

RE: Model_1 Model 1

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer: David Infinger Project Name: Model_1 Lot/Block: . Model: . Address: . Subdivision: . City: Columbia County State: Fl

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.2 Wind Speed: 130 mph Floor Load: N/A psf

ECOF

This package includes 17 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T16795153	a1ge	4/16/2019
2	T16795154	a2	4/16/2019
3	T16795155	a3	4/16/2019
4	T16795156	a4	4/16/2019
5	T16795157	a5	4/16/2019
6	T16795158	b1ge	4/16/2019
7	T16795159	b2	4/16/2019
8	T16795160	b3ge	4/16/2019
9	T16795161	c1ge	4/16/2019
10	T16795162	c2	4/16/2019
11	T16795163	c3gir	4/16/2019
12	T16795164	d1ge	4/16/2019
13	T16795165	d2	4/16/2019
14	T16795166	e1ge	4/16/2019
15	T16795167	e2	4/16/2019
16	T16795168	e3gir	4/16/2019
17	T16795169	f1	4/16/2019



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc under my direct supervision

based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021. Florida COA: 6634

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MiTek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

Albani, Thomas

April 16, 2019



Plate Offsets (X,Y)	[2:0-4-0,0-2-1], [3:0-0-0,0-1-15], [21:0-0- , [38:0-3-8,Edge]	0,0-1-15], [22:0-4-0,0-2-	1], [24:0-0-13,0-1-9], [24:0-3	3-8,Edg	ge], [29:0	-2-8,0-3-0], [33:0-2-8,0-3-0], [38:0-0	-13,0-1-9]
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.06 BC 0.03 WB 0.15 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 23 23 22	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 199 lb	GRIP 244/190 FT = 0%
BOT CHORD 2x4 SF OTHERS 2x4 SF REACTIONS. All b		-	BRACING- TOP CHOR BOT CHOR	D S				ectly applied or 6-0-0 c or 10-0-0 oc bracing.	c purlins.
Max L	Jplift All uplift 100 lb or less at joint(s) 2, Grav All reactions 250 lb or less at joint(24, 22				26, 25,				

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=32ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 32, 33, 34, 35, 36, 37, 30, 29, 28, 27, 26, 25, 22

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

6904 Parke East Blvd. Tampa, FL 36610



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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L	5-9-10 I 10-11-5 5-9-10 5-1-11		16-1-0		21-2-11		<u>26-4-6</u> 5-1-11		32-2-0	
1			5-1-11	5-1-11		5-9-10				
Plate Offsets (X,Y)	[2:0-5-5,0-0-1], [3:0-2-8,0)-3-0], [7:0-2-8,0-	3-0], [8:0-5-5,0-0-1], [10:0	-3-0,0-4-8], [12:0)-3-0,0-	4-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC 0.46 BC 0.79 WB 0.51	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.36 -0.71 0.48	(loc) 11 11 8	l/defl >999 >540 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
3CDL 10.0	Code FBC2017/T	PI2014	Matrix-AS						Weight: 179 lb	FT = 0%
BOT CHORD 2x6 S	P No.2 P No.2 P No.2			BRACING- TOP CHOR BOT CHOR				sheathing dire	ectly applied.	

REACTIONS. (Ib/size) 8=1286/0-5-8, 2=1348/0-5-8 Max Horz 2=147(LC 11) Max Uplift 2=-25(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-4455/1007, 3-4=-3827/842, 4-5=-2924/631, 5-6=-2924/631, 6-7=-3831/844,
- 7-8=-4473/1017
- BOT CHORD 2-13=-848/4056, 12-13=-852/4069, 11-12=-588/3471, 10-11=-590/3474, 9-10=-860/4085, 8-9=-857/4074
- WEBS 5-11=-402/2255, 6-11=-859/316, 6-10=-20/376, 7-10=-613/262, 4-11=-856/314, 4-12=-19/375, 3-12=-600/255

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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	5-9-10 5-9-10	<u>10-11-5</u> 5-1-11	<u> </u>	21-2-11		26-4-6 5-1-11	32-2-0		
late Offsets (X,Y)	[2:0-5-5,0-0-1], [3:0-2-8,0				0-4-8]	9-1-11	5-5-10		1.1.1
OADING (psf) CLL 20.0 CDL 10.0 SCLL 0.0 SCLL 10.0 SCLL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.45 BC 0.78 WB 0.51 Matrix-AS	DEFL. Vert(LL) -0.3 Vert(CT) -0.7 Horz(CT) 0.4	1 12 >541	240 180	PLATES MT20 Weight: 181 lb	GRIP 244/190 FT = 0%	
OT CHORD 2x6 S	P No.2 P No.2 P No.2		K INSTE	BRACING- TOP CHORD BOT CHORD	Structural woo Rigid ceiling d	d sheathing dire rectly applied.	actly applied.	1 1 1 1 1 1	2590 - 14 - 14 - 14
	ze) 2=1347/0-5-8, 8=134 Horz 2=-150(LC 10) Uplift 2=-24(LC 12), 8=-24								
	. Comp./Max. Ten All for 4451/974, 3-4=-3823/815			23/817					

7-8=-4451/984

BOT CHORD	2-14=-797/4053, 13-14=-802/4065, 12-13=-545/3467, 11-12=-546/3467, 10-11=-812/4065,
	8-10=-808/4053
WEBS	5-12=-380/2252, 6-12=-856/314, 6-11=-19/375, 7-11=-601/257, 4-12=-856/314,
	4-13=-19/375, 3-13=-601/258

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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			20-0-0					
Plate Offsets (X,Y)	[2:0-4-0,0-2-1], [3:0-0-0,0-1-15], [13:0-0	-0,0-1-15], [14:0-4-0,0-2-1		-2-8,0-3-	0], [24:0-1-8,1-1-2]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 • BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2017/TPI2014	CSI. TC 0.06 BC 0.03 WB 0.04 Matrix-S	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	0 15	l/defl L/d n/r 120 n/r 120 n/a n/a	PLATES MT20 Weight: 106 lb	GRIP 244/190 FT = 0%	
BOT CHORD 2x4	SP No.2 SP No.2 SP No.2	¥С н	BRACING- TOP CHORD BOT CHORD			irectly applied or 6-0-0 o or 10-0-0 oc bracing.	c purlins.	1 326

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=-85(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 23, 19, 18, 17

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 21, 22, 23, 24, 19, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 23, 19, 18, 17.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.15	Vert(LL) -0.02 16-17 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.26	Vert(CT) -0.04 16-17 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.33	Horz(CT) 0.01 15 n/a n/a	
BCDL 10.0	Code FBC2017/TPI2014	Matrix-AS		Weight: 178 lb FT = 0%
UMBER-			BRACING-	
	P No.2		TOP CHORD Structural wood sheath	ing directly applied
BOT CHORD 2x4 SI			BOT CHORD Rigid ceiling directly ap	

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. All bearings 6-10-8 except (jt=length) 2=0-5-8.

(lb) - Max Horz 2=142(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13

Max Grav All reactions 250 lb or less at joint(s) 10, 10, 13, 14, 12 except 2=623(LC 1), 15=850(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-707/132, 3-5=-319/133, 5-6=-303/140

BOT CHORD 2-17=-4/586, 16-17=-4/586

WEBS 3-16=-475/168, 6-16=-6/440, 6-15=-735/156

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

April 16,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, terection and bracing of trusses and truss systems, see **ANS/TFH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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MiTek



Design valid for use only with MiTeK9 connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safey information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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BRACING-

TOP CHORD

BOT CHORD

N	0	Т	Ē	s	

BCDL

LUMBER-

OTHERS REACTIONS.

TOP CHORD

BOT CHORD

10.0

(lb) -

2x4 SP No.2

2x4 SP No.2

2x4 SP No.2

All bearings 15-0-0. Max Horz 2=-98(LC 10)

1) Unbalanced roof live loads have been considered for this design.

Code FBC2017/TPI2014

Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 18, 19, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Max Grav All reactions 250 lb or less at joint(s) 2, 12, 17, 18, 19, 20, 16, 15, 14

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 18, 19, 16, 15.



Weight: 85 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 0%

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MiTek



- right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 16, 13, 12.



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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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			remem			Carbon and			100000	7.12		
OADING		SPACING-	2-0-0	CSI.	120222	DEFL.	in		I/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.25	TC	0.19	Vert(LL)	0.02	6-12	>999	240	MT20	244/190
	10.0	Lumber DOL	1.25	BC	0.19	Vert(CT)	-0.02	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	K-AS	22 12					Weight: 32 lb	FT = 0%
BOT CHOR MEBS	2x4 S	P No.2 P No.2 ze) 2=380/0-3-8, 4=380/	0-3-8			BOT CHOR		i tigiti oc		ctly applied.		
		Horz 2=-41(LC 10)										
		Uplift 2=-94(LC 12), 4=-94	(IC 12)									

NOTES-

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