

		4-2-13	7-3-0	1.2	-4-0	-	17-8-0	1	22-9-0	25-9	-3 30-0	1-0
		4-2-13	3-0-3	5-	1-0		5-4-0	70	5-1-0	3-0	-3 4-2-	13
Plate Offi LOADING TCLL TCDL BCLL	*********); [2:	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.25 1.25 NO	0,0-2-7], [9: CSI TC BC WB	0-3-14,0-0-6 0.80 0.94 0.44	DEFL Vert(LL) Vert(TL) Horz(TL)	in (lo 0.74 13-1 -1.31 13-1 0.65	c) I/defi 4 >481	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0		Code FBC2004/TF	12002	(Matr	ix)					Weight: 327	lb

BRACING

TOP CHORD

BOT CHORD

22-9-0

25-9-3

Structural wood sheathing directly applied or 2-11-4 oc purlins.

Rigid ceiling directly applied or 7-1-14 oc bracing.

30-0-0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

2 X 4 SYP No.2 WEBS

REACTIONS (ib/size) 2=2963/0-4-0, 9=2963/0-4-0

Max Horz 2=78(LC 5)

Max Uplift2=-1259(LC 5), 9=-1259(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-10338/4556, 3-4=-11145/5131, 4-5=-10559/4887, 5-6=-13646/6285, 6-7=-10595/4855, 7-8=-11184/5097 TOP CHORD

8-9=-10327/4507, 9-10=0/22

2-16=-4140/9363, 15-16=-4197/9483, 15-17=-6336/13789, 17-18=-6336/13789, 14-18=-636/13789, 14-19=-6336/13789, 19-20=-6336/13789, 20-21=-6336/13789, 13-21=-6336/13789, 13-22=-6218/13646, 22-23=-6218/13646, 12-23=-6218/13646, 11-12=-4102/9478, 9-11=-4043/9353

3-16=-387/280, 3-15=-633/1001, 5-15=-3492/1705, 5-14=-378/904, 5-13=-225/124, 6-13=-357/848, 6-12=-3305/1615,

8-12=-663/1047, 8-11=-410/290, 4-15=-2160/4753, 7-12=-2138/4755

NOTES

WEBS

BOT CHORD

2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

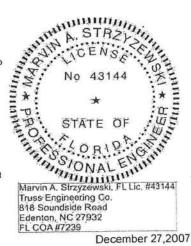
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1259 lb uplift at joint 2 and 1259 lb uplift at joint 9.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	
SUNBURY	HSGRD	SPECIAL	1	2	Job Reference (optional)	4585811

Maronda Homes Inc., Sanford, Florida

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NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 784 lb down and 467 lb up at 7-3-0, 243 lb down and 157 lb up at 9-0-12, 243 lb down and 157 lb up at 11-0-12, 243 lb down and 157 lb up at 15-0-0, 243 lb down and 157 lb up at 16-11-4, 243 lb down and 157 lb up at 18-11-4, and 243 lb down and 157 lb up at 20-11-4, and 784 lb down and 467 lb up at 22-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-46, 4-7=-46, 7-10=-46, 2-15=-40, 12-15=-40, 9-12=-40

Concentrated Loads (lb)

Vert: 15=-784(B) 12=-784(B) 17=-243(B) 18=-243(B) 19=-243(B) 20=-243(B) 21=-243(B) 22=-243(B) 23=-243(B)



Qty SUNBURY_FLORIDA_125 Truss Truss Type E4585813 14 SUNBURY JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:02 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 -1-0-0 4-4-12 4-4-12 2-7-4 1-0-0 Scale: 1/2"=1" 2x4 || 3 6.00 12 4 2x4 || 7-0-0 4-4-12 4-4-12 2-7-4 **PLATES** GRIP DEFL LOADING (psf) SPACING CSI Vdefl. L/d 2-0-0 244/190 1.25 TC 0.44 Vert(LL) -0.10 2-4 >765 240 MT20 16.0 Plates Increase TCLL 1.25 BC 0.42 Vert(TL) -0.262-4 >306 180 7.0 Lumber Increase

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

0.00

n/a

n/a

Weight: 29 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

10.0

10.0

WEBS

TCDL

BCLL

BCDL

2 X 4 SYP No.2

REACTIONS (lb/size) 2=349/0-4-0, 4=282/Mechanical

Max Horz 2=176(LC 6)

Max Uplift2=-111(LC 6), 4=-100(LC 6)

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-105/54 TOP CHORD 2-4=0/0

BOT CHORD WEBS

3-4=-149/173

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

WB 0.03

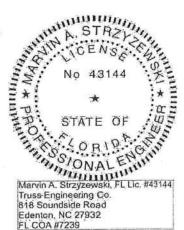
(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 100 lb uplift at joint 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the upon subject of the control structure is the responsibility of the upon subject of the control structure is the responsibility of the upon subject o



Job Truss Truss Type SUNBURY_FLORIDA_125 E4585815 SUNBURY J186 JACK 14 Job Reference (optional) Maronda Homes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:03 2007 Page 1 -1-0-0 4-4-12 7-0-0 1-0-0 4-4-12 2-7-4 Scale: 1/2"=1" 2x4 || 3 6.00 12 6 2x4 = 2×4 || 4-4-12 7-0-0 4-4-12 2-7-4 LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 (loc) I/defl L/d TCLL 1.25 TC 0.40 -0.09 >825 240 16.0 Plates Increase Vert(LL) MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.40 Vert(TL) -0.24 >330 180 BCLL 10.0 Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 n/a n/a Code FBC2004/TP12002 BCDL 10.0 (Matrix) Weight: 29 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=352/0-8-0, 4=273/Mechanical

Max Horz 2=176(LC 6)

2 X 4 SYP No.2

Max Uplift2=-120(LC 6), 4=-97(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-104/53

BOT CHORD 2-4=0/0

WEBS 3-4=-143/167

NOTES (6)

WEBS

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2 and 97 lb uplift at joint 4.

6)

LOAD CASE(S) Standard



December 27,2007

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Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters are reposely interesting the parameters and responsibility of building designer not fixes designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabaccions, quality control, storage, defever, erection and bracing. consult.

AMS/IPTI. ANSI/PTI. DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onotrio Drive, Modison, WI 53719.

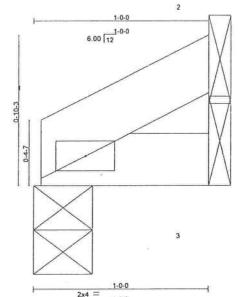


Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125 E45858	20
SUNBURY	J1S	JACK	1	1	Job Reference (optional)	20

Maronda Homes Inc., Sanford, Florida

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Scale = 1.6.2



1-0-0 LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d TCLL 16.0 Plates Increase 1.25 TC 0.01 Vert(LL) -0.00>999 240 >999 TCDL 7.0 Lumber Increase 1.25 BC 0.01 Vert(TL) -0.00 180 0.00 BCLL 10.0 Rep Stress Incr YES WB Horz(TL) -0.00n/a n/a

(Matrix)

PLATES GRIP MT20 244/190

Weight: 3 lb

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

10.0

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=39/0-4-0, 3=18/Mechanical, 2=21/Mechanical Max Horz 1=22(LC 6) Max Uplift1=-3(LC 6), 2=-22(LC 6)

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-17/8 **BOT CHORD** 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 1 and 22 lb uplift at joint
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not Iruss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to Iruss atability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, For general guidance regarding flobrication, qualify control, storage, defevery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

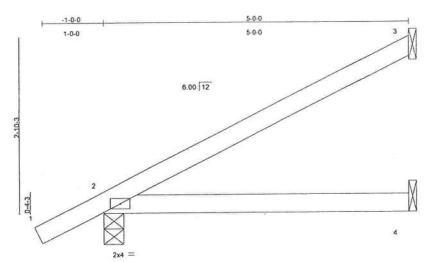


818 Soundside Roa Edenton, NC 27932

SUNBURY FLORIDA 125 Oty Truss Truss Type Job E4585821 JACK SUNBURY J2 Job Reference (optional)
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Maronda Homes Inc., Sanford, Florida

Scale = 1:17.8



GRIP CSI DEFL I/defl L/d **PLATES** LOADING (psf) in (loc) SPACING 2-0-0 Vert(LL) -0.03 >999 240 MT20 244/190 TC 0.21 TCLL 16.0 Plates Increase 1.25 -0.07 2-4 >842 180 BC 0.22 Vert(TL) 1.25 TCDL 7.0 Lumber Increase -0.00 n/a n/a WB 0.00 Horz(TL) YES BCLL 10.0 Rep Stress Incr Weight: 18 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=269/0-4-0, 4=95/Mechanical Max Horz 2=136(LC 6)

Max Uplift3=-95(LC 6), 2=-104(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-74/37

BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3 and 104 lb uplift at joint 2
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

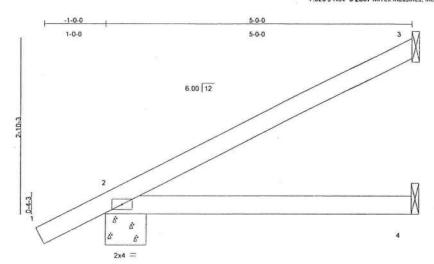


Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125
SUNBURY	J286	JACK	7	1	E4585823
					Job Reference (optional)

Maronda Homes Inc., Sanford, Florida

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Scale = 1:17.8



5-0-0

LOADIN	G (psf)	10	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0		Plates Increase	1.25	TC	0.18	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.06	2-4	>937	180	THE STATE OF THE S	
BCLL	10.0	•	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0		Code FBC2004/TF	12002	(Matr	ix)	1					Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical

Max Horz 2=136(LC 6)

Max Uplift3=-91(LC 6), 2=-114(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

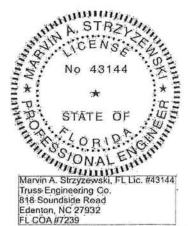
TOP CHORD 1-2=0/21, 2-3=-72/36 2-4=0/0

BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3 and 114 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

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Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding lobrication, quality control, storage, delivery, erection and bracing, consult

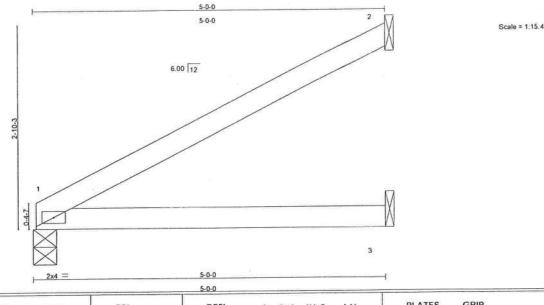
ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Intermation available from Truss Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.



SUNBURY_FLORIDA_125 Qty Truss Type Job E4585828 JACK SUNBURY Job Reference (optional)

Maronda Homes Inc., Sanford, Florida

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PLATES GRIP DEFL CSI (loc) I/defl 1 /d LOADING (psf) SPACING 2-0-0 MT20 244/190 >999 240 TC 0.23 Vert(LL) -0.031-3 Plates Increase 1.25 TCLL 16.0 180 >865 BC 0.21 Vert(TL) -0.071-3 Lumber Increase 1.25 TCDL 7.0 -0.00 n/a Rep Stress Incr WB 0.00 Horz(TL) n/a BCLL 10.0 Weight: 16 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=203/0-4-0, 2=109/Mechanical, 3=95/Mechanical Max Horz 1=104(LC 6)

Max Uplift1=-22(LC 6), 2=-106(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-81/41

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 106 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

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SUNBURY_FLORIDA_125 Truss Type Job Truss E4585829 JACK SUNBURY Job Reference (optional) s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:06 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-0-0 1-0-0 3-0-0 1-0-0 Scale = 1:10.8 6.00 12 0-4-3 3-0-0 3-0-0 GRIP PLATES DEFL SPACING CSI in I/defi L/d LOADING (psf) 2-0-0 244/190 MT20 >999 240 1.25 TC 0.12 Vert(LL) -0.00 2-4 TCLL 16.0 Plates Increase 180 >999 BC 0.07 Vert(TL) -0.01 2-4 1.25 TCDL 7.0 Lumber Increase 3 n/a YES WB 0.00 Horz(TL) -0.00 n/a Rep Stress Incr 10.0

LUMBER

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

10.0

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 11 lb

REACTIONS (lb/size) 3=51/Mechanical, 2=188/0-4-0, 4=55/Mechanical Max Horz 2=95(LC 6)

Code FBC2004/TPI2002

Max Uplift3=-44(LC 6), 2=-101(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-36/18 TOP CHORD

2-4=0/0 **BOT CHORD**

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(Matrix)

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3 and 101 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Mifek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not thus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP1 Quality Citleria, DSB-89 and BCS11 Building Component Salety Internation available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



SUNBURY_FLORIDA_125 Truss Type Qty Job Truss E4585831 J3B6 SUNBURY Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:09 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-0-0 1-0-0 3-0-0 1-0-0 Scale = 1:10.8 6.00 12 D-4-3 Ď. Š. Ų. V. 3-0-0 3-0-0 GRIP DEFL PLATES (loc) I/defl LOADING (psf) SPACING 2-0-0 244/190 Vert(LL) -0.00 2-4 >999 240 MT20 1.25 TC 0.15 TCLL 16.0 Plates Increase BC 0.06 Vert(TL) -0.01 2-4 >999 180 1.25 TCDL 7.0 Lumber Increase WB 0.00 Horz(TL) -0.00 3 n/a n/a YES BCLL 10.0 Rep Stress Incr Weight: 11 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING

TOP CHORD

BOT CHORD

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical

Max Horz 2=95(LC 6)

Max Uplift3=-37(LC 6), 2=-113(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-39/15

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 113 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters along the new parameters and responsibility of the second proper incorporation of component is responsibility of using parameters and proper incorporation of component is responsibility of building designer and truss designer. Bracing shown is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the during construction is the responsibility of the design of the overall structure is the responsibility of the during control structure is the responsibility of the second parameters and proper incorporation and property of the control structure is the responsibility of the second property of the control parameters and property of the control parameters are second property of the control parameters and property of the paramet

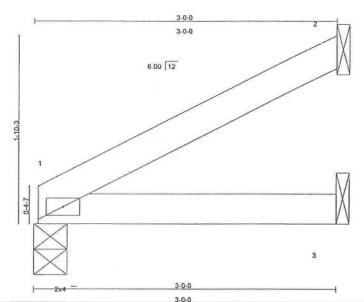


Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	
SUNBURY	J3S	JACK	1	1	Job Reference (optional)	5836

Maronda Homes Inc., Sanford, Florida

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Scale = 1:10.8



LOADIN	IG (psf)	- 1	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0	- 1	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	1-3	>999	240	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	1-3	>999	180	4 40	
BCLL	10.0	•	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0		Code-FBC2004/TF	212002	(Mati	rix)	2.3					Weight: 10 lb	K:

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=117/0-4-0, 2=63/Mechanical, 3=55/Mechanical Max Horz 1=63(LC 6)

Max Uplift1=-11(LC 6), 2=-62(LC 6)

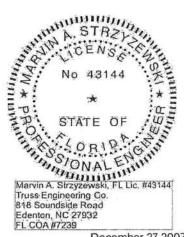
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-49/24 BOT CHORD 1-3=0/0

NOTES (6)

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1 and 62 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for talteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control storage, delivery, erection and bracing, consult. AMSI/TRIOL States (after yet) delivery, erection and bracing. AMSI/TRIOL States (after yet) and yet) and yet) are reported in the yet) and yet).



SUNBURY_FLORIDA_125 Job Truss Truss Type Qty E4585837 SUNBURY JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:11 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3 -1-0-0 1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 2 1-0-0

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

in

-0.00

-0.00

0.00

(loc)

2 >999

3

l/defl

>999

L/d

240

180

n/a

LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

16.0

7.0

10.0

10.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=121/0-4-0, 4=19/Mechanical, 3=-7/Mechanical

2-0-0

1.25

1.25

YES

Max Horz 2=54(LC 6)

Max Uplift2=-111(LC 6), 3=-7(LC 1)

SPACING

Plates Increase

Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

Max Grav2=121(LC 1), 4=19(LC 1), 3=22(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-25/22

BOT CHORD 2-4=0/0

NOTES (6)

Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

TC

BC 0.01

WB 0.00

(Matrix)

0.10

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 7 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



GRIP

244/190

PLATES

Weight: 5 lb

MT20

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TRI Quality Criteria, DSS-89 and BCS11 Building Component Safety Intermation available from Truss Plate Institute, 583 D'Onafrio Drive, Modison, WI 53719.



818 Soundside Road Edenton, NC 27932

SUNBURY_FLORIDA_125 Truss Type Job Truss E4585839 SUNBURY J486 JACK Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:12 2007 Page 1 Maronda Homes Inc., Sanford, Florida 1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 ∆° 1-0-0 2x4 = GRIP DEFL L/d **PLATES** CSI in (loc) I/defl LOADING (psf) SPACING 2-0-0 -0.00 >999 240 MT20 244/190 Vert(LL) 0.11 1.25 TC TCLL 16.0 Plates Increase -0.00 >999 180 BC Vert(TL) Lumber Increase 1.25 0.01 TCDL 7.0 3 n/a 0.00 Horz(TL) n/a WB BCLL 10.0 Rep Stress Incr YES

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

10.0

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 5 lb

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical

Code FBC2004/TPI2002

Max Horz 2=54(LC 6)

Max Uplift2=-116(LC 6), 3=-11(LC 1)

Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-27/26 \

BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(Matrix)

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 11 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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SUNBURY_FLORIDA_125 Truss Type Job Truss F4585847 SUNBURY IGRO MONO TRUSS Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:15 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 -1-5-0 5-1-15 4-8-1 1-5-0 Scale = 1:23.6 3x4 II 4.24 12 3x4 = 10 11 6 12 2x4 11 3x5 2x4 = 9-10-1 5-1-15 5-1-15 Plate Offsets (X.Y): [2:0-1-15.0-0-7] **PLATES** GRIP SPACING 2-0-0 CSI DEFL (loc) LOADING (psf) Vert(LL) -0.03 5-6 >999 240 MT20 244/190 1.25 TC 0.24 Plates Increase TCLL 16.0 1.25 BC 0.33 Vert(TL) -0.06 5-6 >999 180 Lumber Increase TCDL 7.0 Rep Stress Incr NO WB 0.29 Horz(TL) 0.01 n/a n/a 10.0 BCLL Weight: 46 lb Code FBC2004/TPI2002 (Matrix) 10.0 BCDL BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.2

(lb/size) 5=576/Mechanical, 2=536/0-5-11

Max Horz 2=174(LC 3)

Max Uplift5=-237(LC 3), 2=-169(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-7=-905/228, 7-8=-870/250, 3-8=-830/229, 3-9=-123/50, 4-9=-55/0, 4-5=-129/142

BOT CHORD 2-10=-302/815, 10-11=-302/815, 6-11=-302/815, 6-12=-302/815, 5-12=-302/815

WEBS 3-6=0/293, 3-5=-812/290

NOTES

REACTIONS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 5 and 169 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 33 lb up at 4-4-0, 5 lb down and 33 lb up at 4-4-0, 56 lb down and 100 lb up at 7-1-15, 56 lb down and 100 lb up at 7-1-15, and 38 lb down at 1-6-1, and 38 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, 15 lb down at 4-4-0, 15 lb down at 4-4-0, and 55 lb down at 7-1-15, and 55 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 8=-10(F=-5, B=-5) 9=-113(F=-56, B=-56) 10=42(F=21, B=21) 11=-31(F=-15, B=-15) 12=-111(F=-55, B=-55)

A. STRZY
CENS
No 43144

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Marvin A. Strzyzewski, FLLic, #43144

Truss Engineering Co.

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.
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818 Soundside Road Edenton, NC 27932

Job Truss Truss Type SUNBURY_FLORIDA_125 E4585851 SUNBURY JGRD86 MONO TRUSS Job Reference (optional) Maronda Homes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:17 2007 Page 1 -1-5-0 5-1-15 9-10-1 5-1-15 1-5-0 4-8-1 Scale = 1:23.6 3x4 || 4.24 12 3x4 = 0-3-14 13 6 2x4 11 5-1-15 9-10-1 5-1-15 4-8-1 Plate Offsets (X.Y): [2:0-3-3.0-0-15] LOADING (psf) SPACING CSI 2-0-0 DEFL (loc) I/defl L/d PLATES GRIP -0.03 16.0 >999 TC 0.23 Vert(LL) 244/190 TCLL Plates Increase 1.25 5-6 240 MT20 7.0 1.25 BC 0.32 -0.06 >999 180 TCDL Lumber Increase Vert(TL) 5-6 BCLL 10.0 Rep Stress Incr NO WB 0.29 Horz(TL) 0.01 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 46 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=565/Mechanical, 2=495/1-0-1

Max Horz 2=173(LC 3)

Max Uplift5=-223(LC 3), 2=-137(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-7=-883/183, 7-8=-881/187, 8-9=-852/209, 3-9=-817/196, 3-10=-118/46, 4-10=-53/0, 4-5=-125/140 TOP CHORD

BOT CHORD 2-11=-265/802, 11-12=-265/802, 12-13=-265/802, 6-13=-265/802, 6-14=-265/802, 5-14=-265/802

WEBS 3-6=0/284, 3-5=-801/251

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 5 and 137 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 24 lb up at 4-4-12, 24 lb up at 4-4-12, 50 lb down and 94 lb up at 7-2-11, 50 lb down and 94 lb up at 7-2-11, and 40 lb down at 1-6-12, and 40 lb down at 1-6-12 on top chord, and 22 lb up at 1-6-12, 22 lb up at 1-6-12, 12 lb down at 4-4-12, 12 lb down at 4-4-12, and 52 lb down at 7-2-11, and 52 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-46, 2-7=-14, 4-7=-46, 2-11=-20, 5-11=-40

Concentrated Loads (lb)

Vert: 9=7(F=3, B=3) 10=-100(F=-50, B=-50) 12=43(F=22, B=22) 13=-24(F=-12, B=-12) 14=-104(F=-52, B=-52)

Truss Engineering Co.

818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

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818 Soundside Road Edenton, NC 27932

SUNBURY FLORIDA 125 Truss Type Truss Job F4585855 MONO SCISSOR SUNBURY JSI Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:18 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 -1-0-0 1-0-0 7-0-0 Scale: 1/2"=1" 244 11 6.00 12 2x4 = 3.00 12 7-0-0 7-0-0 GRIP PLATES DEFL I/defl L/d CSI (loc) SPACING 2-0-0 LOADING (psf) 244/190 Vert(LL) -0.11 2-4 >742 240 MT20 TC 0.44 Plates Increase 1.25 TCLL 16.0 BC -0.27 2-4 >297 180 0.42 Vert(TL) Lumber Increase 1.25 TCDL 0.00 n/a n/a WB 0.03 Horz(TL) BCLL 10.0 Rep Stress Incr YES Weight: 27 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD

BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=346/0-4-0, 4=283/Mechanical

Max Horz 2=173(LC 6) Max Uplift2=-105(LC 6), 4=-102(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-108/55

BOT CHORD 2-4=-26/26

WEBS 3-4=-149/176

NOTES

- 1) Wind: ASCE 7-02, 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 2 and 102 lb uplift at joint 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. WARRIAN - Veryly design parameters and KEAD NOLES ON THIS AND INCLUDED BILLER KREEKENES PARE MILTER'S BEFORE OSS.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the certain structure is the responsibility of the substituting designer, For general guidance regarding erector. Additional permanent brocking of the overall structure is the responsibility of the modern parameters and the parameters of the overall structure is the responsibility of the parameters. For general guidance regarding erector. Additional permanent brocking of the overall structure is the responsibility of the parameters. For general guidance regarding erector. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



SUNBURY FLORIDA 125 Qty Truss Type Truss Job E4585856 MONO SCISSOR SUNBURY J52 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:19 2007 Page 1 Maronda Homes Inc., Sanford, Florida 5-0-0 -1-0-0 3 1-0-0 5-0-0 6.00 12 -0-4-3 3.00 12 GRIP PLATES DEFL I/defl L/d CSI (loc) LOADING (psf) SPACING 2-0-0 244/190 Vert(LL) -0.03 >999 240 MT20 TC BC

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2

16.0

7.0

10.0

BOT CHORD 2 X 4 SYP No.2

BRACING

Vert(TL)

Horz(TL)

-0.07

-0.00

2-4 >817

3

n/a

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Weight: 18 lb

Rigid ceiling directly applied or 10-0-0 oc bracing.

180

n/a

REACTIONS (lb/size) 3=103/Mechanical, 2=265/0-4-0, 4=95/Mechanical

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

Max Horz 2=133(LC 6)

Max Uplift3=-98(LC 6), 2=-98(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-76/38 TOP CHORD

BOT CHORD 2-4=-19/19

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

0.21

0.22

WB 0.00

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

1.25

YES

- 3)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 3 and 98 lb uplift at joint 2.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

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ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



Job .	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_1	25	
SUNBURY	JS3	MONO SCISSOR	4	1			E4585857
Maronda Homes Inc., Sanford,	Florida			7.0	Job Reference (option 20 s Nov 9 2007 MiTek Ind	al) ustries, Inc. Thu De	c 27 10:03:19 2007 Page 1
	-1-0-0		3-0-0		3	\rightarrow	
	1-0-0		3-0-0				Scale = 1:10.8
		6.00	12			1.2.3	
	1-10-3		///			$ \bigvee$	
		2				<u> </u>	
	0-4-3				4	0-7-10	
			3.0	0 12			
		3x4 =	3-0-0			- 7	
			3-0-0			1	
LOADING (psf)	SPACING 2-0-0		DEFL in	(loc)	l/defl L/d	PLATES	GRIP

-0.00

-0.01

-0.00

2-4 >999

2-4 >999 240

180

n/a

n/a

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

16.0

7.0

10.0

10.0

REACTIONS (lb/size) 3=52/Mechanical, 2=184/0-4-0, 4=55/Mechanical Max Horz 2=92(LC 6)

Code FBC2004/TPI2002

Plates Increase

Lumber Increase

Rep Stress Incr

Max Uplift3=-48(LC 6), 2=-94(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-38/19

BOT CHORD 2-4=-11/11

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

TC 0.11

BC 0.07

WB 0.00

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

1.25

YES

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3 and 94 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



244/190

MT20

Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 12 lb

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 BEFORE VOID.

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ANSI/IPI1 Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Piale Institute, 583 D'Onafio Drive, Madison, WI 53719.



SUNBURY_FLORIDA_125 Truss Type Qty Job E4585858 MONO SCISSOR SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:19 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-0-0 1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 3×4 = 3.00 12 1-0-0 1-0-0 PLATES GRIP LOADING (psf) SPACING CSI DEFL (loc) I/defl L/d 2-0-0 244/190 1.25 TC 0.09 Vert(LL) -0.00 >999 240 MT20 Plates Increase TCLL 16.0 BC 0.01 Vert(TL) -0.00 >999 180 1.25 TCDL 7.0 Lumber Increase WB 0.00 Horz(TL) 0.00 3 n/a n/a Rep Stress Incr 10.0 BCLL Weight: 5 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=114/0-4-0, 4=19/Mechanical, 3=-4/Mechanical Max Horz 2=51(LC 6) Max Uplift2=-98(LC 6), 3=-7(LC 5) Max Grav2=114(LC 1), 4=19(LC 1), 3=14(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/17, 2-3=-25/17

BOT CHORD 2-4=-4/4

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 2 and 7 lb uplift at joint
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE Design valid for use only with MTeX connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the wilding designer. For general guidance regarding distriction, quality control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



SUNBURY FLORIDA 125 Truss Truss Type Oty Job E4585864 JSGRD MONO SCISSOR SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:22 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 1-5-0 5-4-0 1-5-0 Scale = 1:23.4 3x4 11 2-1-12 4.24 12 4x6 = 5x6 = 1.7.4 12 11 2x4 11 2.12 12 10 3×4 = 9-10-1 PLATES GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d 1.25 TC 0.30 Vert(LL) -0.06 2-6 >999 240 MT20 244/190 TCLL 16.0 Plates Increase 1.25 BC 0.46 Vert(TL) -0.112-6 >999 180 7.0 TCDL Lumber Increase NO WR 0.36 Horz(TL) 0.02 5 n/a n/a BCLL 10.0 Rep Stress Incr Weight: 42 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 5-1-0 oc purlins, except TOP CHORD

end verticals.

Rigid ceiling directly applied or 8-5-5 oc bracing.

BOT CHORD

REACTIONS (lb/size) 2=536/0-5-11, 5=580/Mechanical

Max Horz 2=173(LC 3)

2 X 4 SYP No.2

Max Uplift2=-180(LC 3), 5=-248(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/19, 2-7=-1496/472, 7-8=-1456/490, 3-8=-1423/473, 3-9=-210/95, 4-9=-143/35, 4-5=-110/132 TOP CHORD

2-10=-543/1378, 10-11=-534/1381, 6-11=-533/1403, 6-12=-547/1371, 5-12=-539/1407 **BOT CHORD**

3-6=0/273, 3-5=-1212/455 WEBS

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

NOTES

WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 248 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 38 lb up at 4-4-12, 6 lb down and 38 lb up at 4-4-12, 57 lb down and 103 lb up at 7-2-11, 57 lb down and 103 lb up at 7-2-11, and 35 lb down at 1-6-12, and 35 lb down at 1-6-12 on top chord, and 21 lb up at 1-6-12, 21 lb up at 1-6-12, 15 lb down at 4-4-12, 15 lb down at 4-4-12, and 55 lb down at 7-2-11, and 55 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 8=-13(F=-6, B=-6) 9=-114(F=-57, B=-57) 10=42(F=21, B=21) 11=-31(F=-15, B=-15) 12=-111(F=-55, B=-55)

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

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ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



SUNBURY FLORIDA 125 Qty Truss Type Truss Job F4585866 10 MONO TRUSS SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:23 2007 Page 1 Maronda Homes Inc., Sanford, Florida 6-0-0 -1-0-0 6-0-0 Scale = 1:15.8 2x4 |1 3 4.00 12 2x4 GRIP PLATES DEFL I/defl L/d CSI (loc) SPACING 2-0-0 LOADING (psf) Vert(LL) 0.22 >305 240 MT20 244/190 0.23 TC Plates Increase 1.25 TCLL 16.0 BC 0.17 2-4 >393 180 Vert(TL) 0.41 Lumber Increase 1.25 TCDL 0.00 n/a n/a WB 0.00 Horz(TL) YES BCLL 10.0 Rep Stress Incr Weight: 22 lb

BRACING

TOP CHORD

BOT CHORD

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.1D BOT CHORD 2 X 4 SYP No.2

10.0

WEBS

2 X 4 SYP No.2

REACTIONS (lb/size) 2=306/0-3-8, 4=240/0-1-8

Max Horz 2=103(LC 4)

Max Uplift2=-214(LC 4), 4=-159(LC 4)

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/14, 2-3=-57/33, 3-4=-126/126

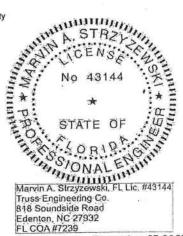
TOP CHORD BOT CHORD 2-4=0/0

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2 and 159 lb uplift at joint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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Job Truss Truss Type SUNBURY_FLORIDA_125 E4585871 SUNBURY RG2 Job Reference (optional) Maronda Humes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:25 2007 Page 1 5-6-6 11-0-13 5-6-6 5-6-7 Scale = 1:20.0 4x6 = 6.00 12 2x4 | 2x4 11 3x4 = 2x4 || 2x4 || 2x4 || 3x4 > 11-0-13 LOADING (psf) SPACING CSI DEFL **PLATES** GRIP 2-0-0 in (loc) 1/defl L/d TC TCIL 16.0 Plates Increase 1.25 0.11 Vert(LL) 244/190 n/a n/a 999 MT20 1.25 BC 0.07 7.0 TCDL Lumber Increase Vert(TL) n/a n/a 999 BCLL 10.0 Rep Stress Incr NO WB 0.03 0.00 5 Horz(TL) n/a n/a Code FBC2004/TPI2002 BCDL (Matrix) Weight: 40 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2 X 4 SYP No.2 REACTIONS (lb/size) 1=106/11-1-13, 5=103/11-1-13, 7=105/11-1-13, 8=268/11-1-13, 6=264/11-1-13 Max Horz 1=-46(LC 4) Max Uplift1=-35(LC 7), 5=-38(LC 7), 8=-167(LC 6), 6=-165(LC 7) Max Grav1=106(LC 1), 5=103(LC 1), 7=105(LC 1), 8=269(LC 10), 6=264(LC 11) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-34/34, 2-3=-34/117, 3-4=-33/114, 4-5=-25/30 **BOT CHORD** 1-8=0/54, 7-8=0/54, 6-7=0/55, 5-6=0/55 3-7=-56/7, 2-8=-141/234, 4-6=-139/233 WEBS

NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 38 lb uplift at joint 5, 167 lb uplift at joint 8 and 165 lb uplift at joint 6.
- 8) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard

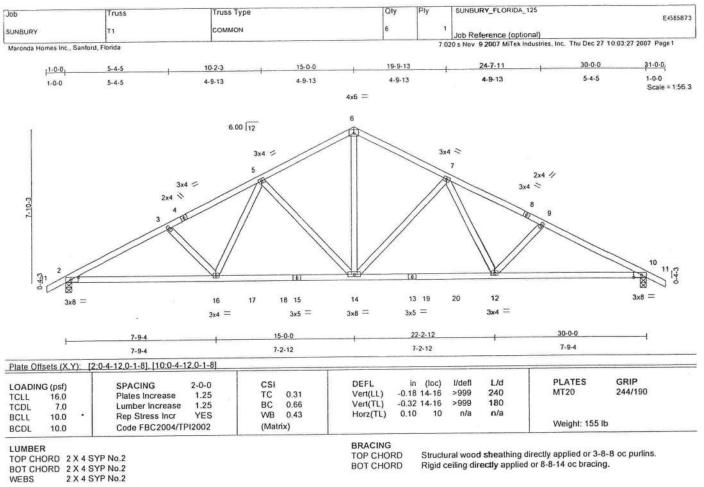


December 27,2007

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REACTIONS (lb/size) 2=1392/0-4-0, 10=1392/0-4-0

Max Horz 2=-137(LC 7)

Max Uplift2=-309(LC 6), 10=-309(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension 1-2=0/20, 2-3=-2441/724, 3-4=-2287/666, 4-5=-2233/682, 5-6=-1589/568, 6-7=-1589/568, 7-8=-2233/682,

TOP CHORD 8-9=-2287/666, 9-10=-2441/724, 10-11=0/20

BOT CHORD

2-16=-519/2136, 16-17=-342/1760, 17-18=-342/1760, 15-18=-342/1760, 14-15=-342/1760, 13-14=-342/1760, 13-19=-342/1760, 19-20=-342/1760, 12-20=-342/1760, 10-12=-519/2136 3-16=-205/215, 5-16=-70/561, 5-14=-568/272, 6-14=-314/1144, 7-14=-568/272, 7-12=-70/561, 9-12=-205/215 WEBS

NOTES

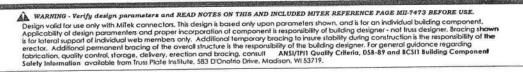
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ff; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard







SUNBURY_FLORIDA_125 Qty Truss Type Job Truss F4585876 COMMON TIA SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:29 2007 Page 1 Maronda Homes Inc., Sanford, Florida 21-0-0 22-0-0 10-6-0 15-5-12 -1-0-0 4-11-12 1-0-0 4-11-12 1-0-0 5-6-4 Scale = 1:40.5 4x6 = 6.00 12 2x4 1 10 9 11 12 3x4 = 3x4 = 3x4 = 13-9-13 21-0-0 7-2-3 7-2-3 6-7-11 7-2-3 Plate Offsets (X,Y): [2:0-2-10.0-1-8], [6:0-2-10.0-1-8] PLATES DEFL (loc) I/def L/d 2-0-0 SPACING LOADING (psf) MT20 244/190 >999 240 1.25 TC 0.23 Vert(LL) -0.118-10 Plates Increase 16.0 TCLL -0.17 180 8-10 >999 BC Vert(TL) 1.25 0.48 7.0 Lumber Increase TCDL n/a 0.04 WB 0.11 Horz(TL) 10.0 Rep Stress Incr YES BCLL Weight: 97 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 4-10-5 oc purlins. TOP CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2 WEBS REACTIONS (lb/size) 2=971/0-8-0, 6=971/0-8-0 Max Horz 2=104(LC 6)

Max Uplift2=-247(LC 6), 6=-247(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-1498/434, 3-4=-1382/436, 4-5=-1382/436, 5-6=-1498/434, 6-7=0/21 TOP CHORD

2-10=-253/1288, 9-10=-85/886, 9-11=-85/886, 11-12=-85/886, 8-12=-85/886, 6-8=-253/1288 BOT CHORD

3-10=-202/207, 4-10=-108/583, 4-8=-108/583, 5-8=-202/207 WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 247 lb uplift at joint 6.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not have designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the certain parameters and proper of the overall structure is the responsibility of the validing designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the validing designer. For general guidance regarding laborication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



Qty SUNBURY_FLORIDA_125 Job Truss Truss Type E4585877 SUNBURY TIAS COMMON Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:29 2007 Page 1 Maronda Homes Inc. Sanford Florida 21-0-0 15-5-12 -1-0-0 5-6-4 10-6-0 4-11-12 4-11-12 5-6-4 1-0-0 Scale = 1:39.7 4x6 = 6.00 12 2x4 1 2x4 // 0-4-3 10 8 3x4 > 3x4 = 3x4 = 3x4 = 21-0-0 7-2-3 13-9-13 7-2-3 7-2-3 6-7-11 Plate Offsets (X,Y): [2:0-2-10.0-1-8], [6:0-2-10.0-1-8] PLATES DEFL I/defi Lld LOADING (psf) SPACING 2-0-0 CSI (loc) 7-9 240 MT20 244/190 TC 0.25 -0,10 >999 TCLL 16.0 Plates Increase 1.25 Vert(LL) -0.17 >999 180 1.25 BC 0.49 Vert(TL) TCDI 7.0 Lumber Increase 0.11 0.04 n/a YES WB Horz(TL) n/a 10.0 Rep Stress Incr BCLL Code FBC2004/TPI2002 (Matrix) Weight: 95 lb BCDL 10.0 BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 6=905/0-8-0, 2=973/0-8-0

Max Horz 2=113(LC 6)

Max Uplift6=-164(LC 7), 2=-247(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-1502/444, 3-4=-1386/446, 4-5=-1395/466, 5-6=-1511/465 TOP CHORD

2-9=-315/1292, 9-10=-147/891, 8-10=-147/891, 8-11=-147/891, 7-11=-147/891, 6-7=-339/1302 **BOT CHORD**

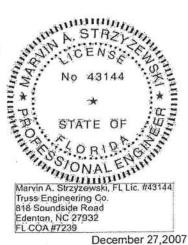
3-9=-202/208, 4-9=-105/583, 4-7=-137/595, 5-7=-209/222 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 6 and 247 lb uplift at joint 2.

LOAD CASE(S) Standard



SUNBURY_FLORIDA_125 Truss Truss Type Job E4585885 VALLEY SUNBURY V2A Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:34 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-1-13 3-6-14 3-6-14 3-6-15 3x4 = 2 6.00 12 2x4 > 2x4 = 7-1-13 7-1-13 Plate Offsets (X,Y): [2:0-2-0,Edge] PLATES DEFL in I/defl L/d GRIP 2-0-0 SPACING LOADING (psf) 244/190 1.25 TC 0.10 Vert(LL) n/a n/a 999 MT20 16.0 Plates Increase TCLL 1.25 BC 0.36 Vert(TL) n/a n/a 999 TCDL 7.0 Lumber Increase Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a BCLL 10.0 Weight: 20 lb BCDL 10.0 Code FBC2004/TPI2002 (Matrix) LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=254/7-1-13, 3=254/7-1-13

Max Horz 1=-24(LC 4) Max Uplift1=-48(LC 6), 3=-48(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-176/143, 2-3=-176/143

BOT CHORD 1-3=-87/136

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 48 lb uplift at joint 3.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stobility during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobication, quality control, storage, delivery, erection and bracing, consult. ANS/IPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



SUNBURY_FLORIDA_125 Job Truss Type Truss E4585886 VALLEY SUNBURY V28 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:34 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-1-13 1-6-14 1-6-15 6.00 12 3 2x4 = 2x4 > Plate Offsets (X,Y): [2:0-2-0.Edge] DEFL in I/defl L/d PLATES GRIP 2-0-0 CSI SPACING LOADING (psf) 244/190 1.25 TC 0.02 Vert(LL) n/a n/a 999 MT20 Plates Increase TCLL 16.0 1.25 BC 0.04 Vert(TL) n/a n/a 999 Lumber Increase TCDL 7.0 3 YES WB 0.00 Horz(TL) 0.00 n/a n/a BCLL 10.0 Rep Stress Incr Code FBC2004/TPI2002 Weight: 8 lb (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 3-1-13 oc purlins. TOP CHORD

BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=82/3-1-13, 3=82/3-1-13

Max Horz 1=-8(LC 4) Max Uplift1=-16(LC 6), 3=-16(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=-57/48, 2-3=-57/48

TOP CHORD **BOT CHORD** 1-3=-29/44

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 1 and 16 lb uplift at joint 3.

LOAD CASE(S) Standard



Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

Oty SUNBURY_FLORIDA_125 Job Truss Truss Type E4585889 VALLEY SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10.03:35 2007 Page 1 Maronda Homes Inc., Sanford, Florida 2x4 || 4-0-0 4-0-0 Scale = 1:8.4 4.00 12 PLATES GRIP DEFL CSI I/defl L/d SPACING (loc) LOADING (psf) MT20 244/190 Vert(LL) n/a n/a 999 1.25 TC 0.09 16.0 Plates Increase TCLL n/a n/a 999 BC. Vert(TL) 7.0 Lumber Increase 1.25 0.08 TCDL 0.00 n/a n/a WB Horz(TL) 10.0 Rep Stress Incr YES 0.00 BCLL Weight: 12 lb (Matrix) 10.0 Code FBC2004/TPI2002 BCDL BRACING LUMBER Structural wood sheathing directly applied or 4-0-0 oc purlins, except TOP CHORD TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 1=127/4-0-0, 3=127/4-0-0

Max Horz 1=41(LC 4)

Max Uplift1 = -24(LC 4), 3=-38(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-30/17, 2-3=-68/72 TOP CHORD

BOT CHORD 1-3=0/0

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1 and 38 lb uplift at joint 3.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. WARRING - Veryy design parameters and XEAD NOTES ON THIS ARD INCLUDED BITTER REFERENCE PAGE BILL-1973 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design poramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the execution parameter bracing of the overall structure is the responsibility of the building designer, for general guidance regarding clobication, qualify control, storage, delivery, erection and bracing, consult.

ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Piate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





RE: ELEV F - SUNBURY FL

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Project Customer: Maronda Homes Project Name: Sunbury FL

Lot/Block: Sanford Address: Sanford

Subdivision: Sanford

City: Sanford

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building. License #: Name:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 7.0

Wind Code: N/A

Wind Speed: N/A mph

Floor Load: 55.0 psf

Roof Load: N/A psf

This package includes 5 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date .
1	E4617771	FA	1/15/08
2	E4617772	FB	1/15/08
3	E4617773	FC	1/15/08
4	E4617774	FD	1/15/08
5	E4617775	FE	1/15/08

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



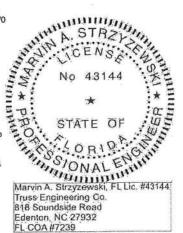
Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

SUNBURY FL Truss Type Joh Truss E4617771 SUNBURY FL ALL ELEVATIONS Job Reference (optional) FLOOR ELEV F 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:14 2008 Page 1 Maronda Homes, Inc. Pittsburgh, PA 0-10-4 0-1-8 9-7-8 9-7-8-1-8 Scale = 1:52.2 9-141 1-0-14 H 9-10-8 11-0-0 9-10-2 1-9-12 9-11-19 TRUSS IS DESIGNED TO SUPPORT A SMALL CONCENTRATED LOAD AT ITS CANTILEVERED END(S). 4x6 = 1.5x4 11 1.5x4 II 3x6 4x6 = 3x6 = 3x8 = 4x6 = 3x4 II 4x8 = = 3x6 4x8 = 3x6 = 1.5x4 3x4 3x4 22 14 15 16 17 19 20 21 10 11 12 13 33 3130 29 27 26 39 37 36 46 45 44 43 42 41 40 38 50 49 48 1.5x4 II 3x6 FP = 4x6 = 4x8 =3x6 = 3x4 4x6 5x6 4x8 = 3x63x6 = 3x4 = 5x8 1.5x4 II 4x6 = 1.5x4 11 3x6 = 3x6 = 3x8 3×4 = 1.5x4 || 3x6 = NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 22-0-10 9-4-14 27-10-8 27-9-0 21-7-8 13-10-8 21-2-6 8-6-0 0-10-14 4-5-10 7-3-14 0-5-2 5-8-6 0-1-8 1-10-8 7-7-2 0-5-2 0-10-14 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [8:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4] Edge], [32:0-1-8, Edge], [37:0-2-8, Edge], [42:0-1-8, Edge], [43:0-1-8, Edge], [45:0-1-8, Edge], [47:0-3-0, Edge], [48:0-1-8, Edge], [50: Edge, 0-1-8] **PLATES** GRIP CSI DEFL in (loc) 1/defl L/d LOADING (psf) SPACING 2-0-0 244/190 -0.27 44-45 >616 MT20 360 Plates Increase 1.00 TC 1.00 Vert(LL) TCLL >424 240 TCDL Lumber Increase 1.00 BC 0.71 Vert(TL) -0.39 44-45 NO WB 0.77 Horz(TL) 0.03 39 n/a n/a 0.0 Rep Stress Incr BCLL Weight: 161 lb (Matrix) Code FBC2004/TPI2002 BCDL BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD TOP CHORD 4 X 2 SYP No.2 *Except* 13-23 4 X 2 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 4 X 2 SYP No.2 *Except* **BOT CHORD** 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 REACTIONS (lb/size) 50=2156/0-6-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (lb) - Maximum Compression/Maximum Tension 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0 TOP CHORD 6.7 = -3375/274, 7-8 = -3375/274, 8-9 = -2177/949, 9-10 = -142/1901, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 11-12 = 0/3301, 12-13 = -231/1277, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/1901, 10-11 = -142/19013-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, **BOT CHORD** 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 36-37=-1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30=-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 A STRZY 2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, WEBS 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0, 7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44, 18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163 1) Unbalanced floor live loads have been considered for this design. Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face 3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. CAUTION, Do not erect truss backwards. 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554 lb down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not hruss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the during construction is the responsibility of the discount of the control structure is the responsibility of the discount of the designer of the control structure is the responsibility of the discount of the design of the control structure is the responsibility of the structure. For general guidance regarding discount of the control structure is the responsibility of the structure. For general guidance regarding the regarding discount of the control structure is the responsibility of the structure. For general guidance regarding the responsibility of the structure is the responsibility of the structure. For general guidance regarding the responsibility of the structure is the responsibility of the structure is the responsibility of the structure. For general guidance regarding the responsibility of the structure is the responsibility of



Job	Truss	Truss Type	Qty	Ply	SUNBURY FL	E4617771
ELEV_F	FA	FLOOR	6	1	SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

Maronda Homes, Inc. Pittsburgh, PA

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LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf) Vert: 24-50=-10, 1-23=-100 Concentrated Loads (lb) Vert: 23=-1554(F) 1=-1554(F)



Qty SUNBURY FL Truss Truss Type Job E4617772 SUNBURY FL ALL ELEVATIONS Job Reference (optional) 10 FLOOR ELEV_F 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:16 2008 Page 1 Maronda Homes, Inc. Pittsburgh, PA 0-10-4 9-7-8 9-7-89-1-8 Scale = 1:52.2 0-9-14| 17-0-14 H 9-10-8 1-0-0 9-10-21 1-9-12 9-11-19 NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 4x6 = 1.5x4 II 3x6 FP= 1.5x4 II 15×4 II 3x6 = 3x8 = 4x6 = 3x4 II 4x8 = 3x6 = 3x6 = = 3x6 4x8 = 3x8= 3x4 = 3x4 = 4x6 1.5x4 II 3x4 22 19 20 23 18 9 10 12 13 15 17 5 2 27 26 25 33 32 3130 29 39 37 46 45 44 43 42 40 38 50 4x6 = 4x8 = 3x6 1.5x4 II 3x6 FP = 3x6 = 3x4 = 4x6 || 5x6 = 5x6 1.5x4 || 1.5x4 || 3x6 = 3x6 || 3x6 = 11 5x6 3x6 = 3×4 = 1.5x4 29-9-0 27-9-0 27-10-8 13-10-8 21-2-5 21-7-8 7-7-2 0-1-81-10-8 7-3-14 0-5-2 5-8-6 4-5-10 0-10-14 7.7.2 0-10-14 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [8:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4] Edge], [32:0-1-8, Edge], [37:0-2-8, Edge], [42:0-1-8, Edge], [43:0-1-8, Edge], [45:0-1-8, Edge], [47:0-3-0, Edge], [48:0-1-8, Edge], [50: Edge, 0-1-8] PLATES DEFL in (loc) I/defl L/d CSI 2-0-0 SPACING LOADING (psf) 244/190 -0.27 44-45 360 >616 1.00 Vert(LL) 1.00 TC TCLL 40.0 Plates Increase -0.39 44-45 >424 240 1.00 BC 0.71 Vert(TL) Lumber Increase TCDL 10.0 0.77 Horz(TL) 0.03 39 n/a n/a NO WB Rep Stress Incr BCLL 00 Weight: 161 lb Code FBC2004/TPI2002 (Matrix) **BCDL** 50 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD TOP CHORD 4 X 2 SYP No.2 *Except* 13-23 4 X 2 SYP No.1D Rigid ceiling directly applied or 6-0-0 oc bracing, Except: **BOT CHORD** 4 X 2 SYP No.2 *Except* **BOT CHORD** 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 REACTIONS (lb/size) 50=2156/0-3-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (Ib) - Maximum Compression/Maximum Tension 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0, 6-7=-3375/274, 7-8=-3375/274, 8-9=-2177/949, 9-10=-142/1901, 10-11=-142/1901, 11-12=0/3301, 12-13=-231/1277, 13-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, TOP CHORD 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, **BOT CHORD** 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 42-43=-949/2177, 41-42--12/21225, 40-41--12/21225, 39-40--330170, 30-39--19120, 37-30--1912/0, 36-37--1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, 3-48=-103/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0, 3-48=-103/0 No 4314 WEBS 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0, 7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44, 18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163 NOTES Unbalanced floor live loads have been considered for this design. Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face 3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. 4) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

Continued on page 2



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown as for lateral support of individual web members only. Additional temporary bracing to insure stobility during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the distinction, quality control storage, delivery, erection and bracing, consult.

ANSI/TPI1 Quality Criteria, DS8-89 and BCS11 Building Component Salety Information.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554

Ib down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



Job	Truss	Truss Type	Oty	Ply	SUNBURYFL	E4617772
ELEV_F	FB	FLOOR	10	1	SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

Maronda Homes, Inc. Pittsburgh, PA

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LOAD CASE(S) Standard
1) Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 24-50=-10, 1-23=-100 Concentrated Loads (lb) Vert: 23=-1554(F) 1=-1554(F)

Job	Truss	Truss Type	Oty	Ply	SUNBURY FL	E4617773
ELEV_F	FC	FLOOR	1	1	SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

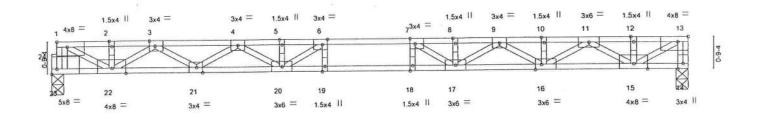
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H 1-0-0 1

0-11-2 | 2-0-0 | 0-11-2 |

Scale = 1:26.3



4	6-8-2		-8-2	8-8-2			15-4	-4	
	6-8-2		1-0-0	1-0-0			6-8-	2	
Plate Offsets (X.Y):	[1:Edge,0-1-8], [3:0-1-12,Edge], [6:0-1-	8.Edge], [7:0-1-8.Edge	e]. [13:0	-3-0.Edge].	[16:0-1-12,Edg	e]. [21:0	-1-12.Edge].	[23:Edge,0-1-8]	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates increase 1.00 Lumber increase 1.00 Rep Stress incr NO Code FBC2004/TPI2002	CSI TC 0.61 BC 0.97 WB 0.64 (Matrix)		DEFL Verl(LL) Verl(TL) Horz(TL)	in (loc) -0.37 18-19 -0.58 18-19 0.07 14	I/defl >496 >313 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 74 lb	GRIP 244/190

LUMBER

TOP CHORD 4 X 2 SYP No.2

BOT CHORD 4 X 2 SYP No.1D

4 X 2 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-7-12 oc purlins, except

end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (Ib/size) 23=1239/0-3-8, 14=834/0-3-8 Max Grav 23=2248(LC 2), 14=834(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

23-24=-2276/0, 1-24=-2272/0, 13-14=-823/0, 1-2=-1606/0, 2-3=-1606/0, 3-4=-3362/0, 4-5=-4469/0, 5-6=-4469/0, 6-7=-4793/0, 7-8=-4450/0, 8-9=-4450/0, 9-10=-3317/0, 10-11=-3317/0, 11-12=-1412/0, 12-13=-1412/0 TOP CHORD

22-23=0/433, 21-22=0/2645, 20-21=0/4045, 19-20=0/4793, 18-19=0/4793, 17-18=0/4793, 16-17=0/3986, 15-16=0/2455, **BOT CHORD**

14-15=0/0

6-19=-126/139, 7-18=-107/157, 1-22=0/1558, 2-22=-99/0, 3-22=-1216/0, 3-21=0/854, 4-21=-813/0, 4-20=0/495,

5-20=-131/103, 13-15=0/1607, 12-15=-121/0, 11-15=-1220/0, 11-16=0/1010, 10-16=-101/0, 9-16=-782/0, 9-17=0/543, 8-17=-144/113, 7-17=-835/65, 6-20=-768/125

WEBS

1) Unbalanced floor live loads have been considered for this design.

- 2) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 3) CAUTION, Do not erect truss backwards.
- 4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

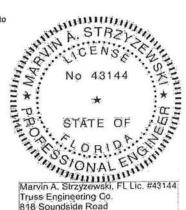
1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 14-23=-10, 1-13=-100

Concentrated Loads (lb)

Vert: 1=-424(F)

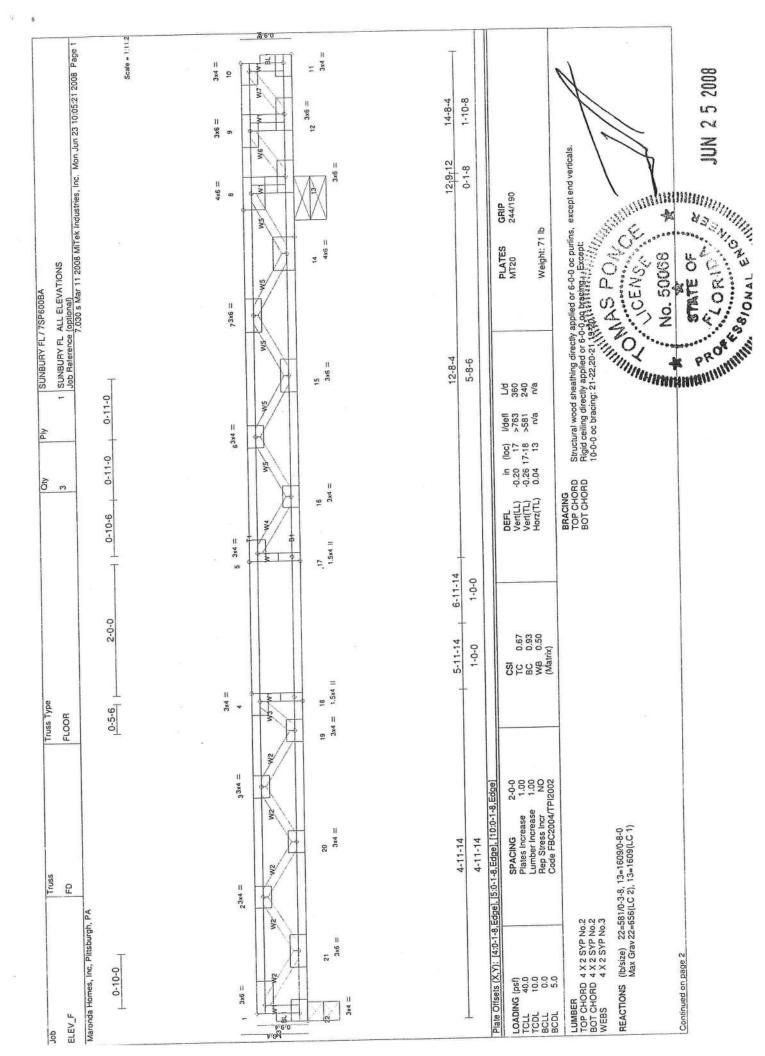


Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and reper incorporation of component is responsibility of building designer - not luss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the distinction, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotria Drive, Madison, WI 53719.





11033	and a second		
	FLOOR	e	1 SUNBURY FL ALL ELEVATIONS The Palerence (options)

12

Maronda Homes, Inc., Pittsburgh, PA

FORCES (Ib) Maximum Compression/Maximum Tension
TOP CHORD 22-23=650/0, 1-23=648/0, 11-24=0/7, 10-24=0/7, 10-2=-863/0, 2-3=-2064/0, 3-4=-2809/0, 4-5=-3010/43, 5-6=-2752/385, 6-7=-1907/892, 7-8=-482/1546, 8-9=0/1941, 9-10=0/873
TOP CHORD 22-23=650/0, 1-23=648/0, 11-124=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10-24=0/7, 10

4-19=-534/316

1) Unbalanced floor live loads have been considered for this design.
2) Attach ribbon block to truss with 3-10d nails applied to flat face.
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and lastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

4 CAUTION. Do not erect truss backwards.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 615 lb down at 14-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
6) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (pl) Vert: 11-22=-10, 1-10=-100 Concentrated Loads (lb) Vert: 10=-615(F)

NO SON A CENSON A CONTRACTION OF THE PROPERTY OF THE PROPERTY

SUNBURY FL Truss Type Job Truss SUNBURY FL ALL ELEVATIONS FLOOR ELEV_F Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:18 2008 Page 1

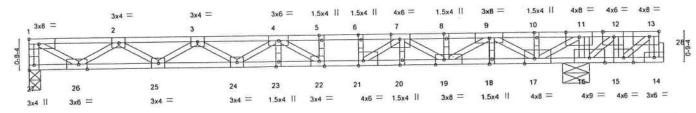
Maronda Homes, Inc. Pittsburgh, PA

1-0-0

0-11-6 0-10-4 0-11-6

0-7-0 0-8-0 0-1-8

Scale = 1:27.5



NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION.

16-0-0 14-0-0 14:1:8 7-10-8 7-5-6 1-10-8 0-1-8 5-8-6 7-5-6 0-5-2 0-5-2

LOADING (psf) TCLL 40.0 TCDL 10.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.	CSI TC 0.95 BC 0.81	DEFL in (loc) Vert(LL) -0.25 22-23 Vert(TL) -0.32 22-23 Horz(TL) 0.04 16	I/defl >658	L/d 360 240 n/a	PLATES GRIP MT20 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr NO Code FBC2004/TPI2002	1000 000 000	H012(12) 0.04 10	1110		Weight: 82 lb

LUMBER

TOP CHORD 4 X 2 SYP No.1D BOT CHORD 4 X 2 SYP No.1D

4 X 2 SYP No.3 WEBS

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 26-27.

REACTIONS (lb/size) 27=562/0-3-8, 16=2658/0-8-0

Max Grav27=708(LC 2), 16=2658(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-27=-699/0, 14-28=0/6, 13-28=0/6, 1-2=-1062/24, 2-3=-2548/191, 3-4=-3309/553, 4-5=-3329/1301, 5-6=-3329/1301, 6-7=-3329/1301, 7-8=-2158/2382, 8-9=-2158/2382, 9-10=-200/3578, 10-11=-200/3578, 11-12=0/4198, 12-13=0/2181

26-27=-0/0, 25-26=-71/1984, 24-25=-347/3079, 23-24=-774/3540, 22-23=-774/3540, 21-22=-1301/3329, 20-21=-1929/2780, 19-20=-1929/2780, 18-19=-2943/1292, 17-18=-2943/1292, 16-17=-4198/0, 15-16=-2181/0,

5-22=-61/204, 6-21=-354/0, 11-16=-1007/0, 1-26=-28/1225, 2-26=-1098/56, 2-25=-143/671, 3-25=-633/185, 3-24=-246/273, 4-24=-270/259, 4-23=0/135, 4-22=-952/158, 11-17=0/1722, 10-17=-101/9, 9-17=-1564/0, 9-18=0/19,

9-19=0/1293, 8-19=-130/0, 7-19=-975/0, 7-20=-117/15, 7-21=0/1281, 13-15=-2667/0, 12-15=0/1493, 12-16=-2559/0

NOTES

WEBS

BOT CHORD

1) Unbalanced floor live loads have been considered for this design.

Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1500 lb down at 15-9-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 14-27=-10, 1-13=-100 Concentrated Loads (lb) Vert: 13=-1500(F)



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and resper incorporation of component is responsibility of building designer - not loss designers. Bracing shown Applicability of design parameters and proper incorporation of component is responsibility of building designer - not loss designers. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the subditing designer, for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the validing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the subditing designer. For general guidance regarding erector, quality control, storage, delivery, erection and bracing, consult and subditional and additional structure. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

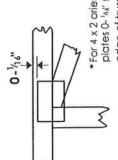


Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss Center plate on joint unless x, y and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/18" from outside edge of truss.

required direction of slots in *This symbol indicates the connector plates. * Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

width measured perpendicular to slots. Second dimension is The first dimension is the plate the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

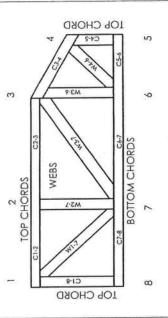
Plate Connected Wood Truss Construction. National Design Specification for Metal ANSI/TPI1:

DSB-89: BCSI1:

Design Standard for Bracing. Building Component Safety Information. Guide to Good Practice for Handling. Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B 9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A

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MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

1.8

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

3

- designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building
- Cut members to bear tightly against each other.
- joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1, Place plates on each face of truss at each
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum platting requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design
- Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted.
 - 15. Connections not shown are the responsibility of others
- Do not cut or after truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all partions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



<u>ქ</u> EAST, COLUMBIA COUNTY, TOWNSHIP **FLORIDA** RANGE

> 11

0

30

60

90

30

LEGEND:

TIMBER RID RIDGE DRIVE

EDGE OF PAVEMENT-

1) MARONDA HOMES

CERTIFIED TO:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN. BENCHMARK NOTE:

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

CURVE TABLE:

C15(P) FADIUS 500.00' 500.00' TANGENT 88.30' 88.30'

FO1 12

21.0

3.8° 7.0° 3.9°

58.571 W 12.84-12 N

48.7

FORMBOARDS FOR CONC FOUNDATION TOP OF FORMBOARD ELEVATION=102.03'

01'57'28' E 158.66' 01'57'28' E 156.66' BEARING BASIS

LOT 17

LENGTH 174.62' 174.79'

CHORD 173.73' 173.90'

CHORD BEARING N 68"14"57" E N 68"15"39" E

DELTA 20'00'36" 20'01'47"

LIGHT POLE-

C15

9'67 50.0

BUILDING SETBACK

21.0

14.2

18.9

20.0 U.E

> Z = TELEPHONE PEDESTAL **E** = CATV RISER FOUND 4" X 4" CONC. MON. NO IDENTIFICATION O = FOUND 1/2" REBAR & CAP 0 = WOOD POWER POLE LB. 6894

ABBREVIATIONS:

A/C = ASPHALT

ASPH = ASPHALT

C = CALCULATED FROM MEASURED

CATV = CABLE TELEVISION

C/B = CONCRETE BLOCK

CLF = CHAIN LINK FENCE

CM = CONCRETE MONUMENT

CONC = CONCRETE

ELEC = ELECTRIC

ELEV = ELEVATION

FND = FOUND

FNC = FENCE

LB = LICENSED SURVEYOR BUSINESS

(M) = FIELD MEASURED

MH = MANHOLE

O.U. = OVERHEAD UTILITIES

P = PLAT

P.U.E. = PUBLIC UTILITIES

P.U.E. = PUBLIC UTILITIES

P.U.E. = TRANSFORMER

TYP = TYPICAL

TYP = TYPICAL

TRANSFORMER
TYPICAL
WATER METER
WATER VALVE

ERTIFICATE OF SURVEYOR:

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF
A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS
TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF JAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUYES, AND CHAPTER 1612—6, FLORIDA ADMINISTRATIVE CODE.

DATE MES E. BRIMKMAN, PSM - FLA. CERT# 5582 0 00/00

BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE:

1" = 30'

DATE: 10/9/2008

"THE BENCHMARK IN QUALITY SERVICE" DRAWN BY: ZL CHECKED BY: J.B.

5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP, INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

6)

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

4

3) 2)

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND

TO BE ACCEPTABLE TO THIS SURVEYOR.

FLOOD NOTE:

LOT 33

THE NOTE:

5 54:13:56 W 93.54 (M)

LOT 32

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

SURVEYOR NOTES:

10.0

 $LOT\ 16$ $\pm 0.50\ \text{ACRES}$ MINIMUM FINISHED FLOOR ELEVATION=100.00'

FIELD WORK COMPLETED ON 10/7/2008 FIELDBOOK 97, PAGE 68

DRAWING NUMBER 160-08

PREPARED FOR: MARONDA

LEGALDESCRIPTION:

LOT SIXTEEN (16) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BU NG

SECTION 10, 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

30

60

90

30

S.W. TIMBER RIDGE DRIVE

1) MARONDA HOMES CERTIFIED TO:

BENCHMARK NOTE:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

BUILDING SETBACK

PROPOSED

,0.02

20.0°

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25", REAR 15", SIDE 10"

CURVE CURVE TABLE:

C15(P) 500.00° TANGENT 88.21

174.62"

20'00'35.9" DELTA

CHORD 173.73

CHORD BEARING N 68"14'57" E

LOT 105 ACRES MFF = 100.00'

16

PROPOSED-CONC PATIO

91 107

'T.84

PROPOSED

PROPOSED SEPTIC
TANK & DRAIN FIELD

PROPOSED RESIDENCE SUNBURY-V RICHT

0.82

LOT 17

WOOD POWER POLE

TELEPHONE PEDESTAL

FLOOD NOTE:

LOT 33

15.0°

LOT 32

3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

2)

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

J

SURVEYOR NOTES:

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED

5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

TITLE BLOCK).

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

4) BUILDING SETBACK LINES DEPICTED HEREON ARE SUBJECT SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

TILE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR MAY EFFECT THIS, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

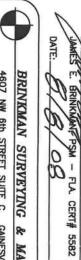
© 13 II X X **×** 11 0 0 11 LEGEND: = FOUND 1/2" REBAR & CAP LB. 6894 = SET 1/2" REBAR & CAP LB. 6894 FOUND NAIL & DISK FOUND 6" X 6" S.R.D. R/W MON. SET NAIL & DISK P.S.M. 5582 SET 4" X 4" CONC. MON. P.S.M. 5582 FOUND 3/4" IRON PIPE FOUND 4" X 4" CONC. MON. FOUND 1/2" REBAR NO IDENTIFICATION CATV RISER NO IDENTIFICATION ABBREVIATIONS: A/C = AIR CONDITIONER A/C = AIR CONDITIONER A/C = AIR CONDITIONER A/C = AIR CONDITIONER C = CALCULATED FROM MEASURED CATV = CABLE TELEVISION C/B = CONCRETE BLOCK CLF = CHAIN LINK FENCE CM = CONCRETE MONUMENT CONC ELEC ELEC FILES FIL LB = LICENSED SURVEYOR BUSINESS M) = FIELD MEASURED H = MANHOLE U. = OVERHEAD UTILITIES P = PLAT = PLAT BOOK = PUBLIC UTILITIES EASEMENT = TRANSFORMER = TYPICAL = ELECTRIC = ELEVATION = FOUND = FENCE = CONCRETE

'HIS IS NOT A BOUNDARY SURVEY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER STG1.7—6, FLORIDA ADMINISTRATIVE CODE.



BRINKMAN SURVEYING & MAPPING INC.

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SCALE: 1" = 30' PHONE: (352) 374-7707 "THE BENCHMARK IN QUALITY SERVICE" FAX: (352) 374-8757 DRAWN BY: ZL

SI SIHT NOT A BOUNDARY SURVEY

DATE: 8/4/08

PREPARED FOR: MARONDA

DRAWING NUMBER

CHECKED BY: J.B.

160-08