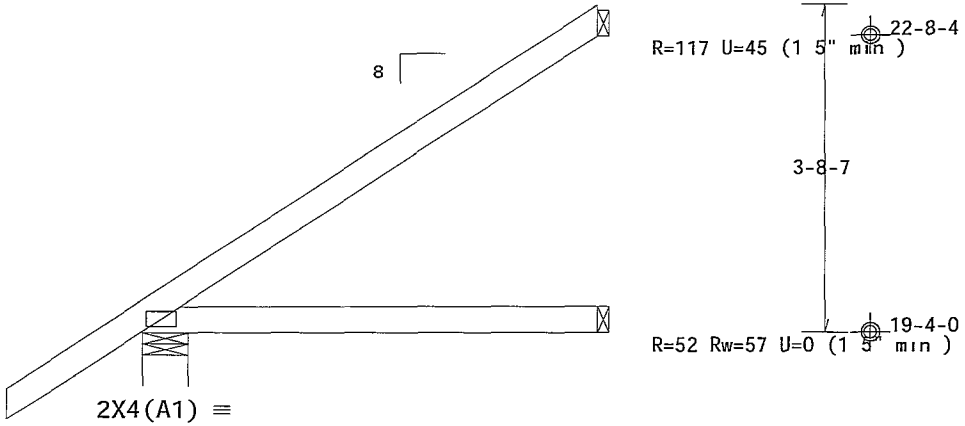
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

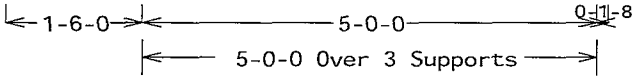
Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design





R=307 U=17 W=6" (6 min)
RL=92/-38

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03-04.0026.13 | QTY:6 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cating handl ng sh pp ng nstall ng and brac ng Ref r to the latest edit on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for fo low the latest edit on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for practices prior to performing these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from th s des gn any fa lure to build the trusses in conformance w th ANSI/TPI 1 or for handl ng sh pping nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this drawing or cover page l st ng th s drawing nd cates acceptance of profess onal engineering respons b l ty solely for the des gn shown The su tab l ity and use of this design for any structure is the responsib l ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more informat on see TPI general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ecsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31222
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	336309
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City - D 31 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

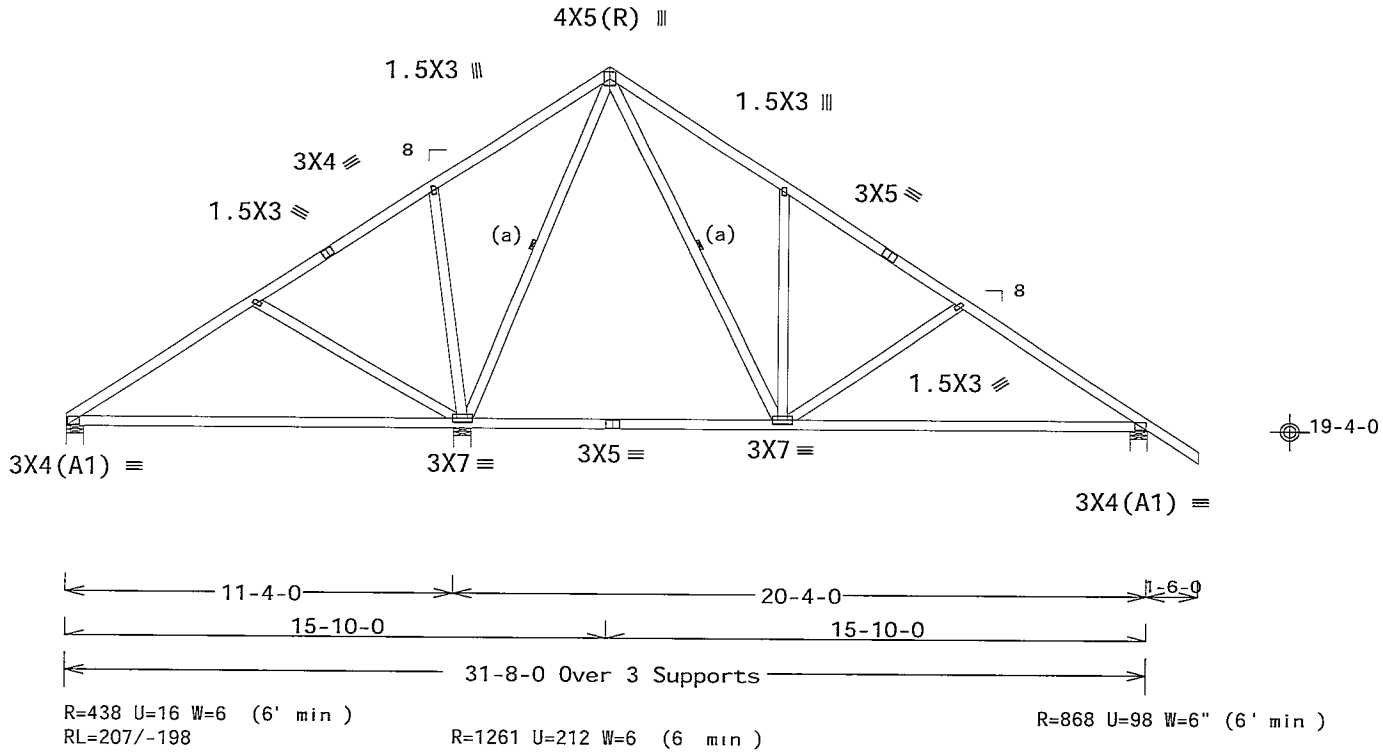
Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

120 mph wind, 24 48 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03-04.0826 13 QTY.1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care n fabricat ng handling sh pp ng install ng and brac ng Re follow the latest ed t on of BCSI (Building Component Safety Information by TPI and WTCA) for practices pr or to perform ng these funct ons Installers shall provide temporary bracing per BC Unless noted otherwise top chord shall have properly attached structural sheathing and botto shall have a properly attached rigid ceiling Locations shown for permanent lateral restra shall have bracing installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons b e for any dev ation fro any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping n stallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on th Joint Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate posit ons A s draw ng or cover page list ng this draw ng ind cates acceptance of professional eng neering responsibility solely for the design shown The su tab l ity and use of th s design for any st the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31223
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	336061
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - D1 20'4" Common Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x6 SP SS-13B
Webs 2x4 SP #3-13B W1 2x4 SP M-30
W3, W6 W8, W10 2x4 SP #2-13B W5 2x4 SP #1-13B
Rt Wedge 2x10 SP #2-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

120 mph wind, 24 98 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Left end vertical not exposed to wind pressure

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 4 50
TC- From 57 plf at 4 50 to 57 plf at 20 33
BC- From 10 plf at 0 00 to 10 plf at 20 33
BC- 1049 82 lb Conc Load at 1 60, 3 60, 5 60, 7 60 9 60,11 60,13 60,15 60,17 60,19 60

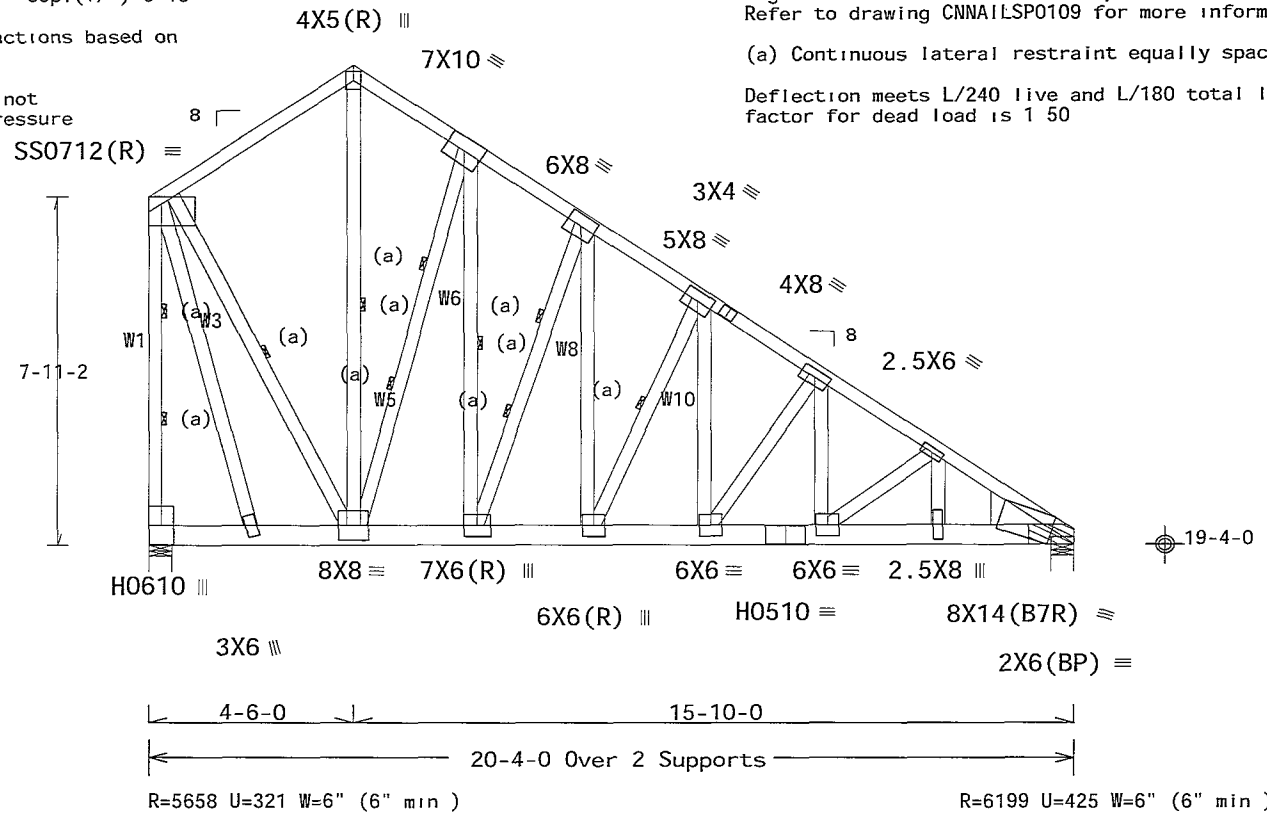
Brg blocks 0 131"x3', min nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	19 833'	1	12"	4	Rigid Surface

Brg block to be same size and species as chord
Refer to drawing CNNAILSP0109 for more information

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave Design Crit FBC2010Com/TPI-2007(STD) FT/RT=10%(0%)/0(0) 12 03 04 0306 13 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handl ng sh pp ng nstall ng and bracing Refer to and follow the latest ed tion of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locat ons shown for permanent lateral restrai of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on this Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page listing th s drawing nd cates acceptance of professional engineering respons b lity solely for the des gn shown The su tab l ty and use of this design for any structure is the respons bility of the Building Des gner per ANSI/TPI 1 Sec 2 For more informat on see This general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31224
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336333
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - DGE 31'8' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

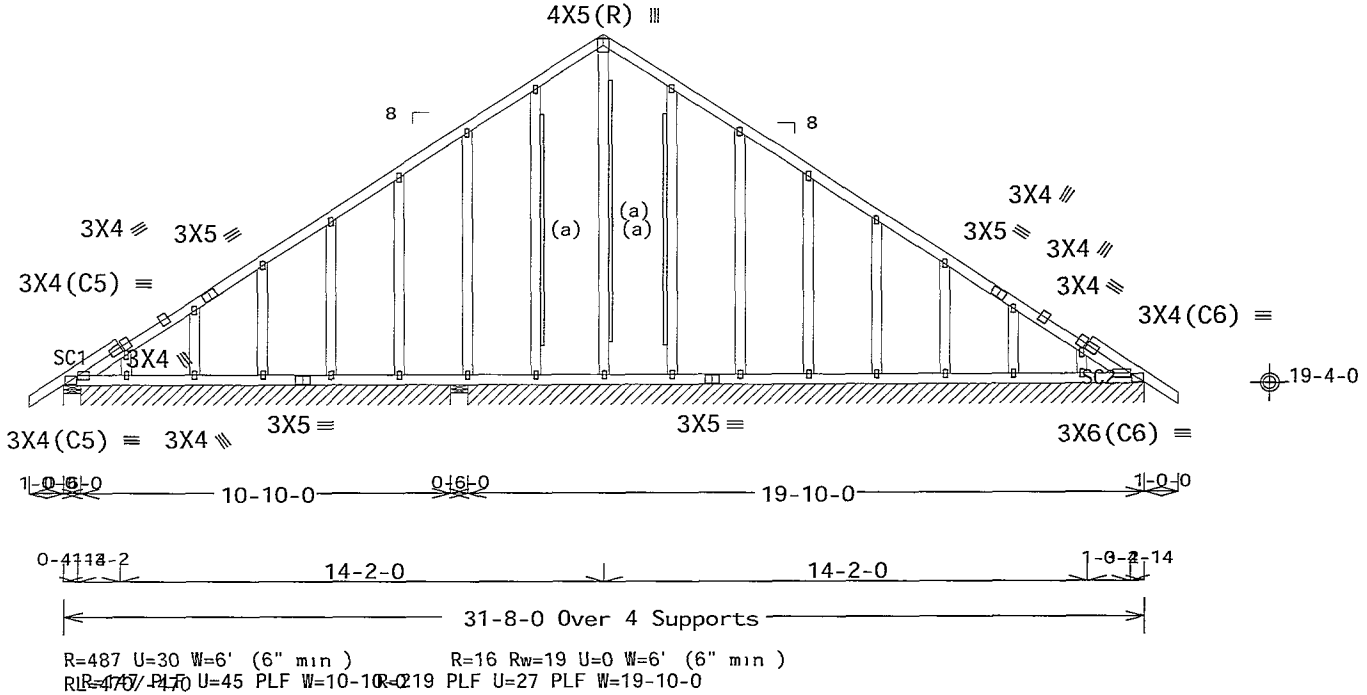
120 mph wind, 24 47 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

(a) 1x4 #3SRB SPF-S or better "L" brace 80% length of web member Attach with 8d Box or Gun (0 113"x2 5",min)nails @ 6" OC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1 50



Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03-04-09 26 13 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31225
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343015
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	336024
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - E1 27'6' Special)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

(J) Hanger Support Required, by others

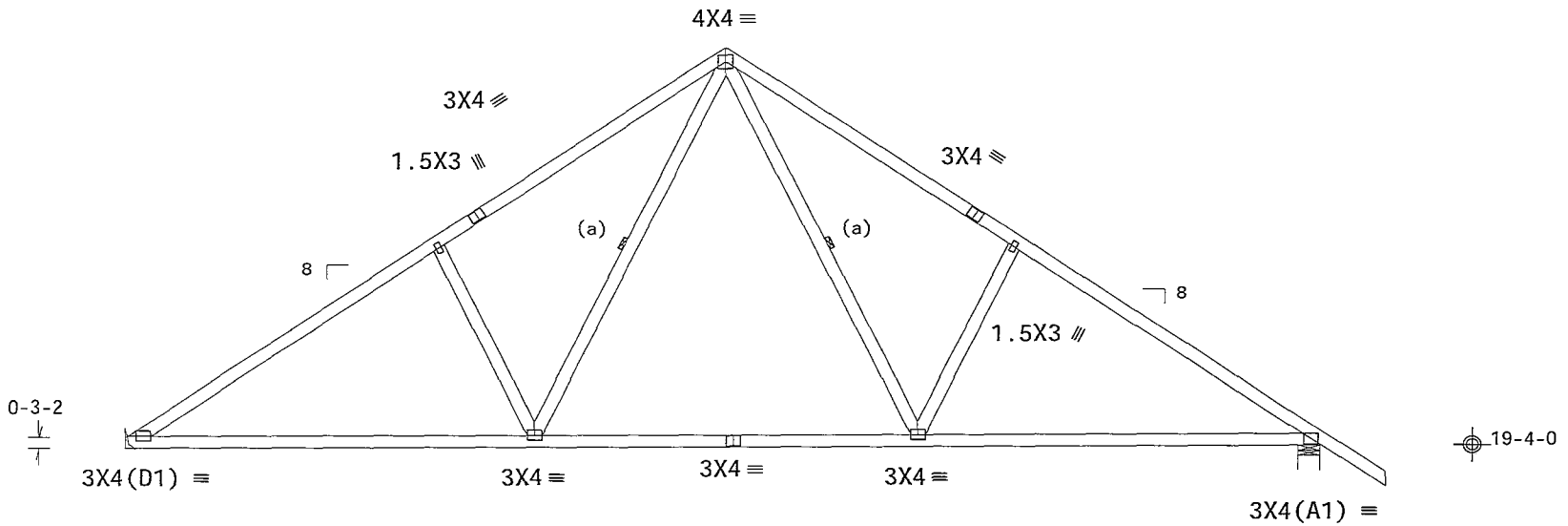
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 23 76 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

MWFRS loads based on trusses located at least 11 88 ft from roof edge



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:7 | FL/-/4/-/-/R/- | Scale = 25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstalling and bracing Refer to follow the latest edition of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for practices prior to perform ng these funct ons Installers shall prov de temporary brac ng per Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31226
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343016
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	336311
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - E2 28' Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

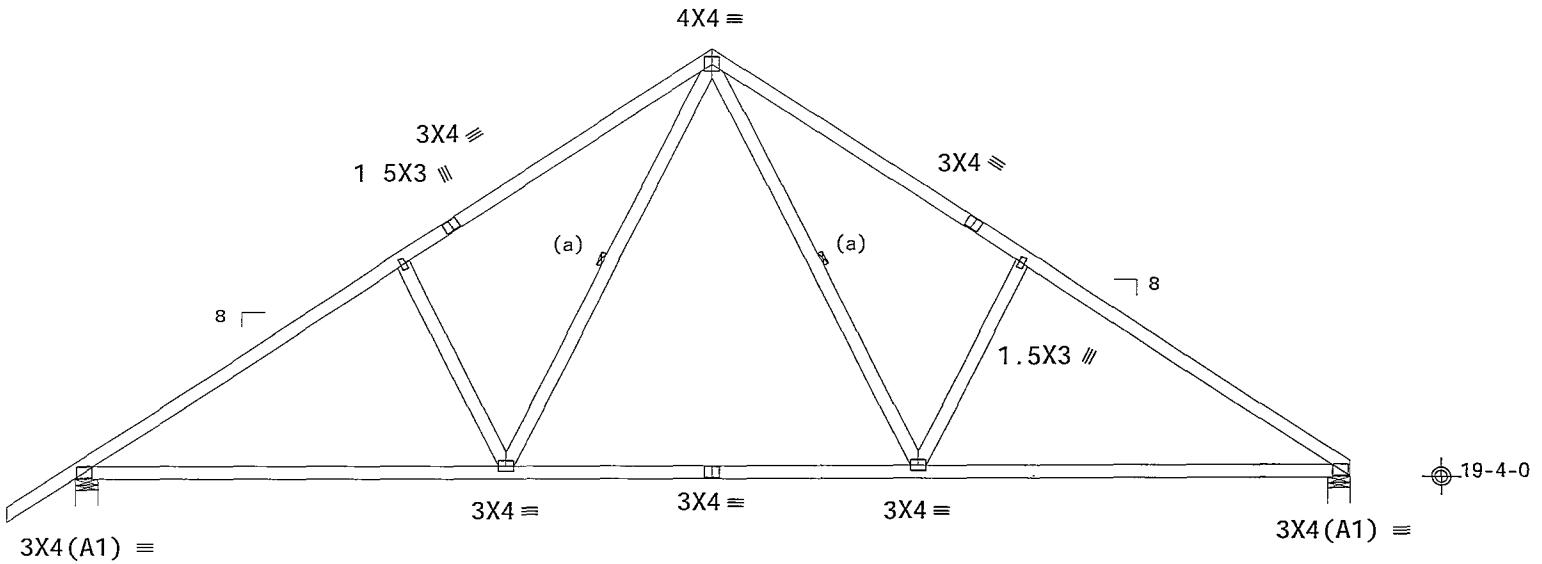
(a) Continuous lateral restraint equally spaced on member

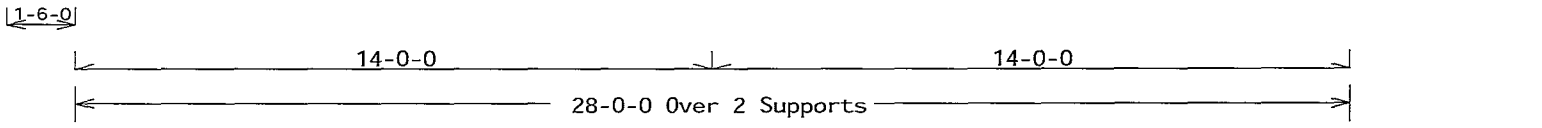
MWFRS loads based on trusses located at least 23 87 ft from roof edge

120 mph wind, 23 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1171 U=0 W=6" (6" min)
RL=174/-184

R=1073 U=0 W=6" (6" min)

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:6 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal with this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see general notes page. ITW-BCG www.twbcg.com, TPI www.tpinst.org, WTCA www.sbcindustry.com, ICC www.iccsafe.org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31227
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343017
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	336318
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - E3 28 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

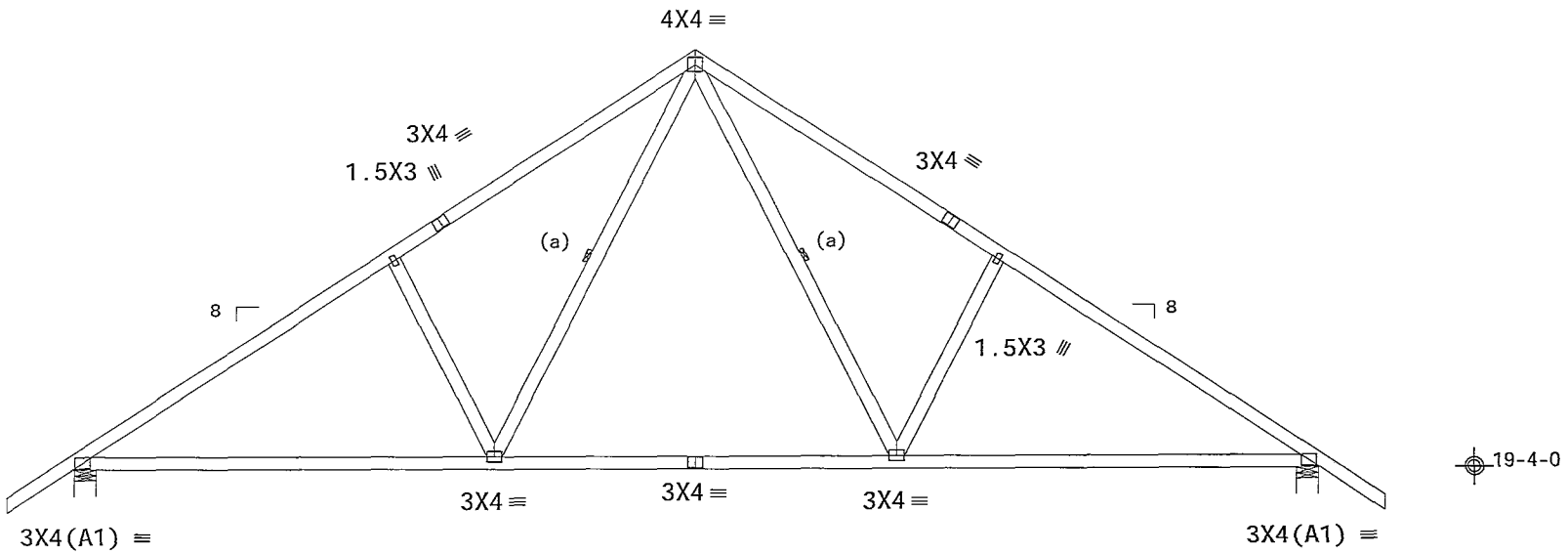
(a) Continuous lateral restraint equally spaced on member

MWFRS loads based on trusses located at least 11 94 ft from roof edge

120 mph wind, 23 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326 13 | QTY 5 | FL/-/4/-/-/R/- | Scale = 25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0 0 PSF
TOT.LD	37.0 PSF
DUR FAC.	1.25
SPACING	24.0"

REF	R9114- 31228
DATE	12/09/13
DRW	HCUSR9114 13343018
HC-ENG	WHK/WHK
SEQN-	336320
FROM	JMW
JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - E4 28' Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS-13B supporting member
(14) 0 148 x3 nails into supporting member,
(4) 0 148 x3 nails into supported member

MWFRS loads based on trusses located at least 11 94 ft from roof edge

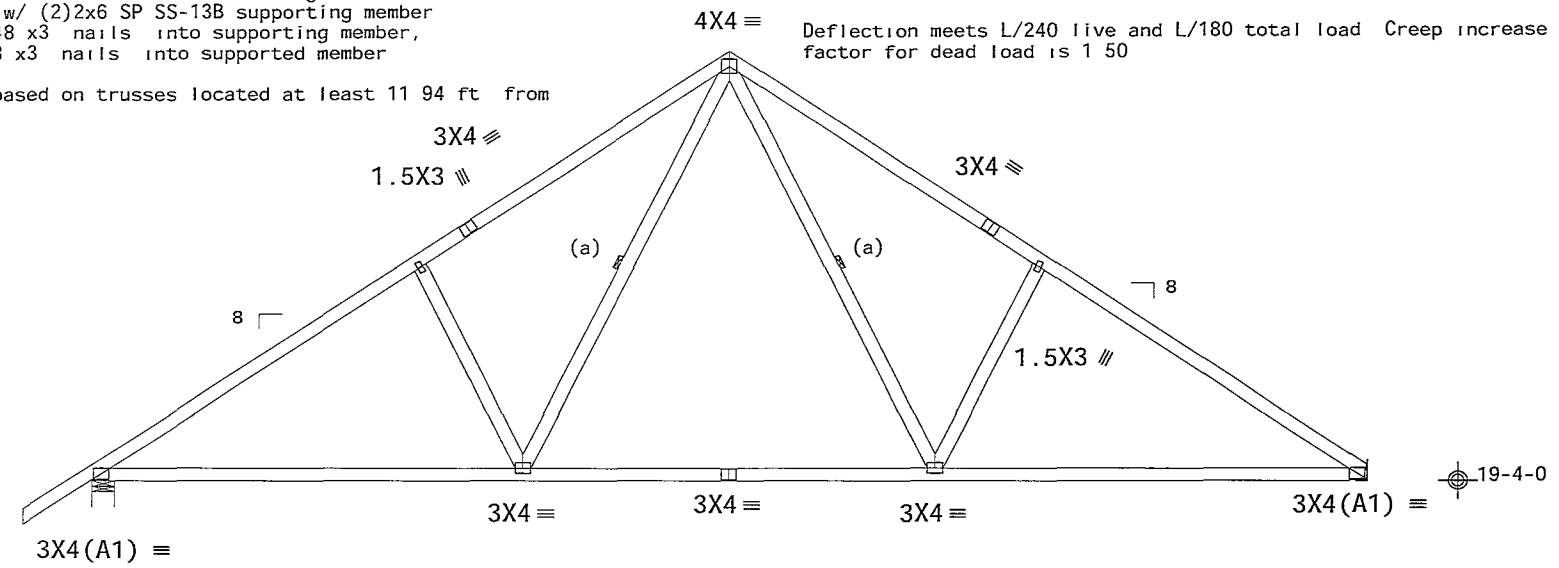
120 mph wind, 23 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

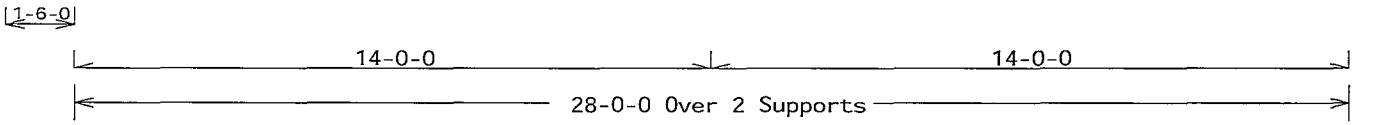
Wind loads and reactions based on MWFRS with additional C&C member design

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1172 U=45 W=6 (6" min)
RL=174/-184

R=1072 U=31
H=H1

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY.2 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31229
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10 0 PSF	DRW	HCUSR9114 13343019
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336314
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - EJ7 7 End Jack)

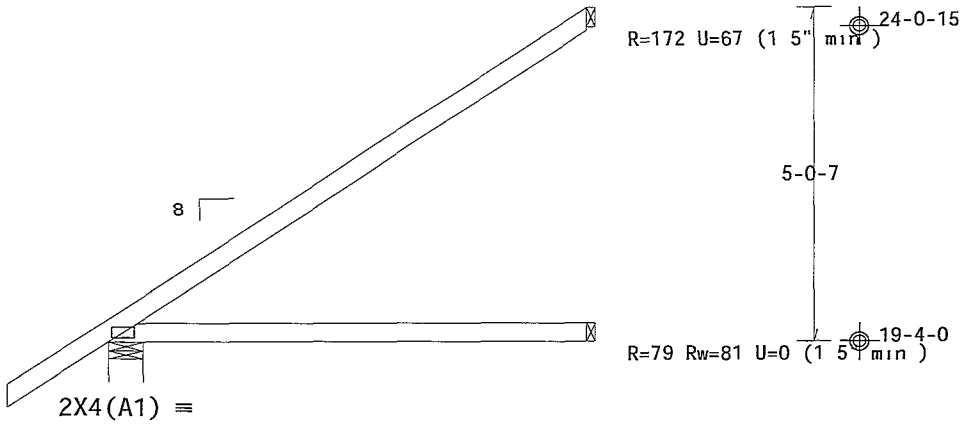
Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12 03 04 0326.13 QTY:19 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31230
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336304
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H11 27'6 Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

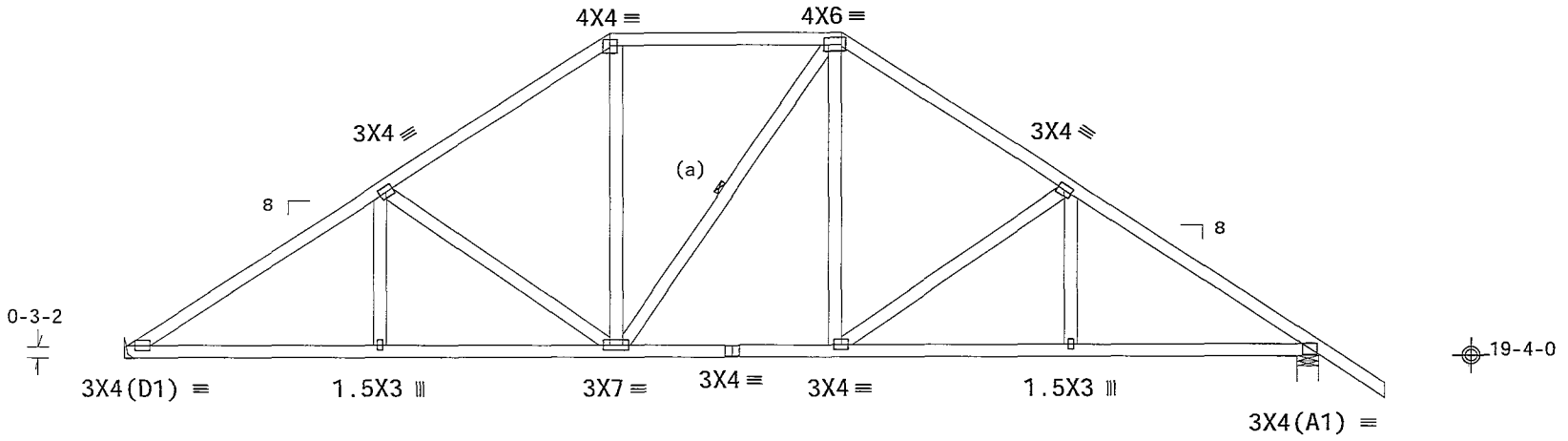
MWFRS loads based on trusses located at least 11 44 ft from roof edge

120 mph wind, 22 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabricat ng handl ng sh pping installing and bracing Re to and follow the latest edit on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locat ons shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to bu ld the truss n conformance w th ANSI/TPI 1 or for handling shipp ng nstallation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons Any deal of this draw ng or cover page list ng this draw ng ind cates acceptance of professional eng neering responsibility solely for the design shown The suitab lity and use of this design for any structure is the responsibil ty of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R9114- 31231
DATE	12/09/13
DRW	HCUSR9114 13343021
HC-ENG	WHK/WHK
SEQN-	336331
FROM	JMW
JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H11A 28' Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS-13B supporting member
(14) 0 148"x3 nails into supporting member,
(4) 0 148"x3' nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

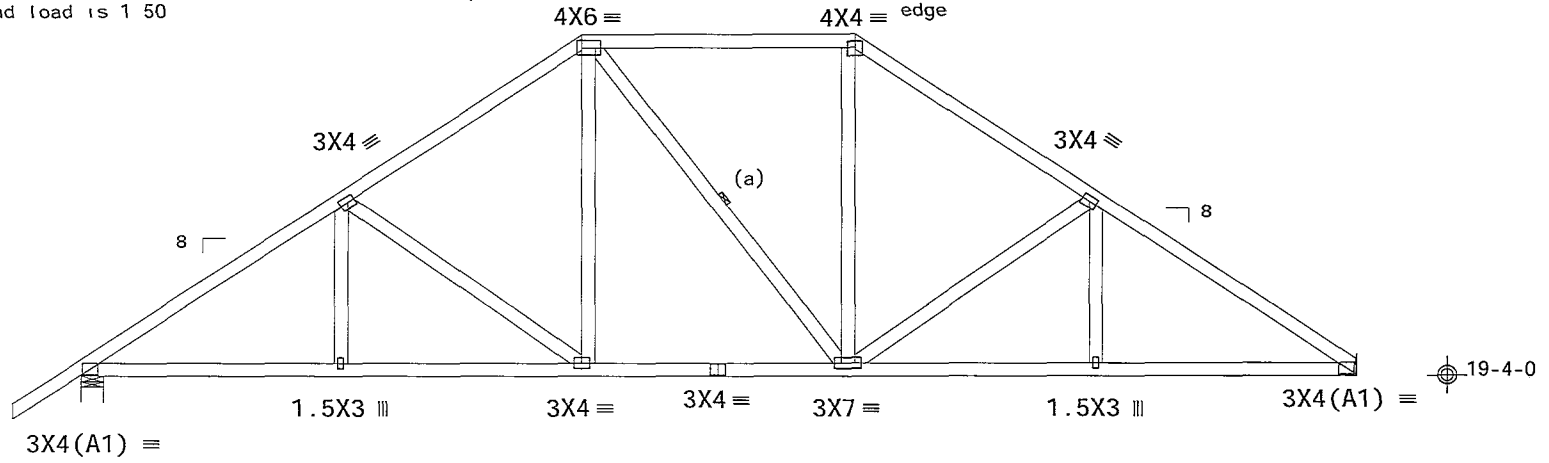
Wind loads and reactions based on MWFRS with additional C&C member design

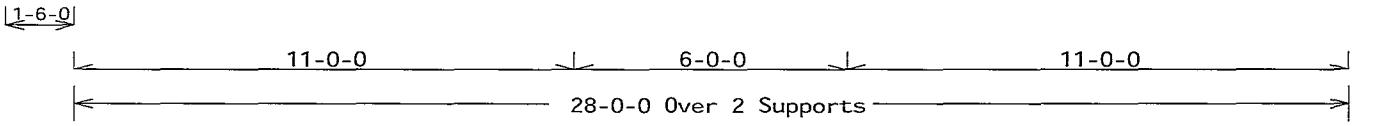
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 11 44 ft from roof edge





R=1172 U=50 W=6" (6" min)
RL=139/-148

R=1072 U=36
H=H1

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326 13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20 0 PSF	REF	R9114- 31232
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343022
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	336329
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H13 27'6" Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

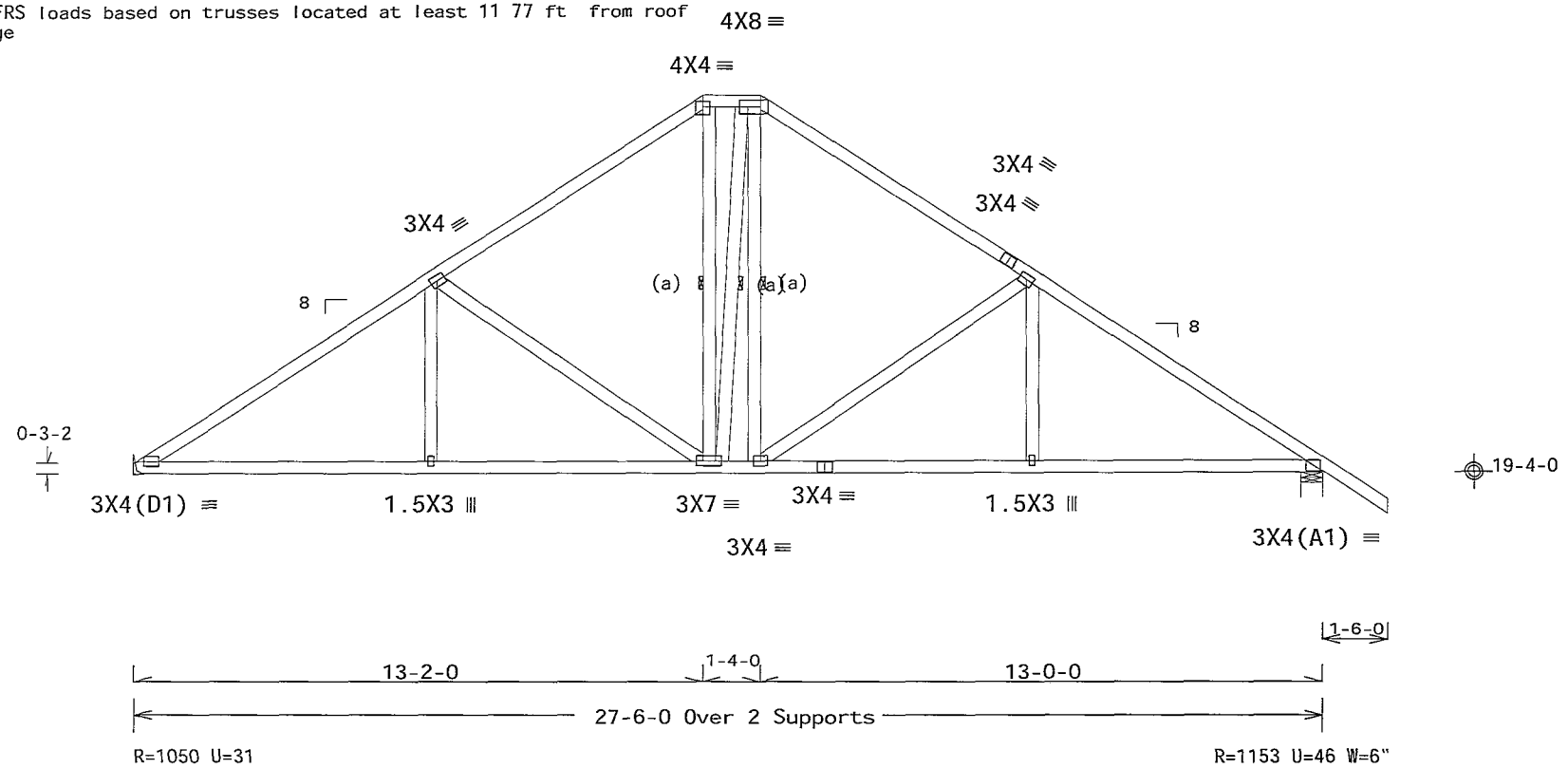
MWFRS loads based on trusses located at least 11 77 ft from roof edge

120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31233
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343023
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	336325
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H13A 28 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS-13B supporting member
(14) 0 148 x3' nails into supporting member,
(4) 0 148'x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

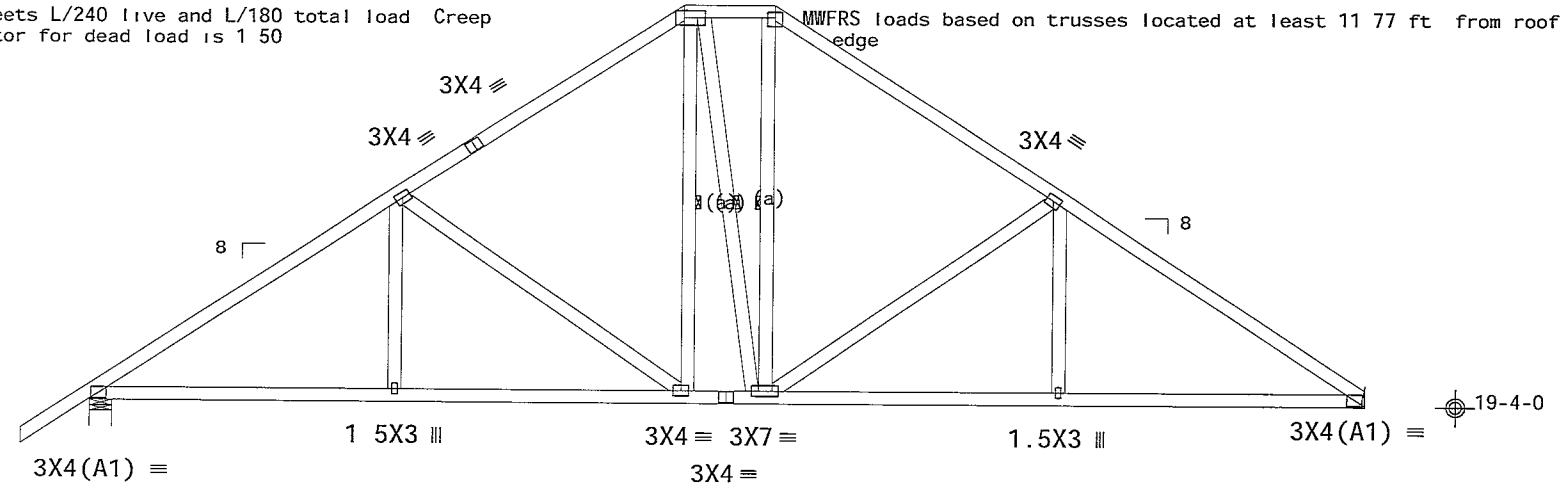
Wind loads and reactions based on MWFRS with additional C&C member design

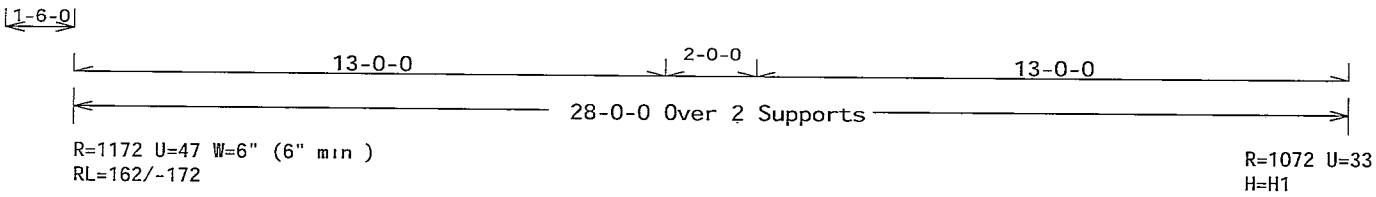
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge





Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31234
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10 0 PSF	DRW	HCUSR9114 13343024
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	336323
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H7 27 4' Stepdown Hip Girder)

Top chord 2x4 SP #1-13B T2 2x4 SP M-30
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

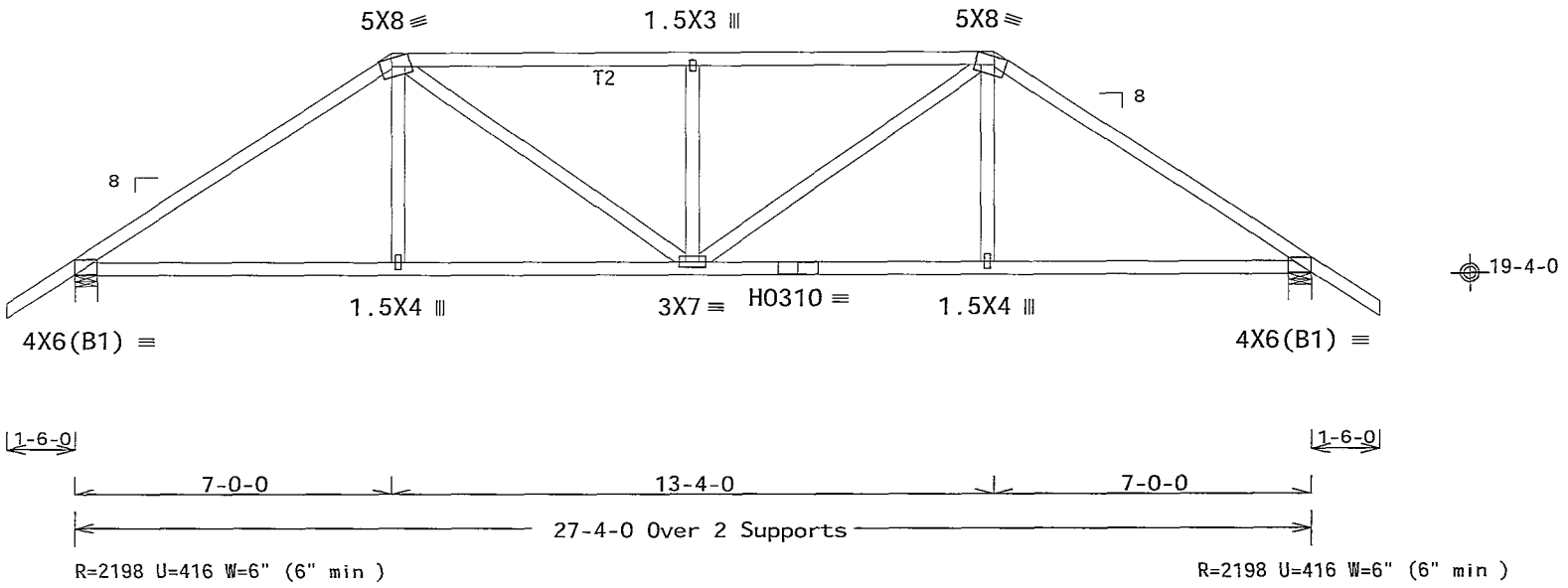
120 mph wind, 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 20 33
TC- From 57 plf at 20 33 to 57 plf at 28 83
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 20 30
BC- From 20 plf at 20 30 to 20 plf at 27 33
BC- From 5 plf at 27 33 to 5 plf at 28 83
TC- 246 45 lb Conc Load at 7 03,20 30
TC- 171 79 lb Conc Load at 9 06,11 06,13 06,14 27 16 27,18 27
BC- 311 85 lb Conc Load at 7 03,20 30
BC- 79 29 lb Conc Load at 9 06,11 06,13 06,14 27 16 27,18 27



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:1 | FL/-/4/-/-/R/- | Scale = 25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM R. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL 20.0 PSF	REF R9114- 31235
TC DL 7 0 PSF	DATE 12/09/13
BC DL 10 0 PSF	DRW HCUSR9114 13343025
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT.LD 37.0 PSF	SEQN- 336337
DUR.FAC 1.25	FROM JMW
SPACING 24.0"	JREF- 1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - H9 27'6" Stepdown Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

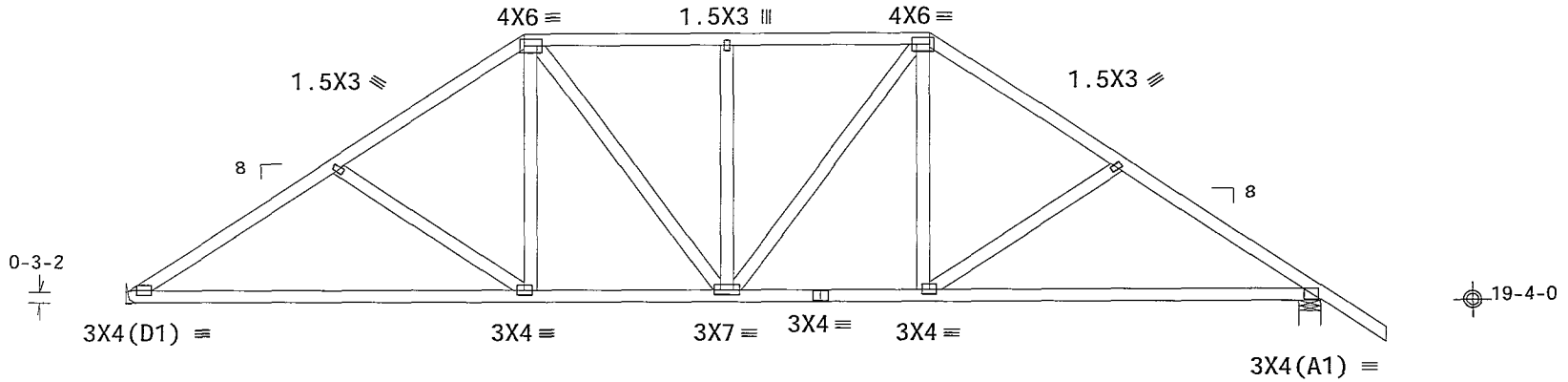
(J) Hanger Support Required, by others

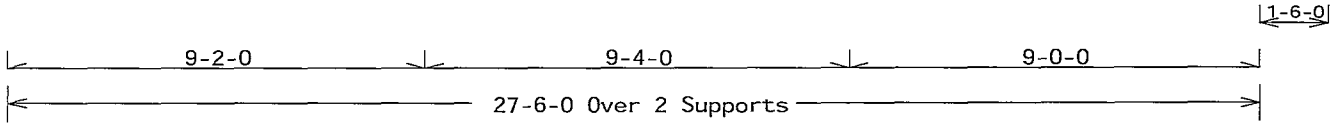
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 20 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC





R=1050 U=117
RL=125/-115
H=H1

R=1153 U=142 W=6"

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 0 0 04 0326 13 | QTY·1 | FL/-/4/-/-/R/- | Scale =.25"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31236
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343026
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	336316
DUR FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - HJ7 9'10"13 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

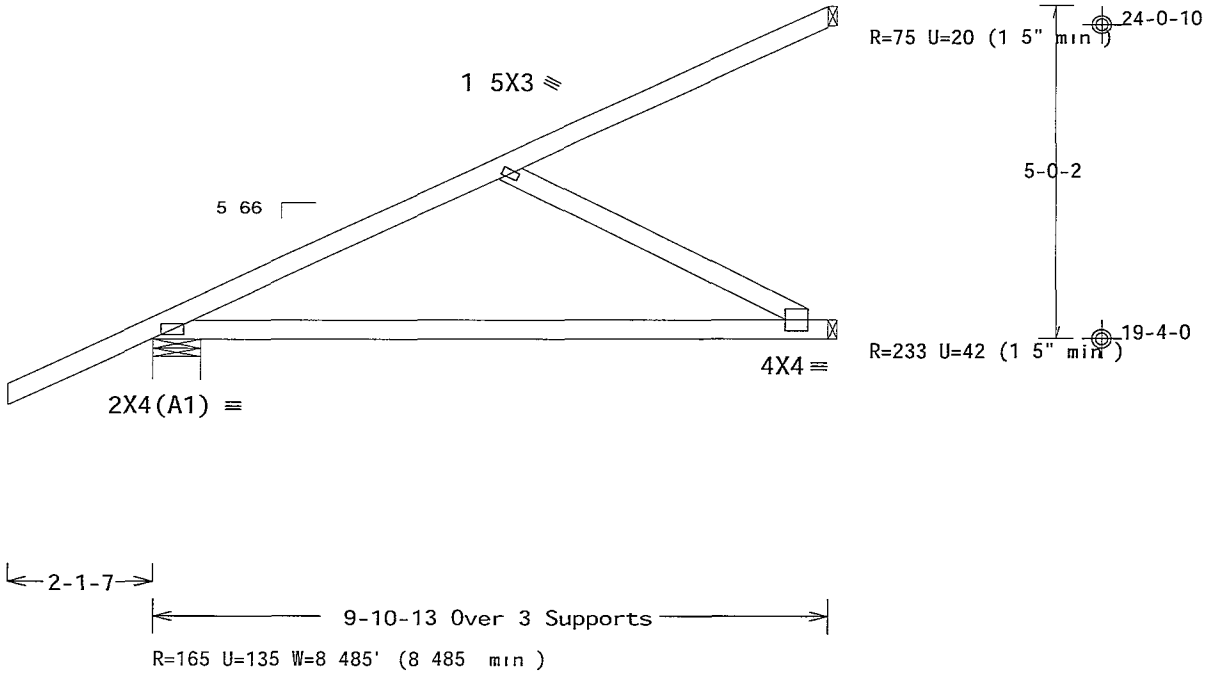
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 21 51 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 12 to 55 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -2 12 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 90
TC- -105 31 lb Conc Load at 1 48
TC- 113 56 lb Conc Load at 4 31
TC- 234 05 lb Conc Load at 7 13
BC- -24 86 lb Conc Load at 1 48
BC- 47 12 lb Conc Load at 4 31
BC- 104 44 lb Conc Load at 7 13



Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 08 04 0326 13 | QTY:3 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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12/09/2013

TC LL	20.0 PSF	REF	R9114- 31237
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10 0 PSF	DRW	HCUSR9114 13343027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	336332
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - MH7 28 Mono Hip Girder)

Top chord 2x4 SP M-30 T1 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B2 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

120 mph wind, 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS-13B supporting member
(14) 0 162 x3 5' nails into supporting member,
(4) 0 162 x3 5' nails into supported member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

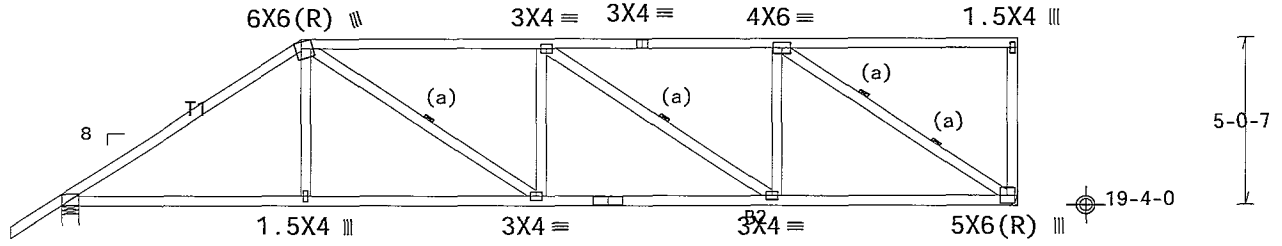
Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 7 00
TC- From 28 plf at 7 00 to 28 plf at 28 00
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 7 03
BC- From 10 plf at 7 03 to 10 plf at 28 00
TC- 246 45 lb Conc Load at 7 03
TC- 171 79 lb Conc Load at 9 06,11 06,13 06,15 06 17 06,19 06,21 06,23 06,25 06,27 06
BC- 311 85 lb Conc Load at 7 03
BC- 79 29 lb Conc Load at 9 06,11 06,13 06,15 06 17 06,19 06,21 06,23 06,25 06,27 06

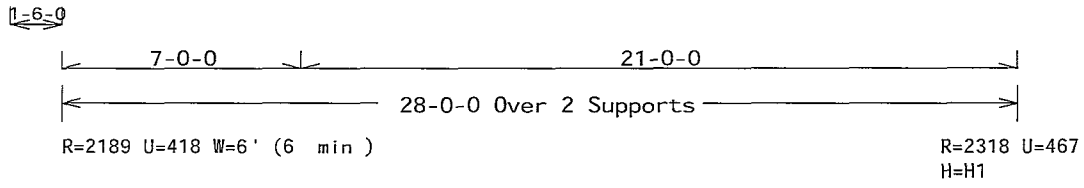
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12 08 04 0326 13 QTY·1 FL/-/4/-/-/R/- Scale = 1875"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstall ng and bracing Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices prior to performing these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ceiling Locations shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 87 or B10 as appl cable

ITW Build ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from this design any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng shipp ng nstallat on & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Joint Data Is unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons A seal on this draw ng or cover page listing this drawing indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This design general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31238
TC DL	7 0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343028
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	336335
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288--Premier Building /Becker Res Roof -- 337 sw rose creek drive Lake City, - MH9 28' Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS-13B supporting member
(14) 0 148 x3' nails into supporting member,
(4) 0 148"x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 20 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

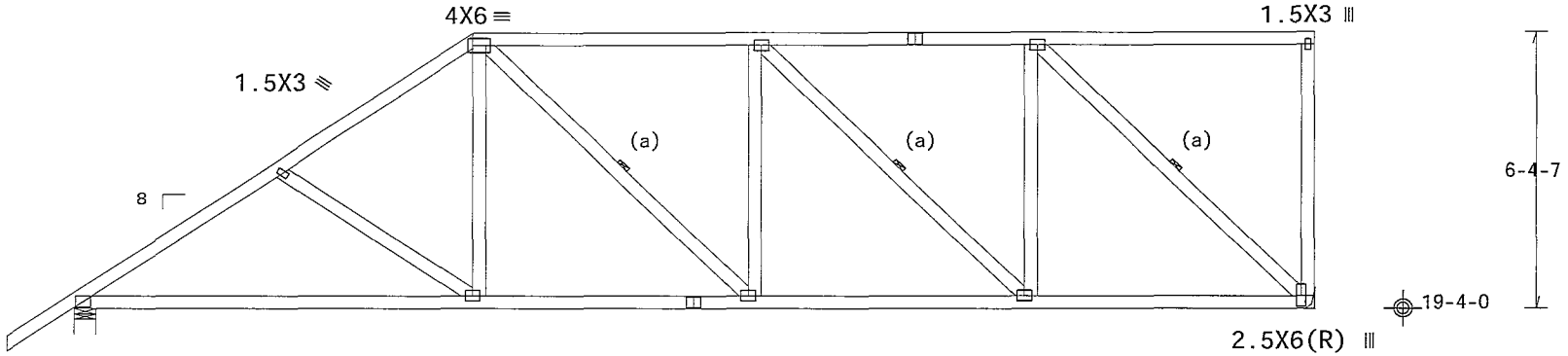
Wind loads and reactions based on MWFRS with additional C&C member design

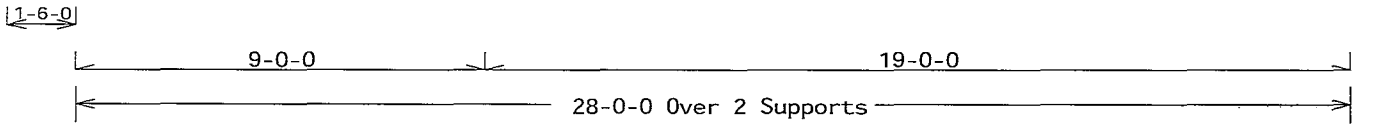
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC





R=1177 U=133 W=6" (6" min)
RL=156/-51

R=1067 U=148
H=H1

Note. All Plates Are 3X4 Except As Shown.

Design Crit· FBC2010Com/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0226.13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	20.0 PSF	REF	R9114- 31239
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343029
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	336327
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V1Y487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail s on y applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum a ternative reinforcement, re-run design w th approp r ate reinforcement type

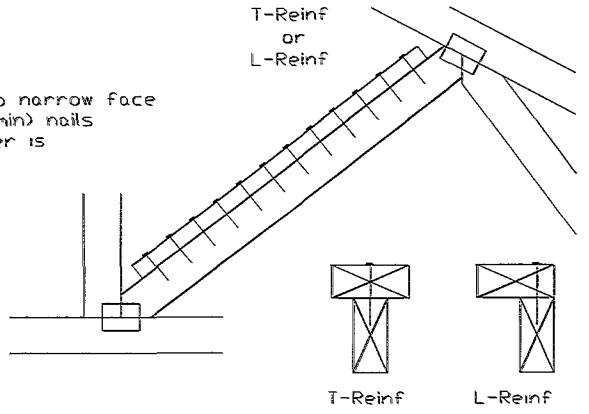
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

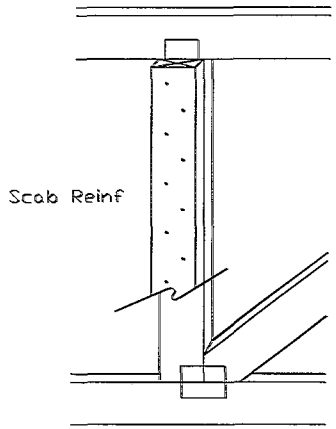
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0 128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0 128"x3 0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length.



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Building Components Group Inc.

Earth City MO 63045

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Refer to drawings 160A-Z for standard plate positions.
ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
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ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

12/09/2013

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 90 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Enclosed Exposure D Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
	24" o.c.	SPF HF	#1 / #2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14' 0"	14 0"	14' 0"	14' 0"
			#3	4 7"	7 9"	8 3"	9 7"	9 11"	11 5"	11' 10"	14 0"	14 0"	14' 0"	14 0"
			Stud	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
			Standard	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
		SP DFL	#1	4 11"	8' 3"	8' 7"	9 9"	10 1"	11 7"	12 1"	14 0"	14 0"	14' 0"	14' 0"
			#2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14 0"	14 0"	14' 0"	14' 0"
			#3	4 7"	6 11"	7' 4"	9 3"	9 10"	11 5"	11 10"	14 0"	14' 0"	14' 0"	14 0"
			Stud	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14 0"	14 0"	14' 0"	14 0"
			Standard	4 7"	6 0"	6 4"	8 0"	8 6"	10 10"	11' 7"	12' 6"	13 5"	14 0"	14' 0"
	16" o.c.	SPF HF	#1 / #2	5 6"	9 5"	9 9"	11 1"	11 6"	13' 2"	13 9"	14 0"	14 0"	14' 0"	14' 0"
			#3	5 3"	9 3"	9 9"	10 11"	11 4"	13 0"	13 7"	14' 0"	14 0"	14' 0"	14 0"
			Stud	5 3"	9 3"	9 7"	10 11"	11 4"	13 0"	13' 7"	14 0"	14 0"	14' 0"	14 0"
			Standard	5 3"	9' 3"	9 7"	10 11"	11 4"	13' 0"	13 7"	14' 0"	14' 0"	14' 0"	14' 0"
		SP DFL	#1	5 8"	9 5"	9 10"	11 2"	11 7"	13' 3"	13 10"	14 0"	14 0"	14 0"	14' 0"
			#2	5 6"	9' 5"	9' 9"	11 1"	11' 6"	13' 2"	13' 9"	14' 0"	14 0"	14' 0"	14' 0"
			#3	5 3"	8 6"	9 0"	10 11"	11' 4"	13' 0"	13 7"	14' 0"	14' 0"	14' 0"	14 0"
			Stud	5 3"	8' 6"	9 0"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14 0"
			Standard	5 3"	7' 4"	7' 10"	9 9"	10' 5"	13' 0"	13 7"	14' 0"	14' 0"	14' 0"	14 0"
	12" o.c.	SPF HF	#1 / #2	6 1"	10 4"	10 8"	12 2"	12 8"	13' 2"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14' 0"	14 0"	14' 0"
			Stud	5 9"	10 2"	10 7"	12 0"	12 6"	14' 0"	14 0"	14 0"	14' 0"	14 0"	14' 0"
			Standard	5 9"	10 2"	10 7"	12' 0"	12 6"	14' 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
		SP DFL	#1	6 2"	10 5"	10 9"	12' 3"	12 9"	14' 0"	14 0"	14' 0"	14' 0"	14' 0"	14 0"
			#2	6 1"	10' 4"	10 8"	12' 2"	12 8"	14' 0"	14 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5 9"	9' 9"	10 5"	12' 0"	12' 6"	14' 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"
			Stud	5 9"	9 9"	10 5"	12 0"	12' 6"	14 0"	14 0"	14 0"	14 0"	14' 0"	14 0"
			Standard	5 9"	8 6"	9' 0"	11 3"	12 1"	14' 0"	14 0"	14 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

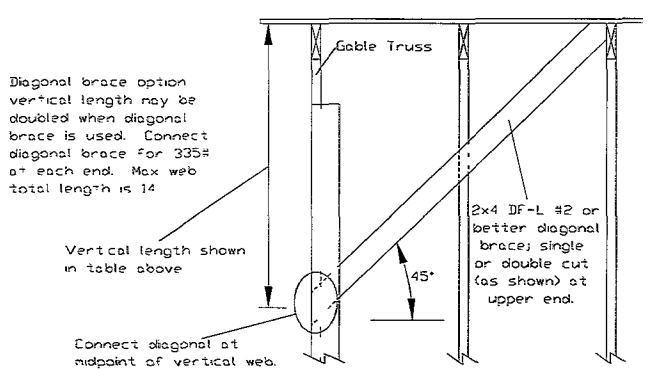
Group B

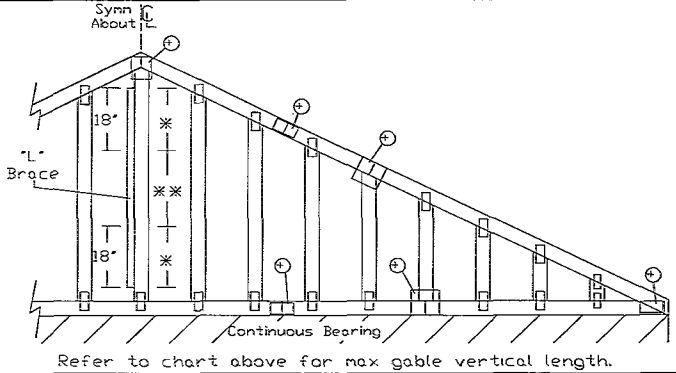
Hem-Fir: #1 & Btr; #1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.





Refer to chart above for max gable vertical length.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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Earth City MO 63045

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ITWBCG: www.itwbcg.com, TPI: www.tpinst.org; WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

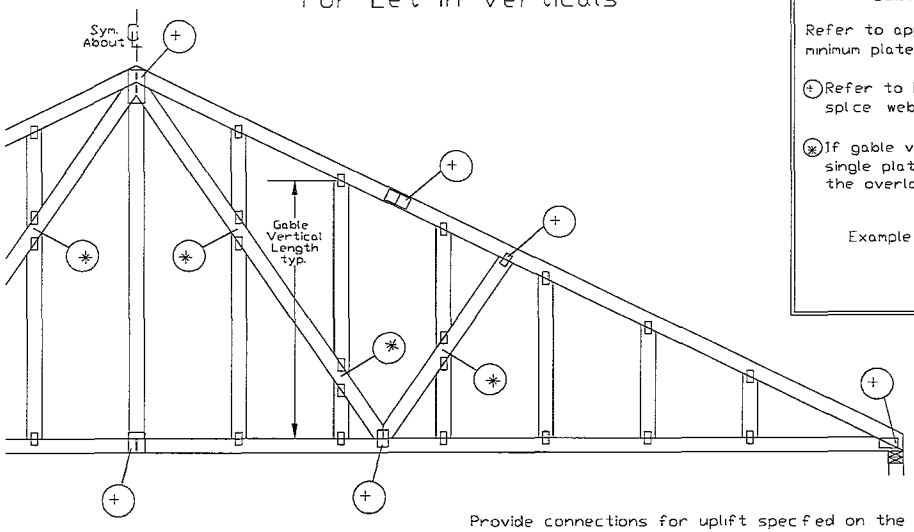
12/09/2013

MAX TOT LD 60 PSF

MAX SPACING 24 0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



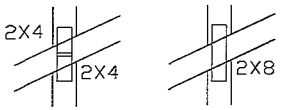
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

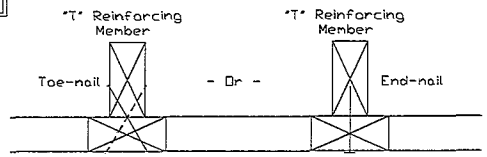
(+) Refer to Engineered truss design for peak splice web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

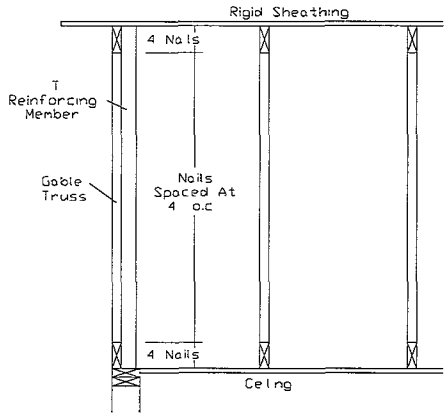
Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord

"T" reinforcing member material must match size specie, and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3" min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212 A20015PED100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

TW
Building Components Group Inc

Earth City MO 63045

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

12/09/2013

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212
MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24.0"

ASCE 7-10. 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00 Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

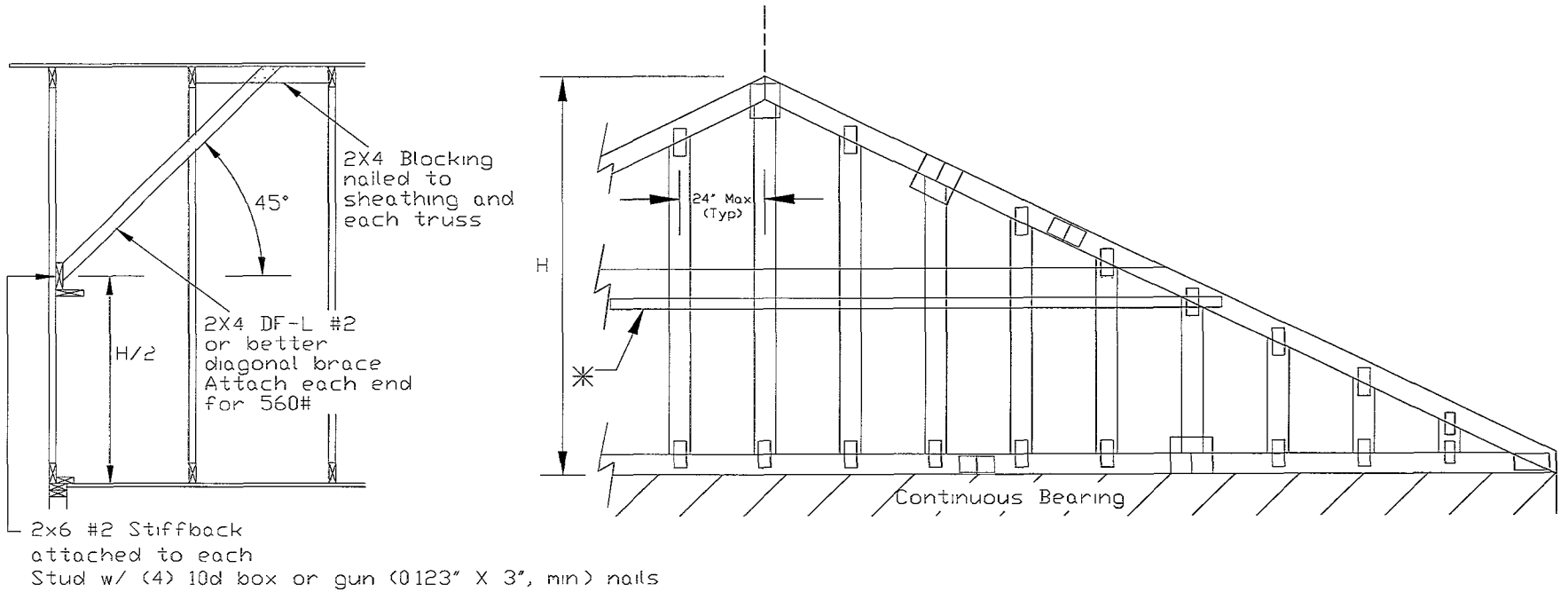
Nails 10d box or gun (0.128"x3",min) nails

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.



TW
Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and allow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

TW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites.
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org, WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

WILLIAM H. KRICK
LICENSE
No 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER

'13

12/09/2013

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 Mph Wind Speed 30' Mean Height Partially Enclosed Exposure C, Kzt = 1.00
Or 100 mph Wind Speed 30' Mean Height Enclosed Exposure D Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" oc	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	7' 6"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 10"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Standard	4' 3"	5' 7"	5' 11"	7' 5"	7' 11"	10' 0"	10' 9"	11' 8"	12' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	9' 0"	9' 4"	10' 7"	11' 0"	2' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	6' 10"	7' 3"	9' 1"	9' 8"	12' 4"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 11"	9' 11"	10' 3"	11' 8"	12' 2"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	7' 10"	8' 4"	10' 6"	11' 3"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

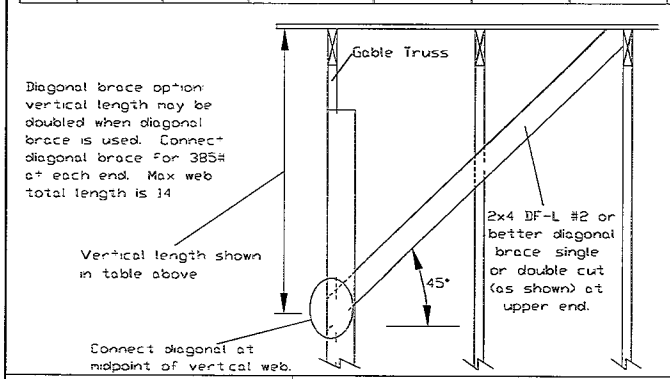
So. Pine lumber design values based on the ALSC January 2012 ruling

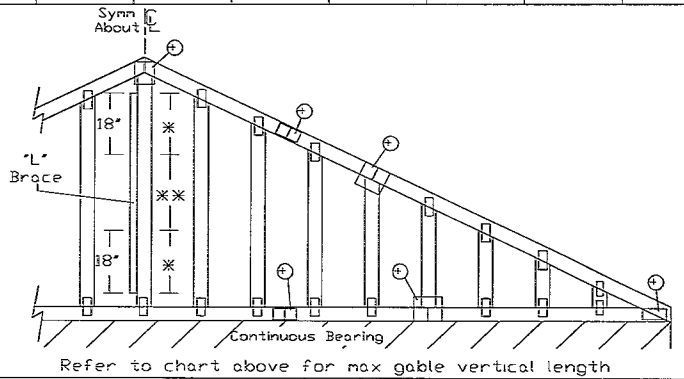
Attach "L" braces with 10d (0.128"x3.0" min) nails

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.





Refer to chart above for max gable vertical length

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail

TW Building Components Group Inc.

Earth City MO 63045

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

MAX TOT LD 60 PSF

MAX SPACING 240

REF	ASCE7-10-GAB12030
DATE	2/14/12
DRWG	A12030ENC100212

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL

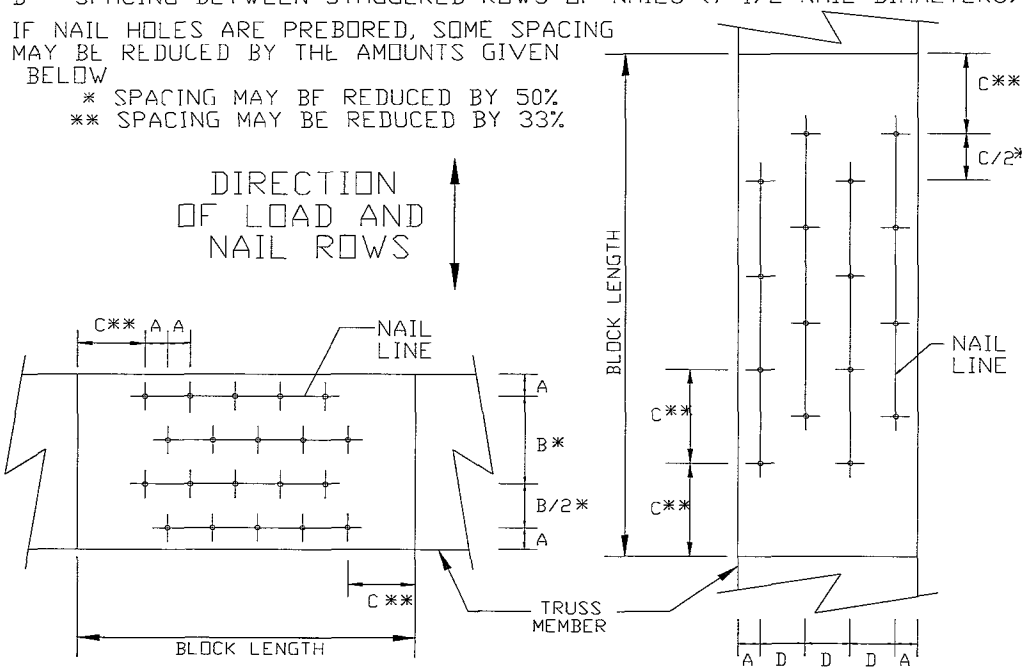
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0 113"X 2 5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0 128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0 128"X 3 25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0 135"X 3 5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0 148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0 131"X 2 5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0 148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0 148"X 3 25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0 162"X 3 5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0 120"X 2 5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0 131"X 2 5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0 120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0 131 X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

Building Components Group Inc.

Earth City MO 63045

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2
ITW-BCG: www.itwbcg.com, TPI www.tpinst.com; WTCA: www.sbcindustry.com, ICC: www.iccsafe.org

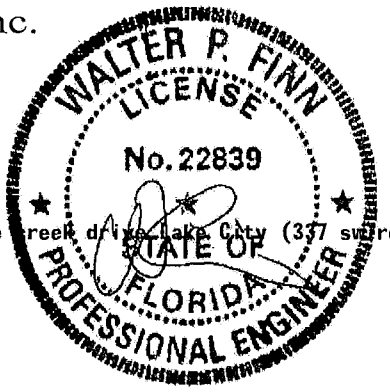
WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V21487-Z0109115902



Truss Fabricator	Anderson Truss Company
Job Identification	13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City (337 sw rose
Truss Count	16
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Com/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration Floor - 55.0 PSF @ 1.00 Duration Wind - 120 MPH ASCE 7-10 -Closed

12/09/2013

Notes

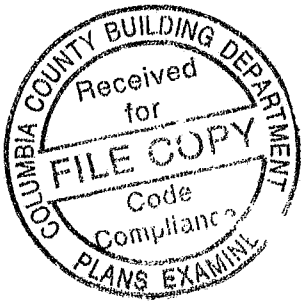
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: STRBRIBR-DEFLCAMB-RIGINSRT-

#	Ref	Description	Drawing#	Date
1	40079-F1	18'8" Floor T	13343030	12/09/13
2	40080-F2	19' Floor Tru	13343001	12/09/13
3	40081-F3	3'8" Floor Tr	13343003	12/09/13
4	40082-F4	15'8" Floor T	13343011	12/09/13
5	40083-F5	15'8" Floor T	13343013	12/09/13
6	40084-F6	15'4" Floor T	13343005	12/09/13
7	40085-F7	11'8" Floor T	13343006	12/09/13
8	40086-F8	19' Floor Tru	13343007	12/09/13
9	40087-F9	19' Floor Tru	13343010	12/09/13
10	40088-F10	18'10" Flat	13343031	12/09/13
11	40089-F11	18'10" Floor	13343012	12/09/13
12	40090--FG	18'8" Gable	13343014	12/09/13
13	40091--FG1	19' Gable	13343004	12/09/13
14	40092--FG2	11'8" Gable	13343008	12/09/13
15	40093--FG3	19' Gable	13343002	12/09/13
16	40094--FG4	10' Gable	13343009	12/09/13



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F1 18'8 Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B B2 4x2 SP M-30
Webs 4x2 SP #3-13B

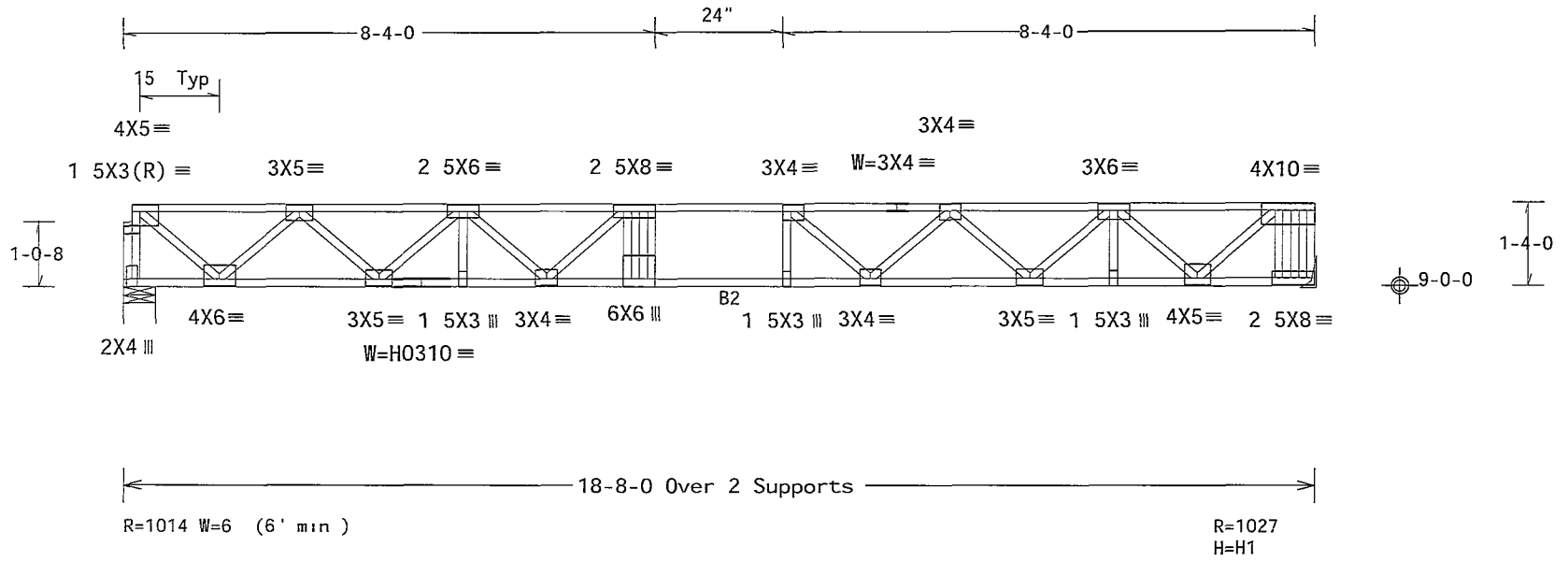
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 28" DL 0 23" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03 04 0326 13 QTY:9 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handling sh pp ng nstalling and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ceiling Locat ons shown for permanent lateral restraint of web shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handl ng sh pping nstallation bracing of trusses Apply plates to each face of truss and pos tion as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page listing th s drawing nd cates acceptance of profess onal engineering responsibility solely for the design shown The suitabil ty and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	40.0 PSF	REF	R9114- 40079
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343030
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	55.0 PSF	SEQN-	1402
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F2 19 Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B B2 4x2 SP M-30
Webs 4x2 SP #3-13B

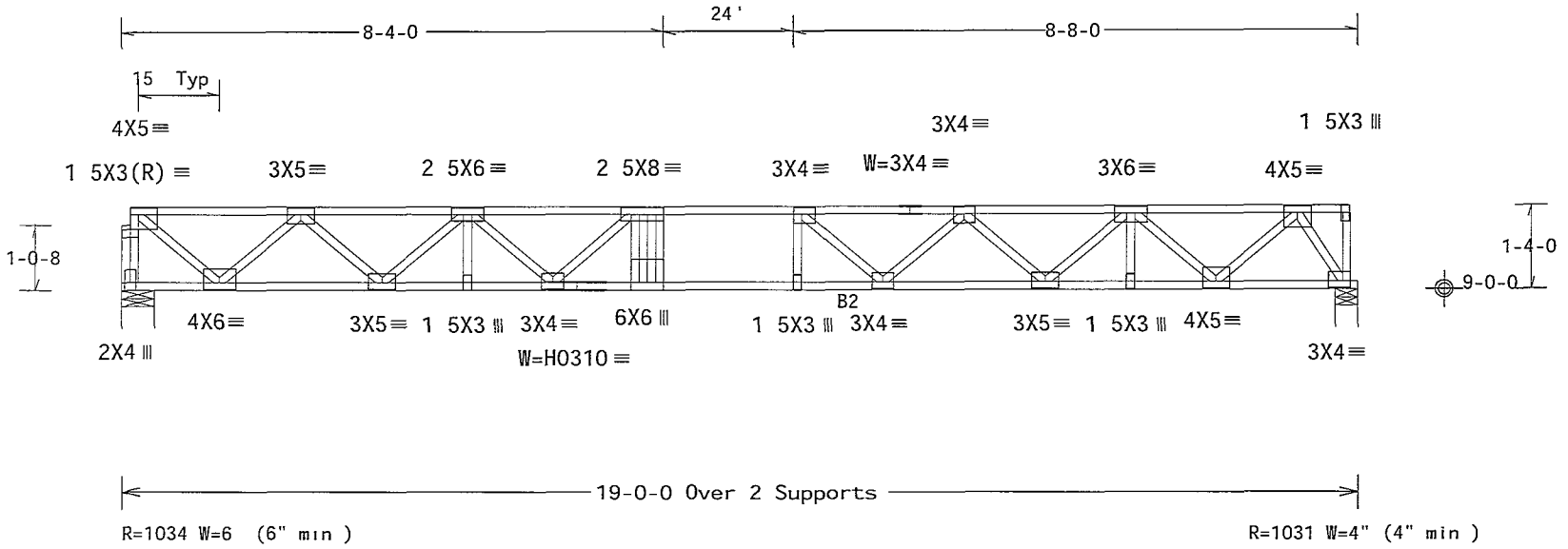
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 32" DL 0 25" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]43 QTY:4 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng shipp ng nstalling and bracing Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices prior to performing these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceil ng Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40080
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343001
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22824
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

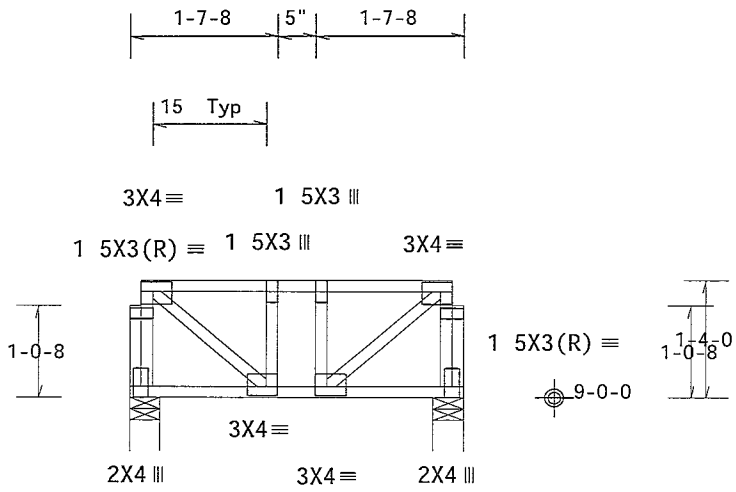
(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F3 3 8 Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up



<3-8-0 Over 2 Supports>

R=189 W=4" (4' min)

R=189 W=4' (4" min)

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:5 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40081
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343003
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD	55.0 PSF	SEQN-	22770
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F4 15'8 Floor Truss)

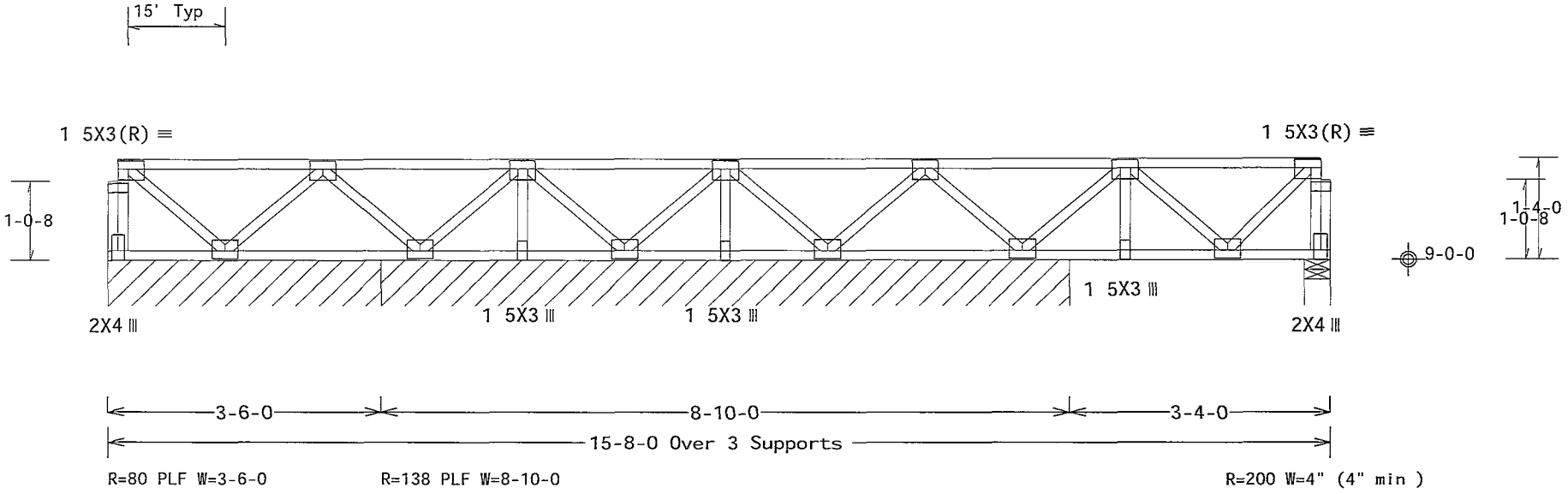
Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up



Note All Plates Are 3X4 Except As Shown.

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this desi any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Data is unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this draw ng or cover page list ng this draw ng ind cates acceptance of professional eng neering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF
TC DL	10.0 PSF
BC DL	5.0 PSF
BC LL	0.0 PSF
TOT.LD.	55.0 PSF
DUR.FAC.	1.00
SPACING	24.0"

REF	R9114- 40082
DATE	12/09/13
DRW	HCUSR9114 13343011
HC-ENG	SSB/WHK
SEQN-	22875
JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F5 15'8' Floor Truss)

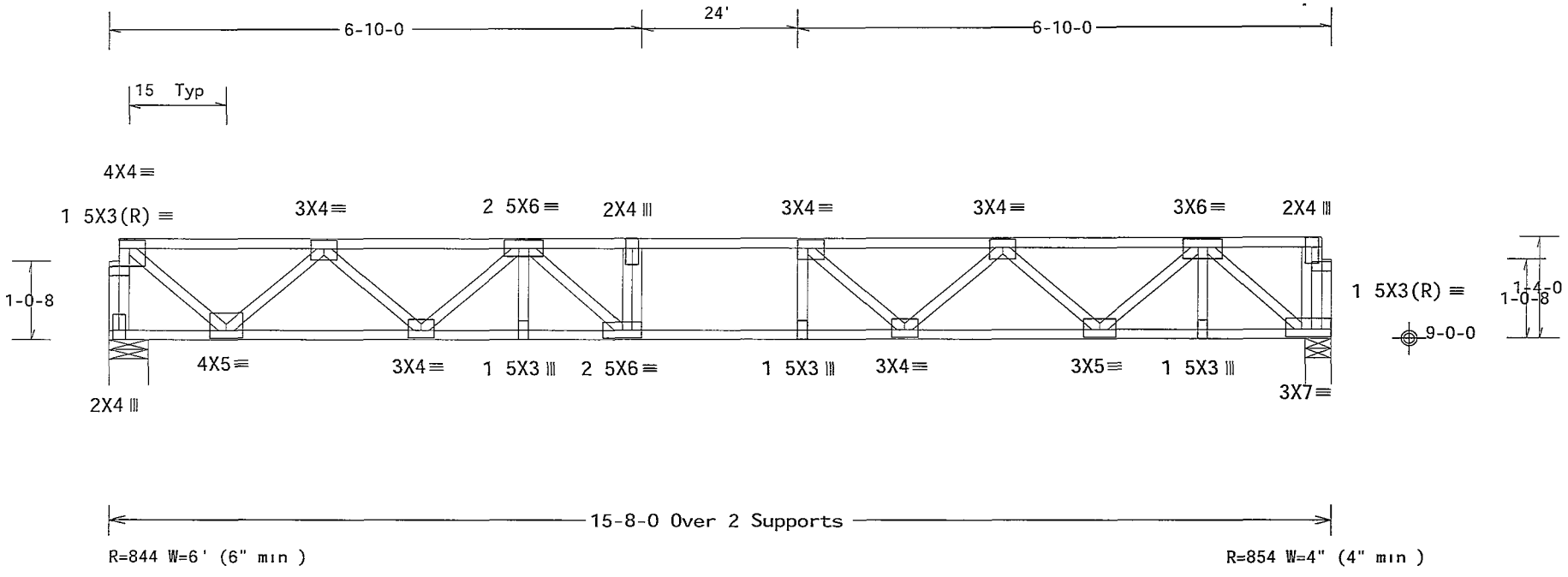
Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40083
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343013
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT LD	55.0 PSF	SEQN-	22820
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F6 15'4" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

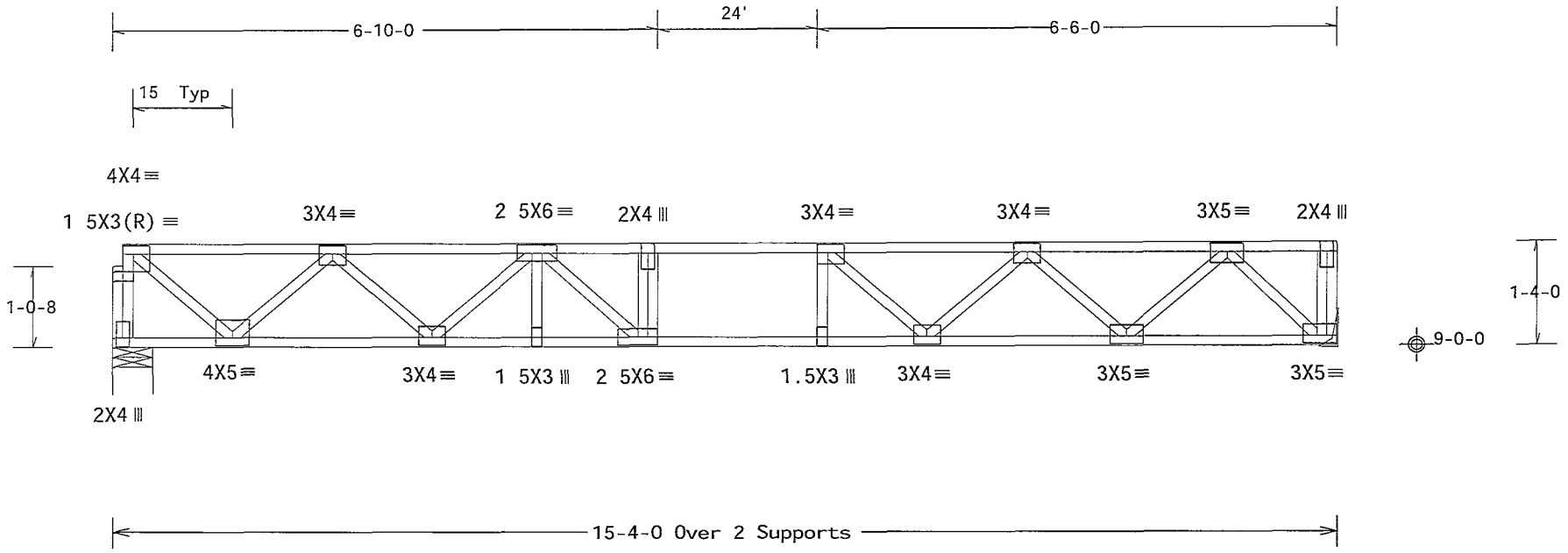
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=12%(0%)/0(0) | 12.03.04 0326 13 | QTY:5 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	40.0 PSF	REF	R9114- 40084
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343005
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22818
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F7 11'8" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP M-30
Webs 4x2 SP #3-13B

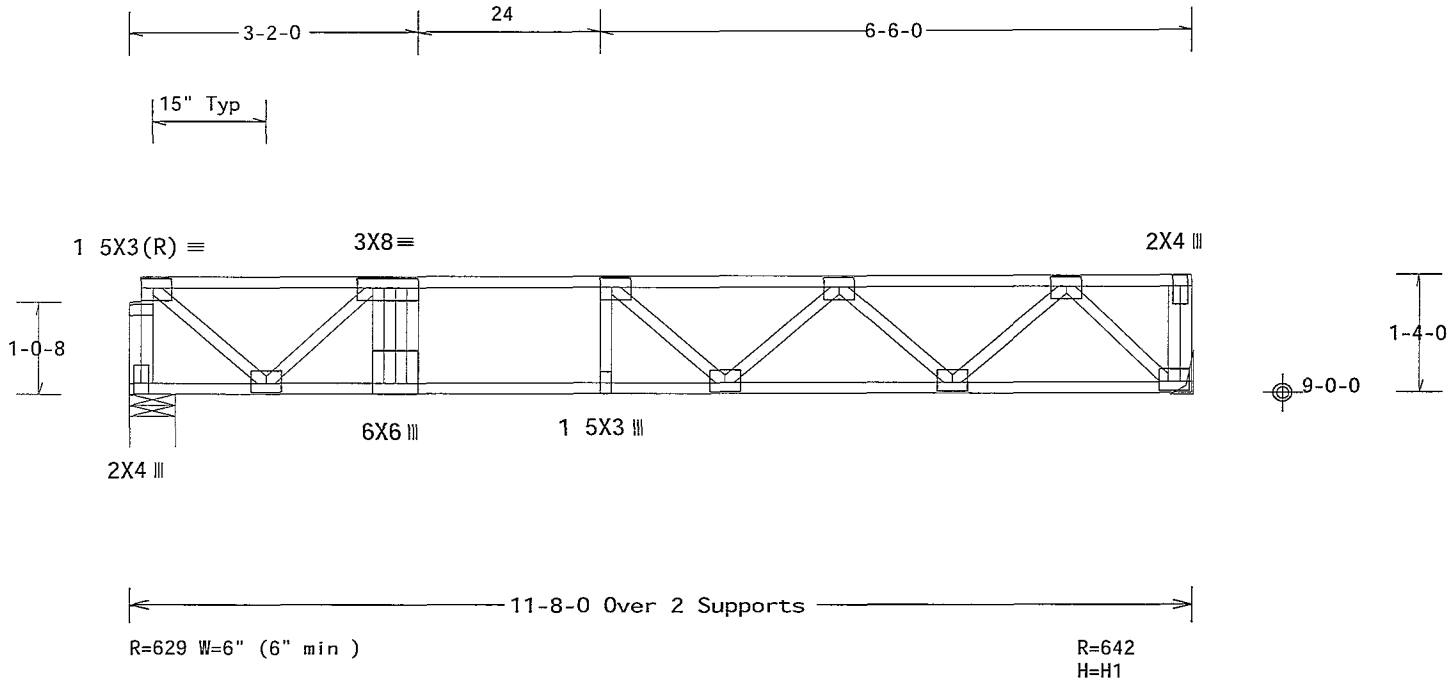
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Note All Plates Are 3X4 Except As Shown.

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 Top QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40085
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343006
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22814
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F8 19 Floor Truss Girder)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

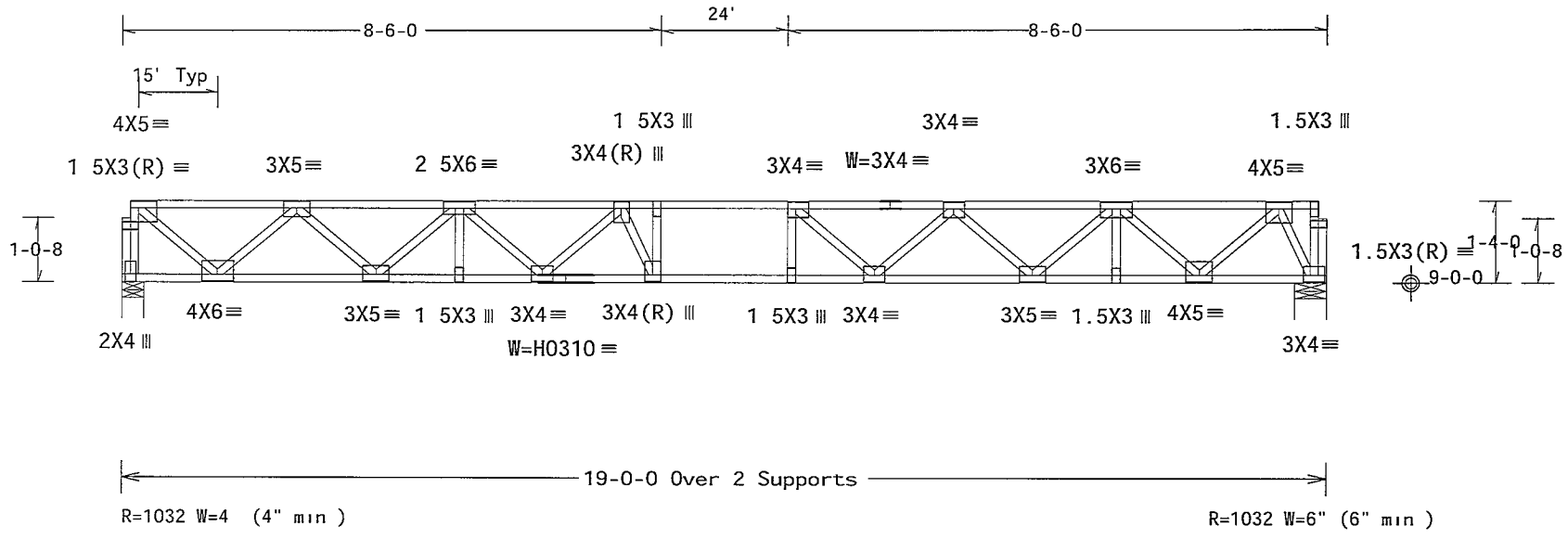
Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 31" DL 0 23" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave | FT/RT=12%(0%)/0(0) | 12.03.04 [illegible] | QTY:1 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40086
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5 0 PSF	DRW	HCUSR9114 13343007
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT LD	55 0 PSF	SEQN-	22834
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F9 19 Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

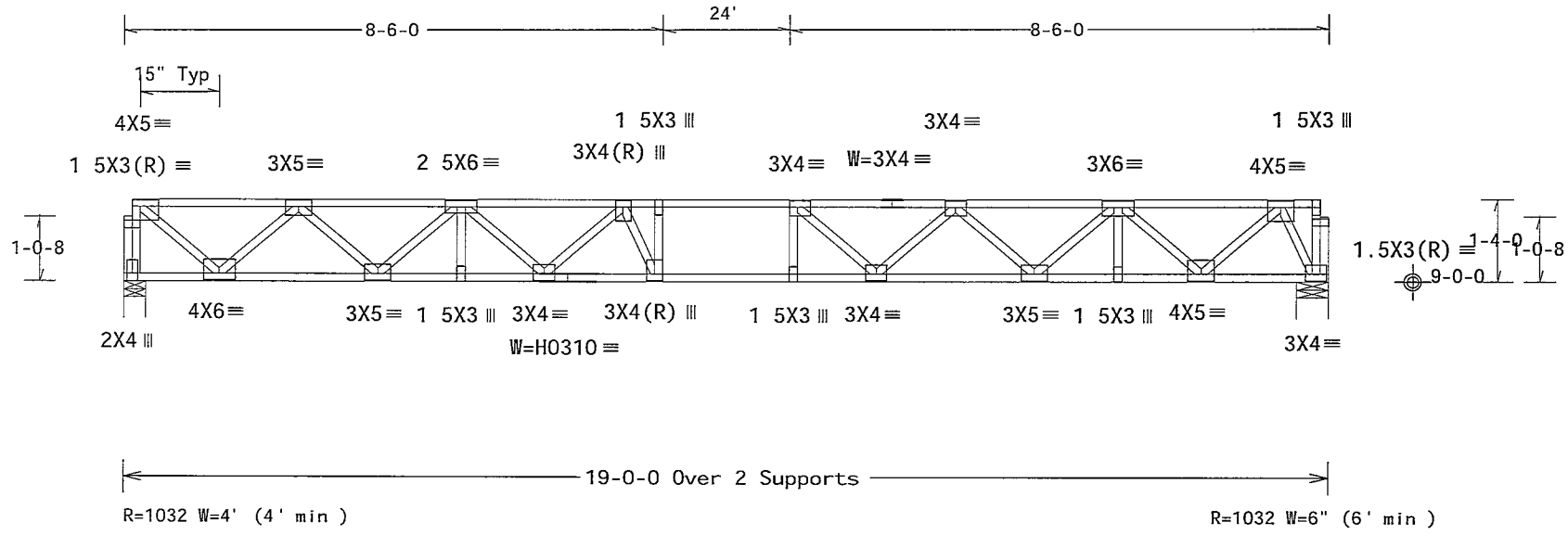
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 31' DL 0 23" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY.10 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40087
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343010
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT LD	55 0 PSF	SEQN-	22776
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F10 18 10 Flat Girder)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B W2, W14 2x4 SP #1-13B
W4, W12 2x4 SP #2-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 27 plf at 0 00 to 27 plf at 18 83
BC- From 10 plf at 0 00 to 10 plf at 18 83
TC- 1026 80 lb Conc Load at 2 15, 4 15, 6 15, 8 15 10 15,12 15,14 15,16 15,18 15

Wind loads and reactions based on MWFRS

Max JT VERT DEFL LL 0 50" DL 0 64' See detail DEFLCAMB0813 for camber recommendations Roofs incorporating this truss require consideration for ponding design by Building Designer

Truss must be installed as shown with top chord up

2 COMPLETE TRUSSES REQUIRED

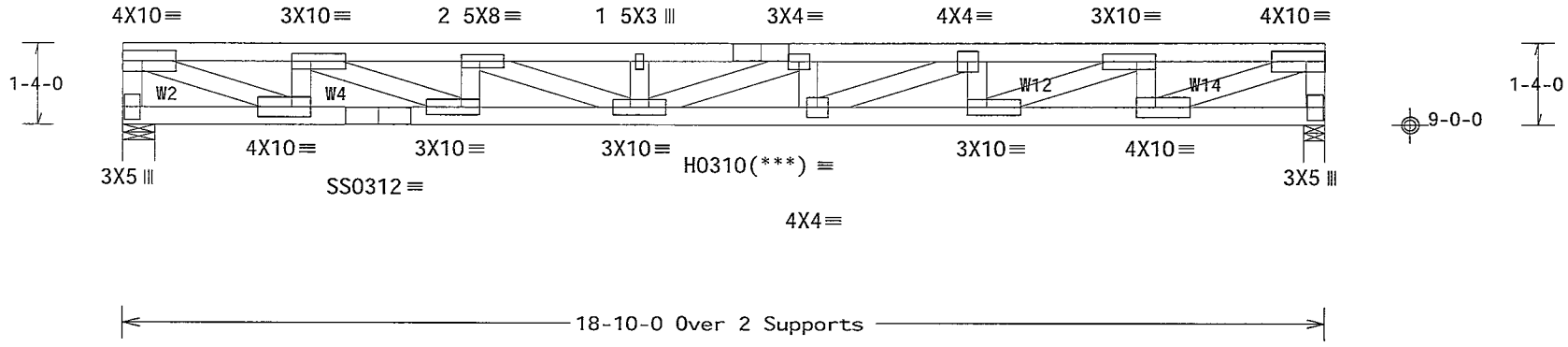
Nail Schedule 0 131 x3", min nails
Top Chord 1 Row @ 3 25" o c
Bot Chord 1 Row @12 00" o c
Webs 1 Row @ 4' o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

(***) 20 gauge Metal Plate or strap, or equivalent required to distribute reaction at joint Refer to detail RIGINSRT1210 for more information

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Calculated vertical deflection is 0 50" due to live load and 0 64" due to dead load at X = 8-1-2



PLT TYP 20 Gauge HS,18 Gauge HS, Design Crit FBC2010Com/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0) 12.03.04 0326 13 QTY.1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses requ re extreme care in fabr cat ng handl ng shipp ng installing and bracing Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid cei l ng Locations shown for permanent lateral restra nt of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

!TW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallat on & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Data is unless noted otherwise Refer to drawings 160A-Z for standard plate posit ons A seal on this drawing or cover page listing this drawing indicates acceptance of professional eng neer ng responsibility solely for the design shown The suitability and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page !TW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	20.0 PSF	REF	R9114- 40088
TC DL	7.0 PSF	DATE	12/09/13
BC DL	10.0 PSF	DRW	HCUSR9114 13343031
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	22951
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - F11 18'10' Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

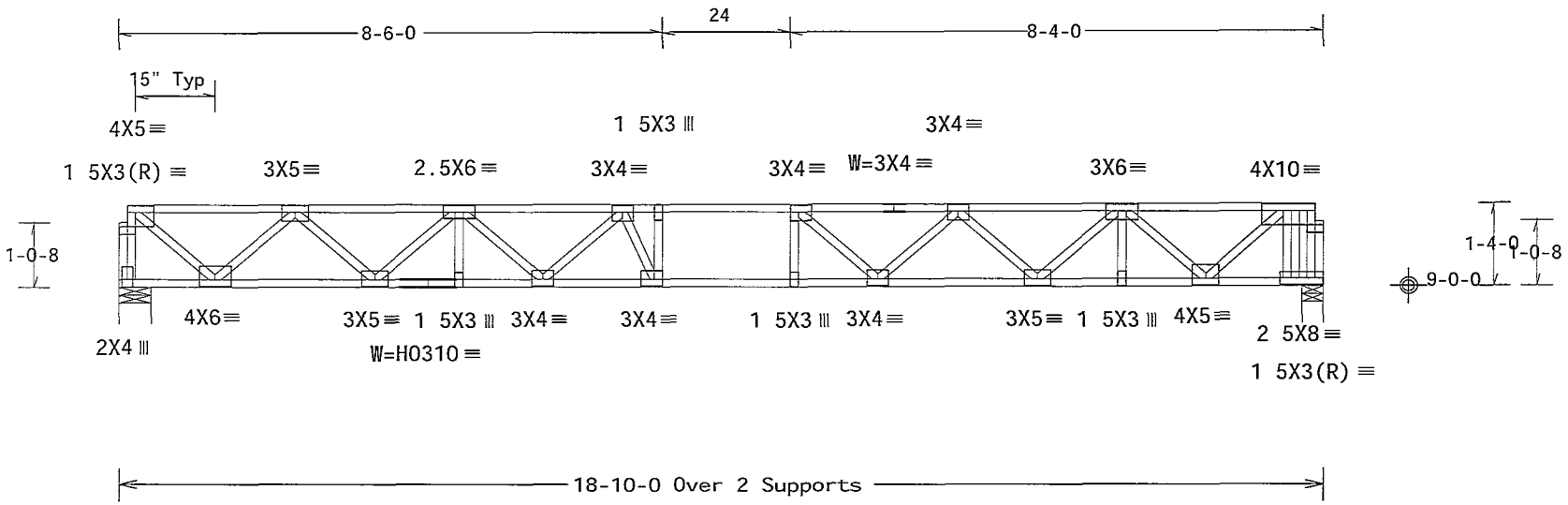
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 29" DL 0 23" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave | FT/RT=12%(0%)/0(0) | 12.03.04.0926.13 | QTY:4 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40089
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343012
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT LD.	55.0 PSF	SEQN-	22782
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - FG 18'8' Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

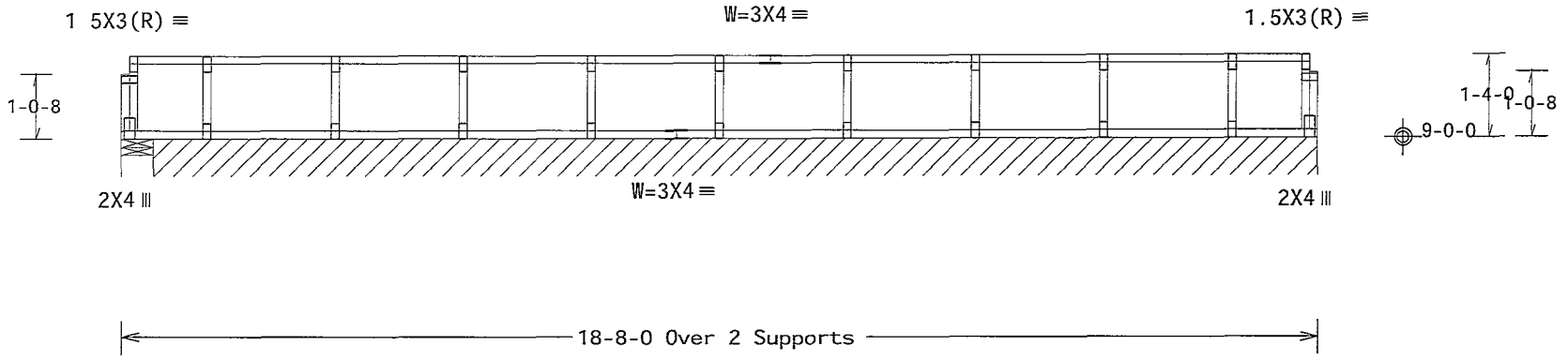
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



R=49 W=6" (6' min)
R=109 PLF W=18-2-0

Note All Plates Are 1 5X3 Except As Shown.

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12.03.04.0806.13 QTY:2 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pp ng nstallation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on th drawing or cover page listing this drawing ind cates acceptance of profess onal engineer ng respons b lity solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsib lity of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40090
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343014
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD	55.0 PSF	SEQN-	22828
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - FG1 19 Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

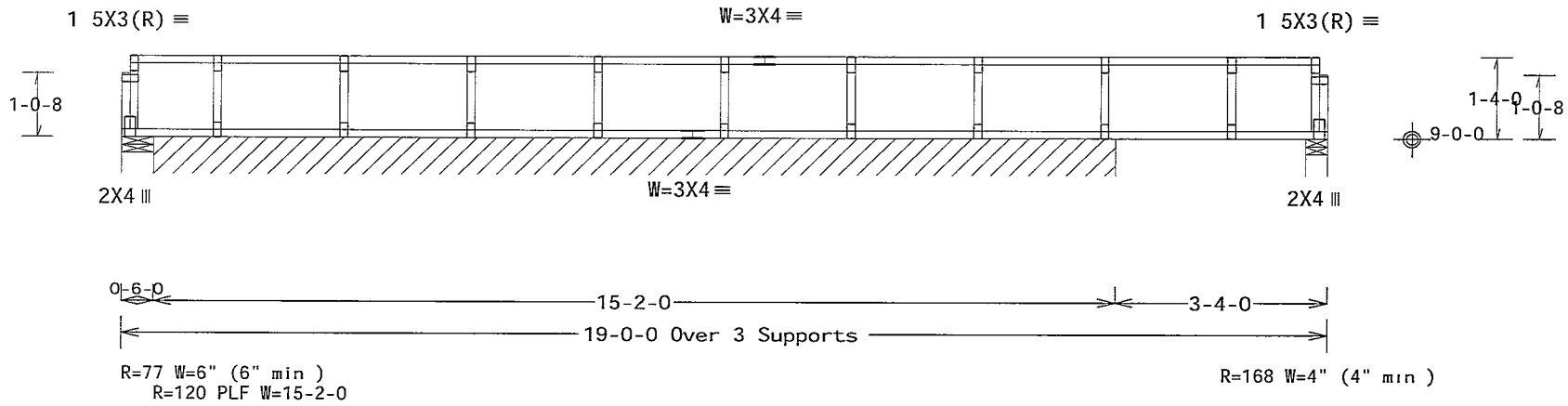
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 QRSC 13 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	40.0 PSF	REF	R9114- 40091
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343004
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22822
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - FG2 11'8" Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

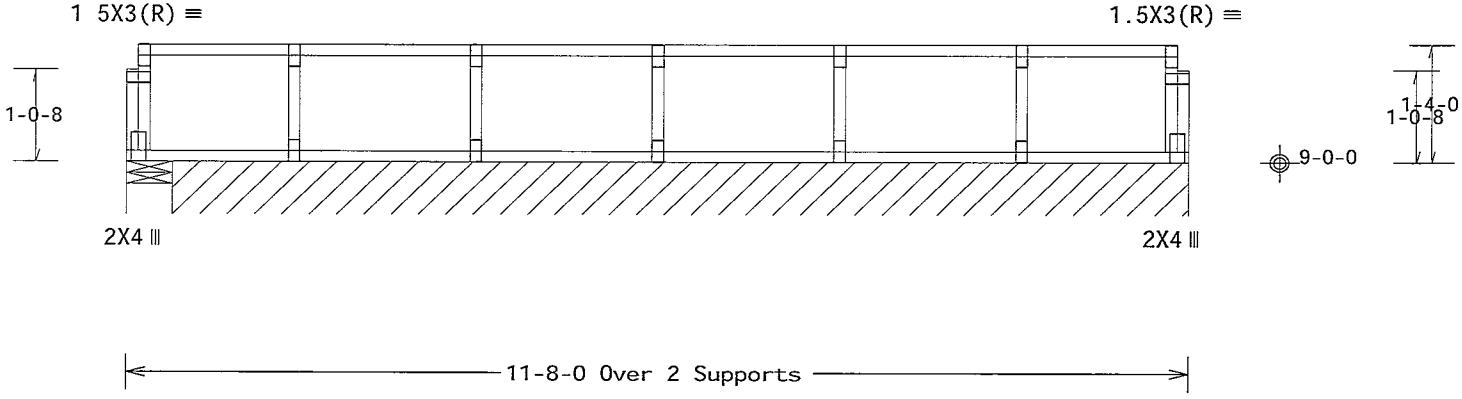
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



R=81 W=6' (6" min)
R=105 PLF W=11-2-0

Note All Plates Are 1.5X3 Except As Shown.

Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12 03.04 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/09/2013

TC LL	40.0 PSF	REF	R9114- 40092
TC DL	10.0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343008
BC LL	0 0 PSF	HC-ENG	SSB/WHK
TOT LD	55 0 PSF	SEQN-	22826
DUR.FAC.	1.00		
SPACING	24 0"	JREF-	1V21487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - FG3 19' Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

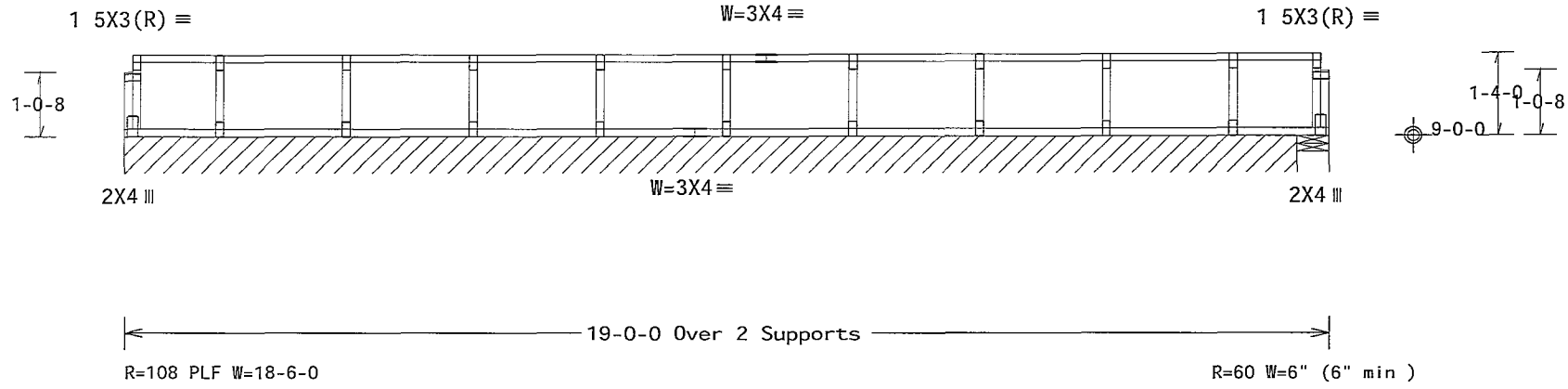
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



Note· All Plates Are 1.5X3 Except As Shown

Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=12%(0%)/0(0) | 12.03.04.0326.13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	40.0 PSF	REF	R9114- 40093
TC DL	10 0 PSF	DATE	12/09/13
BC DL	5.0 PSF	DRW	HCUSR9114 13343002
BC LL	0 0 PSF	HC-ENG	SSB/WHK
TOT LD	55 0 PSF	SEQN-	22797
DUR.FAC.	1.00		
SPACING	24 0"	JREF-	1V21487_Z01

(13-288F--Premier Building /Becker Res Floor -- 337 sw rose creek drive Lake City - FG4 10' Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

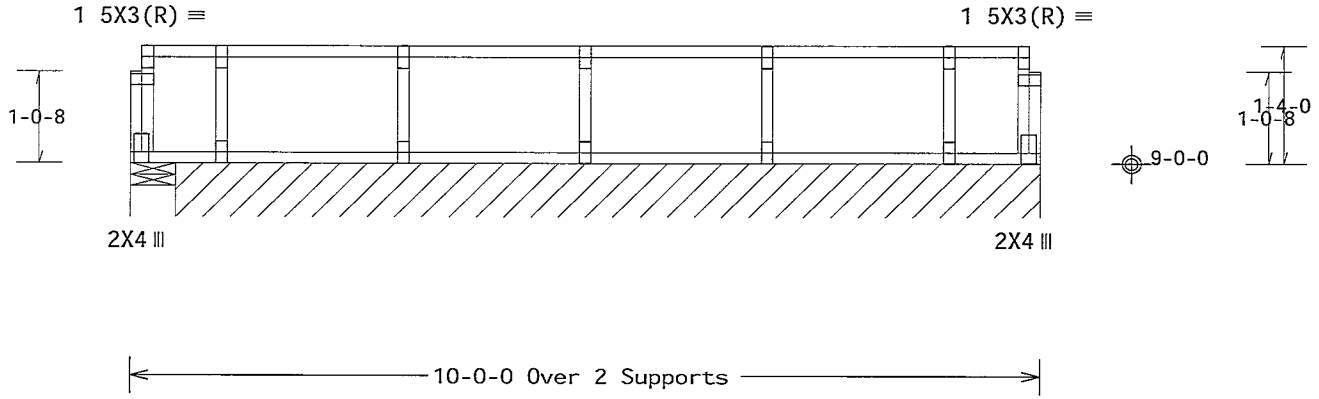
Lumber grades designated with "13B' use design values approved 1/30/2013 by ALSC

Fasten rated sheathing to one face of this frame

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer



R=21 W=6" (6" min)
R=111 PLF W=9-6-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12 03 04 0326 13 QTY.1 FL/-/4/-/-/R/- Scale = 5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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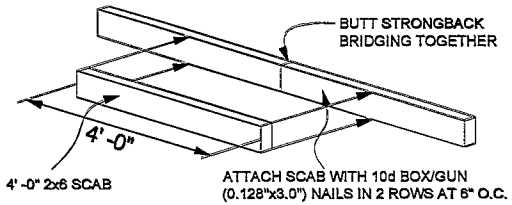
WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/09/2013

TC LL	40 0 PSF	REF	R9114- 40094
TC DL	10 0 PSF	DATE	12/09/13
BC DL	5 0 PSF	DRW	HCUSR9114 13343009
BC LL	0 0 PSF	HC-ENG	SSB/WHK
TOT LD	55.0 PSF	SEQN-	22792
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V21487_Z01

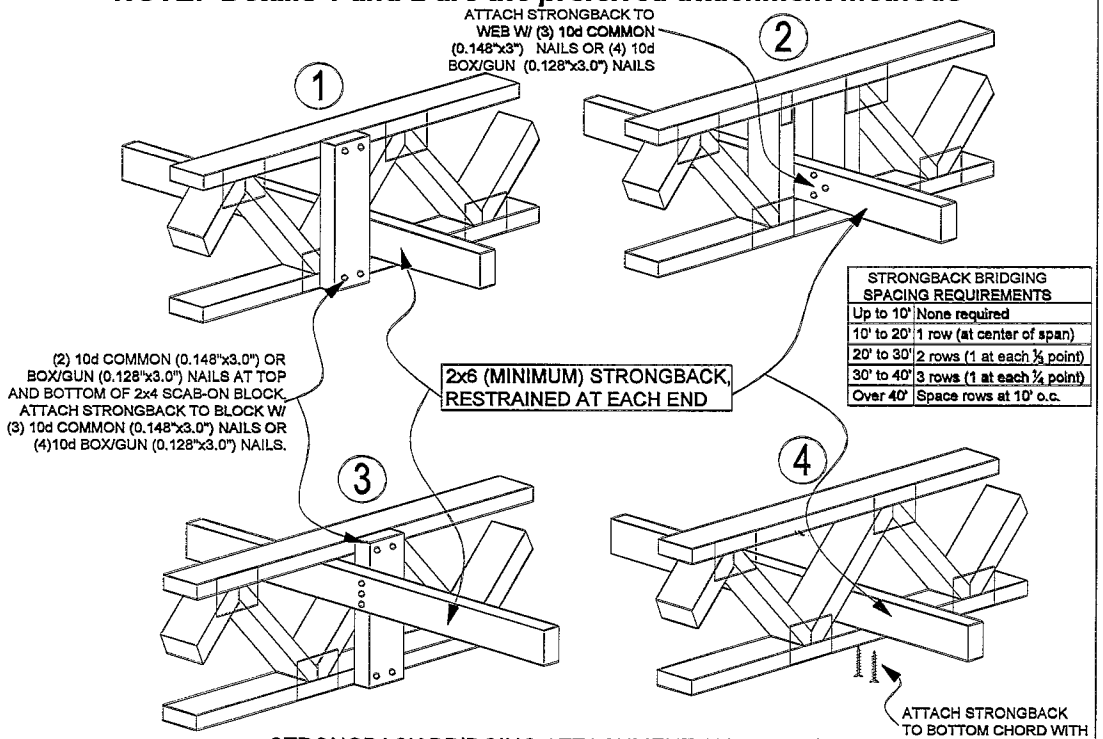
STRONGBACK BRIDGING RECOMMENDATIONS



NOTE: IN LIEU OF SPLICING AS SHOWN LAP STRONGBACK BRIDGING MEMBERS FOR AT LEAST ONE TRUSS SPACING

STRONGBACK BRIDGING SPLICE DETAIL

NOTE: Details 1 and 2 are the preferred attachment methods



STRONGBACK BRIDGING SPACING REQUIREMENTS	
Up to 10'	None required
10' to 20'	1 row (at center of span)
20' to 30'	2 rows (1 at each ⅓ point)
30' to 40'	3 rows (1 at each ¼ point)
Over 40'	Space rows at 10' o.c.

STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES

- All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by ITW Building Compents Group Inc.

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

ITW Building Components Group Inc.

Earth City MO 63045

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	PSF	REF STRONGBACK
TC DL	PSF	DATE 2/28/11
BC DL	PSF	DRWG STRBRIBR0211
BC LL	PSF	
TOT LD	PSF	
DUR FAC	1 00	
SPACING		

Commentary Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads Providng camber has the following advantages

- Helps to ensure level ceilings and floors after dead loads are applied
- Facilitates drainage to avoid ponding on flat or low slope roofs
- Compensates for different deflection characteristics between adjacent trusses
- Improves appearance of garage door headers and other long spans that can appear to "sag"
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads

The amount of camber is dependent on the truss type, span, loading, application, etceteras

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration

The following tables are provided as guidelines for limiting deflection and estimating camber Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0 75" (horizontal)	1 25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1 00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1 5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0 25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0 25 x Deflection from Live Load) + (1 5 x Design Dead Load Deflection)

Note The actual dead load may be considerably less than the design dead load

TW
Building Components Group Inc.

Earth City MO 63045

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible or any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites.
ITWBCG: www.itwbcg.com, TPI: www.tpinst.org, SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 13

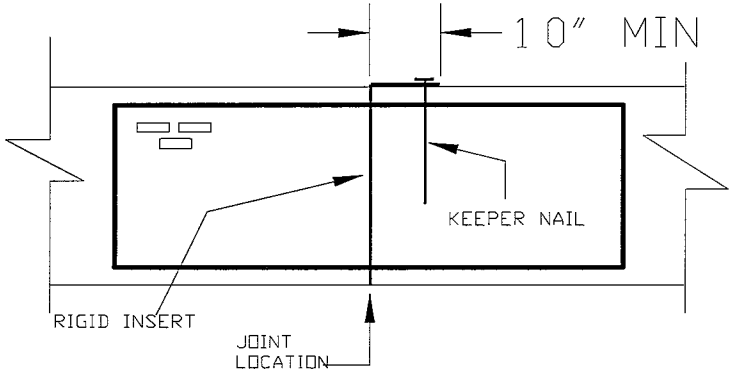
REF	DEFLEC/CAMB
DATE	8/2/13
DRWG	DEFLCAMB0813

RIGID INSERT DETAIL - REINFORCEMENT FOR HIGH STRESS COMPRESSION JOINTS

THIS DETAIL TO BE USED WHEN THE STRESS AT A COMPRESSION SPLICE EXCEEDS 75% OF THE ALLOWABLE COMPRESSION STRESS PER TPI 1 SECTION 7392

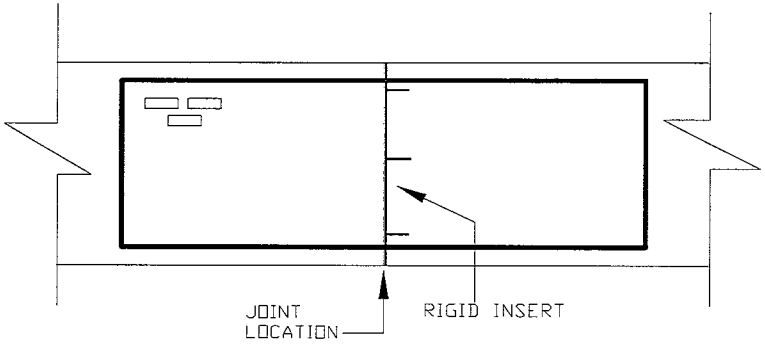
OPTION #1

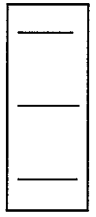
APPLY A 20 GAGE MINIMUM METAL INSERT BETWEEN BUTTED ENDS OF COMPRESSION CHORD MEMBERS TO FULLY COVER THE JOINT BEARING AREA BEND RIGID INSERT OVER THE TOP OR BOTTOM OF THE COMPRESSION MEMBER A MINIMUM OF 1" AND SECURE IN PLACE WITH A KEEPER NAIL KEEPER NAIL IS TO BE SIZED AND SPACED TO AVOID SPLITTING OF THE LUMBER



OPTION #2

APPLY A 20 GAGE MINIMUM METAL INSERT WITH SLOTTED TEETH BETWEEN BUTTED ENDS OF COMPRESSION CHORD MEMBERS TO FULLY COVER THE JOINT BEARING AREA HAMMER RIGID INSERT SECURELY IN PLACE AND FLUSH WITH BUTTED END





RIGID INSERT WITH A MAXIMUM OF 5% SLOTTED (VOID) AREA

ITW Building Components Group Inc.

Earth City MO 63045

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/V/H,S) galv steel Apply plates to each face of truss positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW BCG: www.itwbcg.com, TPI www.tpinst.com WTCA: www.sbcindustry.com, ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

13

TC LL	PSF	REF RIGID INSERT
TC DL	PSF	DATE 12/15/10
BC DL	PSF	DRWG RIGINSRT1210
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

12/09/2013

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V5L487-Z0116130613

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator	Anderson Truss Company
Job Identification	13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive, Lake City, (337 sw rose
Truss Count	15
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Com/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record, Address	The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - N/A Floor - 55.0 PSF @ 1.00 Duration Wind - No Wind

04/16/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: STRBRIBR-DEFLCAMB-CNSY42PL-LSCSYX2A-

Revision

#	Ref	Description	Drawing#	Date
1	94480-F1	18'11" Floor	14106012	04/16/14
2	94481-F2	18'11" Floor	14106014	04/16/14
3	94482-F3	3'5"4 Floor T	14106011	04/16/14
4	94483-F3	3'5"4 Floor T	14106005	04/16/14
5	94484-F4	15'6"4 Floor	14106004	04/16/14
6	94485-F5	15'4" Floor T	14106006	04/16/14
7	94486-F7	11'8" Floor T	14106008	04/16/14
8	94487-F9	19' Floor Tru	14106009	04/16/14
9	94488-F10	18'10" Flat	14106002	04/16/14
10	94489--FG	18'11" Gable	14106013	04/16/14
11	94490--FG1	15'4" Gable	14106001	04/16/14
12	94491--FG2	11'8" Gable	14106010	04/16/14
13	94492--FG3	19' Gable	14106003	04/16/14
14	94493--FG4	10' Gable	14106007	04/16/14
15	94494-F8	19' Floor Tru	14106006	04/16/14

#31678

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F1 18'11" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

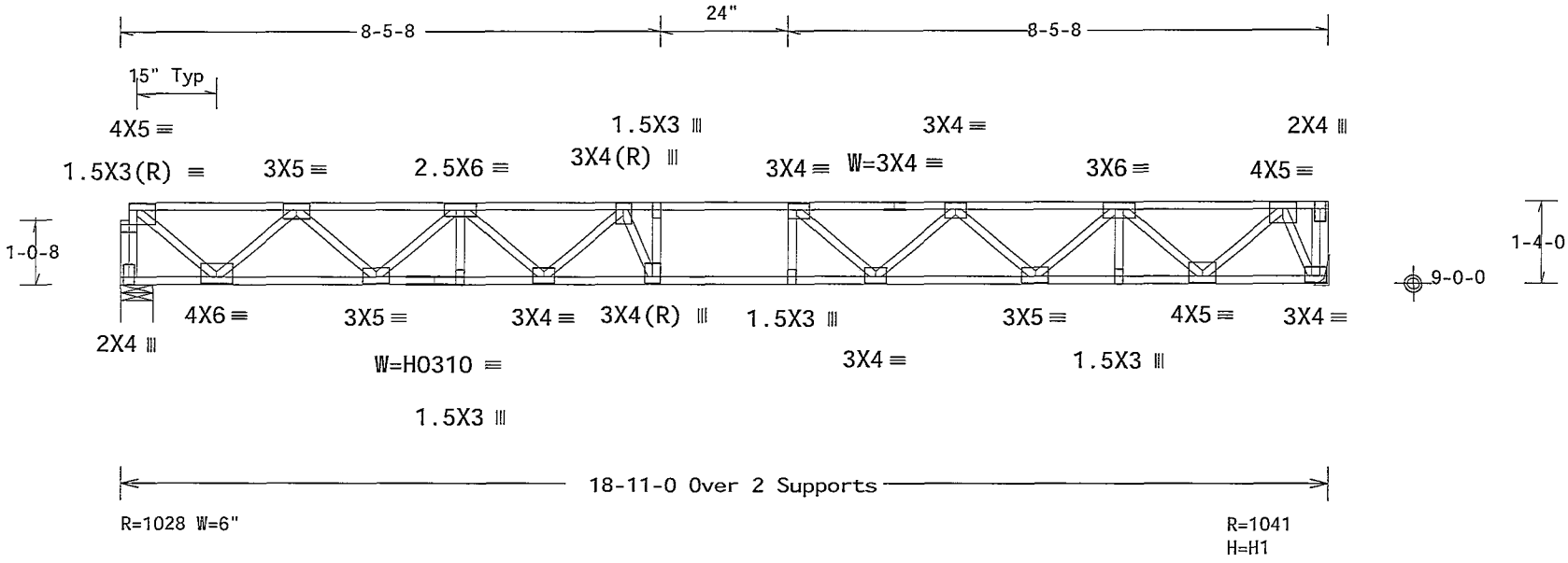
Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 31" DL 0 23" See detail DEFLCAMB0813 for camber recommendations

(J) Hanger Support Required, by others

Truss must be installed as shown with top chord up



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]44 QTY:9 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94480
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106012
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	30801
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F2 18'11" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

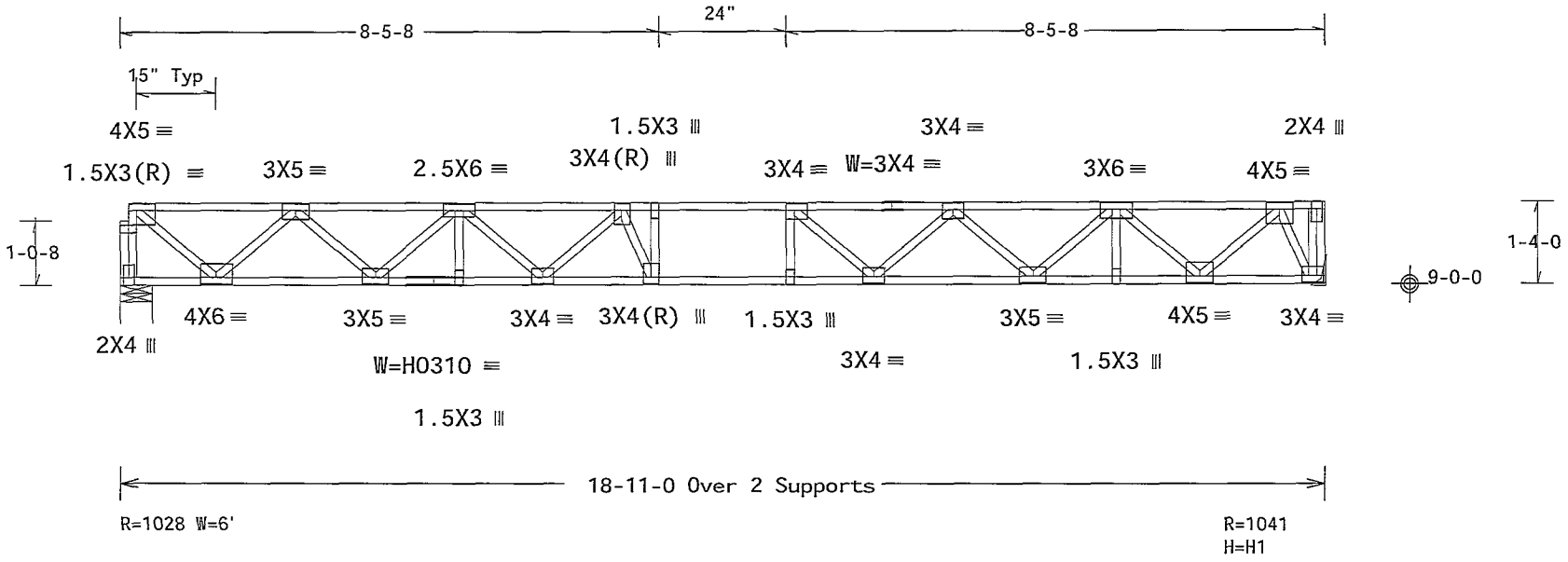
Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL LL 0 31" DL 0 23" See detail DEFLCAMB0813 for camber recommendations

(J) Hanger Support Required, by others

Truss must be installed as shown with top chord up



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:4 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94481
TC DL	10.0 PSF	DATE	04/16/14
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BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	30805
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F3 3'5"4 Floor Truss)

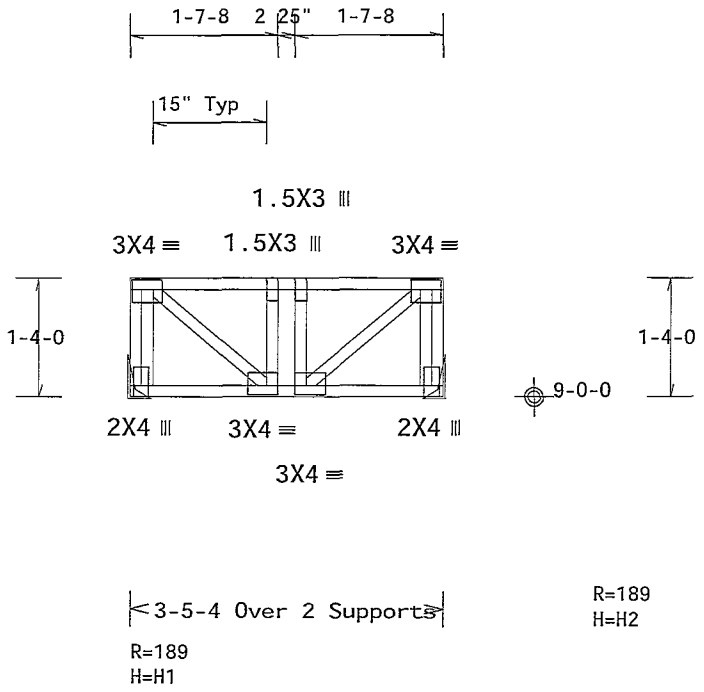
Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(J) Hanger Support Required, by others
(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 Rev 1 QTY:5 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94482
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106011
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	30802
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F3 3'5"4 Floor Truss)

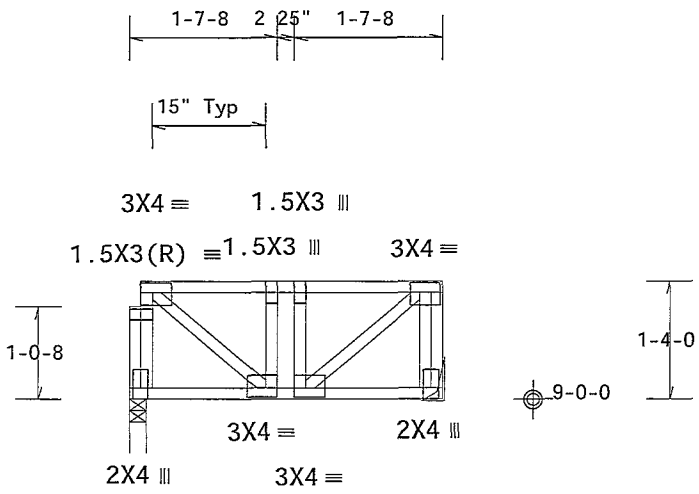
Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave | FT/RT=12%(0%)/0(0) | 12.03.04 Rev1 | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40 0 PSF	REF	R9114- 94483
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106005
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	30806
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F4 15'6"4 Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

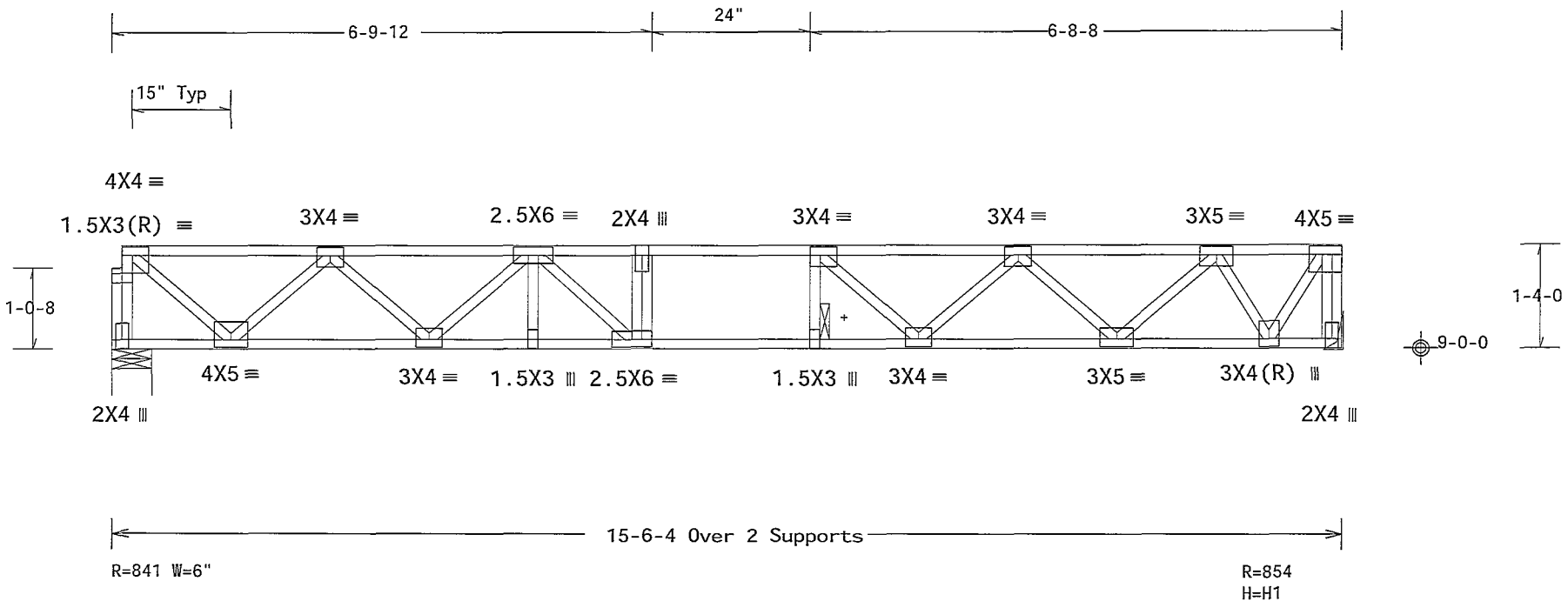
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

+ 2x6 continuous strongback See detail STRBRIBR0211 for bracing and bridging recommendations

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:5 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94484
TC DL	10.0 PSF	DATE	04/16/14
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BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	76390
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F5 15'4" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

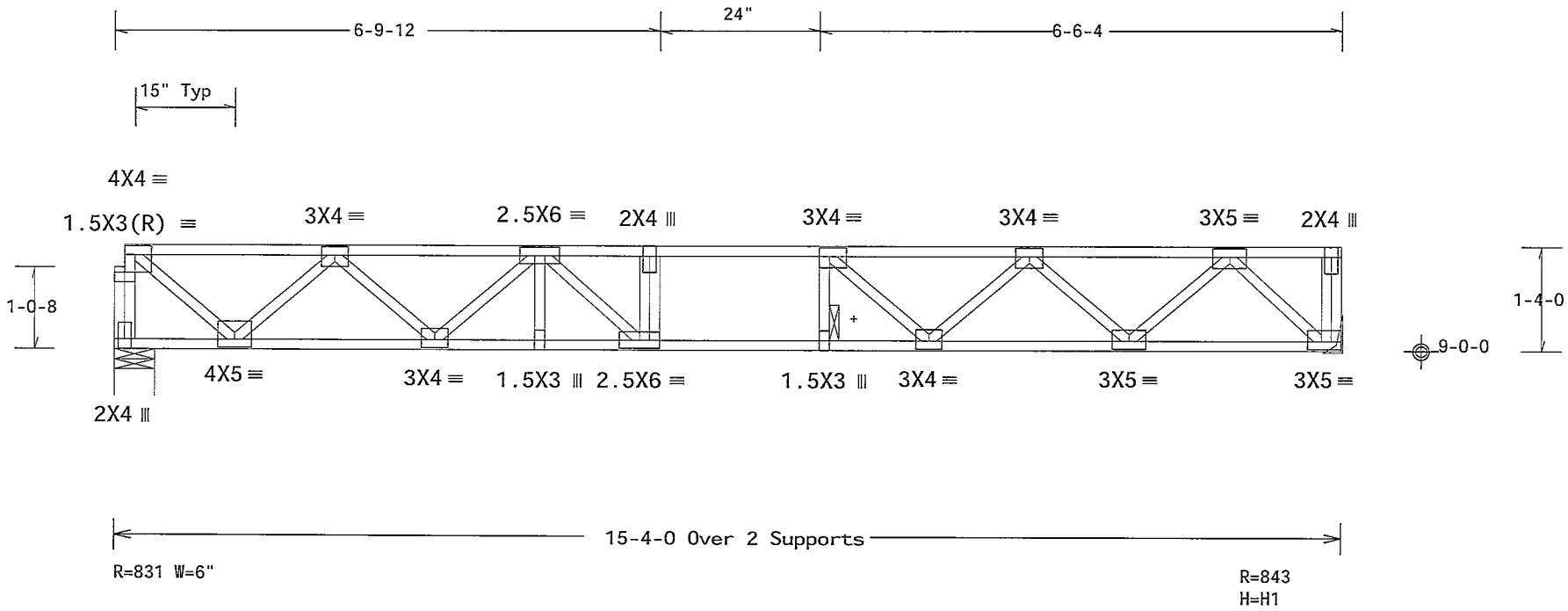
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

+ 2x6 continuous strongback See detail STRBRIBR0211 for bracing and bridging recommendations

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



PLT TYP. Wave

Design Crit: FBC2010Com/TPI-2007(STD)
FT/RT=12%(0%)/0(0) 12.03.04 [illegible] QTY:6 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this desi any failure to build the truss in conformance w th ANSI/TPI 1 or for handling shipping nstallation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94485
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106006
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	55.0 PSF	SEQN-	76389
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F7 11'8" Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

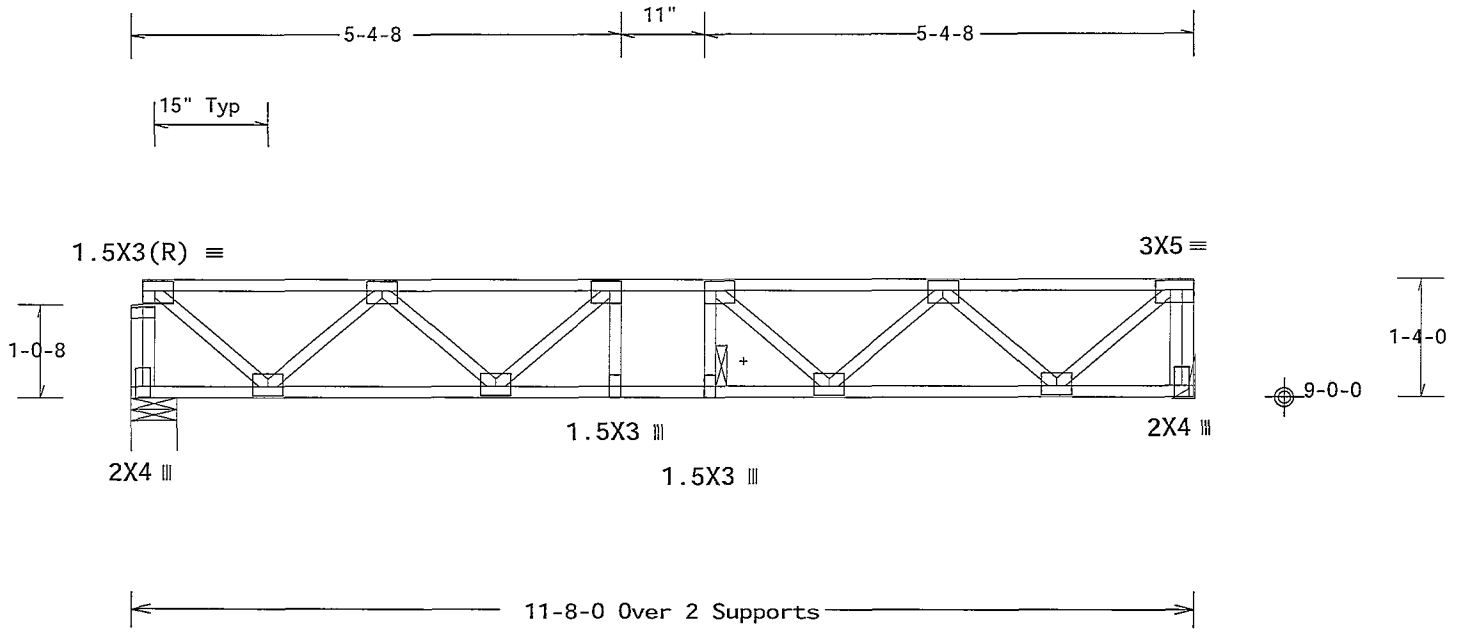
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

+ 2x6 continuous strongback See detail STRBRIBR0211 for bracing and bridging recommendations.

(J) Hanger Support Required, by others

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]4 QTY:3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabr cating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sections B3 B7 or B10 as applicable

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94486
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106008
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	76388
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F9 19' Floor Truss)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

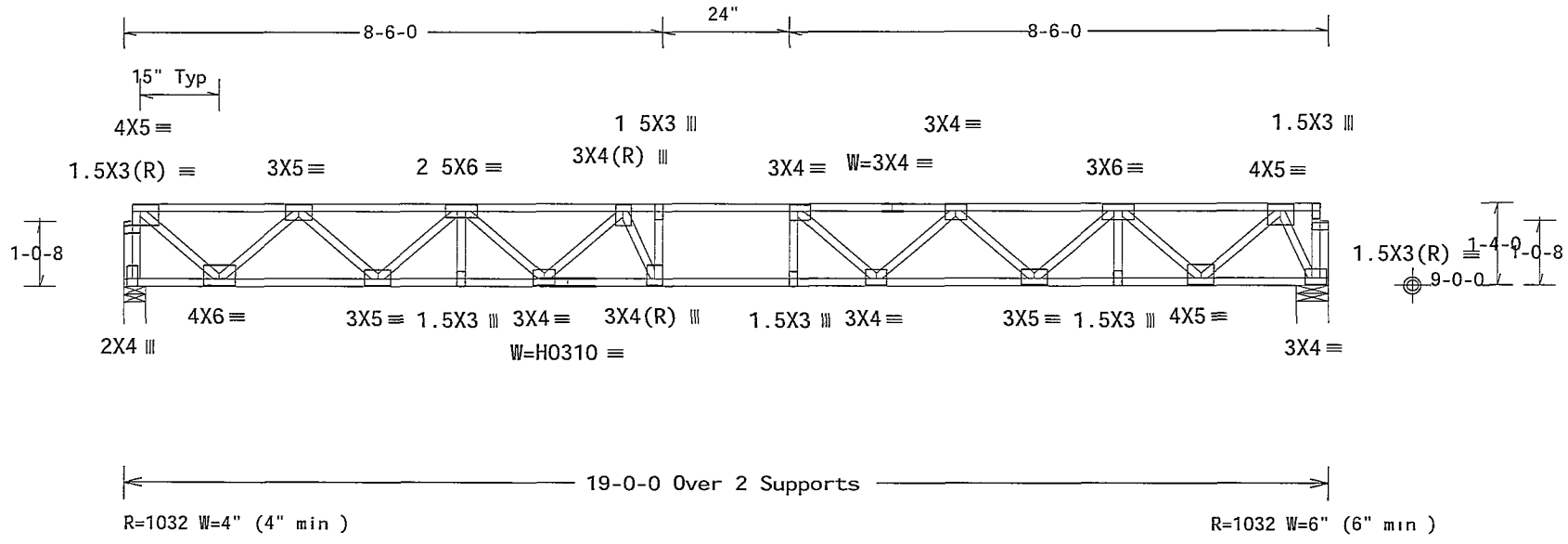
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss must be installed as shown with top chord up

See detail STRBRIBR0211 for bracing and bridging recommendations

Max JT VERT DEFL: LL 0.31" DL 0.23" See detail DEFLCAMB0813 for camber recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1.50



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave | FT/RT=12%(0%)/0(0) | 12.03.04 | QTY:14 FL/-/5/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94487
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106009
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22776
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F10 18'10" Flat Girder)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B ·W2, W16 2x4 SP #1-13B·
W4, W14 2x4 SP #2-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

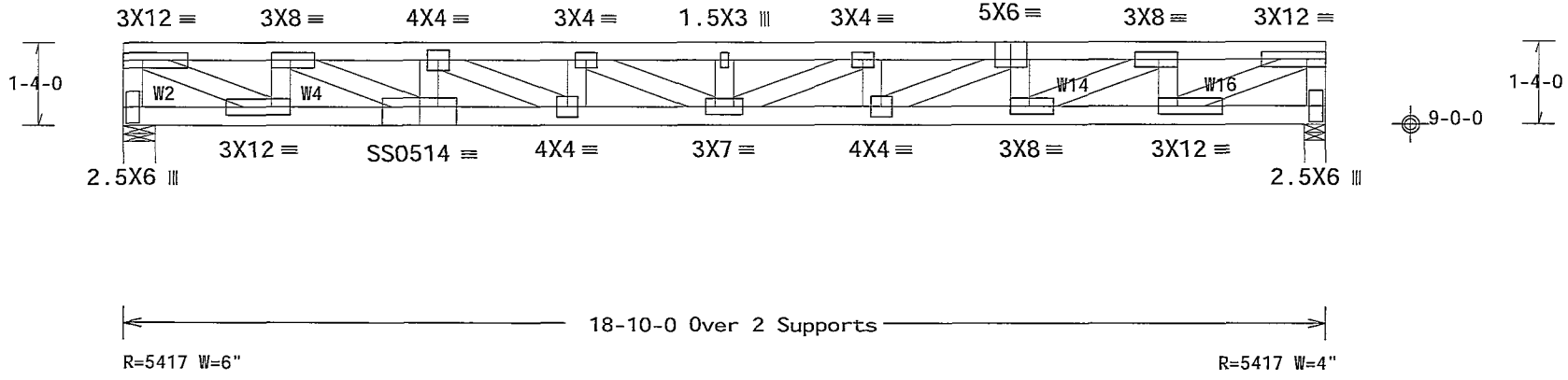
Girder supports 18-11-0 span to TC/BC split one face and 2-0-0 span framing to the TC/BC split opposite face

3 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @ 3 75" o c
Bot Chord 1 Row @12 00" o c
Webs 1 Row @ 4" o c
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting

Truss must be installed as shown with top chord up

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]44 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94488
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	76448
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - FG 18'11" Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

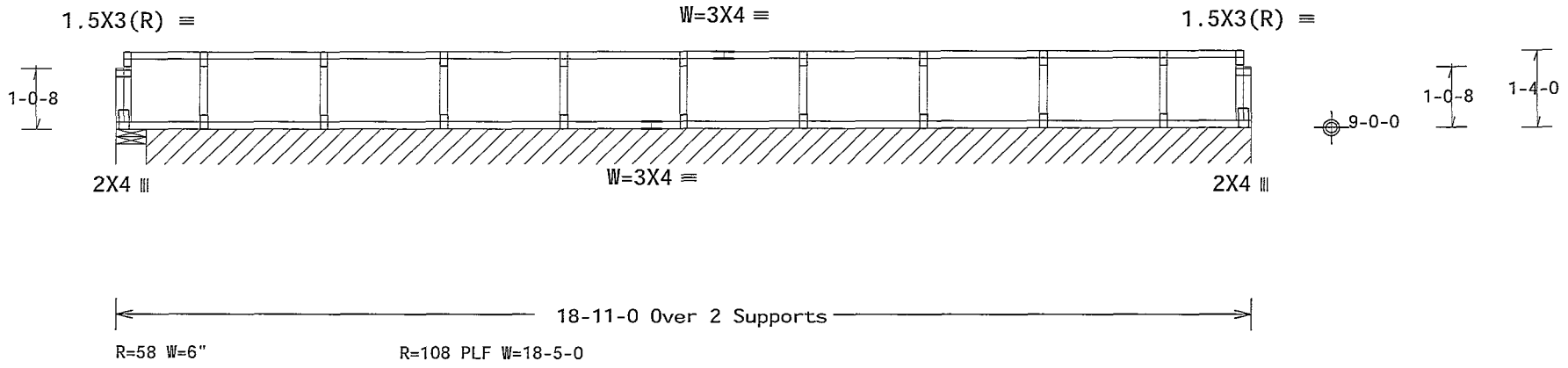
Truss designed for vertical in plane loads only.

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]44 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94489
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106013
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	30808
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - FG1 15'4" Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

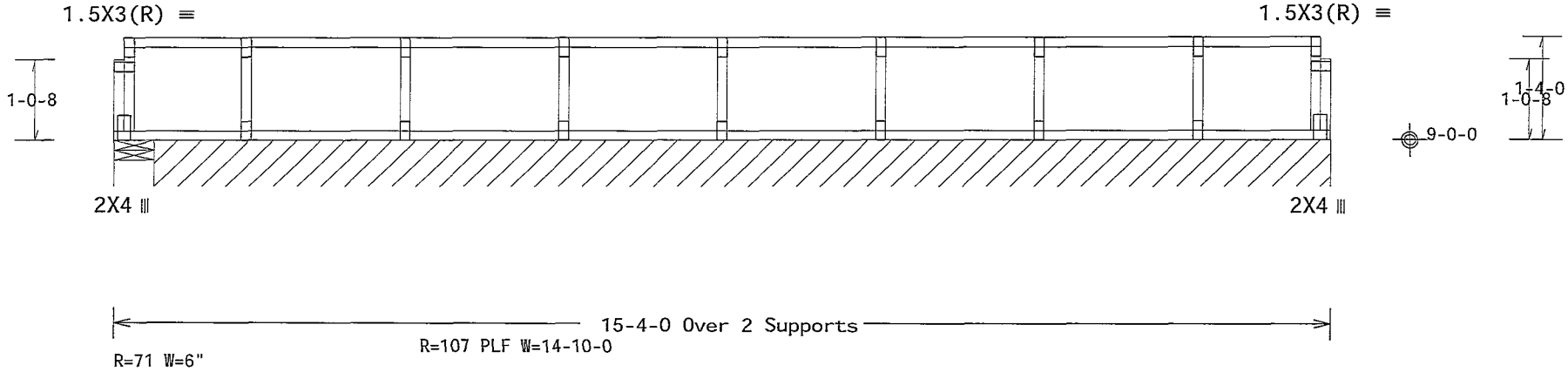
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



Note· All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94490
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	55.0 PSF	SEQN-	29602
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - FG2 11'8" Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

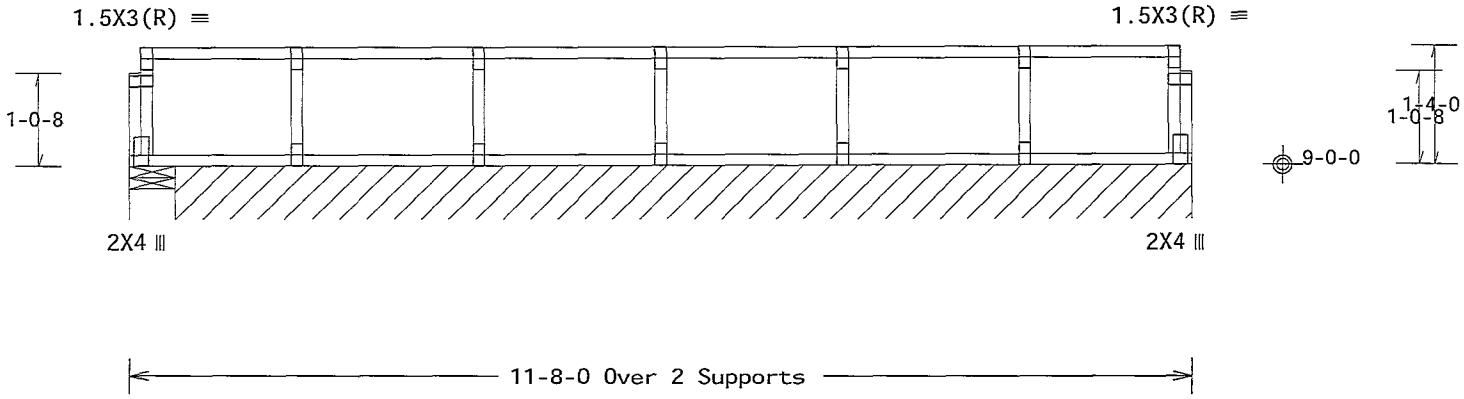
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50.

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



R=81 W=6" (6" min)
R=105 PLF W=11-2-0

Note· All Plates Are 1 5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]43 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94491
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106010
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22826
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - FG3 19' Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

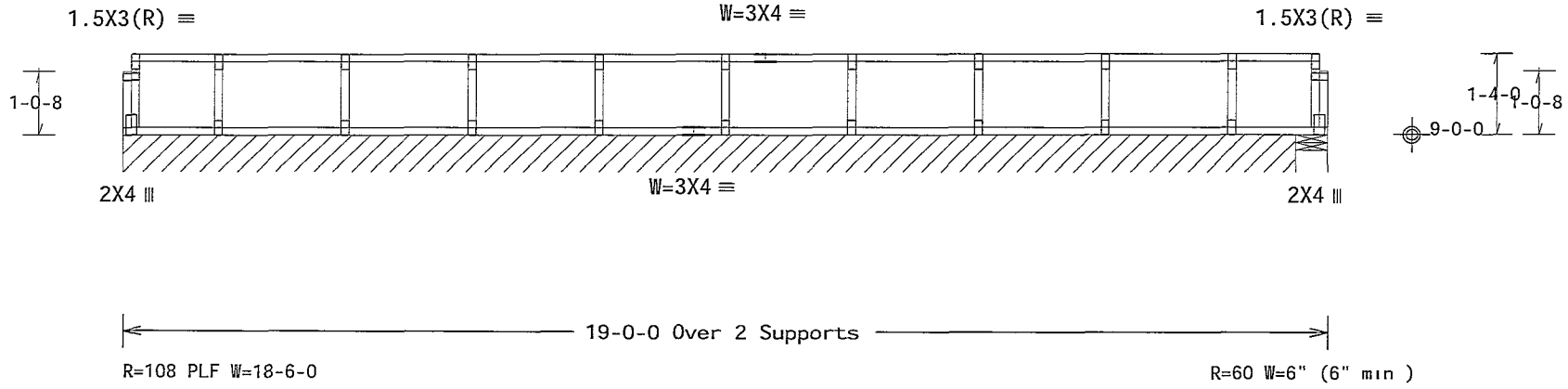
Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer

See detail STRBRIBR0211 for bracing and bridging recommendations

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Fasten rated sheathing to one face of this frame



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94492
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106003
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22797
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - FG4 10' Gable)

Top chord 4x2 SP #1-13B
Bot chord 4x2 SP #1-13B
Webs 4x2 SP #3-13B

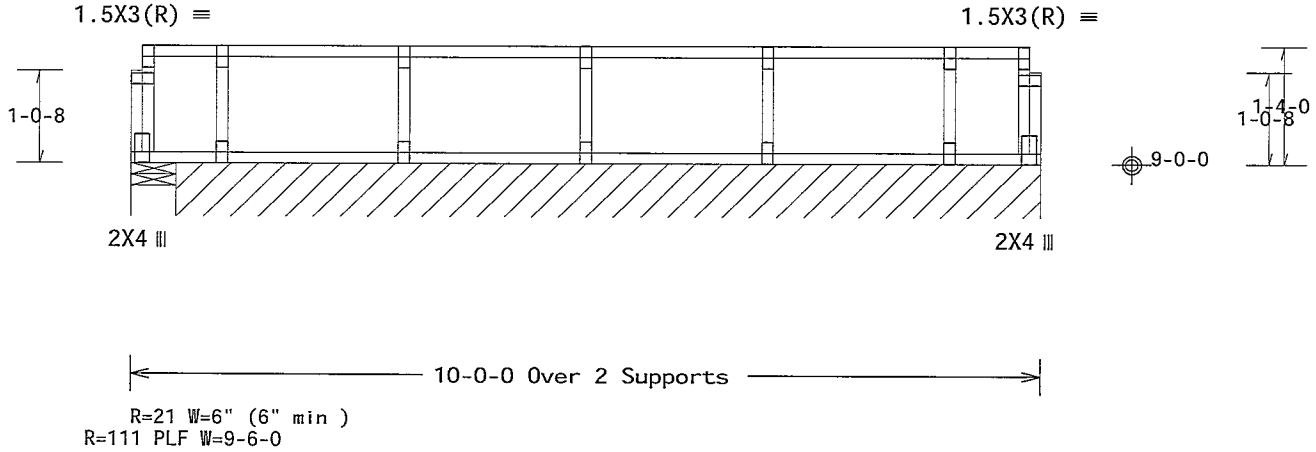
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Fasten rated sheathing to one face of this frame

Deflection meets L/360 live and L/360 total load Creep increase factor for dead load is 1 50

Truss must be installed as shown with top chord up

Sheathing is required for any longitudinal(drag) forces All connections to be designed by the building designer



Note: All Plates Are 1 5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Com/TPI-2007(STD)

FT/RT=12%(0%)/0(0) 12.03.04 [illegible]6pl QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this desig any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate positions A seal on this draw ng or cover page list ng this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/16/2014

TC LL	40.0 PSF	REF	R9114- 94493
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106007
BC LL	0.0 PSF	HC-ENG	SSB/WHK
TOT.LD.	55.0 PSF	SEQN-	22792
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288F4--OWNER BUILDER /Becker Res Floor -- 337 sw rose creek drive Lake City, - F8 19' Floor Truss Girder)

Top chord 4x2 SP 2850f-2 3E
Bot chord 4x2 SP 2850f-2 3E
Webs 4x2 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 00 / Plate Dur Fac =1 00)
TC- From 50 plf at 0 13 to 50 plf at 16 52
TC- From 100 plf at 16 52 to 100 plf at 18 88
BC- From 5 plf at 0 00 to 5 plf at 16 52
BC- From 10 plf at 16 52 to 10 plf at 19 00
TC- 843 49 lb Conc Load at 0 65, 2 65, 4 65, 6 65 8 65,10 52
TC- 641 88 lb Conc Load at 12 52,14 52,16 52

Truss must be installed as shown with top chord up

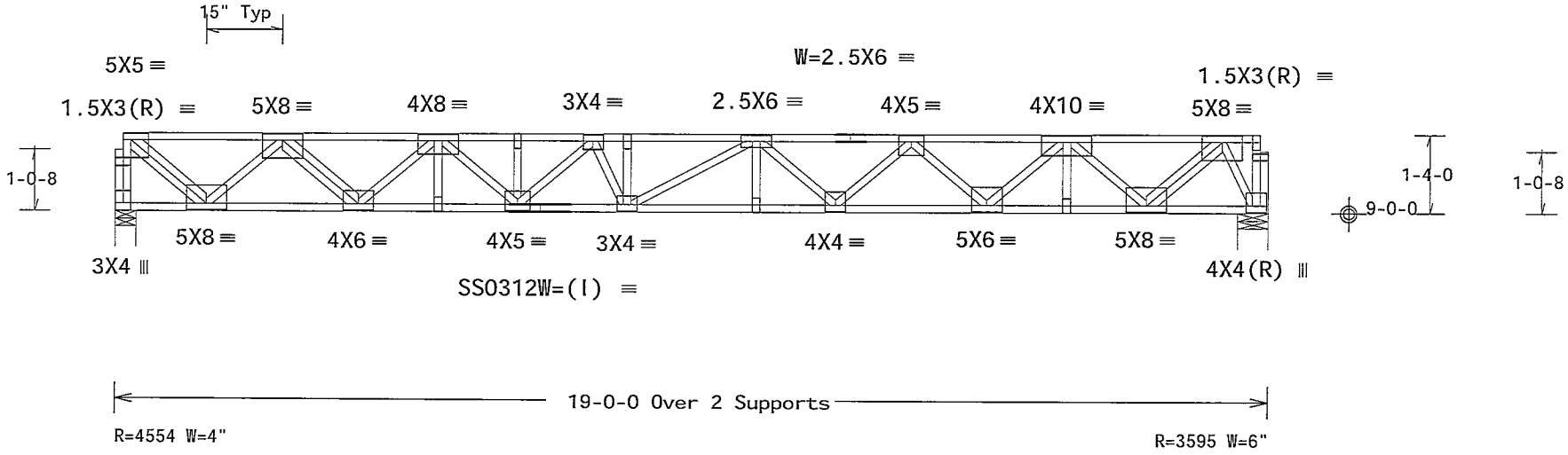
2 COMPLETE TRUSSES REQUIRED

See DWG CNSY42PL0913 or LSCCSYX2A0913 for connection details of 2 ply trusses

See detail STRBRIBR0211 for bracing and bridging recommendations

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance

Max JT VERT DEFL LL 0 46" DL 0 26" See detail DEFLCAMB0813 for camber recommendations



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=12%(0%)/0(0) 12.03.04 [illegible]6 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

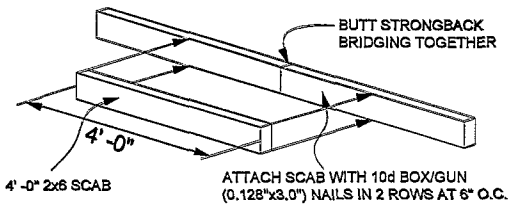
Trusses require extreme care in fabricating handling sh pping installing and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rig d ceiling Locations shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sections B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this desig any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handling shipping installation & brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page list ng this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/16/2014

TC LL	40.0 PSF	REF	R9114- 94494
TC DL	10.0 PSF	DATE	04/16/14
BC DL	5.0 PSF	DRW	HCUSR9114 14106006
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	76462
DUR.FAC.	1.00		
SPACING	24.0"	JREF-	1V5L487_Z01

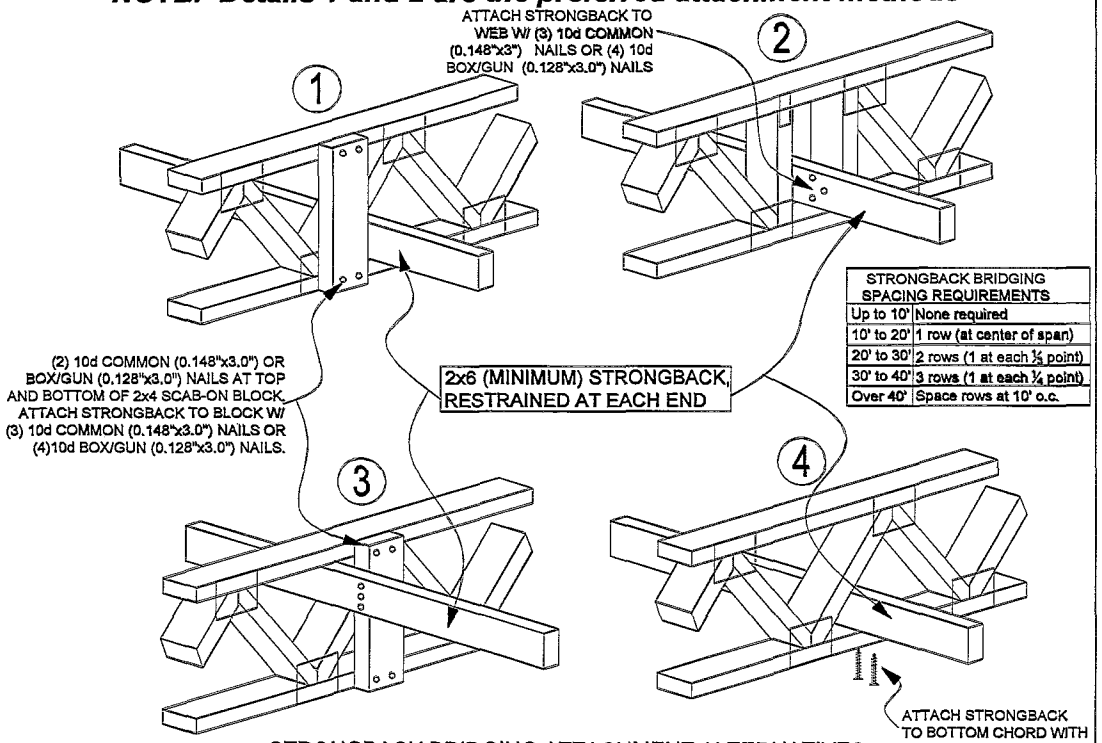
STRONGBACK BRIDGING RECOMMENDATIONS



NOTE: IN LIEU OF SPLICING AS SHOWN, LAP STRONGBACK BRIDGING MEMBERS FOR AT LEAST ONE TRUSS SPACING

STRONGBACK BRIDGING SPLICE DETAIL

NOTE: Details 1 and 2 are the preferred attachment methods



STRONGBACK BRIDGING SPACING REQUIREMENTS	
Up to 10'	None required
10' to 20'	1 row (at center of span)
20' to 30'	2 rows (1 at each ⅓ point)
30' to 40'	3 rows (1 at each ¼ point)
Over 40'	Space rows at 10' o.c.

STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES

- All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by ITW Building Compents Group Inc.

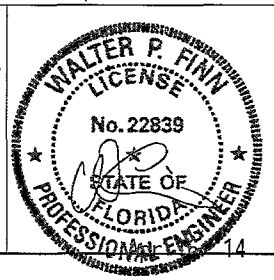
For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

ITW Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7 See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (X/H/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.icesafe.org



TC LL	PSF	REF	STRONGBACK
TC DL	PSF	DATE	2/28/11
BC DL	PSF	DRWG	STRBRIBR0211
BC LL	PSF		
TOT LD	PSF		
DUR FAC	1 00		
SPACING			

Commentary Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads Providing camber has the following advantages

- Helps to ensure level ceilings and floors after dead loads are applied
- Facilitates drainage to avoid ponding on flat or low slope roofs
- Compensates for different deflection characteristics between adjacent trusses
- Improves appearance of garage door headers and other long spans that can appear to "sag"
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads

The amount of camber is dependent on the truss type, span, loading, application, etceteras

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration

The following tables are provided as guidelines for limiting deflection and estimating camber Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	075" (horizontal)	125" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	100 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	15 x Vertical Deflection from Actual Dead Load
Floor Trusses	(025 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(025 x Deflection from Live Load) + (15 x Design Dead Load Deflection)

Note The actual dead load may be considerably less than the design dead load

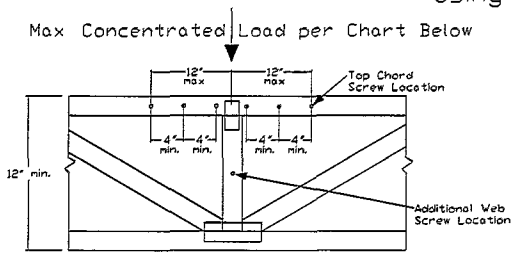
WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

REF	DEFLEC/CAMB
DATE	8/2/13
DRWG	DEFLCAMB0813

04/16/2014

System 42 Ply to Ply Connection Detail

Using Simpson SDS25600 or SDW22634 Strong Drive Screws or Equal

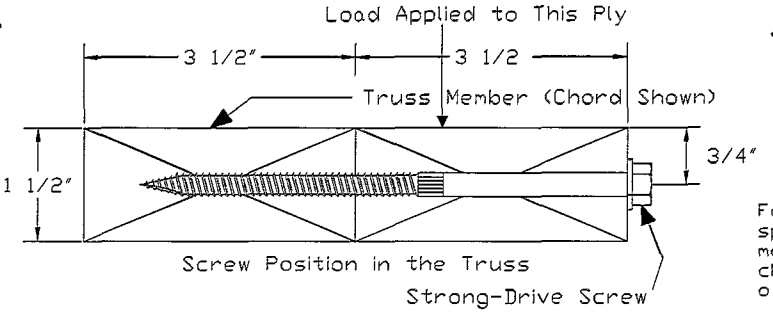


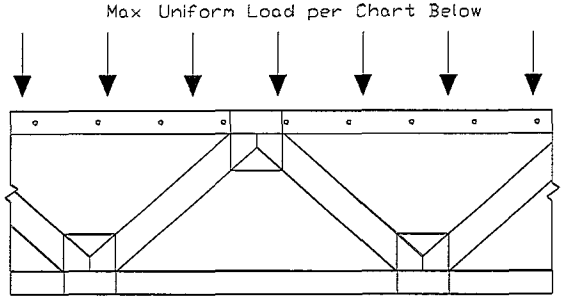
Apply screws to top chord within 12" of the concentrated load location @ 4" o c, min evenly distributing them to each side of the concentrated load A maximum of 6 screws may be applied to the top chord for each concentrated load

For double top chords, evenly distribute the screws over both top chords, using same spacing guidelines specified above The max number of top chord screws is 6 per chord member for a total maximum of 12 screws

If the concentrated load connection requires more screws than 6 per top chord member and the load is located at a panel point where webs intersect the top chord, the remainder of required screws may be applied to those webs below the concentrated load location evenly spaced @ 4" o c, min, keeping the 3" min end distances Each additional screw is worth 560 lb for DFL or SP webs and 400 lb for SPF webs

Refer to ITWBCG sealed drawing for individual truss design





For single top chord, see chart below for screw spacing For double top chord the screw spacing may be doubled (but may not exceed 24" o.c per chord) Screw spacing shall be offset by 1/2 the o c spacing in each chord

Screws need only apply to the extents of that load

For chord sections supporting less than 100 plf apply one screw at each top chord joint location

# of Screws	Maximum Concentrated Load (lbs) (1 00 DF)	
	DFL&SP	SPF
1	560	400
2	1120	800
3	1680	1200
4	2240	1600
5	2800	2000
6	3360	2400
7	3920	2800
8	4480	3200
9	5040	3600
10	5600	4000
11	6160	4400
12	6720	4800

General Notes

1 Screws centered along the 15" dimension of the 4x2 member
2 Minimum end distance of 3"
3 Screws installed with head in loaded member
4 Gap between plies not to exceed 1/8"
5 Screw location may be adjusted up to 1" to avoid conflict with other hardware or to avoid lumber defects
6 Do not install screws in areas where lumber wane exceeds 1/4"
7 Equal loads from both faces or loads that are evenly distributed to each ply do not require connections per this detail
8 For 3x2 members use Simpson s SDS25412 or SDW22500 screws or equal
9 Contact ITWBCG for special connections not covered by this detail

Top Chord Screw o c Spacing (inch)	Maximum Uniform Load (plf) Along Top Chord (1 00 DF)	
	DFL&SP	SPF
4	1680	1200
6	1120	800
8	840	600
10	672	480
12	560	400
14	480	342
16	420	300
18	373	266
20	336	240
22	305	218
24	280	200

TW
Building Components Group Inc.
Earth City MO 63045

WALTER P FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

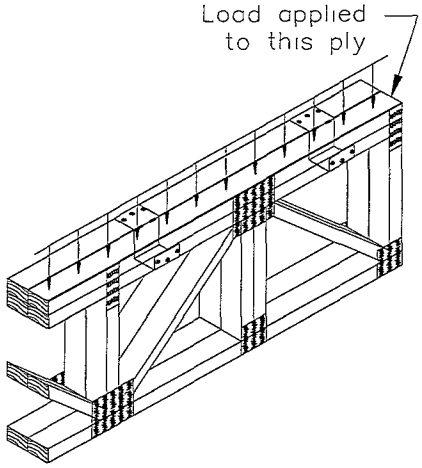
04/16/2014

TC LL		PSF	REF SY42 Connection
TC DL		PSF	DATE 10/11/13
BC DL		PSF	DRWG CNSY42PL0913
BC LL		PSF	
TOT LD		PSF	
DUR FAC	1 00		
SPACING			

SY32/SY42 PLY TO PLY LSC CONNECTION DETAIL

FOR DOWNWARD LOADS ONLY

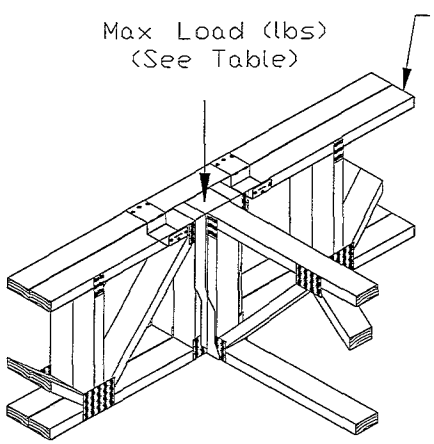
Uniform Load Application



Max TC Uniform Load (plf)			Clip Spacing Along Top Chord
SP	DF	SPF/HF	
935	810	585	12" oc
625	540	390	18" oc
470	405	295	24" oc
375	325	235	30" oc

Maximum LSC spacing is 30" oc

Concentrated Load Application



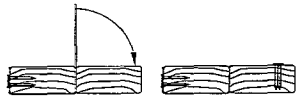
Max Load (lbs)		
SP	DF	SPF/HF
1870	1620	1170

Note
Install LSC adjacent, equidistant, and not more than 6" on each side of concentrated load

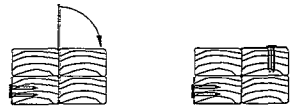
Installation Instructions

1 Position and attach LSC to loaded ply with (3) 0131"x15" nails into narrow face

2 Bend clip over adjacent ply and attach with (3) 0131"x15" nails into wide face



LSC42 for single 4x2 chords
LSC32 for single 3x2 chords



LSC42-2 for stacked 4x2 chords
LSC32-2 for stacked 3x2 chords

Refer to ITW Building Components Group Inc sealed drawing for individual truss design

Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

14

DUR FAC ALL

REF	SY42 Connection
DATE	9/20/13
DRWG	LSCSYX2A0913

04/16/2014

X

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V6C487-Z0113155950

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator	Anderson Truss Company
Job Identification	13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F (337 swrose
Truss Count	50
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Com/TPI-2007(STD)
Engineering Software	Alpine Software,Versions 13.02, 12.03, 14.02.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

05/13/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-PB16010-

#	Ref Description	Drawing#	Date
1	17792--A1 20' Common	14133024	05/13/14
2	17793--A2 20' Common	14133039	05/13/14
3	17794--ADGE 20' Gable	14133018	05/13/14
4	17795--B1 19'4" Common	14133019	05/13/14
5	17796--B2 19'4" Common	14133035	05/13/14
6	17797-BDGE 19'4" Gable	14133002	05/13/14
7	17798--C1 19' Common	14133048	05/13/14
8	17799--C2 19' Common	14133037	05/13/14
9	17800-C3 19' Common Gi	14133020	05/13/14
10	17801--CDGE 10' Gable	14133006	05/13/14
11	17802--CDGE1 19' Gable	14133011	05/13/14
12	17803--CJ1 8'10 Jack	14133026	05/13/14
13	17804--CJ1A 1'1"9 Jack	14133017	05/13/14
14	17805--CJ1B 8"10 Jack	14133043	05/13/14
15	17806--CJ3 2'5"15 Jack	14133022	05/13/14
16	17807--CJ3A 3'4"9 Jack	14133015	05/13/14
17	17808-CJ3B 2'5"15 Jack	14133042	05/13/14
18	17809--CJ5 4'3"4 Jack	14133021	05/13/14
19	17810--CJ5A 5'7"9 Jack	14133014	05/13/14
20	17811--CJ5B 4'3"4 Jack	14133040	05/13/14
21	17812-D 31'8" Stepdown	14133031	05/13/14
22	17813-D1 20'4" Stepdow	14133028	05/13/14
23	17814-DDGE 31'8" Gable	14133001	05/13/14
24	17815-E 28' Stepdown H	14133046	05/13/14
25	17816-E1 28' Stepdown	14133012	05/13/14
26	17817-E2 28' Stepdown	14133023	05/13/14
27	17818-E3 28' Stepdown	14133034	05/13/14
28	17819-E4 28' Stepdown	14133030	05/13/14
29	17820-E5 28' Stepdown	14133033	05/13/14
30	17821-E6 28' Stepdown	14133010	05/13/14
31	17822--E7 7' End Jack	14133027	05/13/14
32	17823-G 29'8" Stepdown	14133001	05/13/14
33	17824-GDGE 29'8" Stepd	14133002	05/13/14
34	17825-H11 28' Stepdown	14133009	05/13/14
35	17826-H11A 28' Stepdow	14133007	05/13/14
36	17827-H13 28' Stepdown	14133003	05/13/14
37	17828-H13A 28' Stepdow	14133041	05/13/14
38	17829-H7 28' Stepdown	14133032	05/13/14
39	17830-H9 28' Stepdown	14133013	05/13/14
40	17831-HJ7 9'1"6 Hip Ja	14133036	05/13/14
41	17832-HJ7A 9'4"6 Hip J	14133045	05/13/14
42	17833-MH7 28' Mono Hip	14133044	05/13/14
43	17834-MH9 28' Mono Hip	14133005	05/13/14
44	17835-PBD 5'7"8 Common	14133025	05/13/14
45	17836-PBD1 2'8"3 Commo	14133029	05/13/14
46	17837-PBDG 5'7"8 Gable	14133004	05/13/14
47	17838-PBE 5'2"11 Stepd	14133047	05/13/14
48	17839-PBE1 5'2"11 Comm	14133008	05/13/14
49	17840-PBG 3'8"10 Commo	14133016	05/13/14
50	17841-PBG1 3'8"10 Comm	14133038	05/13/14

#31678
Revision

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
 Document ID 1V6C487-Z0113155950

Truss Fabricator	Anderson Truss Company
Job Identification	13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F (337 sw rose
Truss Count	1
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Com/TPI-2007(STD)
Engineering Software	Alpine Software,Versions 13.02, 12.03, 14.02.
Structural Engineer of Record	
Address	
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration Floor - N/A Wind - 120 MPH ASCE 7-10 -Closed

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

-Truss Design Engineer-
Walter P Finn

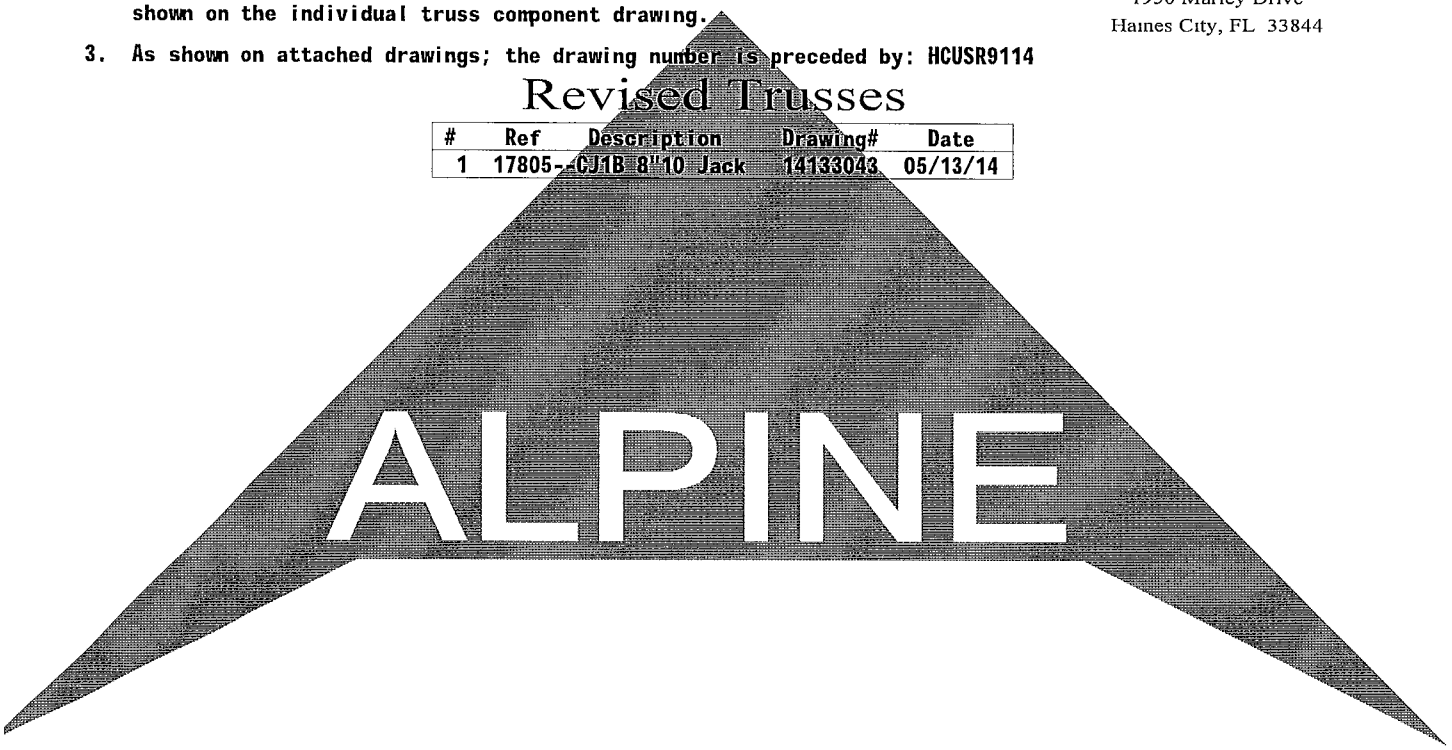
1950 Marley Drive
Haines City, FL 33844

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Revised Trusses

#	Ref	Description	Drawing#	Date
1	17805--CJ1B	8"10 Jack	14133043	05/13/14



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - A1 20' Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x8 SP SS B2 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B' uses design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0 21" due to live load and 0 37" due to dead load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 5-0-0 to 15-0-0

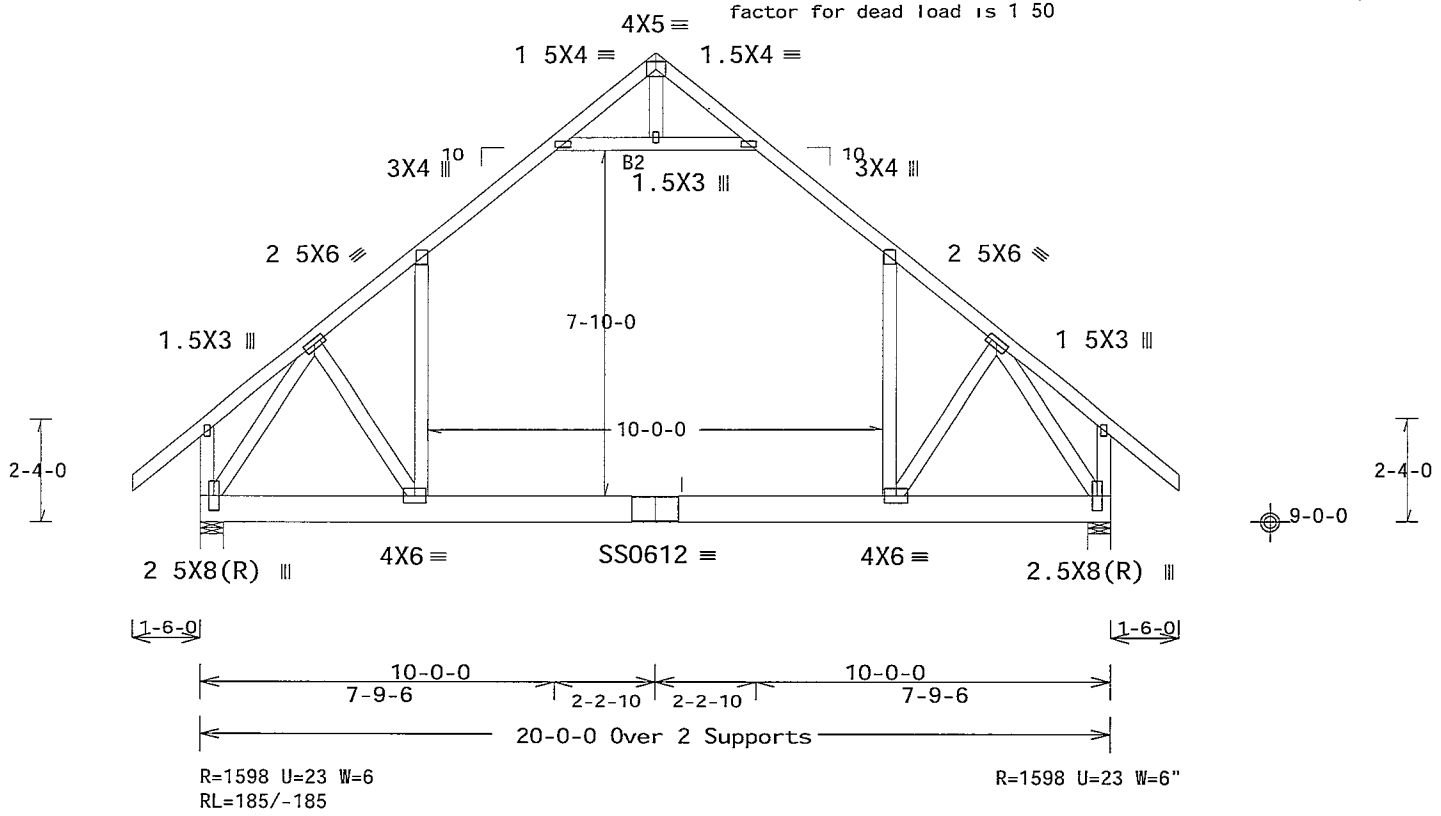
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

End verticals not exposed to wind pressure

Collar-tie braced with continuous lateral bracing at 24' OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 13 02 07 A QTY:5 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this desi any fa lure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pping nstallation bracing of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate posit ons A seal on th s draw ng or cover page l st ng this draw ng ind cates acceptance of professional eng neer ng respons b ty solely for the design shown The su tab l ity and use of th s design for any structure s the respons b l ty of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17792
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133024
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	369575
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - A2 20 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x8 SP SS B2 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0 20" due to live load and 0 35 due to dead load

BC attic room floor loading LL = 40 00 psf, DL = 10 00 psf, from 5-0-0 to 15-0-0

MWFRS loads based on trusses located at least 7 50 ft from roof edge

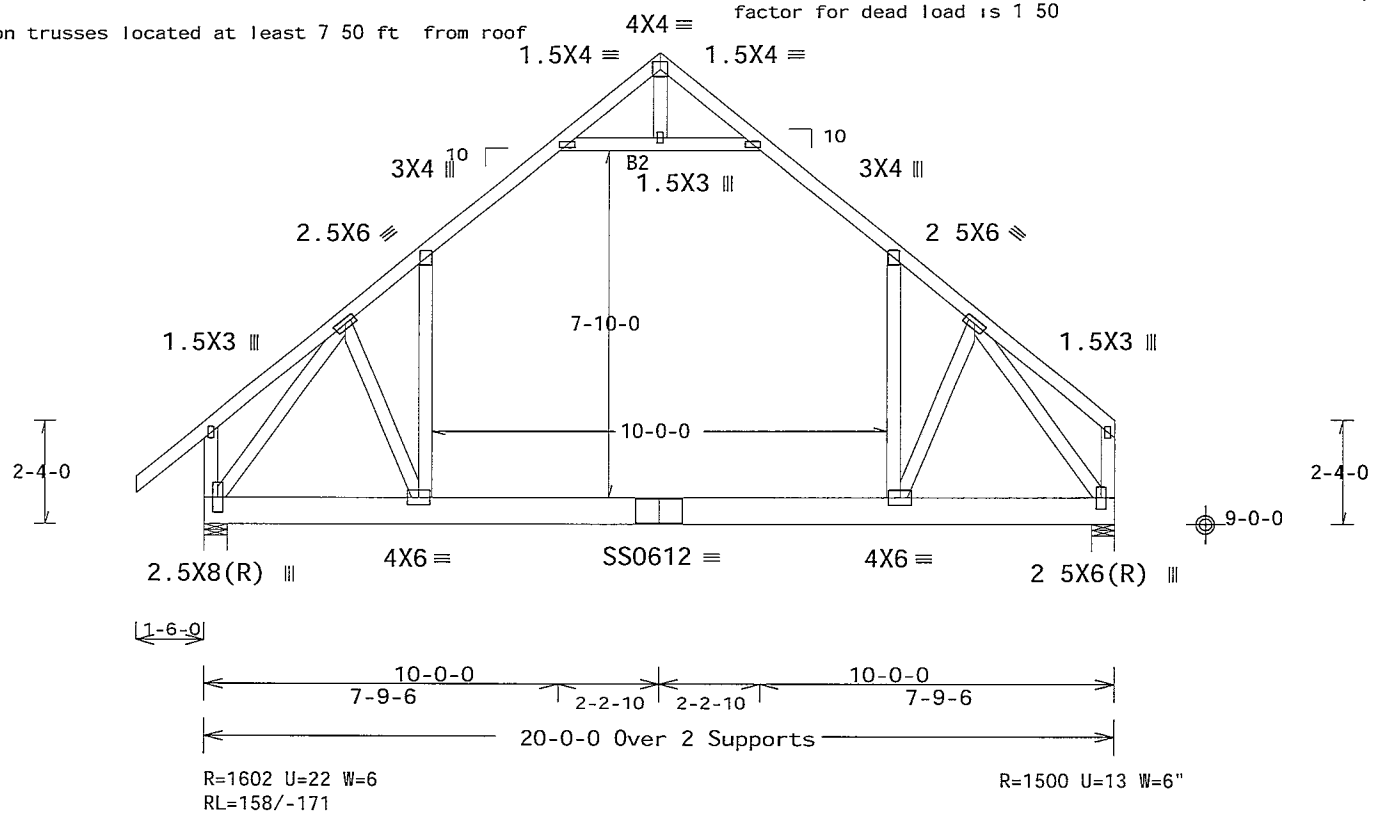
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

End verticals not exposed to wind pressure

Collar-tie braced with continuous lateral bracing at 24" OC or rigid ceiling

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 13 02 07 A4 QTY 4 FL/-/5/-/-/R/- Scale =.25"/Ft.

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installat on bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page l sting th s drawing nd cates acceptance of profess onal engineering respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any structure is the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17793
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133039
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	369596
DUR.FAC.	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - ADGE 20 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

End verticals not exposed to wind pressure

Truss spaced at 24 0 OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Fasten rated sheathing to one face of this frame

(**) 2 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind 15 00 ft mean hgt ASCE 7-10 CLOSED bldg, Located anywhere in roof RISK CAT II EXP B wind TC DL=3 5 psf wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

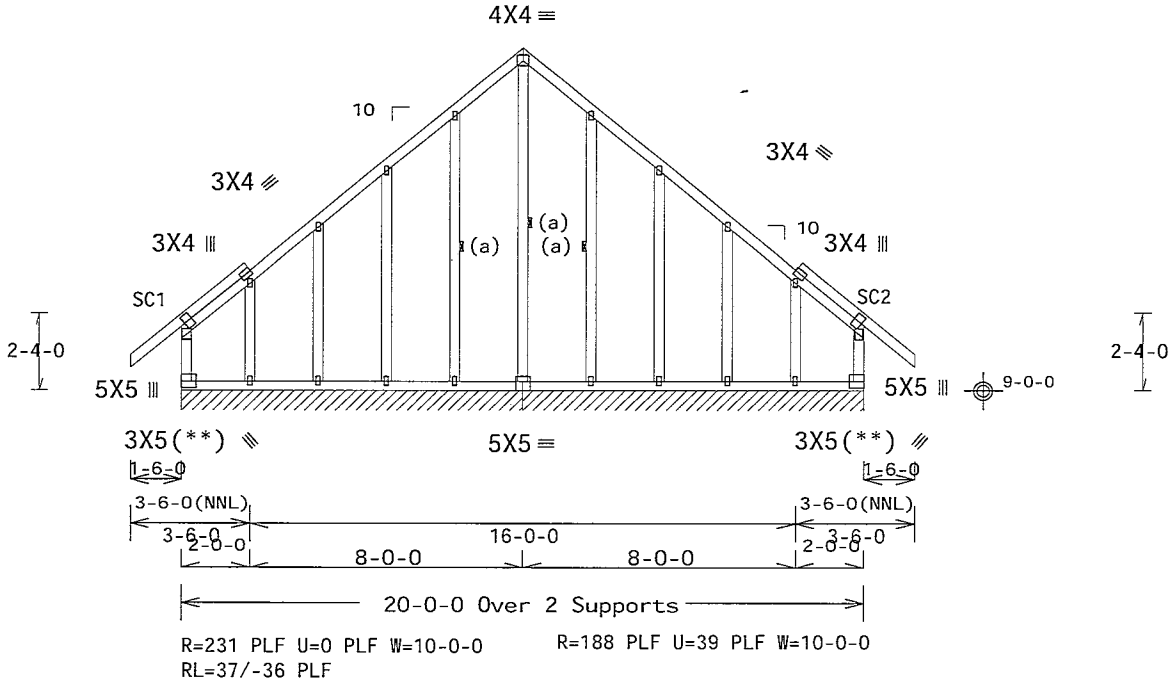
Calculated horizontal deflection is 0 18 due to live load and 0 35 due to dead load

See DWGS A12015ENC100212 GBLLETIN0212 & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=231 PLF U=0 PLF W=10-0-0 R=188 PLF U=39 PLF W=10-0-0
RL=37/-36 PLF

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

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ITW Building Components Group Inc.
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ITW Bu ld ng Components Group Inc (ITWBCG) shal not be respons ble for any deviat on from th s desi any fa lure to build the truss n conformance w th ANSI/TPI 1 or for hand ng sh pp ng installation bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to drawings 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this drawing indicates acceptance of profess onal engineering responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17794
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133018
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD.	37.0 PSF	SEQN-	37770
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - B1 19'4' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

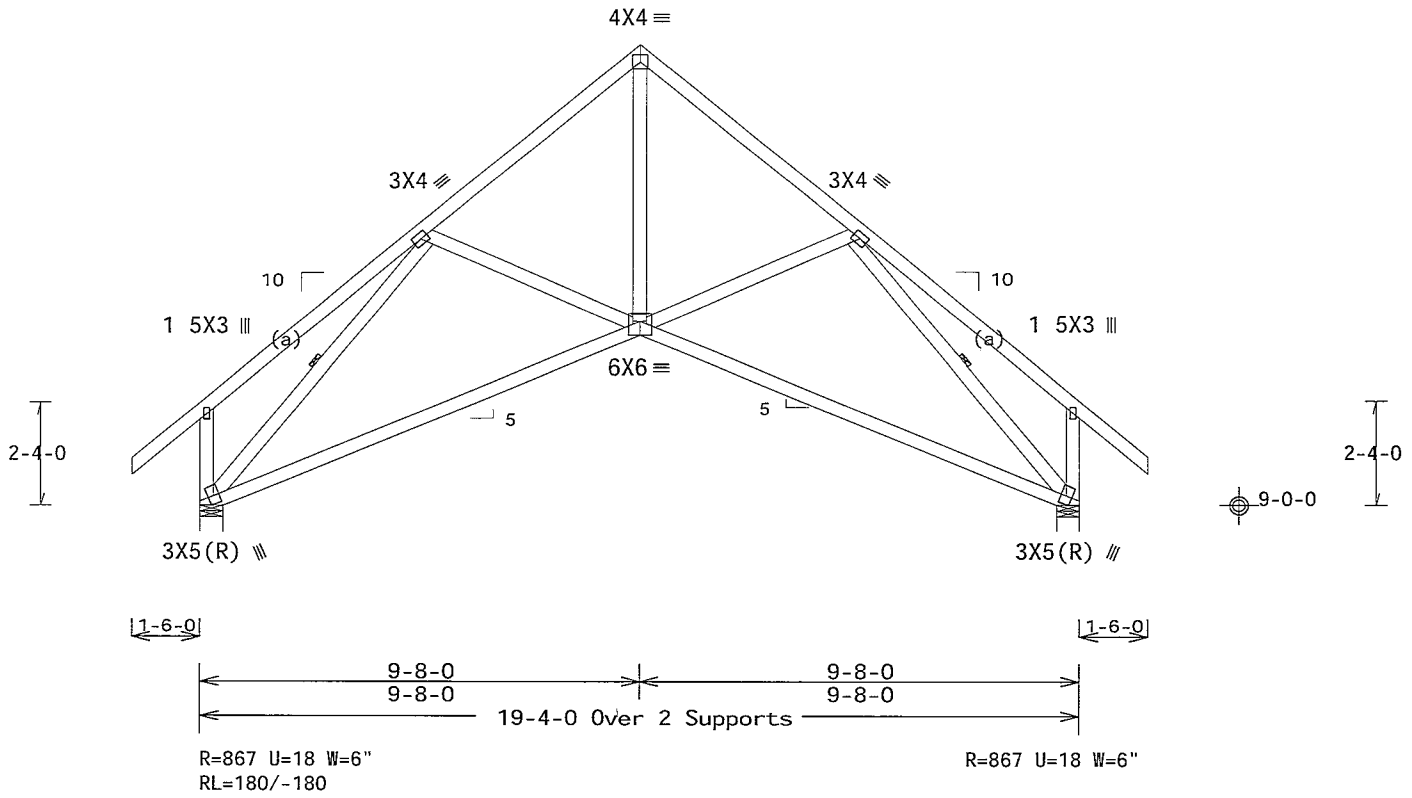
End verticals not exposed to wind pressure

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 QTY:5 FL/-/5/-/-/R/- Scale =.25"/Ft.

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05/13/2014

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24 0"

REF	R9114- 17795
DATE	05/13/14
DRW	HCUSR9114 14133019
HC-ENG	JB/WHK
SEQN-	357621
FROM	JMW
JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - B2 19'4' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

End verticals not exposed to wind pressure

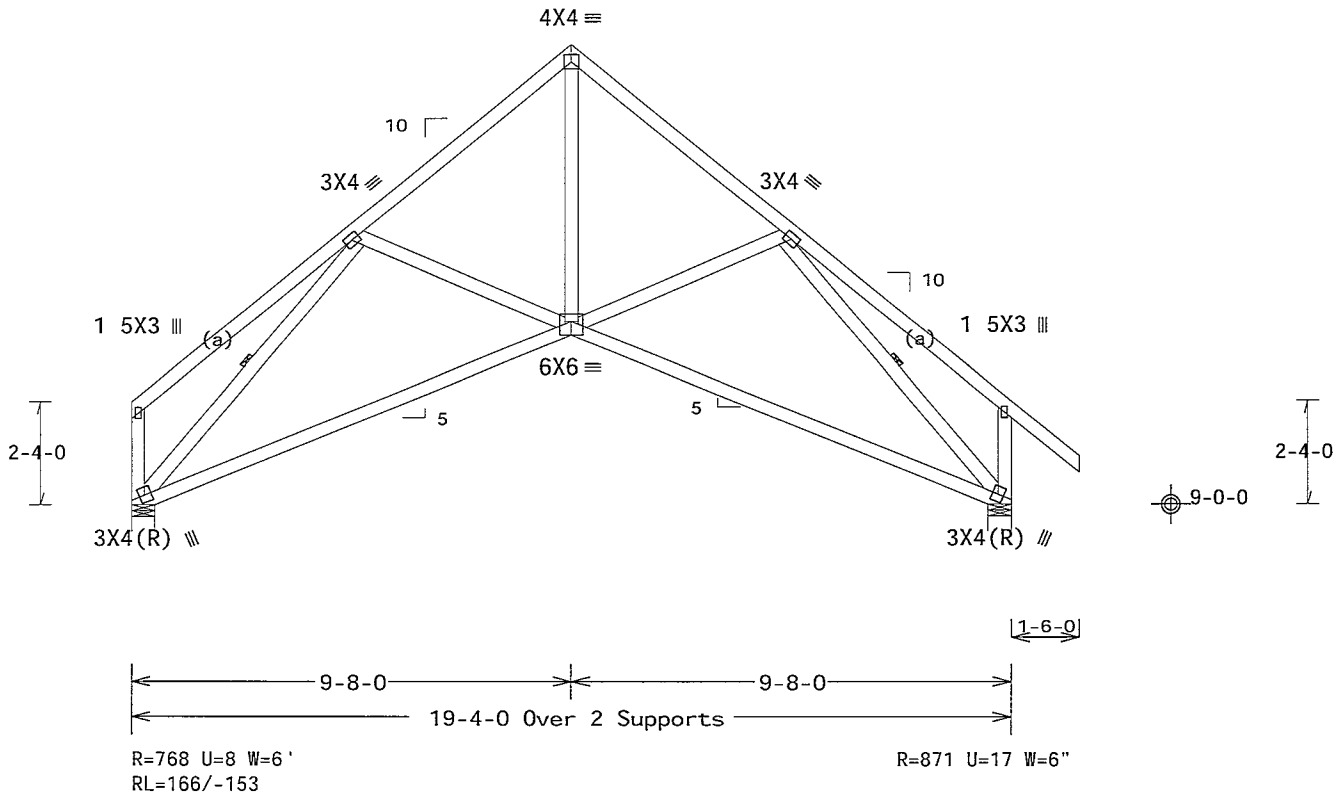
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03 04 0326 14 | QTY.4 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17796
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10 0 PSF	DRW	HCUSR9114 14133035
BC LL	0 0 PSF	HC-ENG	JB/WHK
TOT LD	37 0 PSF	SEQN-	357622
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - BDGE 19 4' Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

End verticals not exposed to wind pressure

Gable end supports 8 max rake overhang

See DWGS A12015ENC100212 GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

(**) 2 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

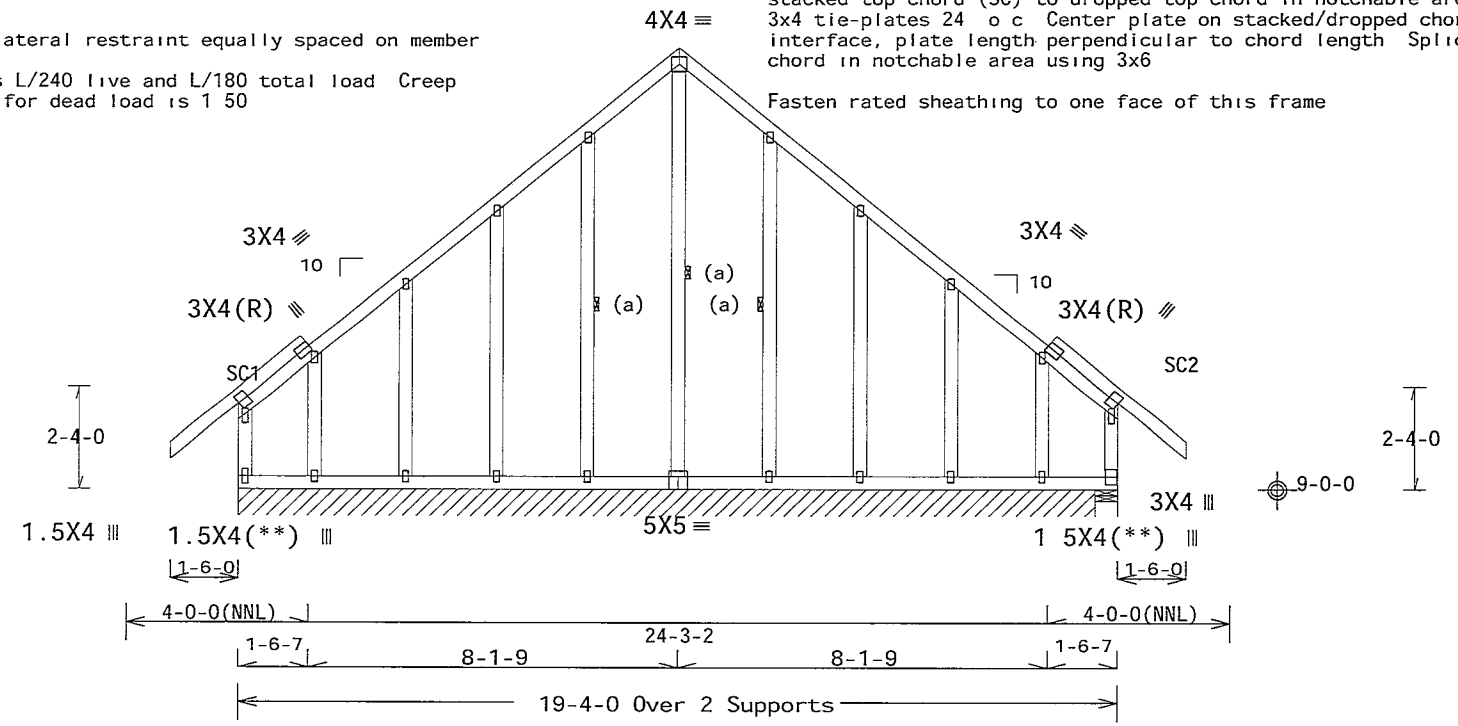
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Calculated horizontal deflection is 0 08' due to live load and 0 18' due to dead load

Stacked top chord must NOT be notched or cut in area (NNL) Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Fasten rated sheathing to one face of this frame



R=80 PLF U=0 PLF W=18-10-0
RL=9/-9 PLF

R=191 Rw=203 U=91 W=6"

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 0228 14 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17797
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37.0 PSF	SEQN-	369605
DUR.FAC.	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - C1 19 Common)

Value Set 13B (Effective 6/1/2013)

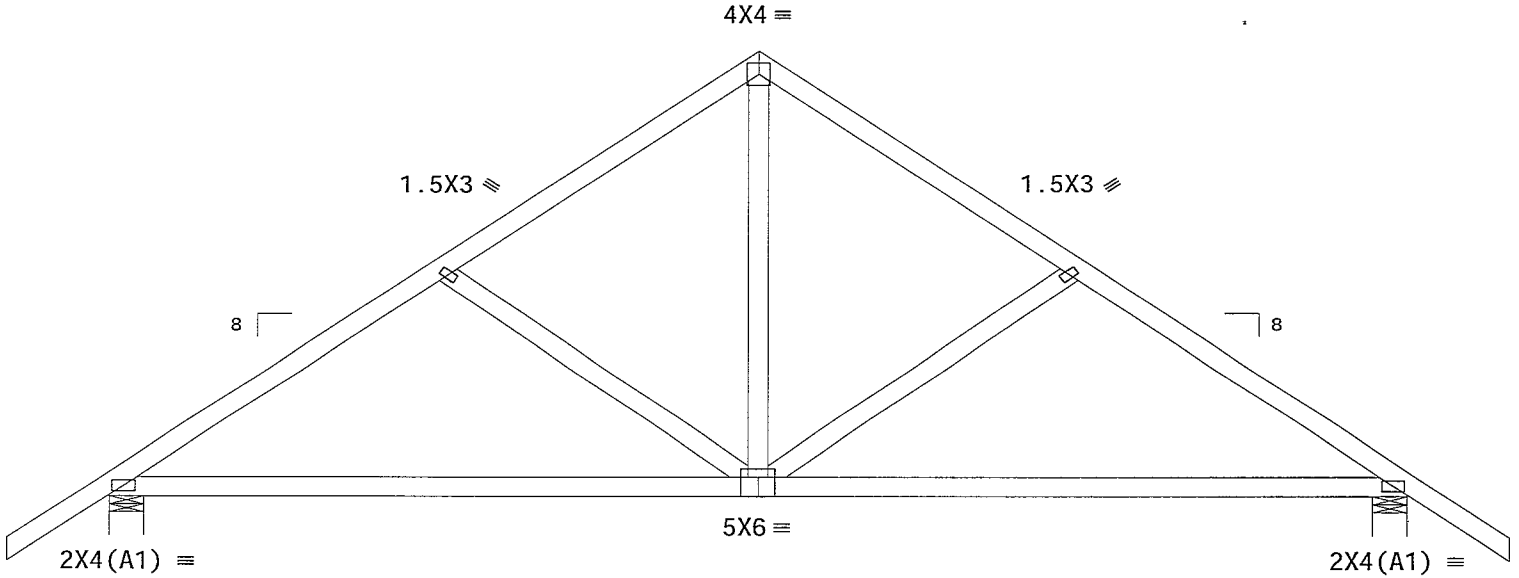
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0228.14 QTY 1 FL/-/5/-/-/R/- Scale = 375"/Ft

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FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17798
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133048
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	37556
DUR.FAC.	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - C2 19 Common)

Value Set 13B (Effective 6/1/2013)

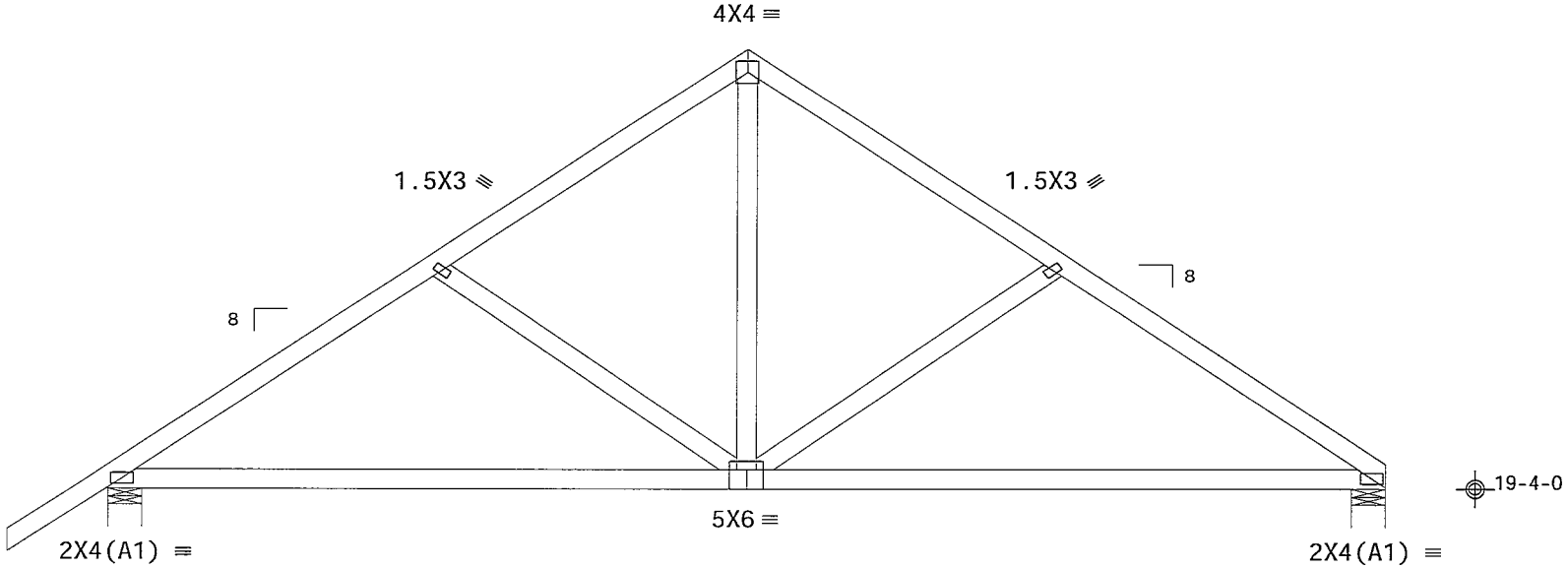
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 [illegible]14 QTY 1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handling sh pp ng nstall ng and bracing Refer to and follow the latest ed tion of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces pr or to perform ng these functions Installers shall provide temporary bracing per BCSI Unless notod otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rig d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any dev at on from th s design any failure to bu ld the truss n conformance with ANSI/TPI 1 or for handling shipp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this drawing or cover page l st ng th s draw ng nd cates acceptance of professional engineering responsib l ty solely for the des gn shown The su tab l ity and use of this design for any structure is the respons b l ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more informat on see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17799
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133037
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	37520
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - C3 19 Common Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x6 SP SS
Webs 2x4 SP #3 W3 2x4 SP #2
Lt Wedge 2x4 SP #3 Rt Wedge 2x4 SP #3

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 9 50
TC- From 57 plf at 9 50 to 57 plf at 19 00
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 10 00
BC- From 20 plf at 10 00 to 20 plf at 19 00
BC- 2215 98 lb Conc Load at 7 06
BC- 1077 97 lb Conc Load at 9 06
BC- 1082 11 lb Conc Load at 11 06,13 06,15 06,17 06

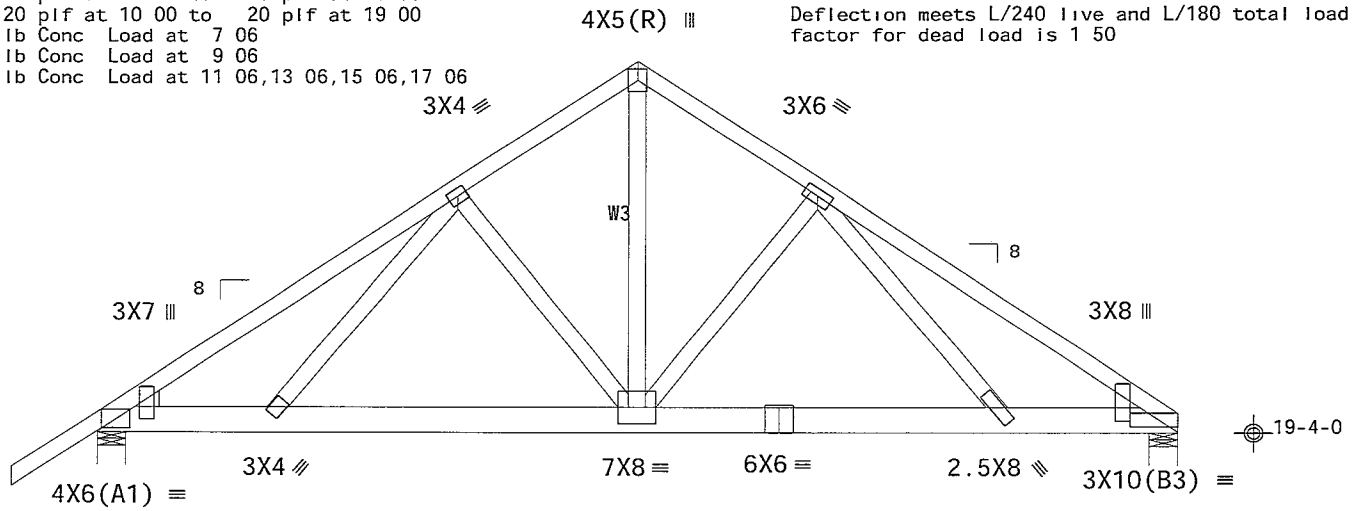
2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131 x3', min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @ 3 75' o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind, 22 37 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)
PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 [illegible] 14 QTY·1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17800
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	371446
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - CDGE 10' Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

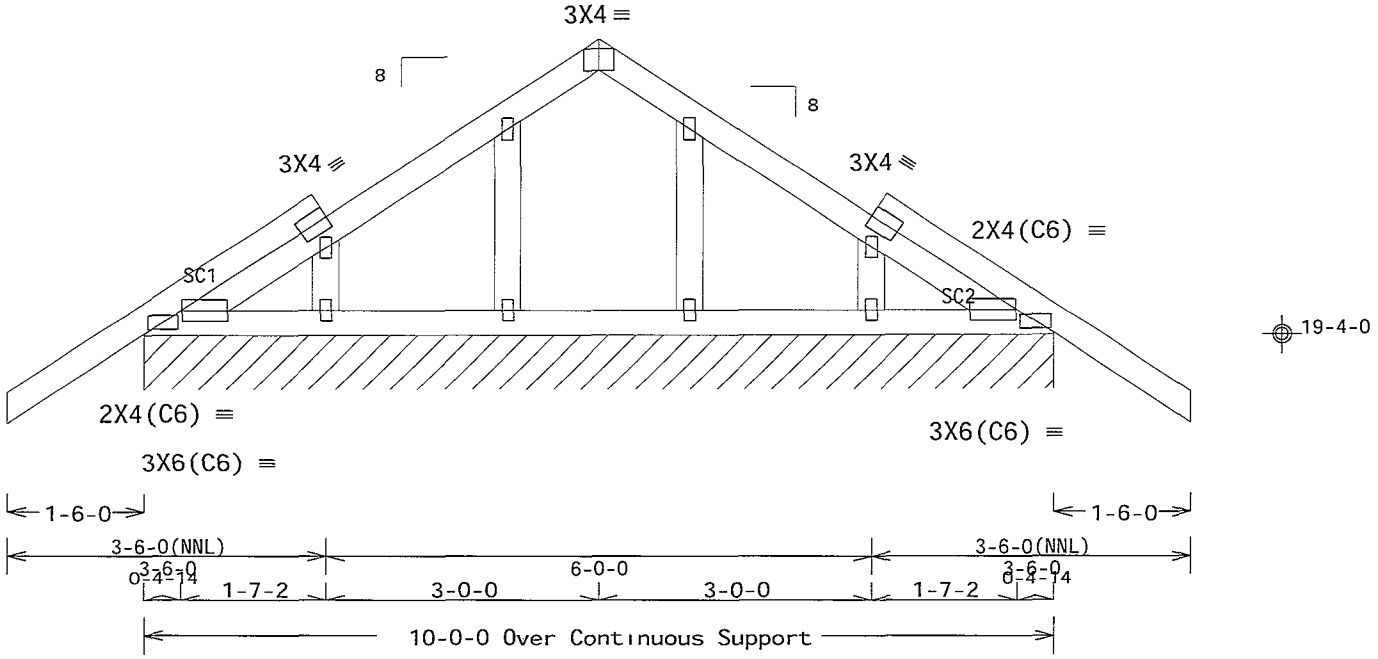
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 70 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=180 PLF U=71 PLF W=10-0-0
RL=11/-18 PLF

Note All Plates Are 1 5X3 Except As Shown

Design Crit· FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07.0806.14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pping installation brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on this drawing or cover page i sting th s drawing nd cates acceptance of professional engineering respons b l ty solely for the des gn shown The su tab l ity and use of this design for any structure is the respons b l ity of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more information see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17801
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371171
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - CDGE1 19' Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

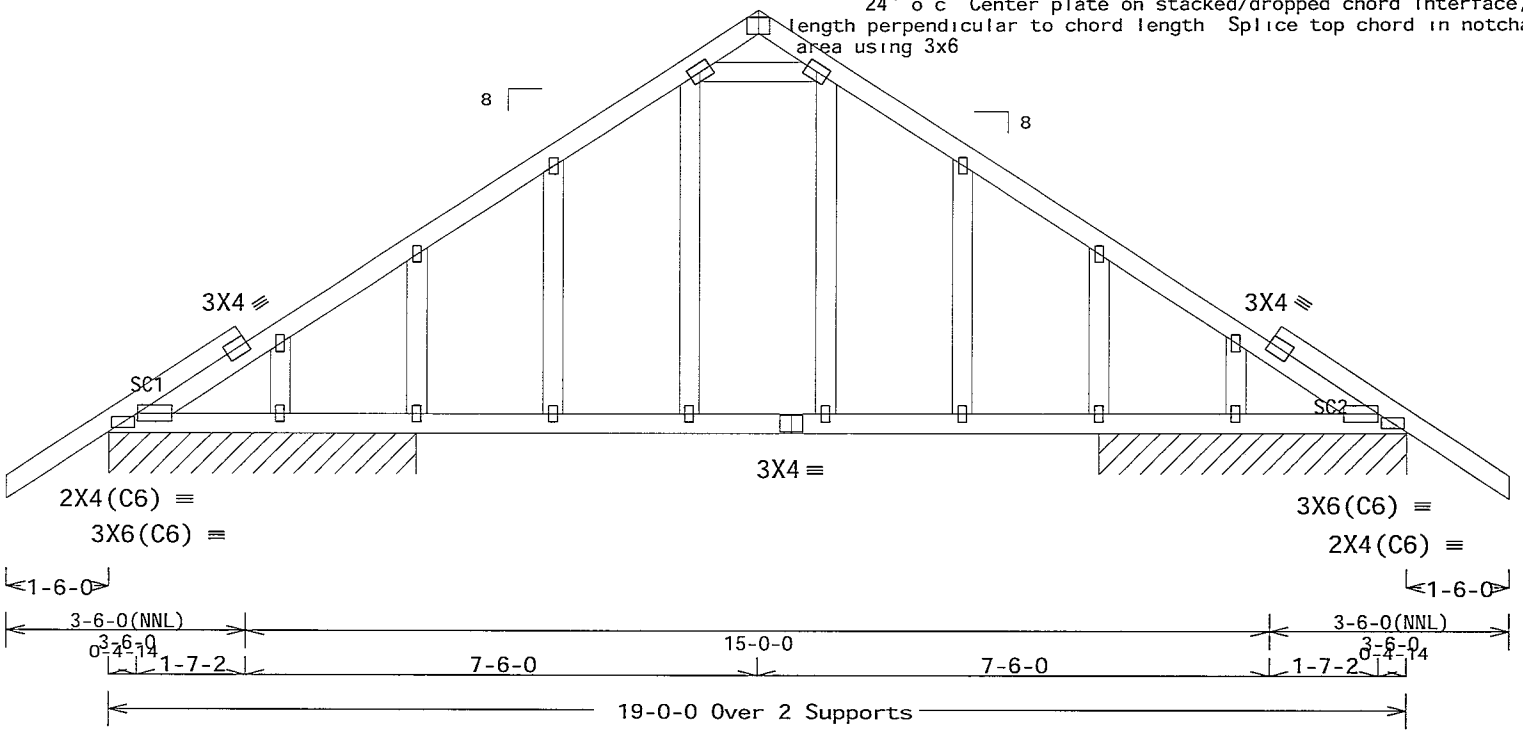
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 20 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0" OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

3X4 ≡ Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=375 PLF U=54 PLF W=4-6-0
RL=62/-62 PLF

R=375 PLF U=54 PLF W=4-6-0

Note All Plates Are 1 5X3 Except As Shown.

Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 [illegible]14 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17802
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133011
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371464
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - CJ1 8"10 Jack)

Value Set 13B (Effective 6/1/2013)

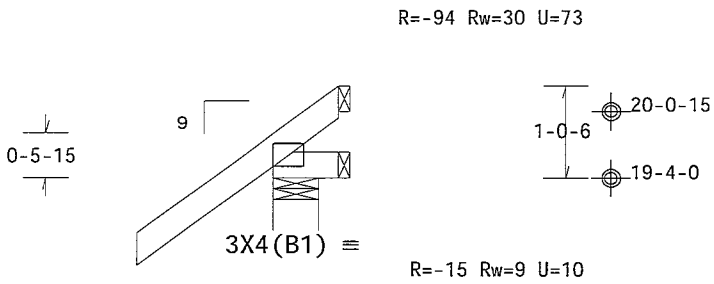
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 19 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 0328 14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17803
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133026
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37.0 PSF	SEQN-	369296
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - CJ1A 1'1'9 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

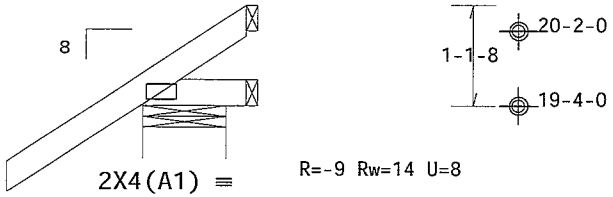
Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

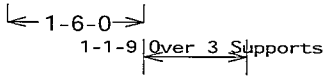
120 mph wind, 19 58 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

R=-38 Rw=19 U=33



R=-9 Rw=14 U=8



R=226 U=38 W=11'
RL=35/-25

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0328 14 QTY 3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17804
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133017
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	369306
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ1B 8'10 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

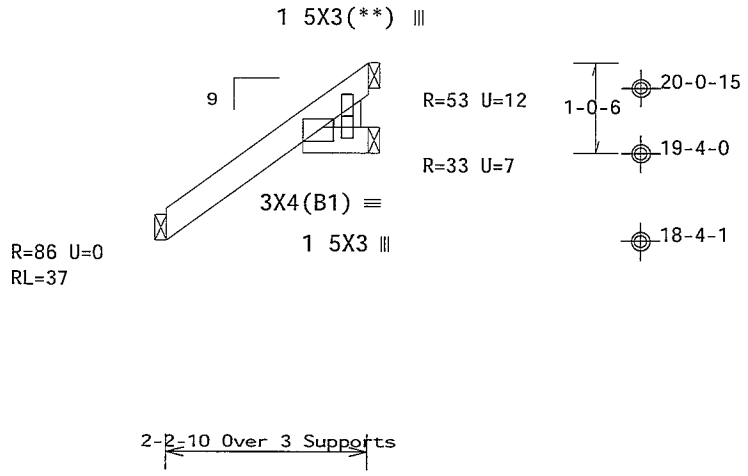
Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind, 19 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 14 02 01 0114 19 QTY 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17805
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133043
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	1775 REV
DUR FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ3 2 5 15 Jack)

Value Set 13B (Effective 6/1/2013)

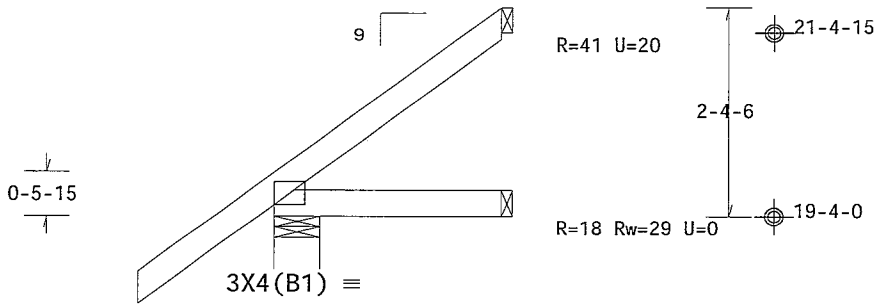
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 20 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0328 14 QTY 2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng sh pping nstall ng and brac ng Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety informat on by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r gid ceiling Locations shown for permanent lateral restra nt of web shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons b e for any dev ation from th s design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pping nstallat on & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherw se Refer to drawings 160A-Z for standard p ate pos tions A seal on th draw ng or cover page list ng this draw ng indicates acceptance of profess onal eng neer ng respons bility solely for the design shown The suitabi ty and use of th s des gn for any structure s the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20 0 PSF	REF	R9114- 17806
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133022
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	369286
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ3A 3 4 9 Jack)

Value Set 13B (Effective 6/1/2013)

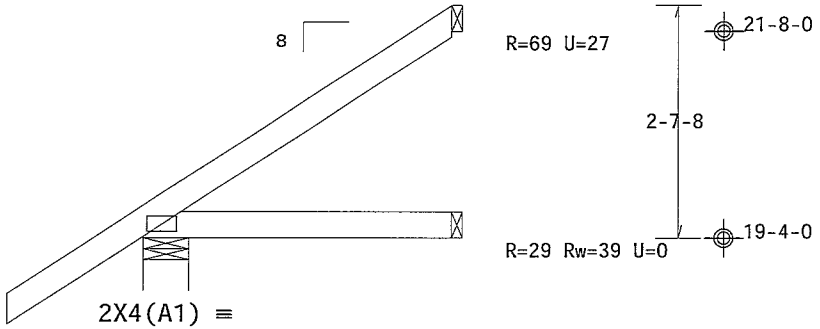
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind 20 33 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07.0038.14 QTY.3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20.0 PSF	REF	R9114- 17807
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133015
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	369283
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ3B 2 5 15 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

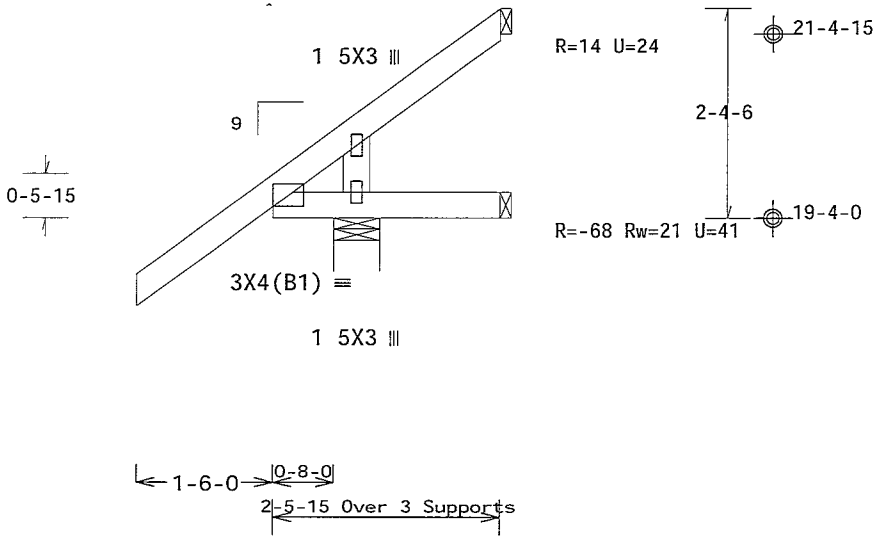
Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 20 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Left cantilever is exposed to wind



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 [illegible] QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17808
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133042
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	369817
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ5 4'3 4 Jack)

Value Set 13B (Effective 6/1/2013)

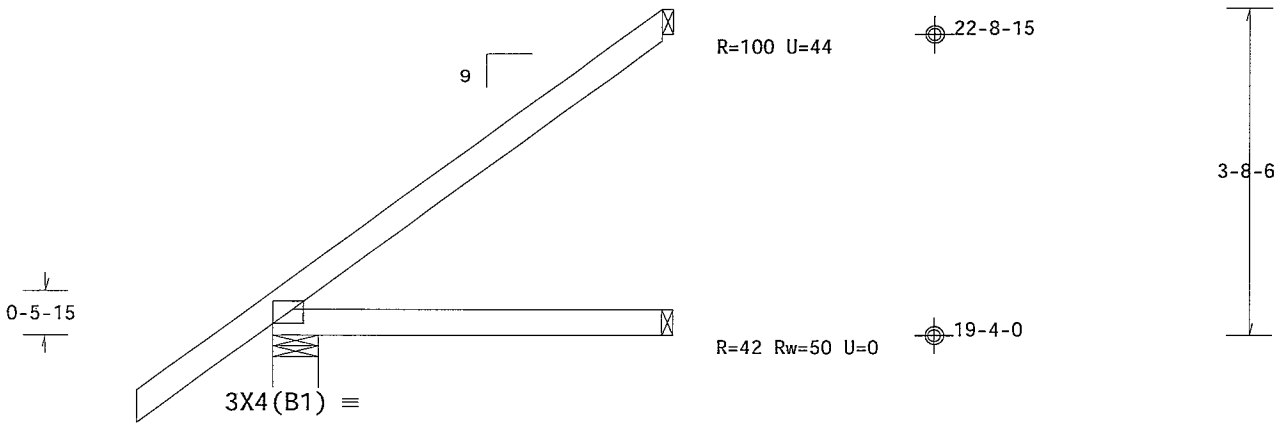
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 87 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 [illegible] QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17809
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133021
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	369293
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - CJ5A 5 7'9 Jack)

Value Set 13B (Effective 6/1/2013)

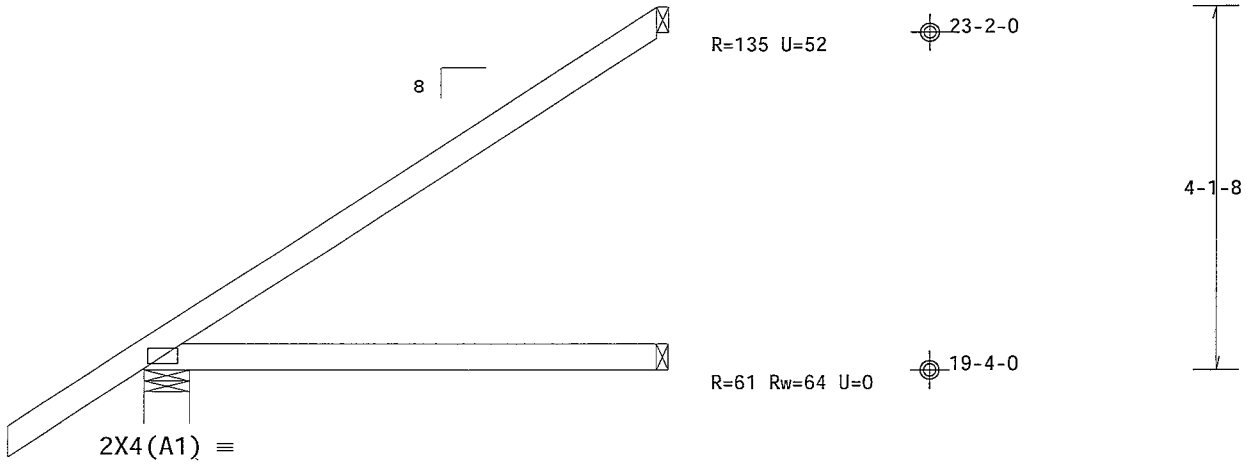
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

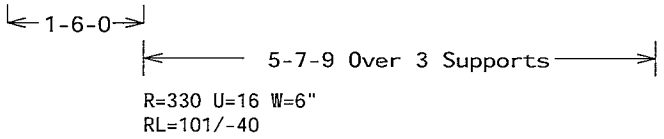
Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 21 08 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design





Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 [illegible]44 QTY:3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the trusses in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see this job's general notes page, ITW BCG www.twbcg.com, TPI www.tpinst.org, WTCA www.sbcindustry.com, ICC www.ccsafe.org.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17810
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133014
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	369277
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - CJ5B 4 3 4 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

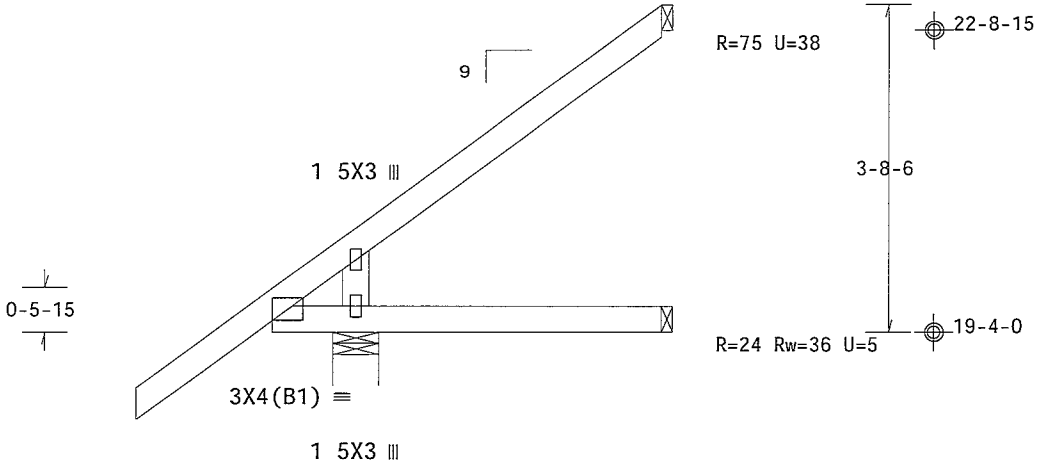
Lumber value set "13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 20 87 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Left cantilever is exposed to wind



Design Crit: FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 [illegible] 44 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17811
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133040
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD	37.0 PSF	SEQN-	369823
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - D 31'8' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

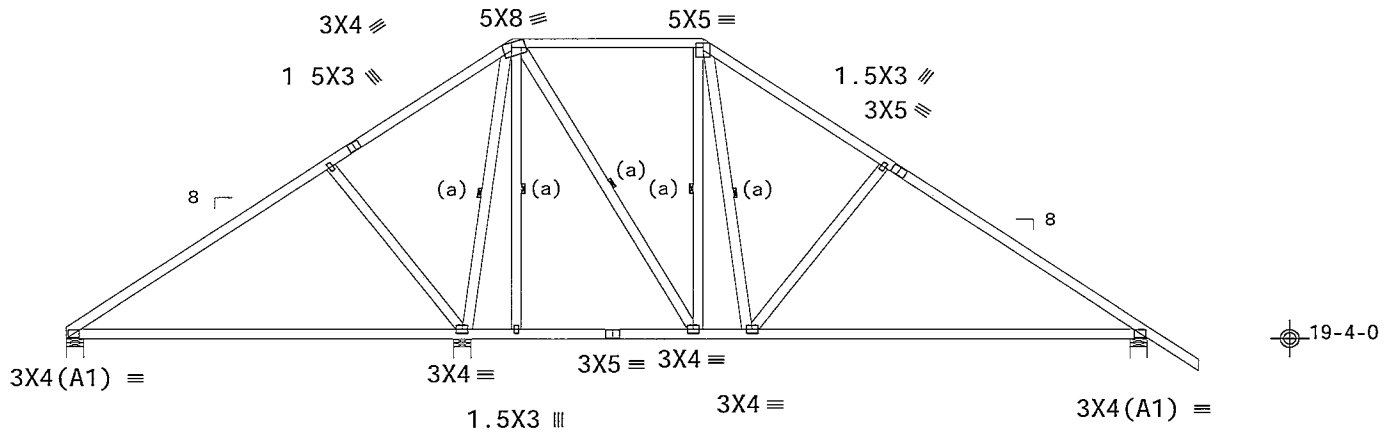
In lieu of structural panels use purlins to brace all flat TC @ 24' OC

120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit· FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 44 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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!TW Bu lding Components Group Inc (!TWBCG) shall not be respons ble for any dev ation from th s design any fa lure to bu ld the truss in conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard p ate posit ons A seal on th s draw ng or cover page l st ng this draw ng indicates acceptance of profess onal eng neer ng respons b l ty solely for the design shown The su tab l ty and use of th s des gn for any structure s the respons b l ty of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more nformat on see This job s general notes page !TW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17812
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10 0 PSF	DRW	HCUSR9114 14133031
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371158
DUR FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - D1 20'4' Stepdown Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x6 SP #2 B2 2x6 SP SS
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 3 16
TC- From 57 plf at 3 16 to 57 plf at 5 84
TC- From 57 plf at 5 84 to 57 plf at 20 33
BC- From 10 plf at 0 00 to 10 plf at 20 33
BC- 1085 00 lb Conc Load at 1 60, 3 60, 5 60, 7 60 9 60
BC- 1082 11 lb Conc Load at 11 60,13 60,15 60,17 60 19 60

Left end vertical not exposed to wind pressure

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131'x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 2 Rows @ 6 00' o c (Each Row)
Webs 1 Row @ 4' o c
Use equal spacing between rows and stagger nails in each row to avoid splitting
4 o c spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

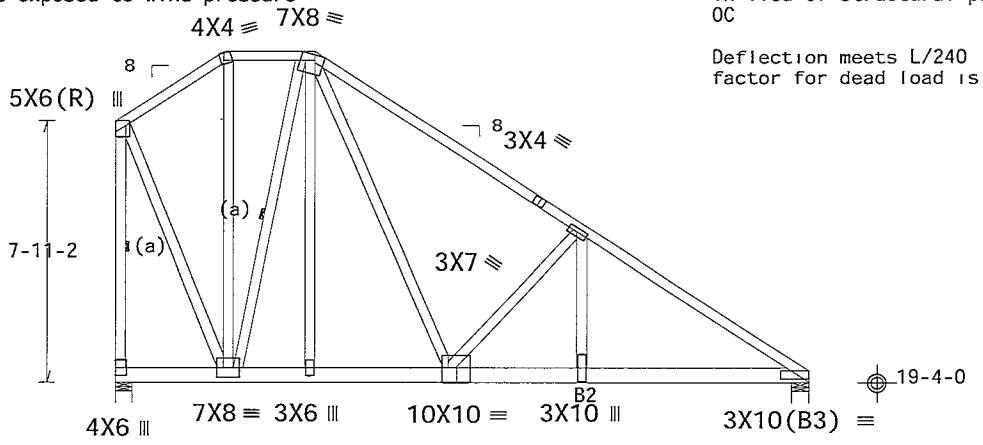
120 mph wind, 24 53 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=5822 U=325 W=6"

R=6372 U=428 W=6

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0000 14 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handling sh pp ng nstal ng and brac ng Refer to and follow the latest edition of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall prov do temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s design any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard p ate pos t ons A seal on th s draw ng or cover page listing th s draw ng nd cates acceptance of profess ona eng neer ng respons b lity solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more nformation see Th s job s general notes page ITW BCG www twbcg com TPI www tp nst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17813
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133028
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371449
DUR FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - DDGE 31 8 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

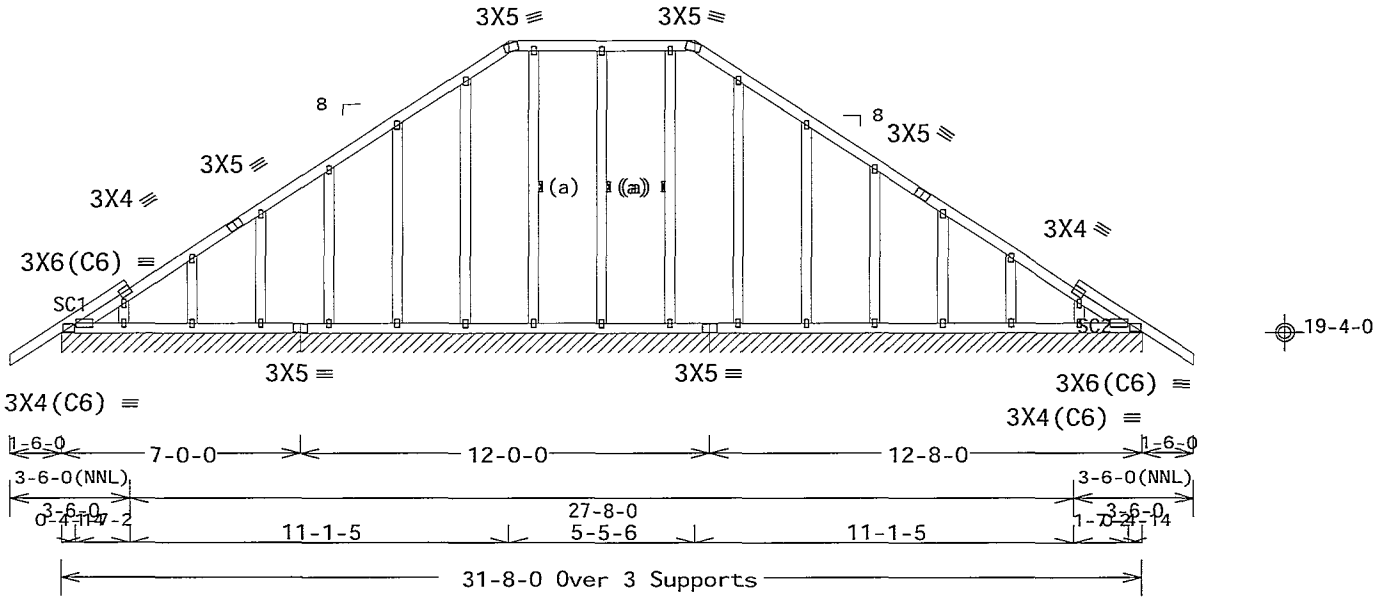
120 mph wind, 23 40 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0 OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC



R=191 PLF U=28 PLF W=7-0-0 R=200 PLF U=33 PLF W=12-8-0
RL=53/-53 PLF R=208 PLF U=16 PLF W=12-0-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 [illegible]14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17814
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	371485
DUR.FAC.	1 25	FROM	JMW
SPACING	24 0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge

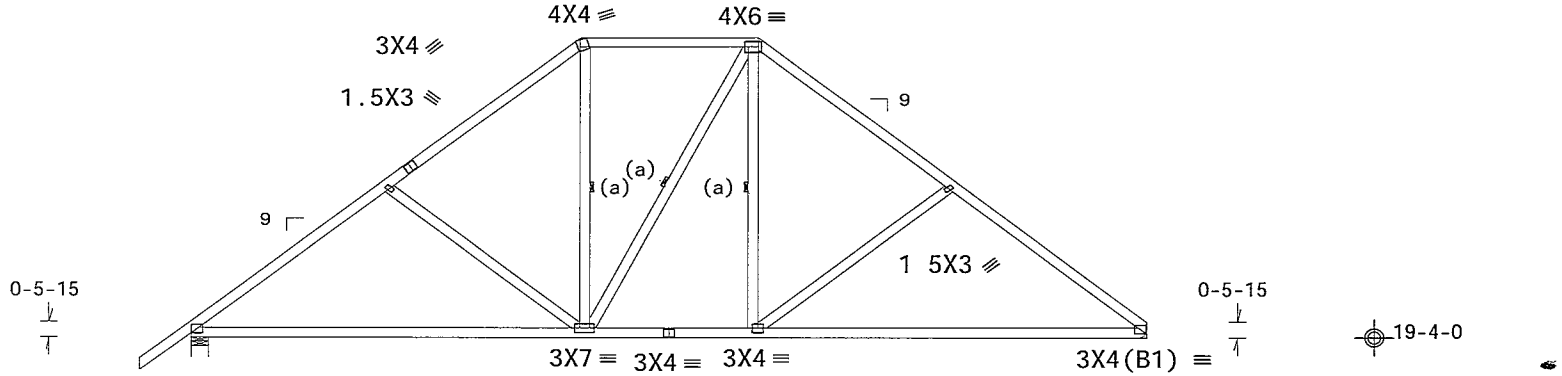
120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

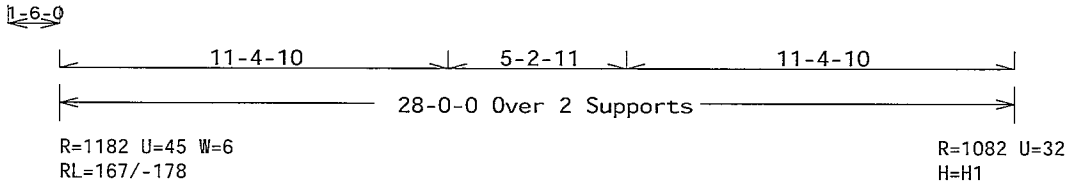
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148 x3' nails into supporting member,
(4) 0 148"x3" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1182 U=45 W=6
RL=167/-178

R=1082 U=32
H=H1

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 [illegible] 14 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17815
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133046
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371400
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - E1 28' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge

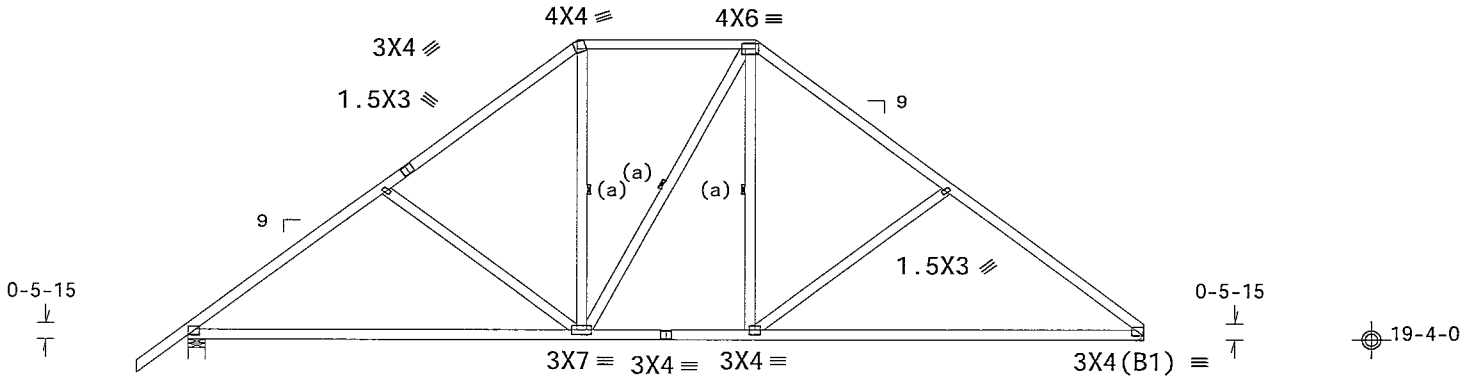
120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

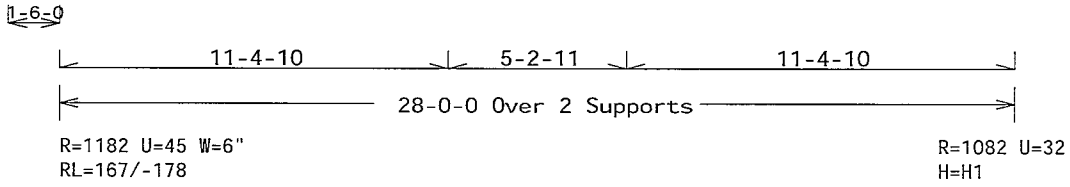
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148 x3' nails into supporting member,
(4) 0 148 x3" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 0000 14 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17816
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	371400
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E2 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

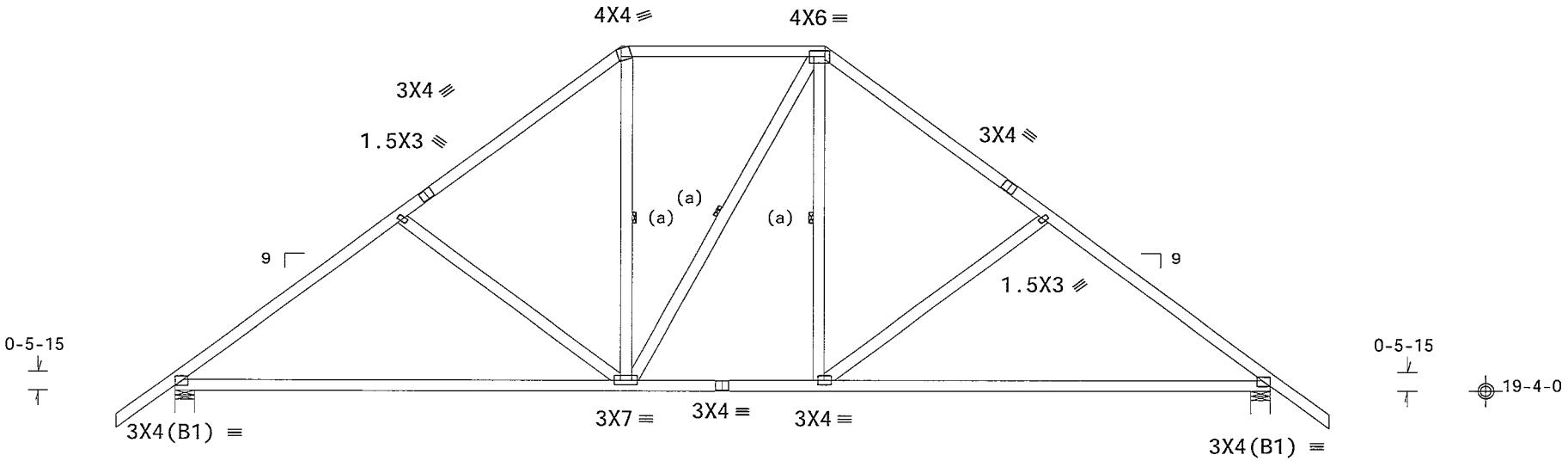
MWFRS loads based on trusses located at least 11 77 ft from roof edge

120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0828 14 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17817
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133023
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371410
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E3 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B' uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

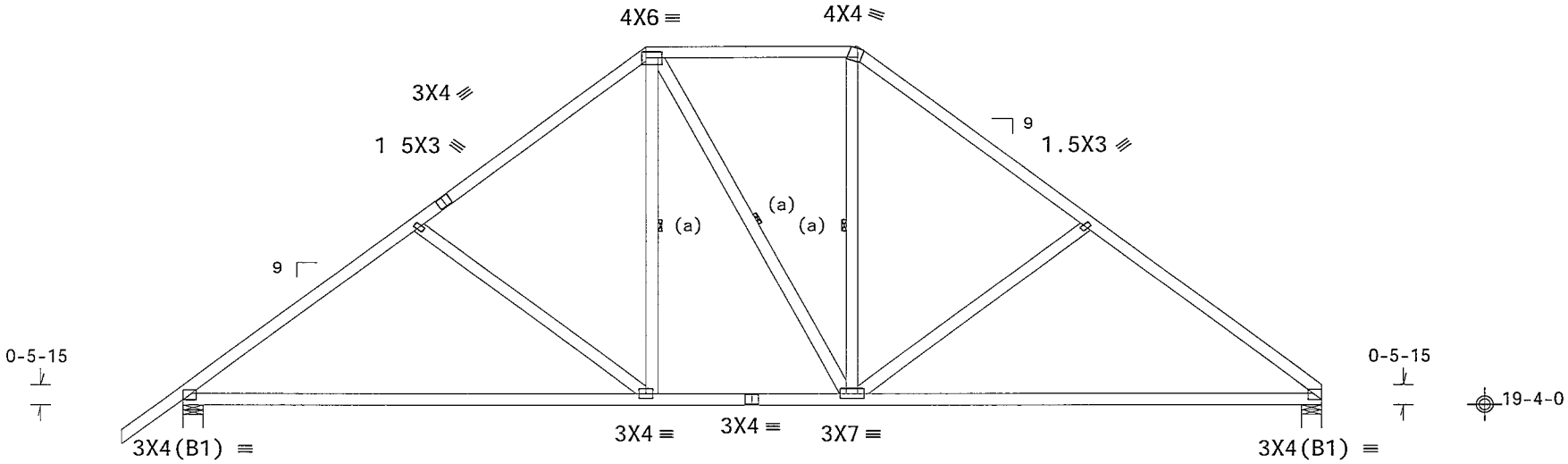
MWFRS loads based on trusses located at least 11 77 ft from roof edge

120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 0208 14 QTY.4 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17818
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133034
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371415
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E4 28' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

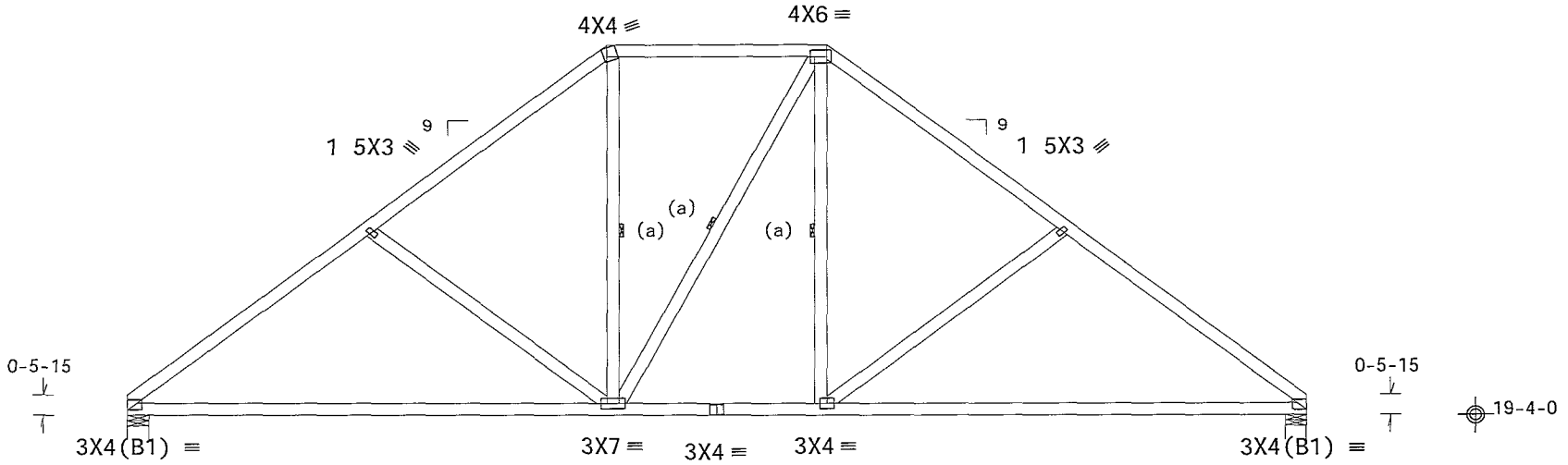
MWFRS loads based on trusses located at least 24 10 ft from roof edge

120 mph wind, 24 10 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 0208 14 QTY 6 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17819
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133030
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	371422
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E5 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

MWFRS loads based on trusses located at least 12 05 ft from roof edge

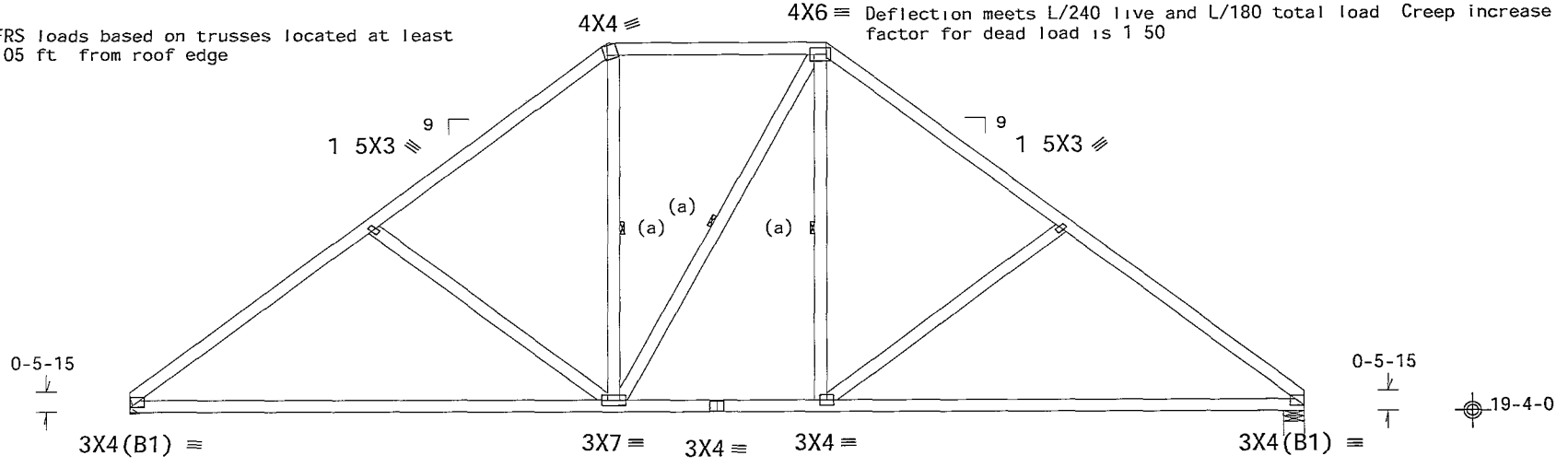
120 mph wind, 24 10 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP #2 supporting member
(14) 0 148"x3' nails into supporting member,
(4) 0 148 x3' nails into supported member

4X6 ≡ Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



11-4-10 | 5-2-11 | 11-4-10

28-0-0 Over 2 Supports

R=1085 U=32
RL=153/-153
H=H1

R=1085 U=32 W=6"

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 0328 14 QTY·5 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care in fabr cat ng handling shipping install ng and brac ng Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r gid ceiling Locat ons shown for permanent lateral restraint of web shall have brac ng installed per BCSI sections B3 B7 or B10 as applicable

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20.0 PSF	REF	R9114- 17820
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133033
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	371427
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E6 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge

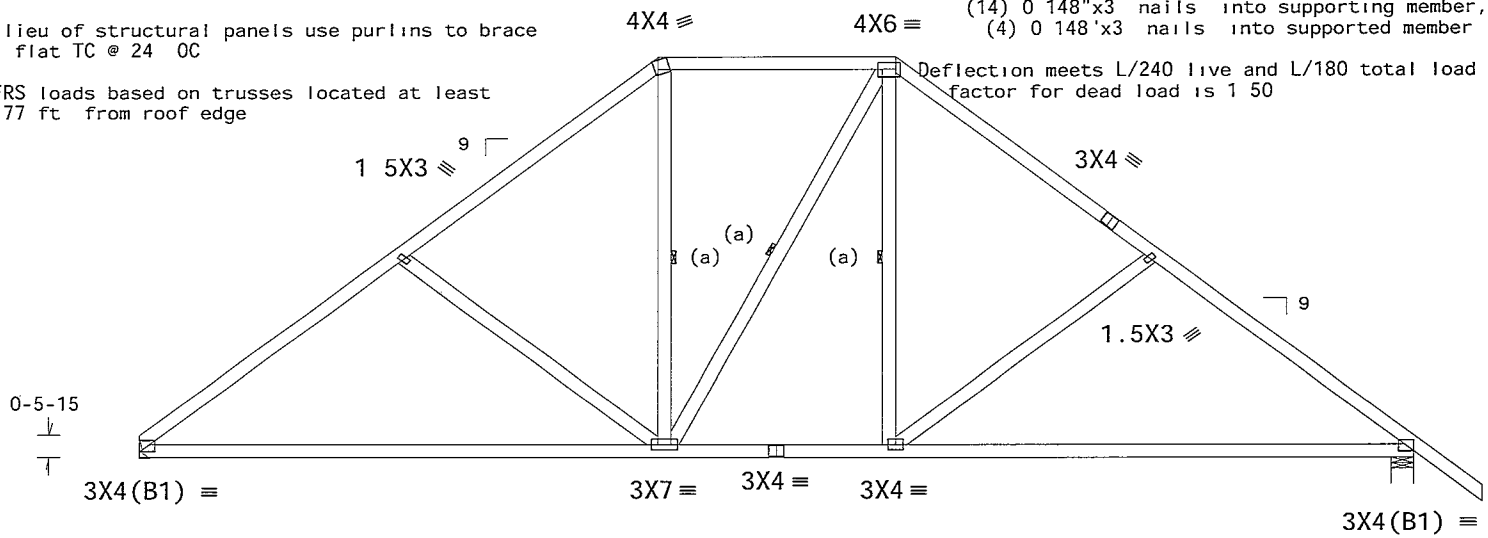
120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

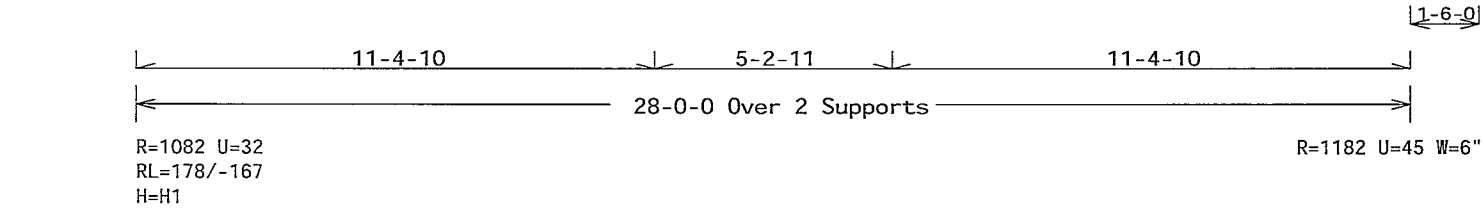
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3 nails into supporting member,
(4) 0 148'x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1082 U=32
RL=178/-167
H=H1

R=1182 U=45 W=6"

Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 13.02.07 0328.14 QTY:2 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17821
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371432
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - E7 7 End Jack)

Value Set 13B (Effective 6/1/2013)

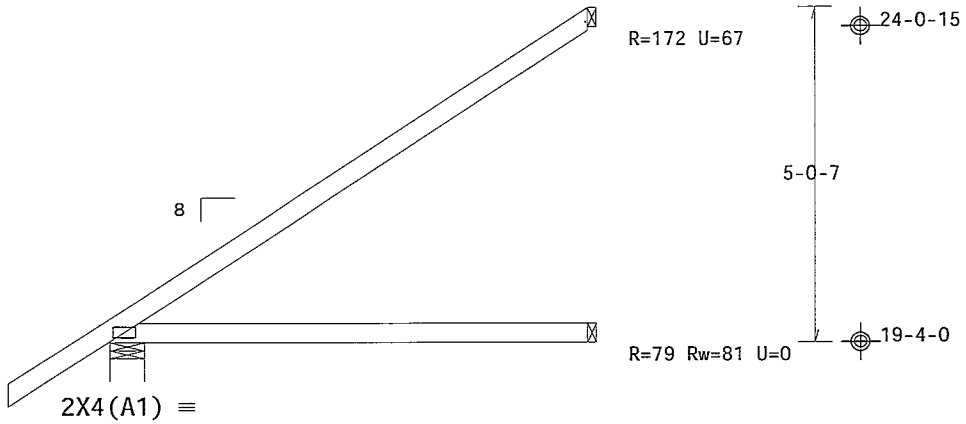
Top chord 2x4 SP #1
Bot chord 2x4 SP #1

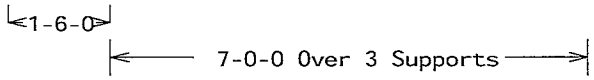
Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design





R=379 U=15 W=6'
RL=123/-44

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0328 14 QTY·20 FL/-/5/-/-/R/- Scale = 375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17822
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133027
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	369276
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - G 29 8 ·Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #2

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

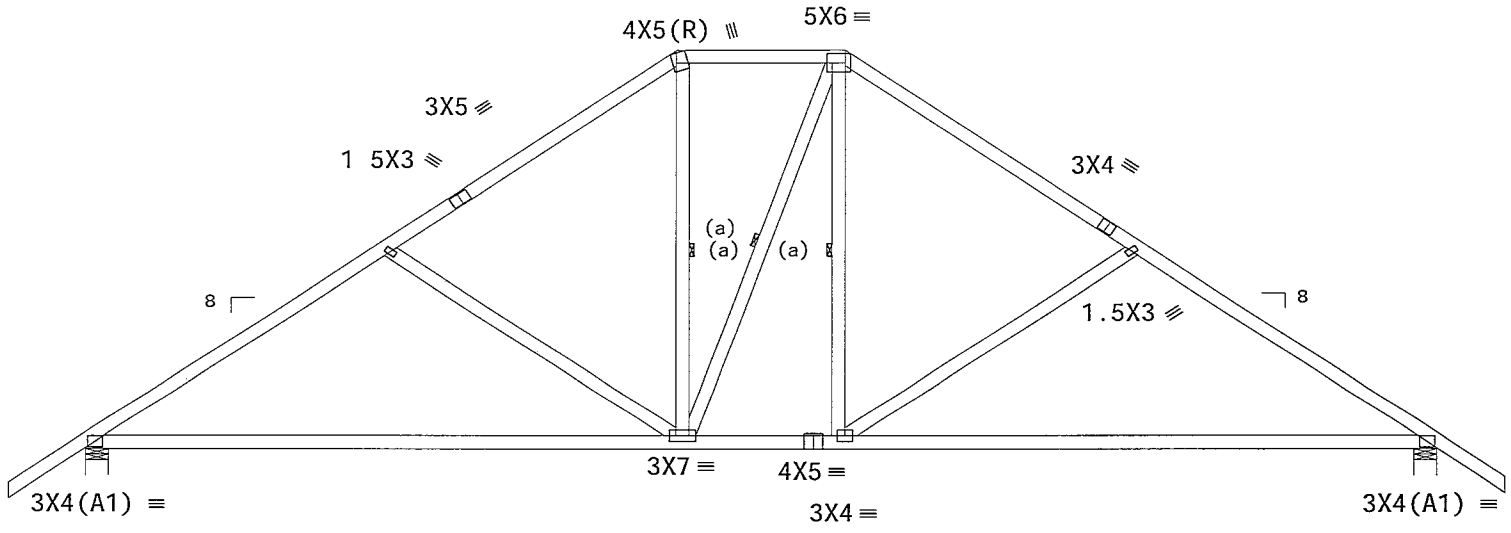
In lieu of structural panels use purlins to brace all flat TC @ 24' OC

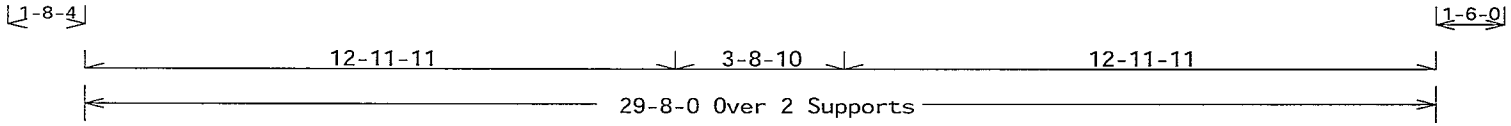
120 mph wind, 23 47 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1236 U=168 W=6
RL=186/-187

R=1232 U=164 W=6"

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0038 14 QTY 3 FL/-/5/-/-/R/- Scale =.25"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17823
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133001
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37 0 PSF	SEQN-	371704
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - GDGE 29 8 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #2
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

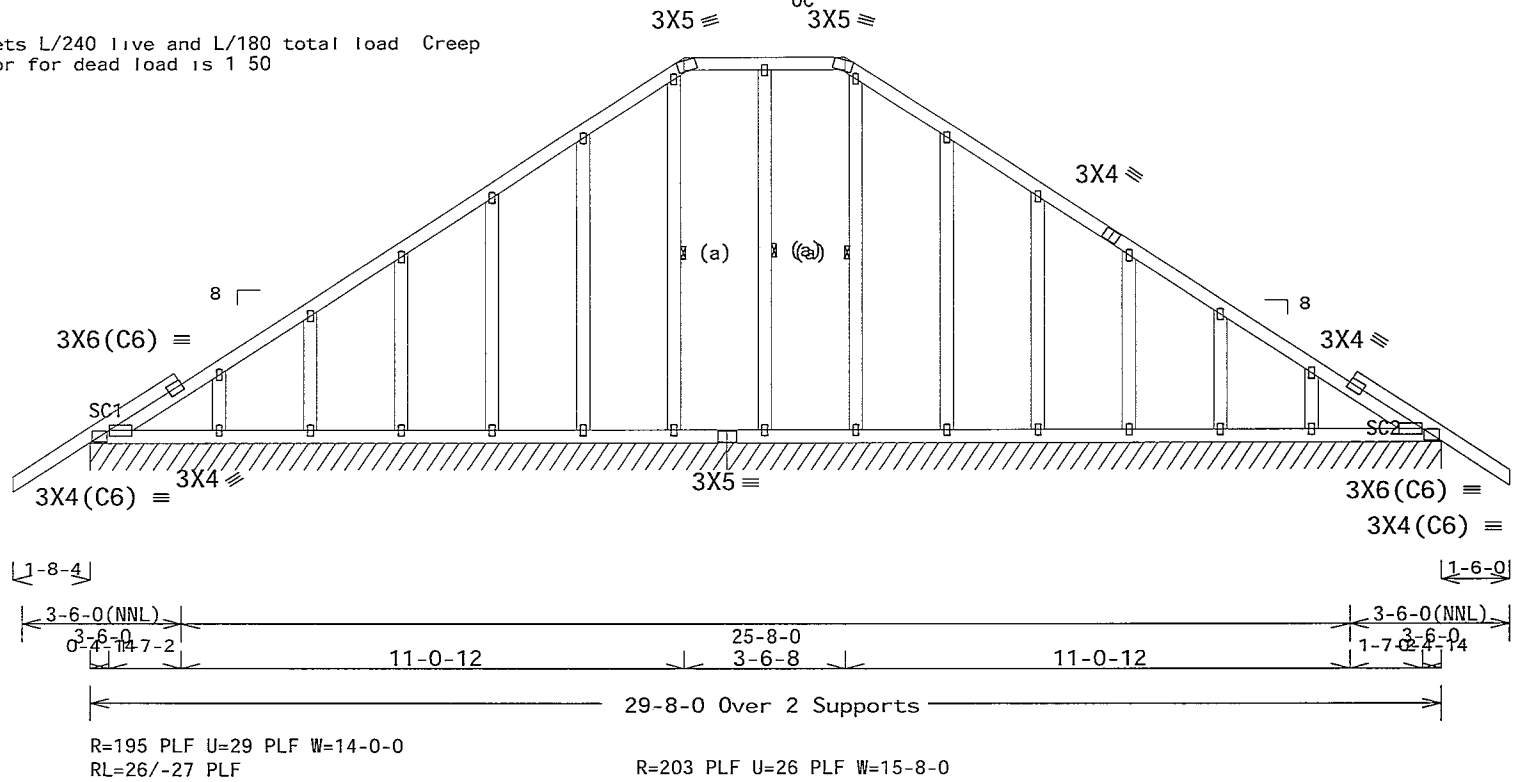
120 mph wind, 23 32 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0' OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC



R=195 PLF U=29 PLF W=14-0-0
RL=26/-27 PLF

R=203 PLF U=26 PLF W=15-8-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 QTY 1 FL/-/5/-/-/R/- Scale = 25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17824
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133002
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD	37.0 PSF	SEQN-	371700
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - H11 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 11 44 ft from roof edge

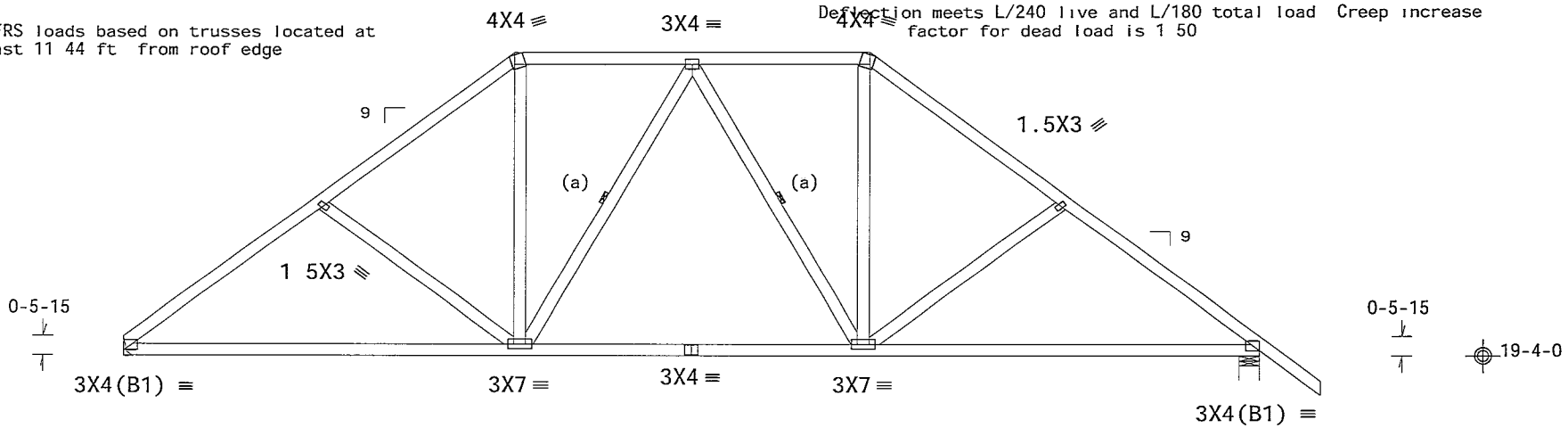
120 mph wind, 22 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

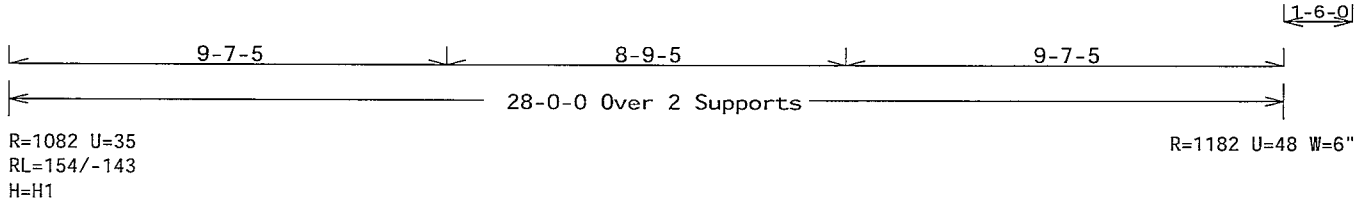
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148'x3' nails into supporting member,
(4) 0 148'x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1082 U=35
RL=154/-143
H=H1

R=1182 U=48 W=6"

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17825
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	371443
DUR.FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - H11A 28 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MWFRS loads based on trusses located at least 11 44 ft from roof edge

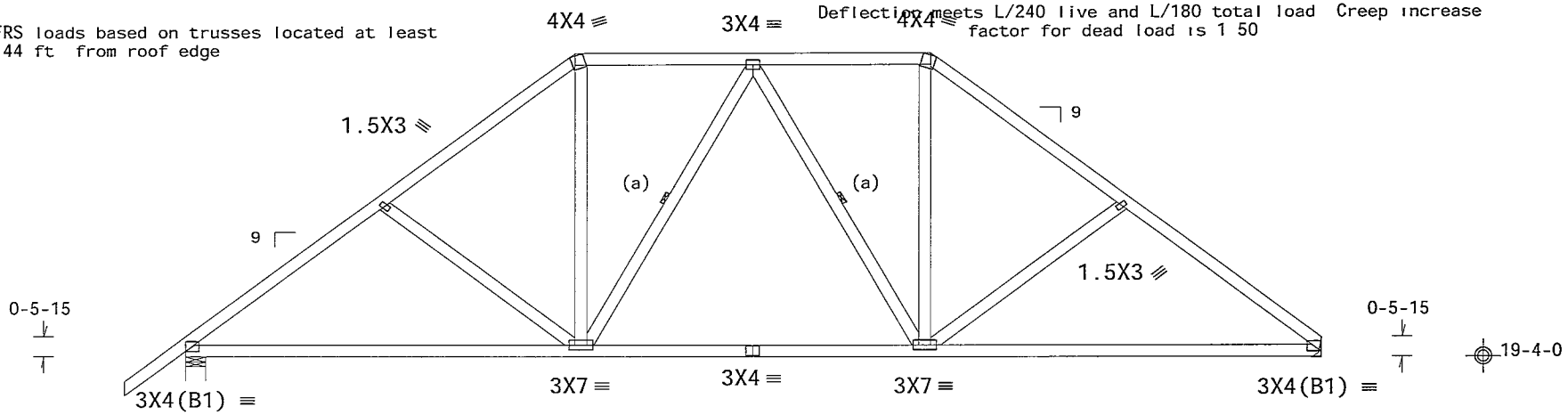
120 mph wind 22 87 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3" nails into supporting member,
(4) 0 148"x3" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 [illegible] QTY·1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17826
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371389
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - H13 28' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge

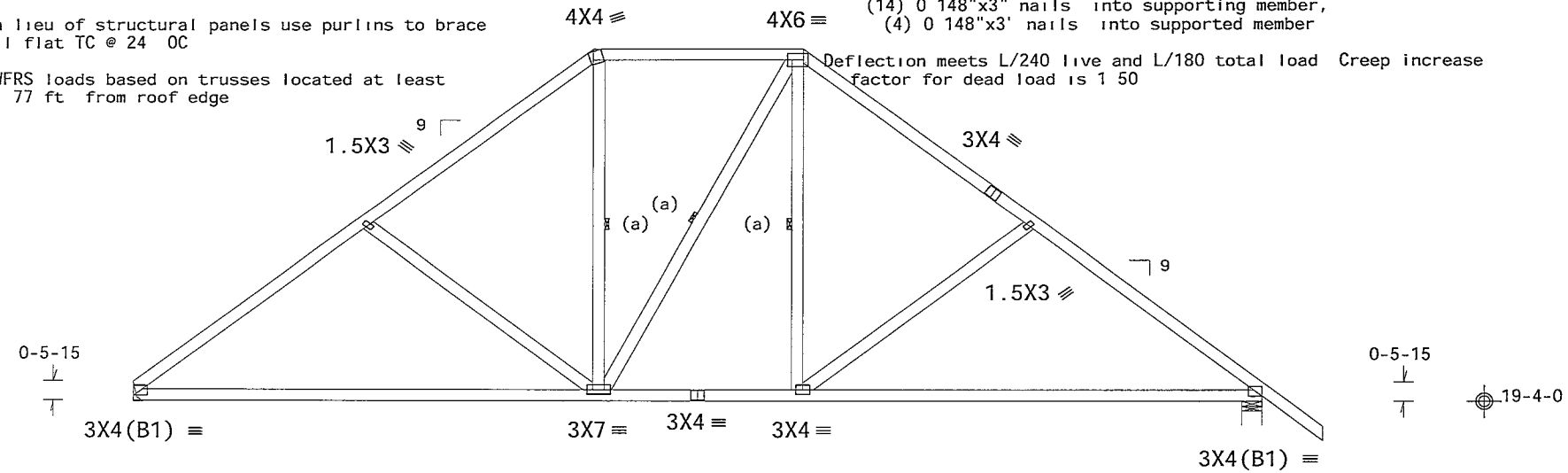
120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

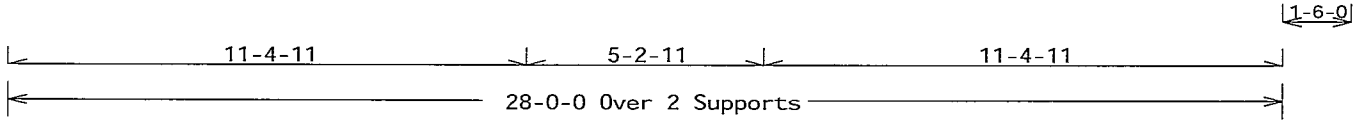
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3" nails into supporting member,
(4) 0 148"x3' nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1082 U=32
RL=178/-167
H=H1

R=1182 U=45 W=6"

Design Crit· FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handl ng shipping install ng and brac ng Refer to and follow the latest edit on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety practices pr or to perform ng these funct ons Installers shall provide temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Bu lding Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from this desig any failure to build the truss in conformance with ANSI/TPI 1 or for handl ng sh pping nstallation & brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Joint Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate posit ons A seal on this draw ng or cover page list ng this drawing indicates acceptance of profess onal eng neer ng responsibility solely for the design shown The suitabil ty and use of th s des gn for any structure is the responsibility of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW-BCG www itwbcg com TPI www tp nst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20 0 PSF	REF	R9114- 17827
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371438
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - H13A 28' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B' uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24' OC

MWFRS loads based on trusses located at least 11 77 ft from roof edge

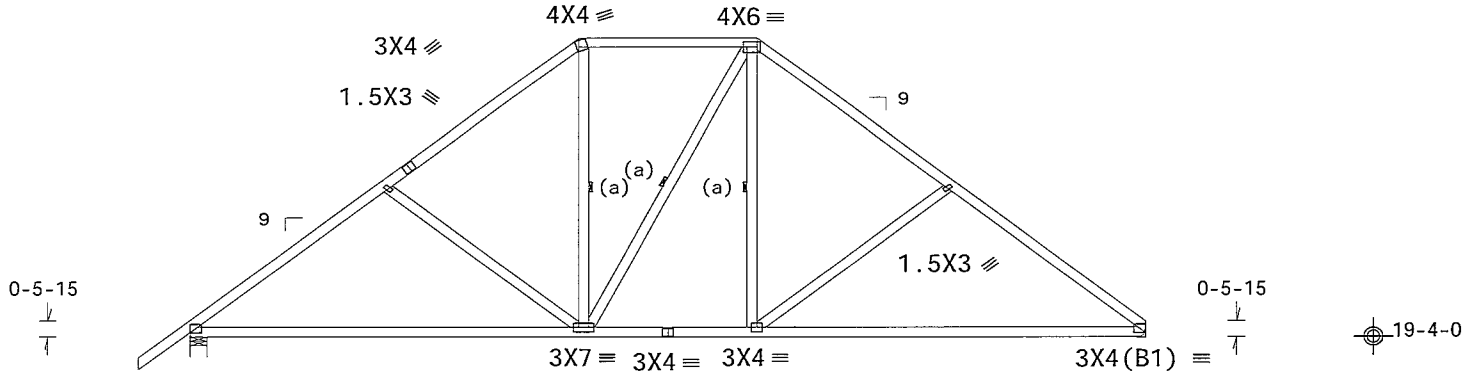
120 mph wind, 23 54 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

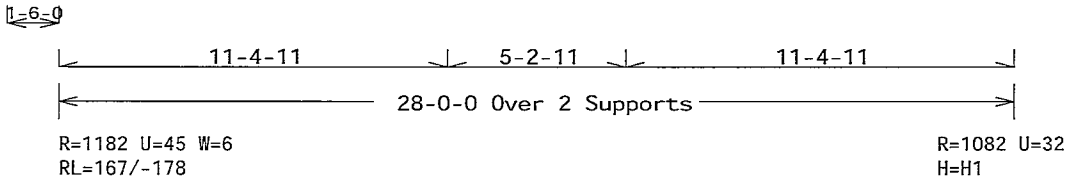
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3' nails into supporting member,
(4) 0 148"x3" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 8 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care in fabricat ng handling sh pping install ng and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r gid ceiling Locations shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev ation from th s design any failure to build the truss n conformance with ANSI/TPI 1 or for handling sh pping installation & brac ng of trusses Apply plates to each face of truss and position as shown above and on the Jo nt Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate pos t ons A seal on th s draw ng or cover page l st ng this draw ng indicates acceptance of profess onal eng neer ng responsibil ty solely for the design shown The suitab lity and use of th s design for any structure s the responsibil ty of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more nformation see This job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL 20.0 PSF	REF R9114- 17828
TC DL 7.0 PSF	DATE 05/13/14
BC DL 10.0 PSF	DRW HCUSR9114 14133041
BC LL 0.0 PSF	HC-ENG WHK/WHK
TOT.LD. 37.0 PSF	SEQN- 371394
DUR FAC. 1.25	FROM JMW
SPACING 24.0"	JREF- 1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - H7 28' Stepdown Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 B2 2x4 SP M-30
Webs 2x4 SP #3

Lumber value set 13B" uses design values approved 1/30/2013 by ALSC

120 mph wind 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

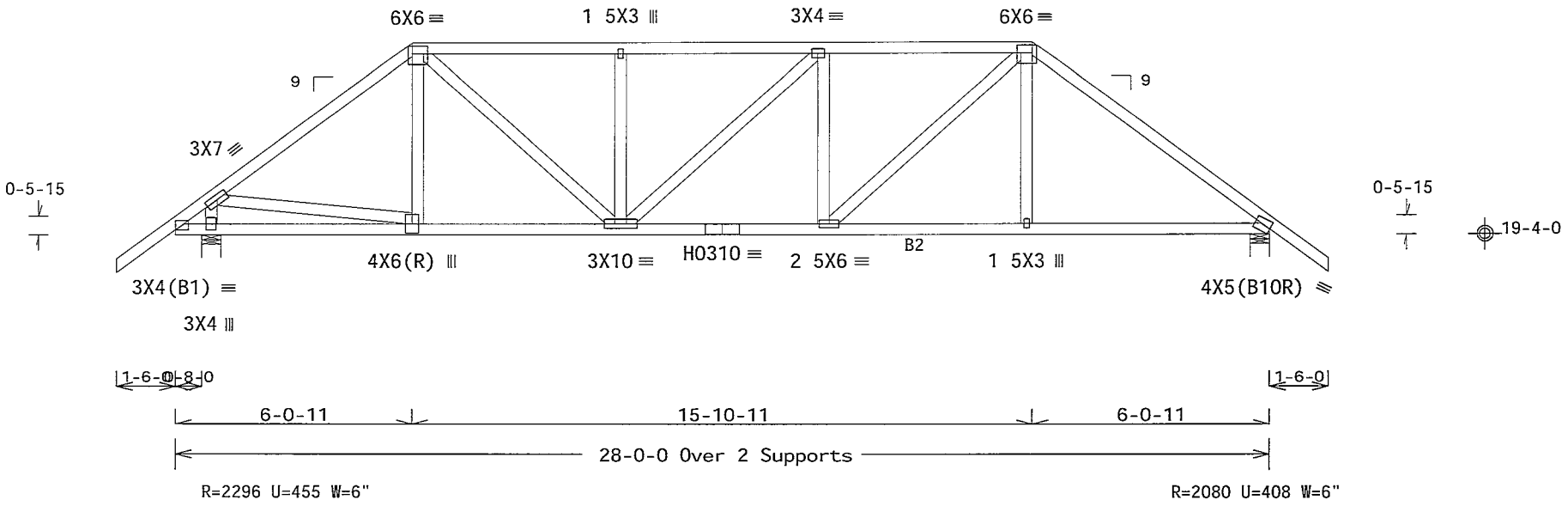
Wind loads and reactions based on MWFRS

Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 6 06
TC- From 29 plf at 6 06 to 29 plf at 21 94
TC- From 57 plf at 21 94 to 57 plf at 29 50
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 6 09
BC- From 10 plf at 6 09 to 10 plf at 21 91
BC- From 20 plf at 21 91 to 20 plf at 28 00
BC- From 5 plf at 28 00 to 5 plf at 29 50
TC- 278 33 lb Conc Load at 6 09
TC- 171 79 lb Conc Load at 8 12,10 12,12 12,14 00 15 88,17 88,19 88
BC- 280 25 lb Conc Load at 21 91
BC- 234 33 lb Conc Load at 6 09
BC- 79 29 lb Conc Load at 8 12,10 12,12 12,14 00 15 88,17 88,19 88,21 91

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 13.02.07 QTY.1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Location shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the trusses in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page stating this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job's general notes page ITW-BCG www.itwbcg.com TPI www.tpinst.org WTCA www.sbcindustry.com ICC www.iccsafe.org

WALTER P FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20.0 PSF	REF	R9114- 17829
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133032
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT LD.	37.0 PSF	SEQN-	369853
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - H9 28' Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

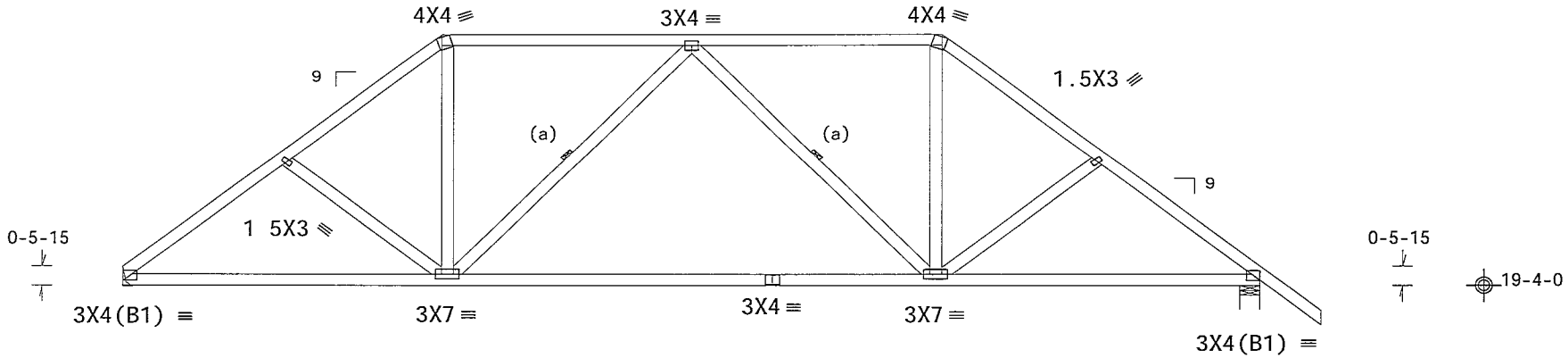
120 mph wind, 22 20 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

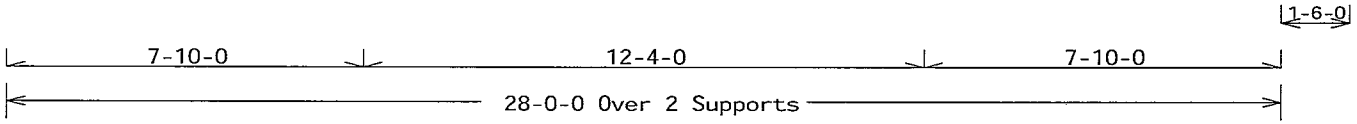
Wind loads and reactions based on MWFRS with additional C&C member design

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3 nails into supporting member,
(4) 0 148 x3 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=1082 U=120
RL=129/-118
H=H1

R=1182 U=143 W=6"

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care n fabr cat ng handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall provide temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons b e for any dev at on from th s des any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling sh pp ng installat on & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Details unless noted otherwise Refer to drawings 160A-Z for standard plate pos tions A seal on this drawing or cover page listing th s drawing ind cates acceptance of professional engineering respons b l ty solely for the des gn shown The su tab l ty and use of this des gn for any structure is the responsib lity of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www twbcg com TPI www tpinst org WTCA www sbcindustry com ICC www ccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20.0 PSF	REF	R9114- 17830
TC DL	7 0 PSF	DATE	05/13/14
BC DL	10 0 PSF	DRW	HCUSR9114 14133013
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371124
DUR.FAC	1.25	FROM	JMW
SPACING	24 0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - HJ7 9 1 6 Hip Jack Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

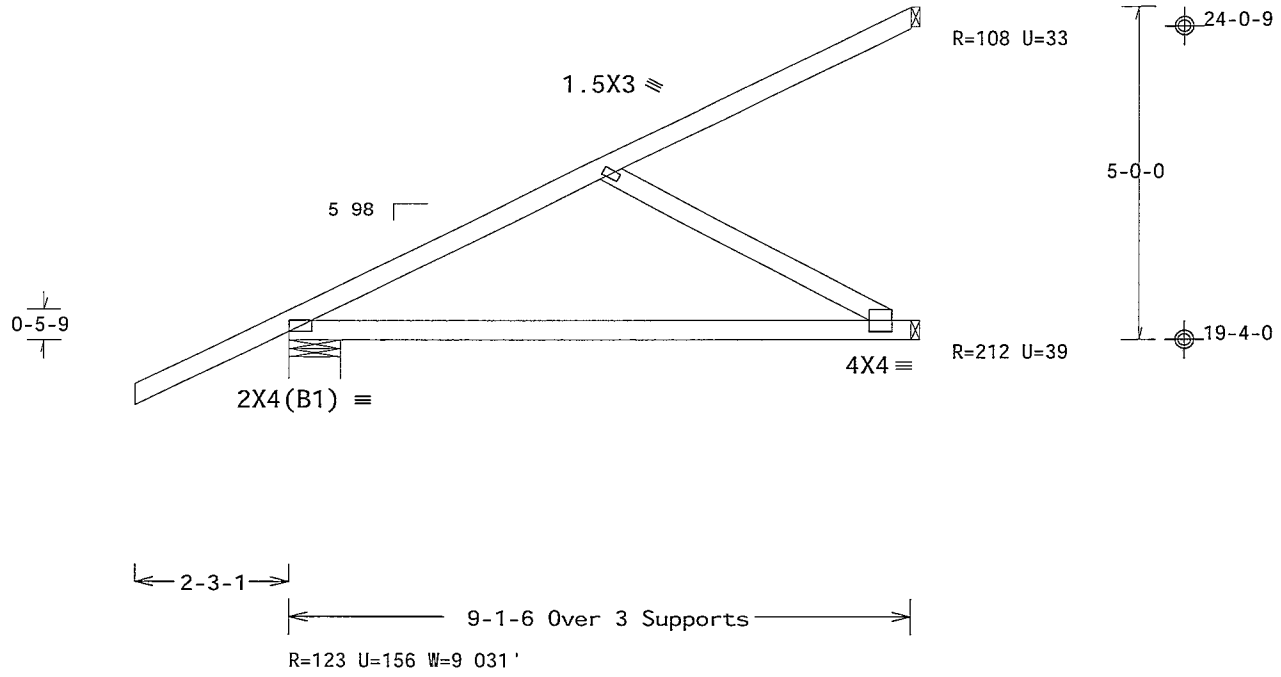
Lumber value set '13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 21 50 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 26 to 56 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 11
BC- From 0 plf at -2 26 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 11
TC- -93 91 lb Conc Load at 1 15
TC- -37 94 lb Conc Load at 1 32
TC- 40 78 lb Conc Load at 3 83
TC- 69 17 lb Conc Load at 4 33
TC- 99 98 lb Conc Load at 6 50
TC- 134 58 lb Conc Load at 7 34
BC- -14 64 lb Conc Load at 1 15
BC- -9 17 lb Conc Load at 1 32
BC- 17 97 lb Conc Load at 3 83
BC- 29 23 lb Conc Load at 4 33
BC- 42 08 lb Conc Load at 6 50
BC- 60 85 lb Conc Load at 7 34



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 QTY 2 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from th s des any failure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Details unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons A sea on th s drawing or cover page l sting th s draw ng nd cates acceptance of professional eng neer ng respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any structure s the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www twbcg com TPI www tp nst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/13/2014

TC LL	20 0 PSF	REF	R9114- 17831
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10 0 PSF	DRW	HCUSR9114 14133036
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	369383
DUR FAC	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - HJ7A 9 4 6 Hip Jack Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B' uses design values approved 1/30/2013 by ALSC

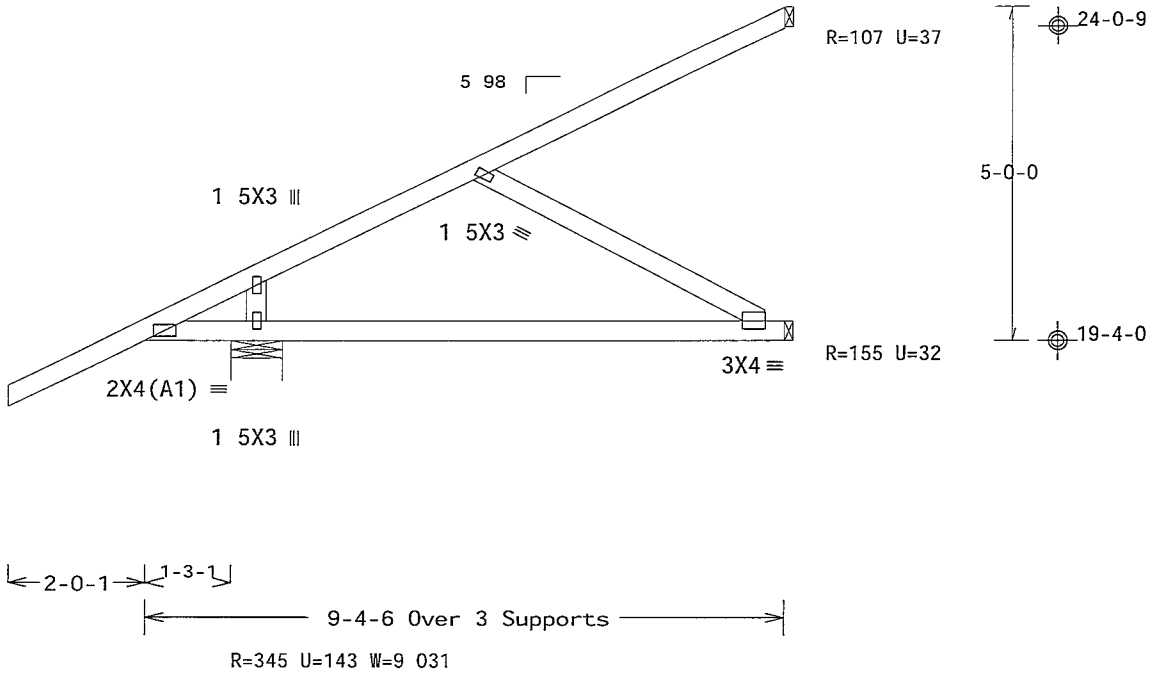
120 mph wind, 21 50 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Left cantilever is exposed to wind

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 01 to 56 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 37
BC- From 0 plf at -2 01 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 9 37
TC- 0 54 lb Conc Load at 1 40
TC- -37 94 lb Conc Load at 1 57
TC- 13 58 lb Conc Load at 4 08
TC- 69 17 lb Conc Load at 4 58
TC- 74 54 lb Conc Load at 6 75
TC- 134 58 lb Conc Load at 7 59
BC- 149 34 lb Conc Load at 1 40
BC- -9 17 lb Conc Load at 1 57
BC- -15 84 lb Conc Load at 4 08
BC- 29 23 lb Conc Load at 4 58
BC- 24 08 lb Conc Load at 6 75
BC- 60 85 lb Conc Load at 7 59



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0328 14 QTY 1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviat on from th s design any fa lure to bu ld the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng installat on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions A seal on th drawing or cover page l sting th s drawing nd cates acceptance of profess onal eng neering respons b l ty solely for the des gn shown The su tab l ty and use of this design for any structure is the respons b l ty of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20.0 PSF	REF	R9114- 17832
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10 0 PSF	DRW	HCUSR9114 14133045
BC LL	0 0 PSF	HC-ENG	SSB/WPF
TOT LD.	37.0 PSF	SEQN-	369842
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - MH7 28 Mono Hip Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP 2850f-2 3E T1 2x4 SP #1
Bot chord 2x4 SP M-30
Webs 2x4 SP #3 W6 2x4 SP #2
Lt Wedge 2x4 SP #3 Lt Splice Block 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

120 mph wind, 21 54 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 162'x3 5' nails into supporting member,
(4) 0 162 x3 5 nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

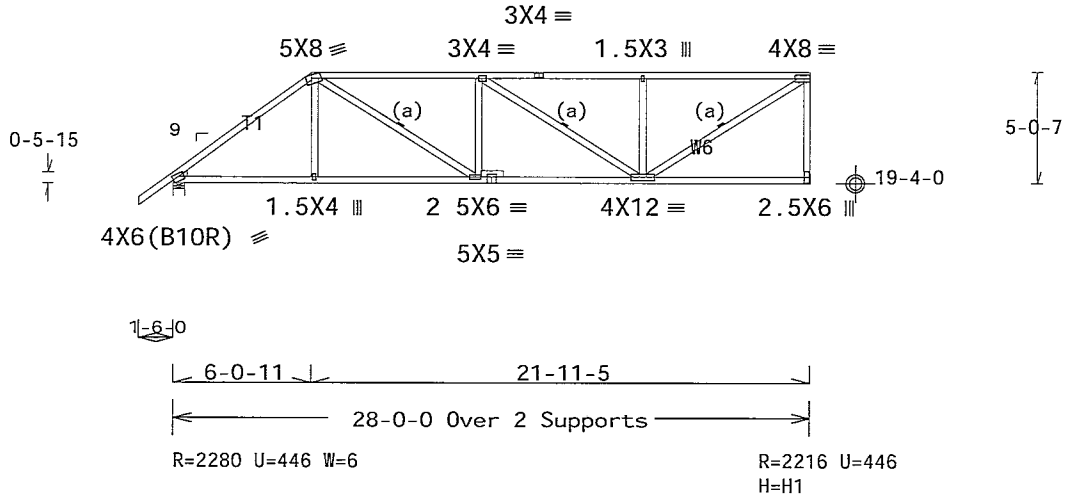
Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at -1 50 to 57 plf at 6 06
TC- From 29 plf at 6 06 to 29 plf at 28 00
BC- From 5 plf at -1 50 to 5 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 6 09
BC- From 10 plf at 6 09 to 10 plf at 28 00
TC- 280 25 lb Conc Load at 6 09
TC- 171 79 lb Conc Load at 8 12,10 12,12 12,14 12 16 12,18 12,20 12,22 12,24 12,26 12
BC- 291 04 lb Conc Load at 6 09
BC- 79 29 lb Conc Load at 8 12,10 12,12 12,14 12 16 12,18 12,20 12,22 12,24 12,26 12

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 0328 14 QTY:1 FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstalling and bracing Refer to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as appl cable

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/13/2014

TC LL	20 0 PSF	REF	R9114- 17833
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133044
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37.0 PSF	SEQN-	369544
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - MH9 28' Mono Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP M-30 B2 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP SS supporting member
(14) 0 148"x3" nails into supporting member,
(4) 0 148"x3' nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 22 20 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

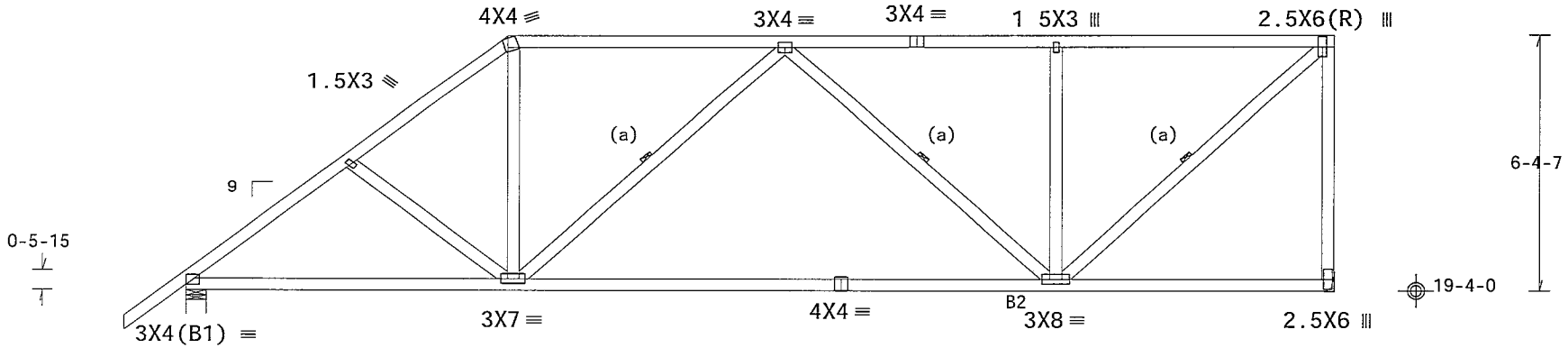
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 0328.14 QTY 1 FL/-/5/-/-/R/- Scale = 25"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17834
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371141
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - PBD 5'7 8 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set "13B uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

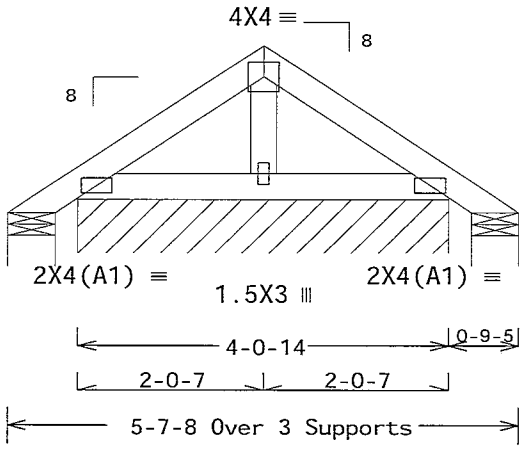
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 2 81
TC- From 57 plf at 2 81 to 57 plf at 5 62
BC- From 4 plf at 0 00 to 4 plf at 5 62

120 mph wind, 29 32 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC



28-6-6
28-4-10

R=4 Rw=18 U=19 W=6 31"
RL=31/-31

R=74 PLF U=27 PLF W=4-0-14

R=4 U=1 W=6 31

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 0328 14 QTY.1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17835
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133025
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37.0 PSF	SEQN-	369390
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - PBD1 2'8 3 Common Girder)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

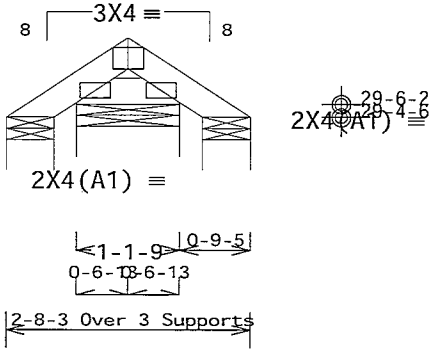
Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 1 34
TC- From 57 plf at 1 34 to 57 plf at 2 68
BC- From 4 plf at 0 00 to 4 plf at 2 68

120 mph wind, 29 81 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18



Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 0328 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17836
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133029
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	371137
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - PBDG 5 7 8 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

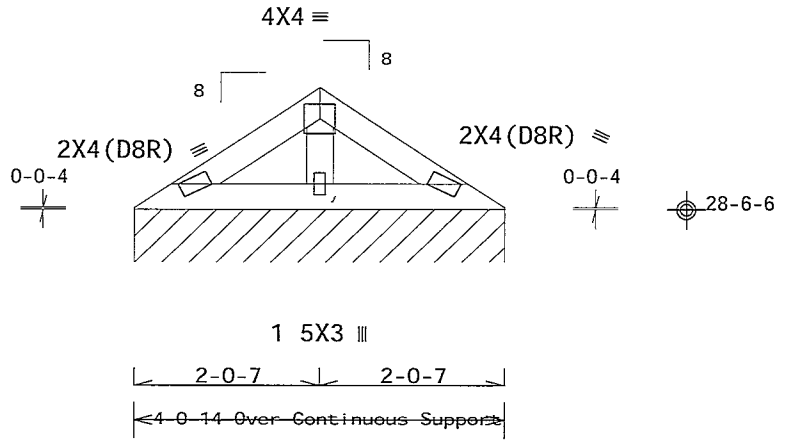
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

120 mph wind, 29 36 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements



R=60 PLT U=15 PLT W=4-0-14

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07-22-44 QTY 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17837
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133004
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	369537
DUR.FAC.	1 25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - PBE 5 2 11 Stepdown Hip)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24 OC, all BC @ 24' OC

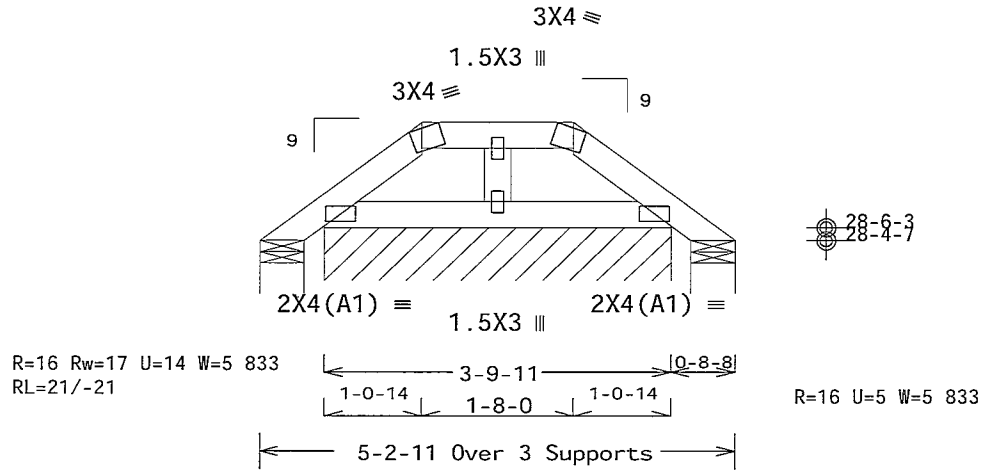
MWFRS loads based on trusses located at least 14 52 ft from roof edge

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 1 78
TC- From 57 plf at 1 78 to 57 plf at 3 45
TC- From 57 plf at 3 45 to 57 plf at 5 23
BC- From 4 plf at 0 00 to 4 plf at 5 23

120 mph wind 29 04 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02 07 QTY 2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17838
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133047
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37.0 PSF	SEQN-	371455
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - PBE1 5 2 11 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

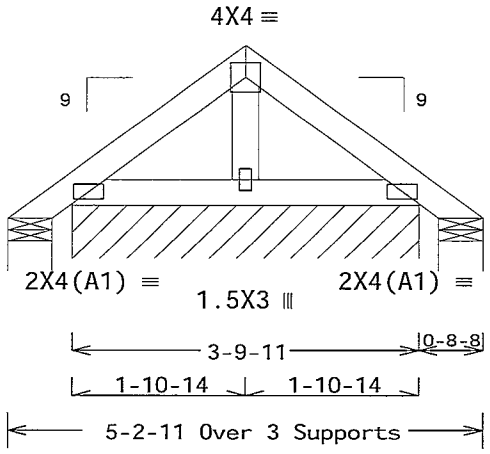
Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 2 61
TC- From 57 plf at 2 61 to 57 plf at 5 23
BC- From 4 plf at 0 00 to 4 plf at 5 23

120 mph wind, 29 35 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

MWFRS loads based on trusses located at least 14 67 ft from roof edge



28-6-3
28-4-7

R=4 Rw=22 U=22 W=5 833
RL=33/-33

R=4 U=1 W=5 833'

R=75 PLF U=11 PLF W=3-9-11

Design Crit' FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07 [illegible]44 QTY:18 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17839
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	371452
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City, F - PBG 3'8"10 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

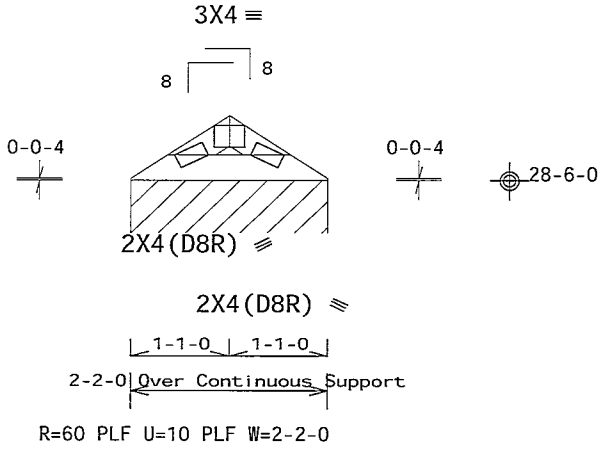
Lumber value set '13B" uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

120 mph wind, 29 02 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design



Design Crit. FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02.07 [illegible]44 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care n fabricating handling shipp ng nstalling and bracing Refer to and follow the latest ed tion of BCSI (Building Component Safety Information by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherw se top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rig d ce l ng Locations shown for permanent lateral restra nt of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s des any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handling shipp ng installat on brac ng of trusses Apply plates to each face of truss and posit on as shown above and on the Jo nt Detai s unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A seal on this drawing or cover page l st ng th s draw ng nd cates acceptance of professional eng neering respons b l ty solely for the des gn shown The su tab l ity and use of this design for any structure s the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

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05/13/2014

TC LL	20.0 PSF	REF	R9114- 17840
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133016
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD	37 0 PSF	SEQN-	369621
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(13-288E1--OWNER BUILDER /Becker Res Roof -- 337 sw rose creek drive Lake City F - PBG1 3 8'10 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set '13B uses design values approved 1/30/2013 by ALSC

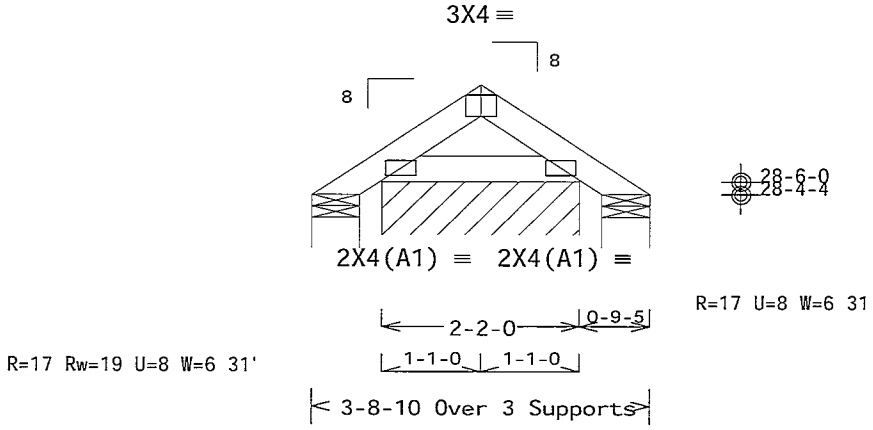
Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 57 plf at 0 00 to 57 plf at 1 86
TC- From 57 plf at 1 86 to 57 plf at 3 72
BC- From 4 plf at 0 00 to 4 plf at 3 72

120 mph wind 28 97 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18



R=17 U=8 W=6 31

R=17 Rw=19 U=8 W=6 31'

R=74 PLF U=22 PLF W=2-2-0

Design Crit FBC2010Com/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 QTY 3 FL/-/5/-/-/R/- Scale =.5"/Ft.

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Orlando FL, 32837
FL COA #0 278

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05/13/2014

TC LL	20 0 PSF	REF	R9114- 17841
TC DL	7.0 PSF	DATE	05/13/14
BC DL	10.0 PSF	DRW	HCUSR9114 14133038
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	371128
DUR FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V6C487_Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum alternative reinforcement, re-run design with appropriate reinforcement type

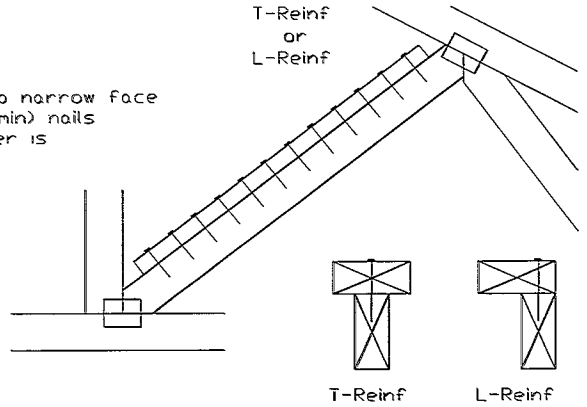
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer s sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

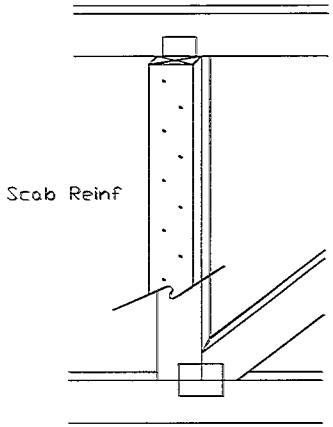
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0128"x3 0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0128"x3 0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



TW
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Earth City MO 63045

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05/13/2014

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed, Exposure C Kzt = 1.00
Or 100 mph Wind Speed 15 Mean Height Enclosed Exposure D Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" oc	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" oc	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" oc	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades.

Group A

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

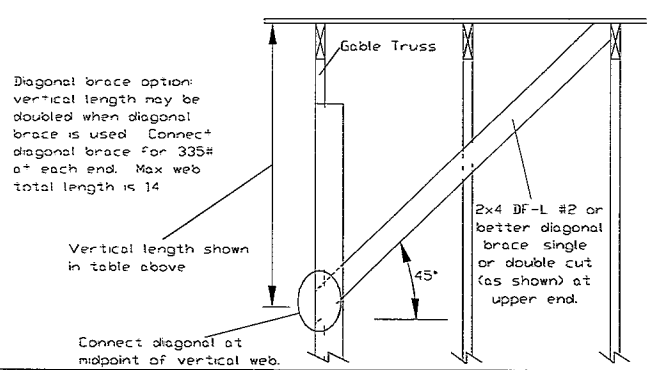
Group B

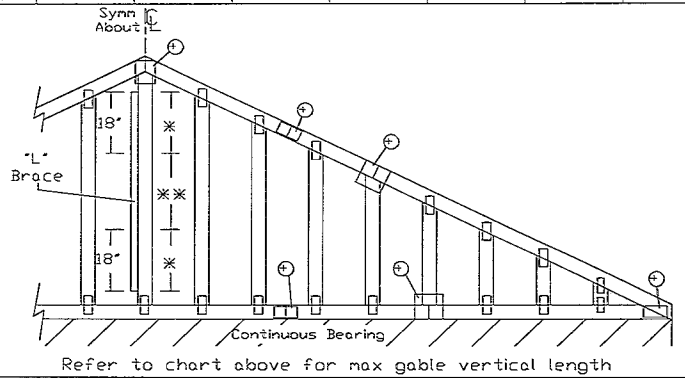
Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards Group B values may be used with these grades.





Refer to chart above for max gable vertical length

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail

TW Building Components Group Inc.

*Earth City MO 63045

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14

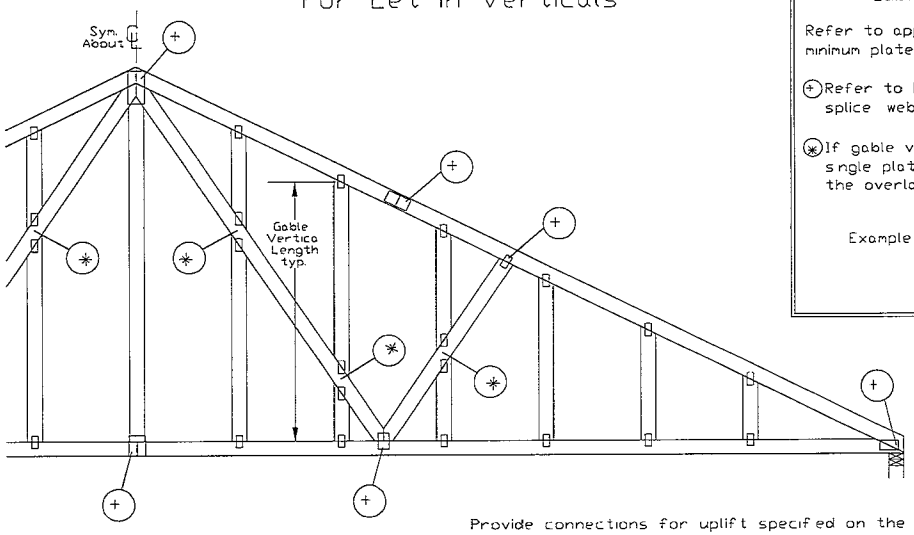
MAX TOT LD 60 PSF

MAX SPACING 24.0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

05/13/2014

Gable Detail
For Let-in Verticals

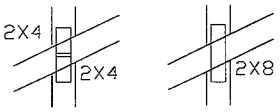


Gable Truss Plate Sizes

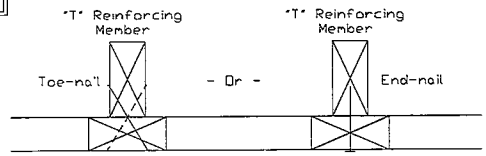
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

⊕ Refer to Engineered truss design for peak splice web and heel plates

⊛ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail)

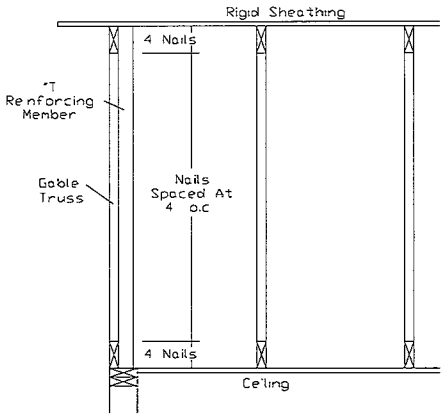
Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"o.c SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3" min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212 A20015PED100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length

Building Components Group Inc.

Earth City MO 63045

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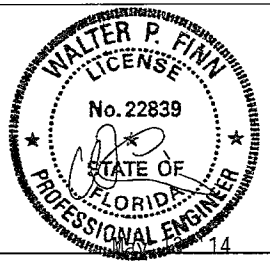
ITWBCG: www.itwbcg.com, TPI: www.tpinst.org, WTCA: www.sbcindustry.org, ICC: www.iccsafe.org



MAX TOT LD 60 PSF
DUR FAC ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

05/13/2014

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00 Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates and other information not shown on this detail

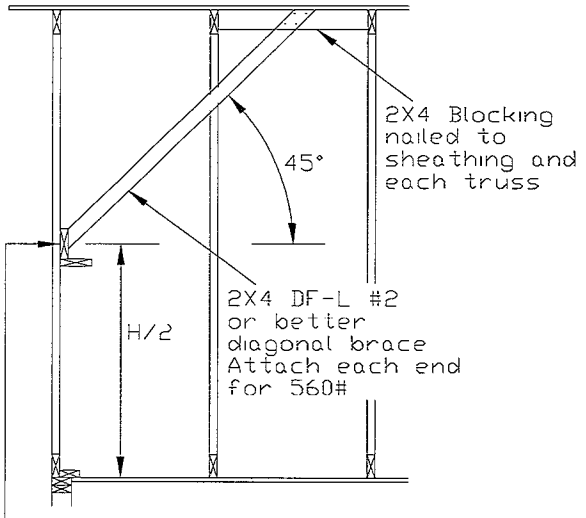
Nails 10d box or gun (0.128"x3",min) nails

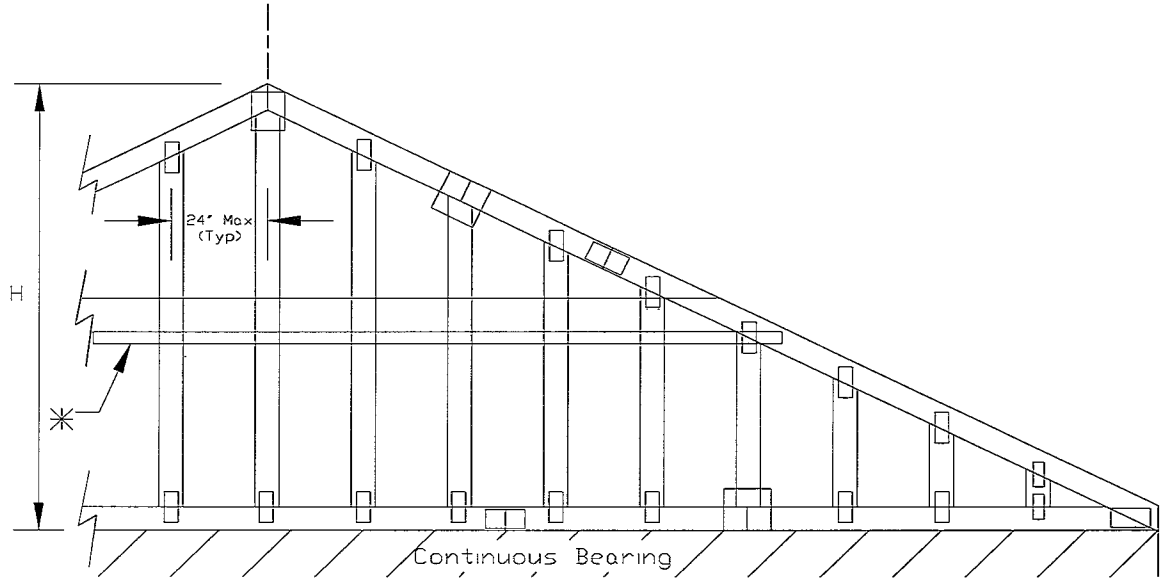
H Less than 4'6" no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c





TW
Building Components Group Inc.

Earth City MO 63045

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org, WTCA: www.sbcindustry.org, ICC: www.iccsafe.org

WALTER P. FINN
LICENSE
No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
5/13/14

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

05/13/2014

Piggyback Detail - ASCE 7-10 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

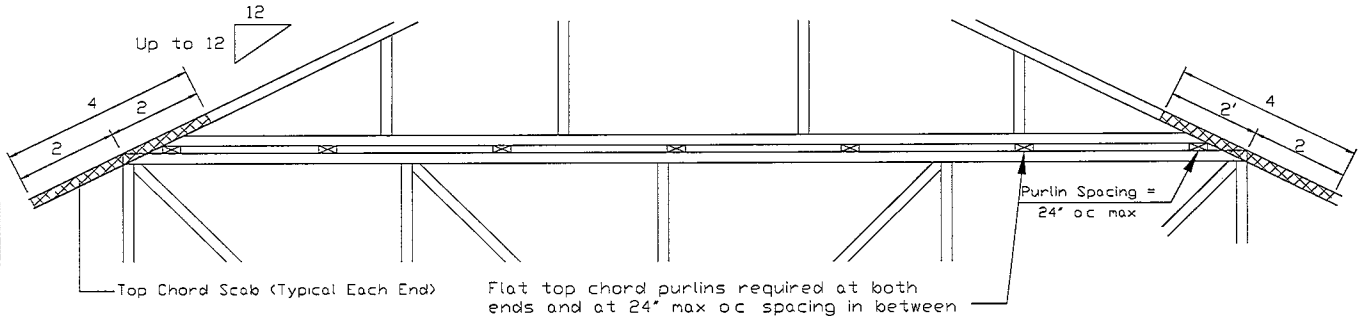
160 mph Wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp C Wind DL= 5.0 psf (min) Kzt=1.0
Or 140 mph wnd 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere n roof Exp D wind DL= 5.0 psf (mn) Kzt=1.0

Note Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins The building Engineer of Record shall provide diagonal brac ng or any other suitable anchorage to permanently restrain purlns and lateral bracing for out of plane loads over gable ends

Maximum truss spacing is 24" o c detal s not applicable if cap supports additional loads such as cupola steeple chimney or drag strut loads

** Refer to Engineer s sealed truss design drawing for piggyback and base truss specificat ons

Detail A Purlin Spacing = 24" o c or less



P ggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c

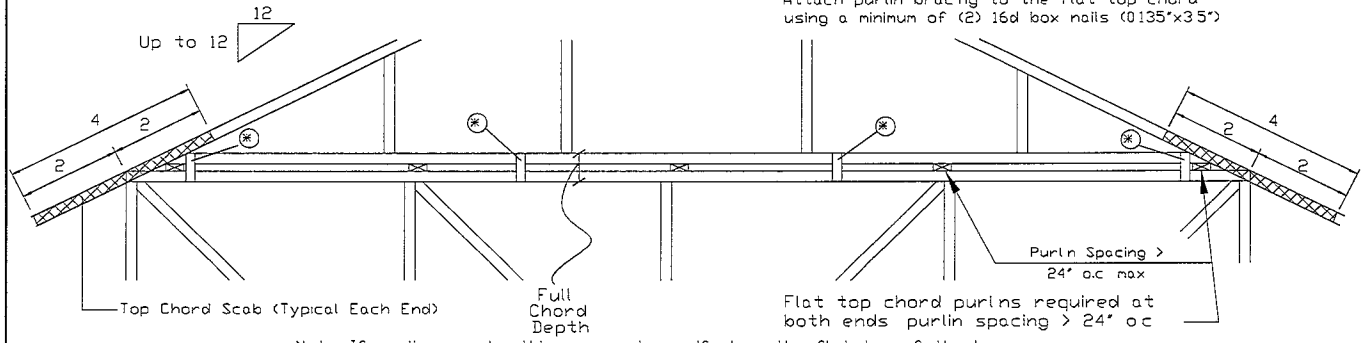
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5")

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails Note Nailing thru holes of wave plate is acceptable

Detail B Purlin Spacing > 24" o c

Piggyback cap truss slant nailed to all top chord purlin brac ng w th (2) 16d box nails (0.135"x3.5") and secure top chord w th 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c

Attach purlin brac ng to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")



Note If purlins or sheathing are not specified on the flat top of the base truss purlins must be installed at 24" o c max and use Detail A

* In addition provide connection with one of the following methods

Trulox
Use 3X8 Trulox plates for 2x4 chord member and 3X10 Trulox plates for 2x6 and larger chord members Attach to each face @ 8 o.c with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord Trulox plates may be staggered 4 o.c front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face) Attach @ 8 o.c with (8) 6d common (0.113"x2") nails per gusset (4) in cap bottom chord and (4) in base truss top chord Gussets may be staggered 4 o.c front to back faces

2x4 Vertical Scabs
2x4 SPF #2 full chord depth scabs (each face). Attach @ 8 o.c with (6) 10d box nails (0.128"x3") per scab (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4 o.c front to back faces

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8 o.c Attach teeth to piggyback at time of fabr cation Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply P ggyback plates may be staggered 4' o.c front to back faces

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ITWBCG: www.twbcg.com, TPI: www.tpinst.org, WTCA: www.sbcindustry.org ICC: www.iccsafe.org

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14

REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212
SPACING	24.0"

05/13/2014