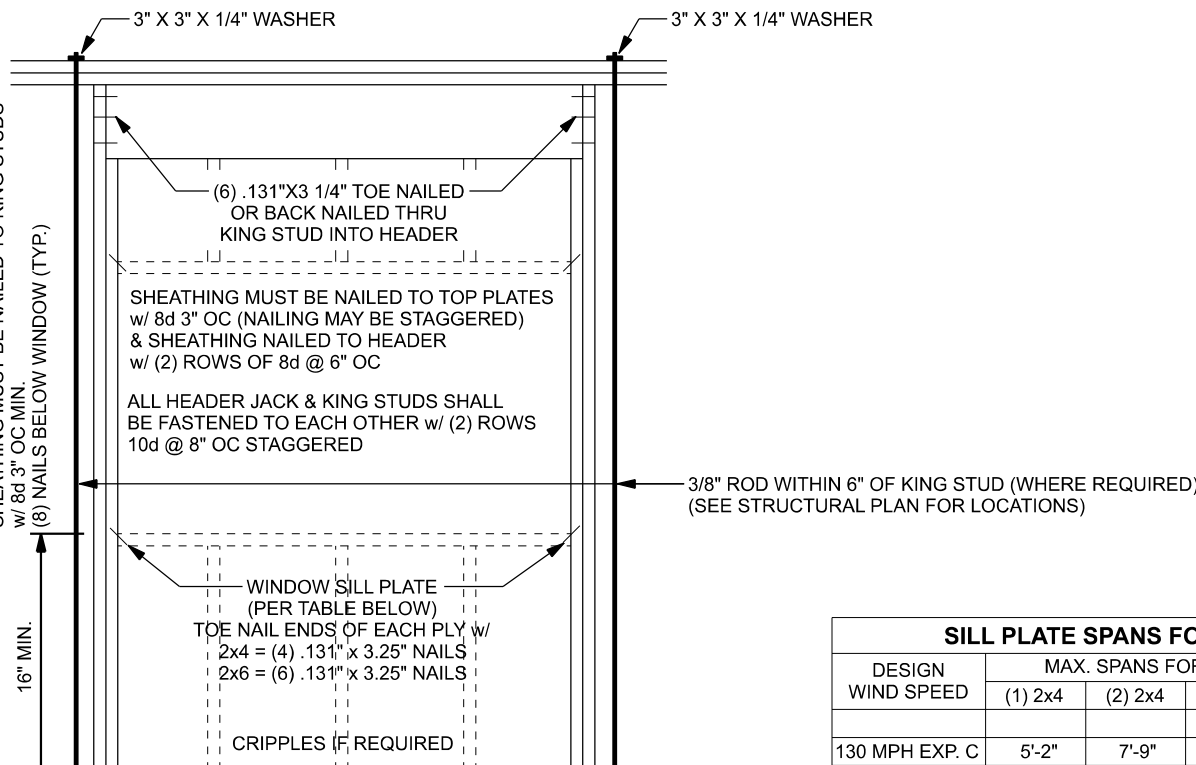


# ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

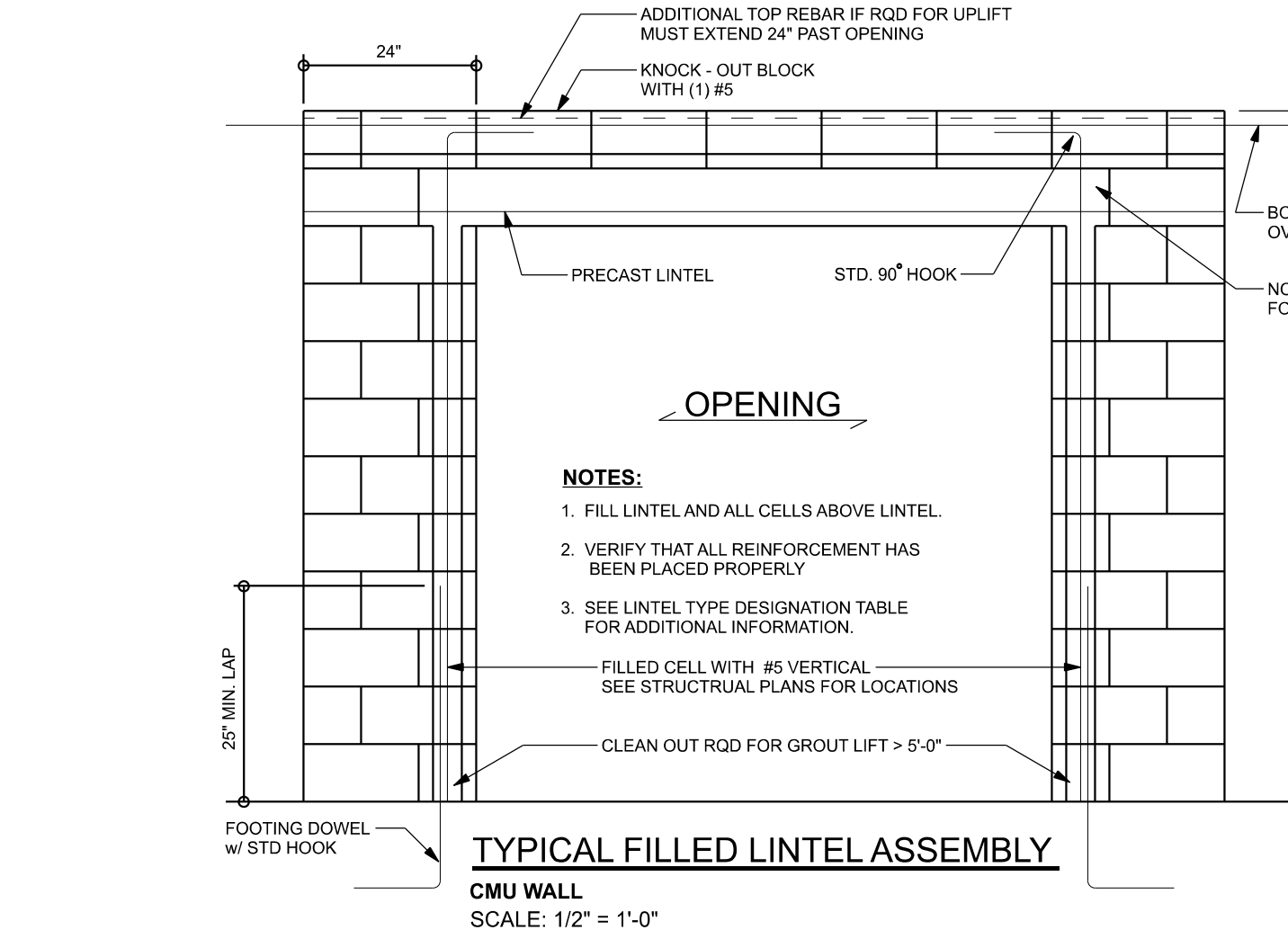
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

Note:  
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.  
Note:  
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

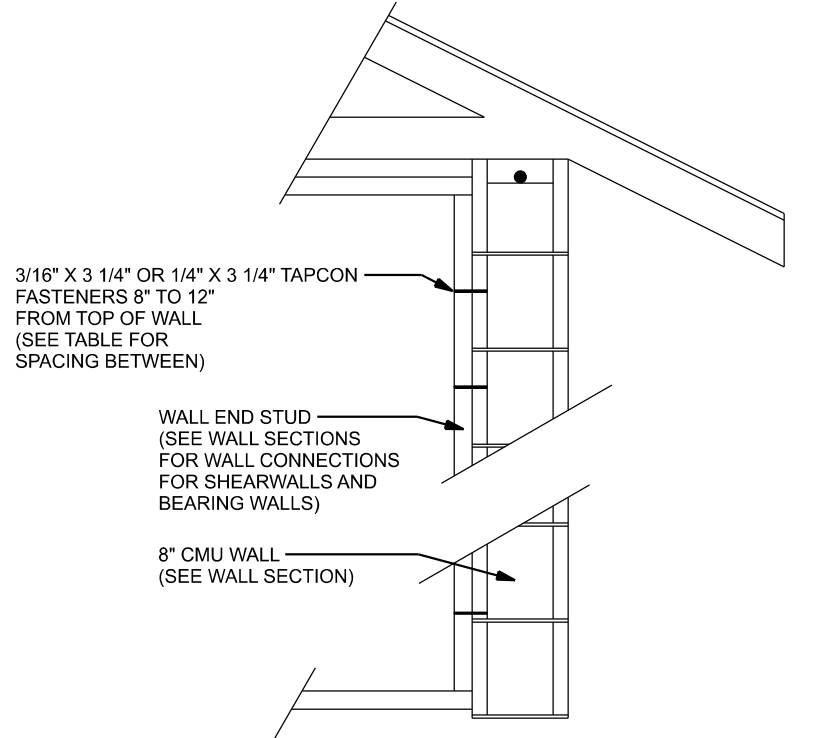


DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.23B
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-2" 7'-9" 7'-7" 11'-3"	FOR OTHER WALL HEIGHTS (H) BILL SPAN SHALL BE DIVIDED BY (H/10)

**TYPICAL HEADER STRAPPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



**TYPICAL FILLED LINTEL ASSEMBLY**  
CMU WALL  
SCALE: 1/2" = 1'-0"

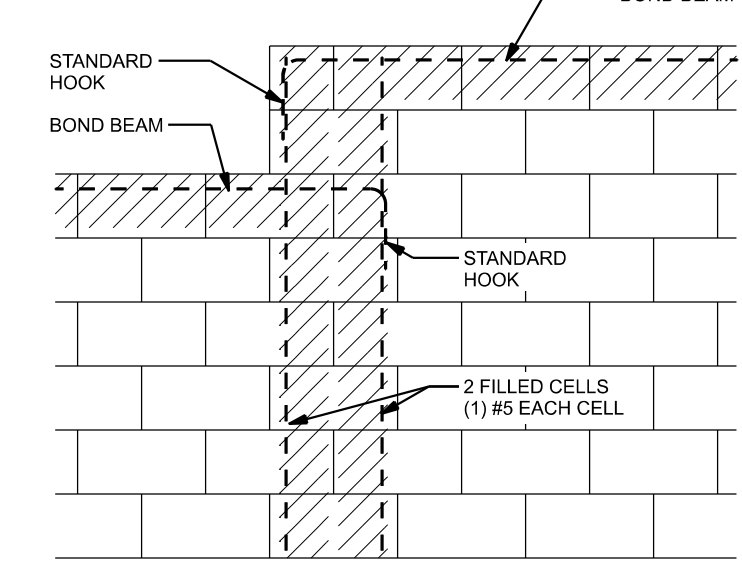


WALL TYPE:	3/16" X 3 1/4" TAPCON MAX SPACING	1/4" X 3 1/4" TAPCON MAX SPACING
INTERIOR SHEAR WALL	6" OC	8" OC
INTERIOR BEARING WALL	16" OC	16" OC
INTERIOR NON-BEARING WALL	48" OC	48" OC

**INT. FRAME WALL TO CMU CONNECTION**

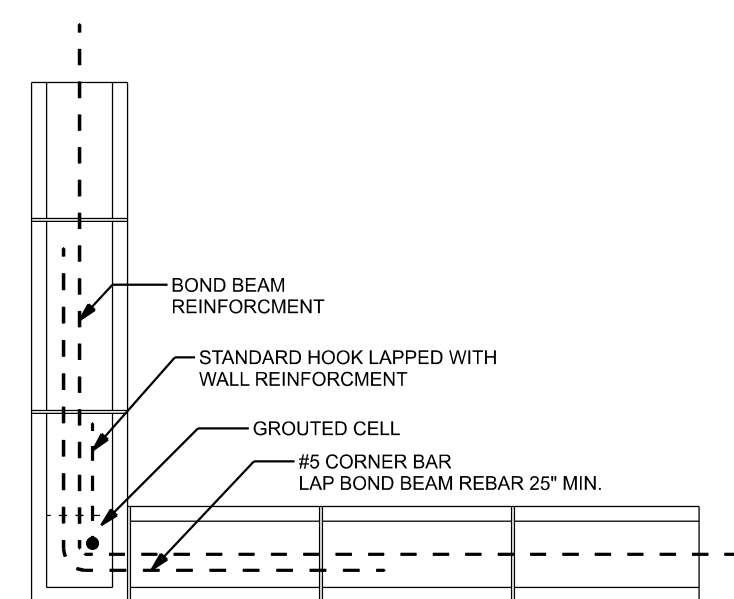
## CHANGES IN BOND BEAM HEIGHT

(BASED ON FBC FIG. R609.2.5)  
SCALE: 1/2" = 1'-0"



## CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT

(BASED ON FBC FIG. R609.2.4)  
SCALE: 3/4" = 1'-0"



**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

MIN. 1/2" ANCHOR  
WITHIN 6" EACH SIDE  
OF PLATE JOINT

Uplift SP#	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8d@1 1/2"	4-8d@1 1/2"
415	280	H3	5-8d@1 1/2"	5-8d@1 1/2"
575	495	H2.5A	9-8d@1 1/2"	9-8d@1 1/2"
1340	1015	H10A	9-10d@1 1/2"	9-10d@1 1/2"
720	620	LTS12-20	6-10d@1 1/2"	6-10d@1 1/2"
1000	860	MTS12-30	7-10d@1 1/2"	7-10d@1 1/2"
1450	1345	HTS20-30	12-10d@1 1/2"	12-10d@1 1/2"
Uplift SP#	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP#	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	805	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP#	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP#	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP#	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
2200	ABU44		12-16d	5/8"x12" Drill & Epoxy
2300	ABU66		12-16d	5/8"x12" Drill & Epoxy
Uplift SP#	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44		12-16d	5/8"x7" Drill & Epoxy
2300	ABU66		12-16d	5/8"x7" Drill & Epoxy

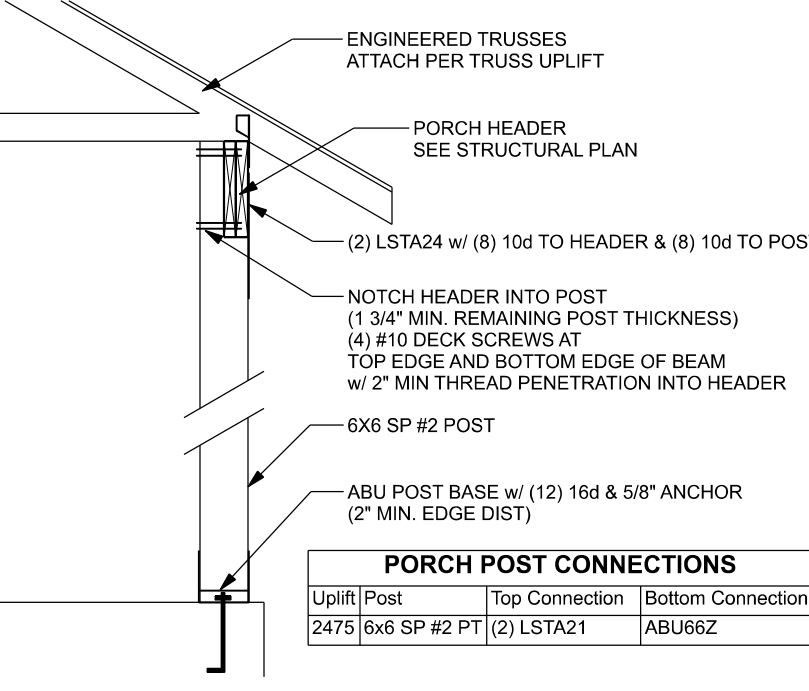
## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

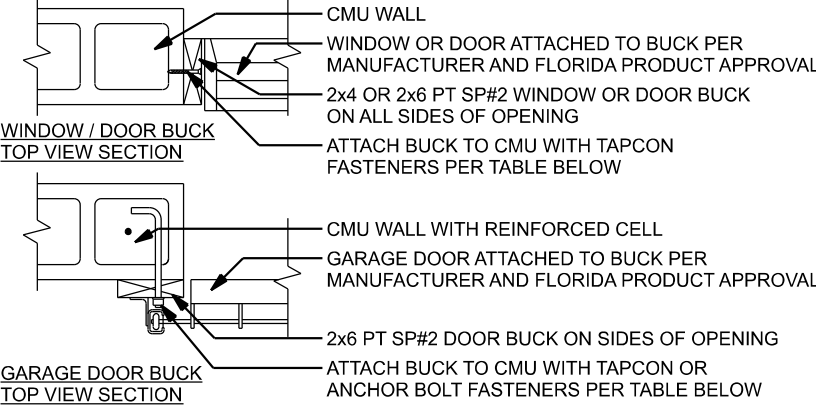
GRADE & SPECIES	TABLE	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**(TYP.) INTERIOR BEARING WALL**  
ONE STORY WOOD FRAME w/ RODS



POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:  
EAVE / HIP ROOF:  
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)  
- USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED  
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER  
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL  
- SILE END  
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1" MIN. FROM TOP & BOTTOM EDGE  
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL

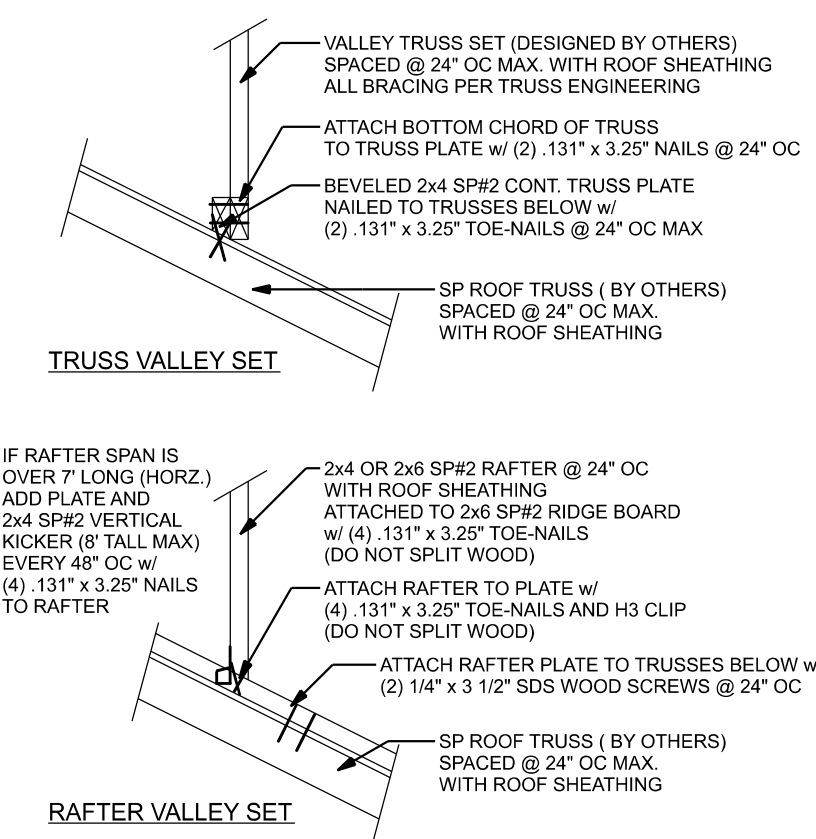
**(TYP.) PORCH POST**  
ONE STORY WOOD



OPENING SIZE / TYPE:	3/16" TAPCON MAX SPACING	1/4" TAPCON MAX SPACING	1/2" ANCHOR BOLTS INTO FILLED CELL
WINDOWS & DOORS UP TO 4' W	14" OC	22" OC	N/A
WINDOWS & DOORS UP TO 6' W	10" OC	16" OC	N/A
WINDOWS & DOORS UP TO 10' W	9" OC	14" OC	N/A
SLIDING DOORS UP TO 8' TALL	9" OC	15" OC	N/A
GARAGE DOOR UP TO 10' WIDE	9" OC	14" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED
GARAGE DOOR UP TO 18' WIDE	4" OC	7" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED

- TAPCON IN FACE OF CMU: 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT, 3" MIN. SPACING  
WINDOWS AND DOORS MAY BE ATTACHED DIRECTLY TO CMU PER MANUFACTURER AND FLORIDA PRODUCT APPROVAL.  
- A 1x PT "SPACER" BUCK MAY BE USED IF WINDOW / DOOR IS ATTACHED TO CMU PER FLORIDA PRODUCT APPROVAL.

**DOOR & WINDOW BUCK ATTACHMENT**



**VALLEY SET FRAMING DETAIL**

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE NECESSARY REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REINFORCEMENT ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN  
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)  
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, F<sub>y</sub> = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (W1.4) CONFORMING TO ASTM A195. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.  
FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 3 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.  
FIBER REINFORCED CONCRETE (FRC) SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5:1 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT CWT OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP SPACED 40" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F <sub>m</sub> = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, F <sub>y</sub> = 40 ksi, Lap splices min. 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GR6, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.  
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	130 MPH
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	II
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&D DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	40 PSF LIVE LOAD
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	20 PSF LIVE LOAD
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)	ZONE 4 INTERIOR	ZONE 5 END & FROM ALL OUTSIDE CORNER
EFFECTIVE WIND AREA (FT2)		
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
8x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)	
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)	

Blake Construction

Ronnie Shuman Res

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

MARK DISOWAY P.E. 53915

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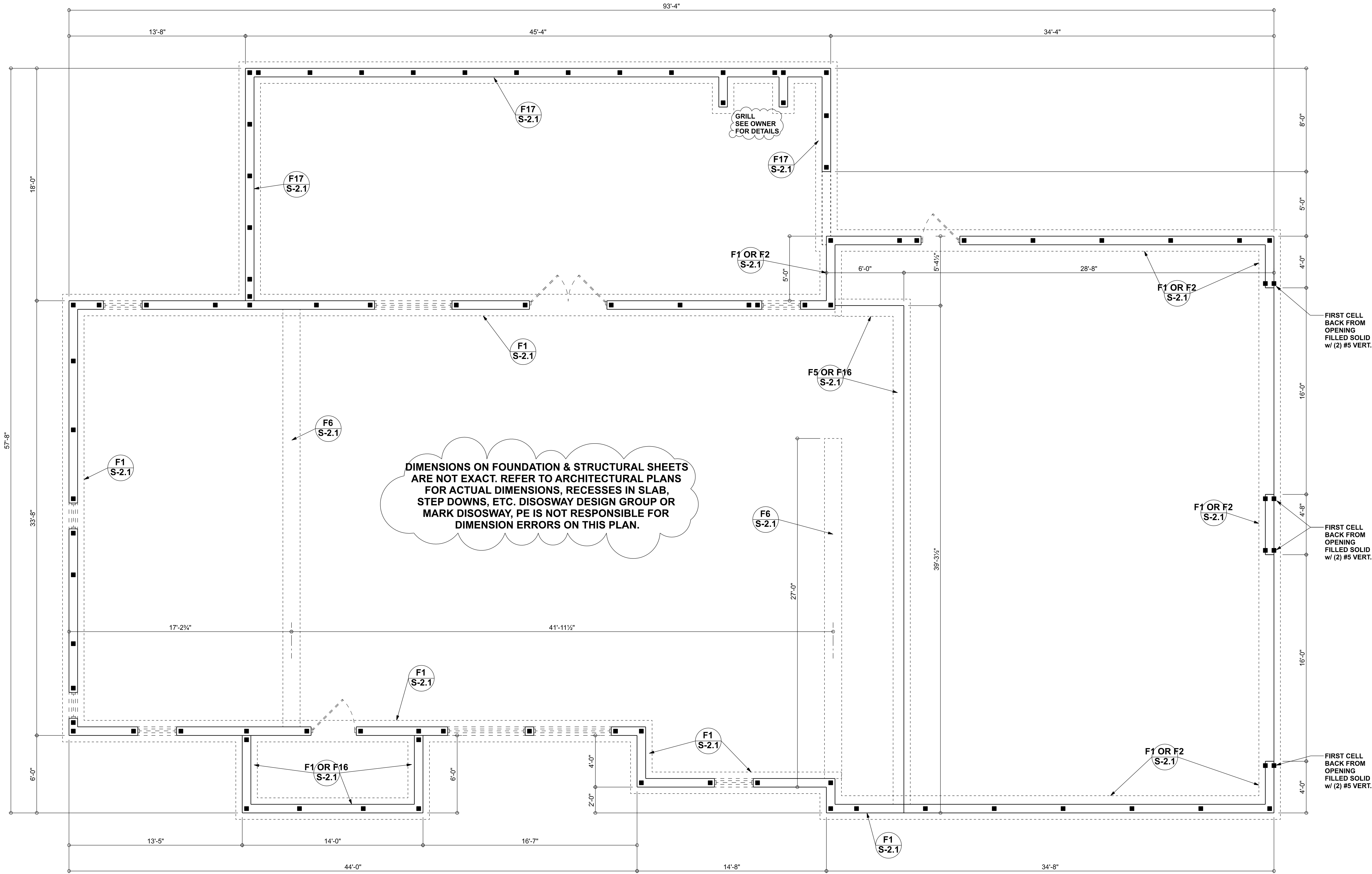
Thursday, August 26, 2021

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JOB NUMBER:  
211179

S-1  
OF 4 SHEETS





FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

Blake Construction

Ronnie Stuman Res

PROJECT ADDRESS:  
205 SW Madison Court  
Lake City, FL 32024

DIMENSIONS:  
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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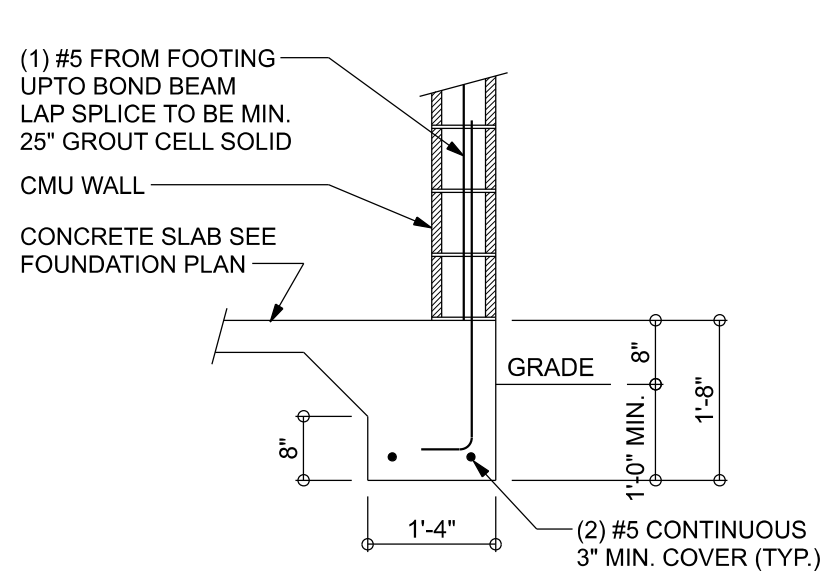
Thursday, August 26, 2021

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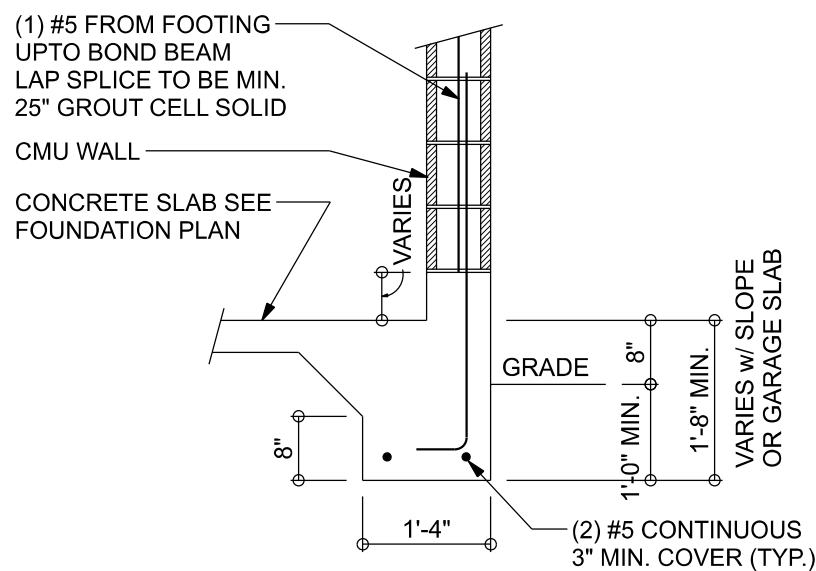
JOB NUMBER:  
211179

S-2  
OF 4 SHEETS

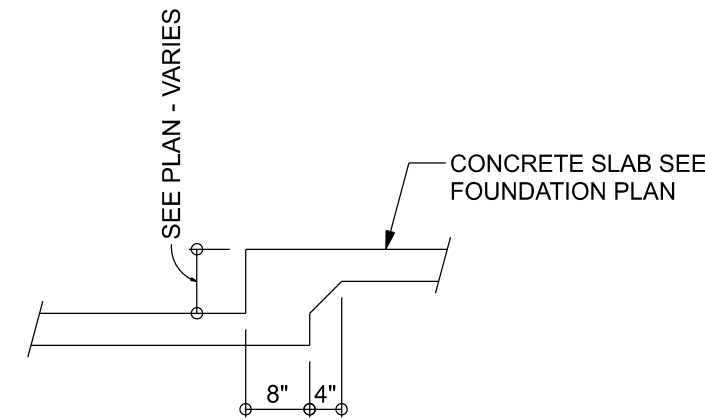




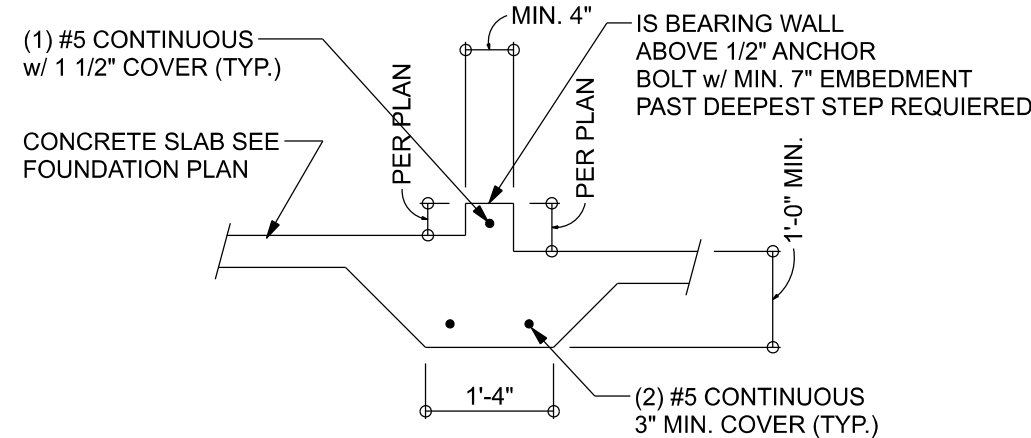
**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



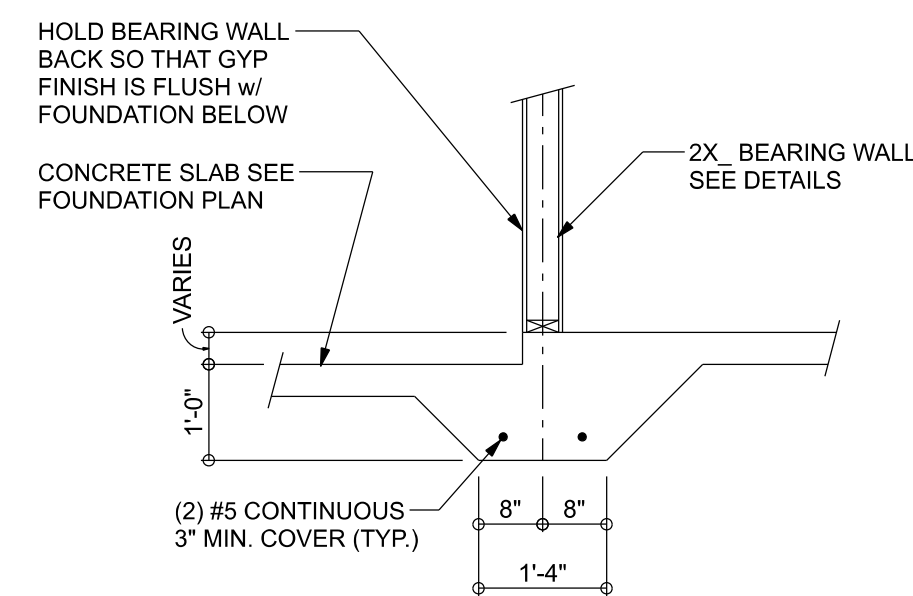
**F2 GARAGE CURB FOOTING**  
SCALE: 1/2" = 1'-0"



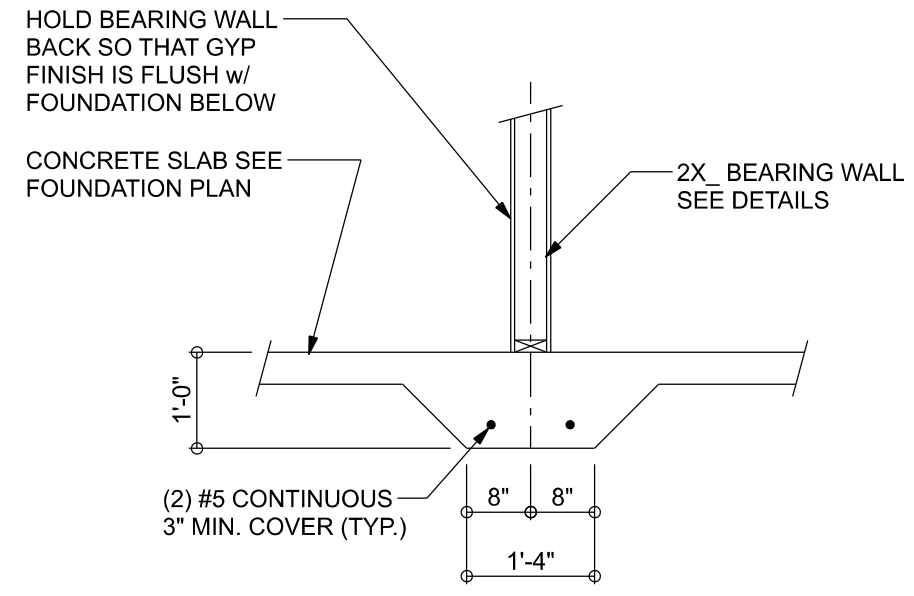
**F3 NON-BEARING STEP DOWN**  
SCALE: 1/2" = 1'-0"



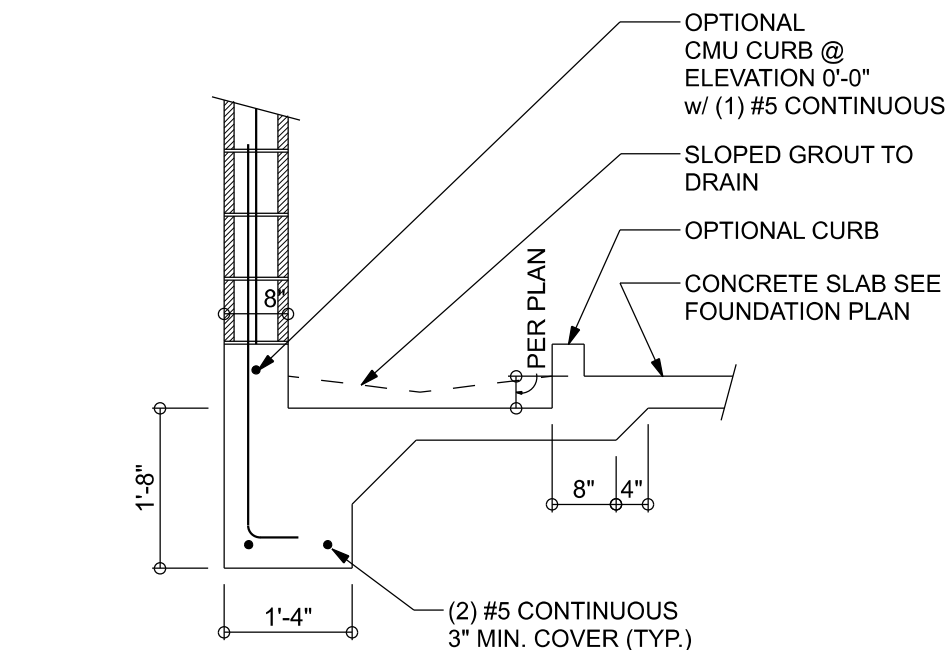
**F4 DOUBLE STEP FOOTING**  
SCALE: 1/2" = 1'-0"



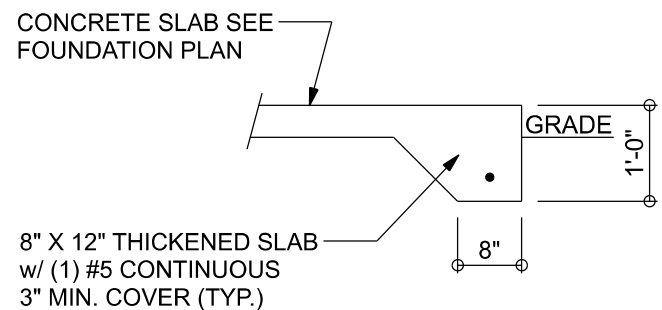
**F5 STEP FOOTING BEARING**  
SCALE: 1/2" = 1'-0"



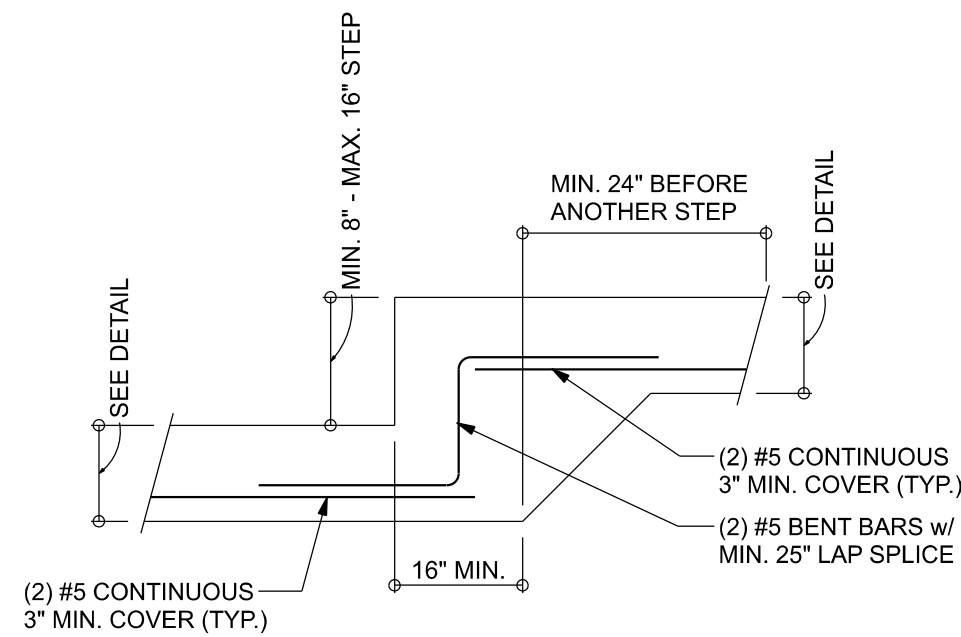
**F6 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



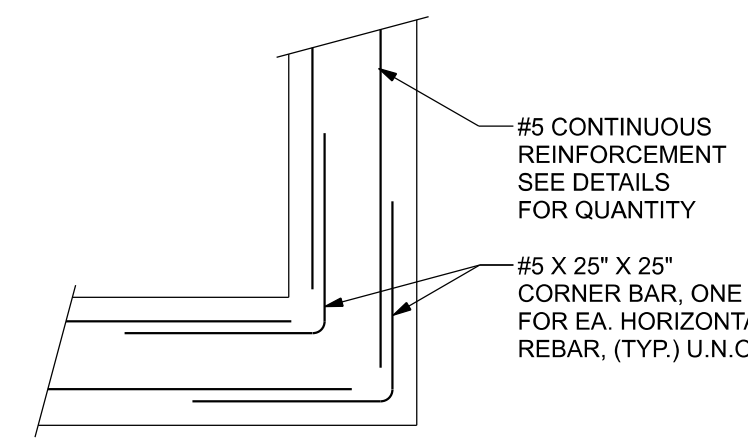
**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"



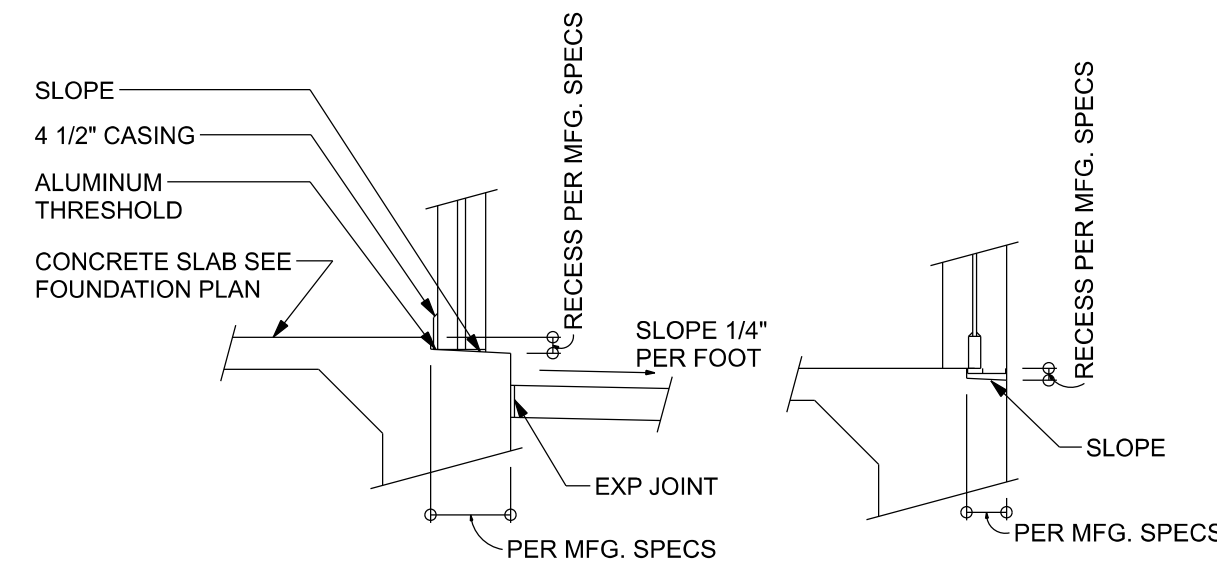
**F8 THICKEND EDGE**  
SCALE: 1/2" = 1'-0"



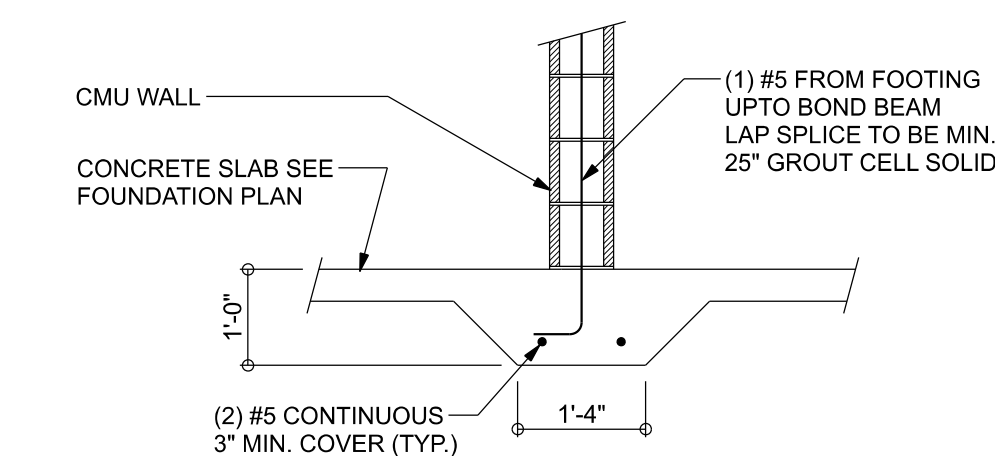
**F9 (TYP.) STEP FOOTING DETAIL**  
SCALE: 1/2" = 1'-0"



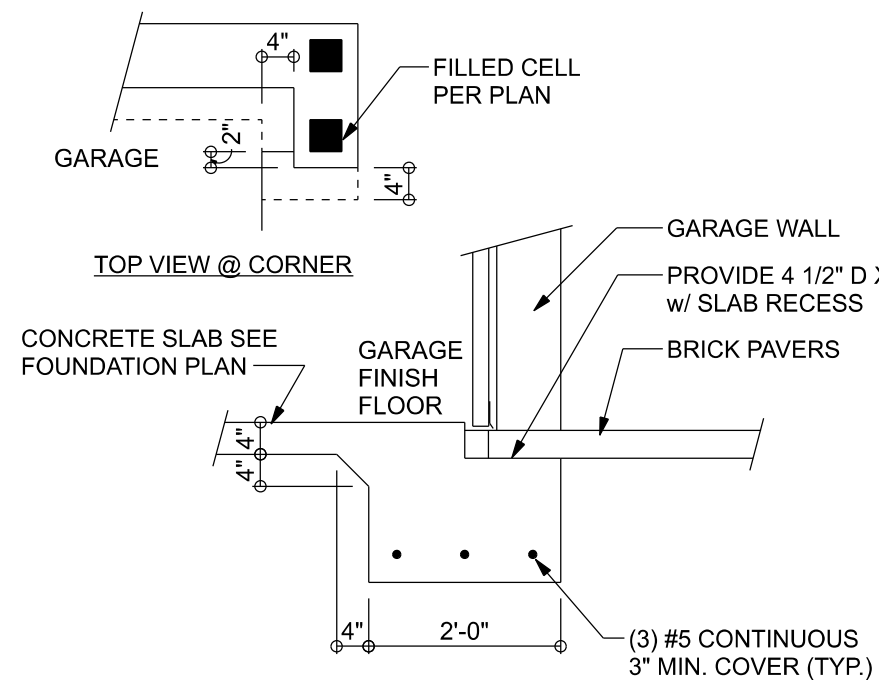
**F10 (TYP.) CORNER BAR DETAIL**  
SCALE: 1/2" = 1'-0"



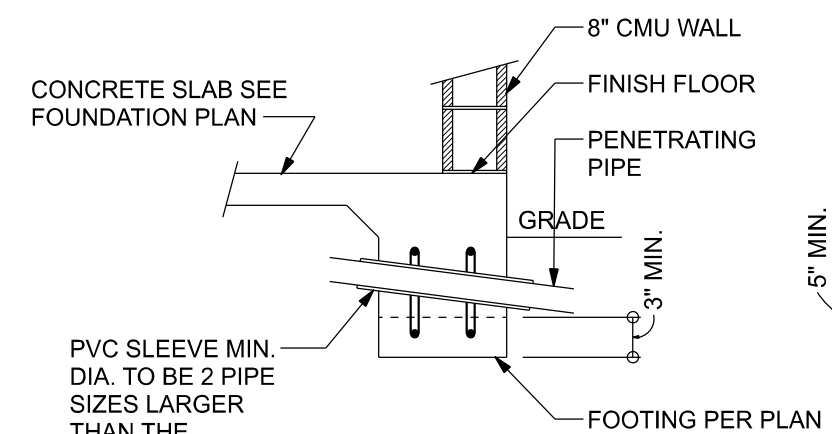
**F11 EXTERIOR DOOR POURED SILLS**  
SCALE: 1/2" = 1'-0"



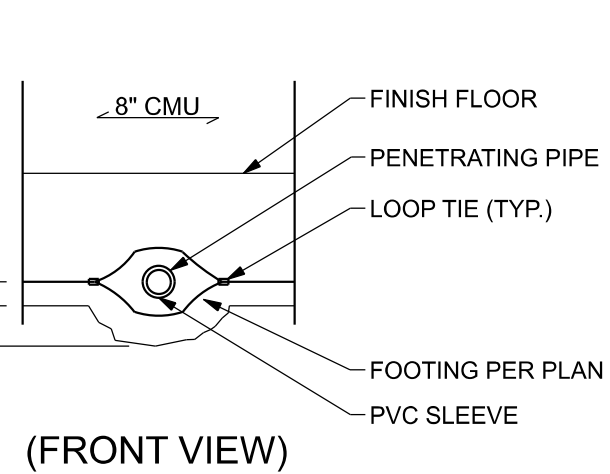
**F12 INTERIOR BEARING FOOTING w/ CMU WALL**  
SCALE: 1/2" = 1'-0"



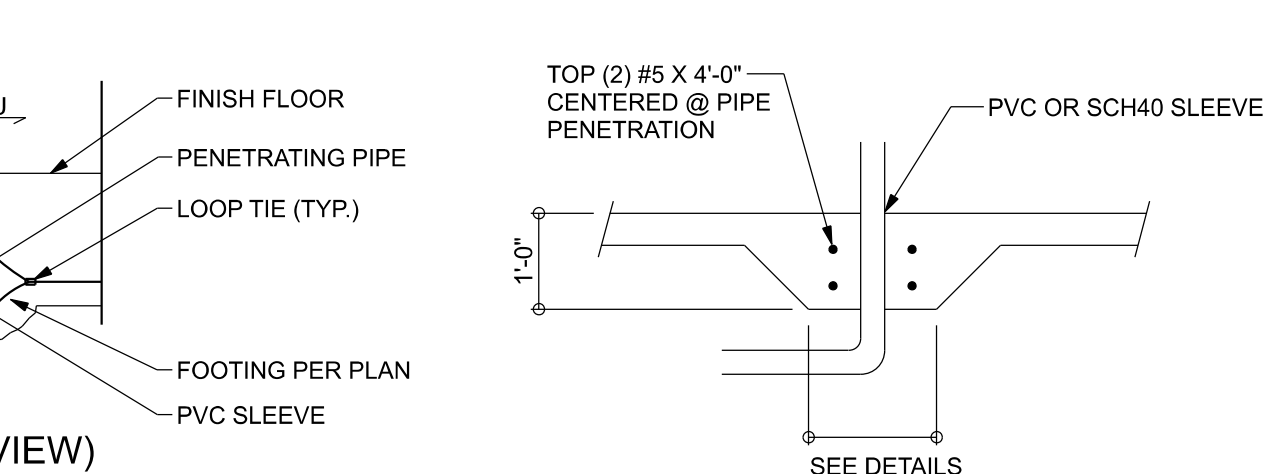
**F13 THICKENED EDGE @ GARAGE DOOR**  
SCALE: 1/2" = 1'-0"



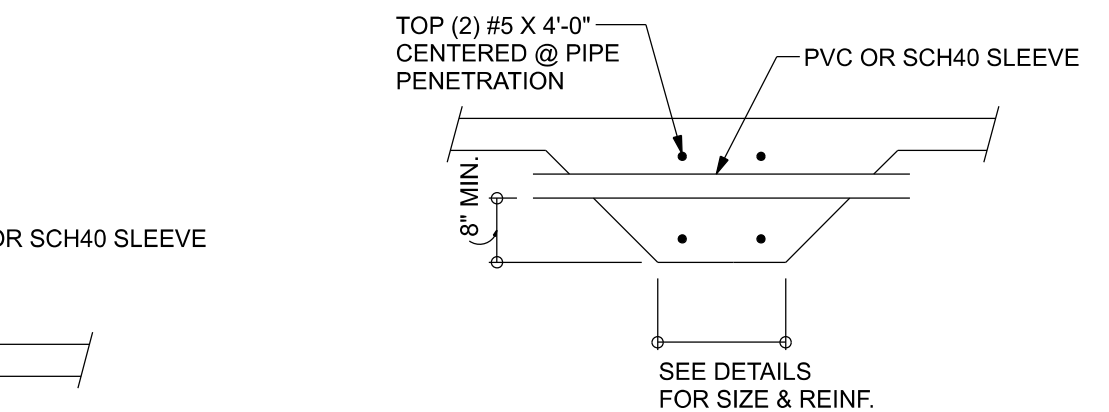
**F14 TYPICAL FOUNDATION PENETRATIONS**  
SCALE: 1/2" = 1'-0"



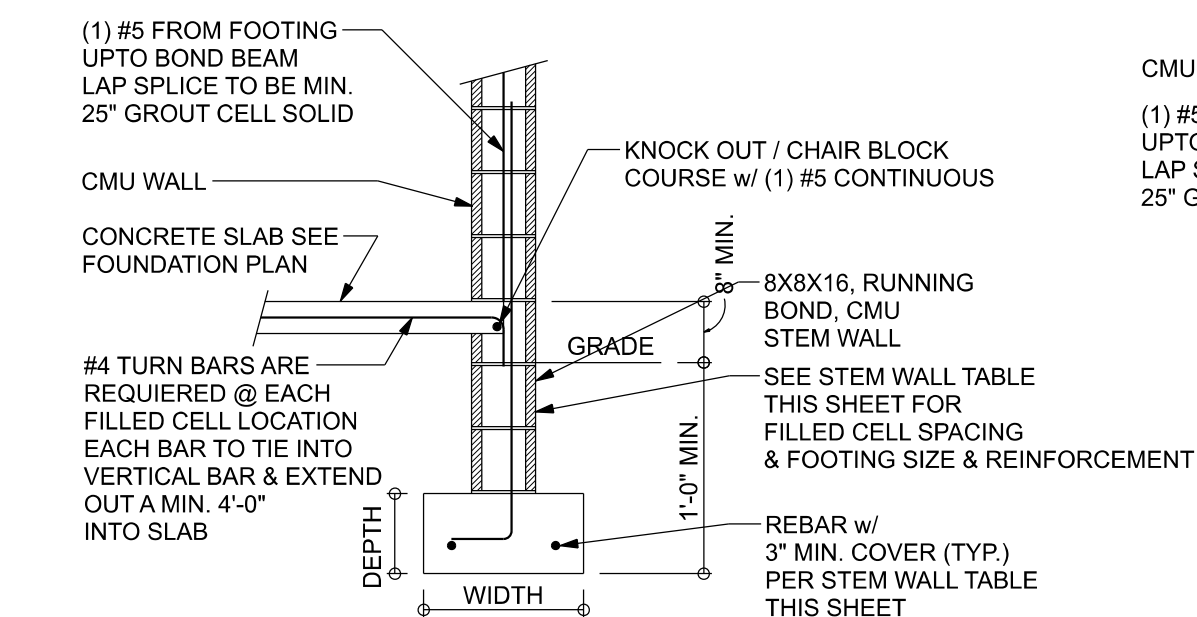
**F15 STEM WALL FOOTING @ PORCH**  
SCALE: 1/2" = 1'-0"



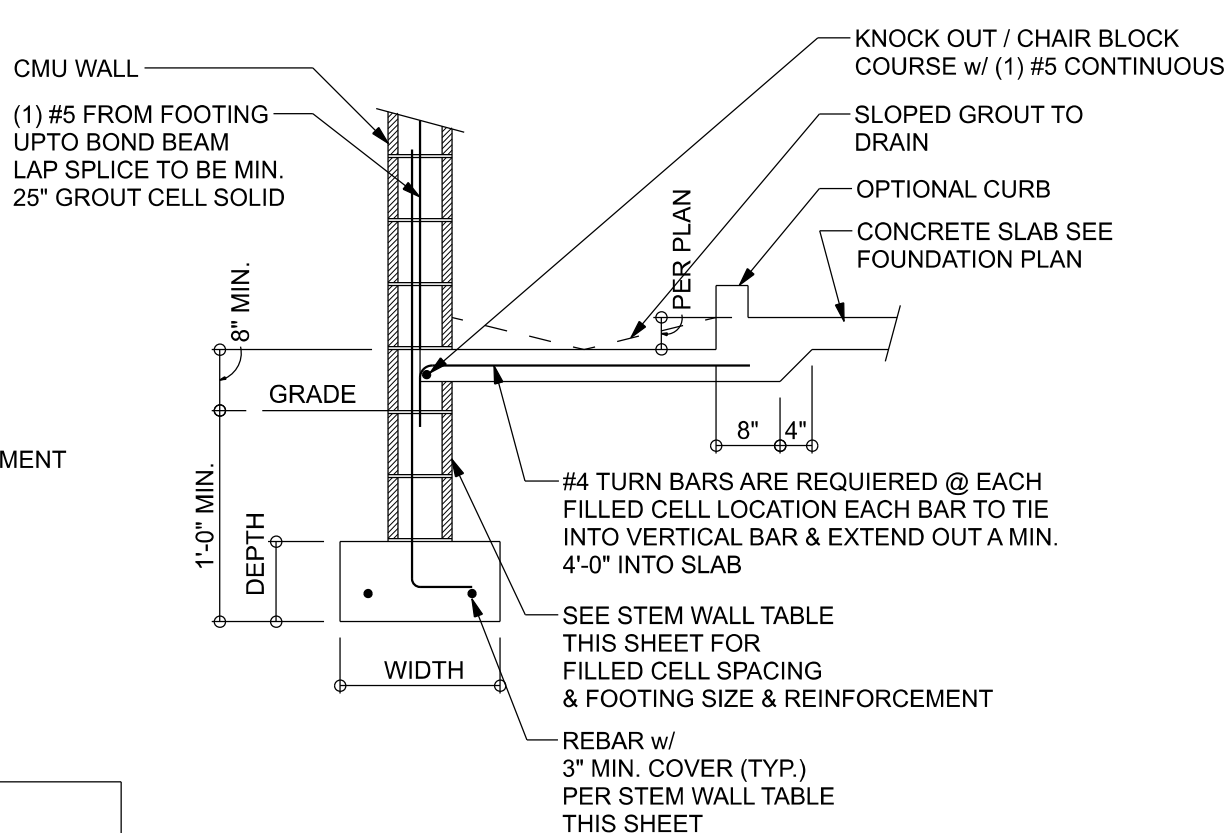
**F16 STEM WALL @ GARAGE STEP DOWN**  
SCALE: 1/2" = 1'-0"



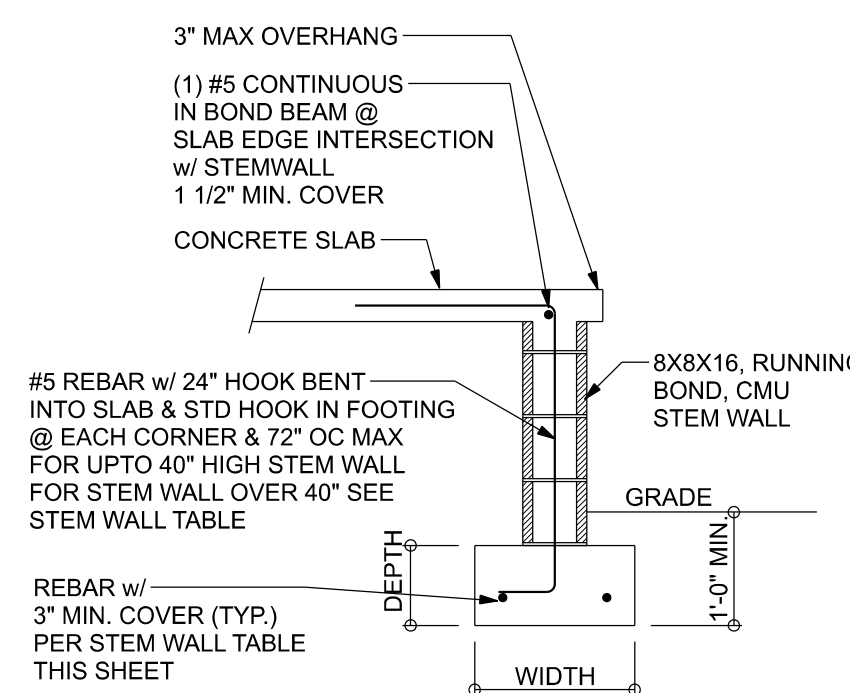
**F17 STEM WALL CURB FOOTING @ SCREENED LANAI**  
SCALE: 1/2" = 1'-0"



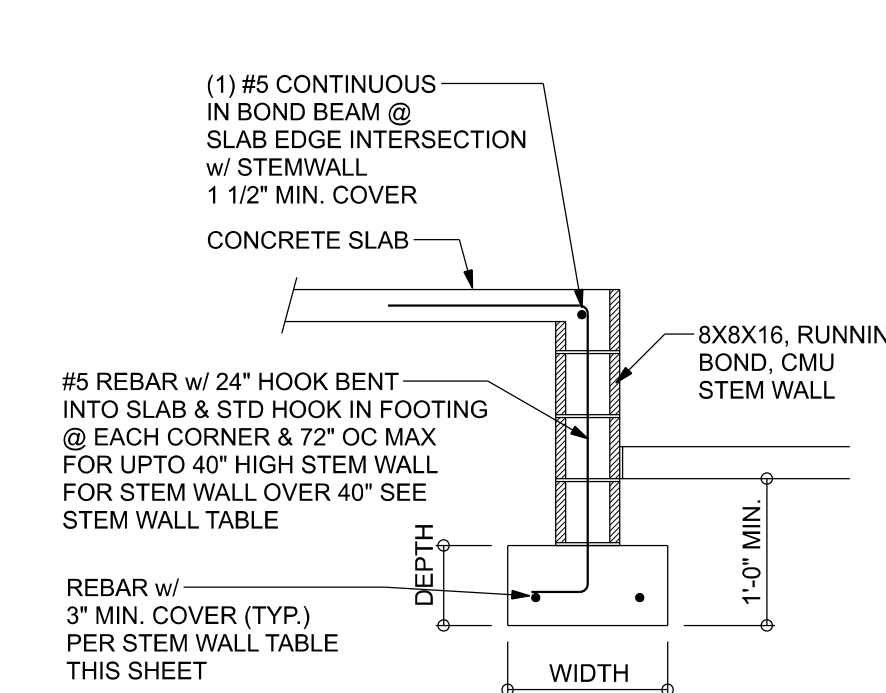
**F1 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



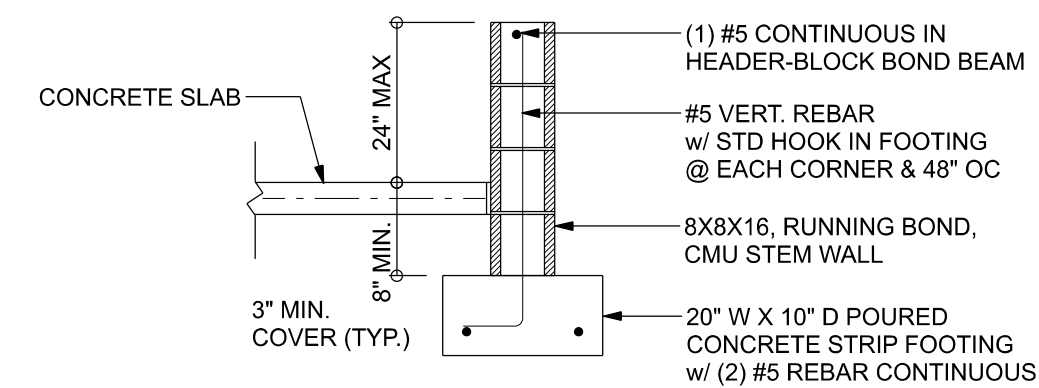
**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"



**F15 STEM WALL FOOTING @ PORCH**  
SCALE: 1/2" = 1'-0"



**F16 STEM WALL @ GARAGE STEP DOWN**  
SCALE: 1/2" = 1'-0"



**F17 STEM WALL CURB FOOTING @ SCREENED LANAI**  
SCALE: 1/2" = 1'-0"

STEM WALL TABLE						
STEMWALL HEIGHT	FOOTING DIMENSION				NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	1-STORY		2-STORY			
	DEPTH	WIDTH	DEPTH	WIDTH		
8" - 40"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"

NOTE: ALL STEM WALL FOUNDATIONS OVER 3'-0" IN HEIGHT TO BE POURED SOLID

## OPTIONAL STEM WALL FOUNDATION

Blake Construction

Ronnie Shuman Res

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205 SW Madison Court  
Lake City, FL 32024

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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Thursday, August 26, 2021

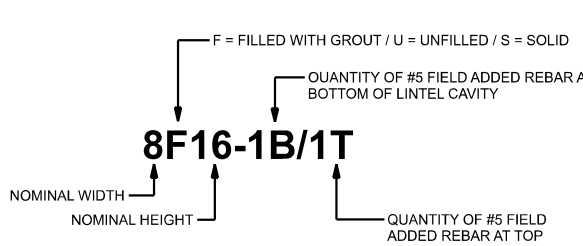
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JOB NUMBER:  
211179

**S-2.1**  
OF 4 SHEETS



# TYPE DESIGNATION



## MATERIALS

1. F: 8" precast lintel = 3500 psi
2. F: prestressed lintel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

## GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Show field lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical oil reinforcing and grouting.
5. All lintels meet or exceed L/500 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. C-20026-05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-208 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:  
Applied vertical load + Applied horizontal load  
Safe vertical load + Safe horizontal load ≤ 1.0
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right.

## SAFE LOAD TABLE NOTES

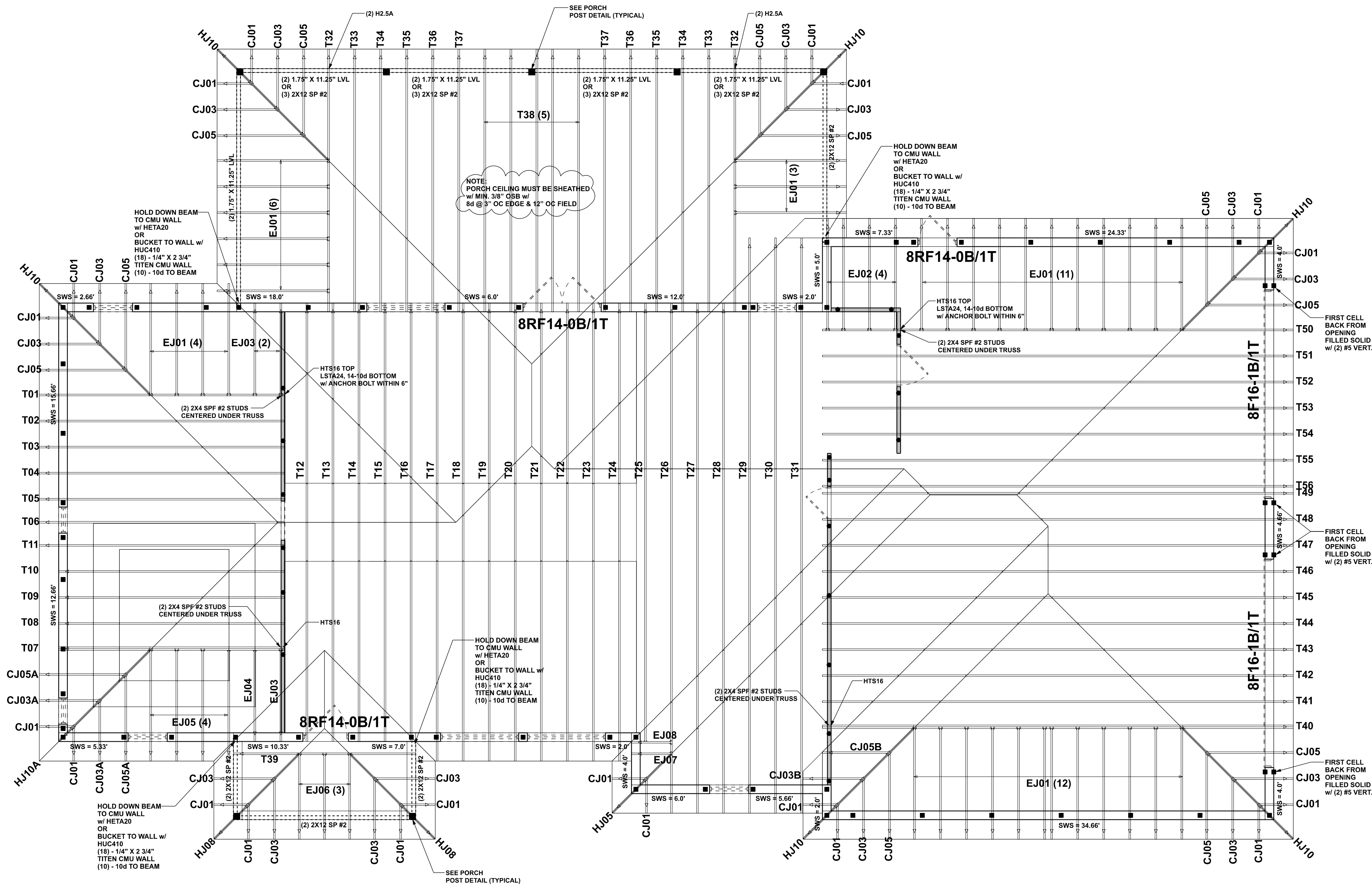
1. All values based on minimum 4 inch nominal bearing.
- Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 9" lintels only.
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at 0-way from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown.
9. All safe loads in units of pounds per linear foot.
10. All safe loads based on simply supported span.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.  
Example: 7'-6" lintel type 8F32-1B safe gravity load = 6472(0.046) (15)(0.071); w/ 15% reduction 6472 x (.85) = 5501 plf

## SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS

LENGTH	TYPE	RUB	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F9-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F36-0B	8F40-0B	8F44-0B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809			
3'-0" (42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809			
4'-0" (48")	PRECAST	1966	2893	4378	5863	7347	8832	10316	11800			
4'-6" (54")	PRECAST	1599	1969	2110	2291	2473	2654	2835	3016			
5'-0" (60")	PRECAST	1217	1363	1438	1513	1587	1662	1736	1810			
5'-10" (70")	PRECAST	1062	1163	1238	1313	1387	1462	1536	1610			
6'-0" (72")	PRECAST	908	1009	1084	1159	1234	1309	1384	1459			
6'-6" (78")	PRECAST	743	844	919	994	1069	1144	1219	1294			
7'-0" (84")	PRECAST	584	685	760	835	910	985	1060	1135			
7'-6" (90")	PRECAST	475	576	651	726	801	876	951	1026			
8'-0" (96")	PRECAST	332	433	508	583	658	733	808	883			
8'-6" (102")	PRECAST	279	380	455	530	605	680	755	830			
9'-0" (108")	PRECAST	226	327	402	477	552	627	702	777			
9'-6" (114")	PRECAST	173	274	349	424	499	574	649	724			
10'-0" (120")	PRECAST	120	221	296	371	446	521	596	671			
10'-6" (126")	PRECAST	98	199	274	349	424	499	574	649			
11'-0" (132")	PRECAST	82	183	258	333	408	483	558	633			
11'-6" (138")	PRECAST	66	167	242	317	392	467	542	617			
12'-0" (144")	PRECAST	50	151	226	301	376	451	526	601			
12'-6" (150")	PRECAST	34	135	210	285	360	435	510	585			
13'-0" (156")	PRECAST	18	119	194	269	344	419	494	569			
13'-6" (162")	PRECAST	12	103	178	253	328	403	478	553			
14'-0" (168")	PRECAST	6	87	162	237	312	387	462	537			
14'-6" (174")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
15'-0" (180")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
15'-6" (186")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
16'-0" (192")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
16'-6" (198")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
17'-0" (204")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
17'-6" (210")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
18'-0" (216")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
18'-6" (222")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
19'-0" (228")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
19'-6" (234")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
20'-0" (240")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
20'-6" (246")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
21'-0" (252")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
21'-6" (258")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
22'-0" (264")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
22'-6" (270")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
23'-0" (276")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
23'-6" (282")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
24'-0" (288")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
24'-6" (294")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			

## SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	RUB	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F9-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F36-0B	8F40-0B	8F44-0B
4'-4" (52")	PRECAST	1635	1740	1845	1950	2055	2160	2265	2370			
4'-6" (54")	PRECAST	1494	1599	1704	1809	1914	2019	2124	2229			
5'-0" (60")	PRECAST	866	971	1076	1181	1286	1391	1496	1601			
5'-10" (70")	PRECAST	810	915	1020	1125	1230	1335	1440	1545			
6'-0" (72")	PRECAST	797	902	1007	1112	1217	1322	1427	1532			
7'-0" (84")	PRECAST	669	774	879	984	1089	1194	1299	1404			
7'-6" (90")	PRECAST	584	689	794	899	1004	1109	1214	1319			
8'-0" (96")	PRECAST	411	516	621	726	831	936	1041	1146			
8'-6" (102")	PRECAST	332	437	542	647	752	857	962	1067			



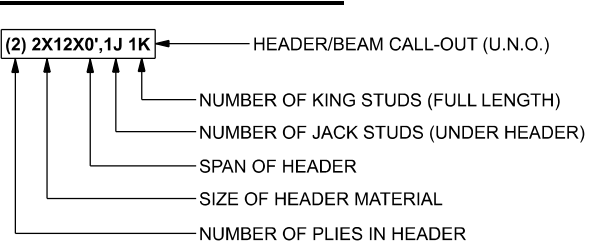
## STRUCTURAL PLAN

SCALE: 3/16" = 1'-0"

## STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC51-03, BC51-B1, BC51-B2, & BC51-B3. BC51-B1, BC51-B2, & BC51-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## FRAME HEADER LEGEND



## THREADED ROD LEGEND



## ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	91.9'	143.3'
REQUIRED	36.0'	28.0'

## UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS

1. USE HETA16 CMU TO TRUSS
2. USE H2.5A FRAME TO TRUSS
3. ALL LINTELS TO BE: 8F16-0B/1T
4. ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SP #2
5. ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE

## ENGINEERED TRUSSES

- ATTACH PER TRUSS UPLIFT TO CMU WALLS:
- HETA16 FOR UP TO 1350 LB UPLIFT
- (2) HETA16 FOR UP TO 2035 LB UPLIFT
- MGT FOR UP TO 3965 LB UPLIFT TO FRAME WALLS:
- (1) H2.5A FOR UP TO 495 LB UPLIFT OR
- (2) H2.5A FOR UP TO 990 LB UPLIFT
- SEE CONNECTOR TABLE OR SIMPSON BOOK FOR ADDITIONAL OPTIONS

Blake Construction

Ronnie Shuman Ros

PROJECT ADDRESS:  
205 SW Madison Court  
Lake City, FL 32024

DIMENSIONS:  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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Thursday, August 26, 2021

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JOB NUMBER:  
211179

S-3

OF 4 SHEETS

CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #26024689