

DATE08/17/2007

Columbia County Building Permit

PERMIT000026145

This Permit Expires One Year From the Date of Issue

APPLICANTCRAIG TIMBERLAKE

PHONE352 472-6850

ADDRESS25370NW 8TH PLACE

NEWBERRYFL32669

OWNERJAN & LEANNE WHITLOW

PHONE

ADDRESS402SW STUART LOOP

LAKE CITYFL32024

CONTRACTORBONNIE JORDAN

PHONE352 472-6850

LOCATION OF PROPERTY47S, TR ON CR 242, TL ON STUART LOOP, LOT ON FIRST CORNER

ON LEFT

TYPE DEVELOPMENTPOOL ENCLOSURE

ESTIMATED COST OF CONSTRUCTION8272.00

HEATED FLOOR AREA

TOTAL AREA

HEIGHT

STORIES

FOUNDATION

WALLS

ROOF PITCH

FLOOR

LAND USE & ZONINGRSF-2

MAX. HEIGHT

Minimum Set Back Requirments:

STREET-FRONT25.00

REAR15.00

SIDE10.00

NO. EX.D.U.0

FLOOD ZONE

DEVELOPMENT PERMIT NO.

PARCEL ID25-4S-16-03170-102

SUBDIVISIONMAXWELL SQUARE

LOT2


BLOCK

PHASE

UNIT

TOTAL ACRES

SCC056711



Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

EXISTING

X07-313

BK

JH

N

Driveway Connection

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: NOC ON FILE

Check # or Cash

CASH

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power

Foundation

Monolithic

date/app. by

date/app. by

date/app. by

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

date/app. by

date/app. by

Framing

Rough-in plumbing above slab and below wood floor

date/app. by

date/app. by

Electrical rough-in

Heat & Air Duct

Peri. beam (Lintel)

date/app. by

date/app. by

date/app. by

Permanent power

C.O. Final

Culvert

date/app. by

date/app. by

date/app. by

M/H tie downs, blocking, electricity and plumbing

Pool

date/app. by

date/app. by

Reconnection

Pump pole

Utility Pole

date/app. by

date/app. by

date/app. by

M/H Pole

Travel Trailer

Re-roof

date/app. by

date/app. by

date/app. by

BUILDING PERMIT FEE \$45.00

CERTIFICATION FEE \$0.00

SURCHARGE FEE \$0.00

MISC. FEES \$0.00

ZONING CERT. FEE \$

FIRE FEE \$0.00

WASTE FEE \$

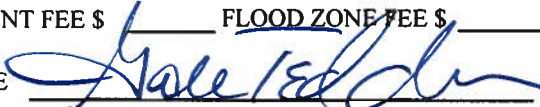
FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$


CULVERT FEE \$

TOTAL FEE45.00

INSPECTORS OFFICE



CLERKS OFFICE



NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Office Use Only Application # 0708-07 Date Received 8/14/07 By GT Permit # 26145
 Application Approved by - Zoning Official _____ Date _____ Plans Examiner _____ Date _____
 Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____
 Comments _____

NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Perri

ame Authorized Person Signing Permit Craig Timberlake Phone 352-472-6850
 Address 25370 NW 8th Place Newberry FL 32669

Owners Name Jan + Leanne Whillow Phone _____

111 Address 402 SW Stuart Loop Lake City, FL 32024

Contractors Name Bonnie Jordan Phone 352-472-6850

Address 25370 NW 8th Place Newberry, FL 32669

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Lawrence Bennett PO Box 214368 S. Daytona, FL 32121

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Eng

Property ID Number 25-45-16-03170-102 Estimated Cost of Construction 8272.00

Subdivision Name _____ Lot 2 Block _____ Unit _____ Phase _____

Driving Directions South on Hwy 47 take right onto CR 242 Left onto Stuart Loop (First road on Left) Follow curve to Right. New home under construct

Type of Construction Screen enclosure Number of Existing Dwellings on Property _____

Total Acreage _____ Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing

Actual Distance of Structure from Property Lines - Front 150' Side 50' Side 30' Rear 80'

Total Building Height _____ Number of Stories _____ Heated Floor Area _____ Roof Pitch _____

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this _____ day of _____ 20____

Personally known _____ or Produced Identification _____

Contractor Signature Bonnie Jordan
 Contractors License Number SC056711
 Competency Card Number _____
 NOTARY STAMP/SEAL

Notary Signature _____ (Revised Sept. :)



Columbia County Building Permit Application

For Office Use Only Application # 0708-07 Date Received 8/3/07 By G Permit # _____

Application Approved by - Zoning Official aps Date 8/14/07 Plans Examiner OK JH Date 8-13-07

Flood Zone N/A Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RLD

Comments _____

Applicants Name: Larry Cole
Address: 25370 NW 8th Place Newberry, FL 32669
Owners Name: Jan & LeAnne Whitlow
911 Address: 402 SW Stuart Loop Lake City, FL 32024
Contractors Name: Bonnie Jordan
Address: 25370 NW 8th Place Newberry, FL 32669

Phone: (352) 472-6850

Phone: _____

Phone : (352) 472-6850

Fee Simple Owner Name & Address: N/A

Bonding Co. Name & Address: N/A

Architect/Engineer Name & Address: Lawrence E. Bennett, P.E. PO Box 214368, South Daytona, FL 32121

Mortgage Lenders Name & Address: Mercantile

Lot 2 Maxwell Sq. S/D

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 25-4S-16-03170-102

Estimated Cost of Construction \$ 8272.00

Subdivision Name: Maxwell Square

Lot 2 Block _____ Unit _____ Phase _____

Driving Directions: SHwy 47 To 262 R To Stuart Loop L, lot on first corner on left.

Type of Construction: Pool Enclosure over existing pool, concrete done by others

Number of Existing Dwellings on Property _____

Total Acreage 0.000 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 90' Side 30' Side 50' Rear 50'

Total Building Height _____ Number of Stories _____ Heated Floor Area _____ Roof Pitch _____

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

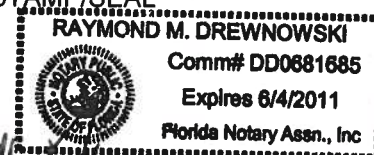
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Craig Sinterlake
Owner, Builder or Agent (Including Contractor)

Craig Sinterlake
Contractor Signature
Contractor's License Number: SCC056711
Competency Card Number _____
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 3 day of August 2007.



Ray M. Drewnowski
Notary Signature

Personally known ✓ or Produced Identification _____

Timberlake Aluminum Construction, Inc.

25370 NW 8TH PLACE
NEWBERRY, FL 32669
(352) 472-6850
FAX (352) 472-6855

THIS LETTER AUTHORIZES Craig C. Timberlake TO OBTAIN PERMITS
AND SIGN ALL APPLICABLE FORMS AND AFFIDAVITS NECESSARY FOR
LICENSES FOR LICENSE NUMBER #SCCO56711.

Bonnie Jordan
BONNIE JORDAN

DATE 01-17-07

SWORN TO AND SUBSCRIBED BEFORE ME THIS 17 DAY OF
JANUARY, 2007

Craig C. Timberlake
NOTARY PUBLIC



Timberlake Aluminum Construction, Inc.

25370 NW 8TH PLACE
NEWBERRY, FL 32669
(352) 472-6850
1-800-976-9890
FAX (352) 472-6855

TO:
RE LETTER OF AUTHORIZATION

THIS LETTER AUTHORIZES CRAIG TIMBERLAKE TO SIGN ALL APPLICABLE
FORMS AND AFFIDAVITS NECESSARY FOR OBTAINING PERMITS OR RENEWAL OF
LICENSE FOR BONNIE L JORDAN SCC 056711.


BONNIE L JORDAN

SWORN TO AND SUBSCRIBED BEFORE ME THIS 3rd DAY OF May 2007

NOTARY PUBLIC
AFFIX SEAL





Jennifer R. Smith
My Commission DD268541
Expires February 08, 2008

Columbia County Property Appraiser

DB Last Updated: 5/11/2007

Parcel: 25-4S-16-03170-102

2007 Proposed Values

Tax Record

Property Card

Interactive GIS Map

Print

Owner & Property Info

Search Result: 1 of 6

Next >>

Owner's Name	WHITLOW JAN B & LEANNE M		
Site Address			
Mailing Address	P O BOX 3485 LAKE CITY, FL 32056		
Use Desc. (code)	VACANT (000000)		
Neighborhood	25416.03	Tax District	2
UD Codes	MKTA01	Market Area	01
Total Land Area	0.000 ACRES		
Description	LOT 2 MAXWELL SQUARE S/D. WD 1113-2583.		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	\$20,500.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$20,500.00

Just Value	\$20,500.00
Class Value	\$0.00
Assessed Value	\$20,500.00
Exempt Value	\$0.00
Total Taxable Value	\$20,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
3/15/2007	1113/2583	WD	I	Q		\$232,700.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	1.000 LT - (.000AC)	1.00/1.00/1.00/1.00	\$20,500.00	\$20,500.00

Columbia County Property Appraiser

DB Last Updated: 5/11/2007

1 of 6

Next >>

Notice of Commencement

State Of Florida

County of Columbia

1. Description of Property Residential
2. Parcel ID# 25-45-16-0317-102 AP
3. General Description of Improvement: Screen Enclosure
4. Owner Name and Address: Jane LeAnne Whitlow
402 SW Stuart Loop Lake City FL 32024
5. Interest in Property: Owner
6. Fee Simple Titleholder(if other than owner): None
7. Contractor Name: Timber Lake Aluminum Construction
8. Surety: None
9. Lender: Mercantile (If you have a loan and payment(draws) will be made from the lender--- the name must be listed.)
10. Person in the State of Florida designated to receive notices or other documents that may be served as provided by Florida Statutes 713.13(1) (a) (7).
NONE

In addition to himself, owner designates : NONE to receive a copy of the Leinor's notice as provided by the Florida Statutes 713.131b

12. Expiration Notice: Notice of Commencement (expires 1 year from the date of recording)

13. Prepared by: Peeler Pools, Inc. for Timber Lake AP Aluminum

14. Return to: Peeler Pools, Inc 9878 S. US Hwy 441 Lake City, FL 32025

Owner Name (Print) LeAnne Whitlow Owner Name Signature LeAnne Whitlow

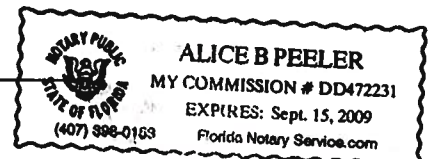
Sworn to and subscribed before me this 22nd day of February, 2007

Personally Known ☒ Produced ID _____ Did/ Did not take Oath _____

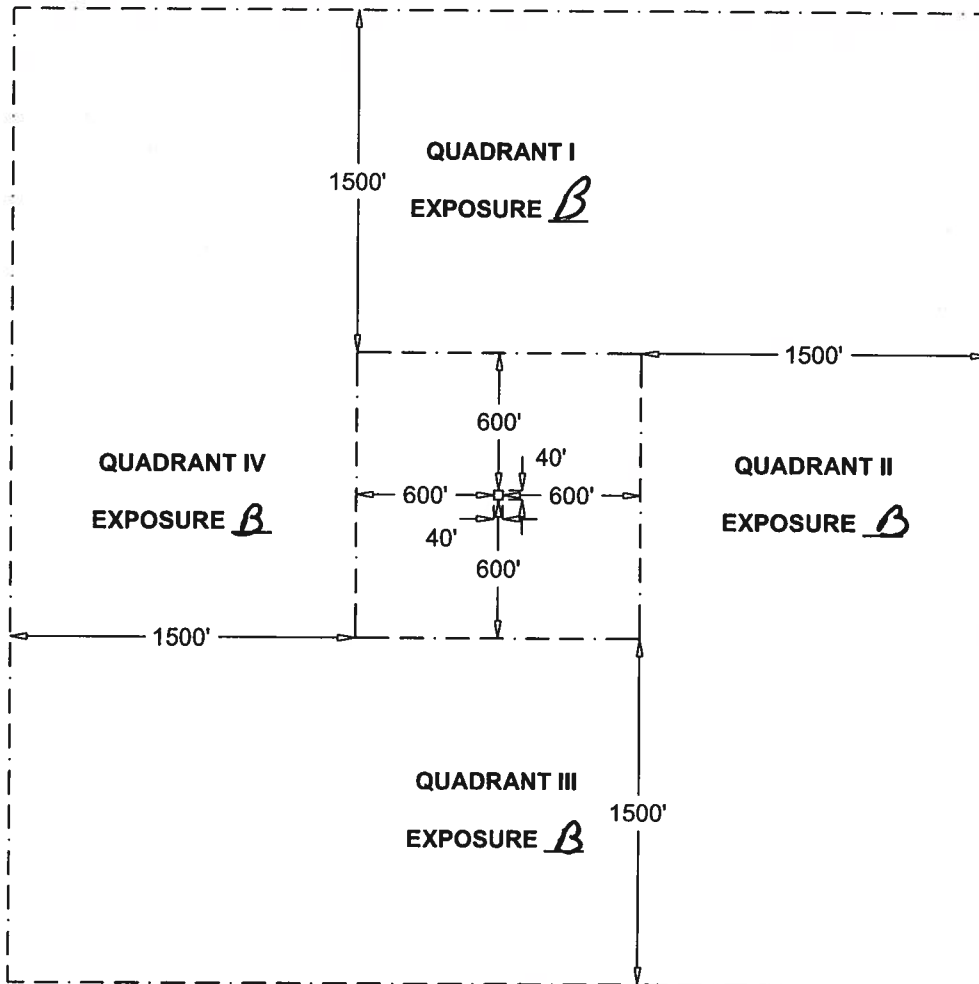
Notary's Name Alice B. Peeler Notary Public State Of Florida _____

Commission Expiry and Number _____

Alice B. Peeler



SITE EXPOSURE EVALUATION FORM



NOTE: ZONES ARE MEASURED FROM STRUCTURE OUTWARD

SITE

SCALE: 1" = 800'

USING THE FOLLOWING CRITERIA, EVALUATE EACH QUADRANT AND MARK IT AS 'B', 'C', OR 'D' EXPOSURE. 'C' OR 'D' EXPOSURE IN ANY QUADRANT MAKE THE SITE THAT EXPOSURE.

- EXPOSURE C:
1. OPEN TERRAIN FOR MORE THAN 1,500 FEET IN ANY QUADRANT.
 2. ANY 'C' EXPOSURE FOR GREATER THAN 600 FEET IN ANY QUADRANT.
 3. NO SHORT TERM CHANGES IN 'B', 2 YEARS BEFORE SITE EVALUATION AND BUILD OUT WITHIN 3 YEARS, SITE WILL BE 'B'.
 4. FLAT, OPEN COUNTRY, GRASSLANDS, PONDS AND OCEAN OR SHORELINES IN ANY QUADRANT FOR GREATER THAN 1,500 FEET.

EXPOSURE D: FLAT, UNOBSTRUCTED AREAS THAT ARE 1,500 FT INLAND FROM THE SHORE LINE AND ARE EXPOSED TO WIND FLOWING OVER WATER FOR A DISTANCE OF AT LEAST 1 MILE.

SITE IS EXPOSURE: B EVALUATED BY: Laig Timberlake DATE: _____

SIGNATURE: Laig LICENSE #: SLC056711



CRAIG TIMBERLAKES, President
HARVEY STOKES, Scribe

Timberlake

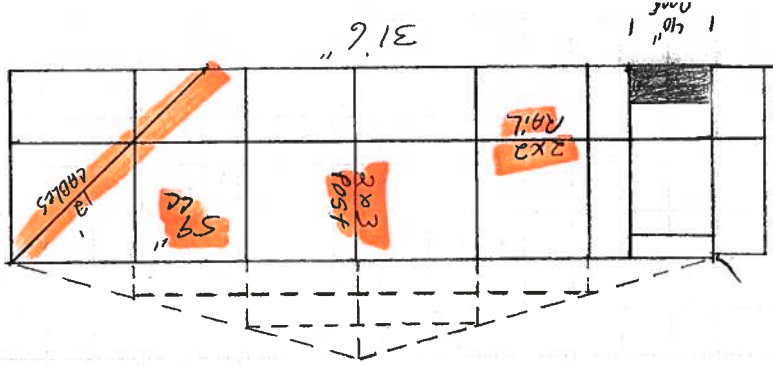
Aluminum Construction, Inc.

Don't clean it - scien it!

License # SCC056711

25370 NW 8th Place
Newberry, Florida 32669
Toll Free 1-800-976-9890
Phone (352) 472-6850
Fax (352) 472-6855

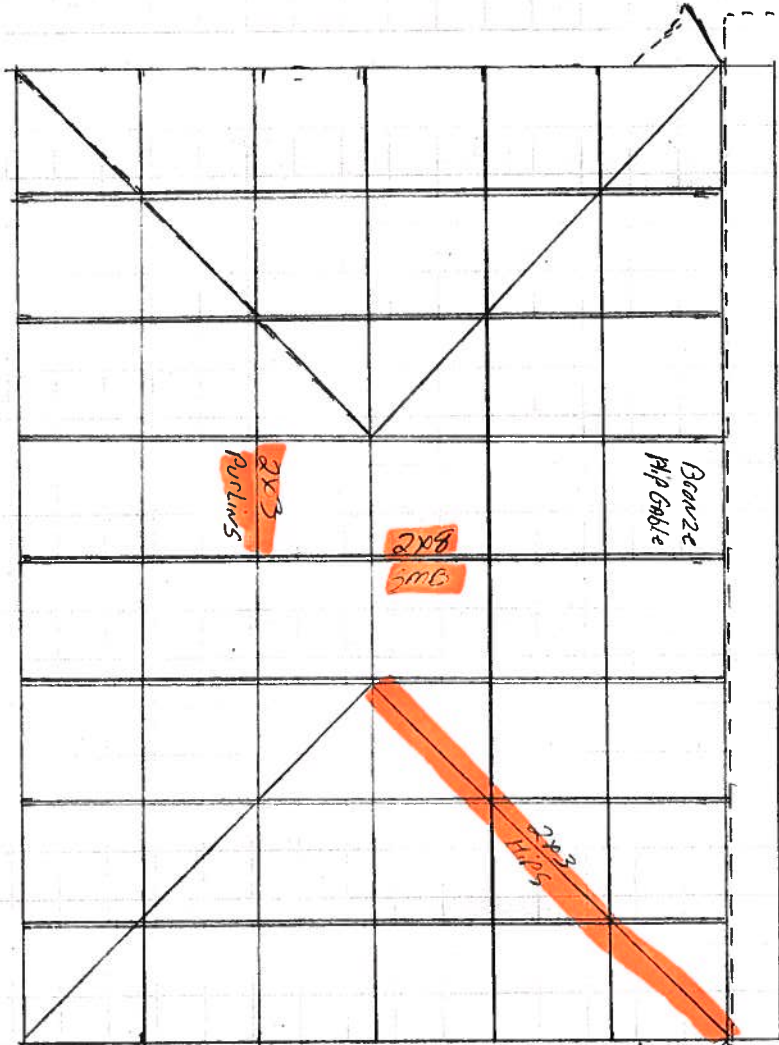
Whitlow / Peeler # 134
402 SW Stuart Loop
Lake City FL



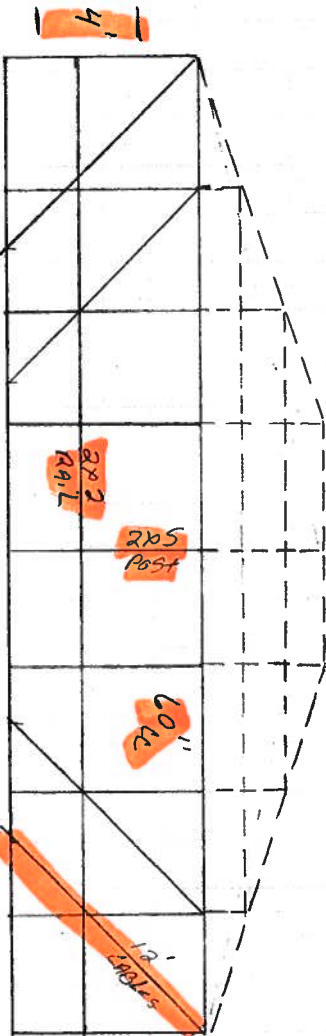
* 8' *

7'-9" 2x3 Post
7'-9" x 1'-10" = 8'-7"

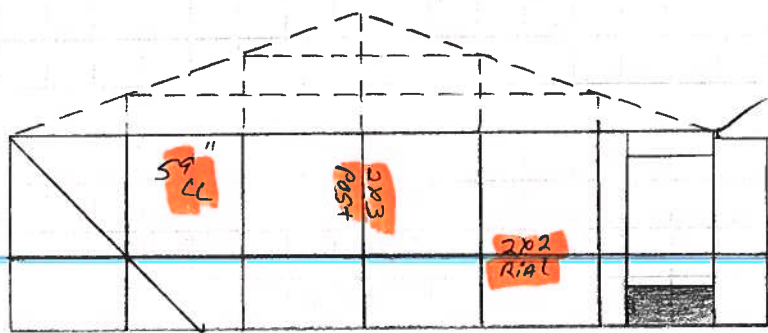
Bearitt 06



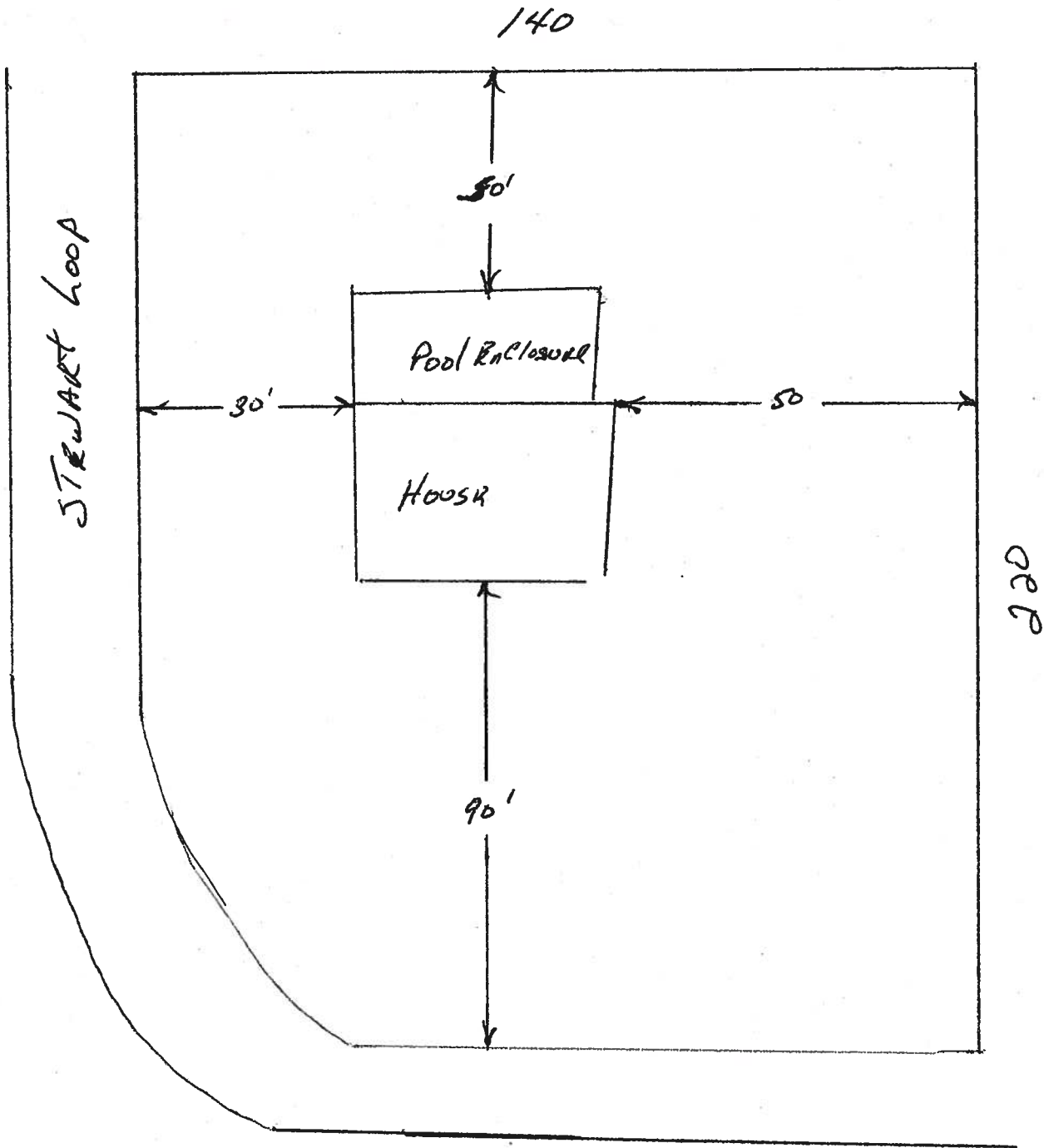
* 28' 8" Beam Span *



* 8' *



* 8' *



Timberlake Aluminum Coast Inc
25370 NW 8th
Aventura, FL 33150

Whitlow
402 SW Stewart Loop
Lake City, FL

Design Check List for Pool Enclosures (Page 1 of 4)

I. Design Statement:

These plans have been designed in accordance with the Aluminum Structures Design Manual by Lawrence E. Bennett and are in compliance with the 2004 Florida Building Code Edition with 2006 Supplements, Chapter 20, ASM35 and The 2005 Aluminum Design Manual Part I-A & II-A; Exposure 'B' or 'C' or 'D'; Importance Factor 0.87 for 100 MPH and 0.77 for 110 MPH and higher; Negative I.P.C. 0.00; ___ MPH Wind Zone for 3 second wind gust; Basic Wind Pressure ___; Design pressures are ___ PSF for roofs & ___ PSF for walls. (see page 1ii for wind loads and design pressures) A 300 PLF point load is also considered for screen roof members.

Notes: Wind velocity zones and exposure category is determined by local code. Design pressures and conversion multipliers are on page 1-ii.

II. Host Structure Adequacy Statement:

I have inspected and verify that the host structure is in good repair and attachments made to the structure will be solid.

Craig Timberlake Phone: 472-6850
Contractor / Authorized Rep* Name (please print)

Craig Timberlake Date: 7-31-07
Contractor / Authorized Rep* Signature

Whitlow / 402 SW Stuart Loop Lake City
Job Name & Address

Note: If the total of beam span & upright height exceeds 50' or upright height exceeds 16', site specific engineering is required.

III. Building Permit Application Package contains the following:

	Yes	No
A. Project name & address on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
B. Site plan or survey with enclosure location	<input checked="" type="checkbox"/>	<input type="checkbox"/>
C. Contractor's / Designer's name, address, phone number, & signature on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
D. Site exposure form completed	<input checked="" type="checkbox"/>	<input type="checkbox"/>
E. Enclosure layout drawing @ 1/8" or 1/10" scale with the following:	<input checked="" type="checkbox"/>	<input type="checkbox"/>
1. Plan view with host structure, enclosure length, projection from host structure, and all dimensions	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. Front and side elevation views with all dimensions & heights	<input checked="" type="checkbox"/>	<input type="checkbox"/>
Note: All mansard wall drawings shall include mansard panel at the top of the wall.		
3. Beam location (show in plan & elevation view) & size (Table 1.1 & 1.6)	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Roof frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to 120 MPH wind zone and / or "C" or "D" Exposure for load width of ___:

Note: Conversion factors do not apply to members subject to point load (P).

Look up span in appropriate 120 MPH span table and apply the following formula:

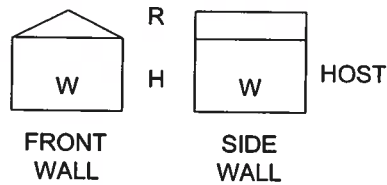
$$\begin{array}{c}
 \text{Span} \\
 @ 120 \text{ MPH}
 \end{array}
 \begin{array}{c}
 \swarrow \\
 \text{Wind Zone Multiplier} \\
 \text{(see page 1ii)}
 \end{array}
 \begin{array}{c}
 \text{ }
 \end{array}
 \begin{array}{c}
 \searrow \\
 \text{Exposure Multiplier} \\
 \text{(see page 1ii)}
 \end{array}
 \begin{array}{c}
 \text{ }
 \end{array}
 \begin{array}{c}
 \swarrow \\
 \text{Required Converted} \\
 \text{Span / Height}
 \end{array}$$

0 (b or d) x 0 (b or d) x 0 (b or d) = 0

- | | | |
|--|-------------------------------------|--------------------------|
| 4. Upright location (show in plan & elevation view) & size (Table 1.3 & 1.6) | <input checked="" type="checkbox"/> | <input type="checkbox"/> |
| 5. Chair rail & girt size, length, & spacing (Table 1.4) | <input checked="" type="checkbox"/> | <input type="checkbox"/> |
| 6. Eave rail size, length, spacing and stitching of (Table 1.2) | <input checked="" type="checkbox"/> | <input type="checkbox"/> |

* Must have attended Engineer's Continuing Education Class within the past two years.

Design Check List for Pool Enclosures (Page 3 of 4)



Example 2: Gable Roof

Front wall @ eave: $\frac{W}{\text{ft.}} \times \frac{H}{\text{ft.}} = \frac{a}{\text{ft.}^2} @ 100\% = \dots \text{ft.}^2$

Front gable rise: $\frac{R}{\text{ft.}} \times \frac{1}{2} \left(\frac{W}{\text{ft.}} \right) = \frac{b}{\text{ft.}^2} @ 100\% = \dots \text{ft.}^2$

Largest side wall: $\frac{W}{\text{ft.}} \times \frac{H}{\text{ft.}} = \frac{c}{\text{ft.}^2} @ 50\% = \dots \text{ft.}^2$

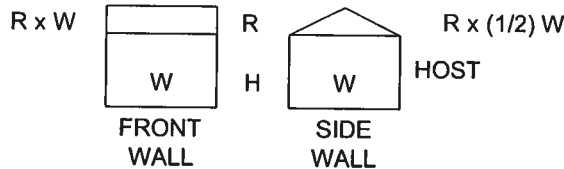
Largest side gable rise: $\frac{R}{\text{ft.}} \times \frac{1}{2} \left(\frac{W}{\text{ft.}} \right) = \frac{d}{\text{ft.}^2} @ 50\% = \dots \text{ft.}^2$

TOTAL = $\dots \text{ft.}^2$

Total area / (233 ft.² / cable for 3/32") = \dots cable pairs
or
Total area / (445 ft.² / cable for 1/8") = \dots cable pairs

Side wall cable calculation: $\frac{c}{\text{ft.}^2} + \frac{d}{\text{ft.}^2} = \dots \text{ft.}^2 @ 100\% = \dots \text{ft.}^2$

Side wall area / (233 ft.² / cable for 3/32") = \dots cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = \dots cable(s)



Example 3: Transverse Gable Roof

Front wall @ eave: $\frac{39'6''}{\text{ft.}} \times \frac{8'}{\text{ft.}} = \frac{316}{\text{ft.}^2} @ 100\% = \dots \frac{316}{\text{ft.}^2}$

Front gable rise: $\frac{5'}{\text{ft.}} \times \frac{1}{2} \left(\frac{39'6''}{\text{ft.}} \right) = \frac{197'6''}{\text{ft.}^2} @ 100\% = \dots \frac{197'6''}{\text{ft.}^2}$

Largest side wall: $\frac{316'}{\text{ft.}} \times \frac{8'}{\text{ft.}} = \frac{252}{\text{ft.}^2} @ 50\% = \dots \frac{126}{\text{ft.}^2}$

Largest side gable rise: $\frac{5'}{\text{ft.}} \times \frac{1}{2} \left(\frac{316'}{\text{ft.}} \right) = \frac{789''}{\text{ft.}^2} @ 50\% = \dots \frac{39'4''}{\text{ft.}^2}$

TOTAL = $\dots \frac{678'10''}{\text{ft.}^2}$

Total area / (233 ft.² / cable for 3/32") = \dots cable pairs
or
Total area / (445 ft.² / cable for 1/8") = $\underline{2}$ cable pairs

Side wall cable calculation: $\frac{252}{\text{ft.}^2} + \frac{789''}{\text{ft.}^2} = \frac{330}{\text{ft.}^2} @ 100\% = \dots \frac{330'9''}{\text{ft.}^2}$

Side wall area / (233 ft.² / cable for 3/32") = \dots cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = $\underline{1}$ cable(s)

Design Check List for Pool Enclosures (Page 4 of 4)

Example 4: Mansard Roof

Front wall @ eave: $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{\text{ft.}^2}{a} @ 100\% = \dots \text{ft.}^2$

Front mansard rise: $\frac{\text{ft.}}{R} \times \frac{1}{2}(\frac{\text{ft.}}{W1} + \frac{\text{ft.}}{W2}) = \frac{\text{ft.}^2}{b} @ 100\% = \dots \text{ft.}^2$

Largest side wall: $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{\text{ft.}^2}{c} @ 50\% = \dots \text{ft.}^2$

Largest side mansard rise: $\frac{\text{ft.}}{R} \times \frac{1}{2}(\frac{\text{ft.}}{W1} + \frac{\text{ft.}}{W2}) = \frac{\text{ft.}^2}{d} @ 50\% = \dots \text{ft.}^2$

TOTAL = $\dots \text{ft.}^2$

Total area / (233 ft.² / cable for 3/32") = \dots cable pairs
or

Total area / (445 ft.² / cable for 1/8") = \dots cable pairs

Side wall cable calculation: $\frac{\text{ft.}^2}{c} + \frac{\text{ft.}^2}{d} = \frac{\text{ft.}^2}{e} @ 100\% = \dots \text{ft.}^2$

Side wall area / (233 ft.² / cable for 3/32") = \dots cable(s)

or

Side wall area / (445 ft.² / cable for 1/8") = \dots cable(s)

Example 5: Dome Roof

Front dome wall @ eave: $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{\text{ft.}^2}{a} @ 100\% = \dots \text{ft.}^2$

Front dome rise: $\frac{\text{ft.}}{R} \times \frac{1}{2}(\frac{\text{ft.}}{W}) = \frac{\text{ft.}^2}{b} @ 100\% = \dots \text{ft.}^2$

Largest side wall: $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{\text{ft.}^2}{c} @ 50\% = \dots \text{ft.}^2$

Largest side dome rise: $\frac{\text{ft.}}{R} \times \frac{\text{ft.}}{W} = \frac{\text{ft.}^2}{d} @ 50\% = \dots \text{ft.}^2$

TOTAL = $\dots \text{ft.}^2$

Total area / (233 ft.² / cable for 3/32") = \dots cable pairs
or

Total area / (445 ft.² / cable for 1/8") = \dots cable pairs

Side wall cable calculation: $\frac{\text{ft.}^2}{c} + \frac{\text{ft.}^2}{d} = \frac{\text{ft.}^2}{e} @ 100\% = \dots \text{ft.}^2$

Side wall area / (233 ft.² / cable for 3/32") = \dots cable(s)

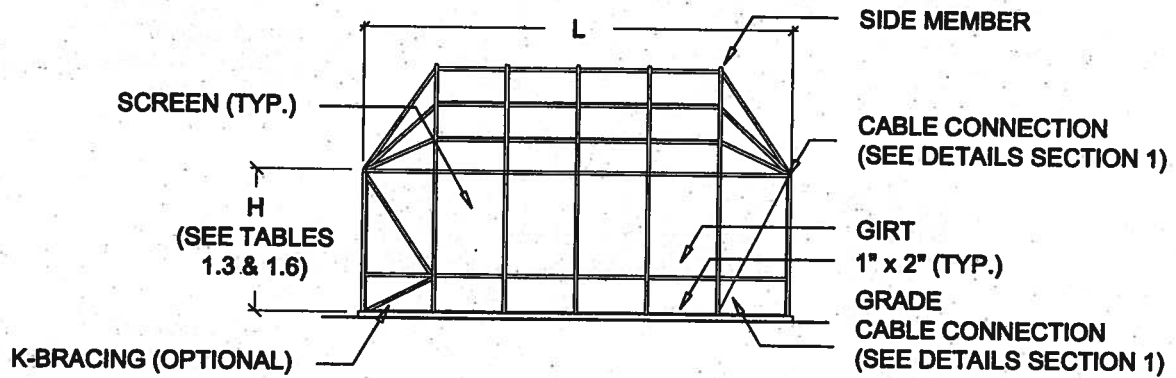
or

Side wall area / (445 ft.² / cable for 1/8") = \dots cable(s)

Notes:

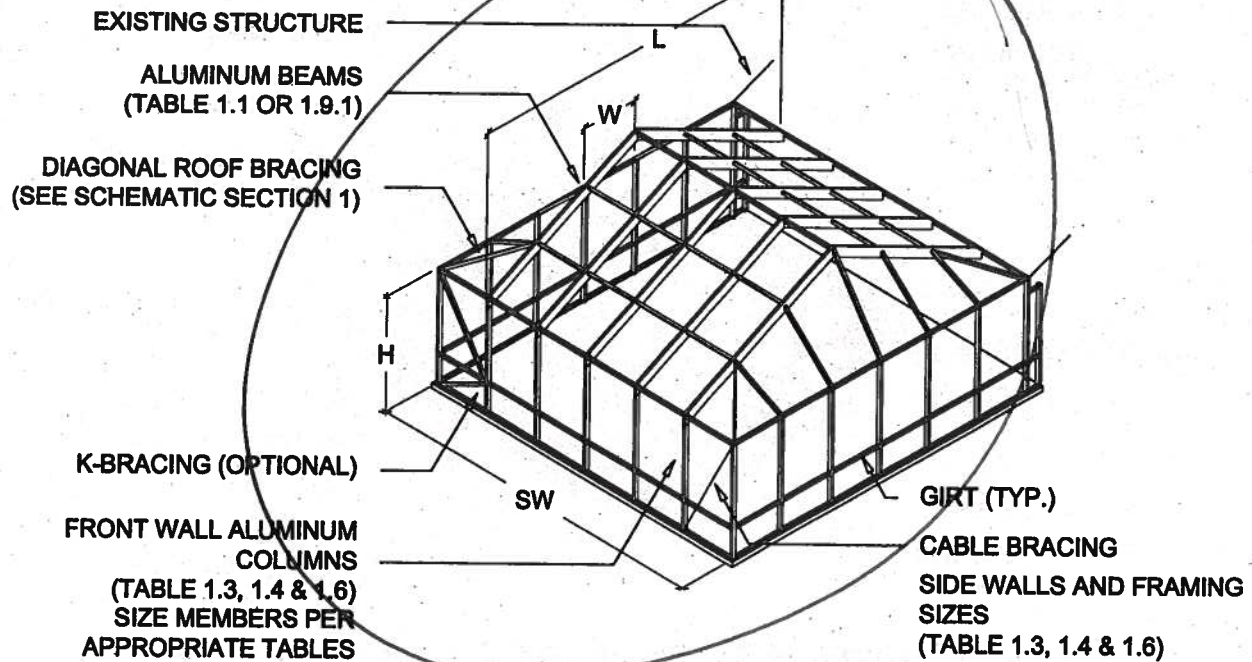
SECTION 1

SCREENED ENCLOSURES



TYPICAL MODIFIED HIP ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL MODIFIED HIP ROOF - ISOMETRIC

SCALE: N.T.S.

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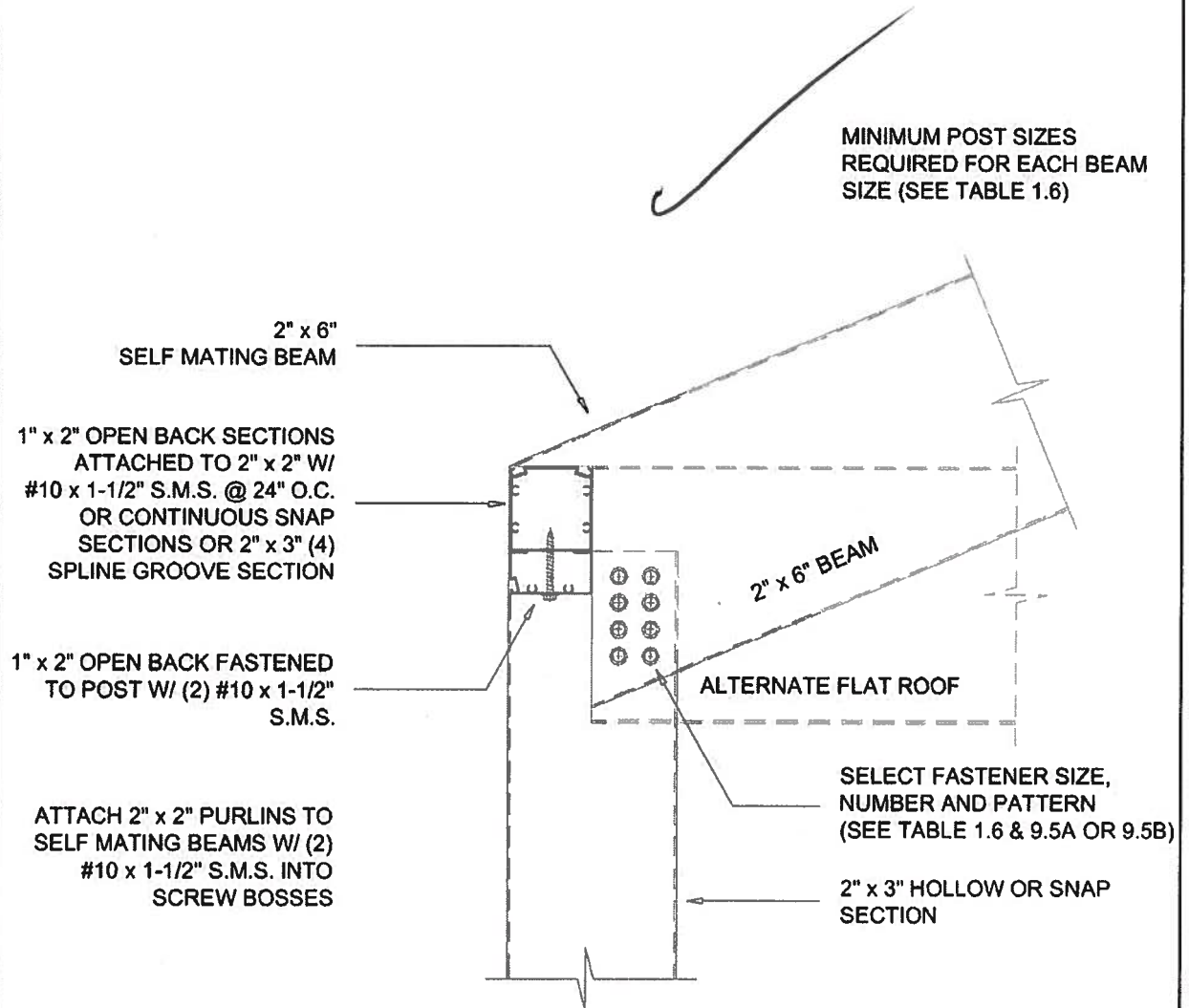
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SCREENED ENCLOSURES

SECTION 1



SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

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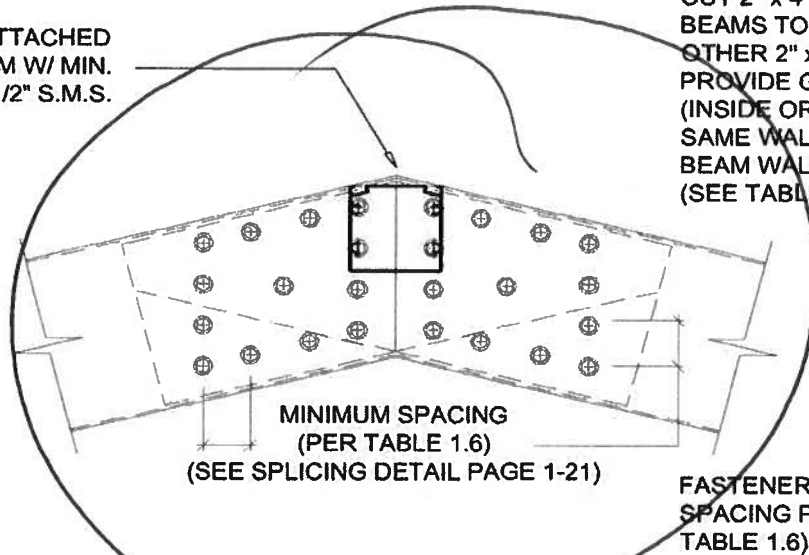
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SECTION 1**SCREENED ENCLOSURES**

2" x 2" PURLINS ATTACHED
TO BEAM W/ MIN.
(3) #10 x 1-1/2" S.M.S.



CUT 2" x 4", 2" x 5", OR 2" x 6"
BEAMS TO SLIDE OVER EACH
OTHER 2" x 7" & LARGER
PROVIDE GUSSET PLATE
(INSIDE OR OUTSIDE BEAM)
SAME WALL THICKNESS AS
BEAM WALLS OR LARGER
(SEE TABLE 1.6)

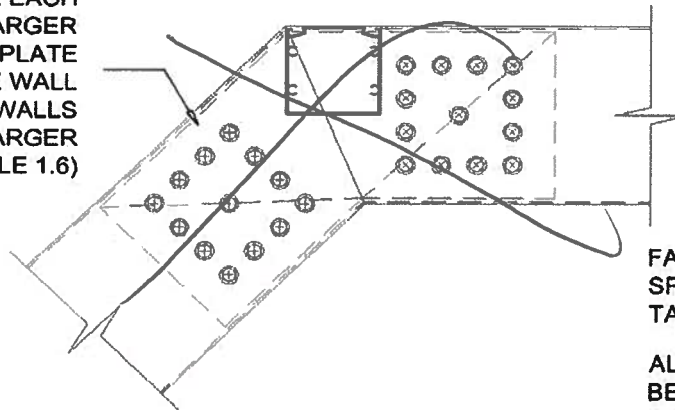
MINIMUM SPACING
(PER TABLE 1.6)
(SEE SPLICING DETAIL PAGE 1-21)

FASTENER SIZE, NUMBER AND
SPACING PER PAGE 1-20(SEE
TABLE 1.6)

ALTERNATE SIDE PLATE CONNECTION DETAIL
GUSSET PLATE MOUNTED INTERNALLY

SCALE: 3" = 1'-0"

CUT 2" x 4", 2" x 5", OR 2" x 6"
BEAMS TO SLIDE OVER EACH
OTHER 2" x 7" & LARGER
PROVIDE GUSSET PLATE
(INSIDE BEAM) SAME WALL
THICKNESS AS BEAM WALLS
OR LARGER
(SEE TABLE 1.6)



FASTENER SIZE, NUMBER AND
SPACING PER PAGE 1-20(SEE
TABLE 1.6)

ALL GUSSET PLATES SHALL
BE A MINIMUM OF 5052 H-32
ALLOY OR HAVE AN ULTIMATE
YIELD STRENGTH OF 30 KSI

ALTERNATE SIDE PLATE CONNECTION DETAIL - MANSARD ROOF
GUSSET PLATE MOUNTED INTERNALLY

SCALE: 3" = 1'-0"

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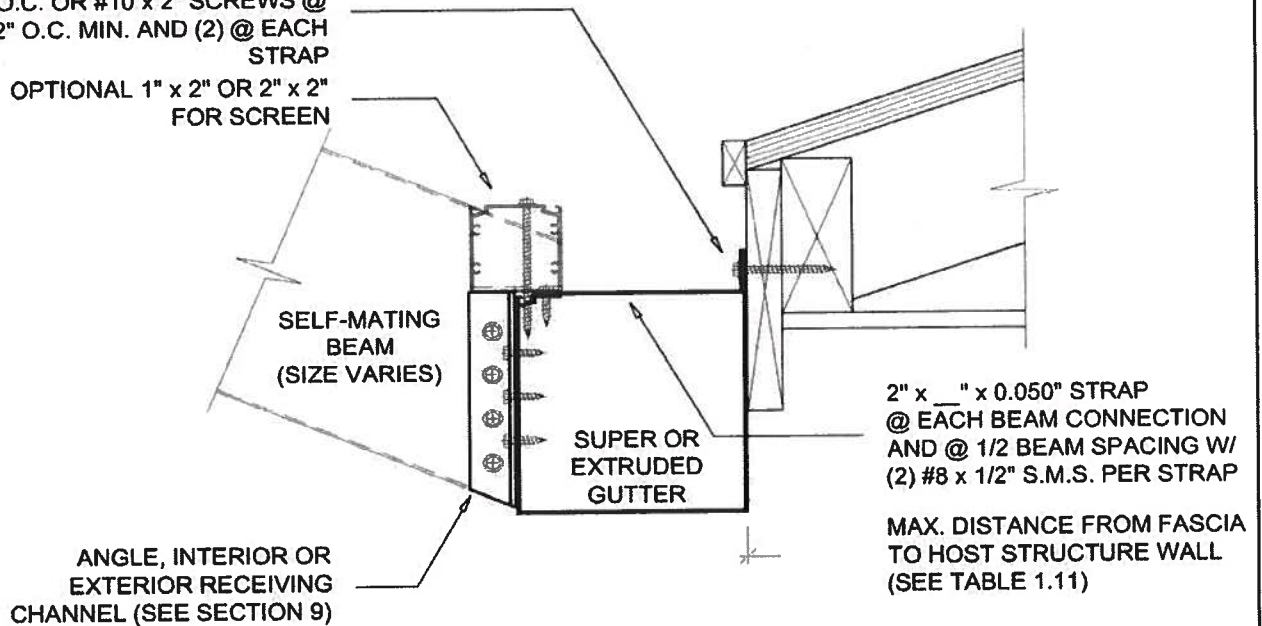
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SCREENED ENCLOSURES

SECTION 1

1/4" x 2" LAG SCREWS @ 24"
O.C. OR #10 x 2" SCREWS @
12" O.C. MIN. AND (2) @ EACH
STRAP
OPTIONAL 1" x 2" OR 2" x 2"
FOR SCREEN



ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

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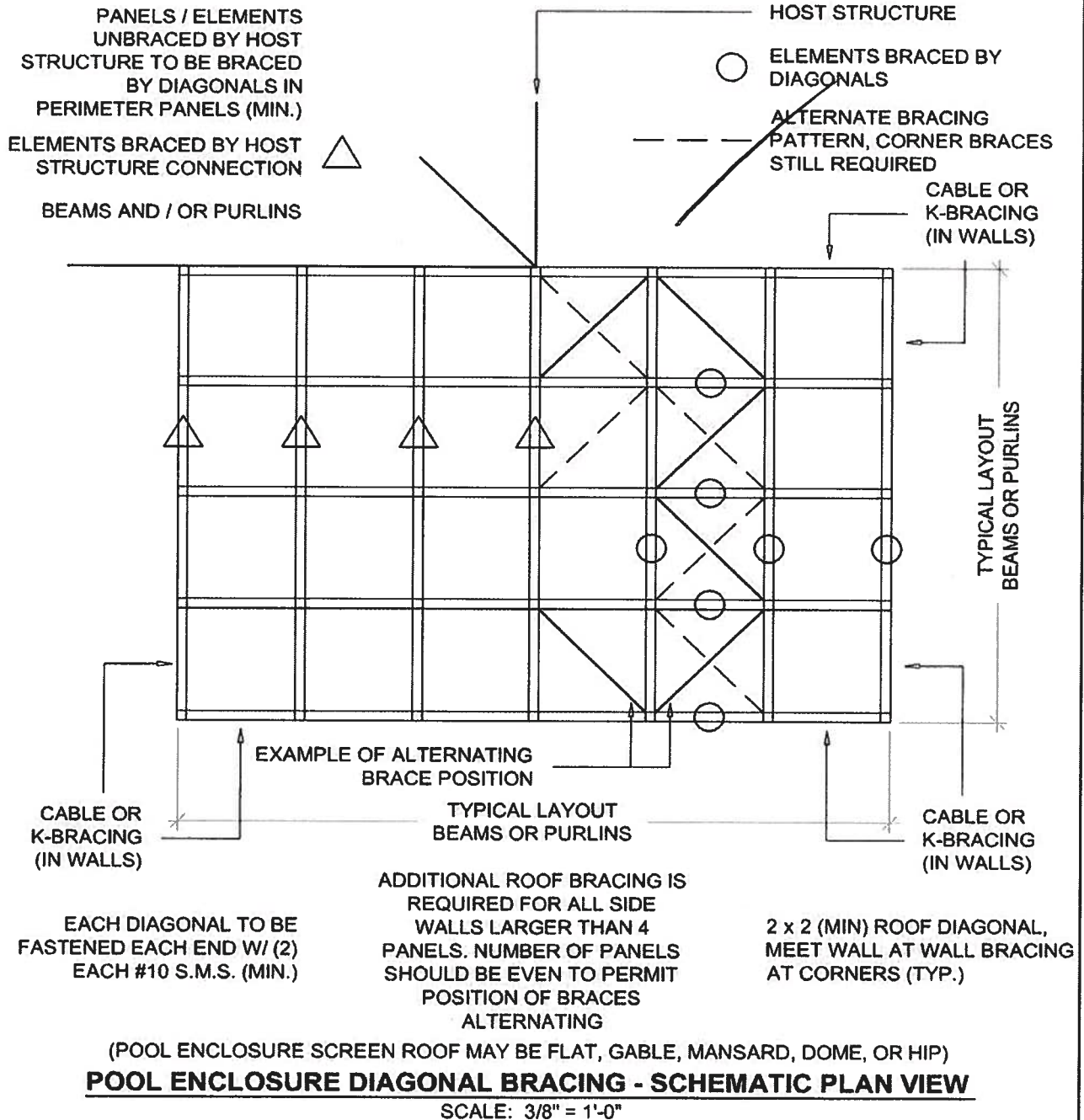
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SECTION 1

SCREENED ENCLOSURES



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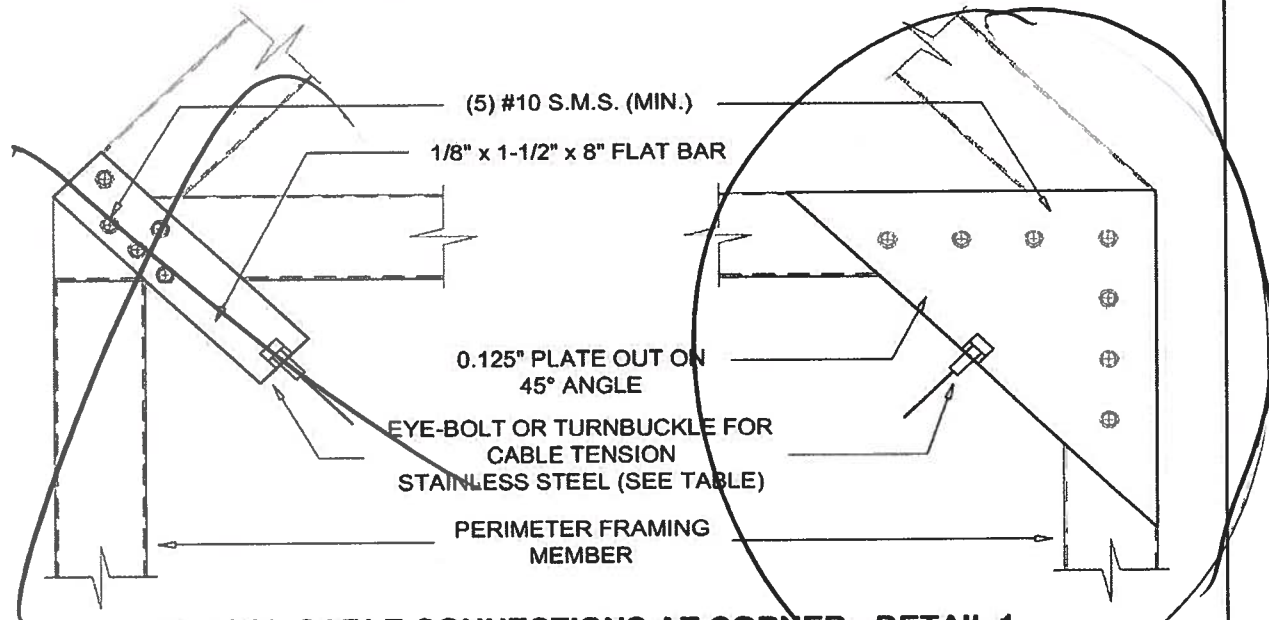
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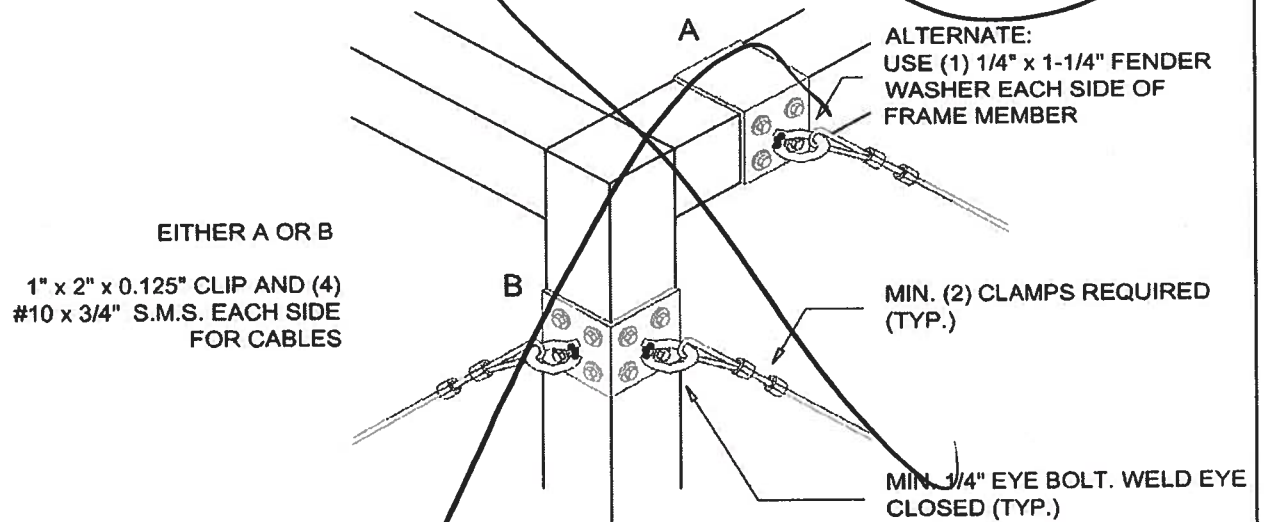
SCREENED ENCLOSURES

SECTION 1



TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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SCREENED ENCLOSURES

SECTION 1

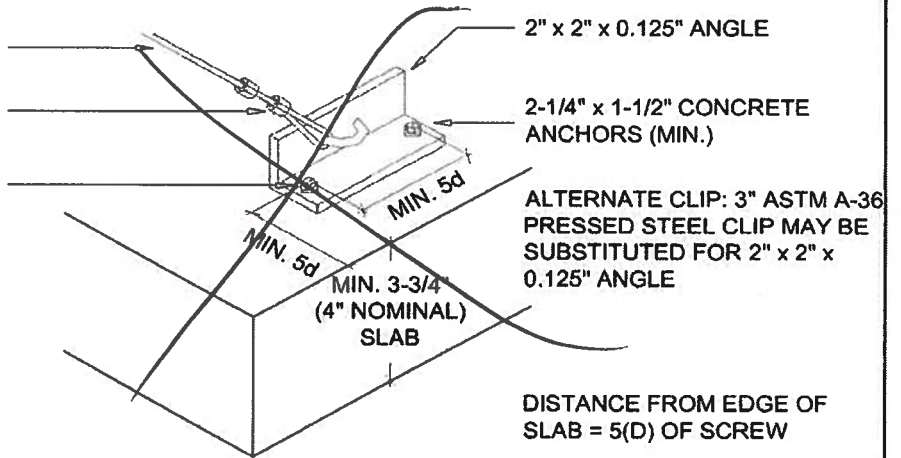
S.S. CABLE @ 40° TO 60° MAX.
ANGLE TO SLAB

CABLE CLAMP

(SEE TABLE)

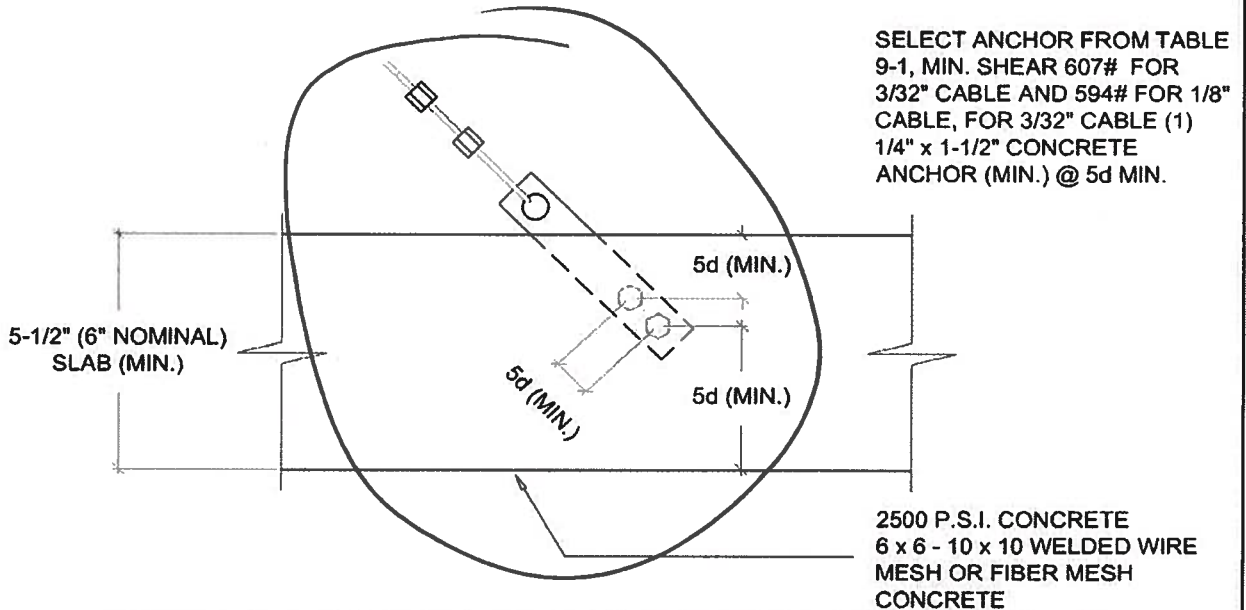
NOTE:
SEE PAGE 1-50 FOR NUMBER
OF CABLES REQUIRED

3-1/2" ASTM A-36 PRESSED
STEEL CLIP MAY BE
SUBSTITUTED FOR
2" x 2" x 0.125" ANGLE



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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SECTION 1

SCREENED ENCLOSURES

PURLINS ANCHORED W/
CLIPS OR #10 SCREWS
THROUGH PURLINS INTO
SCREW BOSSES

EAVE RAILS SHALL BE
STITCHED W/ #10 x 1-1/2" SMS
@ 6" FROM EACH END AND 24"
OC MAX.

FRONT AND SIDE BOTTOM
RAILS ATTACHED TO
CONCRETE W/ 1/4" x 2-1/4"
CONCRETE / MASONRY
ANCHORS @ PRIMARY &
SECONDARY ANGLES OR @ 6"
FROM EACH POST AND 24"
O.C. MAX. AND WALLS MIN. 1"
FROM EDGE OF CONCRETE

GIRTS ANCHORED W/ CLIPS
OR THROUGH #10 SCREWS
INTO SCREW BOSSES

1" x 2" OR 1" x 3"

PURLIN & CHAIR RAIL DETAIL

SCALE: 3" = 1'-0"

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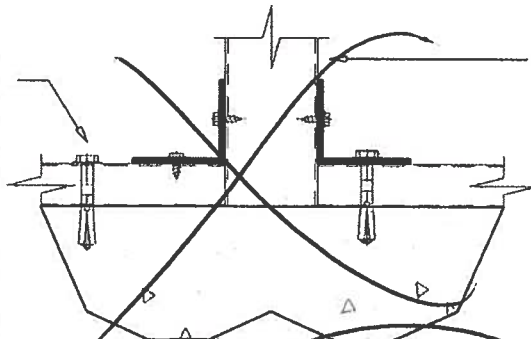
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SCREENED ENCLOSURES

SECTION 1

1" x 2" EXTRUSION ANCHOR
TO CONCRETE W/ CONCRETE
ANCHORS OR THRU PRIMARY
ANGLE 6" MAX. EACH SIDE OF
EACH POST AND @
24" O.C. MAX.
SELECT CONCRETE ANCHORS
FROM SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I.
CONCRETE 6 x 6 - 10 x 10
WELDED WIRE MESH OR
FIBER MESH CONCRETE



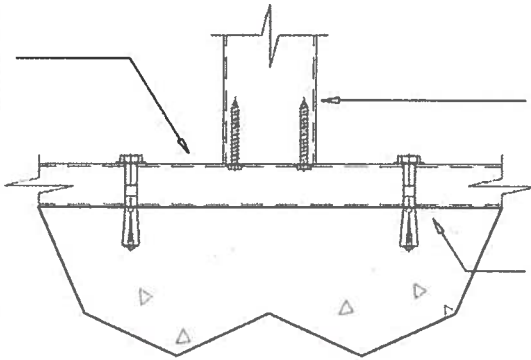
POST SIZE 2" x 4" MAX.

SIDE WALL POST TO PLATE TO CONCRETE DETAIL

SCALE: 3" = 1'-0"

1" x 2" EXTRUSION ANCHOR
TO CONC. W/ CONC. ANCH. 6"
MAX. EA. SIDE OF EA. POST
AND @ 24" O.C. MAX. SELECT
CONCRETE ANCHORS FROM
SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I.
CONC. 6 x 6 - 10 x 10 W.W.M.
OR FIBER MESH CONC.



2" x 2", 2" x 3" OR 2" x 4"
HOLLOW SECTION
(SEE TABLES)

MIN. (3) #10 x 1-1/2" S.M.S. INTO
SCREW BOSSES

MASONRY ANCHOR @ 6" EA.
SIDE OF POST AND @ 24" O.C.
MAX. SELECT CONCRETE
ANCHORS FROM SECTION 9

SIDE WALL HOLLOW POST TO BASE DETAIL

SCALE: 3" = 1'-0"

POOL ENCLOSURE UPRIGHT TO DECK ANCHOR REQUIREMENTS

General Notes and Specifications:

1. The uplift load on a pool enclosure upright is calculated as 1/2 the beam span x the beam spacing x the screen load of 7# / Sq. Ft.

EXAMPLE:

FOR A 2" x 6" BEAM WITH A SPAN OF 23' AND A BEAM & UPRIGHT SPACING
OF 7' USE: 1/2 x 17'-11" x 7' x 10# / Sq. Ft. = 627.2# UPLIFT

2. Table 1.6 of this manual uses the worst case loads for all cases.

3. In all cases there must be a primary anchor within 6" of each side of the upright.

4. For attachment to wood deck (min. 2" nominal thickness) use wood anchors with details shown above (min. 1-3/8" embedment).

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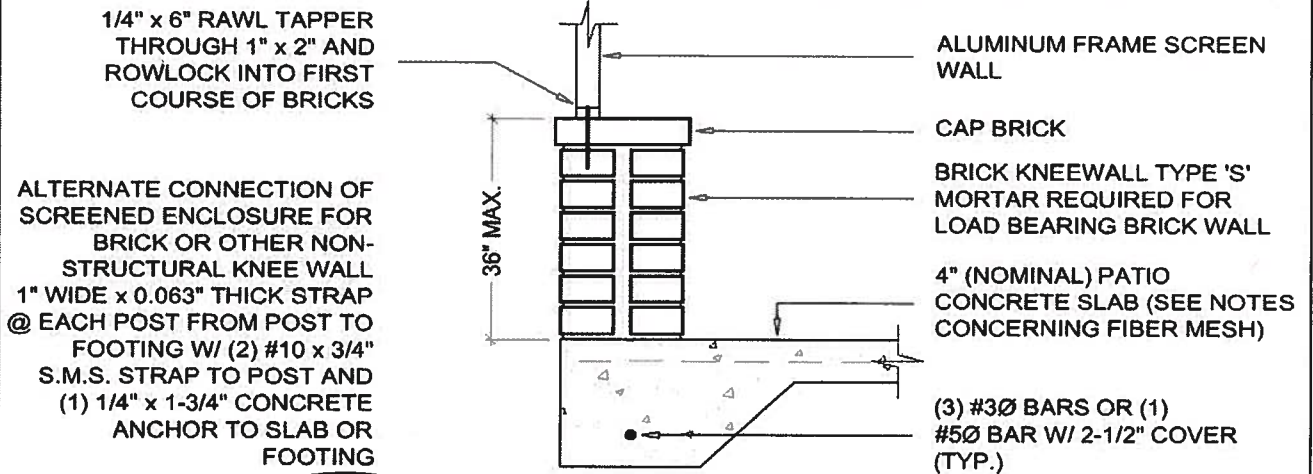
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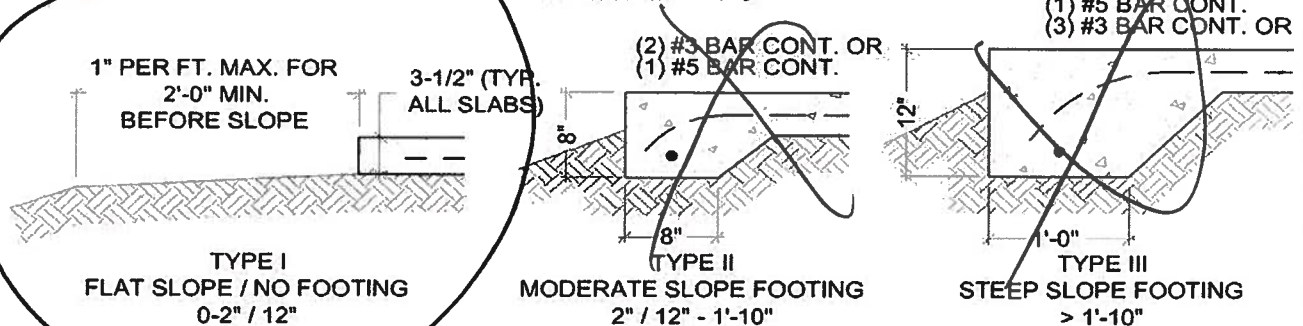
SCREENED ENCLOSURES

SECTION 1



BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 3/4" = 1'-0"



Notes for all foundation types:

1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure category in which case a type II footing is required.
4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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SECTION 1

SCREENED ENCLOSURES

Table 1.1 120 Allowable Spans for Primary Screen Roof Frame Members
Aluminum Alloy 6063 T-6
 For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
 Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

Hollow Sections	Tributary Load Width "W" = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span "L" / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-8"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud	19'-4"	Ud	18'-6"	Ud	17'-9"	Ud

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd
2" x 5" x 0.050 x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud	15'-1"	Ud
2" x 6" x 0.050 x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3"	Ud	18'-5"	Ud	17'-8"	Ud
2" x 7" x 0.055 x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	23'-1"	Ud	21'-11"	Ud	20'-11"	Ud	20'-2"	Ud
2" x 8" x 0.072 x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5"	Ud	28'-7"	Ud	27'-2"	Ud	25'-11"	Ud	24'-11"	Ud
2" x 9" x 0.072 x 0.224"	39'-3"	Pd	35'-11"	Ud	33'-4"	Ud	31'-5"	Ud	29'-10"	Ud	28'-6"	Ud	27'-5"	Ud
2" x 9" x 0.082 x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud	31'-11"	Ud	30'-7"	Ud	29'-5"	Ud
2" x 10" x 0.092 x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6"	Ud	34'-2"	Ud

Snap Sections	Tributary Load Width "W" = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span "L" / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-8"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud	20'-5"	Ud	19'-6"	Ud	18'-9"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud	23'-0"	Ud	22'-0"	Ud	21'-2"	Ud

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
 3. Span is measured from center of beam and upright connection to fascia or wall connection.
 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
 5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
 6. Spans may be interpolated.
 7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.
- Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-1"

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SCREENED ENCLOSURES

SECTION 1

Table 1.2 120 Allowable Spans for Secondary Screen Roof Frame Members

Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B", and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
3" x 2" x 0.045"	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb
3" x 2" x 0.070"	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd
2" x 3" x 0.045"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd

Snap Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb
2" x 3" x 0.045"	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd
2" x 4" x 0.045"	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 3" x 0.050"	11'-5"	Pb	11'-5"	Pb	11'-5"	Pb	11'-4"	Ud	10'-11"	Ud	10'-8"	Ud	10'-3"	Ud
2" x 4" x 0.050"	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb
2" x 5" x 0.062"	22'-4"	Pd	22'-4"	Pd	22'-4"	Pd	21'-7"	Ud	20'-11"	Ud	20'-4"	Ud	19'-7"	Ud

Snap Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb

Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Span is measured from center of beam and upright connection to fascia or wall connection.
3. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
4. Spans may be interpolated.
5. 2" x 4" & 2" x 5" Hollow Glirts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section fastened to beam with clips with 'W' = 5'-0" = 9'-1"

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SECTION 1

SCREENED ENCLOSURES

Table 1.3 110 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b	3'-11"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b	5'-9"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b	5'-6"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b	9'-7"	b

7'-9" x 1.10 = 8'-7"

Self Mating Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-11"	d	10'-10"	d	10'-0"	d	9'-5"	b	8'-8"	b	8'-0"	b	7'-6"	b
2" x 5" x 0.050" x 0.100"	14'-9"	d	13'-5"	d	12'-5"	d	11'-7"	b	10'-8"	b	9'-11"	b	9'-4"	b
2" x 6" x 0.050" x 0.120"	17'-3"	d	15'-8"	d	14'-4"	b	13'-1"	b	12'-0"	b	11'-3"	b	10'-6"	b
2" x 7" x 0.055" x 0.120"	19'-8"	d	17'-6"	b	15'-7"	b	14'-2"	b	13'-1"	b	12'-2"	b	11'-5"	b
2" x 8" x 0.072" x 0.224"	24'-4"	d	22'-1"	d	20'-6"	d	19'-4"	d	18'-4"	d	17'-6"	d	16'-10"	d
2" x 9" x 0.072" x 0.224"	26'-8"	d	24'-3"	d	22'-6"	d	21'-2"	d	20'-1"	d	19'-3"	d	18'-2"	b
2" x 9" x 0.082" x 0.310"	28'-8"	d	26'-0"	d	24'-2"	d	22'-9"	d	21'-7"	d	20'-8"	d	19'-10"	d
2" x 10" x 0.092" x 0.369"	33'-3"	d	30'-3"	d	28'-1"	d	26'-5"	d	25'-1"	d	23'-11"	d	23'-1"	d

Snap Sections	Tributary Load Width "W" = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b	4'-1"	b
2" x 3" x 0.045"	8'-10"	d	8'-1"	d	7'-6"	d	6'-11"	b	6'-3"	b	5'-9"	b	5'-3"	b
2" x 4" x 0.045"	11'-2"	d	10'-2"	d	9'-2"	b	8'-2"	b	7'-5"	b	6'-9"	b	6'-2"	b
2" x 6" x 0.062"	18'-3"	d	16'-7"	d	15'-5"	d	14'-6"	d	13'-9"	d	13'-2"	d	12'-8"	b
2" x 7" x 0.062"	20'-7"	d	18'-9"	d	17'-5"	d	16'-4"	d	15'-7"	d	14'-10"	d	14'-2"	b

Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select upright length "H".
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. Spans may be interpolated.
9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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SECTION 1

SCREENED ENCLOSURES

Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members

Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width "W" = Upright Spacing											
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"	13'-0"	14'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)											
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b
3" x 2" x 0.045"	8'-4"	d	7'-4"	b	6'-6"	b	5'-10"	b	5'-4"	b	4'-11"	b
3" x 2" x 0.070"	9'-5"	d	8'-6"	d	7'-9"	b	7'-0"	b	6'-5"	b	5'-11"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b

Snap Sections	Tributary Load Width "W" = Upright Spacing											
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"	13'-0"	14'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)											
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width "W" = Upright Spacing											
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"	13'-0"	14'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)											
3" x 2" x 0.045"	9'-7"	b	8'-3"	b	7'-3"	b	6'-6"	b	5'-11"	b	5'-6"	b
3" x 2" x 0.070"	11'-5"	b	9'-10"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b
2" x 3" x 0.045"	11'-2"	d	9'-9"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b
2" x 4" x 0.050"	12'-6"	b	10'-9"	b	9'-6"	b	8'-7"	b	7'-11"	b	7'-4"	b
2" x 5" x 0.062"	19'-3"	b	16'-7"	b	14'-9"	b	13'-5"	b	12'-4"	b	11'-6"	b

Snap Sections	Tributary Load Width "W" = Upright Spacing											
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"	13'-0"	14'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)											
2" x 2" x 0.044"	8'-10"	d	7'-8"	b	6'-9"	b	6'-0"	b	5'-5"	b	4'-11"	b

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans/heights to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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SECTION 1

SCREENED ENCLOSURES

Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright or Post	Upright or Post/Beam	Minimum Purlin, Girt & Knee Brace Size	Notes	Minimum Number of Screws*			Beam Stitching Screw at 24" OC
				#8 x 1/2"	#10 x 1/2"	#12 x 1/2"	
2 x 4 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H	2" x 3" x 0.044"	Full Lap	14	12	10	#12
2 x 8 SMB	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and Spacing of Screws		Gusset Plate Thickness	
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	0.063"
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1"	2" x 8" x 0.072" x 0.224"	0.125"
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0.308"	0.190"
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250"
3/8"	1"	2"		

* 0.082" wall thickness, 0.310" flange thickness

** (1) Stitching screw at 16" O.C. max.

Connection Example:

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate
(14) #8 x 1/2" sms ea. side of beam & upright.

Note:

1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of screws is based on the maximum allowable moment of the beam.
4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
5. Hollow splice connections can be made provided the connection is approved by the engineer.
6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
7. The side wall upright shall have a minimum beam size as shown above, i.e., a 2" x 4" upright shall have a 2" x 3" beam.
8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

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