

A CUSTOM HOME FOR:

THE MOSS'S

PROJECT ADDRESS:

Lot 29, Cobblestone S/D
Lake City, Florida 32055



SHEET INDEX

- A1 FRONT & REAR ELEVATIONS
- A2 LEFT & RIGHT ELEVATIONS
- A3 DIMENSIONED FLOOR PLAN
- A4 ELECTRICAL PLAN
- A5 FOUNDATION PLAN & DETAILS

AREA SUMMARY		
LIVING AREA	2,936	S.F.
GARAGE AREA	660	S.F.
COVERED PORCH AREA	336	S.F.
ENTRY PORCH AREA	163	S.F.
TOTAL AREA	4,095	S.F.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS

February 10, 2025

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

MODEL 2742 LEFT-HAND GARAGE FOR:
Moss Residence
Property Address: Lot 29, Cobblestone S/D, Lake City, Florida 32055
GIBALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

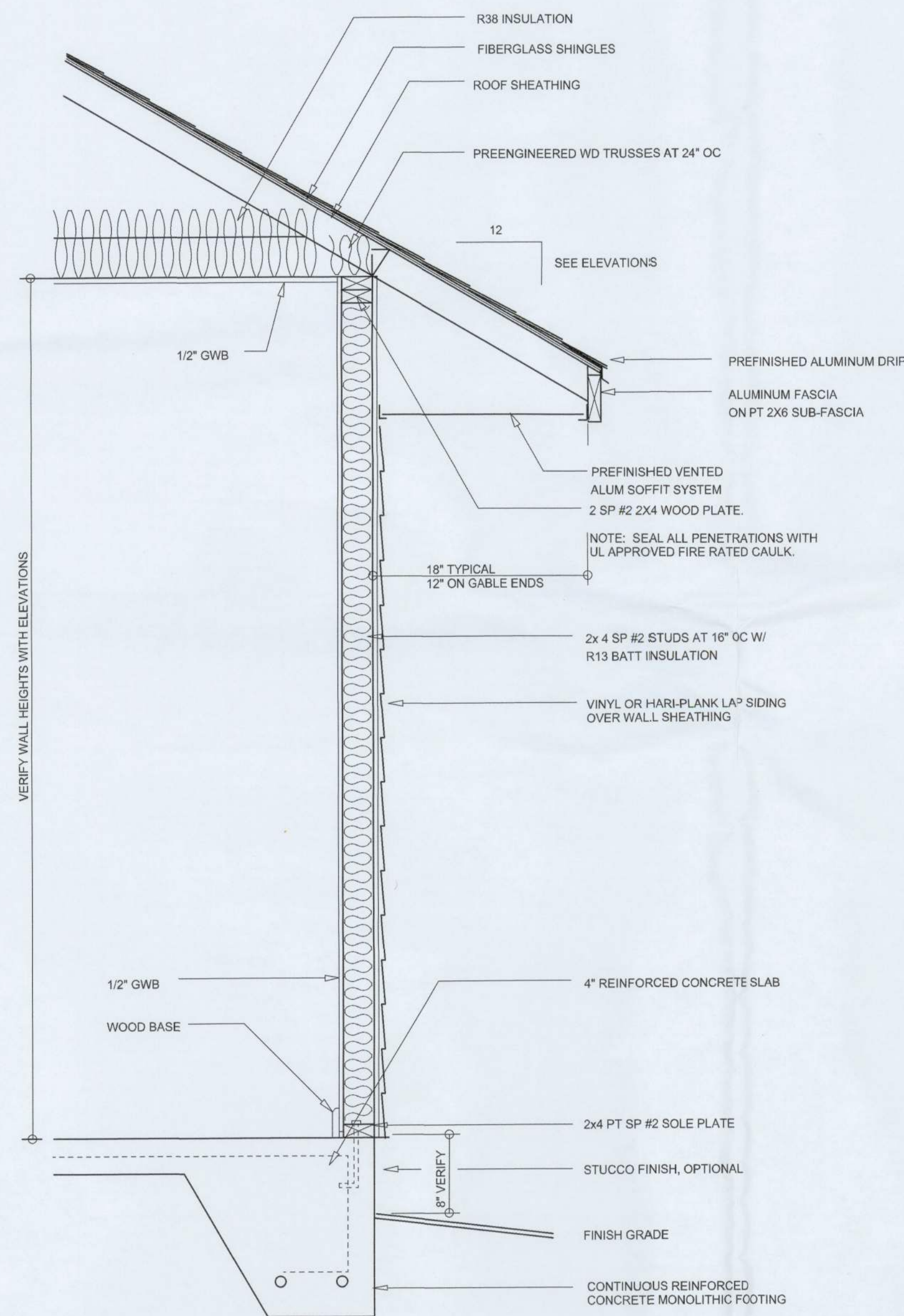
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JOB NUMBER
20240902

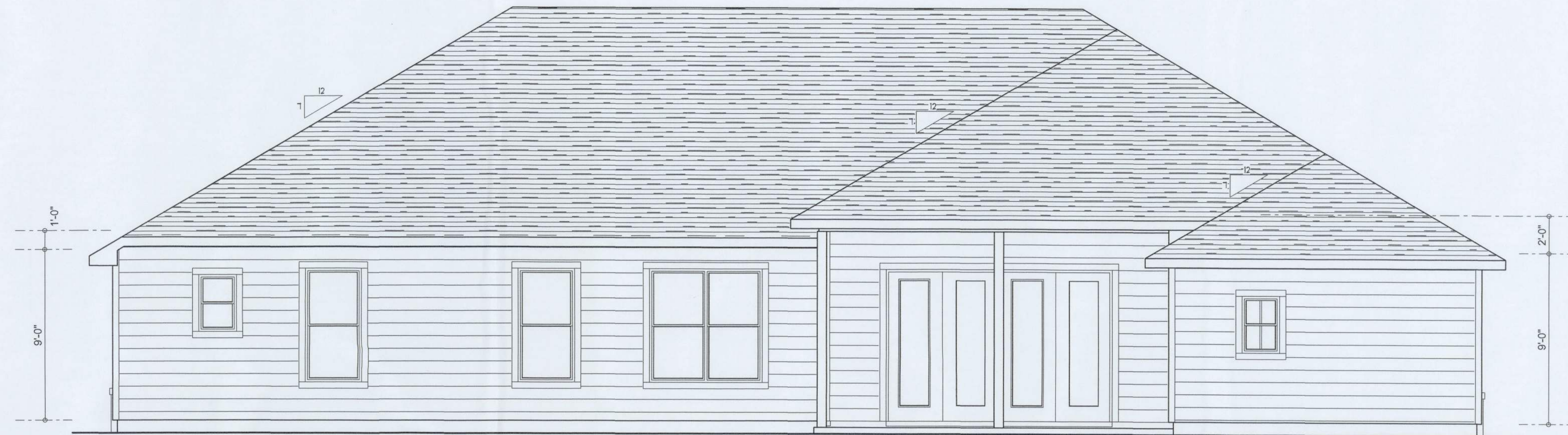
SHEET NUMBER
COVER



WVM



TYPICAL WALL SECTION
SCALE: 1" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
November 18, 2024

SOFTPLAN
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FRONT & REAR ELEVATIONS
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

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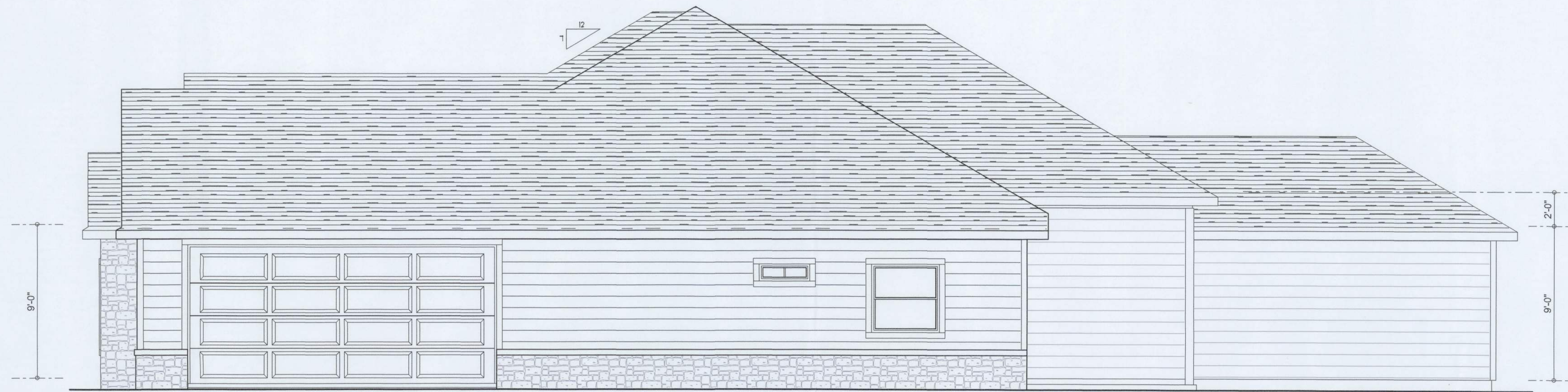
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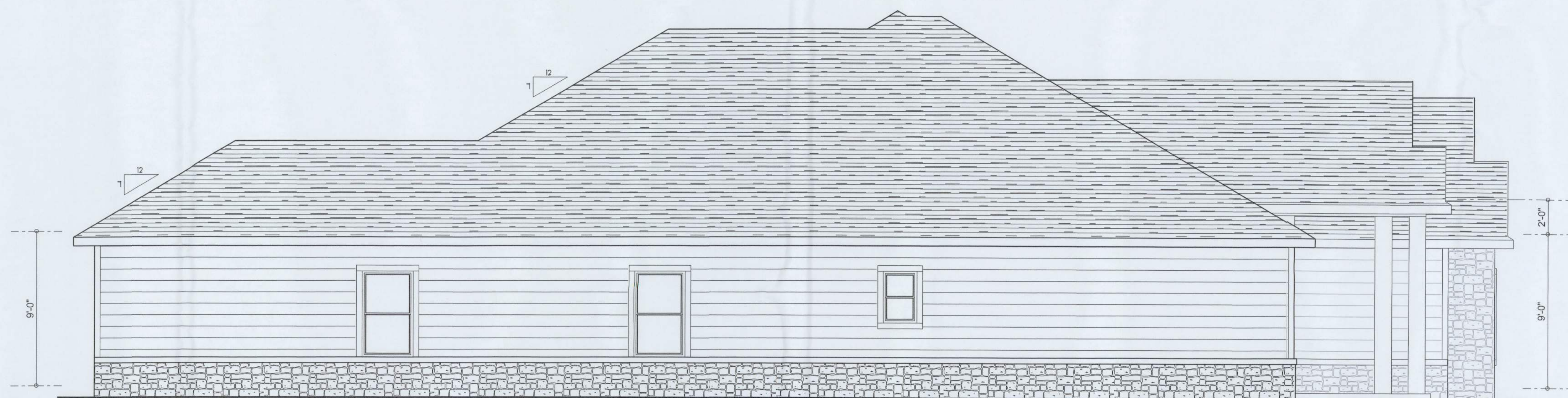
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A.1

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RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

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REVISIONS
November 18, 2024

SOFTPLAN
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LEFT & RIGHT ELEVATIONS
SCALE: 1/4" = 1'-0"

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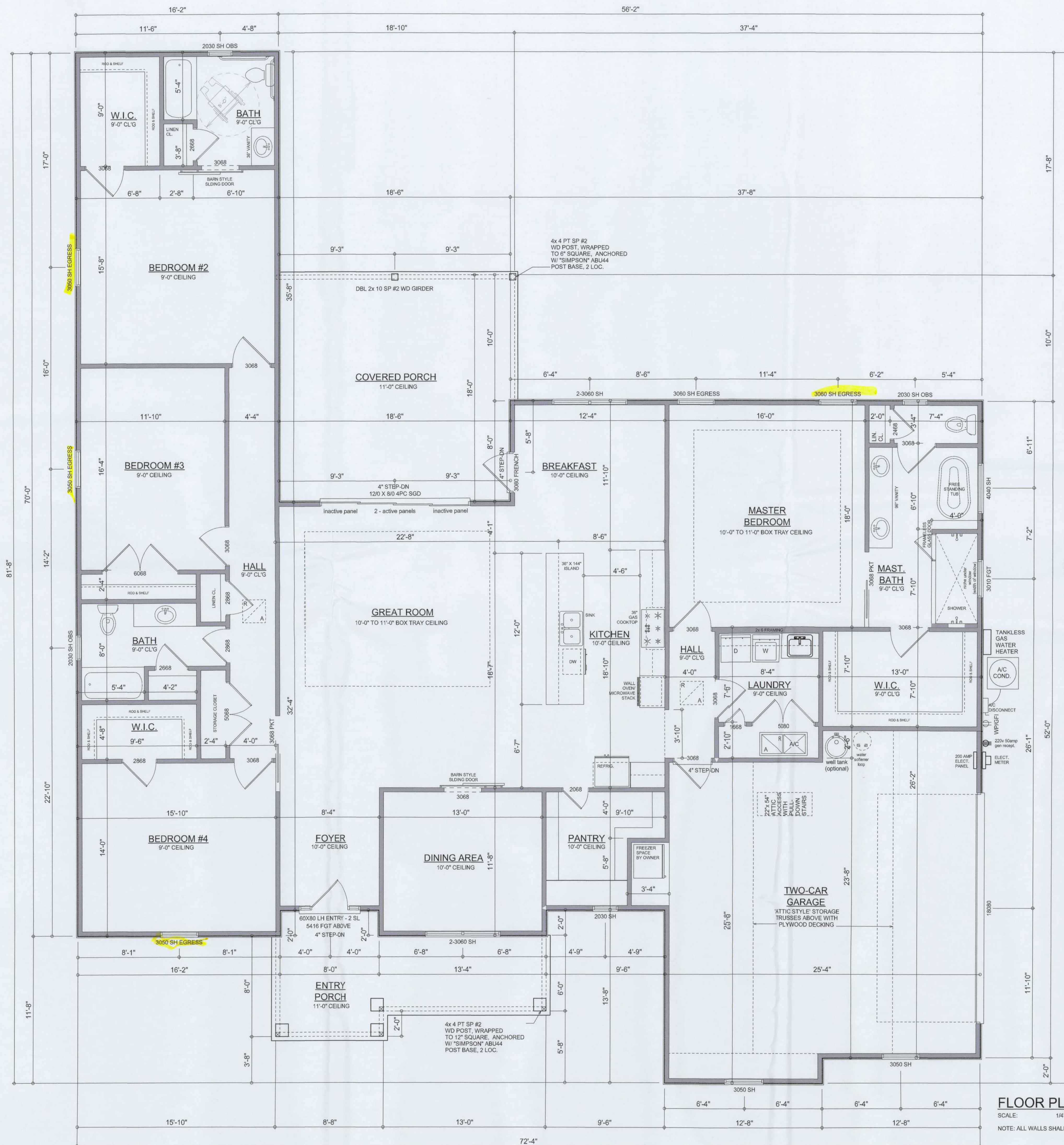
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A.2

Wm C. Tyers



- Garage fire separations shall comply with the following:
1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
 2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

FLOOR PLAN
SCALE: 1/4" = 1'-0"
NOTE: ALL WALLS SHALL BE 9'-0" UNLESS OTHERWISE NOTED.

AREA SUMMARY		
LIVING AREA	2,936	S.F.
GARAGE AREA	660	S.F.
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DIMENSIONED FLOOR PLAN
SCALE: 1/4" = 1'-0"

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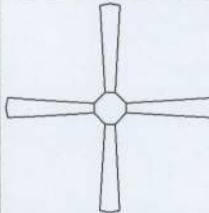














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A.3

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

Will C. My

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	ETHERNET JACK
	CIRCUIT FOR MINI-SPLIT A/C UNIT
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	DIMMER WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
<div>48" FLOOR</div>	2 OR 4 TUB FLUORESCENT FIXTURE

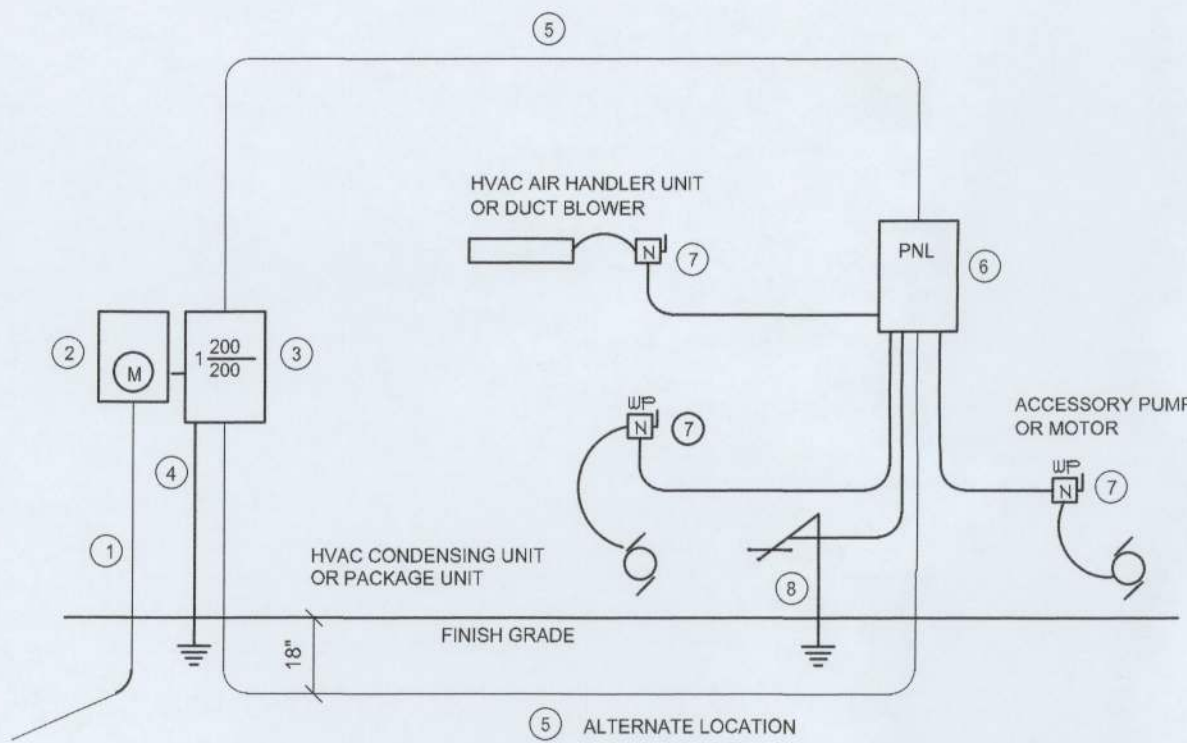
NOTE:
ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGH-EFFICIENCY
LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE BATTERY BACKUP POWER.
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2020 (NFPA-70) NATIONAL
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



- Service/Feeder Entrance Conductors: 2" rigid conduit, min. 18" deep, w/ continuous Ground Bonding Conductor. Service/Entrance Conductors shall not be spliced except that bolted connections at the Meter, Disconnecting Devices and Panel shall be allowed.
- Meter Enclosure, weatherproof, U.L. Listed.
- Main Disconnect Switch: fused or Main BRKR, weatherproof, U.L. Listed.
- Service entrance Ground: 1" - iron/steel rod x 8'-0" long and/or concrete encased foundation steel rebar x 20'-0" long. Grounding Conductor shall be bonded to each piece of Service/Entrance Equipment, and shall be sized per Item #5, below.
- 200 AMPERE SERVICE: 3-#2/0-USE-Cu, 1-#4-Cu-GND, 2" Conduit.
- House Panel (PNL), U.L. Listed, sized per schedule.
- Equipment Disconnect Switch: non-fused, in weatherproof enclosure, size according to Panel Schedule loads.
- Provide Ground Bond Wire to metal piping, size in accordance with the Service Ground Conductor.

NOTE:
THE MINIMUM AIC RATING FOR PANEL BOARDS, BRKRS
AND DISCONNECT SWITCHES SHALL BE 22,000 AIC.

ELECTRICAL RISER DIAGRAM: 200A

SCALE: NONE



ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

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SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

MODEL 2742 LEFT-HAND GARAGE FOR:
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JOB NUMBER
20240902

SHEET NUMBER
A.4

Wm C. Fry

CONCRETE / MASONRY /
METALS GENERAL NOTES:

- DESIGN SOIL BEARING PRESSURE: 1000 PSF.
- EXPANSIVE SOILS: WHERE DIRECTED BY THE SOILS ENGINEER, SOIL AUGMENTATION PER THE SOILS ENGINEER'S SPECIFICATIONS SHALL BE IMPLEMENTED PRIOR TO PLACING ANY FOUNDATIONS - TESTS AS SPECIFIED SHALL BE PERFORMED TO DETERMINE THE SUITABILITY OF THE SUB-GRADE TO SUPPORT THE DESIGN LOADS.
- CLEAN SAND FILL OVER STRIPPED AND COMPACTED EXISTING GD. SHALL BE PLACED IN 12" LIFTS. BOTH SUB-SOIL AND FILL COMPACTION SHALL BE NOT LESS THAN 98% AS MEASURED BY A MODIFIED PROCTOR TEST AT THE RATE OF ONE TEST FOR EACH 1500 SF OF BUILDING PAD AREA, OR FRACTION THEREOF, FOR EACH 12" LIFT.
- REINFORCING STEEL SHALL BE GRADE 60 AND MEET THE REQUIREMENTS OF ASTM A615, ALL BENDS SHALL BE MADE COLD.
- WELDED WIRE MESH SLAB REINFORCING SHALL MEET THE REQUIREMENTS OF ASTM A185 - MIN. YIELD STRESS = 85 KSI.
- CONCRETE SHALL BE STANDARD MIX $F'c = 3000$ PSI FOR ALL FTGS. SLABS, COLUMNS AND BEAMS OR SHALL BE STANDARD PUMP MIX $F'c = 3000$ PSI. STRENGTH SHALL BE ATTAINED WITHIN 28 DAYS OF PLACEMENT. MIXING, PLACING AND FINISHING SHALL BE AS PER ACI STANDARDS.
- CONCRETE BLOCK SHALL BE AS PER MANUFACTURER'S PRODUCT GUIDE FOR ASTM C-90 REQUIREMENTS WITH MEDIUM SURFACE FINISH - $F'm = 1500$ PSI.
- MORTAR SHALL BE TYPE "M" OR "N" FOR ALL MASONRY UNITS.
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 STANDARDS FOR STRENGTH, BOLTS SHALL BE ASTM A307 / GRADE 1 OR A325, AS PER PLAN REQUIREMENTS.
- WELDS SHALL BE AS PER "AMERICAN WELDING SOCIETY" STANDARDS FOR STRUCTURAL STEEL APPLICATIONS.
- 2X4 P/T WOOD SILL, CONT., ALL AROUND, W/ 5/8" A.B. W/ 3" SQ. X 1/4" PLATE WASHERS WITHIN 6" FROM EACH CORNER, EA. WAY, & WITHIN 6" FROM ALL WALL OPENINGS / ENDS - 1/2" A.B. W/ 2" SQ. WASHERS ALONG EACH RUN @ 48" O.C. MAX. - ALL ANCHOR BOLTS SHALL HAVE A MINIMUM OF 8" EMBEDMENT INTO THE CONCRETE.

NOTE:
THE DESIGN WIND SPEED FOR THIS PROJECT IS 130 MPH PER 2020 FBC (7TH EDITION) AND LOCAL JURISDICTION REQUIREMENTS

NOTE:
ADDED FILL SHALL BE APPLIED IN 8" LIFTS - EA. LIFT SHALL BE COMPACTED TO 98% DRY COMPACTION PER THE "MODIFIED PROCTOR" METHOD.

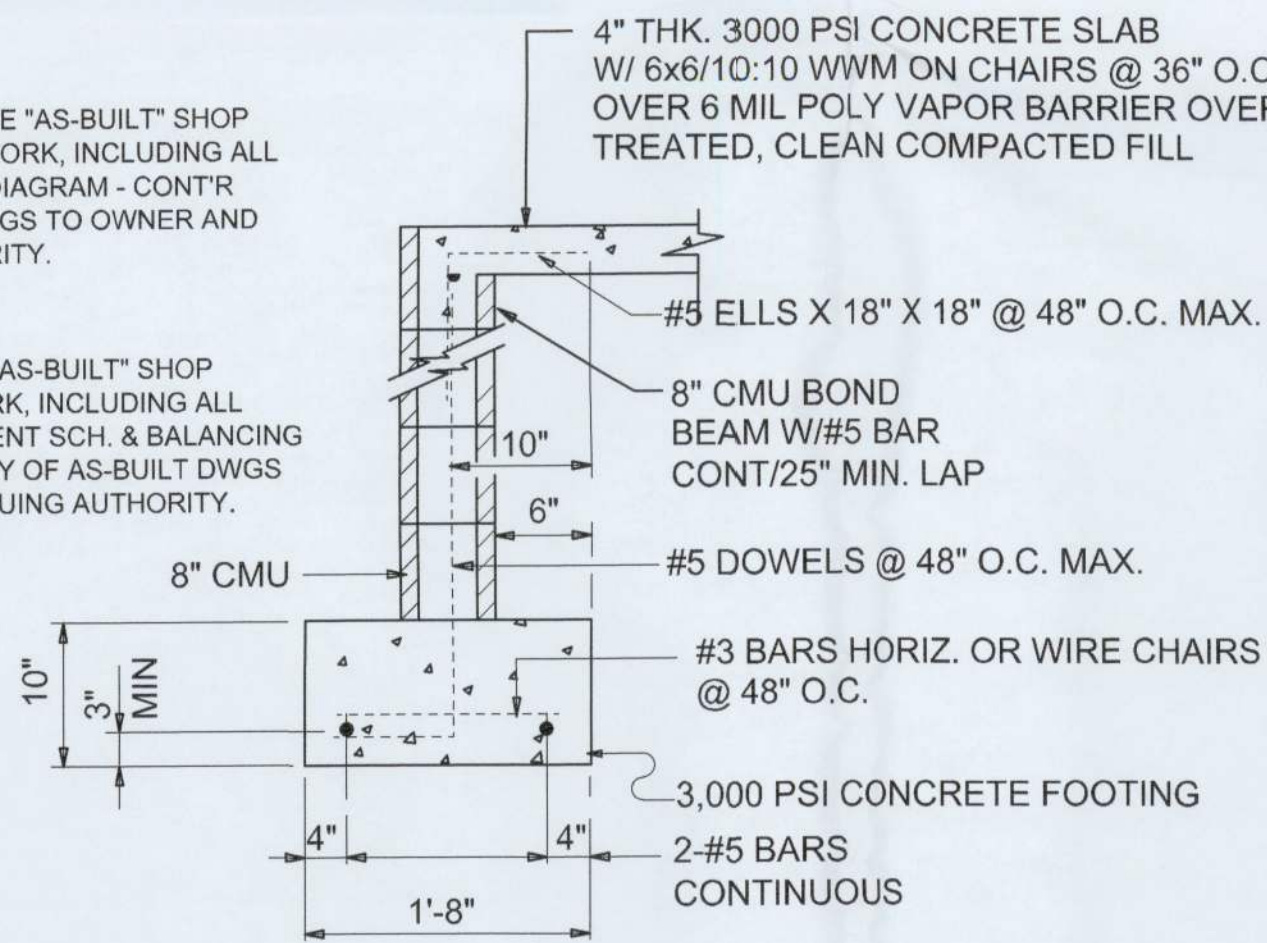
NOTE:
PLUMBING CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL PLUMBING WORK, INCLUDING ALL PLUMBING LINE LOCATIONS AND RISER DIAGRAM - CONTR. SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

NOTE:
H.V.A.C. CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL H.V.A.C. WORK, INCLUDING ALL DUCTWORK LOC., SIZES, LINES, EQUIPMENT SCH. & BALANCING REPORT - CONTR. SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

NOTE!
PRIOR TO THE CONSTRUCTION OF THE FOUNDATION, THE CONTRACTOR SHALL COORDINATE ANY INTERIOR BEARING LOCATION CONDITIONS PER THE TRUSS ENGINEERED SHOP DRAWINGS WITH THE FOUNDATION PLAN. ANY INTERIOR BEARING LOCATIONS OR ANY POINT LOADS OF 4.0 K OR GREATER SHALL BE SUPPORTED VIA A MODIFIED FOUNDATION PLAN TAKING THESE LOADS INTO CONSIDERATION. THE CONTRACTOR SHALL MAKE THE ENGINEERED TRUSS SHOP DRAWINGS AVAILABLE TO THE ARCHITECT FOR THE PURPOSE OF RENDERING SUCH MODIFICATIONS PRIOR TO POURING ANY CONCRETE.

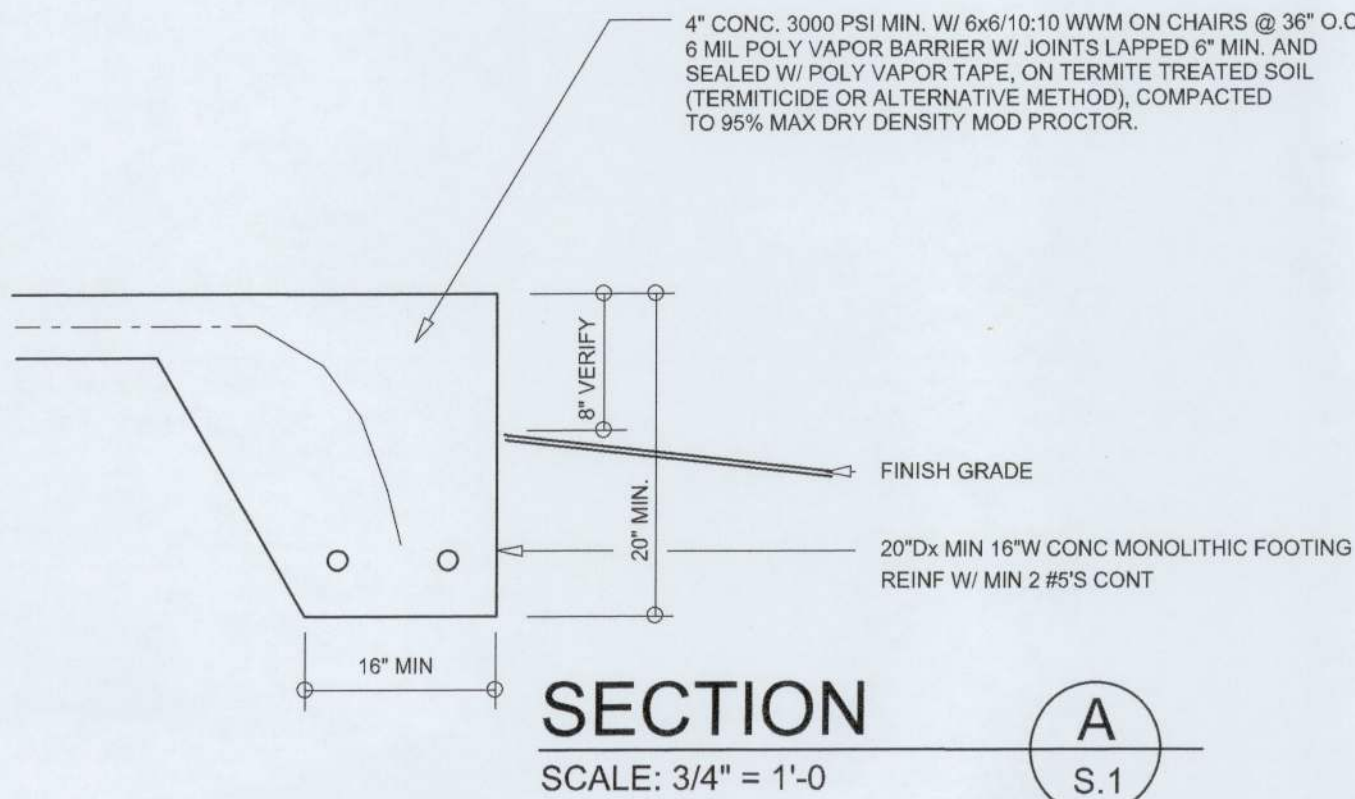
INTERIOR BEARING WALLS:

IT IS THE BUILDING CONTRACTOR'S RESPONSIBILITY TO VERIFY WITH THE TRUSS ENGINEERING ANY AND ALL INTERIOR BEARING WALL LOCATIONS AND FURNISH THE ENGINEER OR ARCHITECT OF RECORD TRUSS INFO SO THICKENED FOOTING'S CAN BE SIZED AND LOCATED ON THE FOUNDATION PLAN.



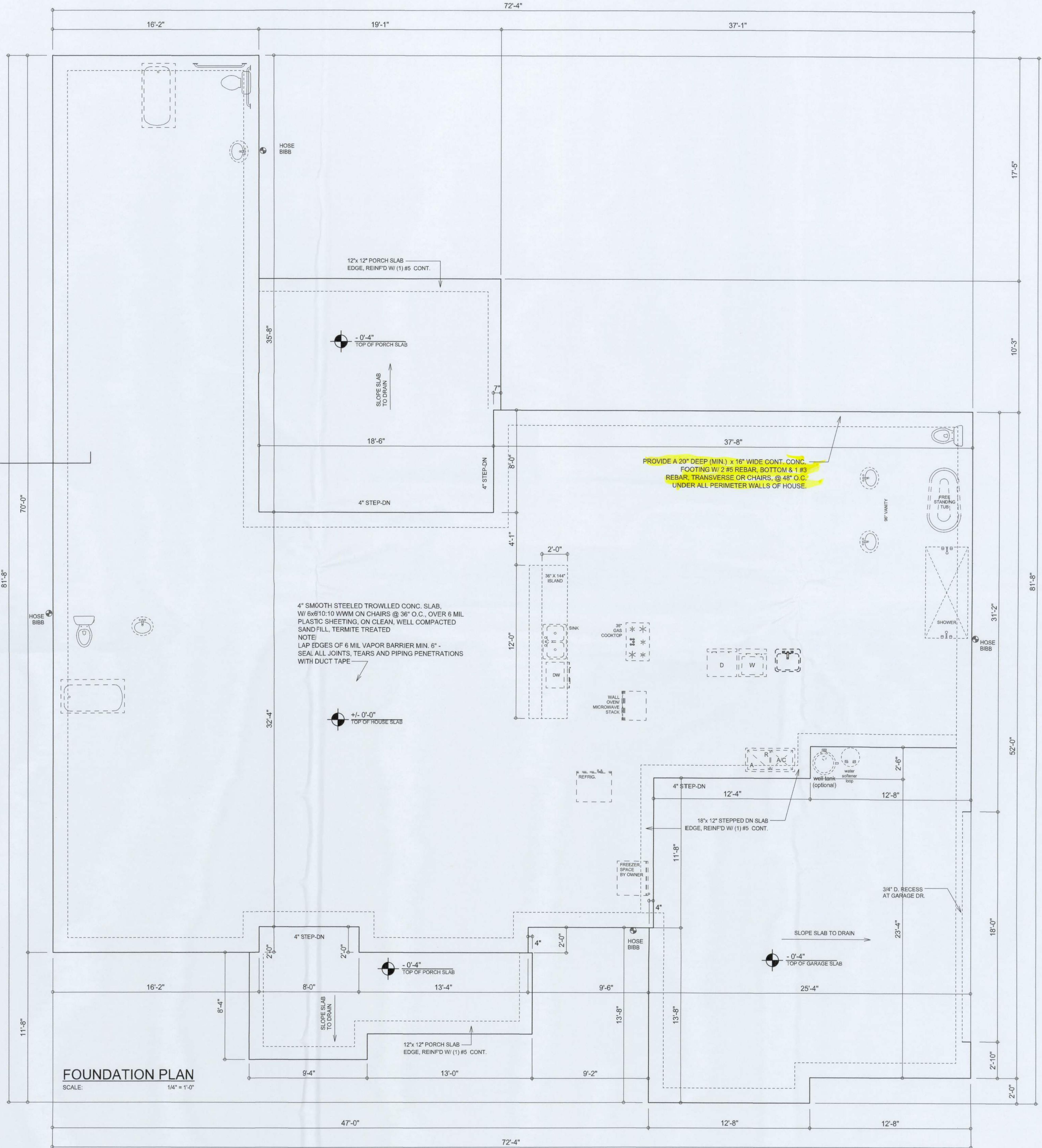
OPTIONAL
SCALE: 3/4" = 1'-0"

A
S.1



SECTION
SCALE: 3/4" = 1'-0"

A
S.1



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

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FOUNDATION PLAN
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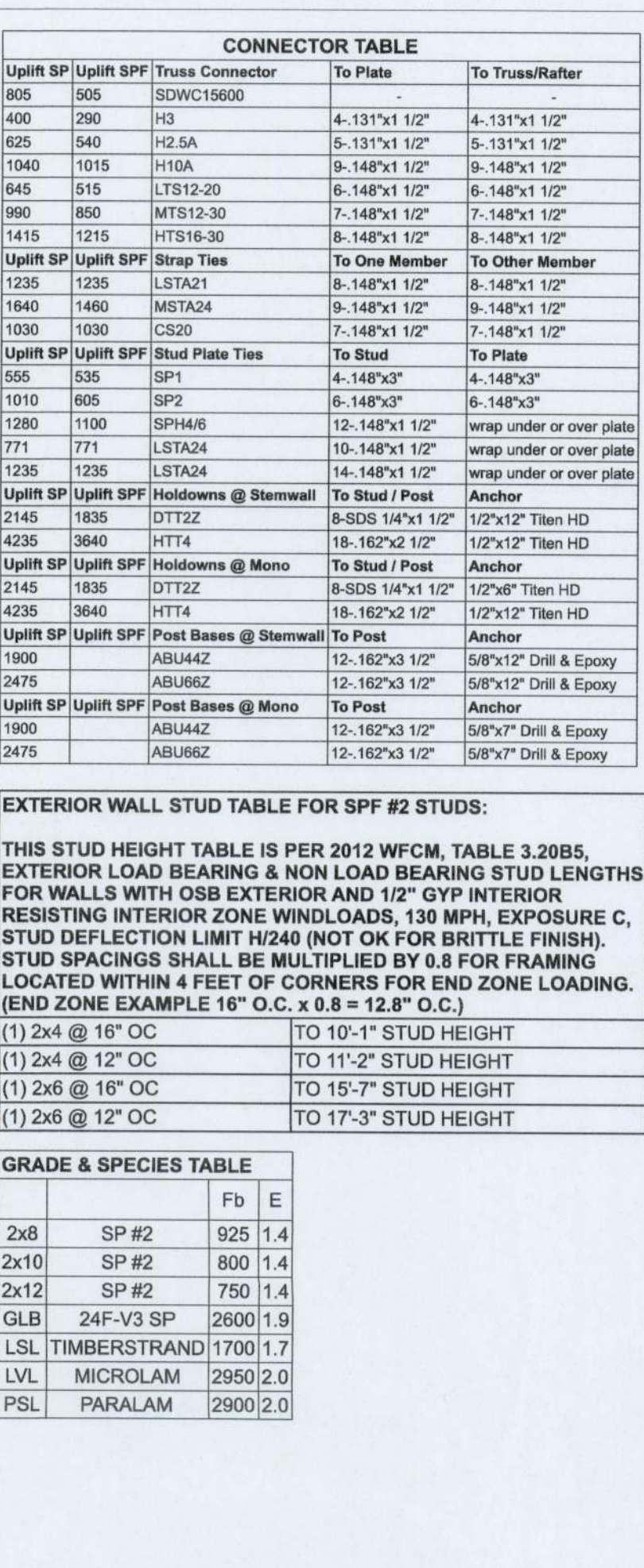
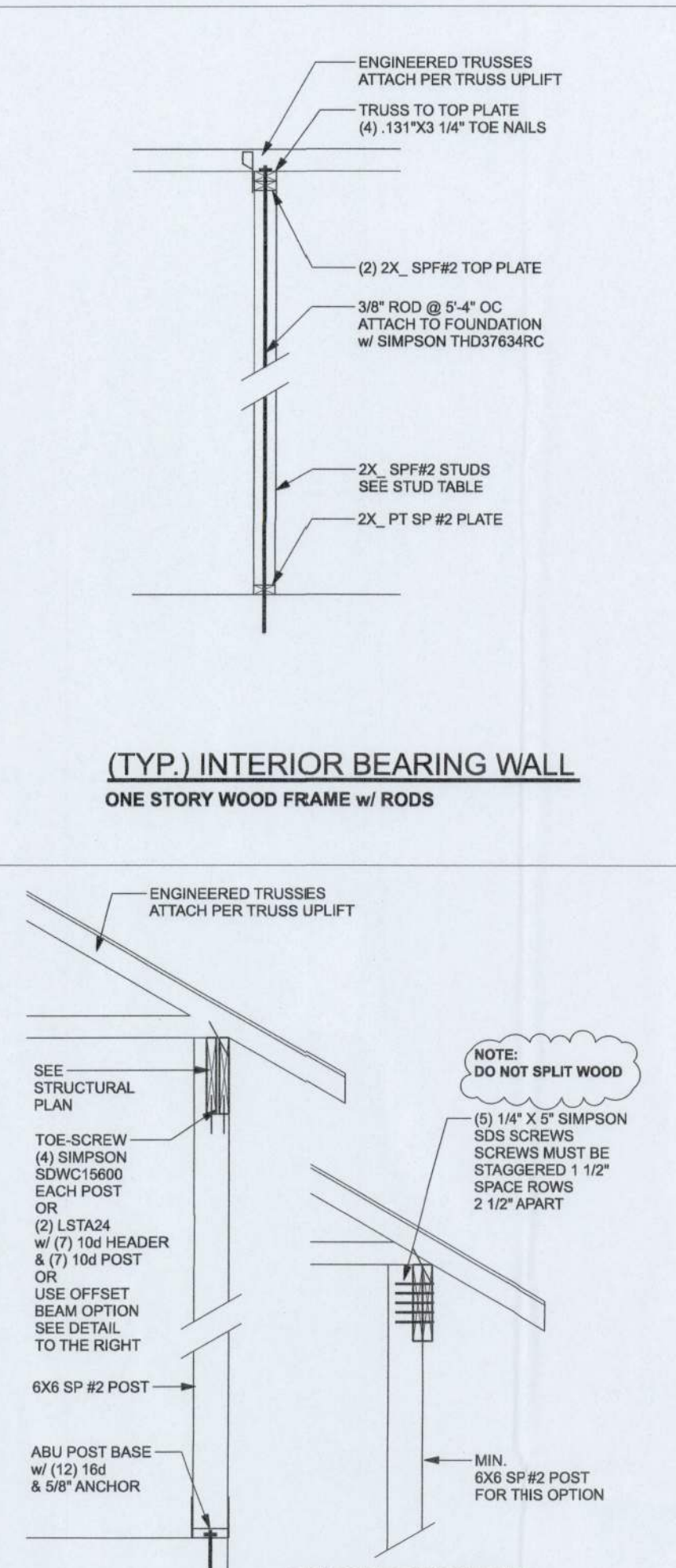
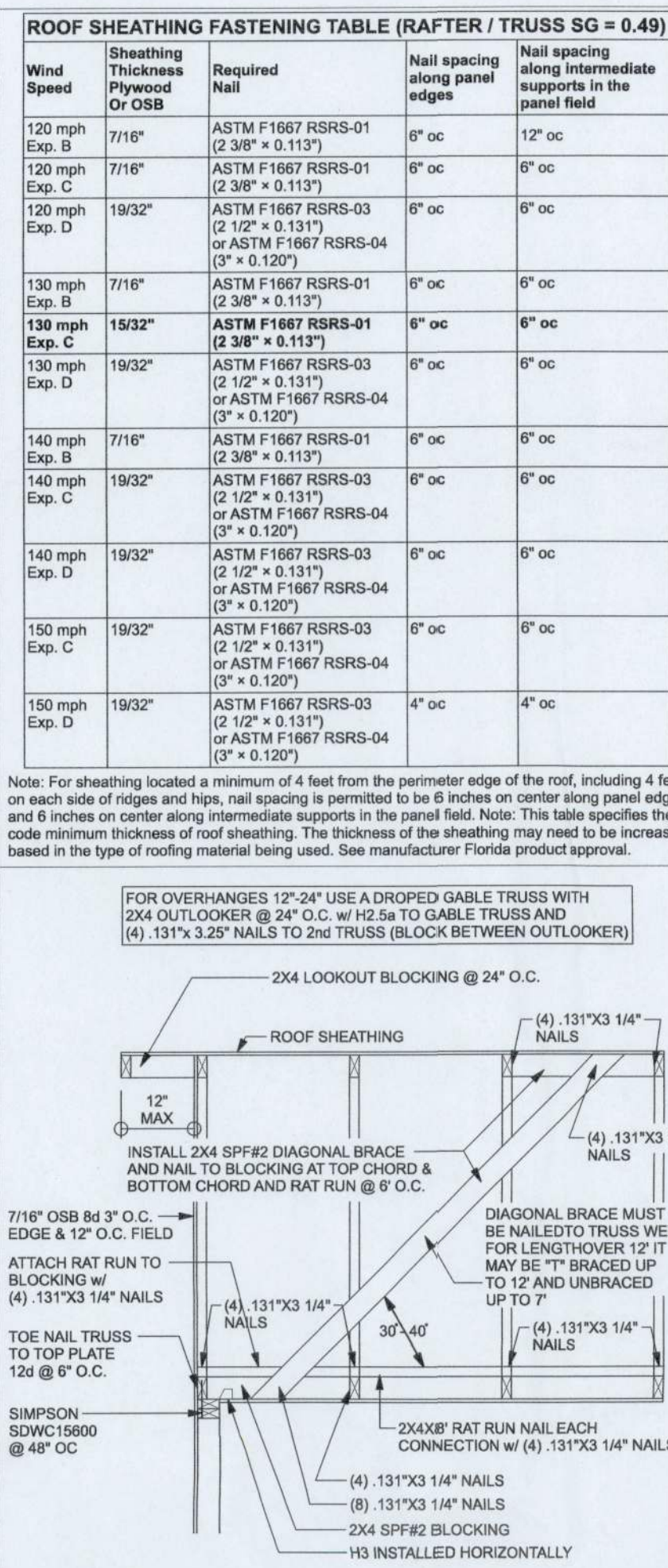
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SHEET NUMBER
A.5

Will C. Myers



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBRI, TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, LOCATION, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, REACTION LOCATIONS, JOINT DETAILING, CONNECTIONS TO BEARING WALLS, ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURERS DESIGN ENGINEER. IF THE BUILDERS RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS TO BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR WIND UPLIFT CONNECTIONS TO BEARING STRUCTURE. STAIN 2X6 RAFTERS TO MATCH EXISTING WIND UPLIFT CONNECTION 415 LB EACH

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN IS A SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 150 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONTRACT: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, FC = 2500 PSI
WELDED REBAR REINFORCED SLAB: 6" x 8' x W14 x 41 + 8# - 8XSU, WELDED REBAR REINFORCEMENT BAR(S) (W.W.) CONFORMING TO ASTM A195, LOCATED IN MIDDLE OF THE SLAB COUNTERTOP WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 9"

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT SHALL BE 12" THICK TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD BUT THE MANUFACTURERS RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUBMIT TO PROVIDE ASTM C 1116 CERTIFICATION.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND SHALL BE LIMITED TO ONE PER FOOT. THE PURPOSE OF THE REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER'S REQUEST) IS TO REDUCE CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A15, GRADE 40, DEFORMED BARS, F.Y. = 40 KSI, ALL LAP SPICES 40" / DO 25% FOR #5S; UNDO, UNDO, ALL REBAR REMOINT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH THE CONTRACTORS INSTALLATION INSTRUCTIONS SHALL BE FOLLOWED TO ACHIEVE RATED LOADS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, SHEATHING UNDERSHEATHED APPLIED PERPENDICULAR TO DIAPHRAGM, UNDER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTIONS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR BOLTS, ANCHORS, AND REACTIONS, UPLIFTS, AND BEARING LOCATIONS. THE CONTRACTOR SHALL EQUIVALENT DESIGN OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED PERFORMANCE CRITERIA. THE CONTRACTORS INSTALLATION INSTRUCTIONS SHALL BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR REINFORCED IN CAST-IN PLACE.

BUILDER'S RESPONSIBILITY:

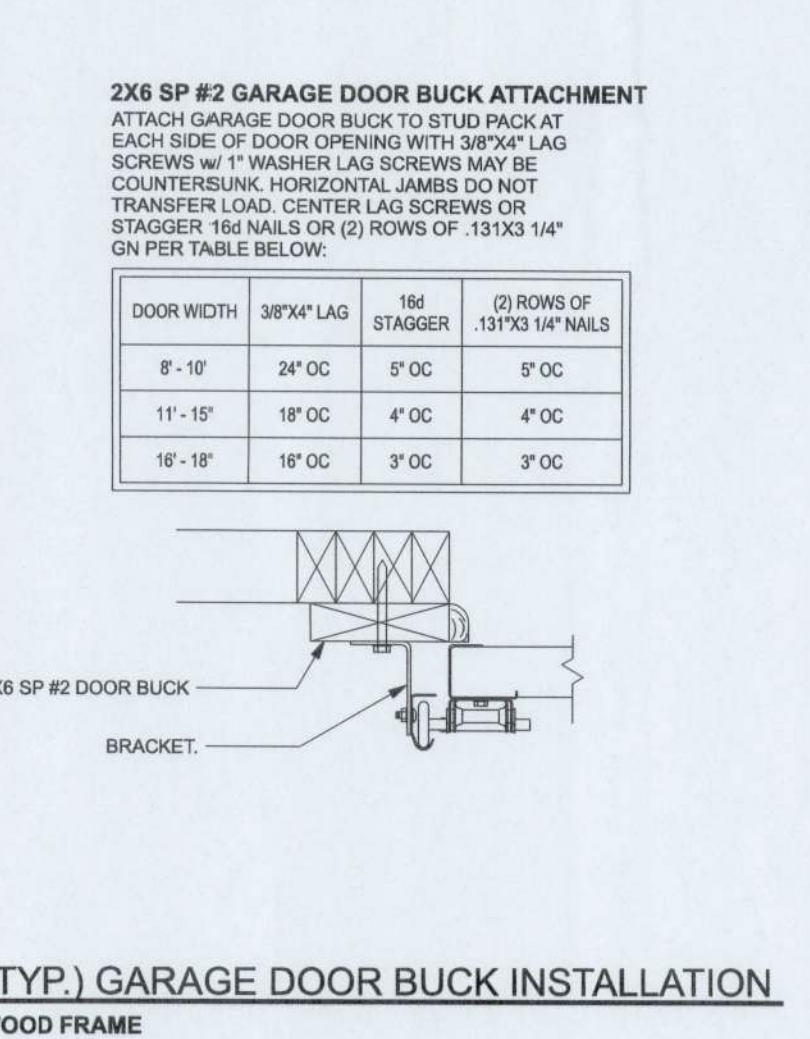
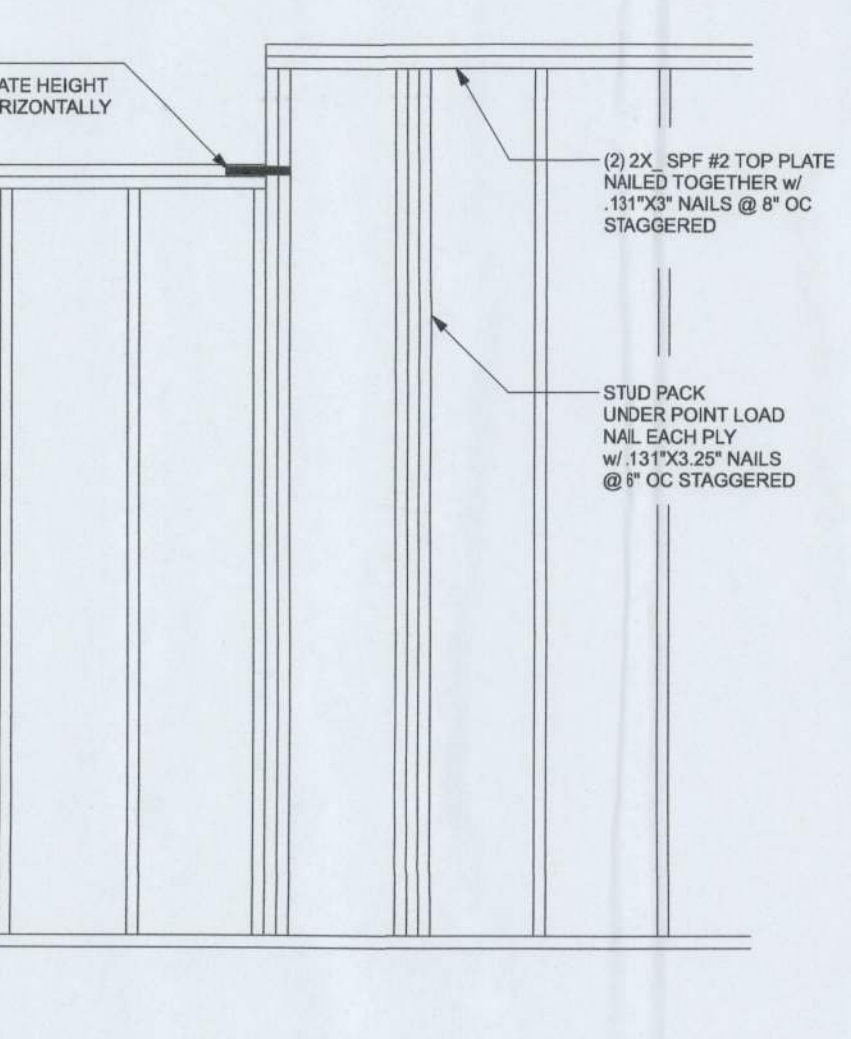
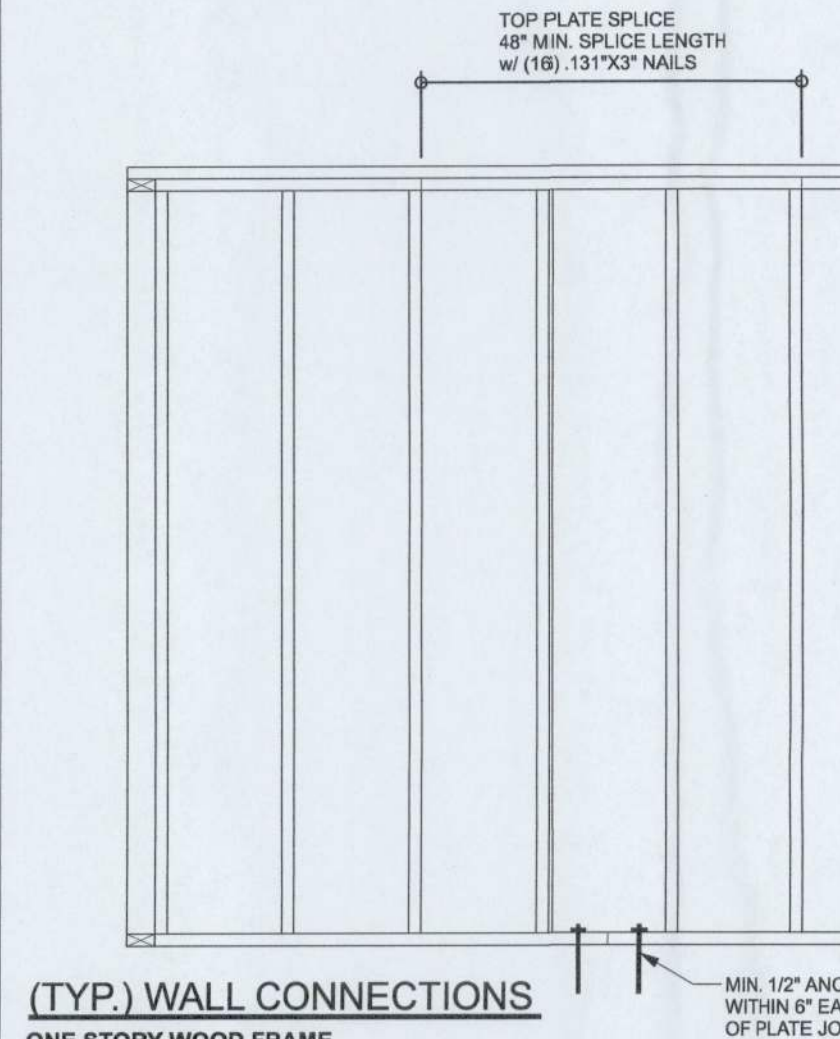
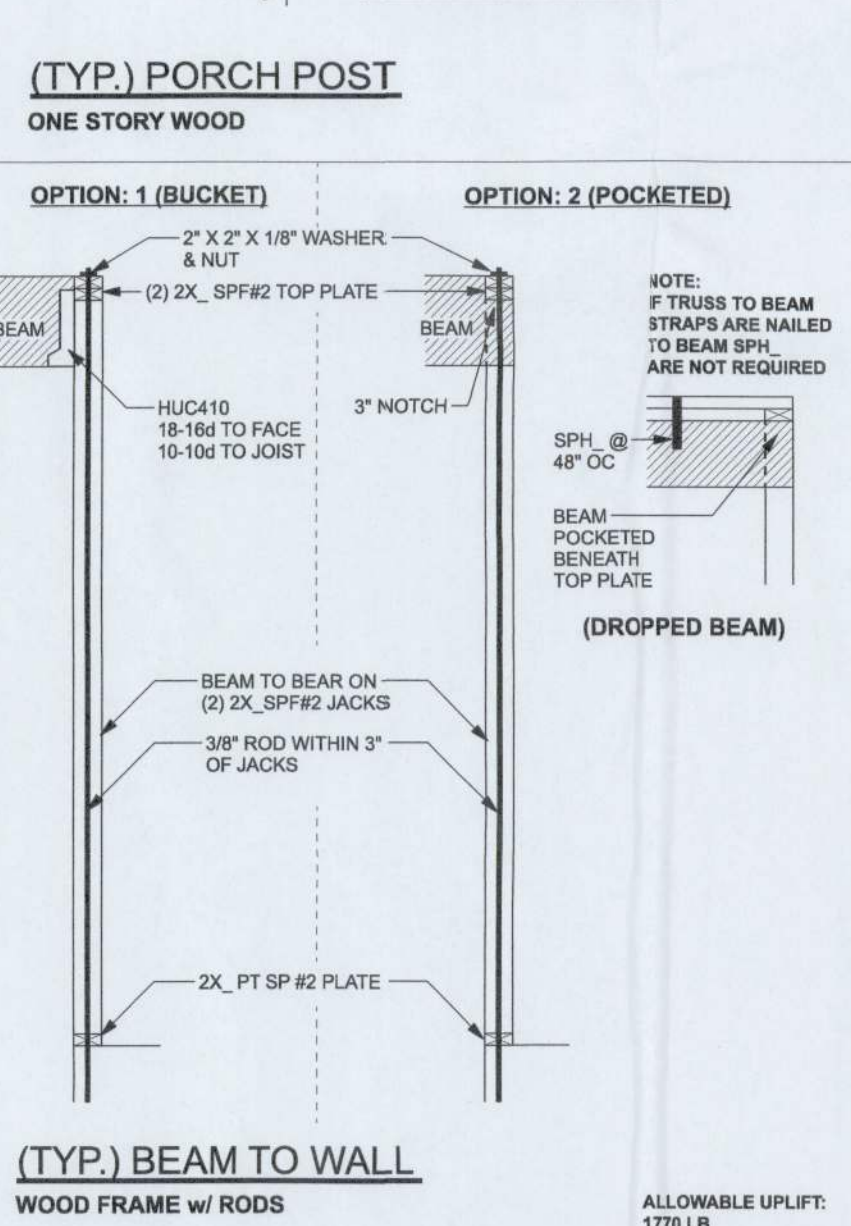
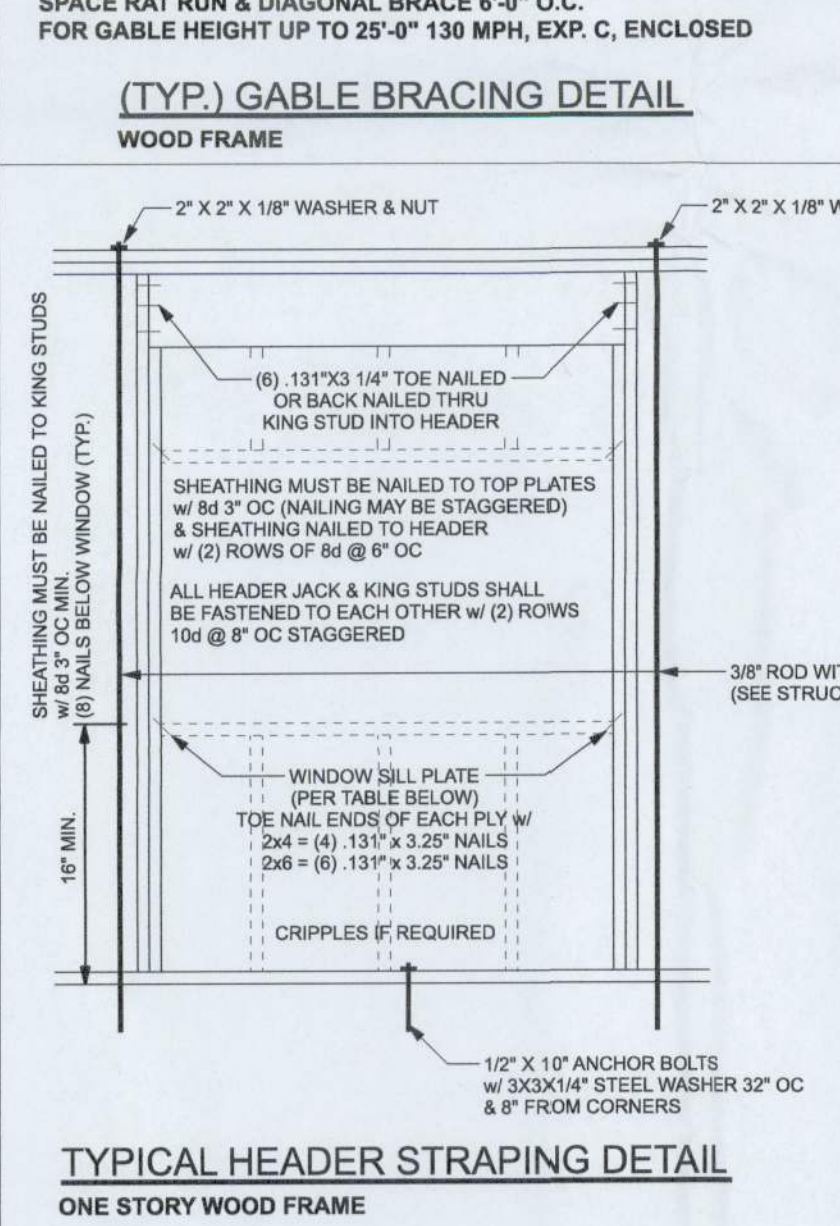
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE NOT PART OF THE TRUSS ENGINEERING, INCLUDING THE FOLLOWING:

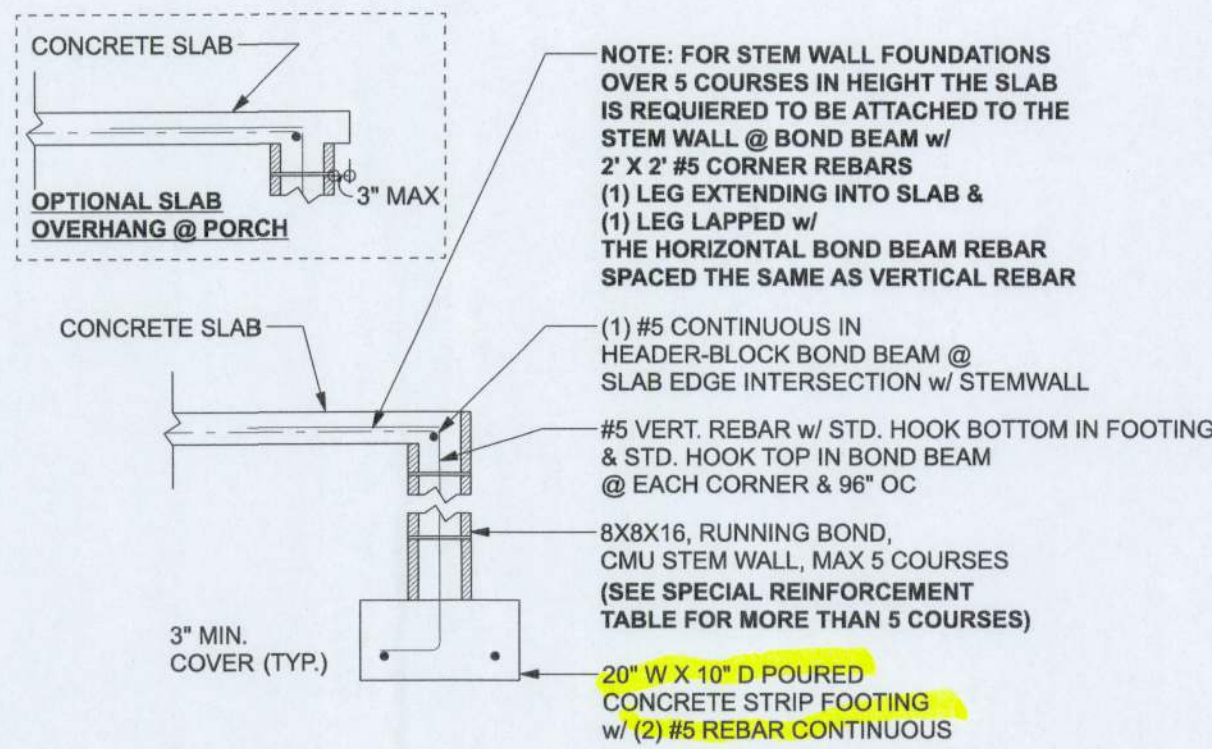
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DERRIS ZONE, AND FLOOD ZONE.
PROVIDE THE TRUSS ENGINEERING WITH ALL NECESSARY DATA, WHICH COMPLY WITH FBRI REQUIREMENTS FOR THE STATED WIND SPEED DESIGN PRESSURES.
PROVIDE A CLEAR ACCESS ROAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THAT THE TRUSS ENGINEERING INCLUDED ENGINEERING INCLUDES TRUSSES, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

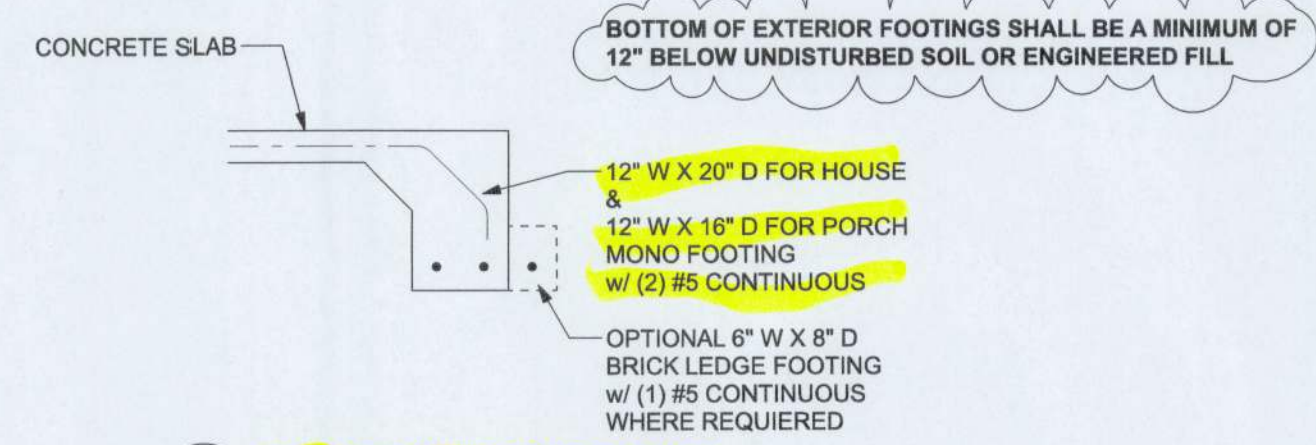
ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBRI.
BASED ON REACTION UPLIFTS, AND BEARING LOCATIONS. TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE TRUSS ENGINEER TO VERIFY THE TRUSS ENGINEERING OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECTNESS.

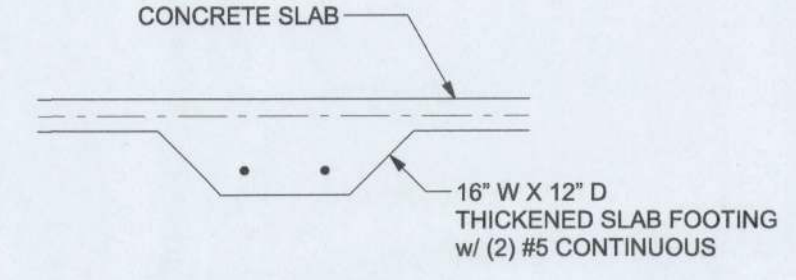




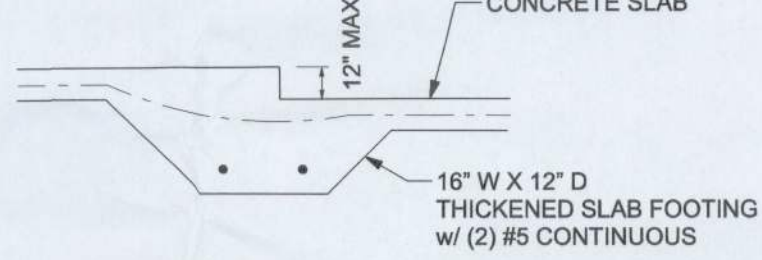
F1 S-2 **OPTIONAL STEM WALL FOOTING**
SCALE: 1/2" = 1'-0"



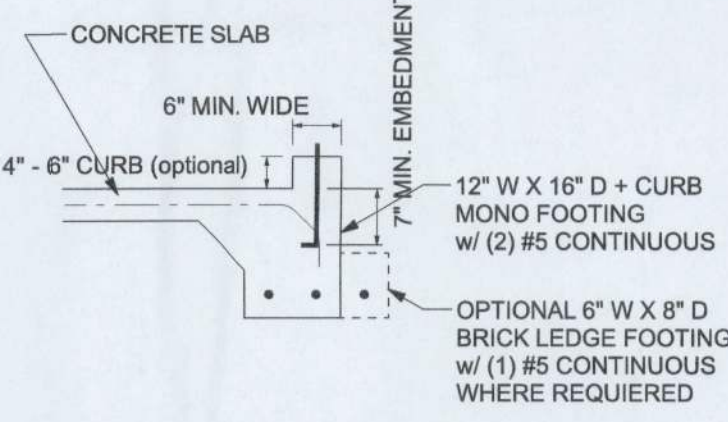
F1 S-2 **MONOLITHIC FOOTING**
SCALE: 1/2" = 1'-0"



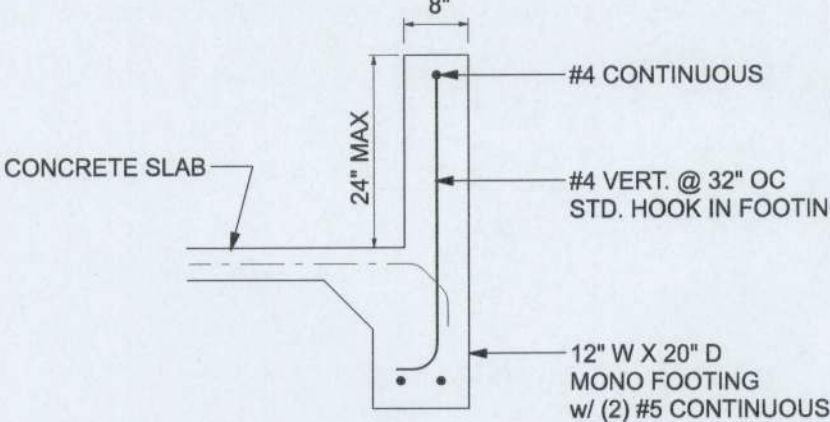
F2 S-2 **INTERIOR BEARING FOOTING**
SCALE: 1/2" = 1'-0"



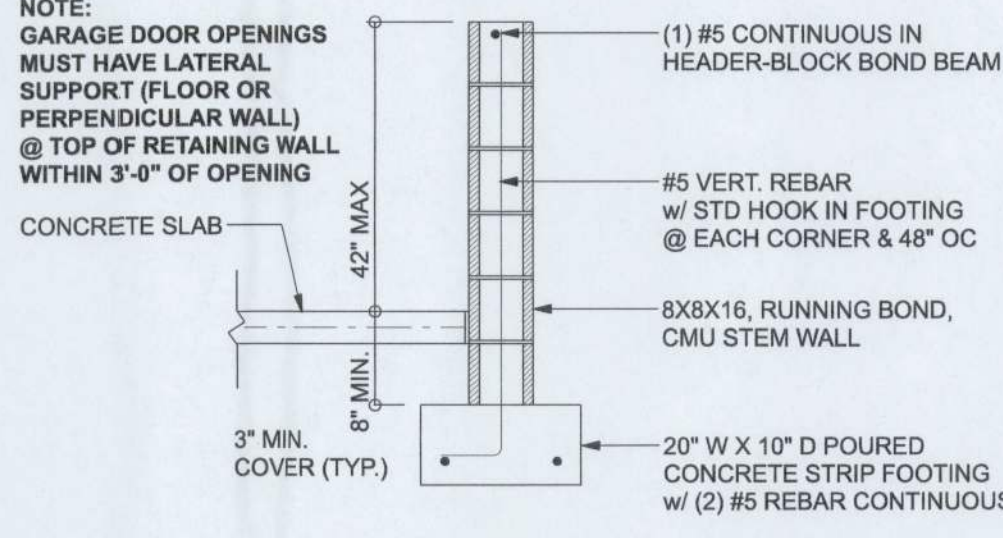
F3 S-2 **INTERIOR BEARING STEP FOOTING**
SCALE: 1/2" = 1'-0"



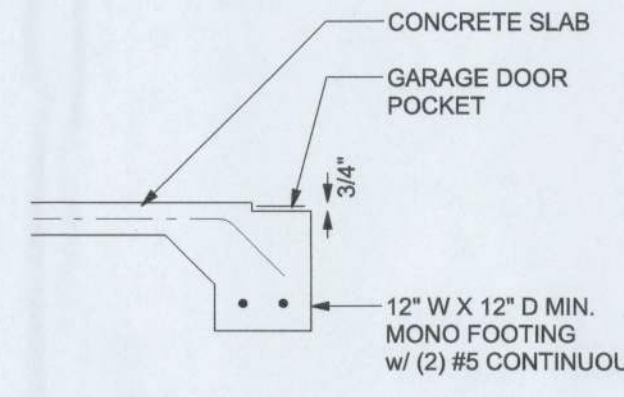
F4 S-2 **MONOLITHIC CURB FOOTING**
SCALE: 1/2" = 1'-0"



MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 **OPTIONAL STEM WALL CURB FOOTING**
SCALE: 1/2" = 1'-0"



F5 S-2 **GARAGE DOOR POCKET FOOTING**
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

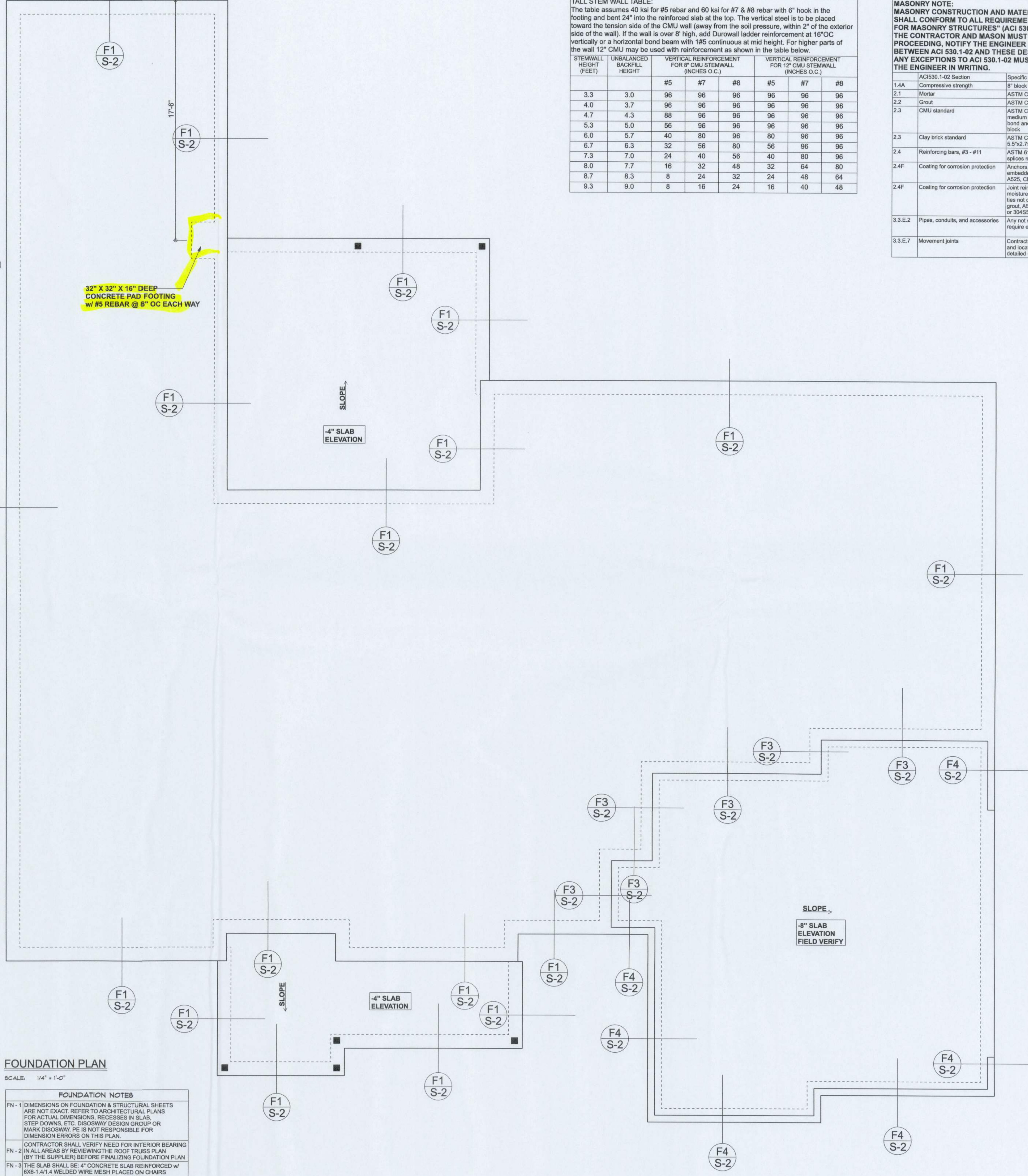
- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER BEFORE FINALIZING FOUNDATION PLAN)
- FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN
- FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X8-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1'-0" DEPTH OR FIBER MESH CONCRETE, 4-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1/65 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls 17m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, LNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splice min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



Gibraltar Contracting, LLC

Moss Res.

PROJECT ADDRESS:
Cobleskill SCD, Lake City, FL
Lot 29



DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

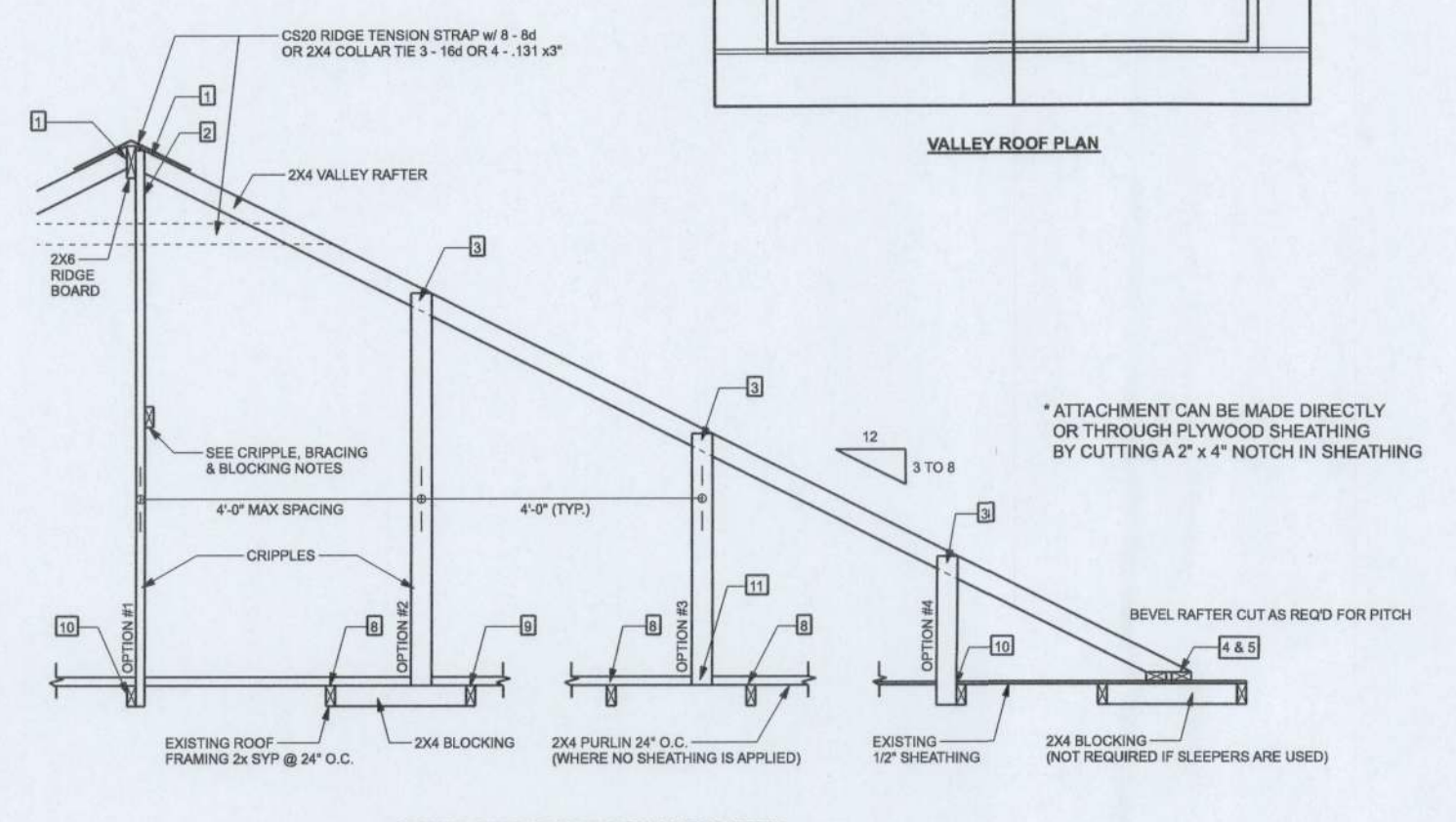
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to my engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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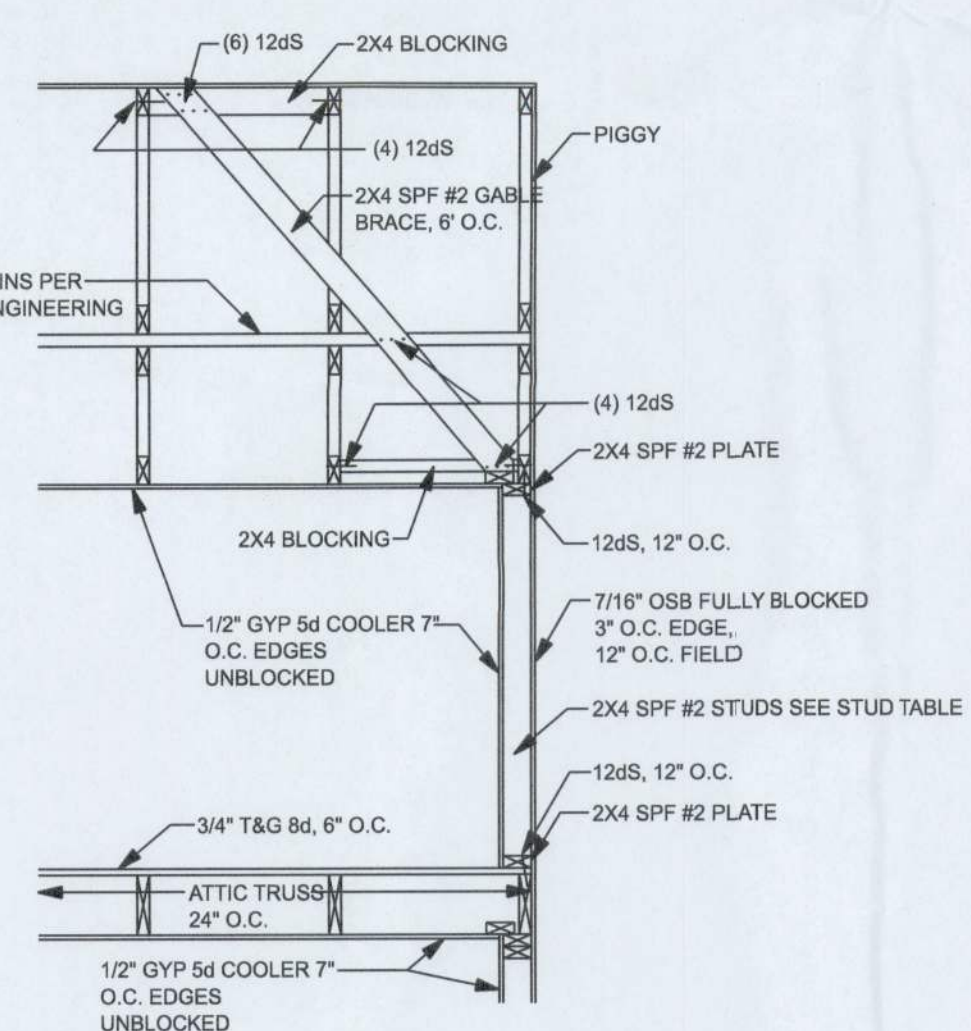
JOB NUMBER:
250415
S-2
OF 3 SHEETS



TRUSS
 TRUSS UNDER VALLEY FRAMING
 VALLEY RAFTER OR RIDGE
 CRIPPLE
 CRIPPLES 4'-0" O.C. For 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES		
1	2X4 RAFTERS TO RIDGE	3-16d OR 6 - 131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6 - 131 x 3" FACE NAILS
3	CRIPPLE TO RAFTER	3 - 16d OR 6 - 131 x 3" FACE NAILS
4	RAFTERS TO SLEEPER OR BLOCKING	6-16d OR 12 - 131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8 - 131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3-16d OR 6 - 131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3-16d OR 6 - 131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP)	3-16d OR 6 - 131 x 3" NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8 - 131 x 3" NAILS
10	TRUSS TO BLOCKING	3-16d OR 6 - 131 x 3" END NAILS
11	CRIPPLE TO TRUSS	3-16d OR 6 - 131 x 3" FACE NAILS
12	SLEEPER TO PURLIN	3-16d OR 6 - 131 x 3" FACE NAILS

SCALE: N.T.S



SCALE: 1/2"=1'-0"

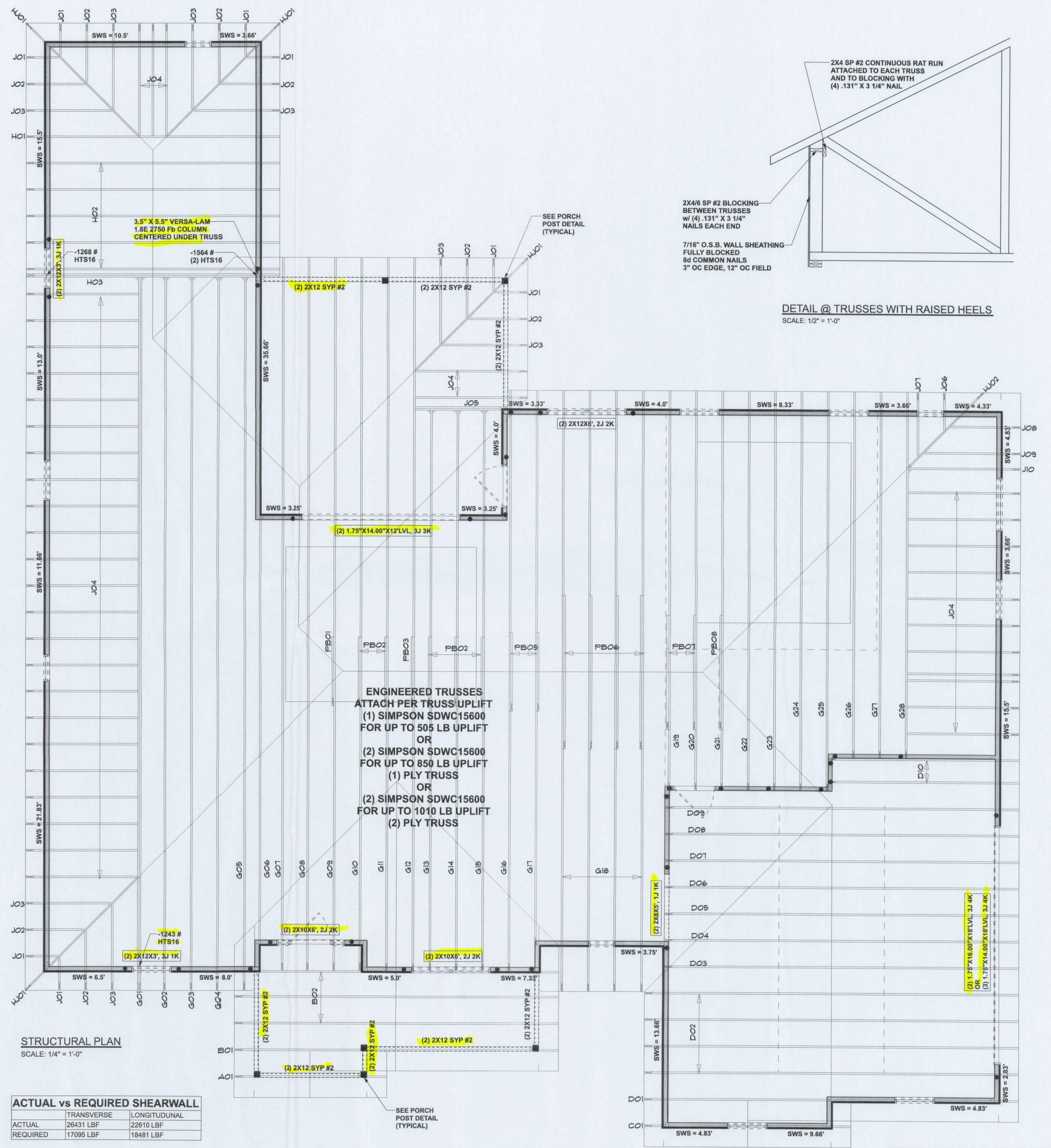
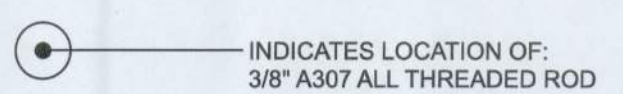
SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
SN-3	USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
SN-4	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-103, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

(2) 2X6X0', 1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)

- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

INDICATES LOCATION OF:
3/8" A307 ALL THREADED ROD



2X4/6 SP #2 BLOCKING
BETWEEN TRUSSES
w/ (4) .131" X 3 1/4"
NAILS EACH END

7/16" O.S.B. WALL SHEATHING—
FULLY BLOCKED
8d COMMON NAILS
3" OC EDGE, 12" OC FIELD

SCALE: 1/2" = 1'-0"

**ENGINEERED TRUSSES
ATTACH PER TRUSS UPLIFT**

**(1) SIMPSON SDWC15600
FOR UP TO 505 LB UPLIFT
OR
(2) SIMPSON SDWC15600
FOR UP TO 850 LB UPLIFT
(1) PLY TRUSS
OR
(2) SIMPSON SDWC15600
FOR UP TO 1010 LB UPLIFT
(2) PLY TRUSS**

ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDUNAL
ACTUAL	26431 LBF	22610 LBF
REQUIRED	17095 LBF	18481 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO.
JOB #25-2306

PROJECT ADDRESS:
Lot 29 Cobblestone S/D, Lake City, FL

FL PE 53915

This item has been digitally signed and sealed by
Mark Disosway PE on digital signature date.
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verified on any electronic copies.



DIMENSIONS:
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
250415

S-3
OF 3 SHEETS