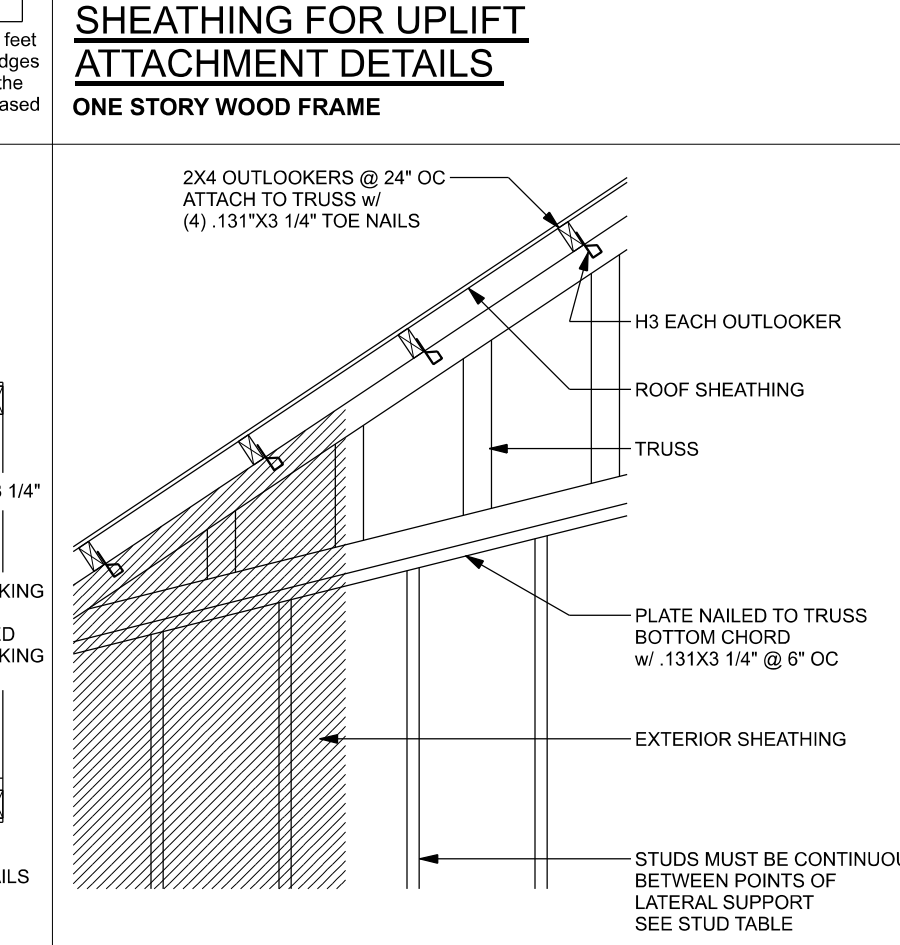
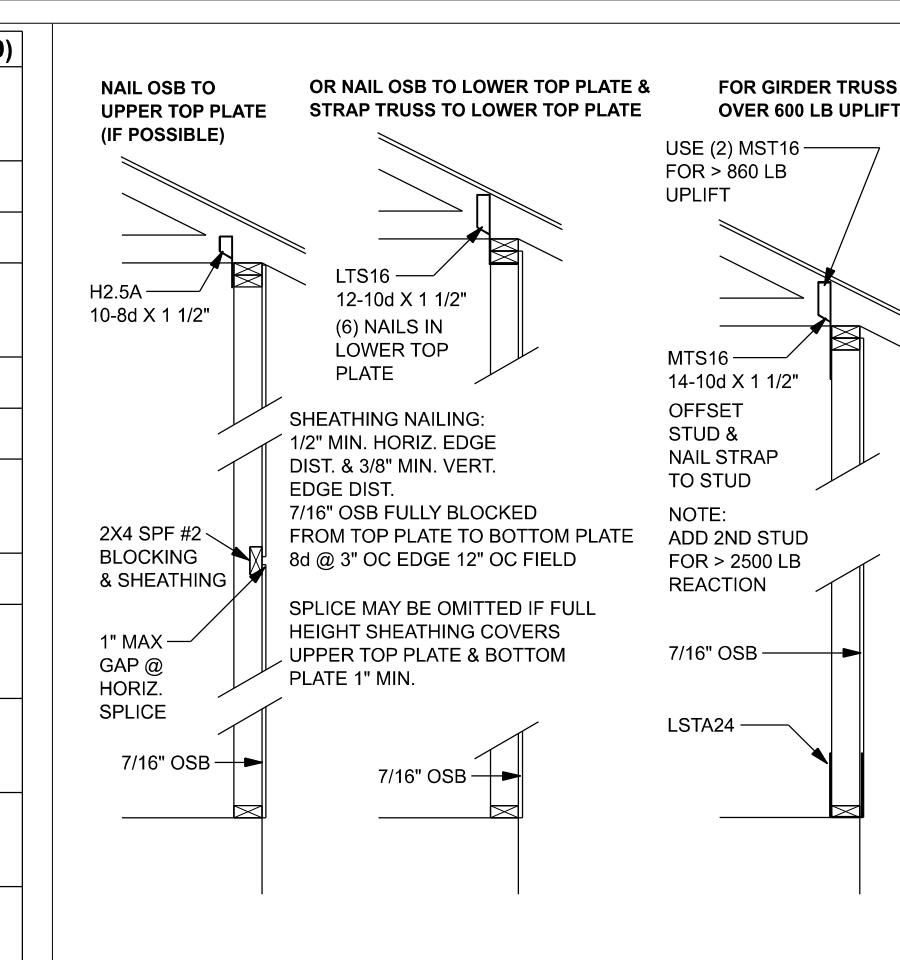
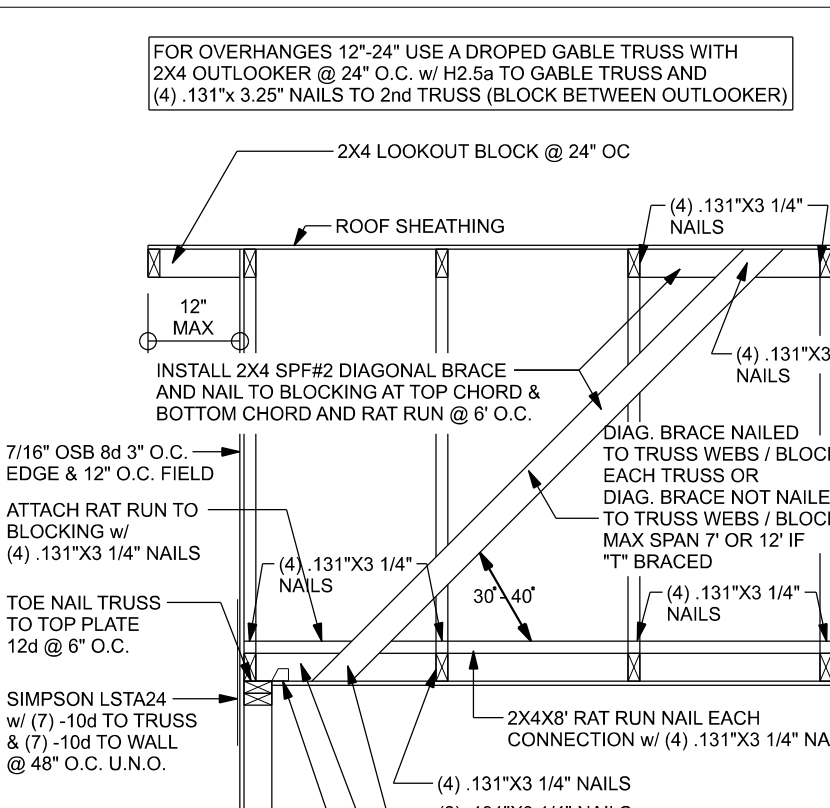


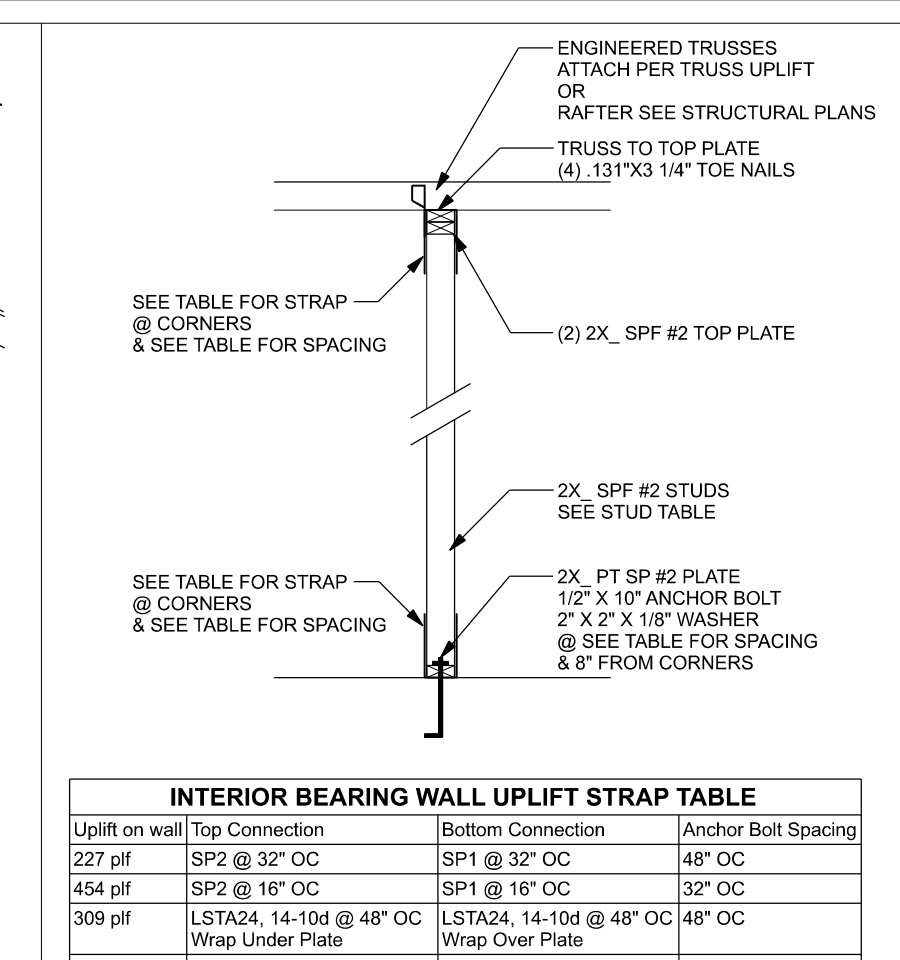
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood OR OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2.12" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

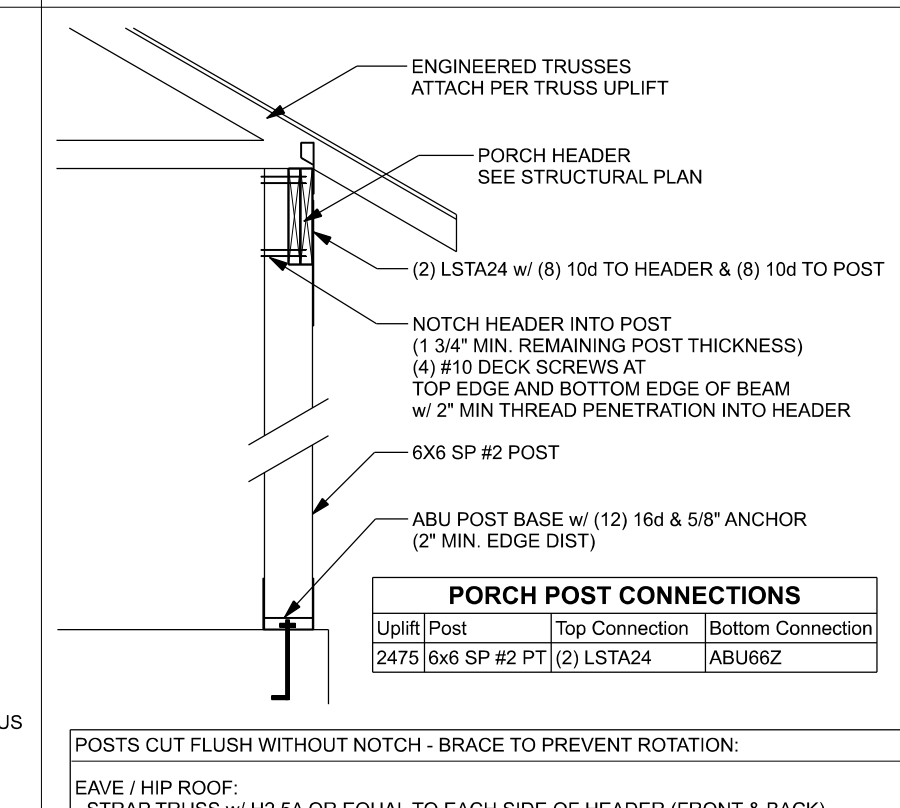
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



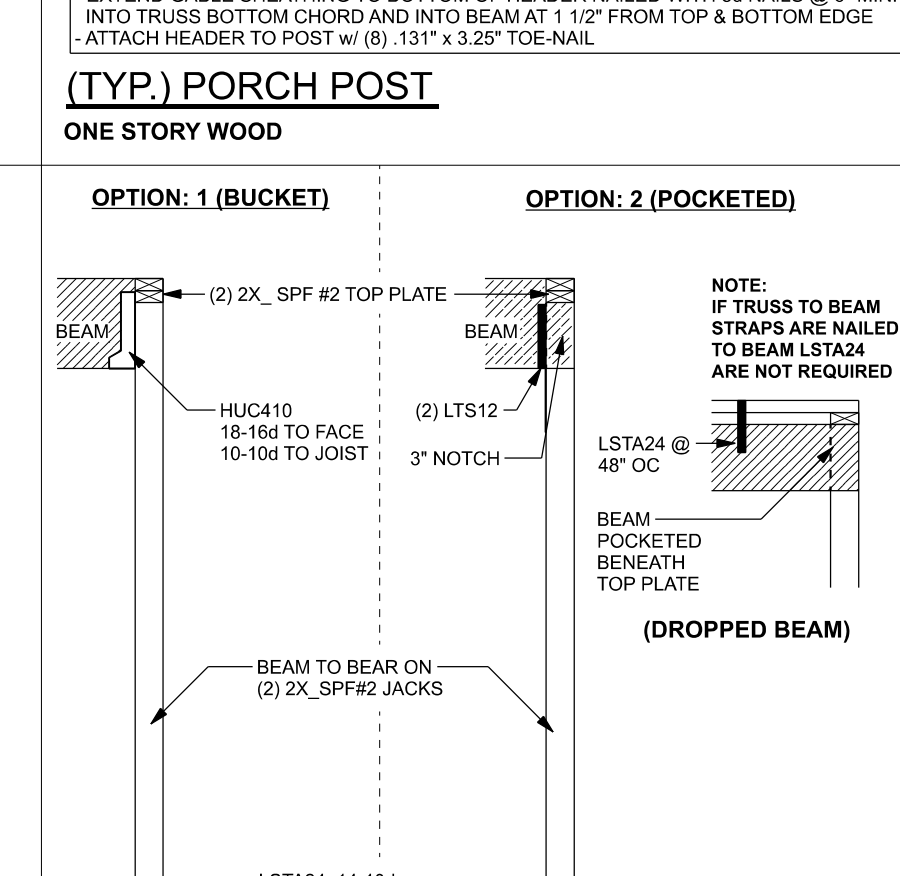
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



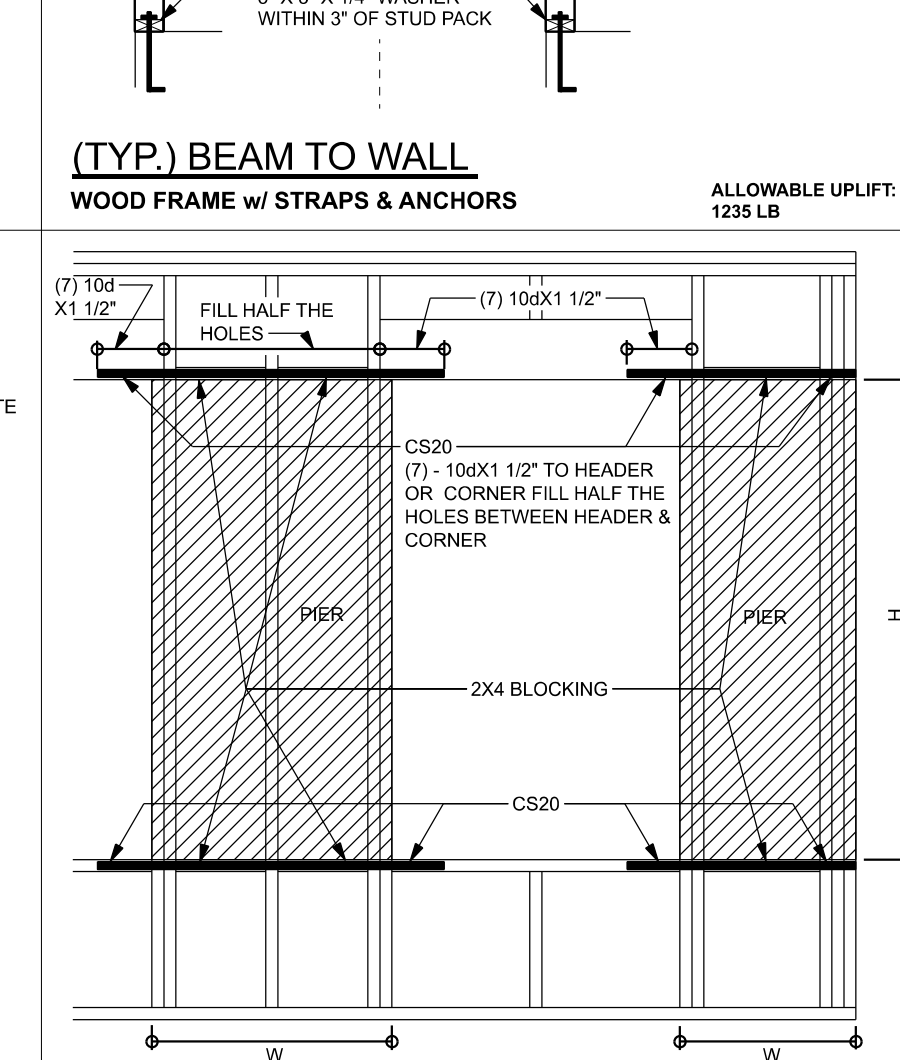
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



OPENING FORCE TRANSFER
WOOD FRAME

CONNECTOR TABLE

Uplift SP	Uplift SP#	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
475	290	H3	4-8dX1 1/2"	4-8dX1 1/2"
515	495	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SP#	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SP#	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SP#	Holdowns @ Mono	To Stud / Post	Anchor
1925	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SP#	Post Bases @ Stewall	To Post	Anchor
2475	ABU4Z	12-16d	5/8"x12" Drill & Epoxy	
2475	ABU6R2	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SP#	Post Bases @ Mono	To Post	Anchor
1900	ABU4Z	12-16d	5/8"x12" Drill & Epoxy	
2475	ABU6R2	12-16d	5/8"x12" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SP#2 STUDS:

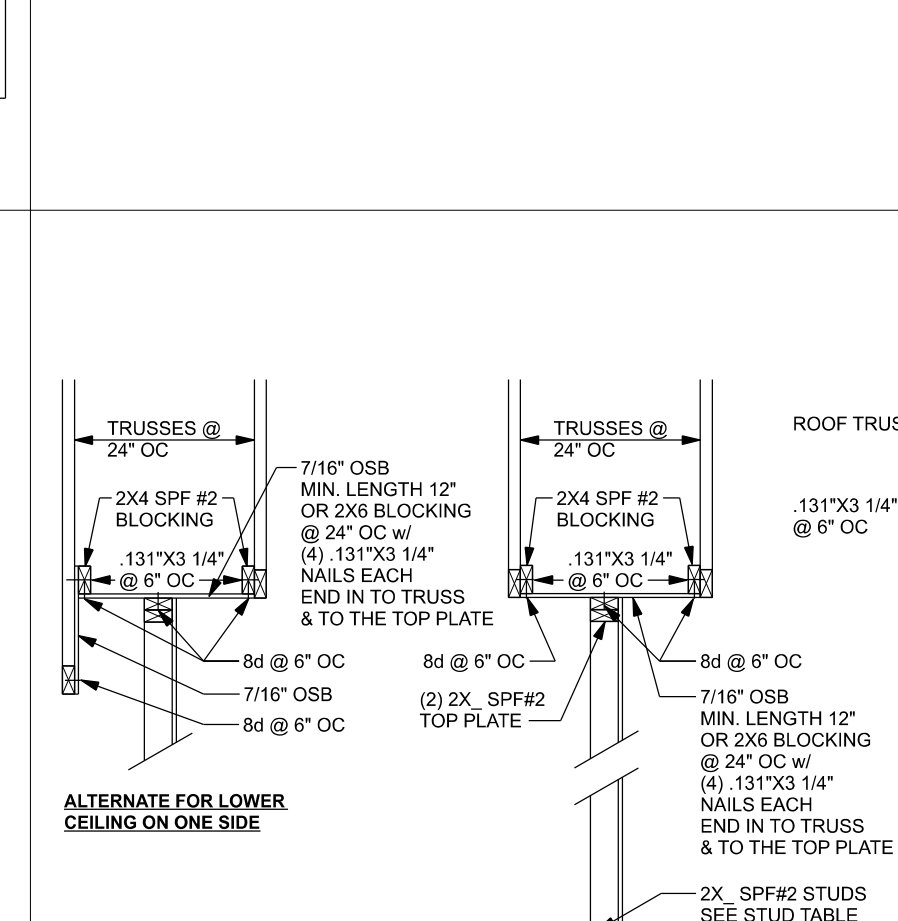
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT
(1) 2x8 @ 16" OC	TO 22'-4" STUD HEIGHT

GRADE & SPECIES TABLE

	Fb	E
2x10	SP #2	925 1.4
2x8	SP #2	800 1.4
2x12	SP #2	750 1.4
GLB	24F-V3 SP	2600 1.9
LSL	TIMBERSTRAND	1700 1.7
LVL	MICROLAM	2950 2.0
PSL	PARALAM	2900 2.0

ROOF SYSTEM DESIGN:



(NOTE: FULL HEIGHT 3/8" ROD IS NOT REQUIRED @ ENDS OF SHEARWALLS THAT ARE ATTACHED w/ INTERSECTING SHEARWALL PER CORNER FRAMING DETAIL

ALSO HTT4 HOLD DOWN CAN DO USED IN PLACE OF THE FULL HEIGHT 3/8" ROD

INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F8 + 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1119. SUPPLIER TO PROVIDE ASTM C 1119 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE LEFT DO NOT CUT W/WW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS), UNO, ALL REINFORCEMENT SHALL BE DETAILD AND PLACED IN ACCORDANCE WITH 318-16 (I.G.).

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CONCRETE.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

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Blake Construction

Price Residence

PROJECT ADDRESS:

Lake City, FL

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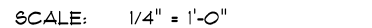
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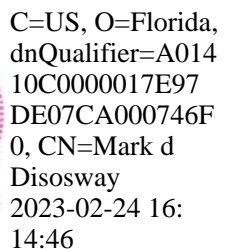


FN-1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, CHAIRS, ETC. DIMENSIONS OF RECESSES OR MARK DISOWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN-2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING WALLS AS PER REQUIREMENTS OF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN-3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ XG-1.4-1.4 WELDED WIRE MESH PLACED ON CHAIRS OR 4" DEPTH OR FIBER REINFORCED CONCRETE, 6-MIL POLY TAPE BARRIER W/ 6" LAP'S SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (BY OTHER CONTRACTOR) AND OVER TERMITE-TREATMENT (BY OTHER CONTRACTOR) (SEE TERMITE-TREATMENT POLICY CAN BE USED INSTEAD)

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF SPECIFICATION FOR MASONRY STRUCTURES¹ (ACI 308.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 308-1.02 AND THE DESIGN DRAWINGS. THERE ARE ANY EXCEPTIONS TO ACI 308-1.02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
	<div> <div>Specific Requirements</div> <div> ACI308-1.02 Section 1.4A Compressive strength 2.1 Mortar 2.2 Grout 2.3 CMU standard 2.4 Clay brick standard 2.4A Reinforcing bars, C# #11 2.4F Coating for corrosion protection 2.4F Coating for corrosion protection 3.3.2.2 Pipes, conduits, and accessories 3.3.6.7 Movement joints </div> </div>
	<div> <div>Specific Requirements</div> <div> 8" block bearing walls, Fm = 1500 psi ASTM C270, Type N, UNF ASTM C476, admixtures require approval ASTM C 90-02, Normal weight, hollow, medium surface finish, 8" nominal thickness and 12"x12" or 16"x16" nominal column block ASTM C216-02, Grade SW, Type FBS, 5.5"x7.5"x11.5" ASTM A575, Grade 40, Fy = 40 ksi, Lap splice min 60 dia bars, (ASTM A615) Anchor bolts, steel metal ties completely embedded in mortar or grout, ASTM A325, Class GR60, 0.60 wt% Zn or 304SS Joint reinforcement in walls exposed to moisture or water ties, anchors, steel metal ties not completely embedded in mortar or grout, ASTM A193, Class B, 1.50 oz/tie or 304SS Any not shown on the project drawings require engineering approval. </div> </div>
	<div> <div>Contractor assumes responsibility for type and location of movement joints if not shown on project drawings</div> </div>

PROJECT ADDRESS:

This item has been digitally signed and sealed by Mark Disosway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020)

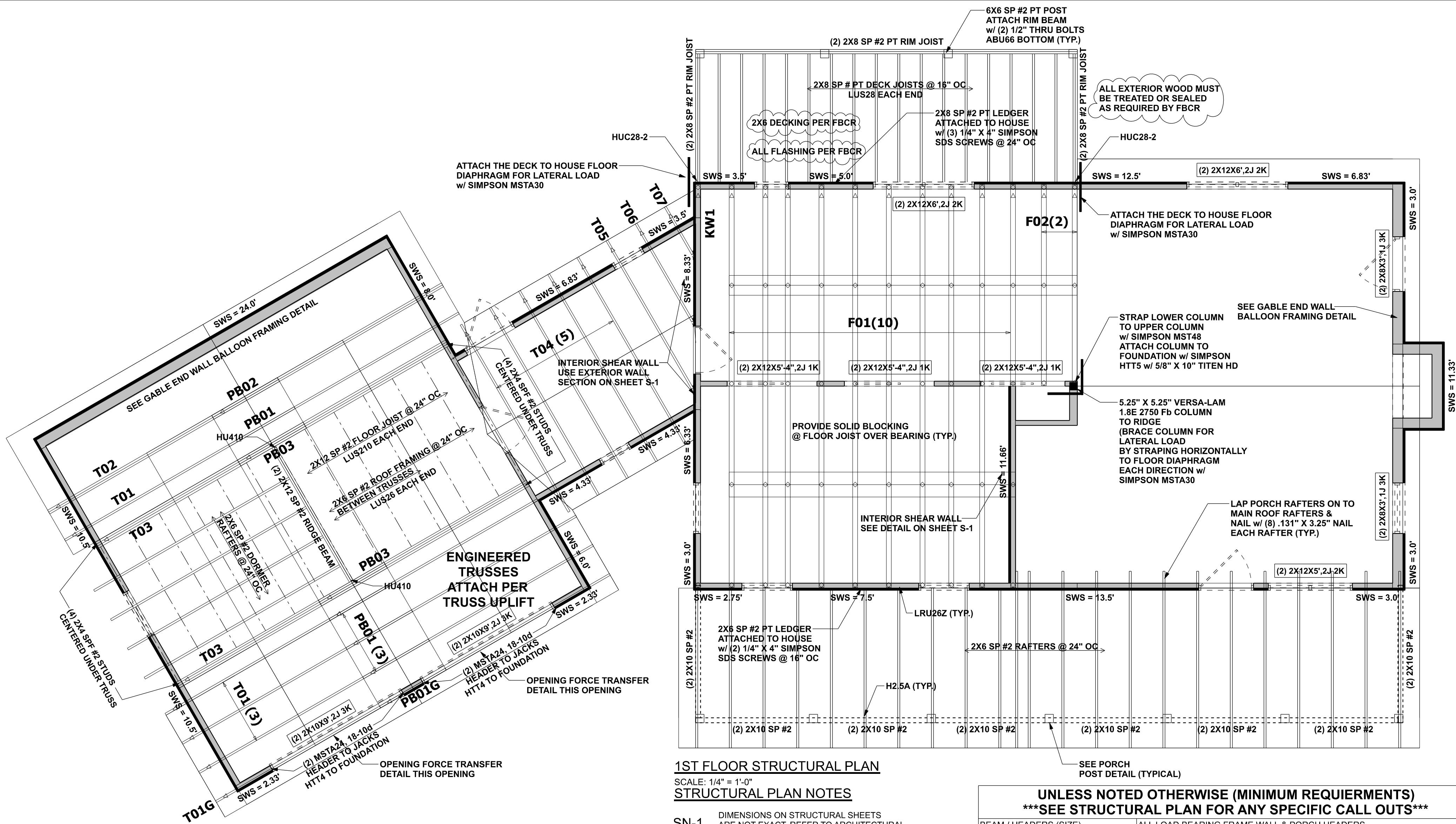
LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
220864

S-2
7 SHEETS

OF 7 SHEETS



BONUS ROOM / GABLE END BRACING

SCALE: 1/2"=1'-0"

IS PER
ENGINEERING

(6) 12dS
2X4 BLOCKING
(4) 12dS
PIGGY
2X4 SPF #2 GABLE
BRACE, 6" O.C.
(4) 12dS
2X4 SPF #2 PLATE
2X4 BLOCKING
12dS, 12" O.C.
7/16" OSB FULLY BLOCKED
3" O.C. EDGE,
12" O.C. FIELD
2X4 SPF #2 STUDS SEE STUD TABLE
12dS, 12" O.C.
2X4 SPF #2 PLATE
3/4" T&G 8d, 6" O.C.
ATTIC TRUSS
24" O.C.
1/2" GYP 5d COOLER 7"
O.C. EDGES
UNBLOCKED

SEE GABLE END WALL
BALLOON FRAMING DETAIL

(2) MSTA24, 18-10d
HEADER TO JACKS
HTT4 TO FOUNDATION

(2) 1.75" X 20.00" LVL RIDGE BEAM

(4) 2X6 SPF #2 STUDS
CENTERED UNDER LVL
STRAP DOWN TO HEADER
BELOW w/ (2) HTS20

2X10 SP #2 RAFTERS @ 24" OC

H10 (TYP.)

CS20, 10-10d
RIDGE TENSION
STRAP (TYP.)
LRU210Z (TYP.)

STRAP LVL TO
STUD PACK
w/ (2) HTS20

2X10 SP #2 RAFTERS @ 24" OC

STRAP LVL TO COLUMN
w/ (4) HTS20

5.25" X 5.25" VERSA-LAM
1.8E 2750 Fb COLUMN
CENTERED UNDER LVL

(2) 1.75" X 20.00" LVL RIDGE BEAM

(4) 2X8 SPF #2 STUDS
CENTERED UNDER LVL
STRAP DOWN TO HEADER
BELOW w/ (2) HTS20

2X10 SP #2 RAFTERS @ 24" OC

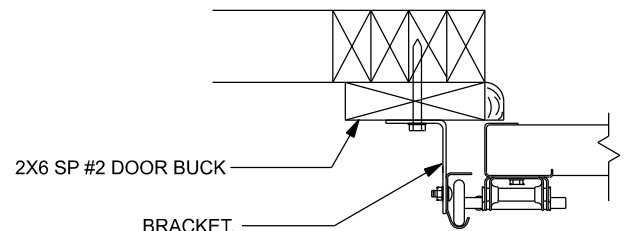
SEE GABLE END WALL
BALLOON FRAMING DETAIL

(2) MSTA24, 18-10d
HEADER TO JACKS
HTT4 TO FOUNDATION

(3) 2X12S 2J 2K

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



1 PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within 6"
up to 2250 lb	(2) HTS16	HTT4

MULTI PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within
up to 2250 lb	(2) HTS16	HTT4
up to 3375 lb	(3) HTS16	HTT4

1 PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within 6"
up to 2250 lb	(2) HTS16	HTT4

MULTI PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w anchor bolt within
up to 2250 lb	(2) HTS16	HTT4
up to 3375 lb	(3) HTS16	HTT4

Uplift	Top Connection	Bottom Connection
up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6"
up to 2250 lb	(2) HTS16	HTT4
up to 3375 lb	(3) HTS16	HTT4

PROJECT ADDRESS:
Lake City, FL



FL PE 53915
This item has been digitally signed and sealed by
Mark Disosway PE on digital signature date.
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signed and sealed and the signature must be
verified on any electronic copies.

C=US, O=Florida,
dnQualifier=A01
10C0000017E97
DE07CA0007461
0, CN=Mark d
Disoway
2023-02-24 16:15
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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OF 7 SHEETS