Julius Lee

RE: 327272 - RICHARD KEEN - GLENN RES.

1109 Coastal Bay Blvd. **Boynton Beach, FL 33435**

Site Information:

Project Customer: RICHARD KEEN Project Name: 327272 Model: GLENN RES.

Lot/Block:

Subdivision:

Address: 185 SW ARROWHEAD TER

City: COLUMBIA CTY

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JAMES H. JOHNSTON

License #: CRC1328128

Address: 650 SW MAIN BLVD.

City: LAKE CITY,

State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 52 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	14241488	CJ1	3/4/010	18	14241505	HG13	3/4/010
2	14241489	CJ1A	3/4/010	19	14241506	HG17	3/4/010
3	14241490	CJ3	3/4/010	20	14241507	HG23	3/4/010
4	14241491	CJ3A	3/4/010	21	14241508	HJ2	3/4/010
5	14241492	CJ3B	3/4/010	22	14241509	HJ7	3/4/010
6	14241493	CJ5	3/4/010	23	14241510	HJ9	3/4/010
7	14241494	CJ5A	3/4/010	24	14241511	HJ9A	3/4/010
8	14241495	CJ5B	3/4/010	25	14241512	HJ9B	3/4/010
9	14241496	EJ5	3/4/010	26	14241513	HJ9C	3/4/010
10	14241497	EJ7	3/4/010	27	14241514	T01	3/4/010
11	14241498	EJ7A	3/4/010	28	14241515	T02	3/4/010
12	14241499	EJ7B	3/4/010	29	14241516	T03	3/4/010
13	14241500	EJ8	3/4/010	30	14241517	T04	3/4/010
14	14241501	EJ8A	3/4/010	31	14241518	T05	3/4/010
15	14241502	EJ8B	3/4/010	32	14241519	T06	3/4/010
16	14241503	EJ8C	3/4/010	33	14241520	T07	3/4/010
17	14241504	HG03	3/4/010	34	14241521	T08	3/4/010

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

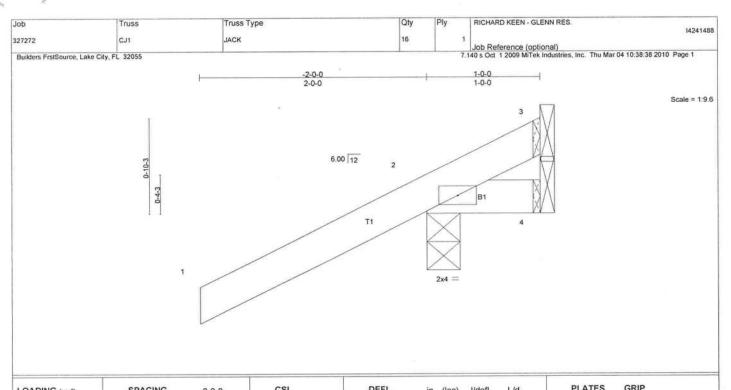
My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



1 of 4

Julius Lee



BCLL BCDL	0.0 * 5.0	Rep Stress Incr Code FBC2007/TI	YES PI2002	(Mat	0.00 rix)	Horz(TL) Wind(LL)	0.00	2	n/a >999	n/a 240	Weight: 7 lb	·
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.00	2	>999	360	MT20	244/190
LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/a	FLATES	GRIF

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

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REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=109(LC 6)

Max Uplift 2=-371 (LC 6), 4=-12 (LC 4), 3=-90 (LC 1) Max Grav 2=256 (LC 1), 4=14 (LC 2), 3=163 (LC 6)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES (8-9)

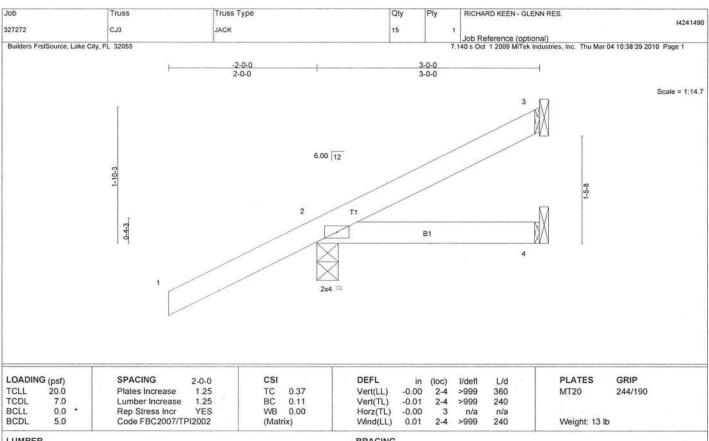
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2, 12 lb uplift at joint 4 and 90 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

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STATE OF



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

LICENSE

No 34869

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=166(LC 6)

Max Uplift3=-36(LC 7), 2=-311(LC 6), 4=-36(LC 4) Max Grav 3=31(LC 1), 2=250(LC 1), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 311 lb uplift at joint 2 and 36 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

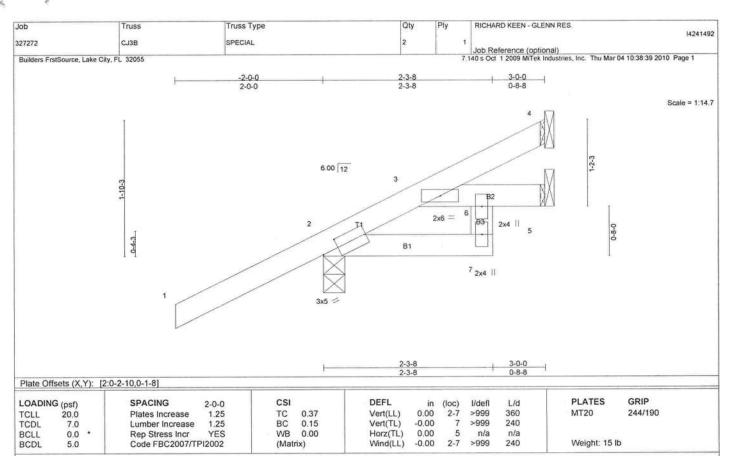
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

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🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fluss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the suiting construction is the responsibility of the event of the designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult — ANSI/TPI Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except*

B3: 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=41/Mechanical, 2=253/0-3-8, 5=9/Mechanical

Max Horz 2=166(LC 6)

Max Uplift 4=-45(LC 6), 2=-265(LC 6)

Max Grav 4=41(LC 1), 2=253(LC 1), 5=48(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 4 and 265 lb uplift at joint 2

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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No 34899

STATE OF

FLORIDA

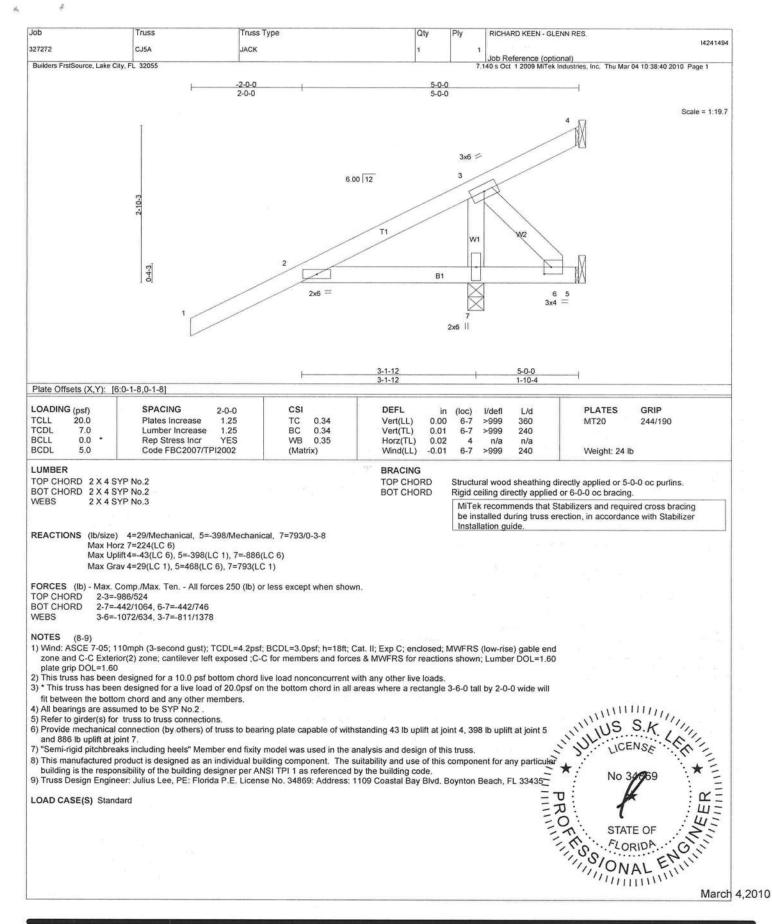
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March 4,2010

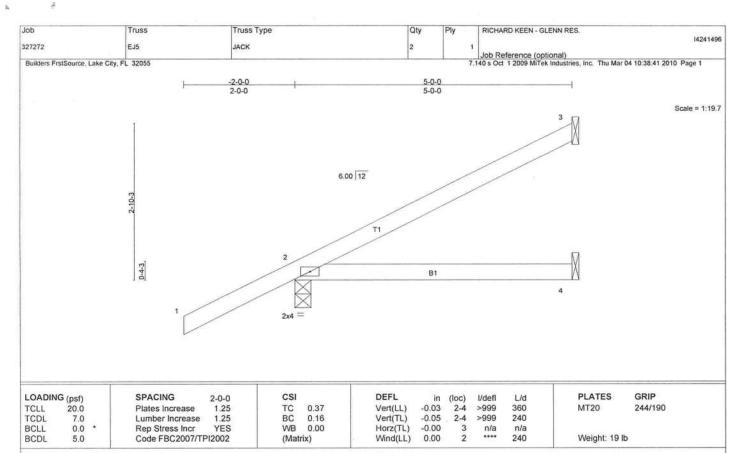
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide,

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REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=224(LC 6)

Max Uplift3=-114(LC 6), 2=-266(LC 6)

Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 3 and 266 lb uplift at joint 2.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

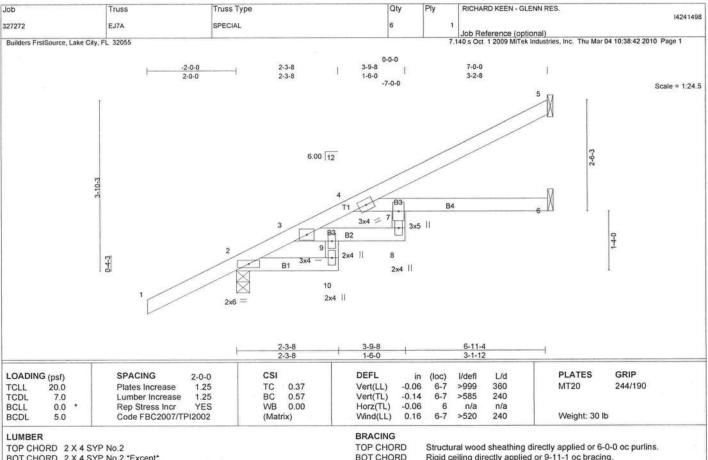
LOAD CASE(S) Standard

March 4,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. NARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER KEERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to instability during construction is the responsibility of the event studies of the control of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TPI Quality control, storage, delivery, erection and bracing, consult.

ANSI/TPI Quality Control. Storage and BCS11 Building Component Safety Information available from Truss Plate Institute, 383 D'Onofito Drive, Madison, WI 53719.



BOT CHORD 2 X 4 SYP No.2 *Except* B3: 2 X 4 SYP No.3

Rigid ceiling directly applied or 9-11-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

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No 34669

FLORIDA

REACTIONS (lb/size) 5=121/Mechanical, 2=357/0-3-8, 6=82/Mechanical

Max Horz 2=203(LC 6)

Max Uplift 5=-83(LC 6), 2=-190(LC 6), 6=-16(LC 6)

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 2 confit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections

6) Provide mechanical consecutions ROFE

and 16 lb uplift at joint 6.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES 14241500 327272 EJ8 MONO TRUSS Job Reference (optional) 140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:43 2010 Page 1 Builders FrstSource, Lake City, FL 32055 4-0-0 4-0-0 -2-0-0 2-0-0 Scale = 1:26.9 6.00 12 2x4 > 0-4-3 3x5 = 3x4 =

Plate Offsets (X,Y): [2:0-2-10,0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-6	>647	360	MT20	244/190
CDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.25	2-6	>367	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	5	n/a	n/a	***************************************	
BCDL	5.0	Code FBC2007/TI	PI2002	(Matr	ix)	Wind(LL)	-0.01	2-6	>999	240	Weight: 35 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WERS

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=92/Mechanical, 2=381/0-3-8, 5=141/Mechanical

Max Horz 2=223(LC 6)

Max Uplift4=-75(LC 6), 2=-198(LC 6), 5=-47(LC 6) Max Grav 4=92(LC 1), 2=381(LC 1), 5=147(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-299/143 BOT CHORD 2-6=-348/224

WEBS 3-6=-251/389

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Extenor(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2

Semi-rigid pitchbreaks including heets" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Rountee B.

LOAD CASE(S) Standard 30

March 4,2010

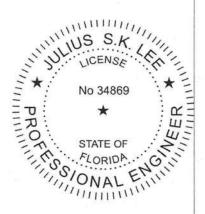
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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	EJ8A	MONO TRUSS	1		1	1424150
					Job Reference (optional)	
Builders FrstSource, L	ake City, FL 32055				7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10	0:38:43 2010 Page 2

Builders FrstSource, Lake City, FL 32055

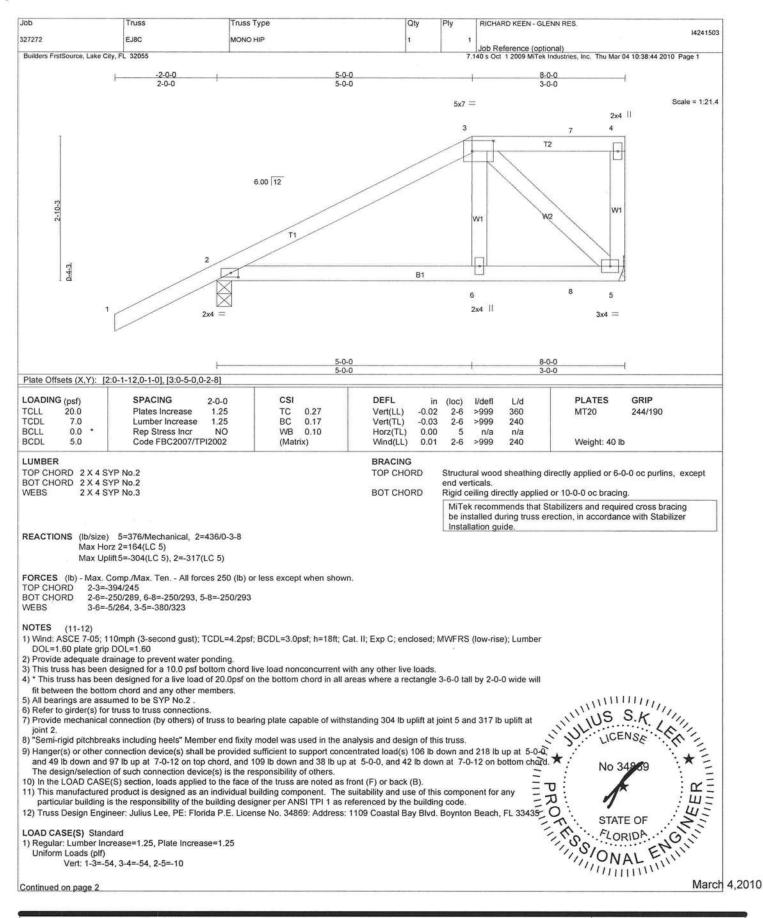
LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 1=-250 5=-826(B) 6=-285 7=-496(B)





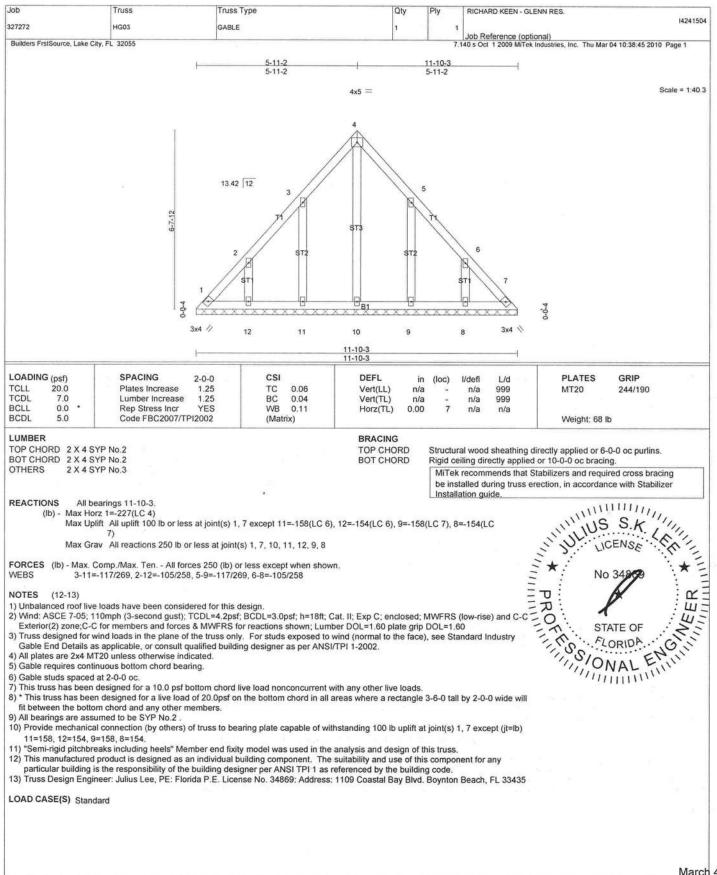
March 4,2010



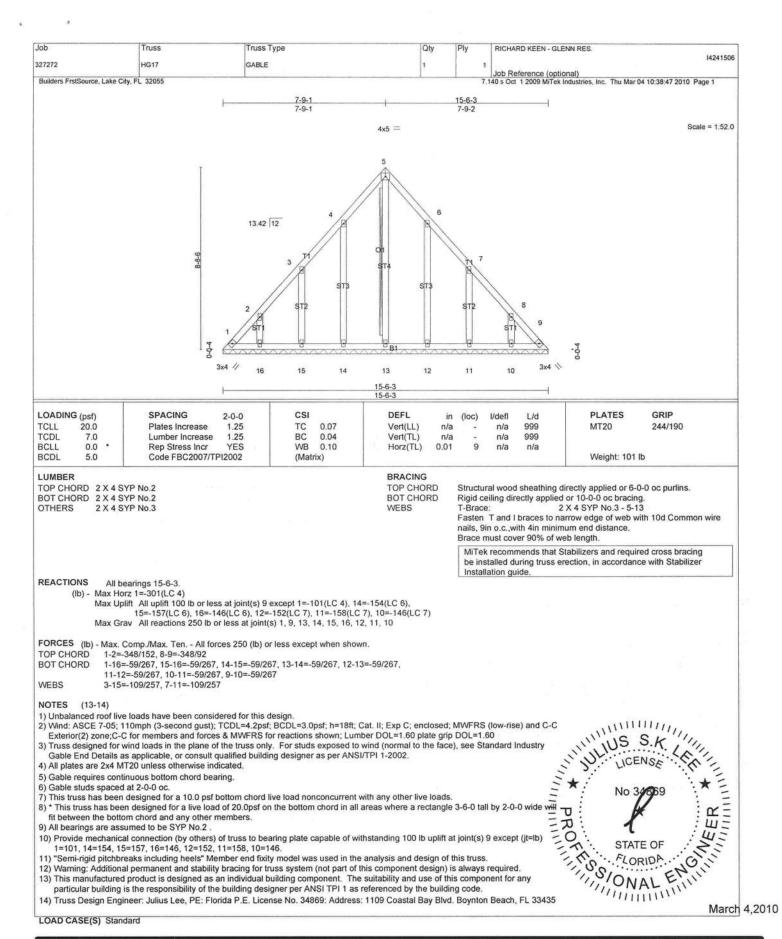
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March 4,2010



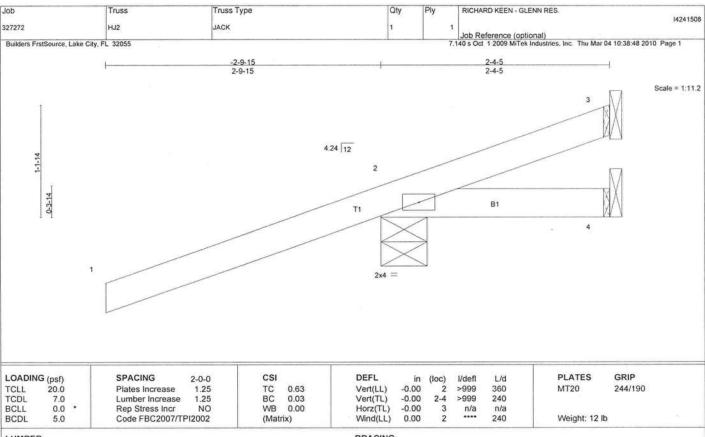
LOAD CASE(S) Standard

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14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

March 4,2010



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-4-5 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide,

No 34

FLORIDA

REACTIONS (lb/size) 3=-68/Mechanical, 2=355/0-5-11, 4=10/Mechanical

Max Horz 2=128(LC 3)

Max Uplift3=-68(LC 1), 2=-444(LC 3) Max Grav 3=124(LC 3), 2=355(LC 1), 4=31(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=444.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

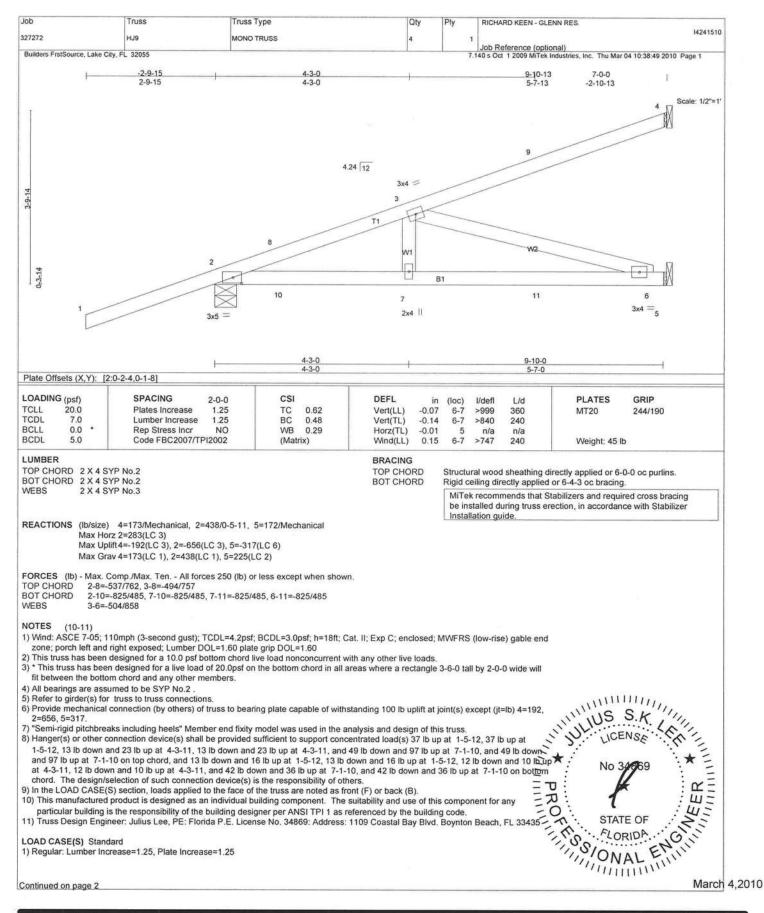
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

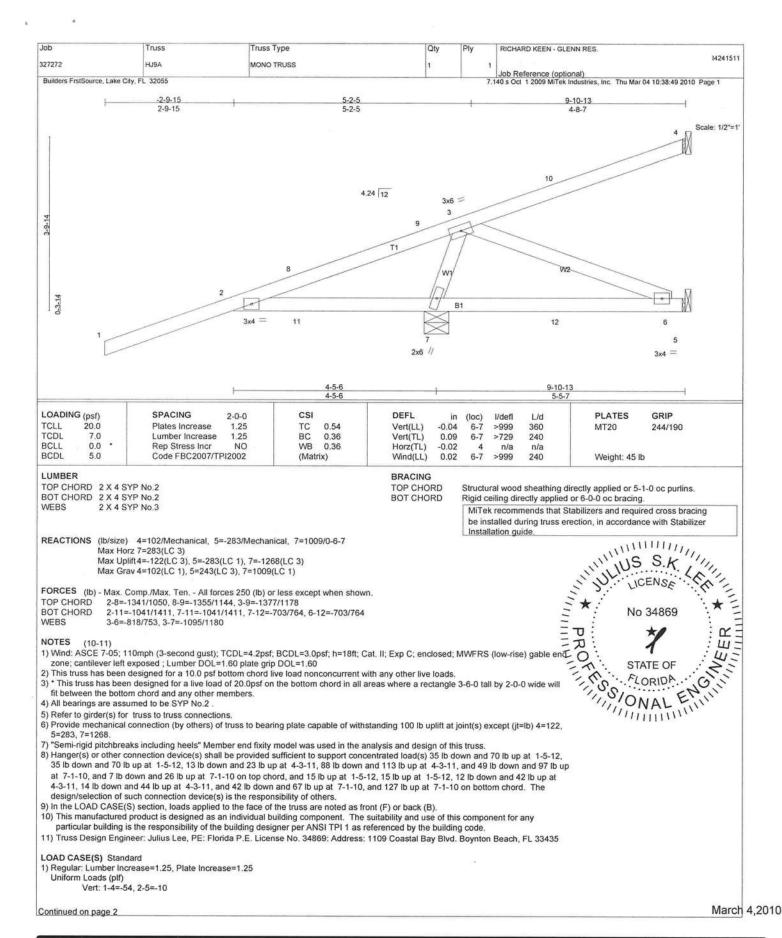
▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to instability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from truss Plate Institute, 583 D'Onofrio Drive, Madison. WI 53719.



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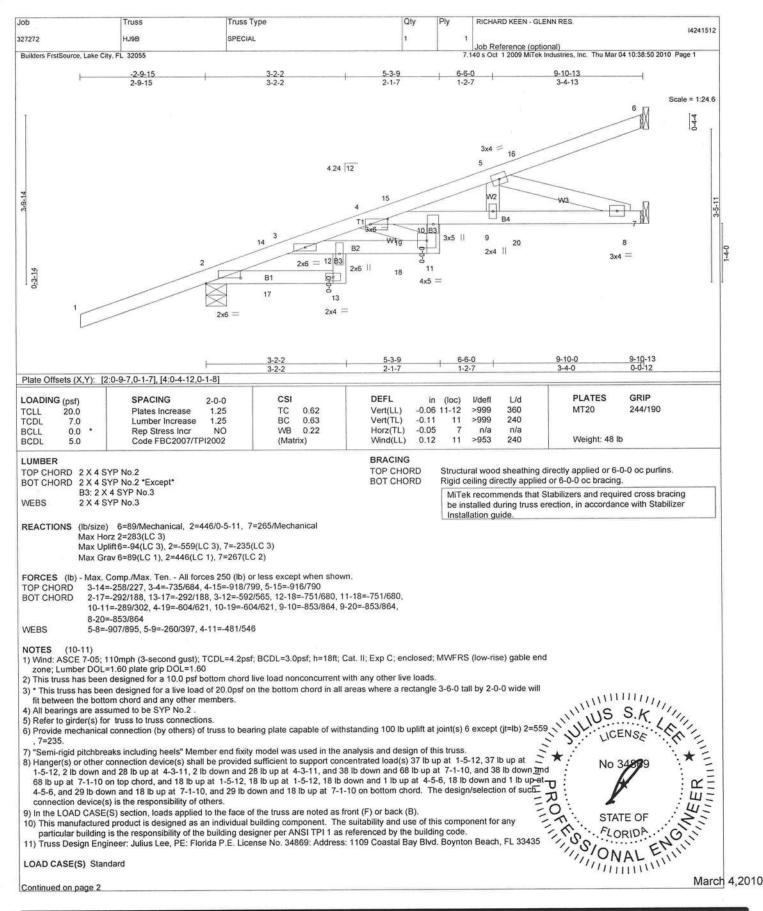
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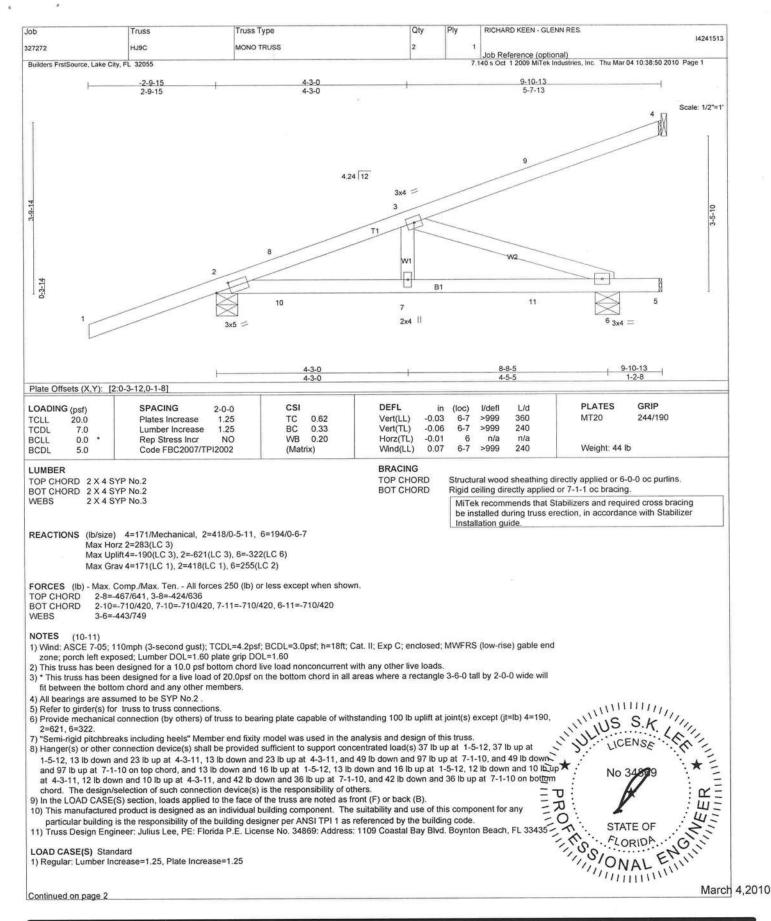
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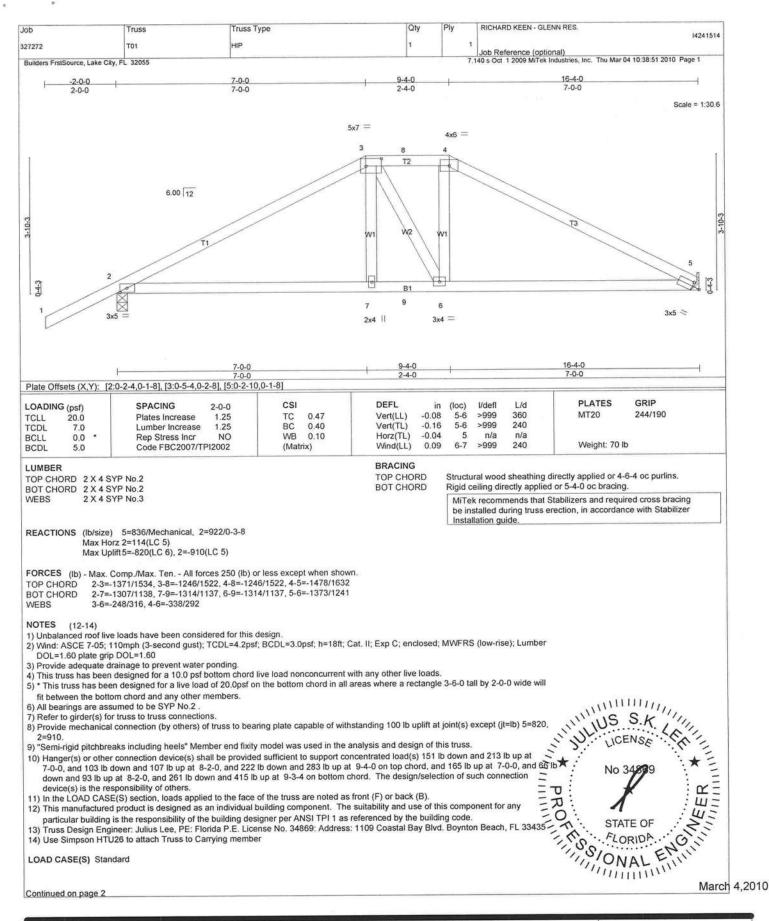
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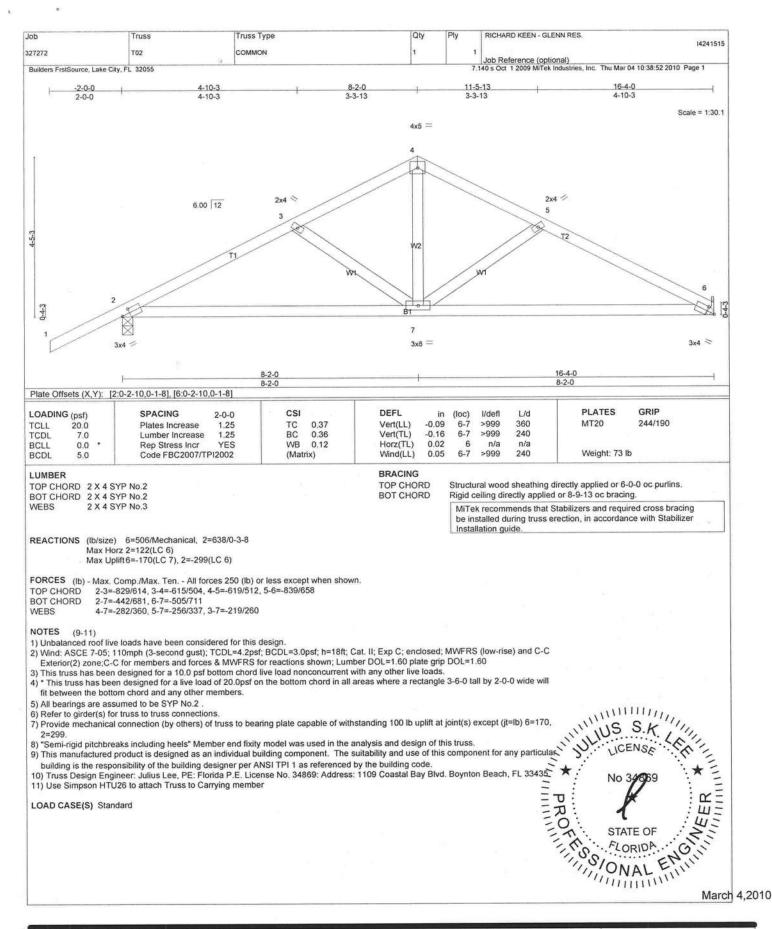
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ob	Truss	Truss Type	Q	-5/61		RICHARD KEEN - GLENN RES.	14241516
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building designe (3) Truss Design E LOAD CASE(S) St (1) Regular: Lumber Uniform Loads (p Vert: 1-3	er per ANSI TPI 1 as referen ngineer: Julius Lee, PE: Floi tandard Increase=1.25, Plate Increa olf) i=-54, 3-5=-54, 5-6=-54, 6-7	ced by the building code. fida P.E. License No. 34869: A se=1.25				ent for any particular building is the respons Beach, FL 33435	ibility of the
Concentrated Los Vert: 14=	ads (lb) =-38(F) 15=-38(F) 16=-1103	(F) 17=-131(F) 18=-131(F) 19	=-221(F) 20=-221(F) 21=-	366(F)			
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March 4,2010

russ Type Qty RICHARD KEEN - GLENN RES. Job Truss 14241518 T05 327272 SPECIAL Job Reference (optional) 7.140 s Oct 1.2009 MiTek Industries, Inc. Thu Mar 04 10:38:54 2010 Page 1 Builders FrstSource Lake City FL 32055 16-0-0 4-0-0 28-4-0 | 30-0-0 | 32-0-0 | 2-0-0 | 1-8-0 | 2-0-0 | 2-0-0 12-0-0 20-11-4 4-11-4 5-4-14 6-7-2 6x8 = 6.00 12 3×4 3 6 3x4 = 4x5 = 1-2-3 10 14 12 13 3x6 = 16 15 3x5 > 3x4 = 3x6 = 2x4 11 3x8 = 3x5 = 1-8-0 Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-1-12,0-3-0], [5:0-4-10,Edge], [9:0-2-10,0-1-8] PLATES GRIP DEFL LOADING (psf) SPACING CSI 2-0-0 in (loc) I/defl L/d TC Vert(LL) -0.14 12-13 >999 360 MT20 244/190 TCLL 20.0 Plates Increase 1 25 0.50 -0.31 12-13 BC 0.65 >999 240 1.25 Vert(TL) TCDL 7.0 Lumber Increase 9 n/a BCLL 0.0 Rep Stress Incr YES WB 0.42 Horz(TL) 0.09 n/a 0.21 12-13 >999 240 Weight: 164 lb BCDL 5.0 Code FBC2007/TPI2002 Wind(LL) LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins, except TOP CHORD 2 X 4 SYP No.2 2-0-0 oc purlins (5-2-3 max.): 4-5, 7-8. BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 4-3-8 oc bracing. BOT CHORD WEBS 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 2=1067/0-3-8, 9=1067/0-3-8 Max Horz 2=134(LC 6) Max Uplift 2=-421(LC 6), 9=-437(LC 7) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1779/1266, 3-4=-1340/1042, 4-5=-1167/1042, 5-6=-1357/1090, 6-7=-2179/1577, 7-8=-1473/927, 8-9=-1605/1001 TOP CHORD 2-16=-930/1526, 15-16=-930/1526, 14-15=-543/1133, 13-14=-543/1133, **BOT CHORD** 12-13=-1037/1663, 11-12=-2123/3149, 9-11=-686/1350 3-15=-440/422, 4-15=-137/264, 5-13=-277/358, 6-13=-616/590, 6-12=-263/500, WEBS 7-12=-1279/984, 7-11=-1855/1496, 8-11=-527/752 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C 6) All bearings are assumed to be SYP No.2.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=42 in the particular building heels" Member end fixity model was used in the analysis and design of this truss.

8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss To wild an analysis and design of this truss. Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 7) Provide meuraning 9=437.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was 10 km 2-1.

9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss 10 km 2-1.

10) This manufactured product is designed as an individual building component. The suitability and use of this component or particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

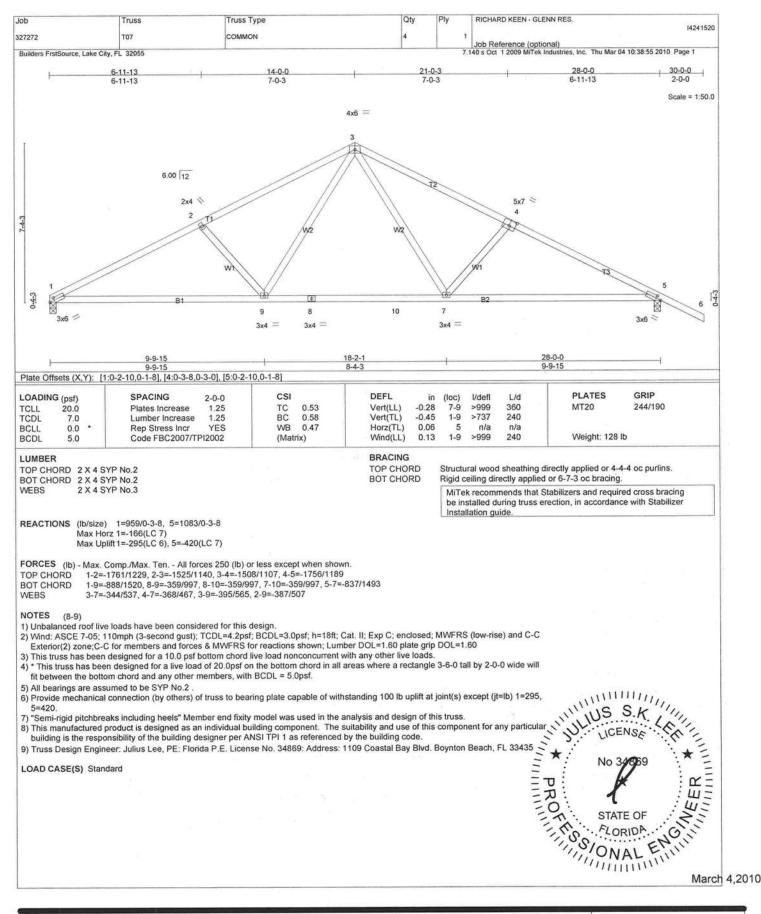
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435- ON STATE OF Standard

Standard March 4,2010

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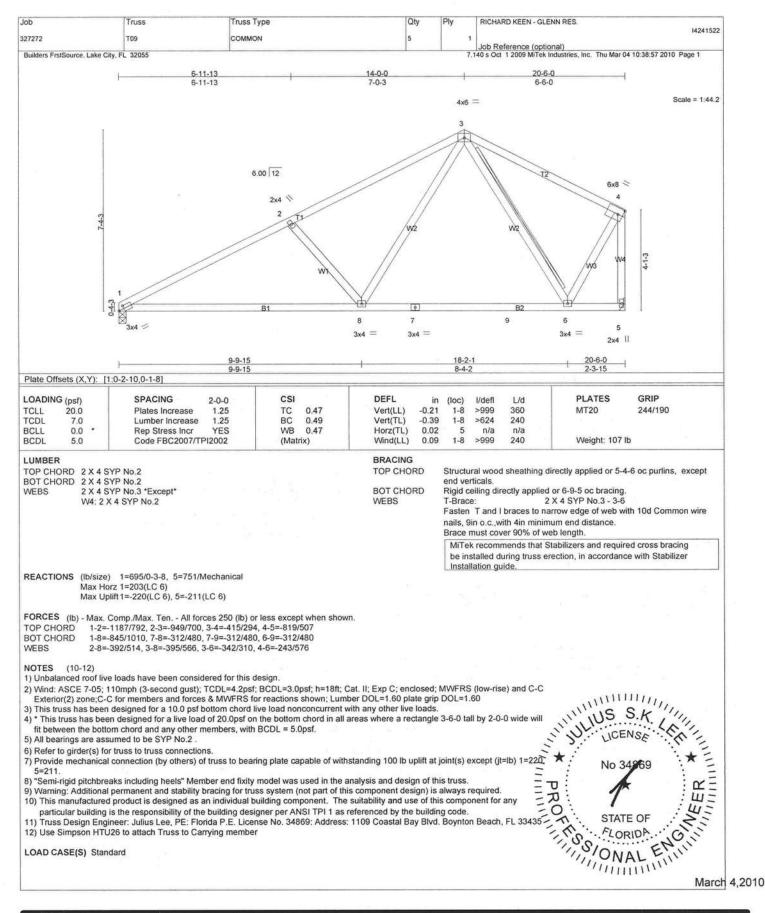
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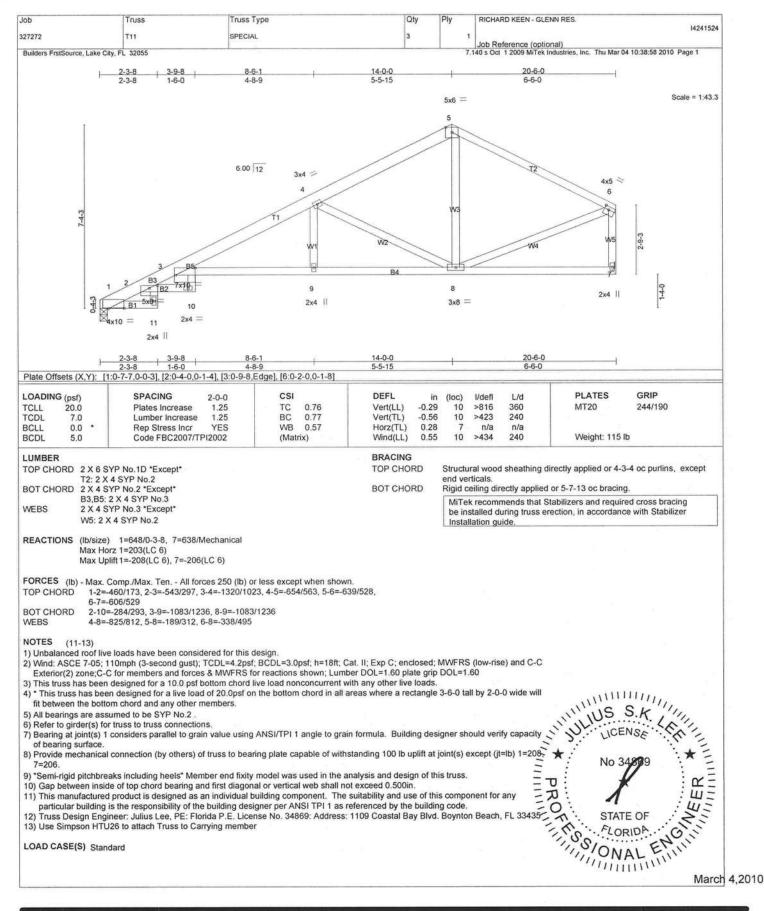
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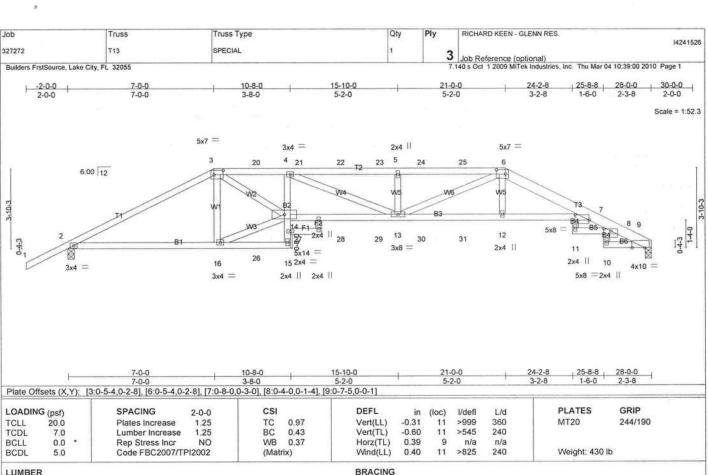
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TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

T3: 2 X 6 SYP No.1D 2 X 4 SYP No.2 *Except*

BOT CHORD B2,B4: 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1730/0-3-8, 2=1818/0-3-8

Max Horz 2=114(LC 5)

Max Uplift 9=-947(LC 6), 2=-1301(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-3287/2357, 3-20=-5710/3872, 4-20=-5709/3872, 4-21=-5968/3628, TOP CHORD

21-22=-5968/3628, 22-23=-5968/3628, 5-23=-5968/3628, 5-24=-5966/3626,

24-25=-5968/3626, 6-25=-5970/3627, 6-7=-4729/2709, 7-8=-722/452, 8-9=-948/538 2-16=-2085/2836, 4-14=-561/208, 14-27=-3870/5778, 27-28=-3870/5778

28-29=-3870/5777, 13-29=-3870/5777, 13-30=-2587/4579, 30-31=-2587/4579, 12-31=-2587/4579, 7-12=-2577/4558

3-16=-828/404, 14-16=-2157/2911, 3-14=-2062/3416, 4-13=0/334, 5-13=-529/338,

WEBS 6-13=-1077/1566, 6-12=-157/455

NOTES (14-15)

BOT CHORD

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

8) All bearings are assumed to be SYP No.2

9) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=947,

11) "Pin all pitchbreaks" Member end fixity model was used in the analysis and design of this truss.

12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. Continued on page 2

No 34299

ROSTATE OF

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MOIN

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

10-0-0 oc bracing: 14-15

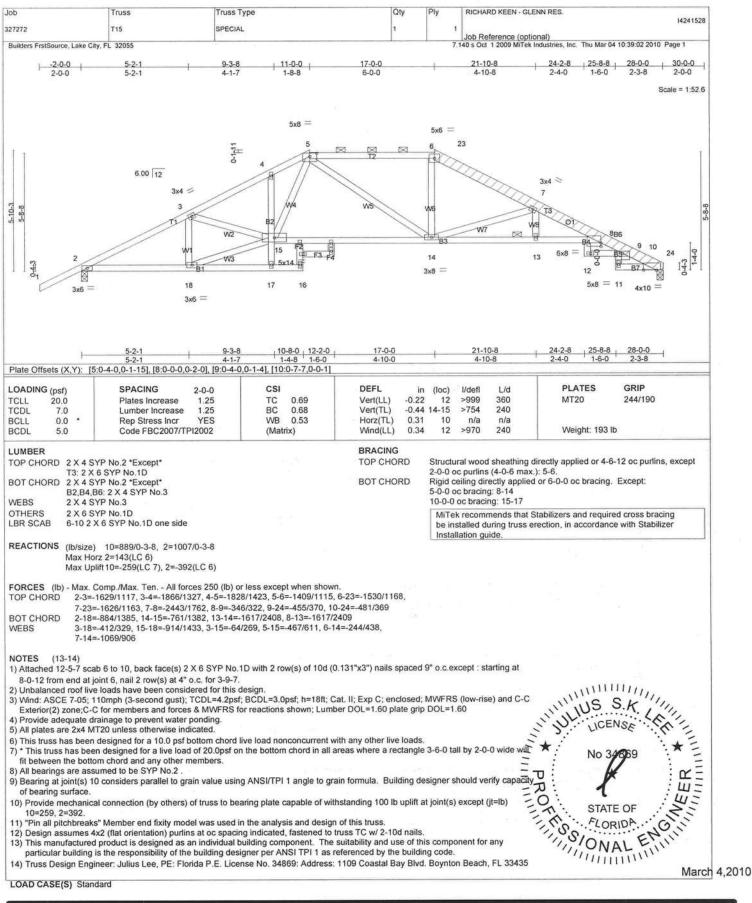
March 4,2010

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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES 14241527 327272 T14 SPECIAL Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:01 2010 Page 1 Builders FrstSource, Lake City, FL 32055 30-0-0 14-0-0 9-0-0 19-0-0 22-6-0 24-2-8 | 25-8-8 | 28-0-0 5-0-0 1-8-8 1-6-0 5-0-0 2-0-0 4-9-4 4-2-12 4x6 || 3x8 = 5x6 = 23 152 6.00 12 3x4 = 3 -10-3 4-8-8 W6 13 B7 1 3x8 = 12 5x8 = 11 18 17 16 4x10 = 3x6 = 3x6 = 10-8-0 12-2-0 14-0-0 1-4-8 1-6-0 1-10-0 4-6-4 Plate Offsets (X,Y): [8:0-0-0,0-2-0], [9:0-4-0,0-1-4], [10:0-7-8,0-0-5] PLATES GRIP LOADING (psf) CSI DEFL SPACING 2-0-0 in (loc) I/defl L/d TC 0.63 Vert(LL) -0.22 13-14 >999 360 MT20 244/190 TCII 20.0 Plates Increase 1.25 -0.43 240 1.25 BC 0.59 Vert(TL) 8-13 >762 TCDL 7.0 Lumber Increase 0.46 0.32 n/a BCLL 0.0 Rep Stress Incr YES WB Horz(TL) 10 n/a Code FBC2007/TPI2002 Wind(LL) 0.33 12 >996 240 Weight: 184 lb BCDL (Matrix) BRACING LUMBER Structural wood sheathing directly applied or 4-4-8 oc purlins, except TOP CHORD 2 X 4 SYP No.2 *Except* TOP CHORD 2-0-0 oc purlins (4-6-12 max.): 4-6. T3: 2 X 6 SYP No.1D BOT CHORD 2 X 4 SYP No.2 *Except **BOT CHORD** Rigid ceiling directly applied or 5-9-2 oc bracing. Except: B2,B4,B6: 2 X 4 SYP No.3 4-8-0 oc bracing: 8-13 2 X 4 SYP No.3 5-10-0 oc bracing: 14-15 WEBS 2 X 6 SYP No.1D 10-0-0 oc bracing: 15-17 OTHERS 6-10 2 X 6 SYP No.1D one side LBR SCAB MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 10=889/0-3-8, 2=1007/0-3-8 Max Horz 2=128(LC 6) Max Uplift 10=-243(LC 7), 2=-375(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1644/1110, 3-4=-1889/1297, 4-5=-1691/1243, 5-6=-1687/1261, 6-23=-1807/1316, TOP CHORD 7-23=-1867/1309, 7-8=-2753/1980, 8-9=-346/317, 9-24=-455/365, 10-24=-481/364 **BOT CHORD** 2-18=-883/1401, 4-15=-382/621, 14-15=-1206/2005, 13-14=-1206/2005, 8-13=-1856/2745 3-18=-375/301, 15-18=-903/1433, 3-15=-96/331, 5-15=-469/295, 5-13=-473/270, WEBS 6-13=-361/572, 7-13=-1135/936 cc JULIUS S.K NOTES (13-14) 1) Attached 10-2-10 scab 6 to 10, back face(s) 2 X 6 SYP No.1D with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 5-9-14 from end at joint 6, nail 2 row(s) at 4" o.c. for 3-9-7. 2) Unbalanced roof live loads have been considered for this design. 3) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. 5) All plates are 2x4 MT20 unless otherwise indicated. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. No 3 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide with fit between the bottom chord and any other members. U 8) All bearings are assumed to be SYP No.2. 9) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity 70 of bearing surface. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1G 10=243, 2=375. 11) "Pin all pitchbreaks" Member end fixity model was used in the analysis and design of this truss 12) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails. 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. Continues Designe Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 March 4,2010

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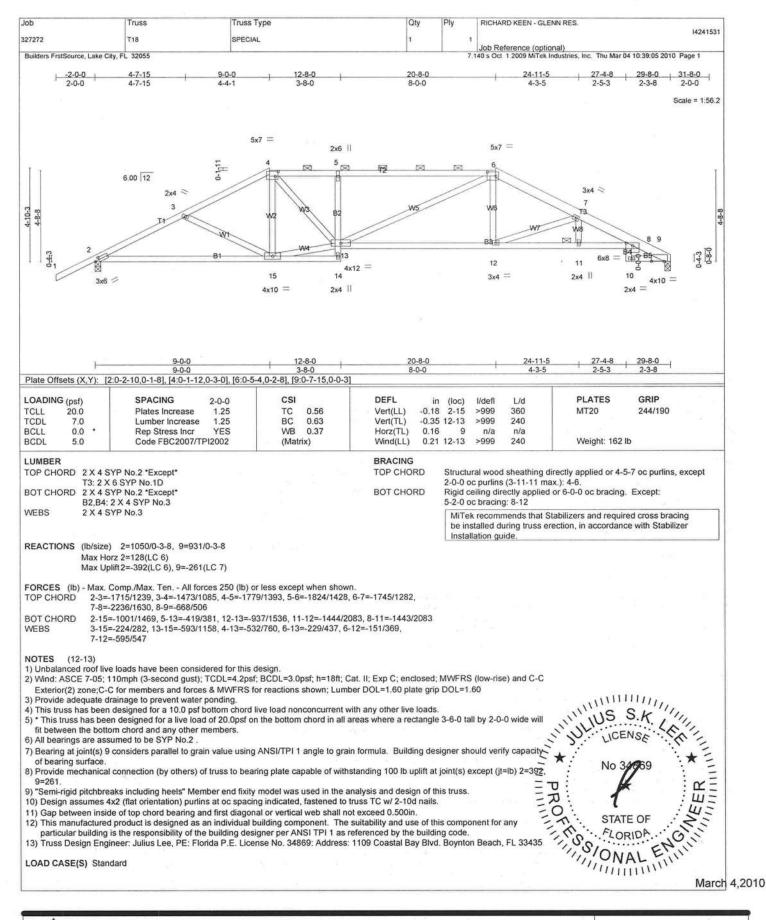
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES 14241530 327272 T17 HIP Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:04 2010 Page 1 Builders FrstSource, Lake City, FL 32055 12-2-12 5-2-12 17-5-4 22-8-0 29-8-0 31-8-0 7-0-0 7-0-0 2-0-0 Scale = 1:56.2 4x6 = 4x6 = 3x4 = 4 14 16 5 18 6 6.00 12 3-10-3 0-4-3 21 10 22 23 24 19 20 9 11 5x6 = 5x6 = 6x10 = 3x6 = 3x6 = 7-0-0 7-0-0 7-10-0 Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge], [10:0-5-0,0-3-4] LOADING (psf) PLATES GRIP SPACING CSI DEFL in (loc) 2-0-0 I/defl L/d Vert(LL) -0.29 10-11 360 MT20 244/190 Plates Increase 1.25 TC 0.64 >999 TCLL 20.0 0.93 Vert(TL) -0.61 10-11 >574 240 TCDL 7.0 Lumber Increase 1.25 BC NO WB 0.88 Horz(TL) 0.15 n/a n/a BCLL 0.0 Rep Stress Incr Code FBC2007/TPI2002 (Matrix) Weight: 137 lb Wind(LL) 0.55 10-11 >636 240 BCDL 5.0 BRACING LUMBER Structural wood sheathing directly applied or 2-11-2 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 3-8-1 oc bracing BOT CHORD 2 X 4 SYP No.1D 2 X 4 SYP No.3 WEBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 2=1797/0-3-8, 7=1734/0-3-8 Max Horz 2=97(LC 5) Max Uplift2=-1329(LC 5), 7=-1236(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3314/2478, 3-12=-2904/2270, 12-13=-2903/2270, 4-13=-2903/2270, 4-14=-4007/3137, 14-15=-4007/3137, 15-16=-4007/3137, 5-16=-4007/3137, 15-16=-4007/3137, TOP CHORD 5-17=-2785/2111, 17-18=-2785/2111, 6-18=-2786/2111, 6-7=-3178/2296 2-11=-2157/2862, 11-19=-2891/3814, 19-20=-2891/3814, 20-21=-2891/3814, **BOT CHORD** 10-21=-2891/3814, 10-22=-2813/3774, 22-23=-2813/3774, 23-24=-2813/3774, 9-24=-2813/3774, 7-9=-1953/2742 WEBS 3-11=-803/1031, 4-11=-1189/940, 4-10=-320/434, 5-10=-411/454, 5-9=-1284/1068, 6-9=-877/1086 NOTES (11-12)STATULUS S.K 1) Unbalanced roof live loads have been considered for this design. 2) Wind; ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SYP No.2 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1329, No 34869 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 150 lb down and 223 lb up at 7-10-0, SIN 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). March 4,2010 Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with Milek connectors, This design is based only upon parameters shown, and is for an individual building component.
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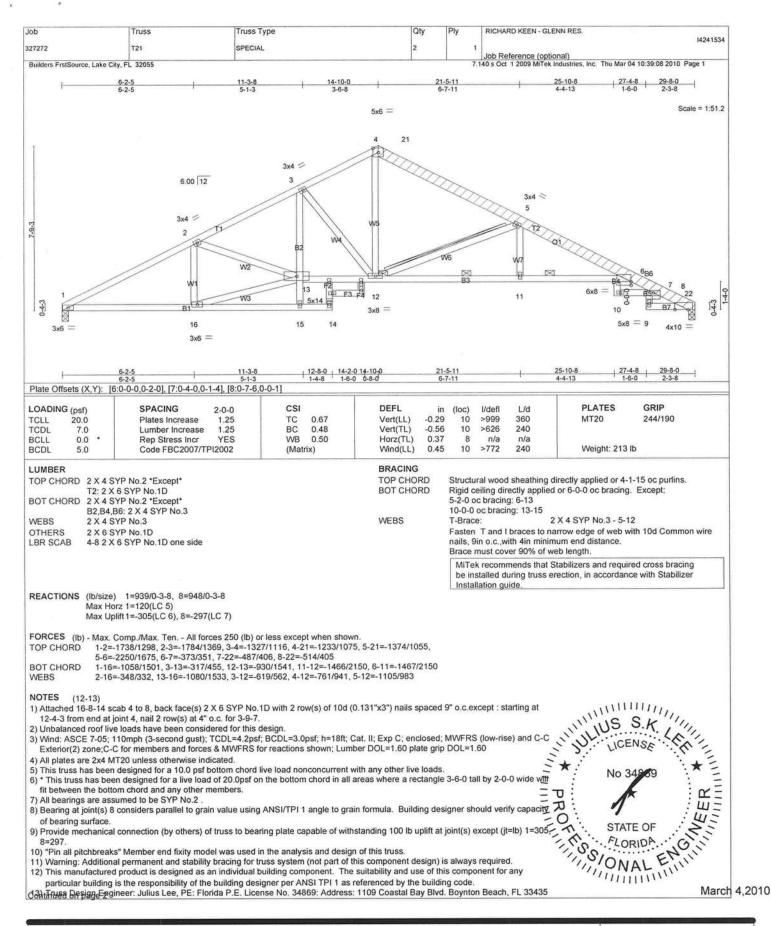
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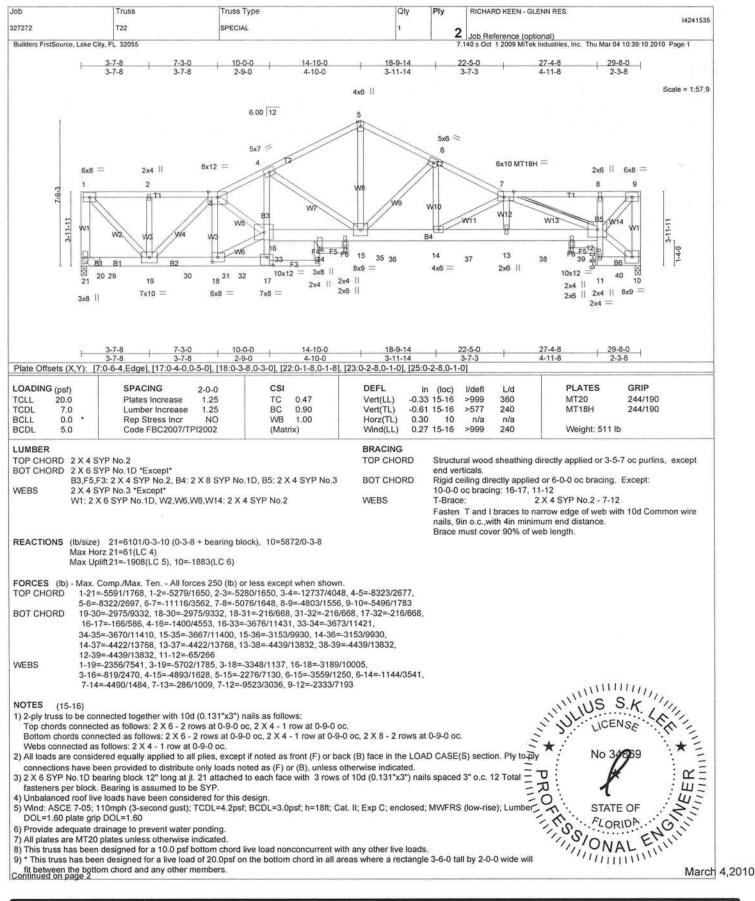
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	(10.107
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Builders FrstSource, Lake City, F	L 32055			7,1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:06 201	0 Page 2
LOAD CASE(S) Standard	1					
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March 4,2010



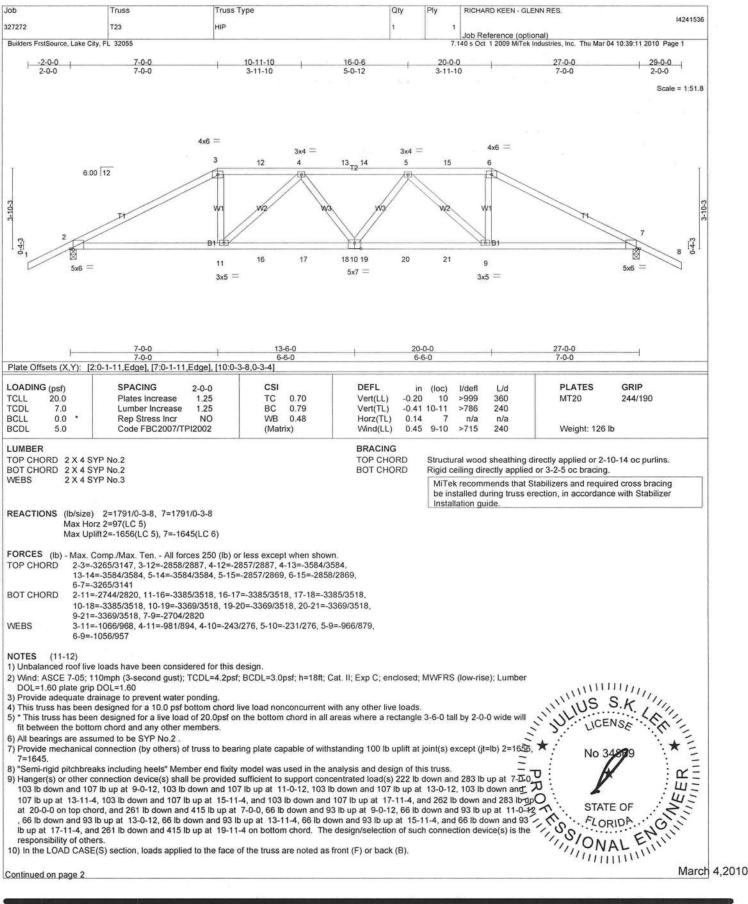
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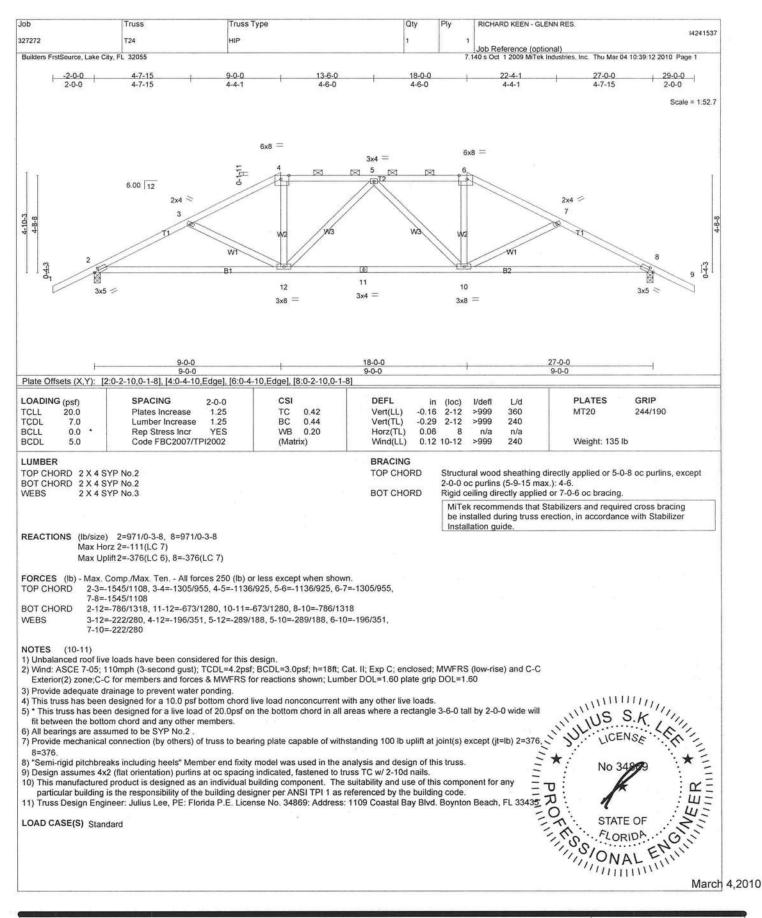
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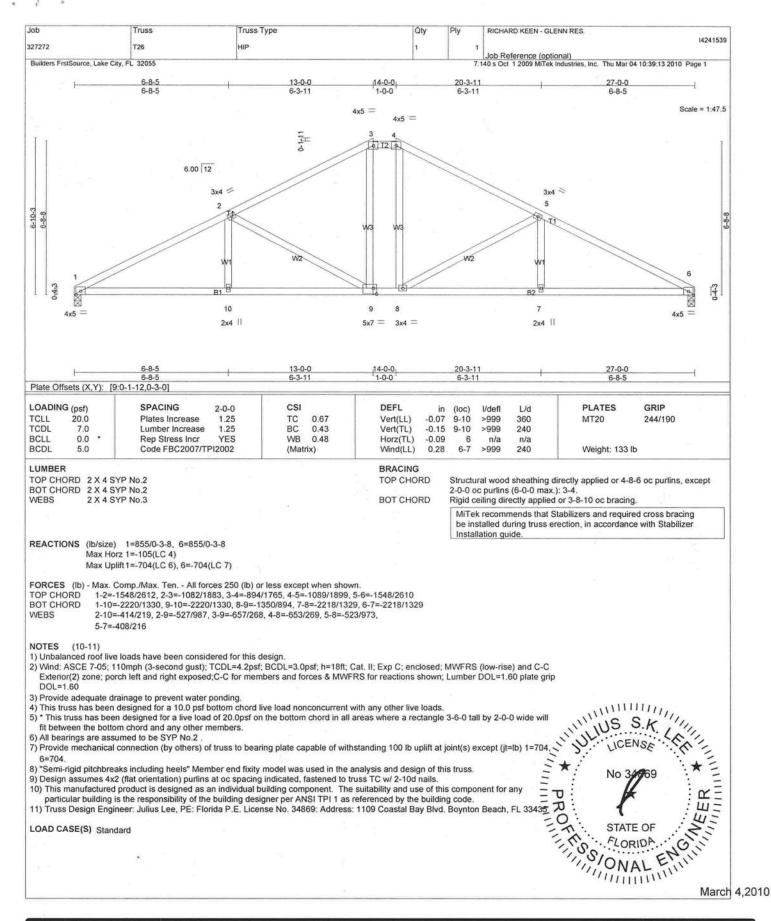
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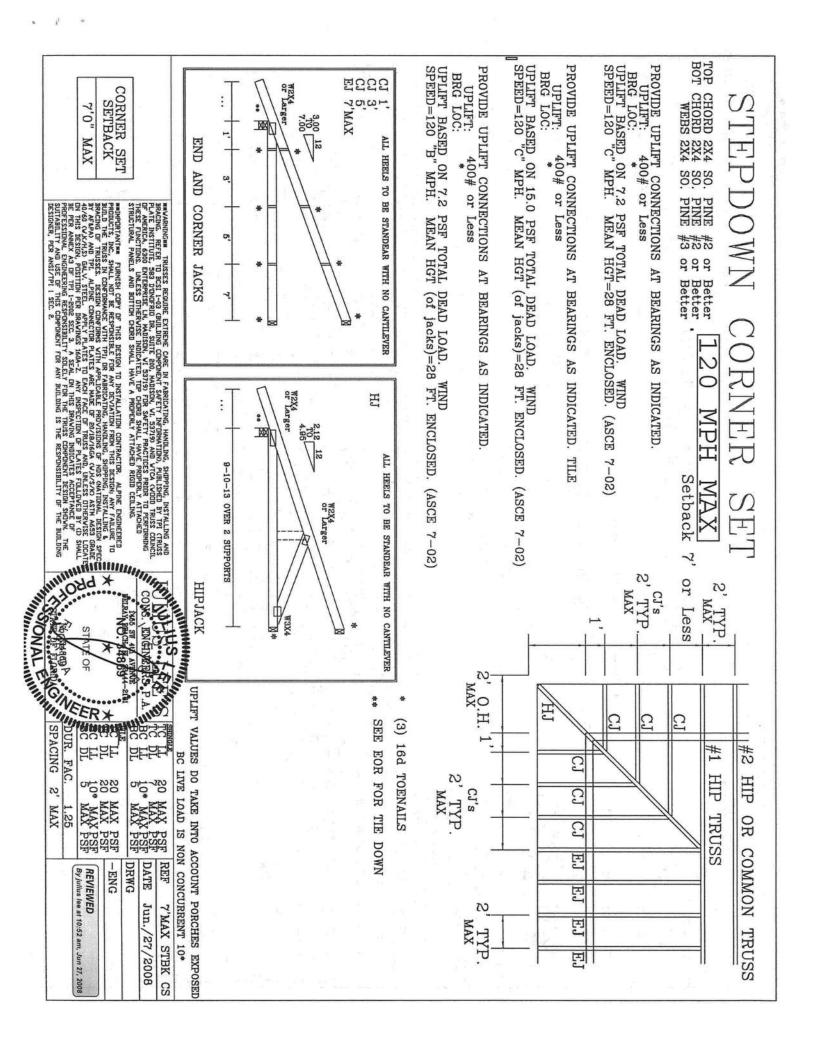
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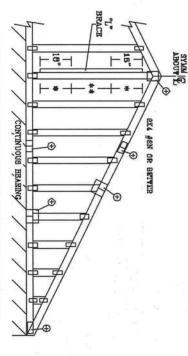
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ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE 0

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	LFL	1		j	TIL	ij	UT	ביו	ļ	<u> </u>	1	C)		TIT		ひてに	1	t			(A)		TIT	I I	ひて	1	SPACING SPECIES	CARIE VERTICAL	
STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	≱ 1 / #8	STANDARD	STUD	43	#2	+1	STANDARD	CUIS	8	\$1 / #2	STANDARD	STUD	£ 3	#23	14	STANDARD	STUD	8	£1 / #2	GRADE	BRACE	
4. 0.		4 22	4' 4"	4 5"	8' 11"		3' 11"		3' 8"			3' 11°		8. 7.	8, 5,			3' 0"		3,	3' 6"	3' 6"		8' 1"	3' 1"	e a	BRACES	NO.	
5. 6		8 6	6' 11"	8 11°	5' 4"	6' 3"	8' 3"	6' 11"	4' 9"	5' 6"	6, 5,	8' 4"	8' 4"	4' 8"	B' 6"	5	6 4	8' 10"		4. 6.		5' 6"		4' 6"	4' 5"	5 6,	GROUP A	(1) 1X4 °L"	
5 8		6, 20	7' 6"	7' 8"	5' 4"	6. 3.	8, 3,	7. 8.	4' 9°	5' 8"	6. 7.	8' 10"	B' 10"	4. B.	6' 6"	5' 5"	6. 6.	8' 10"	4' 8"	4' 6"	6' 11"	5' 11"	3' 9"	4' 5"	4' 5"	6, 8,	GROUP B	L" BRACE .	
7 3	8 3	8, 3,	B' 3°	8, 3,	7' 1"	B 3"	e' 3"	8' 3"	6' 3"	7' 3"	7. 4.	7' 8"	7' B"	6, 5,	7' 2"	7 2*	7' 8"	6' 1"	5' 11"	6. 0,	6, 8,	6' 8"	6. 0.		6, 10,	6' 6"	GROUP	(1) 2X4 "L"	
7 3		В, 6,	8' 11"	B 11°	7' 1"	B' 3°	e' 3"	8' 6"	6' 3"	7' 3"	7' 4"	8' 1"	B' 1°	Ø. 22	2 2	7' 2"	7. 8.	6 1	5' 11"	6. 0.	7' 0"	7' 0"		5' 10°	5' 10"	6. 8.	A GROUP B	L" BRACE .	
8 8		9, 10,	9' 10"	8' 10°	9, 6,	9' 10"	8, 10,		A' 5"	B' 11"		B' 11"	8' 11"	8. 3.	B' 11"	8' 11"	B. 11	8' 11"	7' 10"	7' 10"	7' 10"	7' 10"	6. 9.	7' 10"	7' 10"	7' 10"	GROUP A	(2) 2X4 "L"	
8 8	10' 4"	10' 4"	10' 7"	10' 7"	9' 6'	9' 10"	8' 10"	10' 1"	8' 5"	8' 5"	8. 6.	8, 3,	8, 2,	6' 3'	8' 11"	8' 11"	9. 23.	6, 11,	8' 0"	8, 1,	8' 5°	8' 5"		7' 10"	7' 10"	8′0°	GROUP	BRACE **	
11 4	12' 11"	12 11		1 7	-	18, 10,	12' 11"	12' 11"	8, 8,	11' 4"	11' 5"	11, 9,	11, 8,	9' 7"	11, 1,"		11. 9.	8' 0"	8, 3,	9' 4"	10' 3"	10' 3"	-	9' 1"	9' 1"	10" 3"	B GROUP A	(1) 2X0	
11 4	13. 1.	18' 3"	13' 11°	13' 11"	11' 1"	12' 10"	12' 11"		8, 8,	11' 4"	11. 6.	12' B"	12' B"	8. 4.	11. 1.	11' 2"	12' 1"	8' 0"	8 3	9' 4"	11' 1"	11' 1"	7' 10"	9, 1,	9' 1"	10' 7"	A GROUP E	"L" BRACE .	
14 0	14. 0	14' 0"			14	14' 0"	14 0	14. 0.	13' 9"	14 0	14' 0"	14' 0"	14 00	18. 11.	14' 0"	14' 0"	14. 0	10' 10"	12' 3°		12' 8"	12' 3"	10' 7"	12' 8"	12' 3"	12' 3"		,T, 8X2 (2)	
14 0	14. 0	14' 0"	14' 0"	14. 0	14' 0"	14' 0"	14' 0"	14. 0	13' 3"	14. 0	14. 0	14' 0°	14' 0"	18. 11.	14' 0°	14' 0"	14. 0.	10' 10"	12' 6"	12' 8"	19' 2"	13' 2"	10' 7"	12' 3"	12' 3"	12' 7"	GROUP A GROUP B	FIRACE **	
CABLE END SUPPORTS LOAD FROM 4' 0"	CONTINUOUS BRARING (5 PSF TC DEAD LOAD).	PROVIDE UPLIFF CONNECTIONS FOR 180 FLF OVER	LIVE LOAD DEPLECTION CHITERIA IS L/240.		GARLE TRUSS DETAIL NOTES:			900	T	N PINE DOUGLAS			MIN-MIN		GROUP B.			9	dinas or	-CARCH SOUTH		STUD #3 ST	42		CROID A:	BRACING GROUP SPECIES AND GRADES:	ח		£



DIAGONAL BRACE OPTION:
VERTICAL LENUTH MAY BE
DOUBLED WIEND DIAGONAL
HRACE IS USSED. CONNECT
HRACE IS USSED. CONNECT
HRACE IS USSED. TOWN BEGING
AT EACH END. MAX WEB
TOWAL LENGTH IS 14.

GABLE THUBS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 BP OR
DT-L #Z OH
BETTER DIAGONAL
BRACE; SINGLE
OR DOLUME

AT UPPER END

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

ASE END SUPPORTS LOAD FROM 4' 0" DVILLDWERS WITH 8' 0" DVILHANG, DR 12" PLYWOOD OVERHANG. INDOUS BEARING (6 PSF TC DIAD LOAD). AD DEPLECTION CHITERIA IS L/240.

ATIACE EAGH "L" BRACE WITH 10d NAILS.

\$ POR (1) "L" BRACE; SPACE NAILS AF 2" O.C.

\$ FUR (2) "L" BRACES; SPACE NAILS AT 3" O.C.

\$ FUR (3) "L" BRACES; AND 4" O.C. BETFEEN ZONES.

T. BRACING MUST BE A MINIMUM OF BOX OF WEB

MEMBER LENGTH.

S DEBIG	PEAK, SPLICE, AND HEEL
30	GREATER THAN 11' 6"
N)	GREATER THAN 4' 0', BUT
1X4	IESS THAN 4' 0"
ON	VEHINCAL LENGIN
E SIZE	GABLE VERTICAL PLAT

NO MARCH	NG. REPER ID BOST 1-90 CONTINUE COMPUNENT SAFETY (RECORDINATION PUBLISHED OF THE VIRGIS- E INSTITUTE, 383 D'ONTFRID DR. SUTTE 200, RADISON, ME 25719) AND VITCA KUDDI TRUSS COLNOCI. BERUA, 6-30 E MITRAPRISE IN, MOUSON, VI 53719) FUR SAFETY PRACTICES PRIDE ID PERFORMINI	1456 SM 444 AVENUE		DWG MITEK STD GABLE SO' E HT
	L PANELS AND BOTTON		. 1	-ENG
STATE OF SERVICE	REVIEWED		MAX. TOT. LD. 60 PSF	
Will Co.	By julius lee at 12:00 pm, Jun 11, 2008			
WILL CONTROL OF THE PARTY OF TH		No: 34869 STATE OF FLORIDA	MAX. SPACING 24.0"	

BOT CHORD CHORD WEBS 2X4 2X4 200 品品品 BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

쳠

5

30,

34

88

52

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

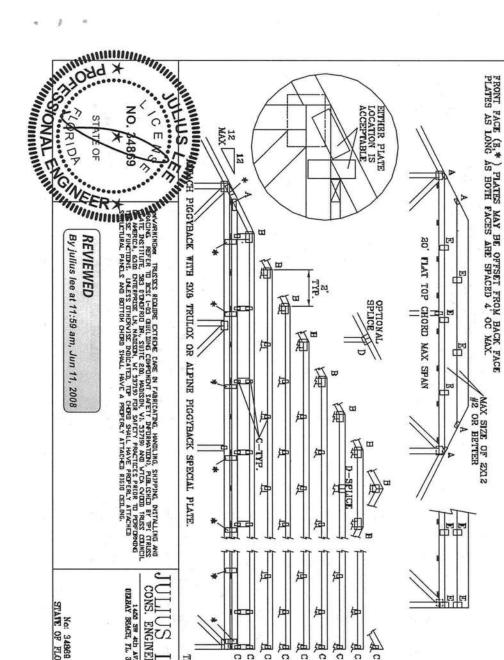
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
11G MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MBAN HGT, FBG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 FSF, WIND BC DL-5 FSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=6 PSF, WIND BC DL=6 PSF CLOSED EXP. C.



D 5X4 5X5 5X5 5X8 E 4X8 OR 3X8 TRULOX AT 4' OC,	C 1.5X8 1.5X4	H 4X8 5X8	
DLOX AT 4' DC	1.5X4 1.5X4	5X8 5X8	

ATTACH TRULOX PLATES WITH (8) 0.120 X 1.375" NAILS, (EQUAL, PER PACE PER PLY. (4) NAILS IN EACH MEMBER BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION. 28 38

	C)(0	ATTACH TEETH TO THE PABRICATION. ATTACH (4) 0.120" X 1.375" NAP PIGGYBACK SPECIAL PL SPACE 4" OC OR LESS.		10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
) (\bigcirc	TH TO THE ATTACK N. ATTACK	* PIC	MEMBER.	MEMBER. MEMBER.	NO BRACING		WEE
196	۰	۵ '	٥	AP-1-	* PIGGYBACK SPECIAL PLATE	OR HETT	"T" BRACE. SAME GRADE, BER. OR HETTER, AND 80% BER. ATTACH WITH 8d NA	NG	RR	WEB BRACING CHART
	0		o a	田田二	SPECIAL	RACE. SAME GRADE, OR BETTER, AND 80% ATTACH WITH 16d N	ACE. SAME GRADE, SPECIES R HETTER, AND 80% LENGTH ATTACH WITH 8d NAILS AT 4		REQUIRED BRACING	G CHAR
	٥	o a '	4	AT THE TIME OF TING TRUSS WITH JE PER PLY. API H TRUSS FACE A	PLATE	RADE, SI 80% L			BRACING	Τ
	a	. '	4	NT THE TIME OF NG TRUSS WITH PER PLY. APPLY TRUSS FACE AND		ACE. SAME GRADE, SPECIES AS R BETTER, AND 80% LENGTH OF ATTACH WITH 18d NAILS AT 4° OF	SPECIES AS LENGTH OF		4,	
	-	zų,				OF WEB	C. WEB			

CONS.

IUS LEE'S

THIS DRAWING

REPLACES DRAWINGS 634,016 834,017 & 847,045

8 1/4"

C

1400 SW 4th AVENUE

No: 34869 STATE OF FLORIDA

SPACING

24.0

1.15 DUR.

47 PSF

AT FAC.

1.25 DUR.

50 PSF

FAC.

-ENG

DRWG MITEK STD

PIGGY

1.33 DUR.

55 PSF AT 33 DUR. FAC.

DATE REF

09/12/07 PIGGYBACK

MAX LOADING

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

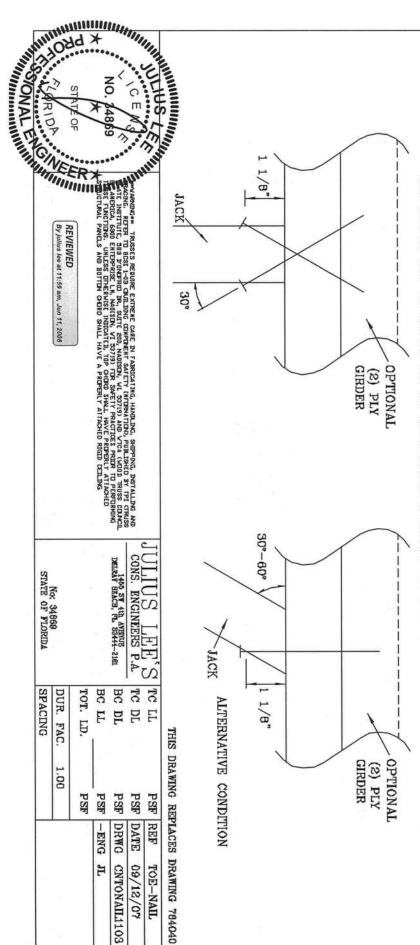
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 END DISTANCE, SPACING: "EDGE DISTANCES, I SPACINGS FOR NAILS AND SPIKES SHALL BE PREVENT SPLITTING OF THE WOOD." - EDGE DISTANCE, END DISTANCES AND SUFFICIENT TO

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A FRAMING INTO A SINGLE TOE-NAILED CONNECTION FOR JACK OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS	
RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-	MUNIXAM
RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-	
OF 16d (0.162"X3.5") COMMON TOE-	RESISTANCE
(0.162"X3.5") COMMON TOE-	OF
) COMMON TOE-	16d
TOE-	(0.162"X3.5")
TOE-	COMMON
	TOE-

UMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
23	197#	256#	181#	234#	156#	203#	154#	199#
အ	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
Ð	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALU	ES MAY	ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.	TA AH CIE	PROPRIATE	DURATION	OF LOAD F	ACTOR.	



NONCHM TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND NG. REPER TO BEST 1-93 CHUILING COMPIDENT SAFETY (REPENTION). BULLISHED BY FET CRIVES INSTITULE, SIBS PORNEME IN, SUITE SON, NAINISM, NEL SOT(5) NOW OTCA (NOID) TRUSS CONCELLED, AND STOAL NEW SECOND TRUSS CONCELLED, AND STOAL PRACTICES PRIEM TO PERFORMING FRUITIONS. UNITESS INDICATED, THE OFFICE SHALL HAVE PEPERLY ATTACHED TURAL PAWELS AND BOTTON CHOID SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

C

DELRAY BEACH, FL SO444-2161

BC LL BC DL

TOT.

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PSF PSF PSF PSF

> -ENG DRWG

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CNTONAIL1103 09/12/07

TC DL

DATE

No: 34868 STATE OF FLORIDA

SPACING DUR. FAC.

1.00

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

TRULOX CONNECTION

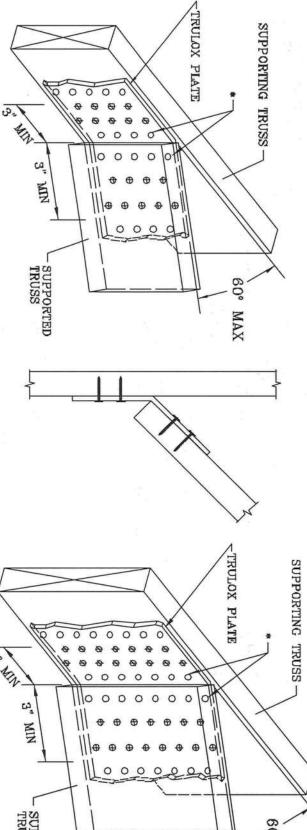
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi \)).

NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 EXCEED THE DURATION OF LOAD. TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

INFORMATION NOT SHOWN THIS DETAIL FOR LUMBER, PLATES, AND OTHER REFER TO ENGINEER'S SEALED DESIGN REFERENCING



TRULOX PLATE SIZE **6X8** 3X6 REQUIRED NAILS PER TRUSS 15 9 MAXIMUM LOAD UP OR DOWN #066 350#

REVIEWED

MINIMUM 3X6 TRULOX PLATE

3. MIN SUPPORTED TRUSS 60% MAX

THIS DRAWING REPLACES DRAWINGS 1,158,986 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

MINIMUM 5X6 TRULOX PLATE

No: 34869 STATE OF FLORIDA	100	DELRAY BEACH, IL 33444-2161	INEERS P	IULIUS LEE'S
	-ENG JL	DRWG	DATE	REF

NO. 84869

NO. 84869 NNO- TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANGLING, SHOPDING, INSTALLING AND AS REFER TO 5031-00 GRULLING CORPORDT SAFETY METROWATION, PUBLISHED BY THE KIRKSS CONSTITUTE, SED TOWNETHE INS. SUTTE BY, MARISEN, UT. 33759 AND VICA VOUTH TRUSS COUNTIES, RICHA FOR SAFETY PRACTICES PRICE TO PERFORM FUNCTIONS. UTLESS OTHERWISE, MICHAEL TOP CARDO SALL HAVE PROPERLY ATTACHED TURNEL PAPELS AND SOTTON CHURCO SWALL HAVE A PROPERLY ATTACHED RICHARD STALL HAVE A PROPERLY ATTACHED RICHARD SELLING.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

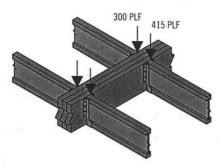
			以為一門門的,數		Co	onnector Pattern		
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
Connector Type	Number of Rows	Connector On-Center Spacing	2" 2" 134"	134"	1½" 3½"	134" 31/4" 134"	1 3V2	134
			3½" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3")	2	12"	370	280	280	245		
Nail(0)	3	12"	555	415	415	370		
1/2" A307		24"	505	380	520	465	860	340
Through Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	510	455		THE SHEET SHEET HE
SDS 1/4" x 31/2"(4)	2	19.2"	850	640	640	565		A Street of the
		16"	1,020	765	765	680		AND DESCRIPTION OF THE PERSON
		24"				455	465	455
SDS 1/4" x 6"(3)(4)	2	19.2"				565	580	565
		16"				680	695	680
		24"	480	360	360	320		State of the state of
USP WS35 (4)	2	19.2"	600	450	450	400		
		16"	715	540	540	480	Market State	10年以第四屆共產黨的
1100 W00 WW		24"				350	525	350
USP WS6 (3)(4)	2	19.2"				440	660	440
		16"	OOF CO			525	790	525
33/8"		24"	635	475	475	425		
TrussLok(4)	2	19.2"	795	595	595	530		
		16"	955	715	715	635		AND ADDRESS OF THE
5"		24"		500	500	445	480	445
TrussLok(4)	2	19.2" 16"		625	625	555 CCF	600	555
			Commission of the Commission o	750	750	665	725	665
63/4"		24"	Ange State	DESTRUCTION OF THE PARTY OF THE		445	620	445
TrussLok(4)	2	19.2" 16"	20/10-10-10-10-10-10-10-10-10-10-10-10-10-1			555	770	555
		16		THE STREET OF STREET	THE REAL PROPERTY.	665	925	665

- Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
 Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
 of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

Julius Lee

RE: 327272 - RICHARD KEEN - GLENN RES.

1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Site Information:

Project Customer: RICHARD KEEN Project Name: 327272 Model: GLENN RES.

Lot/Block: Subdivision:

Address: 185 SW ARROWHEAD TER

State: FL City: COLUMBIA CTY

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JAMES H. JOHNSTON

License #: CRC1328128

Address: 650 SW MAIN BLVD.

State: FL City: LAKE CITY,

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 52 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet

conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	14241488	CJ1	3/4/010	18	14241505	HG13	3/4/010
2	14241489	CJ1A	3/4/010	19	14241506	HG17	3/4/010
3	14241490	CJ3	3/4/010	20	14241507	HG23	3/4/010
4	14241491	CJ3A	3/4/010	21	14241508	HJ2	3/4/010
5	14241492	CJ3B	3/4/010	22	14241509	HJ7	3/4/010
6	14241493	CJ5	3/4/010	23	14241510	HJ9	3/4/010
7	14241494	CJ5A	3/4/010	24	14241511	HJ9A	3/4/010
8	14241495	CJ5B	3/4/010	25	14241512	HJ9B	3/4/010
9	14241496	EJ5	3/4/010	26	14241513	HJ9C	3/4/010
10	14241497	EJ7	3/4/010	27	14241514	T01	3/4/010
11	14241498	EJ7A	3/4/010	28	14241515	T02	3/4/010
12	14241499	EJ7B	3/4/010	29	14241516	T03	3/4/010
13	14241500	EJ8	3/4/010	30	14241517	T04	3/4/010
14	14241501	EJ8A	3/4/010	31	14241518	T05	3/4/010
15	14241502	EJ8B	3/4/010	32	14241519	T06	3/4/010
16	14241503	EJ8C	3/4/010	33	14241520	T07	3/4/010
17	14241504	HG03	3/4/010	34	14241521	T08	3/4/010

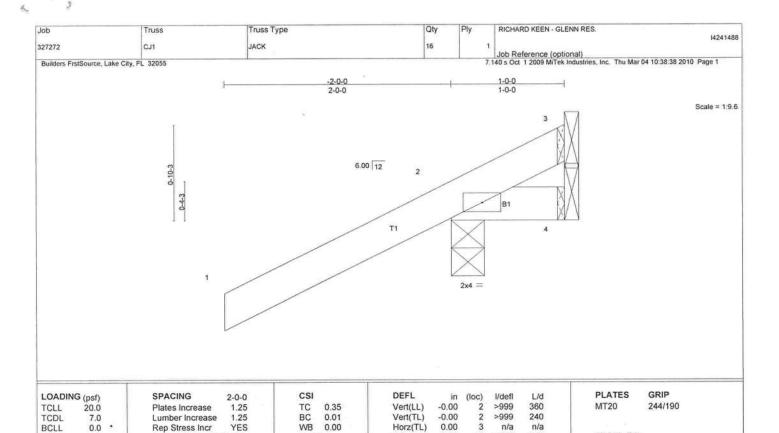
The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.





LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

BRACING TOP CHORD **BOT CHORD**

Wind(LL)

2

>999

240

0.00

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 7 lb

No 34

FLORIDA

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=109(LC 6)

Max Uplift 2=-371(LC 6), 4=-12(LC 4), 3=-90(LC 1)

Code FBC2007/TPI2002

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with the bottom chord live load of 20 per fit between the bottom chord live load of 20 per fit between the bottom chord.

(Matrix)

fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2, 12 lb uplift at joint 4 and 90 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

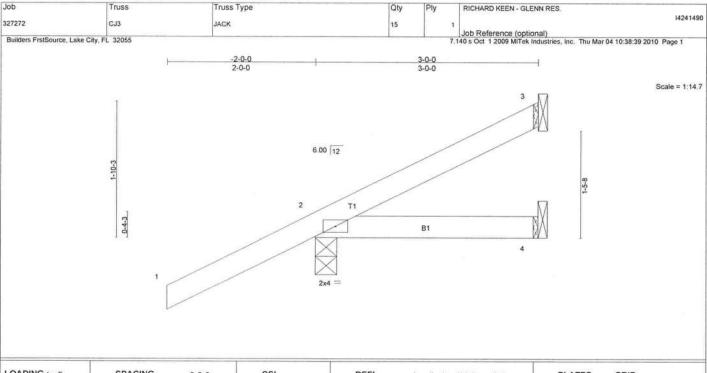
March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information

available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	2-4	>999	240	MANUAL P	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TP	12002	(Matr	rix)	Wind(LL)	0.01	2-4	>999	240	Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

U

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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. Was Sky

LICENSE

No 34869

STATE OF

FLORIDA

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=166(LC 6)
Max Uplift3=-36(LC 7), 2=-311(LC 6), 4=-36(LC 4)
Max Grav 3=31(LC 1), 2=250(LC 1), 4=42(LC 2)

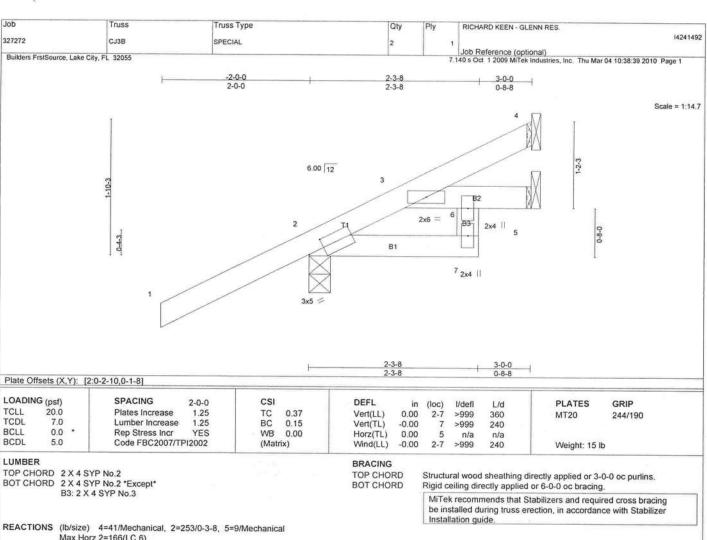
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 311 lb uplift at joint 2 and 36 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE, Design valid for use only with Milek connectors. This design is lossed only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional femporary bracing to insus stability during construction is the responsibility of the suit of the designer. For general guidance regarding febrication, quality control, storage, delivery, erection and bracing, consult — ANSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.



Max Horz 2=166(LC 6)

Max Uplift4=-45(LC 6), 2=-265(LC 6)

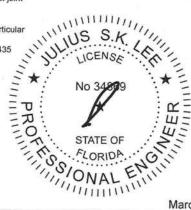
Max Grav 4=41(LC 1), 2=253(LC 1), 5=48(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 4 and 265 lb uplift at joint
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

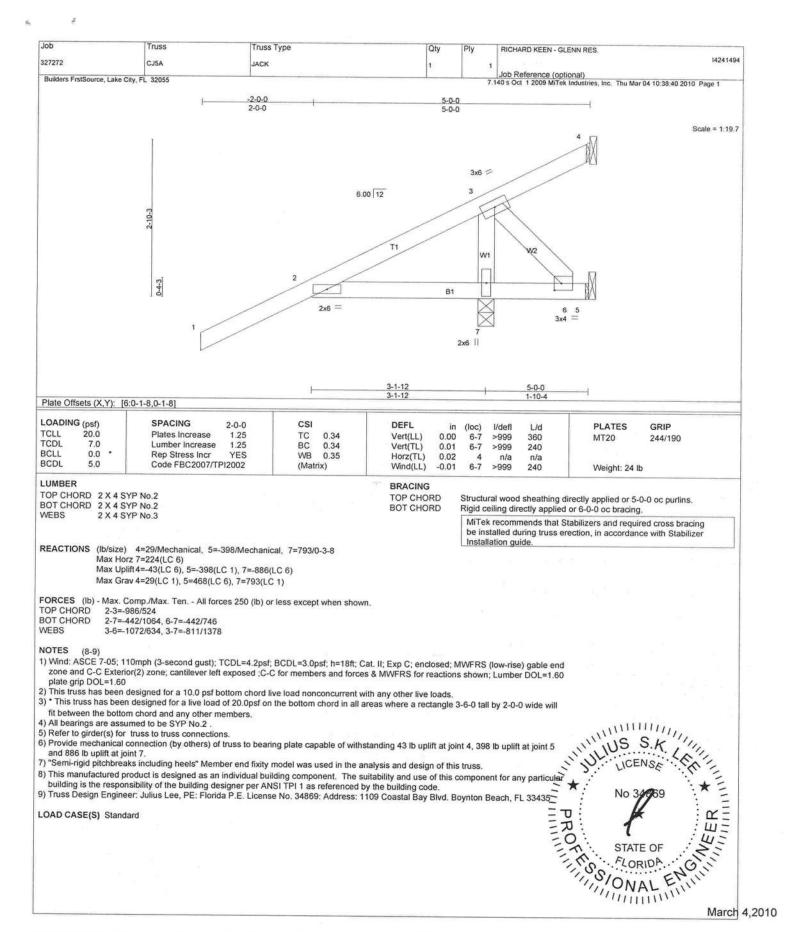
LOAD CASE(S) Standard



March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII 7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insert stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, qualify control, storage, delivery, erection and bracing, consult
ANSI/TPI Quality Certification.

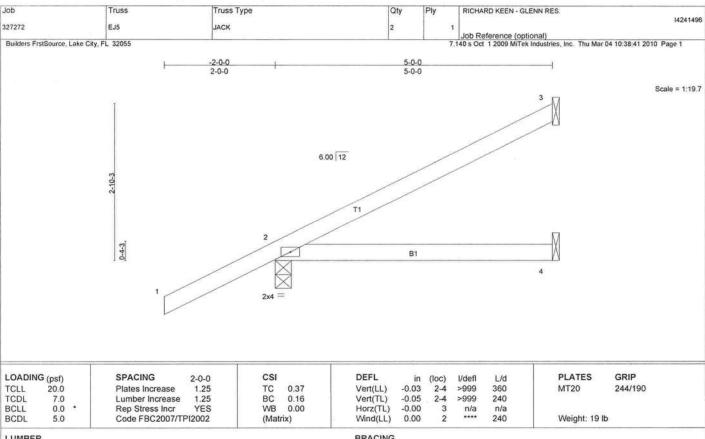
Safety Information available from Truss Plate Institute, 583 D'Onotino Drive, Madison, WI 53719.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer- not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult — ANSI/PII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. WIS SE

LICENSE

No 34669

STATE OF

SOIONAL FLORIDA

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=224(LC 6)

Max Uplift3=-114(LC 6), 2=-266(LC 6)

Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18tr; Cat. II, Exp.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

joint 2.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

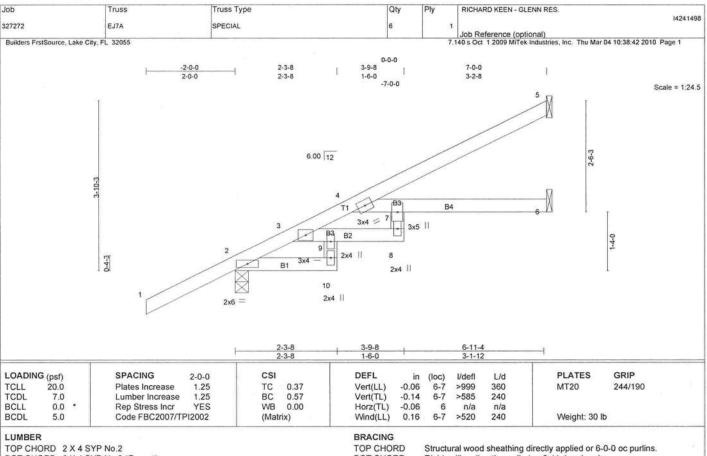
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTe Aconnectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. ANS/ITH1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



BOT CHORD 2 X 4 SYP No.2 *Except*

B3: 2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 9-11-1 oc bracing.

D

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. WINS SE

NS

SOIONAL FLORIDA

LICENSE

No 34669

REACTIONS (lb/size) 5=121/Mechanical, 2=357/0-3-8, 6=82/Mechanical

Max Horz 2=203(LC 6)

Max Uplift5=-83(LC 6), 2=-190(LC 6), 6=-16(LC 6) Max Grav 5=121(LC 1), 2=357(LC 1), 6=106(LC 2)

FORCES (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-363/231

BOT CHORD 3-9=-307/192, 8-9=-394/299, 4-7=-299/394

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C-Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 5, 190 lb uplift at joint 2 and 16 lb uplift at joint 6.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Flonda P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 4,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIL-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is bosed only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to invest stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Chotino Drive, Madison, WI 53719.

1109 Coastal Bay Blvd. Boynton, FL 33435

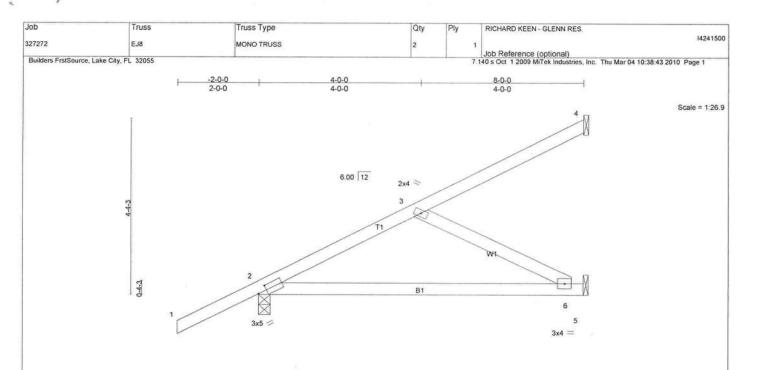


Plate Offsets (X Y): [2:0-2-10 0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-6	>647	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.25	2-6	>367	240	10000177-7	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TF	212002	(Matr	rix)	Wind(LL)	-0.01	2-6	>999	240	Weight: 35 lb	Y:

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=92/Mechanical, 2=381/0-3-8, 5=141/Mechanical

Max Horz 2=223(LC 6)

Max Uplift 4=-75(LC 6), 2=-198(LC 6), 5=-47(LC 6) Max Grav 4=92(LC 1), 2=381(LC 1), 5=147(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-299/143 TOP CHORD BOT CHORD 2-6=-348/224 WEBS 3-6=-251/389

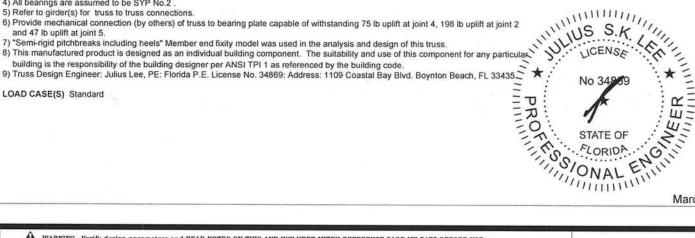
NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.

LOAD CASE(S) Standard

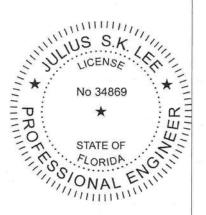


March 4,2010

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LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 1=-250 5=-826(B) 6=-285 7=-496(B)

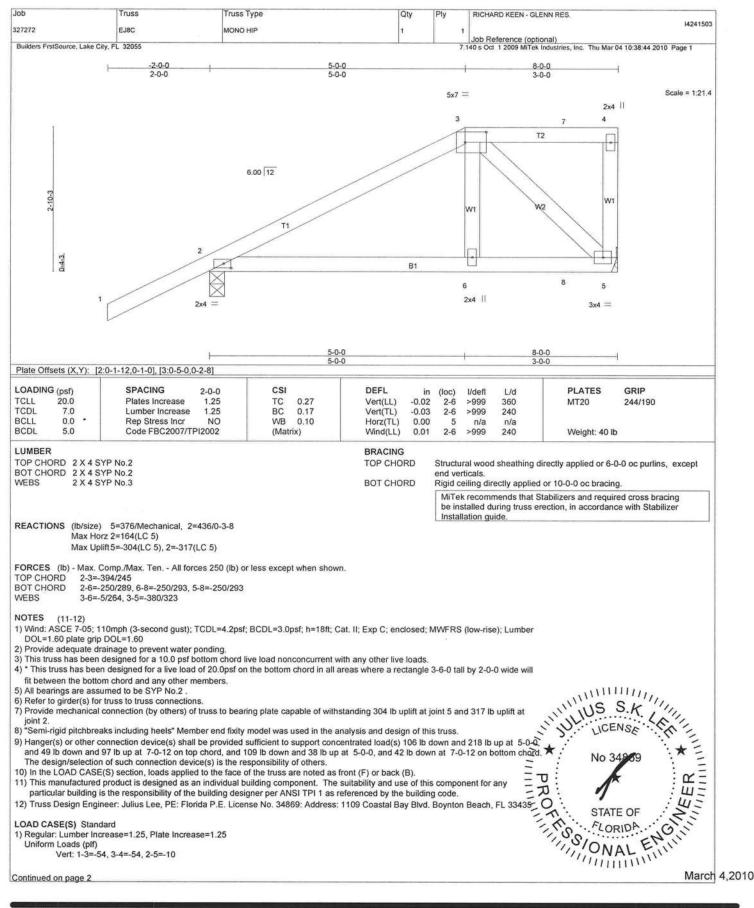




March 4,2010

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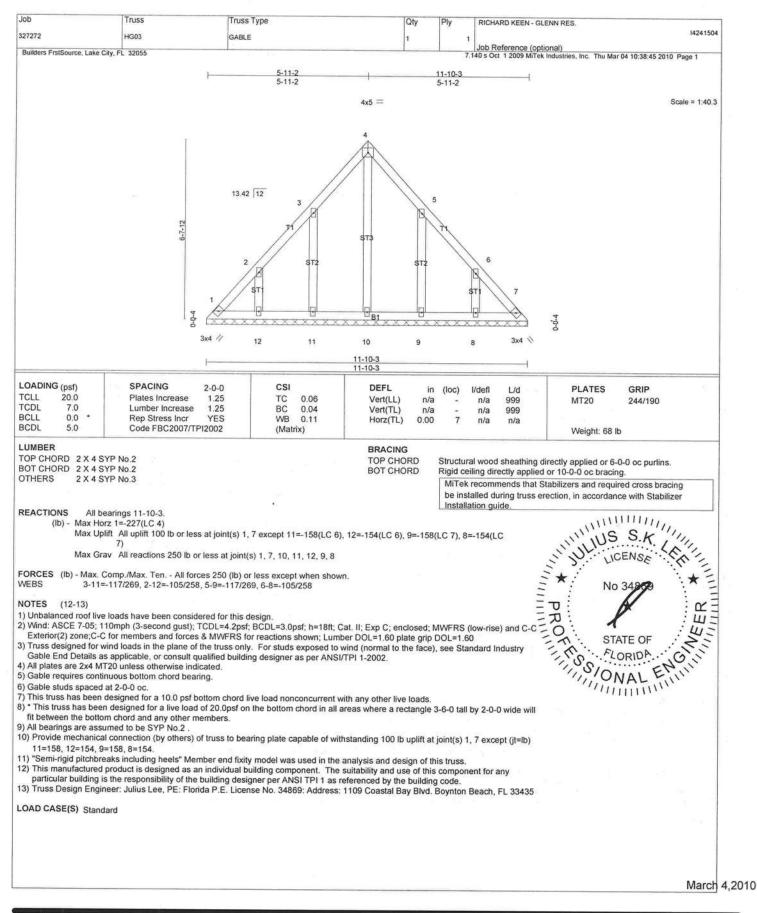
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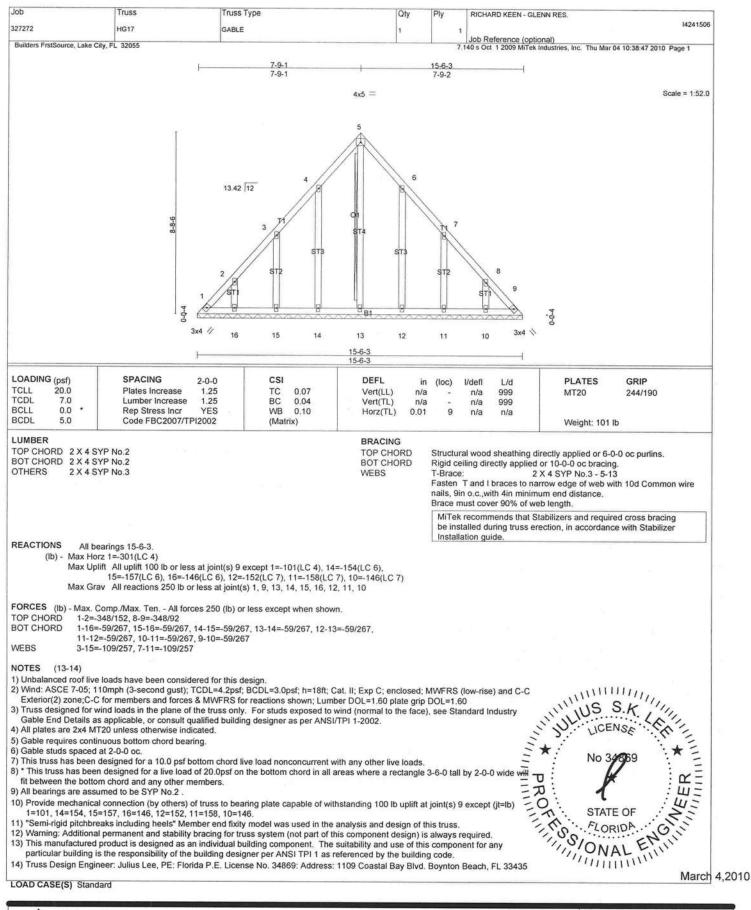
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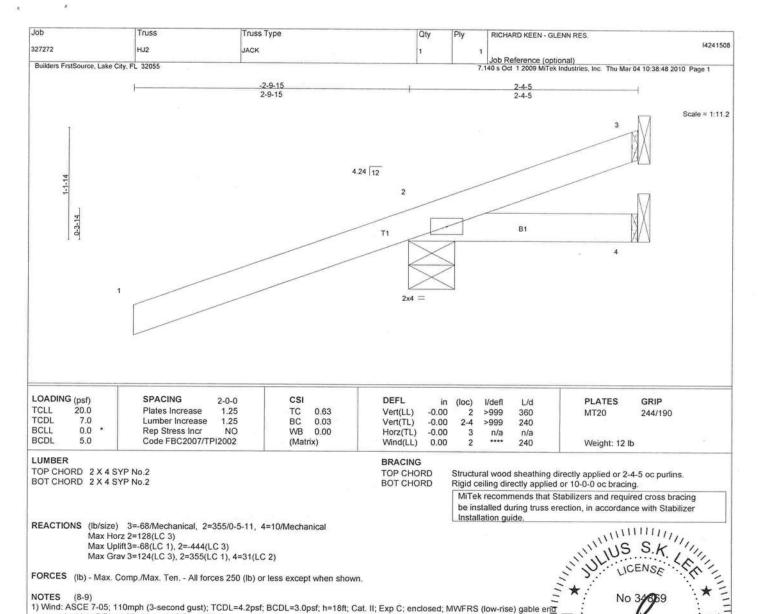
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Max Horz 2=128(LC 3) Max Uplift 3=-68(LC 1), 2=-444(LC 3)

Max Grav 3=124(LC 3), 2=355(LC 1), 4=31(LC 2)

REACTIONS (lb/size) 3=-68/Mechanical, 2=355/0-5-11, 4=10/Mechanical

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MVVV 1.0.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

STATE OF S

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

2 Ш

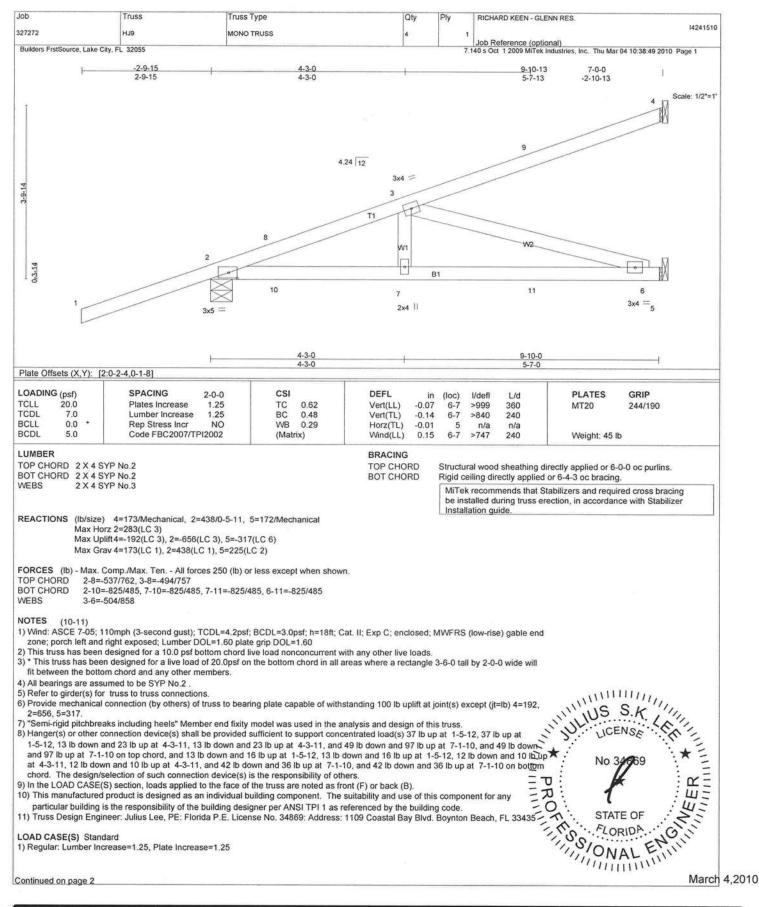
be installed during truss erection, in accordance with Stabilizer

Installation guide.

March 4,2010

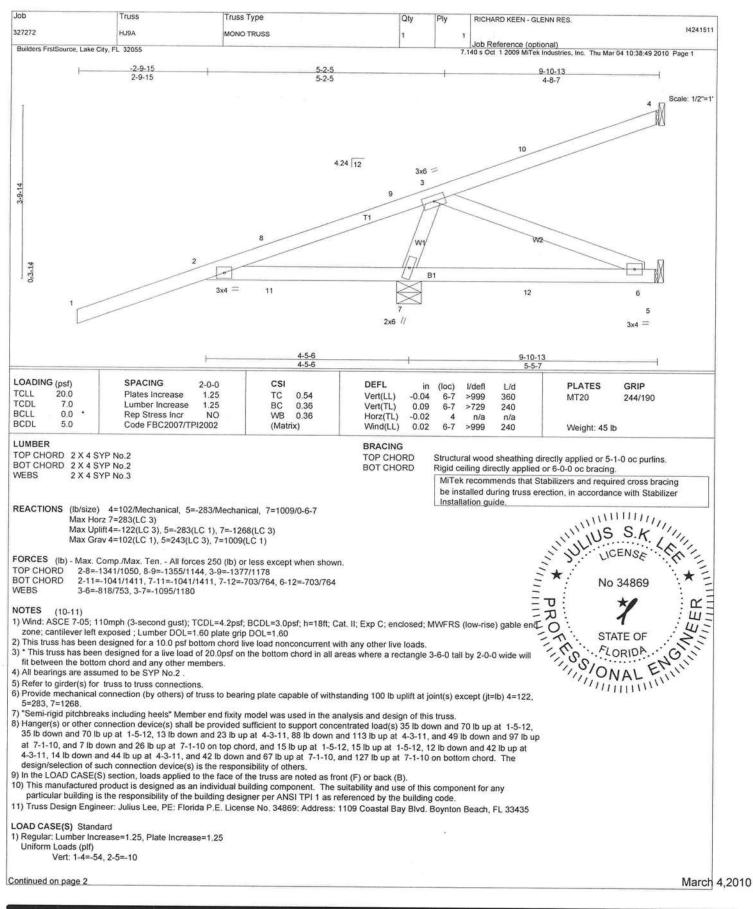
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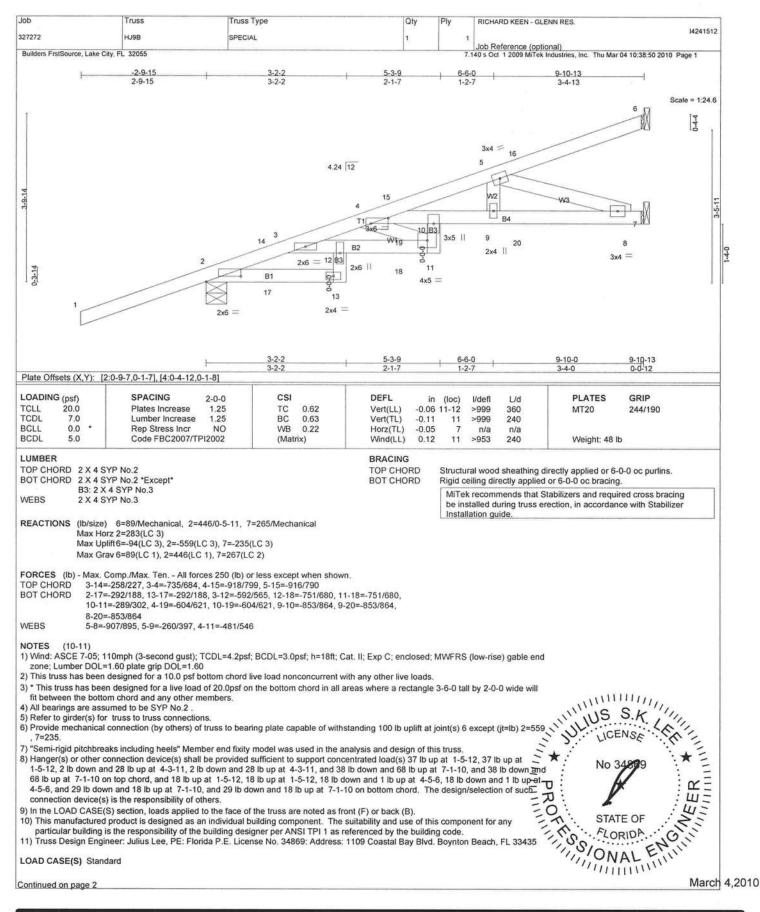


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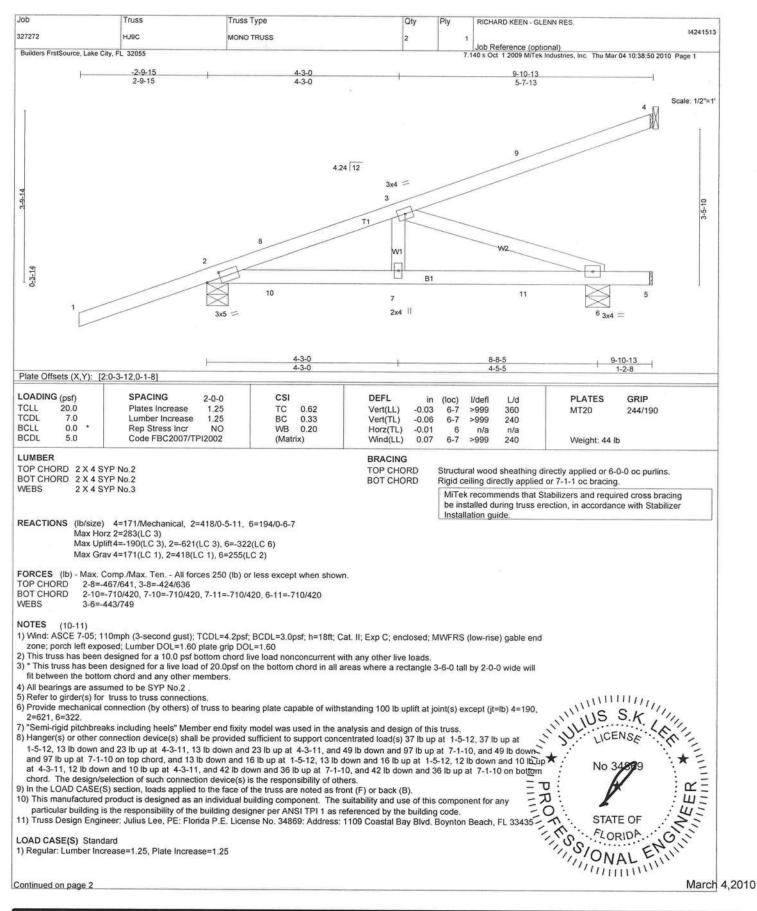
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ANSI/TRI Quality Critical, DSB-89 and BCS11 Building Component Safety Information.



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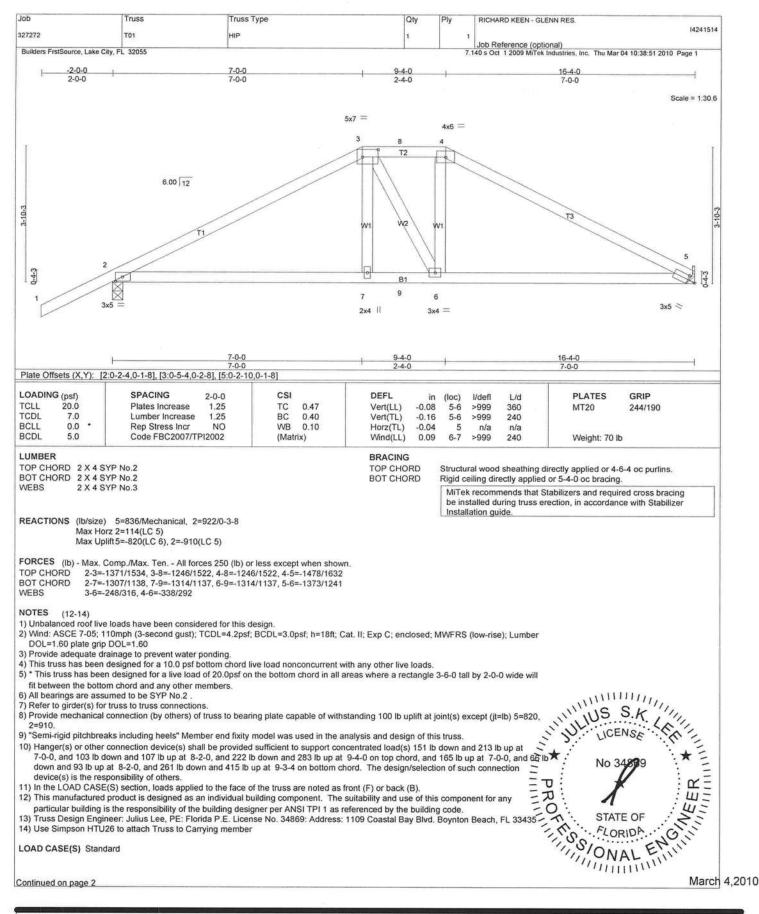
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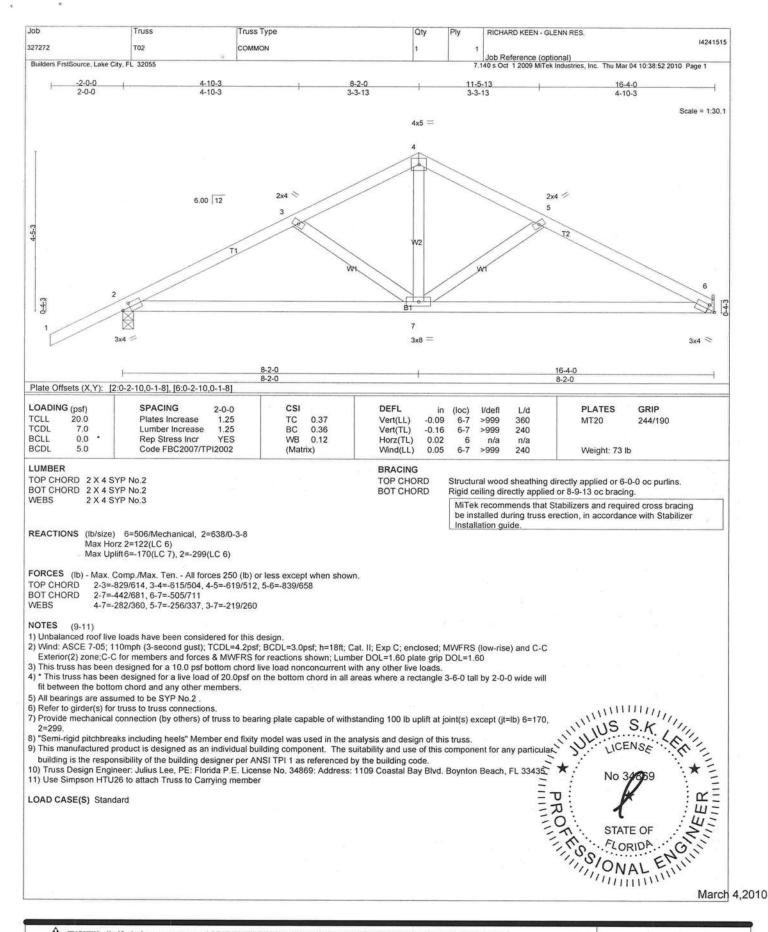
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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - GLENN RES.	
327272	T03	SPECIAL	1		Bit Hashington London Street English Stroken Stroken Stroken State (Francisco Paris)	14241516
Builders FrstSource, Lake City, F	L 32055			2	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04	10.29.52.2010
13) Truss Design Engineer LOAD CASE(S) Standard 1) Regular: Lumber Increase Uniform Loads (plf) Vert: 1-3=-54, 3 Concentrated Loads (lb)	r: Julius Lee, PE: Florida P. se=1.25, Plate Increase=1.3 -5=-54, 5-6=-54, 6-7=-54, 7	E. License No. 34869: Address: 1	109 Coastal Bay Blvd.	compon	ent for any particular building is the respor	nsibility of the
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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES. 14241518 327272 T05 SPECIAL Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:38:54 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 28-4-0 30-0-0 32-0-0 2-0-0 1-8-0 2-0-0 16-0-0 12-0-0 2-0-0 5-4-14 6-7-2 4-0-0 4-11-4 5-4-12 Scale = 1:58.8 5x8 = 6x8 = 6.00 12 3x4 > 6 3x4 = 3 6-2-8 4x5 = 10 14 3x6 = 16 15 13 11 3x5 > 3x4 = 3x6 = 2x4 || 3x4 = 3x8 = 3x5 = 30-0-0 Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-1-12,0-3-0], [5:0-4-10,Edge], [9:0-2-10,0-1-8] LOADING (psf) SPACING CSI DEFL PLATES 2-0-0 in (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.50 Vert(LL) -0.14 12-13 >999 360 MT20 244/190 0.65 -0.31 12-13 TCDL 7.0 Lumber Increase 1.25 BC Vert(TL) >999 240 BCLL 0.0 Rep Stress Incr WB 0.42 0.09 9 YES Horz(TL) n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.21 12-13 >999 240 Weight: 164 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins, except BOT CHORD 2 X 4 SYP No.2 2-0-0 oc purlins (5-2-3 max.): 4-5, 7-8. WEBS 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 4-3-8 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 2=1067/0-3-8, 9=1067/0-3-8 Max Horz 2=134(LC 6) Max Uplift 2=-421(LC 6), 9=-437(LC 7) FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1779/1266, 3-4=-1340/1042, 4-5=-1167/1042, 5-6=-1357/1090, 6-7=-2179/1577, 7-8=-1473/927, 8-9=-1605/1001 **BOT CHORD** 2-16=-930/1526, 15-16=-930/1526, 14-15=-543/1133, 13-14=-543/1133, 12-13=-1037/1663, 11-12=-2123/3149, 9-11=-686/1350 WEBS 3-15=-440/422, 4-15=-137/264, 5-13=-277/358, 6-13=-616/590, 6-12=-263/500, 7-12=-1279/984, 7-11=-1855/1496, 8-11=-527/752 NOTES (10-11)1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C 6) All bearings are assumed to be SYP No.2.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=421.

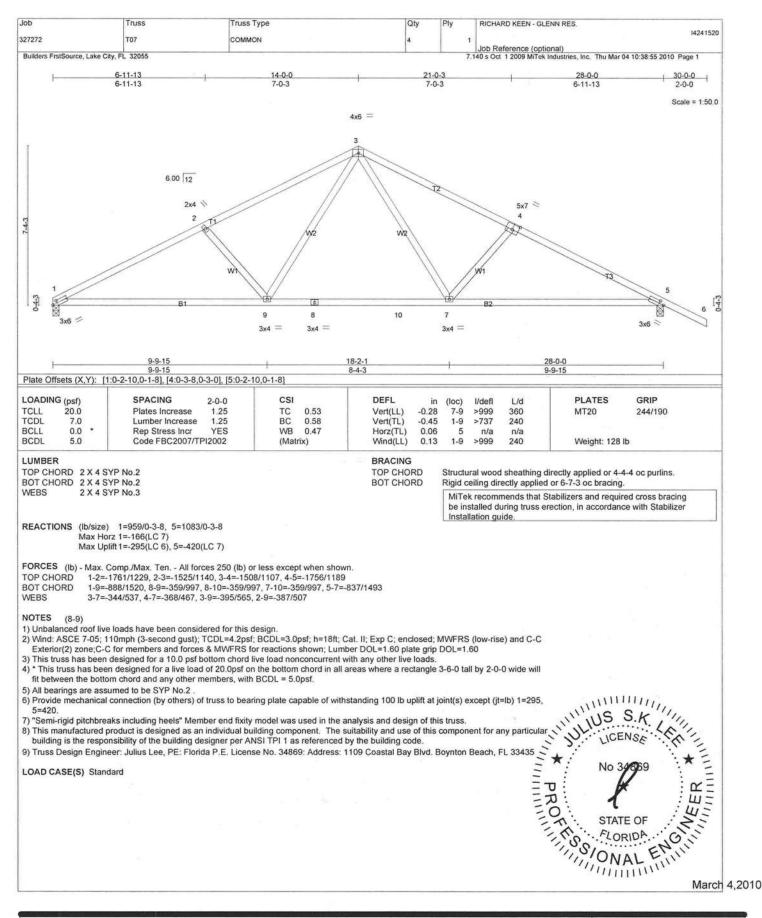
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss

10) This manufactured product is designed as an individual hourse. Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 7) Provide mechanical colors | 9-437.
8) "Semi-rigid pitchbreaks including heets" Member end fixity model was used in the seminary of the semi NINEF March 4,2010

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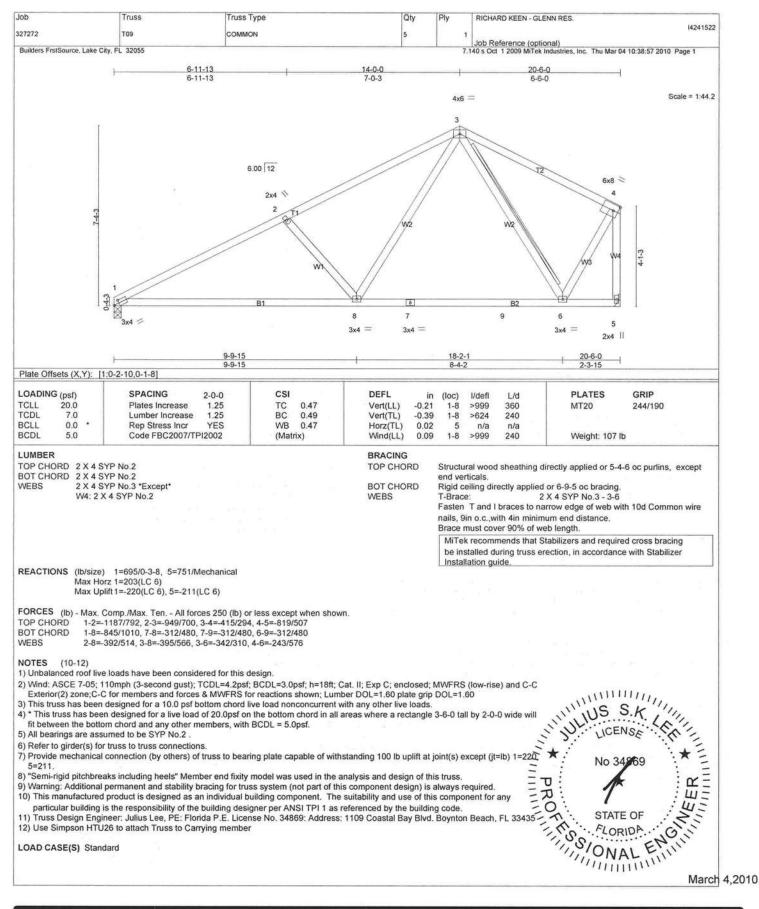
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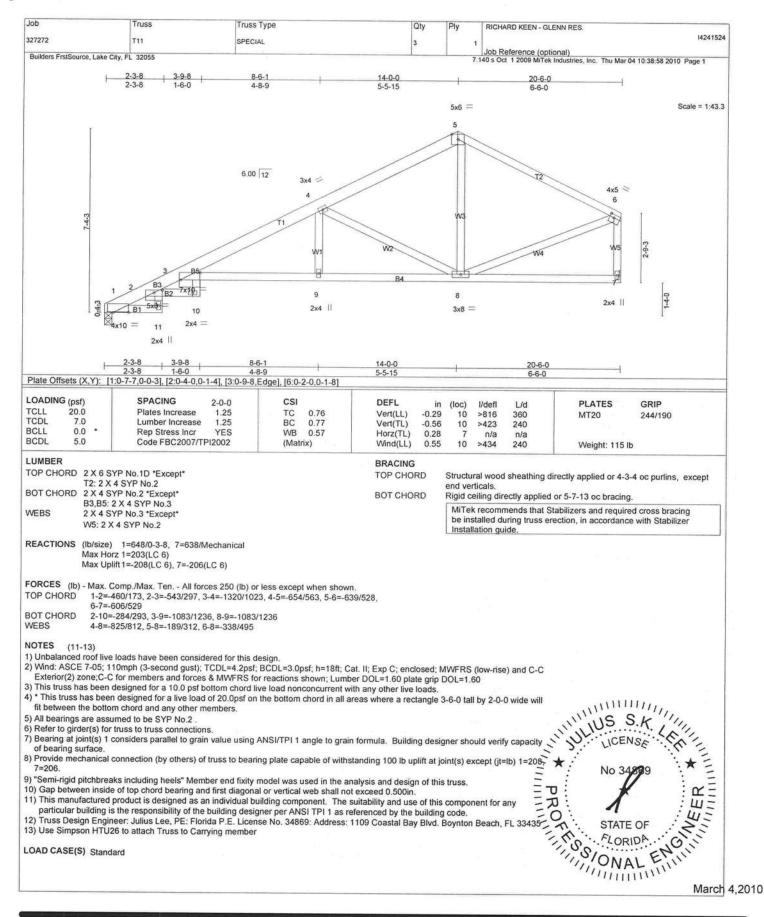


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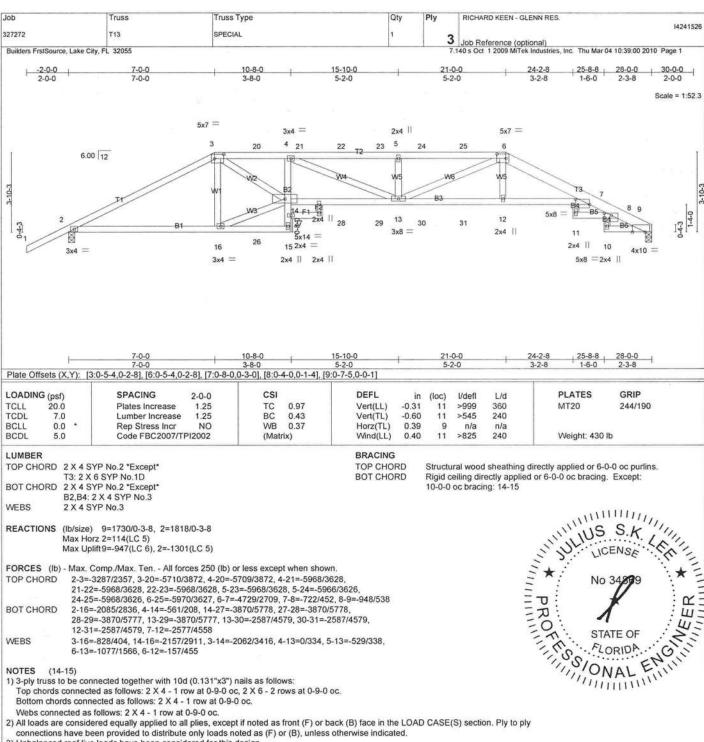
Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.



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3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be SYP No.2

9) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=947,

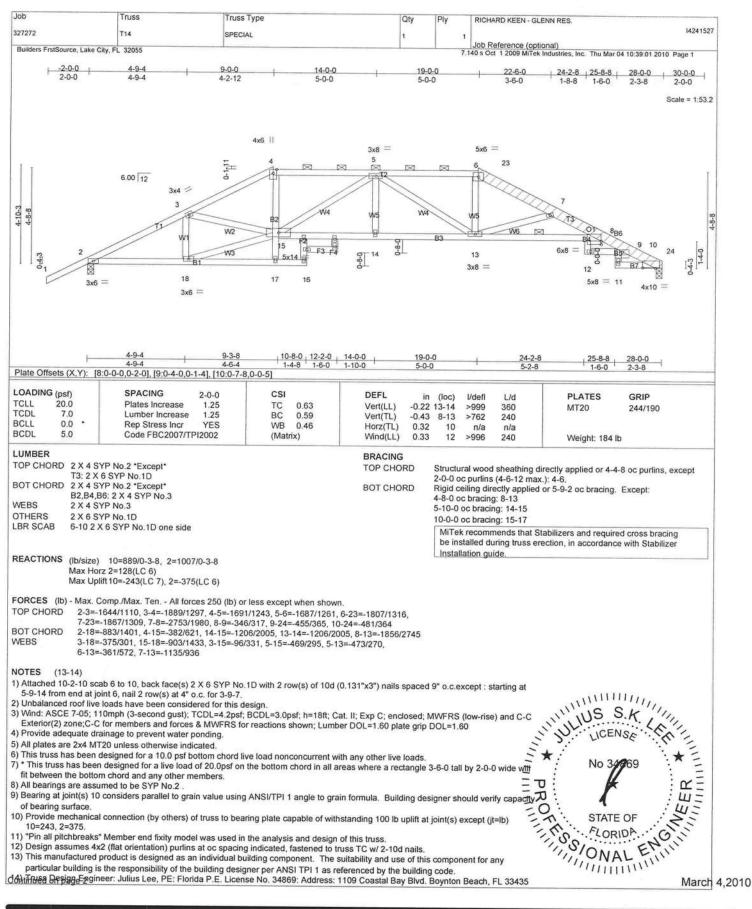
11) "Pin all pitchbreaks" Member end fixity model was used in the analysis and design of this truss.

12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. Continued on page 2

March 4,2010

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1109 Coastal Bay Blvd. Boynton, FL 33435

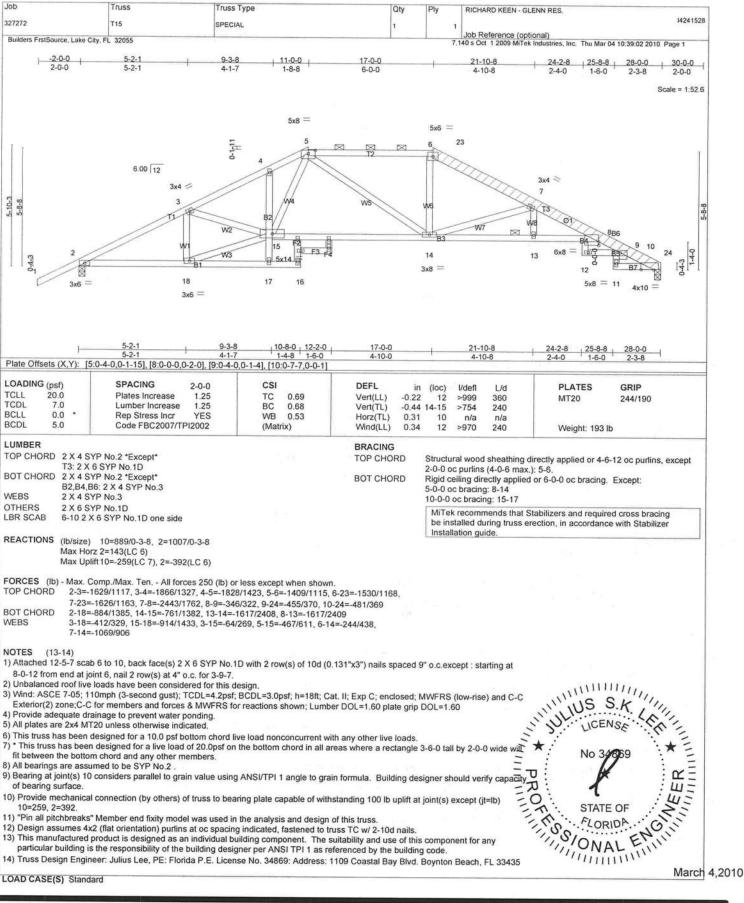


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AMSI/TEI Quality Citleria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.



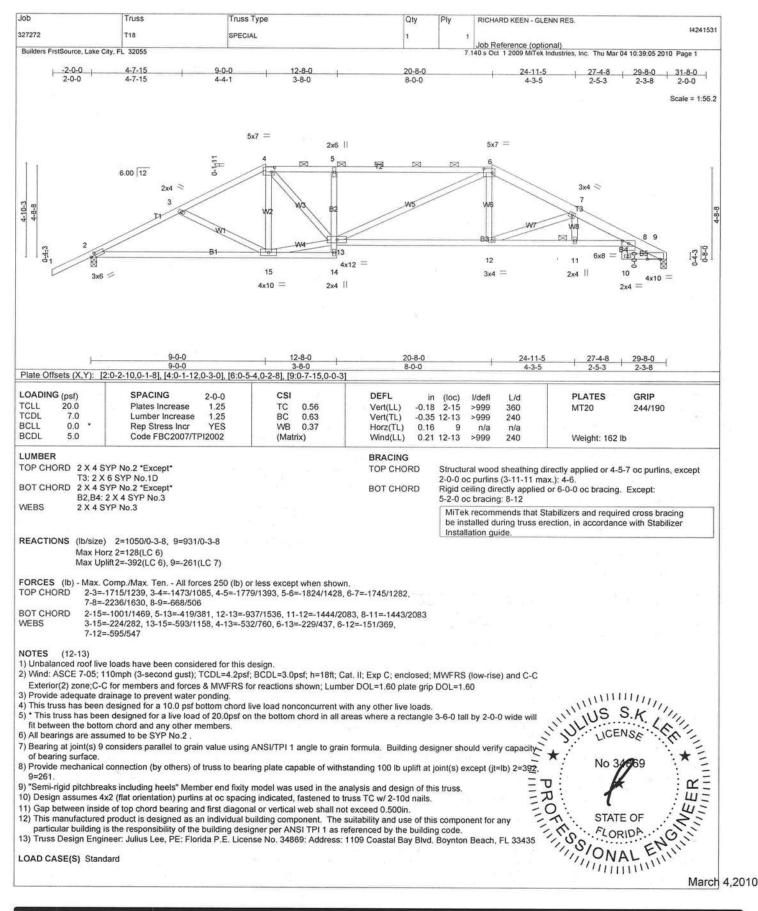
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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES. 14241530 327272 T17 HIP Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:04 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 12-2-12 17-5-4 29-8-0 7-0-0 Scale = 1:56.2 4x6 = 4x6 = 3 12 4 14 5 6.00 12 3-10-3 19 20 21 10 23 24 11 9 5x6 = 5x6 = 6x10 = 3x6 = 3x6 = 7-0-0 7-0-0 14-10-0 7-10-0 Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge], [10:0-5-0,0-3-4] LOADING (psf) SPACING CSI DEFL PLATES GRIP in (loc) L/d I/defl TCLL 20.0 TC Vert(LL) -0.29 10-11 Plates Increase 1.25 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.93 Vert(TL) -0.61 10-11 >574 240 BCLL 0.0 Rep Stress Incr NO WB 0.88 Horz(TL) 0.15 n/a n/a Code FBC2007/TP12002 BCDL 5.0 (Matrix) Wind(LL) 0.55 10-11 >636 240 Weight: 137 lb BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins. BOT CHORD 2 X 4 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 3-8-1 oc bracing. WEBS 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 2=1797/0-3-8, 7=1734/0-3-8 Max Horz 2=97(LC 5) Max Uplift 2=-1329(LC 5), 7=-1236(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TOP CHORD 2-3=-3314/2478, 3-12=-2904/2270, 12-13=-2903/2270, 4-13=-2903/2270, 4-14=-4007/3137, 14-15=-4007/3137, 15-16=-4007/3137, 5-16=-4007/3137, 5-17=-2785/2111, 17-18=-2785/2111, 6-18=-2786/2111, 6-7=-3178/2296 **BOT CHORD** 2-11=-2157/2862, 11-19=-2891/3814, 19-20=-2891/3814, 20-21=-2891/3814, 10-21=-2891/3814, 10-22=-2813/3774, 22-23=-2813/3774, 23-24=-2813/3774, 9-24=-2813/3774, 7-9=-1953/2742 WEBS 3-11=-803/1031, 4-11=-1189/940, 4-10=-320/434, 5-10=-411/454, 5-9=-1284/1068, 6-9=-877/1086 (11-12)THINITING S.K 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SYP No.2 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1329, No 34869 7=1236. "Semi-rigid pitchbreaks including heels" Member 2014. "Semi-rigid pitchbreaks including heels" 7=1236 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 150 lb down and 223 lb up at 7-100, ONEF 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). March 4,2010 Continued on page 2

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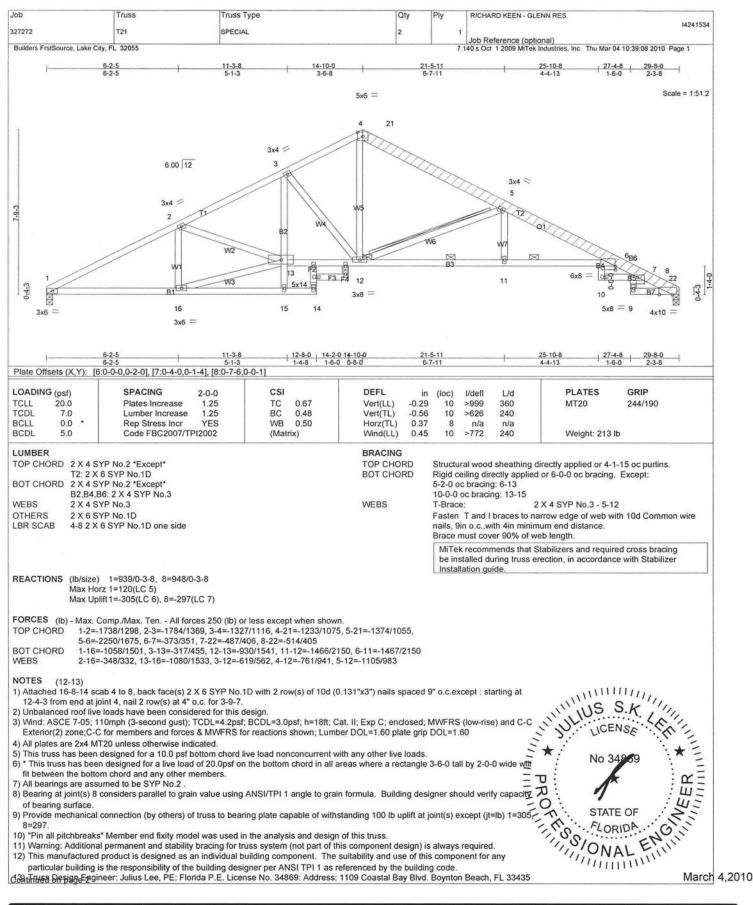
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327272	Truss	Truss Type SPECIAL	Qty 1	Ply 1	MANUFACTOR OF STEININ RES.	14241532
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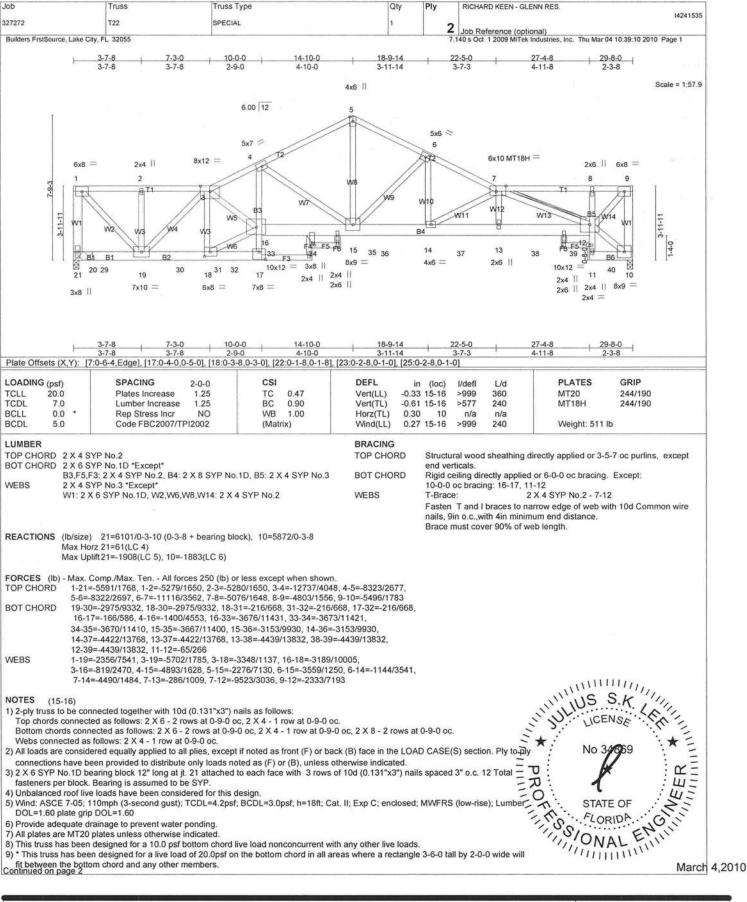
Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

March 4,2010



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Job Truss Truss Type Qty RICHARD KEEN - GLENN RES 14241536 327272 T23 HIP Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Thu Mar 04 10:39:11 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 2-0-0 7-0-0 10-11-10 16-0-6 20-0-0 3-11-10 5-0-12 3-11-10 7-0-0 2-0-0 Scale = 1:51.8 4x6 = 4x6 = 3x4 = 3x4 3 12 13_{T2} 14 5 15 6.00 12 0-4-3 16 17 1810 19 20 21 9 11 5x6 = 5x6 = 5x7 = 3x5 = 3x5 = 20-0-0 7-0-0 Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge], [10:0-3-8,0-3-4] LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 in (loc) I/defl L/d TCLL TC Vert(LL) -0.20 244/190 20.0 Plates Increase 1.25 0.70 10 >999 360 MT20 TCDL 7.0 1.25 BC 0.79 -0.41 10-11 >786 240 Lumber Increase Vert(TL) 0.48 BCLL 0.0 Rep Stress Incr NO WB Horz(TL) 0.14 n/a n/a BCDL Code FBC2007/TPI2002 (Matrix) 0.45 9-10 240 Weight: 126 lb 5.0 Wind(LL) >715 LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-10-14 oc purlins. BOT CHORD 2 X 4 SYP No.2 BOT CHORD Rigid ceiling directly applied or 3-2-5 oc bracing. 2 X 4 SYP No.3 WEBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 2=1791/0-3-8, 7=1791/0-3-8 Max Horz 2=97(LC 5) Max Uplift 2=-1656(LC 5), 7=-1645(LC 6) FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-3265/3147, 3-12=-2858/2887, 4-12=-2857/2887, 4-13=-3584/3584, TOP CHORD 13-14=-3584/3584, 5-14=-3584/3584, 5-15=-2857/2869, 6-15=-2858/2869, 6-7=-3265/3141 BOT CHORD 2-11=-2744/2820, 11-16=-3385/3518, 16-17=-3385/3518, 17-18=-3385/3518, 10-18=-3385/3518, 10-19=-3369/3518, 19-20=-3369/3518, 20-21=-3369/3518, 9-21=-3369/3518, 7-9=-2704/2820 WEBS 3-11=-1066/968, 4-11=-981/894, 4-10=-243/276, 5-10=-231/276, 5-9=-966/879, 6-9=-1056/957 NOTES (11-12)TILLIUS S.K 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SYP No.2 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1656, ** No 3 7=1645 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 222 lb down and 283 lb up at 7-40 lb 103 lb down and 107 lb up at 9-0-12, 103 lb down and 107 lb up at 13-0-12, 103 lb NINEF 103 lb down and 107 lb up at 3-0-1.

107 lb up at 13-11-4, 103 lb down and 107 lb up at 15-11-4, and 103 lb down and 93 lb up at 9-0-12, 66 lb down and 93 lb up at 11-0-42 at 20-0-0 on top chord, and 261 lb down and 415 lb up at 7-0-0, 66 lb down and 93 lb up at 15-11-4, and 66 lb down and 93 lb up at 13-0-12, 66 lb down and 93 lb up at 13-0-12, 66 lb down and 93 lb up at 13-11-4, and 66 lb down and 93 lb up at 15-11-4 and 66 lb down and 93 lb up at 15-11-4 on bottom chord. The design/selection of such connection device(s) is the 107 lb up at 13-11-4, 103 lb down and 107 lb up at 15-11-4, and 103 lb down and 107 lb up at 17-11-4, and 262 lb down and 283 lb 10-0 STATE OF at 20-0-0 on top chord, and 261 lb down and 415 lb up at 7-0-0, 66 lb down and 93 lb up at 9-0-12, 66 lb down and 93 lb up at 11-0-12 FLORIDA Ib up at 17-11-4, and 261 lb down and 415 lb up at 19-11-4 on bottom chord. The design/selection of such connection device(s) is the S ONAL 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Minnin March 4,2010 Continued on page 2

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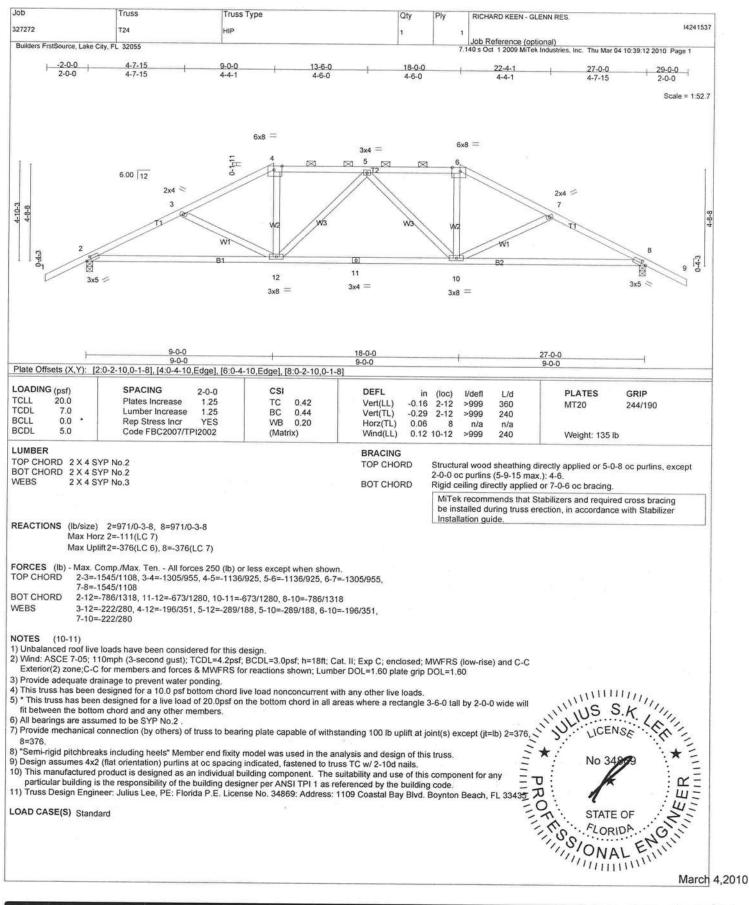
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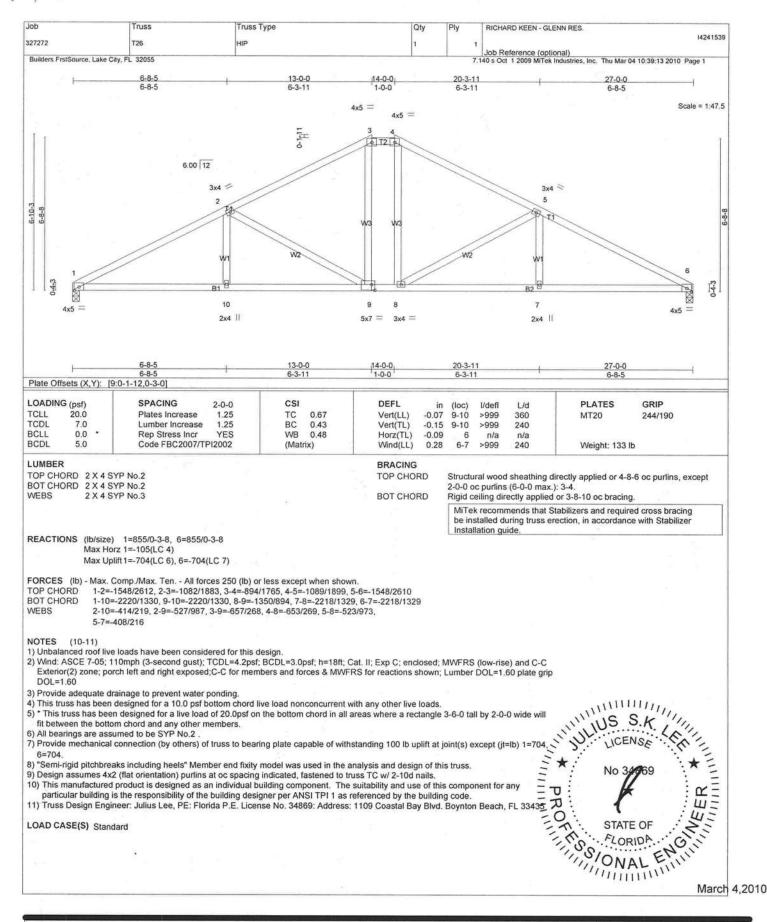
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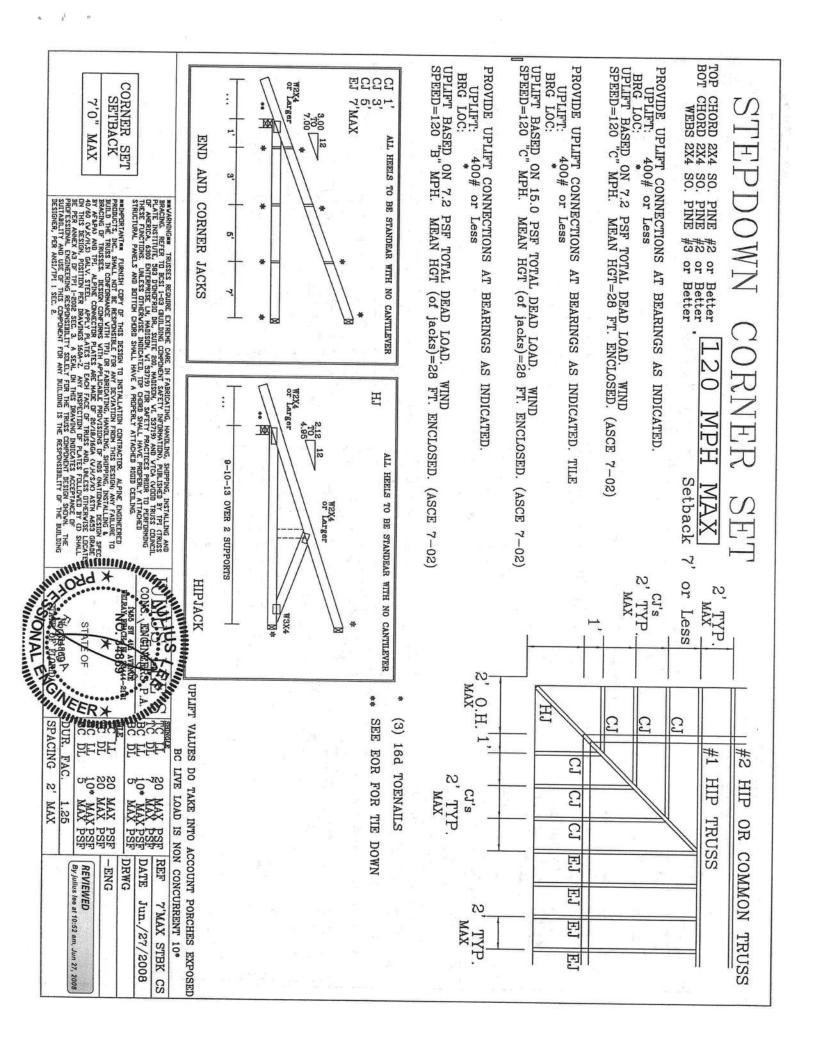
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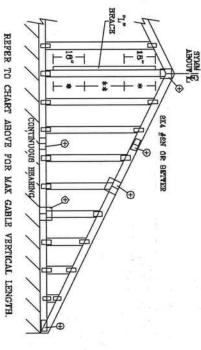
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ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE 0



VERTICAL LENGTH MAY BE
DOUBLED WINN DIAGONAL
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ERACT IS USED. CONNECT
ERACONAL ERACE FOR SEG 4
AT ERACH END. MAX WEB
TOTAL LEXIGIH IS 14*.

GABLE THUBS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DT-L #2 OB
BETTER DIAGONAL
BRACE, SINGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

CAL WEB

OUTLOOKERS WIN E' O' OVERHANG, OR 12" de uplait connections for 180 fla over vinuous bearing (5 psp vc dead load). AD DEPLECTION CRITERIA IS L/240.

ATTACE EACH 'L' BRACE WITH 104 NAILS.

* FOR (1) 'L' BRACE, SPACE NAILS AF 2' O.C.

* FOR (2) 'L' BRACES, SPACE NAILS AT 3' O.C.

EN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

EN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

PLYWOOD OVERHANG.

MEMBER LENGTH. T. BRACING MUST BE A MINIMUM OF BOX OF WEB

+ REFER TO COL	GREATER THAN	GREATER THAN 1	TESS THAN 4'	VERTICAL O	COLUMN TANKS
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	C E MONAPONGIN TRUSSES REBUIRE EXTREME CARE IN FARRICATING, HANDLING, SUPPONG, INSTALLING AND	CONS. ENGINEERS P.A.		DATE 11/26/09
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	By Julius lee at 12:00 pm, Jun 11, 2008	1		
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*	"MUMMINIMINIMINIMINIMINIMINIMINIMINIMINIM			

BOT CHORD 2X4 2X4 \$ 65 to 金金金 BETTER BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER. SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

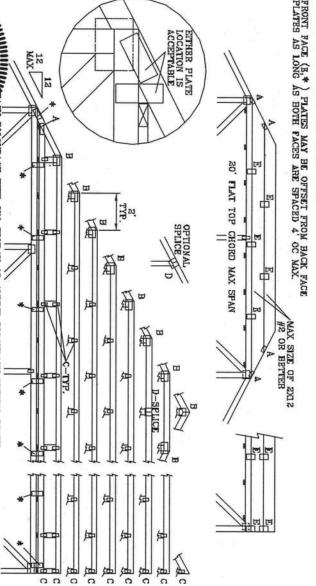
PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED HENEATH THE TOP CHORD OF SUPPORTING TRUSS.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C. WIND TC DL-6 FSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MEAN HGT, FEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-6 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND HC DL=6 PSF CLOSED EXP. C,



E	8	C	Ħ	>	TYPE	TOINT
4XB O	5 X 4	1.5X9	4X8	2X4	30'	
R 3X6 TI	6X6	1.5X4	5X6	2.5X4	34'	SPANS
4XB OR 3X6 TRULOX AT 4'	9X9	1.5X4	бхв	2.6X4	88,	SPANS UP TO
4' DC,	БХӨ	1.5X4	БХӨ	3X6	52'	

ATTACH TRULOX PLATES WITH (6) 0.120° X 1.975" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

1100	TOTAL	TINNEL	AUTOWIG AGMICANT
o' To 7'9"	Ö	1,6,4	NO BRACING
7'9" TO 10'	TO	10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	25		2x4 "T" DRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR HETTER, AND 80% LENGTH OF WEH MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PARRICATION. ATTACH TO SUPPORTING TRUSS TITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS. * PIGGYBACK SPECIAL PLATE 8 1/4" M

IUS LEE'S THIS DRAWING REPLACES DRAWINGS MAX LOADING 634,016 634,017 & 847,045 REF PIGGYBACK

1.15 DUR. 1.25 DUR. 1.33 DUR. 50 PSF 55 PSF AT 33 DUR. FAC. 47 PSF AT FAC. DATE -ENG DRWGMITEK STD PIGG I 09/12/07

SPACING

24.0

NO. 44869

NO. 44869

NO. 44869

NO. 54869

NO. 54869 CONS. DINEAN BENCH, IL. 33444 -2161 No: 94868 STATE OF FLORIDA

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

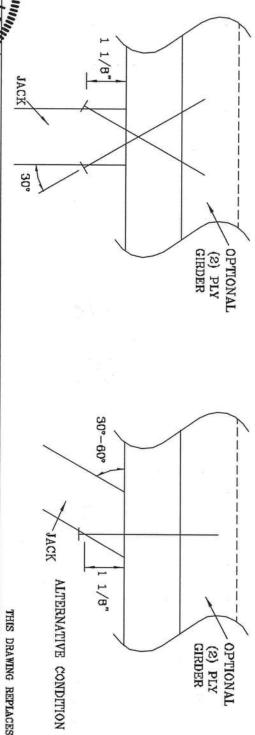
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

FRAMING INTO A SINGLE TOE-NAILED CONNECTION FOR JACK OR DOUBLE PLY SUPPORTING GIRDER.

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ALL VALUE	5	4	ಒ	N	TOE-NAILS	NUMBER OF
S MAY E	493#	394#	296#	187#	1 PLY	SOUTH
ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE
ED BY API	452#	361#	271#	181#	1 PLY	DOUGLAS
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
DURATION	390#	312#	234#	156#	1 PLY	
OF LOAD F	507#	406#	304#	203#	2 PLIES	HEM-FIR
ACTOR.	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR



THINING ON AL EMILIA	THE STATE OF	100 A	STATE OF	*	NO. 24869	CERG	White Control in	WHITE STATE
	249	By julius lee at 11:59 am, Jun 11, 2008	REVIEWED	SEDICITIZAL PARELS AND BRITCH OFFICE SHALL HAVE A PRIDERLY ATTACHED REED CELDING	TTE 200, NADISON, WL 33719) AND VTCA (WE CON, VI 33719) FOR SAFETY PRACTICES PRINT	WAY ARRONG ** TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND REACONG. REPER TO BEST 1-03 CRUILING COMPONENT SAFETY (METRONG THIS ISSEED BY TRE CRUISS	, in	JACK 30
	STATE OF FLORIDA	No: 34869	77	200	THUSS COLVOR. 1460 SW 4th AVENUE TO PERFORMING DELRAY HEACH, PL 83444-2161	CONS. ENGIN	JULIUS LEE'S TO IL	
12	SPACING	DUR. FAC. 1.00	TOT. LD. PSF	BC LL PSF	BC DL PSF	TC DL PSF		THIS DRAWING RI
16				-ENG JL	PSF DRWG CNTONALL103	DATE 09/12/07	PSF REF TOE-NAIL	THIS DRAWING REPLACES DRAWING 784040

TRULOX CONNECTION DETAIL

SHOWN (+). PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

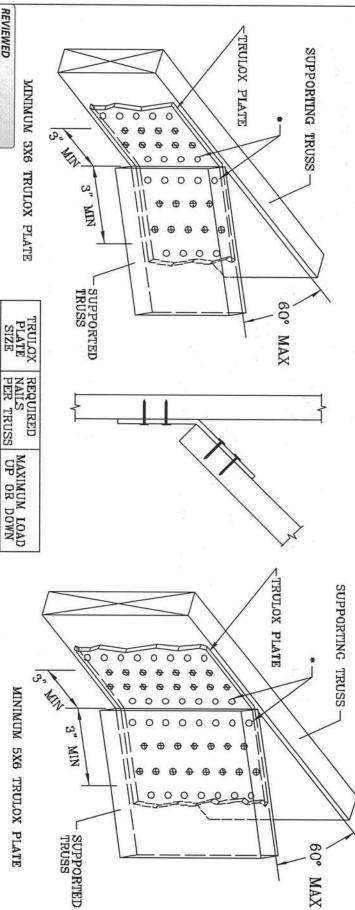
NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO, PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

MAX



NO. 44859

REVIEWED

By julius lee at 11:58 am, Jun 11, 2008

3X6

MAXIMUM LOAD UP OR DOWN

MINIMUM 5X6 TRULOX PLATE

15 9

#088 350#

JULIUS CONS. ENGIN

US LEE'S

THIS DRAWING REPLACES DRAWINGS 1,158,986 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

DRIVAL BEYON' IL' 32444-SIBI

DRWG DATE REF

CNTRULOX1103

TRULOX 11/26/09

-ENG

IL

No: 34869 STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

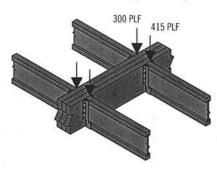
					Co	onnector Pattern		A STATE OF THE	
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F	
			L	DESCRIPTION OF THE PROPERTY OF	0.1	Extracorporal		1	
			2"				2'	2" 100 000	
		0							
Connector Type	Number of	Connector On-Center							
connector type	Rows	Spacing	2"				2"	2"	
			1 年				T	The second of	
			134"	134"	13/4" 31/2"	11/4" 31/4" 13/4"	31/2"	11/1"	
10-(0.1000 - 20)			31/2" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply	
10d (0.128" x 3")	2	12"	370	280	280	245	and the Hotel and the Hotel		
Nail ⁽¹⁾	3	12"	555	415	415	370			
1/4 4207		24"	505	380	520	465	860	340	
1/2" A307 Through Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425	
		16"	760	570	785	695	1,290	505	
SDS 1/4" x 31/2"(4)	2	24"	680	510	510	455			
		19.2"	850	640	640	565		And the second	
		16"	1,020	765	765	680			
SDS 1/4" x 6"(3)(4)	2	24"				455	465	455	
	2	19.2"				565	580	565	
		16"				680	695	680	
ÚSP WS35 (4)	2	24"	480	360	360	320		A CORRECTION	
		19.2"	600	450	450	400			
		16"	715	540	540	480			
USP WS6 (3)(4)	2		24"				350	525	350
		19.2"				440	660	440	
		16"				525	790	525	
33/8°° TrussLok(4)		24"	635	475	475	425			
	2	19.2"	795	595	595	530			
		16"	955	715	715	635		ELECTRIC SERVICE SERVI	
5"		24"		500	500	445	480	445	
TrussLok ⁽⁴⁾	2	19.2"		625	625	555	600	555	
Tusses.		16"		750	750	665	725	665	
63/4"		24"				445	620	445	
TrussLok(4)	2	19.2"		1 1 1 1	130	555	770	555	
		16"	00年6世纪10年6月	SOUTH STATE OF THE		665	925	665	

- Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be %6" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
 Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16-33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
 of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 134" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

