

DATE 12/05/2006

Columbia County Building Permit

PERMIT
000025275

This Permit Expires One Year From the Date of Issue

APPLICANT CHARLENE CRAWFORD PHONE 386.961.1086
ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024
OWNER GATEWAY DEVELOPERS OF LAKE CITY,LLC PHONE 386.961.1086
ADDRESS 195 SW FIELDSTONE COURT LAKE CITY FL 32055
CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141
LOCATION OF PROPERTY 90-W TO HEATHRIDGE,TL TO FIELDSTONE COURT,TR AND IT'S
THE 4TH LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 114400.00
HEATED FLOOR AREA 2288.00 TOTAL AREA 3259.00 HEIGHT 10.00 STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 7'12 FLOOR CONC
LAND USE & ZONING RSF-2 MAX. HEIGHT 35
Minimum Set Back Requirements: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-147 SUBDIVISION EMERALD COVE
LOT 47 BLOCK PHASE 1 UNIT TOTAL ACRES 0.50

000001267 CBC1253543
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
18"X32"MITERED 06-1034-N BLK JTH N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: 1 FOOT ABOVE ROAD. NOC ON FILE.

Check # or Cash 3815

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 575.00 CERTIFICATION FEE \$ 16.30 SURCHARGE FEE \$ 16.30
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 707.60
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number

TM File No: 05-631

Inst:2005022542 Date:09/14/2005 Time:14:45

Doc Stamp-Deed : 1360.10

DC, P. DeWitt Cason, Columbia County B:1050 P:841

WARRANTY DEED

This Warranty Deed, made this 7th day of September, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor and GATEWAY DEVELOPERS OF LAKE CITY, LLC, A Florida Limited Liability Company, whose Document number is L04000093284 and whose FEI number is 202222207 and whose post office address is 2806 West US Highway 90, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 43, 44, 45, 46 and 47, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

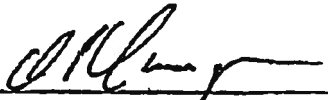


In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

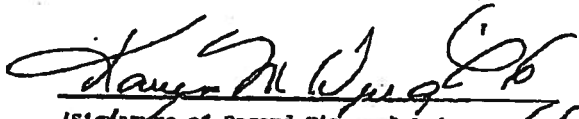
Signed, sealed and delivered in our presence:

D D P CORPORATION


(Signature of First Witness)
TERRY MCDAVID
(Typed Name of First Witness)

BY:  (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)


(Signature of Second Witness)
Karen M. Wright
(Typed Name of Second Witness)

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1380.10
DC,P.DeWitt Cason,Columbia County B:1058 P:842

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of September, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is personally known to me or who has produced ~~Personal Knowledge~~ as identification and who did not take an oath.

My Commission Expires:


Notary Public
Printed, typed, or stamped name:

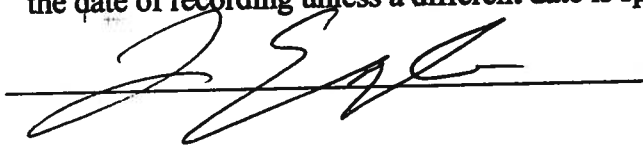


NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

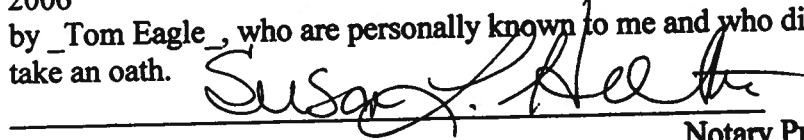
1. Description of property: Lot 47 Emerald Cove Subdivision
2. General description of improvement: Construction of Dwelling
3. Owner information:
 - a. Name and address: Gateway Developers of Lake City, LLC
872 SW Jaguar Drive
Lake City, FL 32025
 - b. Interest in property: Fee Simple
 - c. Name and address of fee simple title holder (if other than Owner): None
4. Contractor: James Mack Lipscomb
5. Surety n/a
 - a. Name and address: Inst: 2006027718 Date: 11/22/2006 Time: 13:31
DC, P. DeWitt Cason, Columbia County B: 1102 P: 2204
 - b. Amount of bond:
6. Lender: Mercantile
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None
8. In addition to himself, Owner designates _____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).



Signature of Owner

The foregoing instrument was acknowledged before me this 21 day of November, 2006

by Tom Eagle, who are personally known to me and who did not take an oath.



Notary Public

My commission expires: 5/19/2009



Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AARONNOTARY.com

Columbia County Building Permit Application

For Office Use Only Application # 0611-52 Date Received 11-22-06 By G Permit # 1267/2527
 Application Approved by - Zoning Official BLK Date 05.12.06 Plans Examiner OKJTH Date 11-24-06
 Flood Zone X Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES Low Den.
 Comments EX HENR

☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☒ State Road Info ☒ Parent Parcel # ☒ Development Permit

Name Authorized Person Signing Permit Charlene Crawford Fax 386 719 9586 Phone 386 867 0814

Address 1964 SW Cypress Lake rd Lake City FL 32024

Owners Name Gateway Development of Lake City LLC Phone 961-686

911 Address 195 SW Fieldstone Ct Lake City FL 32055

Contractors Name James M. Lipscomb Phone 623-9141

Address 255 SE Woods Terrace Lake City FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address GTC Design Group P.O. Box 117 Pine Hill FL 32064

Mortgage Lenders Name & Address N/A

Circle the correct power company FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-35-16-02438-147 Estimated Cost of Construction \$150,000.00

Subdivision Name Emerald Cove Lot 47 Block Unit Phase

Driving Directions Go west turn left on Heathridge. Turn Rt on Fieldstone, 4th lot on left

Type of Construction Stucco & Stone Number of Existing Dwellings on Property 0

Total Acreage .15 Lot Size .15 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 30 Side 28' Side 14' Rear 8'

Total Building Height Number of Stories 1 Heated Floor Area 2288 Roof Pitch 5

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter [Signature]

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 21 day of Nov. 2006

Personally known ✓ or Produced Identification

James M. Lipscomb
Contractor Signature

Contractors License Number 1253345
Competency Card Number DD431203
NOTARY STAMP [Signature] Expires: MAY 19, 2009
WWW.BARONNOTARY.COM

[Signature]
Notary Signature (Revised Sept. 2006)

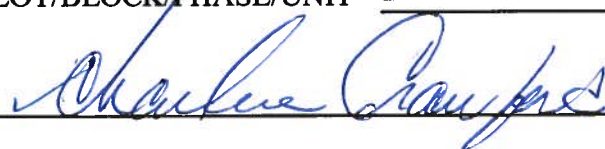
Columbia County Building Department Culvert Permit

Culvert Permit No.
000001267

DATE 12/05/2006 PARCEL ID # 33-3S-16-02438-147
APPLICANT CHARLENE CRAWFORD PHONE 386.961.1086
ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024
OWNER GATEWAY DEVELOPERS OF LAKE CITY, LLC PHONE 386.961.1086
ADDRESS 195 SW FIELDSTONE COURT LAKE CITY FL 32055
CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141
LOCATION OF PROPERTY 90-W TO HEATHRIDGE, TL TO FIELDSTONE COURT, TR AND IT'S T
4TH LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 47 1

SIGNATURE



INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.

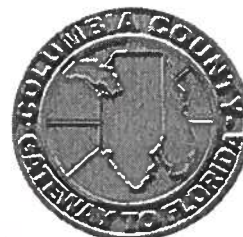


Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.

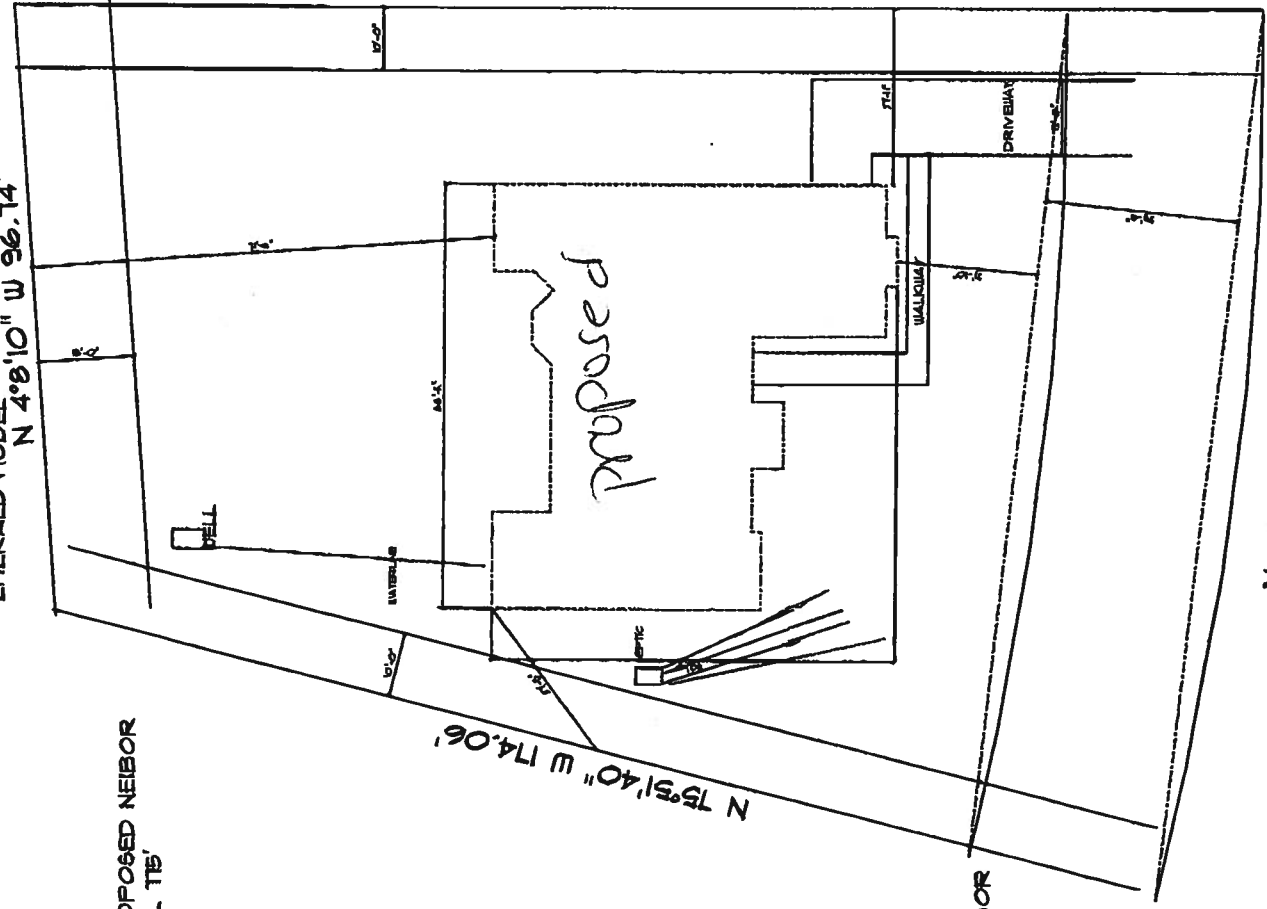
135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



06-1034-N
SITE PLAN

9



N 6°16'20" E 140.31'

FIELDSTONE COURT

Sallye Thayer

$$1'' = 30'$$

60/11/21
Hofmann
S

6-7-74

Columbia CHD

1157

2-4th

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXXXXXXXXXXXXXXXXXX~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

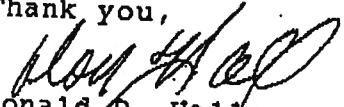
June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,


Donald D. Hall
DDH/jk

EMBER MASTER™

VENT-FREE GAS LOG HEATERS

The FMI Ember Master Gas Log Heaters offer you what ordinary gas logs and wood cannot...the warmth, cleanliness and economy of vent-free space heating combined with the beauty and convenience of gas logs. FMI's unique, clean burning design and precision positioned logs permit vent-free operation with the fireplace damper closed. With no venting required, there is no heat loss up the chimney. Plus they require no electricity, so you always have heat - even in the event of power outage.

A Model To Fit Every Need

FMI's Ember Master Gas Log Heaters are available in two log sizes: 18" and 24", each model is available in propane and natural gas. Choose from two types of controls: *Variable Manually Controlled Models* - Allow you to choose any heat setting and flame height desired by simply turning the control knob. *Remote Control "Ready" Models* - Offer you heat at the push of a button or the flip of a switch. FMI's Remote Control Ready Heaters offer you the choice of four types of remote controls as well as the ability to operate the log heater with a variable manual control. Optional remote accessories include: the hand held thermostatic remote control, the hand held on/off remote control, the wall mount thermostat control and the wall mount on/off switch.

FMI Vent-Free Gas Logs install in any masonry, manufactured solid fuel burning fireplace, or AGA or UL certified Vent-Free Universal Firebox. All (manual variable) log heaters are also listed to the ANSI Z21.60 Vent-Free Log Decorative Standard.

Safety Features Built Into Every Gas Log Heater

FMI Vent-Free Gas Log Heaters are designed certified by the American Gas Association (ANSI Z21.11.2) and meet or exceed all regulations and safety performance standards for vent-free gas heaters. Additionally, these log heaters perform well within nationally recognized guidelines for indoor air quality.

The dual-purpose safety pilot system protects against oxygen depletion and any interruption in the fuel supply. If either occurs, it shuts off the gas flow to the burner turning the heater off. An internal pressure regulator controls fluctuations in your gas pressure. These features ensure clean and reliable heat without the worry and inconvenience of vented gas logs or burning wood.

Log Size	No. of Logs	Part/Model Number		Btu Output	Shipping Wt.
		Natural	Propane		
Variable Manually Controlled Models					
18"	5	01984/VFN18MV	01985/VFP18MV	16,000 to 26,000	26 Lbs.
24"	6	01986/VFN24MV	01987/VFP24MV	20,000 to 33,000	28 Lbs.
Remote Control Ready/Fill/Full Models					
18"	5	01988/VFN18R	01989/VFP18R	16,000 to 26,000	32 Lbs.
24"	6	01990/VFN24R	01991/VFP24R	20,000 to 33,000	34 Lbs.
Remote Control Accessories (Must be purchased Separately)					
Part/Model Number		Description			
01994/FHRC		Receiver and Hand Held Thermostat Remote Control Kit			3 Lbs.
01995/FHRC		Receiver and Hand Held On/Off Remote Control Kit			3 Lbs.
01996/FWMT1		Wall Mount Thermostat Control Kit			1Lb.
01997/FWMS2		Wall Mount - On/Off Switch Kit			1Lb.
Accessories					
01244/HDABK		Hood - Flat Black Enamel - Adjustable 28" to 49"			5 Lbs.
01245/HDABR		Hood - Polished Solid Brass - Adjustable 28" to 49"			5 Lbs.

Log Sizing Requirements

Log Size	Minimum Firebox Size				Gas Connection
	Height	Depth	Front Width	Rear Width at 4" Depth	
18"	17"	14"	20"	20"	1/2" NPT
24"	17"	14"	26"	21"	1/2" NPT

A fireplace hood accessory may be required to deflect heat away from the mantel shelf. Firebox opening: from 28" to 49" wide. Install easily with glass doors or to the front of the fireplace opening. Refer to the Ember Master Vent-Free Gas Log Owner's Manual for more details.

IMPORTANT

- Installation must be done by qualified service persons.
- Read Owner's Manual before using.
- Check local codes and ordinances for permitted uses.
- Approved for manufactured (mobile) home installation. Not for use in recreational vehicles.
- We reserve the right to amend product specifications without notice.
- Use with adequate air (ventilation) only. Humidifies while it heats.
- Provides water vapor in the area heated. Refer to Owner's Manual for specifics.
- Operating heater at very high elevation could cause nuisance outages. Product cannot be converted between fuel types.
- The only warranty we offer is our standard warranty.
- Please read the warranty for any limitations or disclaimers.
- All products carry a one year warranty.



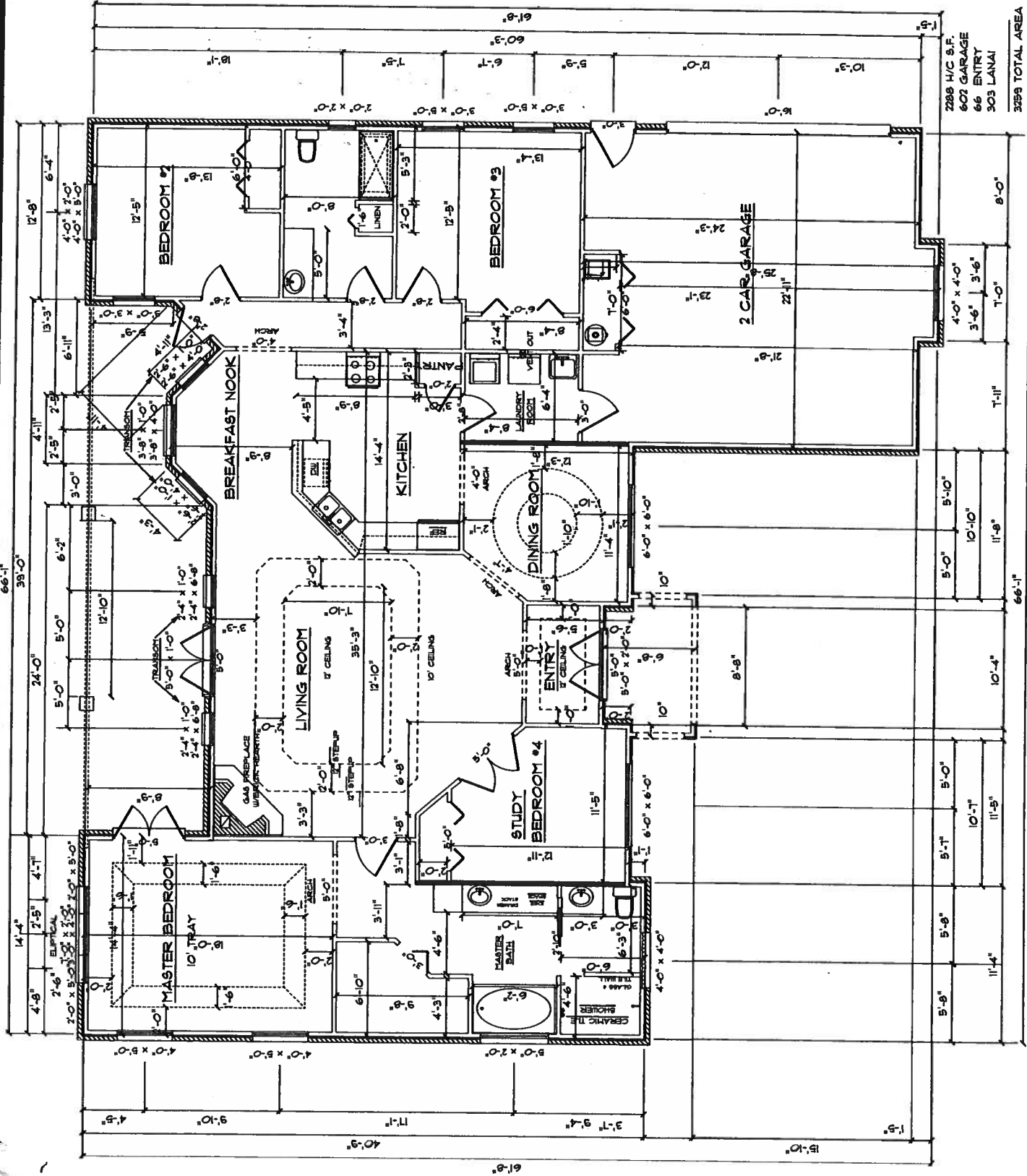
Made in U.S.A.



FMI Fireplaces Manufacturers Incorporated

P.O. #58488 5/99 Printed in U.S.A.
http://www.fmionline.com

For More Information
Call 1-800-888-2050



2298 H/C S.F.
 602 GARAGE
 66 ENTRY
 303 LANAI
 3299 TOTAL AREA

EMERALD COVI



COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ----- 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner
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☐	☐
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All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.

☒	☐
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Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.

☒	☐
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Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

☒	☐
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Wind-load Engineering Summary, calculations and any details required
Plans or specifications must state compliance with FBC Section 1609.

The following information must be shown as per section 1603.1.4 FBC

- a. Basic wind speed (3-second gust), miles per hour (km/hr).
- b. Wind importance factor, I_w , and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.
- c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.
- d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
- e. Components and Cladding. The design wind pressures in terms of psf (kN/m^2) to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional.

Elevations including:

☒	☐
☒	☐
☒	☐

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation

- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned.
- b) Shear walls identified.
- c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
- d) Show safety glazing of glass, where required by code.
- e) Identify egress windows in bedrooms, and size.
- f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
- g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
- h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
- All posts and/or column footing including size and reinforcing
- Any special support required by soil analysis such as piling
- Location of any vertical steel.

Roof System:

- a) **Truss package including:**
 - 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
 - 2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) **Conventional Framing Layout including:**
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 1. All materials making up wall
 2. Block size and mortar type with size and spacing of reinforcement
 3. Lintel, tie-beam sizes and reinforcement
 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
 6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 7. Fire resistant construction (if required)
 8. Fireproofing requirements
 9. Shoe type of termite treatment (termiteicide or alternative method)
 10. Slab on grade
 - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 11. Indicate where pressure treated wood will be placed
 12. Provide insulation R value for the following:

- a. Attic space
- b. Exterior wall cavity
- c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
 - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms
- h) Exhaust fans in bathroom

HVAC information

- a) **Energy Calculations** (dimensions shall match plans)
- b) Manual J sizing equipment or equivalent computation
- c) **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water**

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	Emerald Model	Builder:	
Address:		Permitting Office:	COLUMBIA
City, State:		Permit Number:	25275
Owner:	Gateway Development	Jurisdiction Number:	224000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 52.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2288 ft ²	13. Heating systems	
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		a. Electric Heat Pump	Cap: 52.0 kBtu/hr
a. U-factor:	Description Area		HSPF: 7.00
(or Single or Double DEFAULT)	7a. (Dble Default) 348.1 ft ²	b. N/A	
b. SHGC:		c. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 348.1 ft ²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 40.0 gallons
a. Slab-On-Grade Edge Insulation	R=0.0, 237.6(p) ft		EF: 0.97
b. N/A		b. N/A	
c. N/A		c. Conservation credits	
9. Wall types		(HR-Heat recovery, Solar	
a. Frame, Wood, Exterior	R=13.0, 1410.7 ft ²	DHP-Dedicated heat pump)	
b. N/A		15. HVAC credits	
c. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
d. N/A		HF-Whole house fan,	
e. N/A		PT-Programmable Thermostat,	
10. Ceiling types		MZ-C-Multizone cooling,	
a. Under Attic	R=30.0, 3407.0 ft ²	MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Con. Ret: Con. AH: Garage	Sup. R=6.0, 150.0 ft		
b. N/A			

Glass/Floor Area: 0.15

Total as-built points: 33931

Total base points: 33960

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: GARY GILL

DATE: 10/16/02

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X SPM X SOF = Points			
.18	2288.0	20.04	8253.3	Double, Clear	E	0.0	0.0	32.0	42.06	1.00	1346.0
				Double, Clear	E	0.0	0.0	72.0	42.06	1.00	3028.6
				Double, Clear	E	0.0	0.0	10.0	42.06	1.00	420.6
				Double, Clear	S	0.0	0.0	30.0	35.87	1.00	1076.0
				Double, Clear	S	0.0	0.0	4.0	35.87	1.00	143.5
				Double, Clear	W	0.0	0.0	20.0	38.52	1.00	770.5
				Double, Clear	W	0.0	0.0	8.0	38.52	1.00	308.2
				Double, Clear	W	0.0	0.0	14.7	38.52	1.00	565.5
				Double, Clear	W	0.0	0.0	3.7	38.52	1.00	141.4
				Double, Clear	SW	0.0	0.0	10.0	40.16	1.00	401.6
				Double, Clear	SW	0.0	0.0	2.5	40.16	1.00	100.4
				Double, Clear	NW	0.0	0.0	10.0	25.97	1.00	259.7
				Double, Clear	NW	0.0	0.0	2.5	25.97	1.00	64.9
				Double, Clear	W	0.0	0.0	31.1	38.52	1.00	1196.6
				Double, Clear	W	0.0	0.0	4.7	38.52	1.00	179.5
				Double, Clear	W	0.0	0.0	20.0	38.52	1.00	770.5
				Double, Clear	W	0.0	0.0	12.0	38.52	1.00	462.3
				Double, Clear	W	0.0	0.0	2.0	38.52	1.00	77.0
				Double, Clear	N	0.0	0.0	10.0	19.20	1.00	192.0
				Double, Clear	N	0.0	0.0	40.0	19.20	1.00	768.0
				Double, Clear	N	0.0	0.0	9.0	19.20	1.00	172.8
				As-Built Total:		348.1			12445.6		
WALL TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM = Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior		13.0	1410.7	1.50	2116.0		
Exterior	1410.7	1.70	2398.2								
Base Total:		1410.7	2398.2	As-Built Total:		1410.7		2116.0			
DOOR TYPES				Area X BSPM = Points		Type		Area X SPM = Points			
Adjacent	0.0	0.00	0.0	Exterior Wood		100.1	6.10	610.3			
Exterior	140.1	4.10	574.2								
Base Total:		140.1	574.2	As-Built Total:		140.1		854.3			
CEILING TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM X SCM = Points	
Under Attic	2288.0	1.73	3958.2	Under Attic		30.0	3407.0	1.73 X 1.00	5894.1		
Base Total:		2288.0	3958.2	As-Built Total:		3407.0		5894.1			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT			
FLOOR TYPES	Area	X BSPM	= Points	Type	R-Value	Area	X SPM = Points
Slab	237.6(p)	-37.0	-8792.3	Slab-On-Grade Edge Insulation	0.0	237.6(p)	-41.20 -9790.4
Raised	0.0	0.00	0.0				
Base Total:			-8792.3	As-Built Total:			237.6 -9790.4
INFILTRATION Area X BSPM = Points				Area X SPM = Points			
	2288.0	10.21	23360.5			2288.0	10.21 23360.5
Summer Base Points: 29752.1				Summer As-Built Points: 34880.2			
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier X Credit Multiplier = Cooling Points
29752.1	0.4266	12692.2		34880.2	1.00	1.147	0.263 1.000 10503.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2288.0	12.74	5246.8	Double, Clear	E	0.0	0.0	32.0	18.79	1.00	601.4
				Double, Clear	E	0.0	0.0	72.0	18.79	1.00	1353.1
				Double, Clear	E	0.0	0.0	10.0	18.79	1.00	187.9
				Double, Clear	S	0.0	0.0	30.0	13.30	1.00	398.9
				Double, Clear	S	0.0	0.0	4.0	13.30	1.00	53.2
				Double, Clear	W	0.0	0.0	20.0	20.73	1.00	414.6
				Double, Clear	W	0.0	0.0	8.0	20.73	1.00	165.8
				Double, Clear	W	0.0	0.0	14.7	20.73	1.00	304.3
				Double, Clear	W	0.0	0.0	3.7	20.73	1.00	76.1
				Double, Clear	SW	0.0	0.0	10.0	16.74	1.00	167.4
				Double, Clear	SW	0.0	0.0	2.5	16.74	1.00	41.8
				Double, Clear	NW	0.0	0.0	10.0	24.30	1.00	243.0
				Double, Clear	NW	0.0	0.0	2.5	24.30	1.00	60.7
				Double, Clear	W	0.0	0.0	31.1	20.73	1.00	643.8
				Double, Clear	W	0.0	0.0	4.7	20.73	1.00	96.6
				Double, Clear	W	0.0	0.0	20.0	20.73	1.00	414.6
				Double, Clear	W	0.0	0.0	12.0	20.73	1.00	248.7
				Double, Clear	W	0.0	0.0	2.0	20.73	1.00	41.5
				Double, Clear	N	0.0	0.0	10.0	24.58	1.00	245.8
				Double, Clear	N	0.0	0.0	40.0	24.58	1.00	983.1
				Double, Clear	N	0.0	0.0	9.0	24.58	1.00	221.2
				As-Built Total:				348.1	6963.4		
WALL TYPES				Type		R-Value		Area X WPM = Points			
Area X BWPM = Points											
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior		13.0		1410.7		3.40 4796.4	
Exterior	1410.7	3.70	5219.6								
Base Total:				As-Built Total:				1410.7		4796.4	
DOOR TYPES				Type		R-Value		Area X WPM = Points			
Area X BWPM = Points											
Adjacent	0.0	0.00	0.0	Exterior Wood				100.1		12.30 1230.6	
Exterior	140.1	8.40	1176.4	Exterior Wood				40.0		12.30 492.0	
Base Total:				As-Built Total:				140.1		1722.6	
CEILING TYPES				Type		R-Value		Area X WPM X WCM = Points			
Area X BWPM = Points											
Under Attic	2288.0	2.05	4690.4	Under Attic		30.0		3407.0		2.05 X 1.00 6984.3	
Base Total:				As-Built Total:				3407.0		6984.3	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
FLOOR TYPES Area X BWPM = Points				Type	R-Value	Area X	WPM	=	Points
Slab	237.6(p)	8.9	2114.9	Slab-On-Grade Edge Insulation	0.0	237.6(p)	18.80		4467.4
Raised	0.0	0.00	0.0						
Base Total:			2114.9	As-Built Total:			237.6		4467.4
INFILTRATION Area X BWPM = Points				Area X WPM = Points					
	2288.0	-0.59	-1349.9			2288.0	-0.59		-1349.9
Winter Base Points:			17098.2	Winter As-Built Points:					23584.2
Total Winter X System = Heating Points Multiplier Points				Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points					
				(System - Points)	(DM x DSM x AHU)				
17098.2	0.6274		10727.4	(sys 1: Electric Heat Pump 52000 btuh.,EFF(7.0) Ducts:Con(S),Con(R),Gar(AH),R6.0					
				23584.2	1.000	(1.000 x 1.169 x 1.00)	0.487	1.000	13430.5
				23584.2	1.00	1.169	0.487	1.000	13430.5

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING									
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank X Ratio	Multiplier X Credit	= Total Multiplier
4		2635.00	10540.0	40.0	0.97	4	1.00	2499.18	1.00
				As-Built Total:					9996.7

CODE COMPLIANCE STATUS							
BASE				AS-BUILT			
Cooling Points	+	Heating Points	= Total Points	Cooling Points	+	Heating Points	= Total Points
12692		10727	33960	10504		13431	33931

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circ breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Project Information for:

Builder: LIPSCOMB EAGLE
 Lot: LOT 47 EMERALD COVE
 Subdivision: N/A
 County or City: COLUMBIA COUNTY
 Truss Page Count: 42

Date: 5/31/2006
 Start Number: 1528

Truss Design Load Information (UNO)

Design Program: MiTek 5.2 / 6.2

Gravity Wind Building Code: FBC2004
 Roof (psf): 42 Wind Standard: ASCE 7-02
 Floor (psf): 55 Wind Speed (mph): 110

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LIPSCOMB, JAMES MACK CBC1253543
 Address: 255 SE WOODS TERRACE
 LAKE CITY, FL 32025

Designer: 81

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987

Company: Structural Engineering and Inspections, Inc. EB 9196
 Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	EJ7	0531061528	5/31/2006	41	T26	0531061568	5/31/2006
2	EJ7A	0531061529	5/31/2006	42	T27	0531061569	5/31/2006
3	EJ7B	0531061530	5/31/2006				
4	EJ7G	0531061531	5/31/2006				
5	EJ7T	0531061532	5/31/2006				
6	PB1	0531061533	5/31/2006				
7	PB2	0531061534	5/31/2006				
8	PB3	0531061535	5/31/2006				
9	T01G	0531061536	5/31/2006				
10	T02	0531061537	5/31/2006				
11	T02A	0531061538	5/31/2006				
12	T02G	0531061539	5/31/2006				
13	T03	0531061540	5/31/2006				
14	T03G	0531061541	5/31/2006				
15	T04	0531061542	5/31/2006				
16	T04G	0531061543	5/31/2006				
17	T05	0531061544	5/31/2006				
18	T05A	0531061545	5/31/2006				
19	T05B	0531061546	5/31/2006				
20	T05C	0531061547	5/31/2006				
21	T05G	0531061548	5/31/2006				
22	T06	0531061549	5/31/2006				
23	T07	0531061550	5/31/2006				
24	T08	0531061551	5/31/2006				
25	T09	0531061552	5/31/2006				
26	T10	0531061553	5/31/2006				
27	T11	0531061554	5/31/2006				
28	T12	0531061555	5/31/2006				
29	T14	0531061556	5/31/2006				
30	T15	0531061557	5/31/2006				
31	T16	0531061558	5/31/2006				
32	T17	0531061559	5/31/2006				
33	T18	0531061560	5/31/2006				
34	T19	0531061561	5/31/2006				
35	T20	0531061562	5/31/2006				
36	T21	0531061563	5/31/2006				
37	T22	0531061564	5/31/2006				
38	T23	0531061565	5/31/2006				
39	T24	0531061566	5/31/2006				
40	T25	0531061567	5/31/2006				

MAY 31 2006


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Licensee Details

Licensee Information

Name: LIPSCOMB, JAMES MACK (Primary Name)
LIPSCOMB & EAGLE DEVELOPMENT INC (DBA Name)
Main Address: 255 SE WOODS TERRACE
 LAKE CITY Florida 32025
County: COLUMBIA

License Mailing:

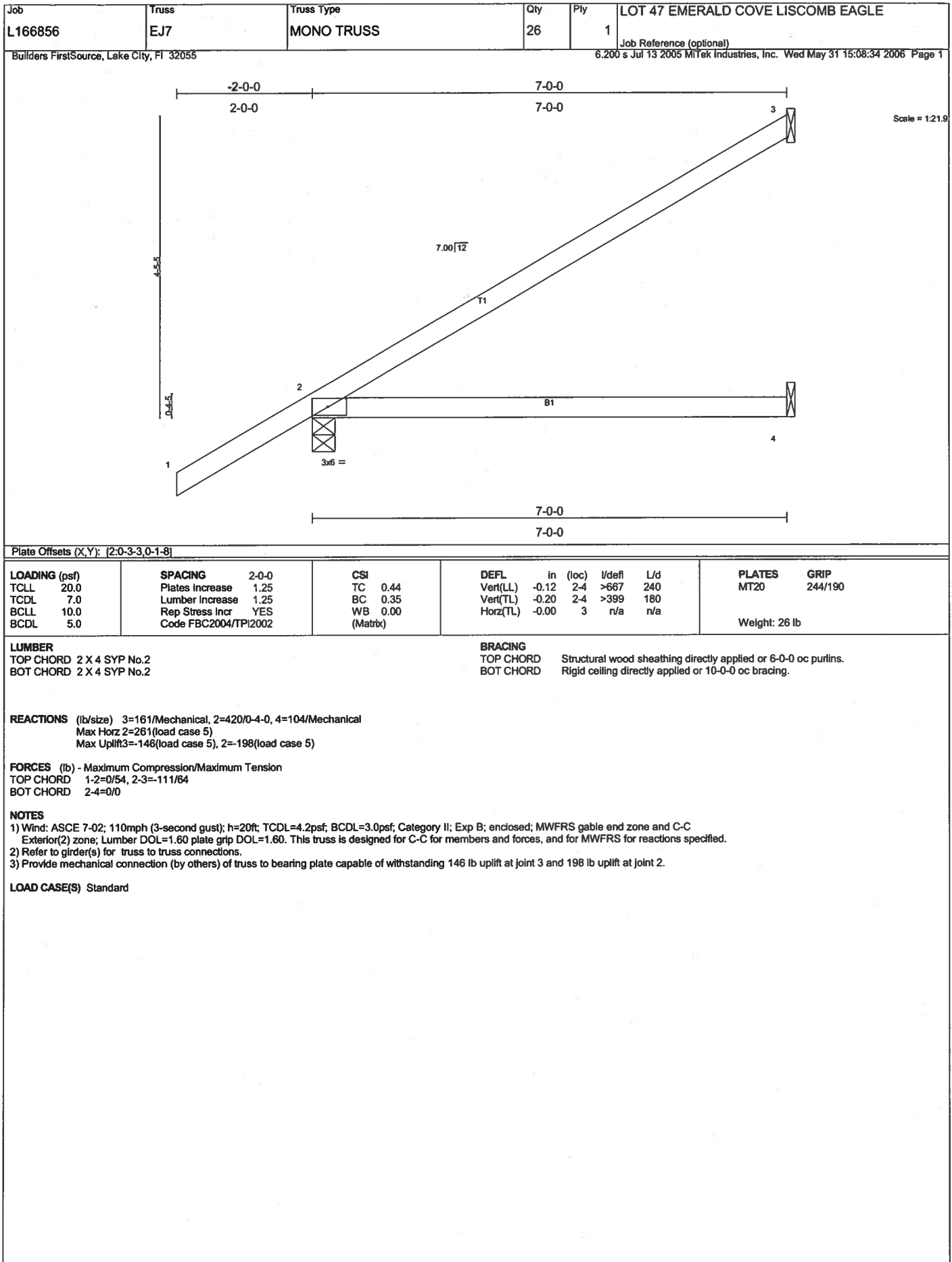
LicenseLocation: 2806 U S HWY WEST SUITE 101
 LAKE CITY FL 32055
County: COLUMBIA

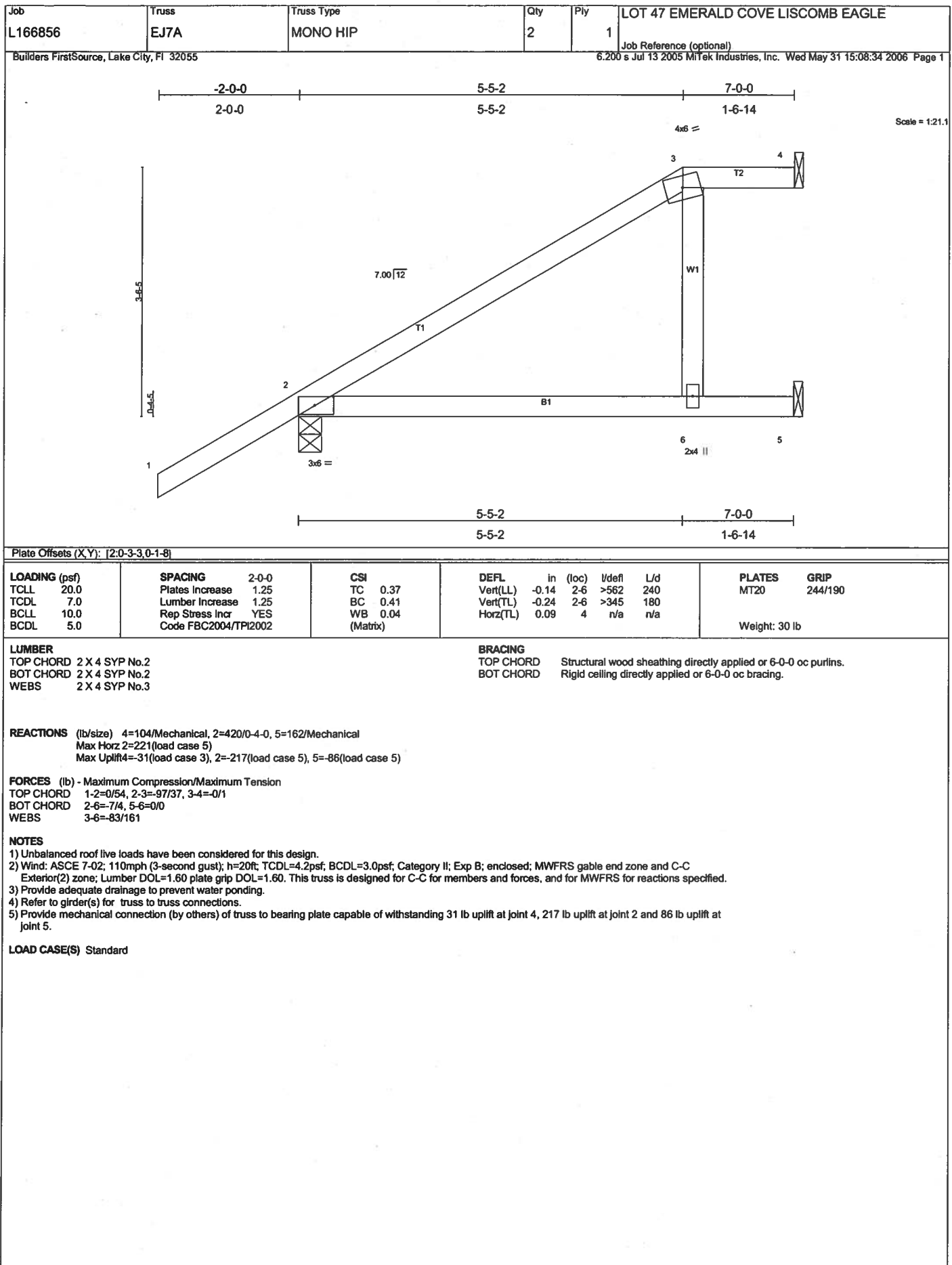
License Information

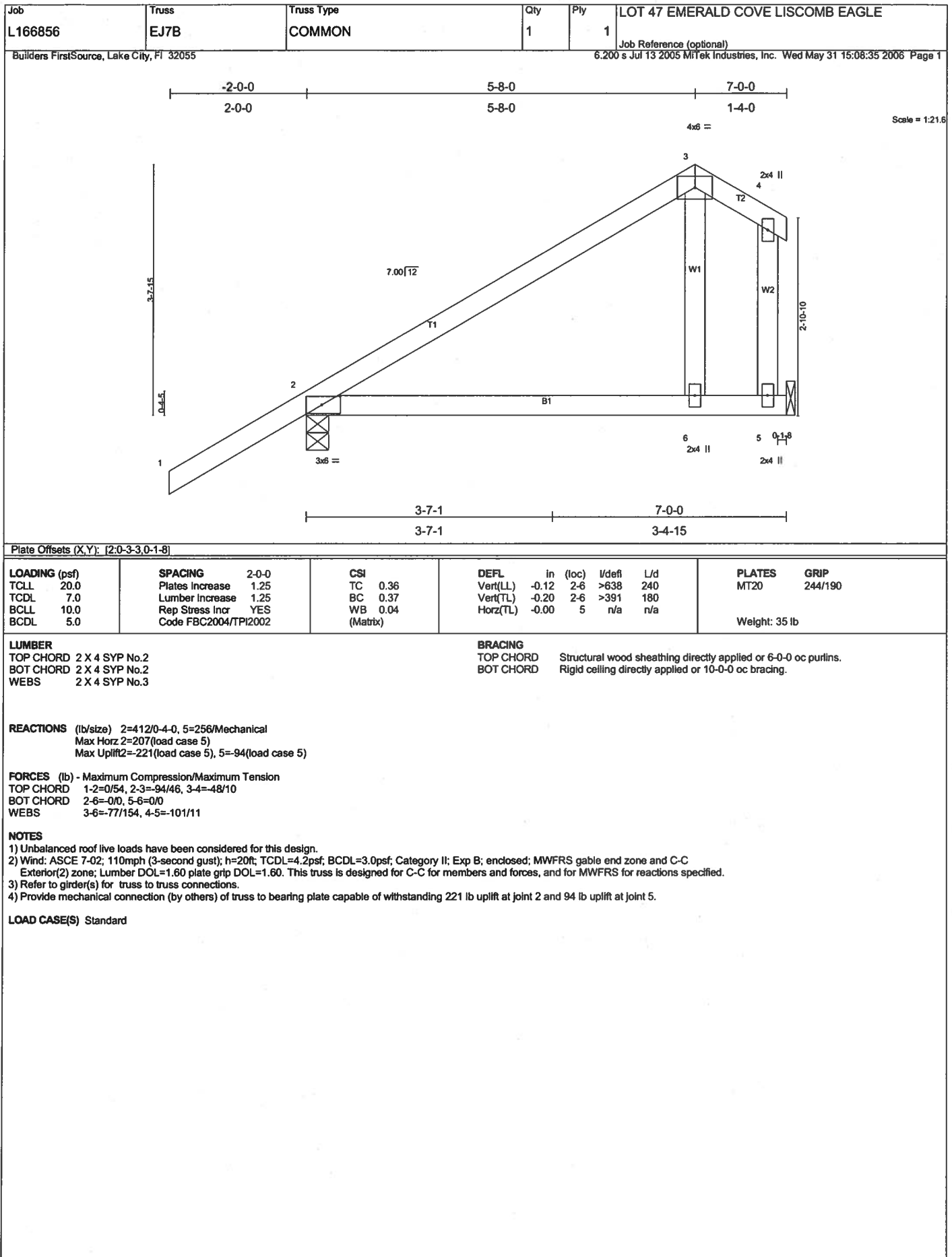
License Type: Certified Building Contractor
Rank: Cert Building
License Number: CBC1253543
Status: Current,Active
Licensure Date: 11/02/2005
Expires: 08/31/2006

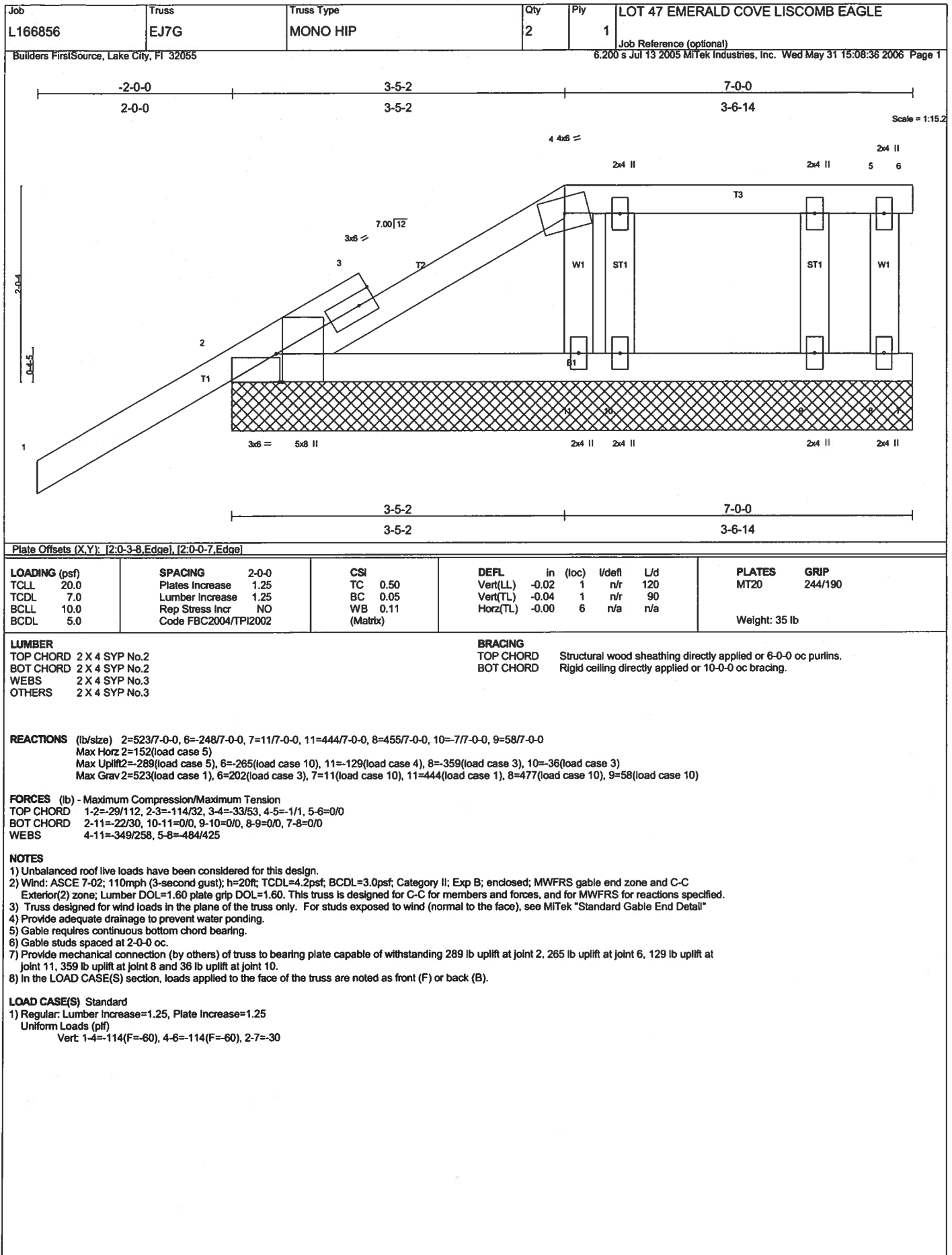
Special Qualifications	Qualification Effective
Qualified Business License Required	11/02/2005

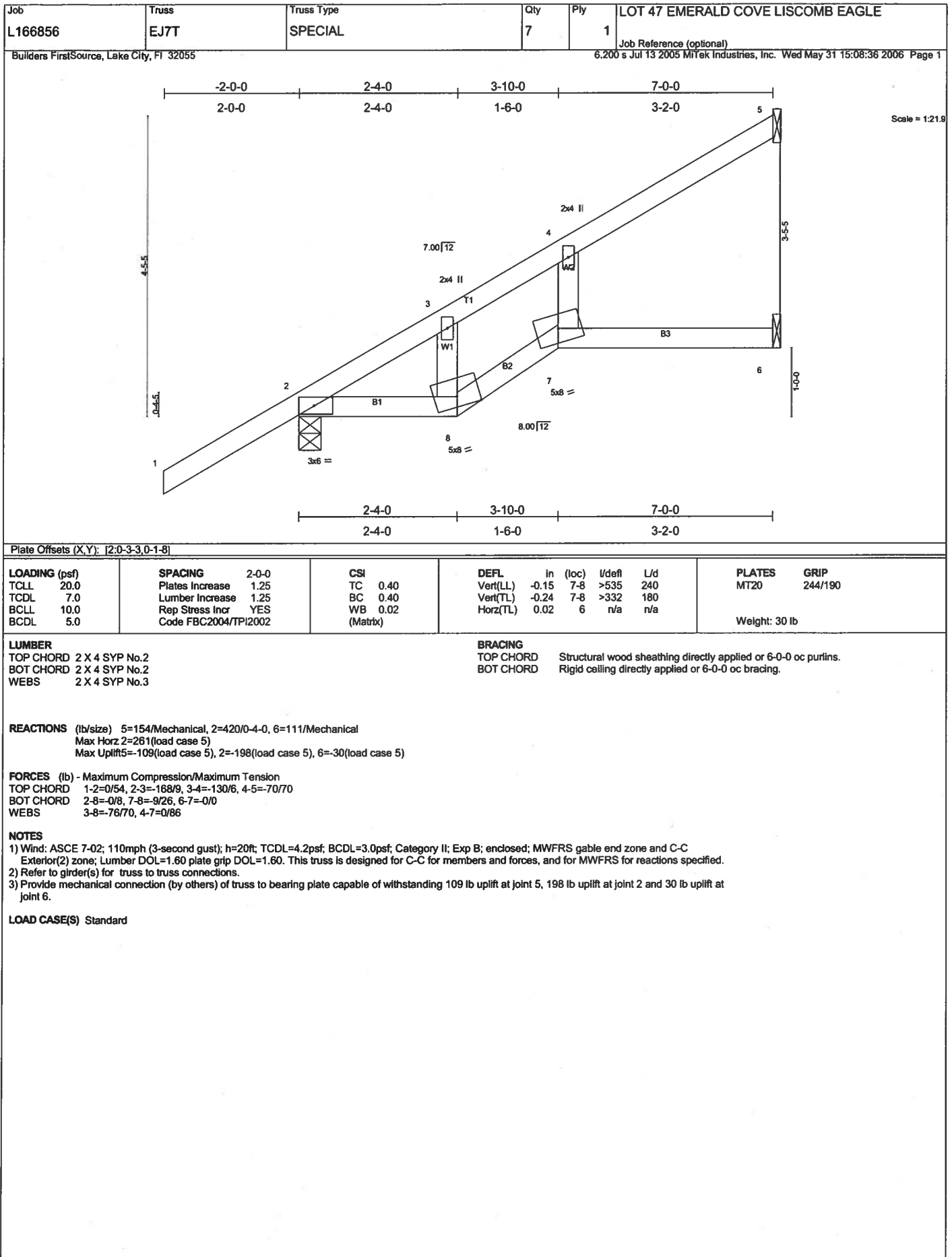
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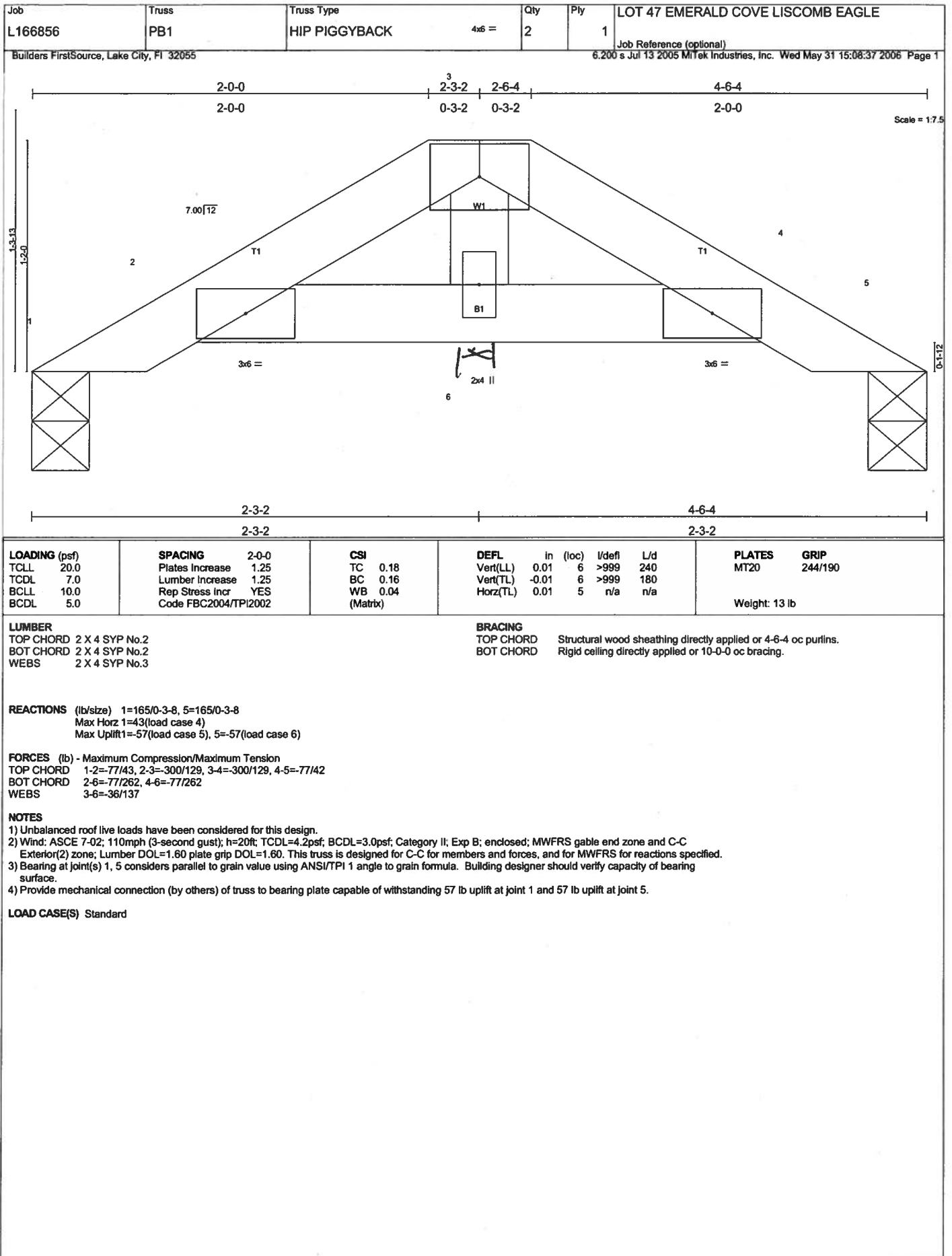












Job L166856	Truss PB2	Truss Type PIGGYBACK	Qty 9	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 15:08:37 2006 Page 1		

Scale = 1:7.5

Plate Offsets (X,Y): {3:0-3:0,Edge}					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL) 0.01 2-4 >999	240
TCDL 7.0	Lumber Increase	1.25	BC 0.13	Vert(TL) -0.02 2-4 >999	180
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL) 0.01 5 n/a	n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
			PLATES GRIP		
			MT20 244/190		
			Weight: 12 lb		

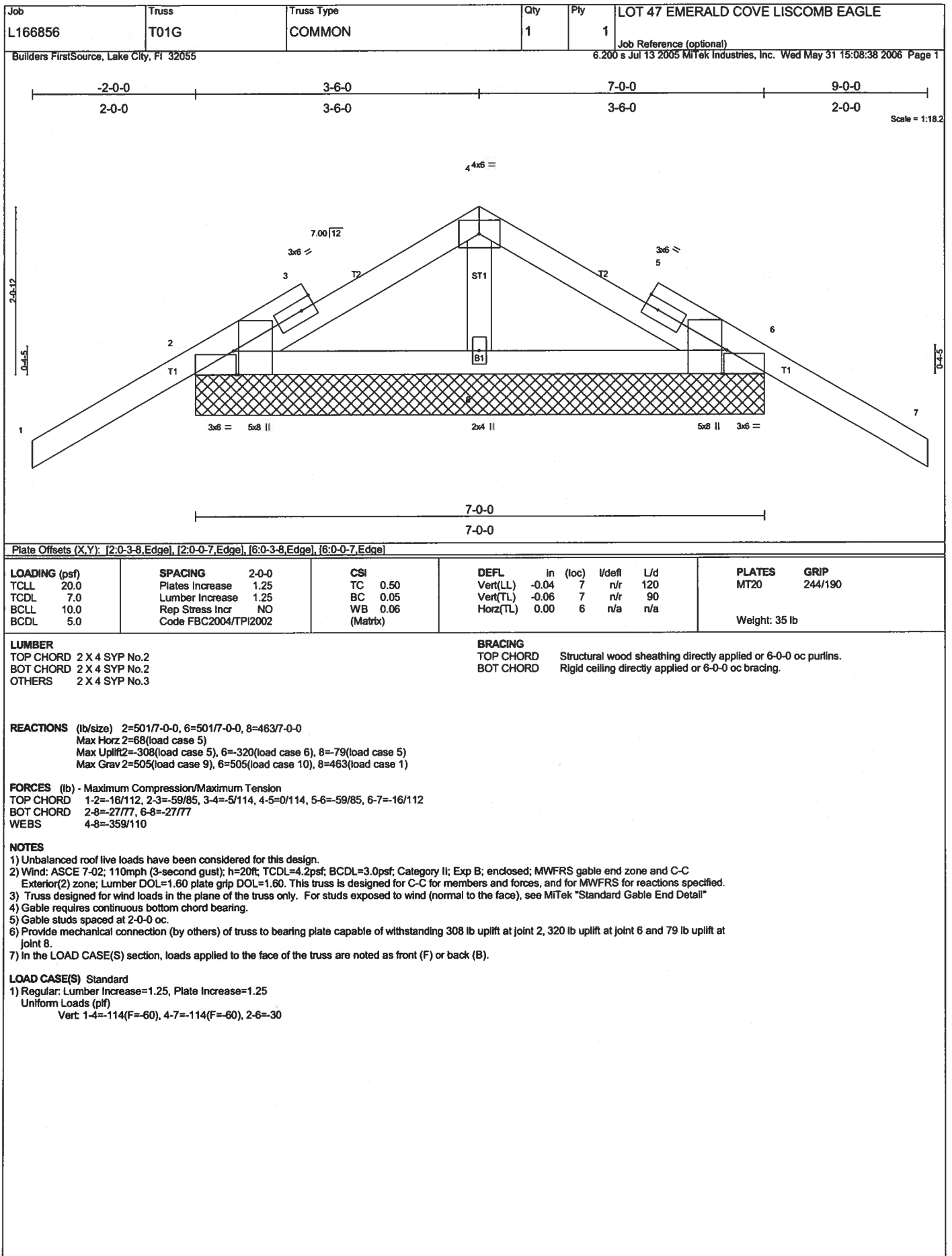
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
---	---

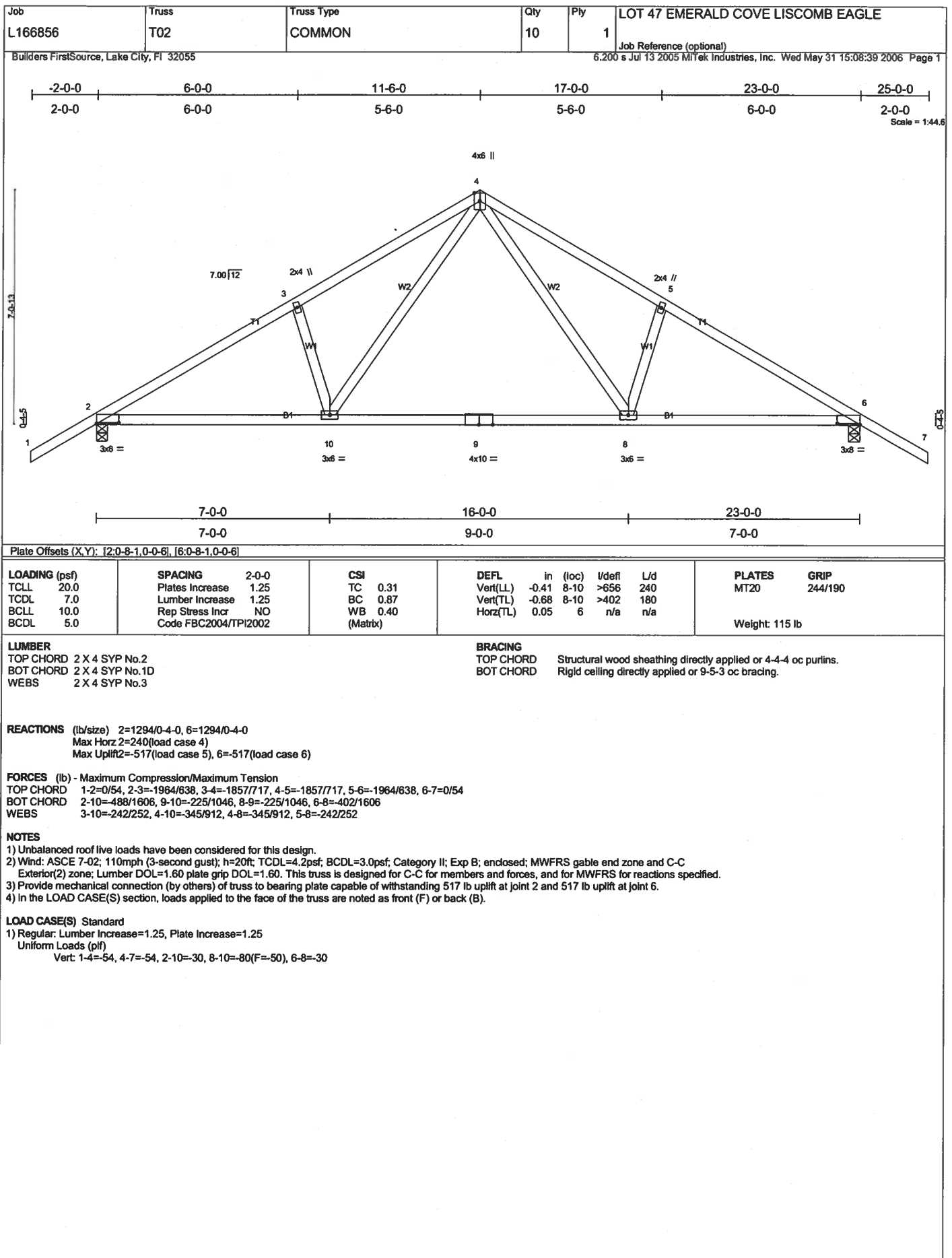
REACTIONS (lb/size) 1=165/0-3-8, 5=165/0-3-8
 Max Horz 1=43/load case 4)
 Max Uplift 1=57/load case 5), 5=57/load case 6)

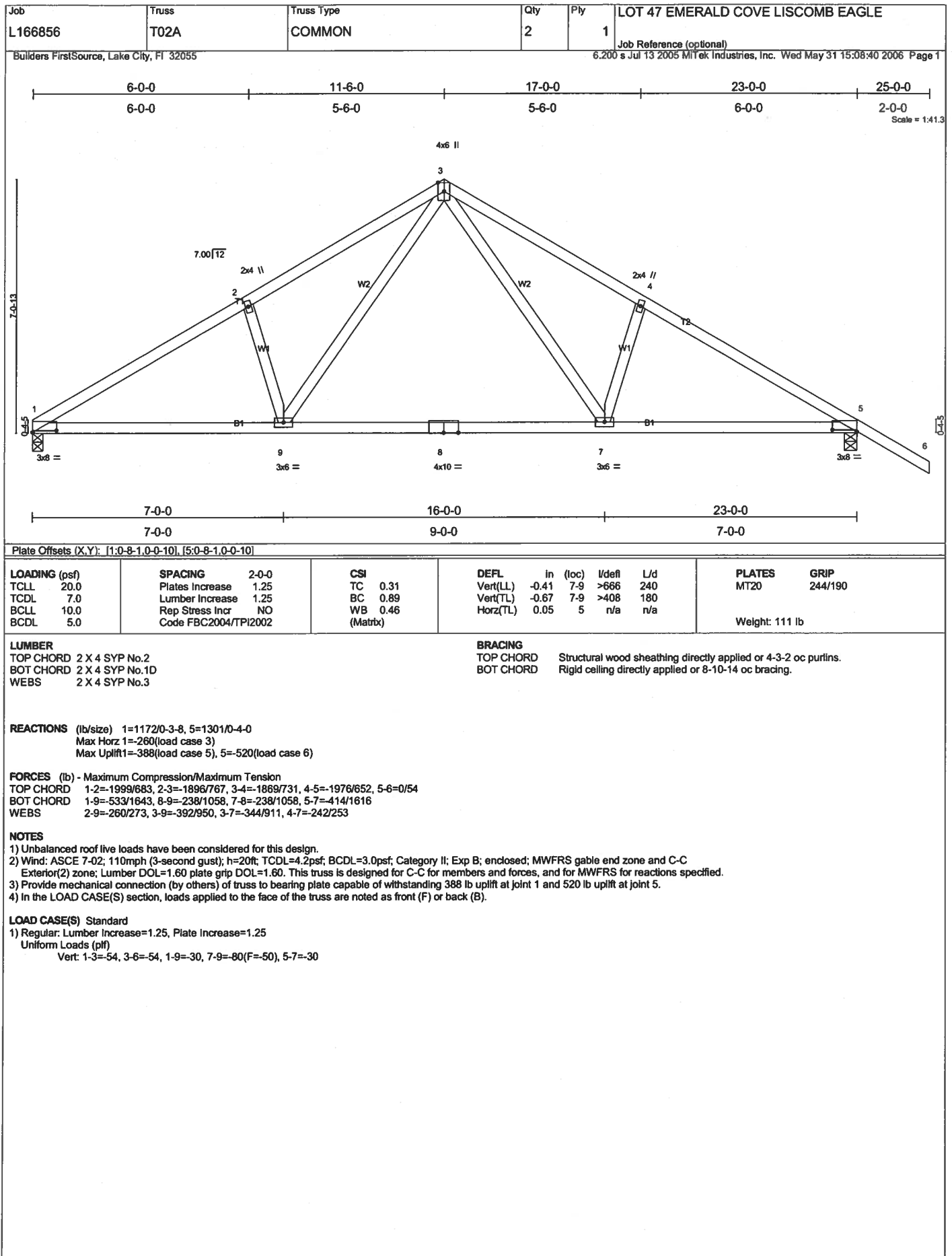
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-77/43, 2-3=-227/110, 3-4=-227/110, 4-5=-77/42
 BOT CHORD 2-4=-66/219

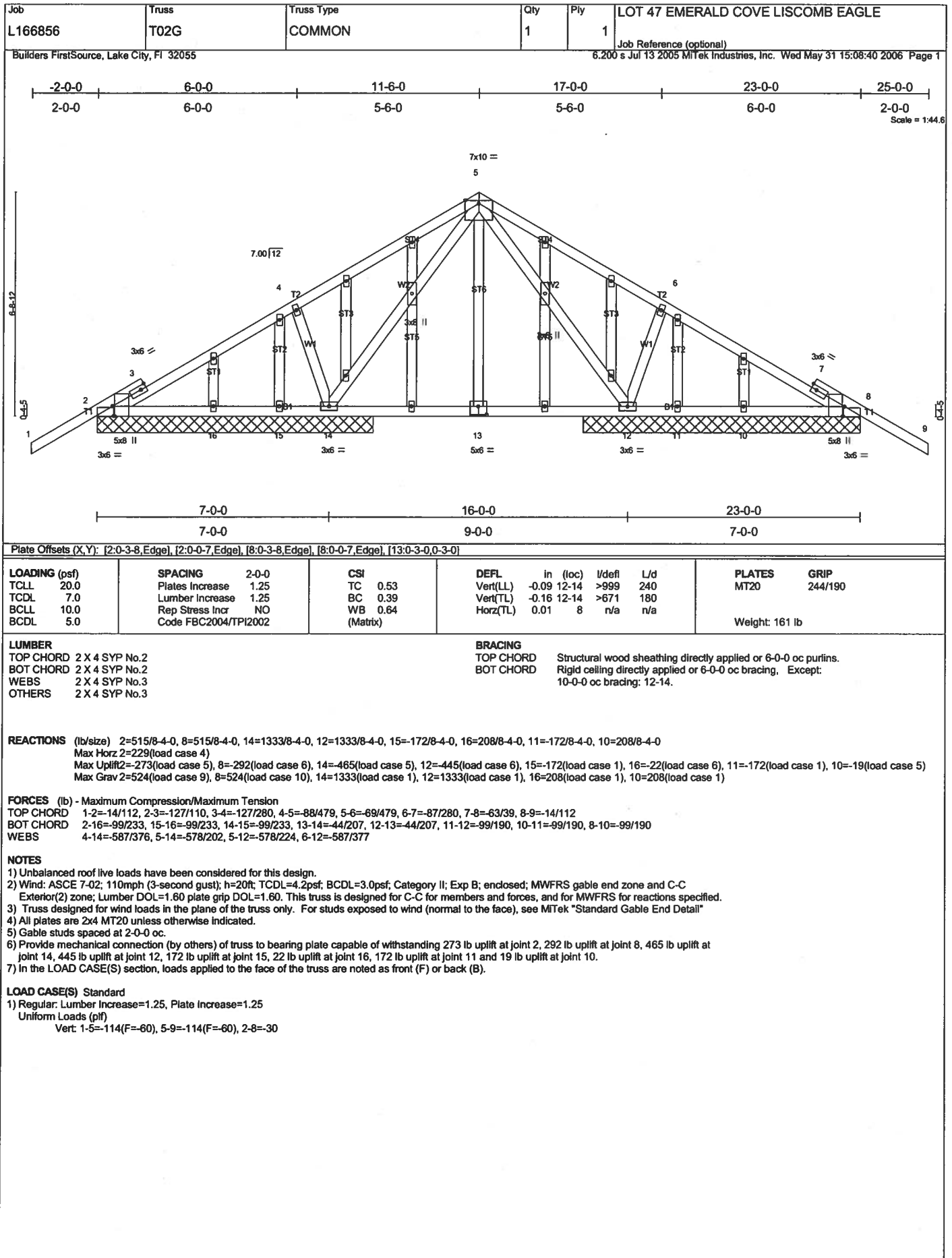
NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1 and 57 lb uplift at joint 5.

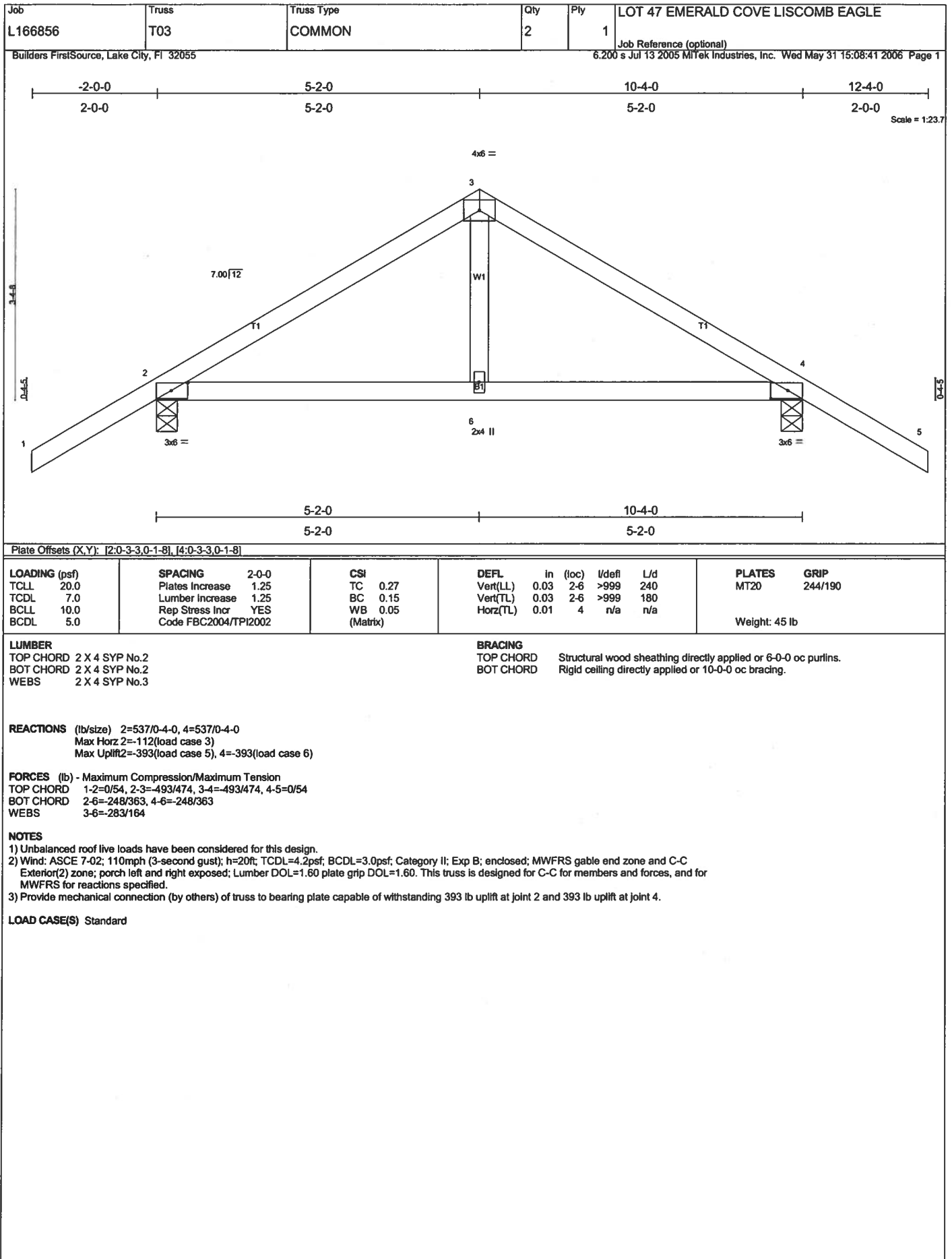
LOAD CASE(S) Standard

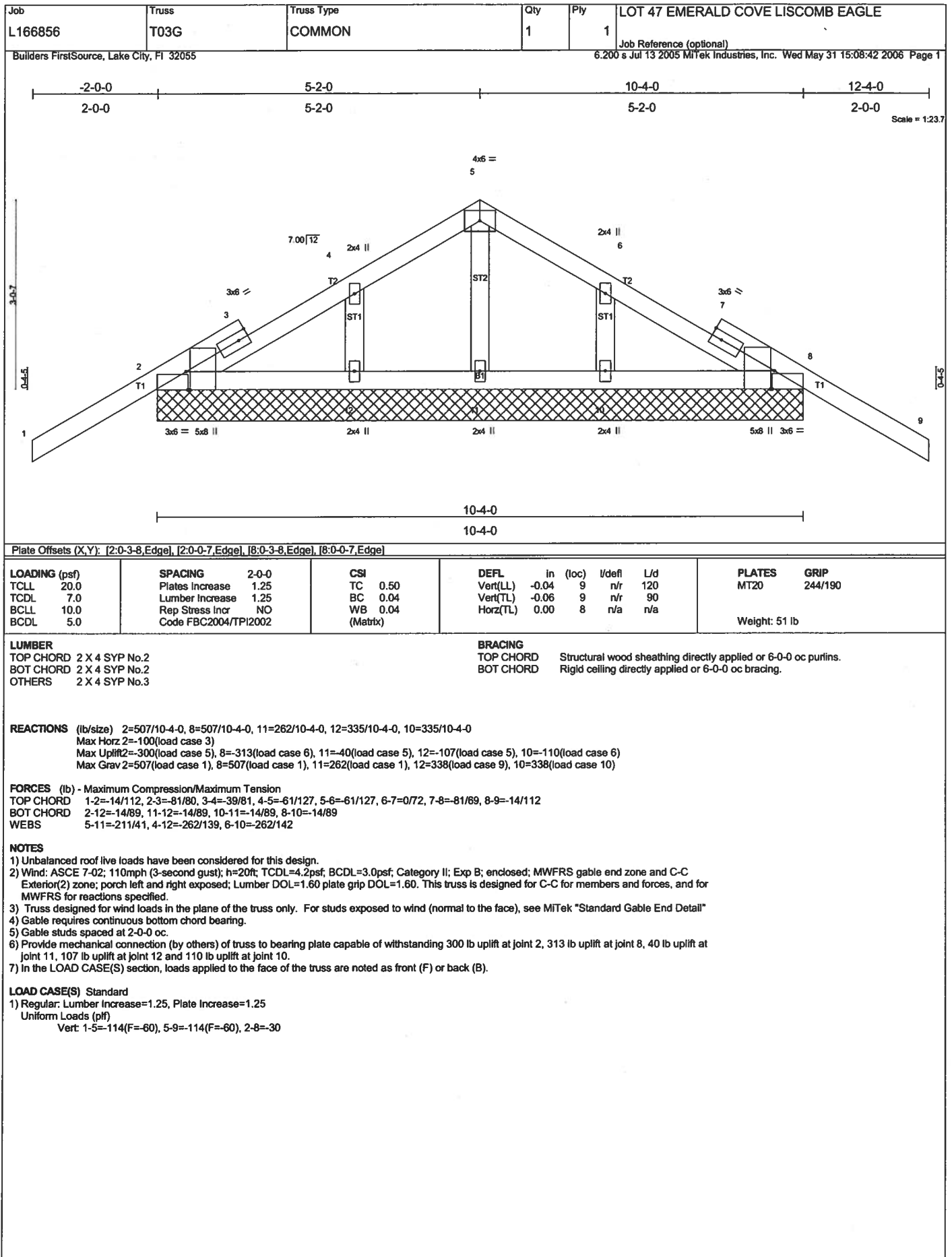


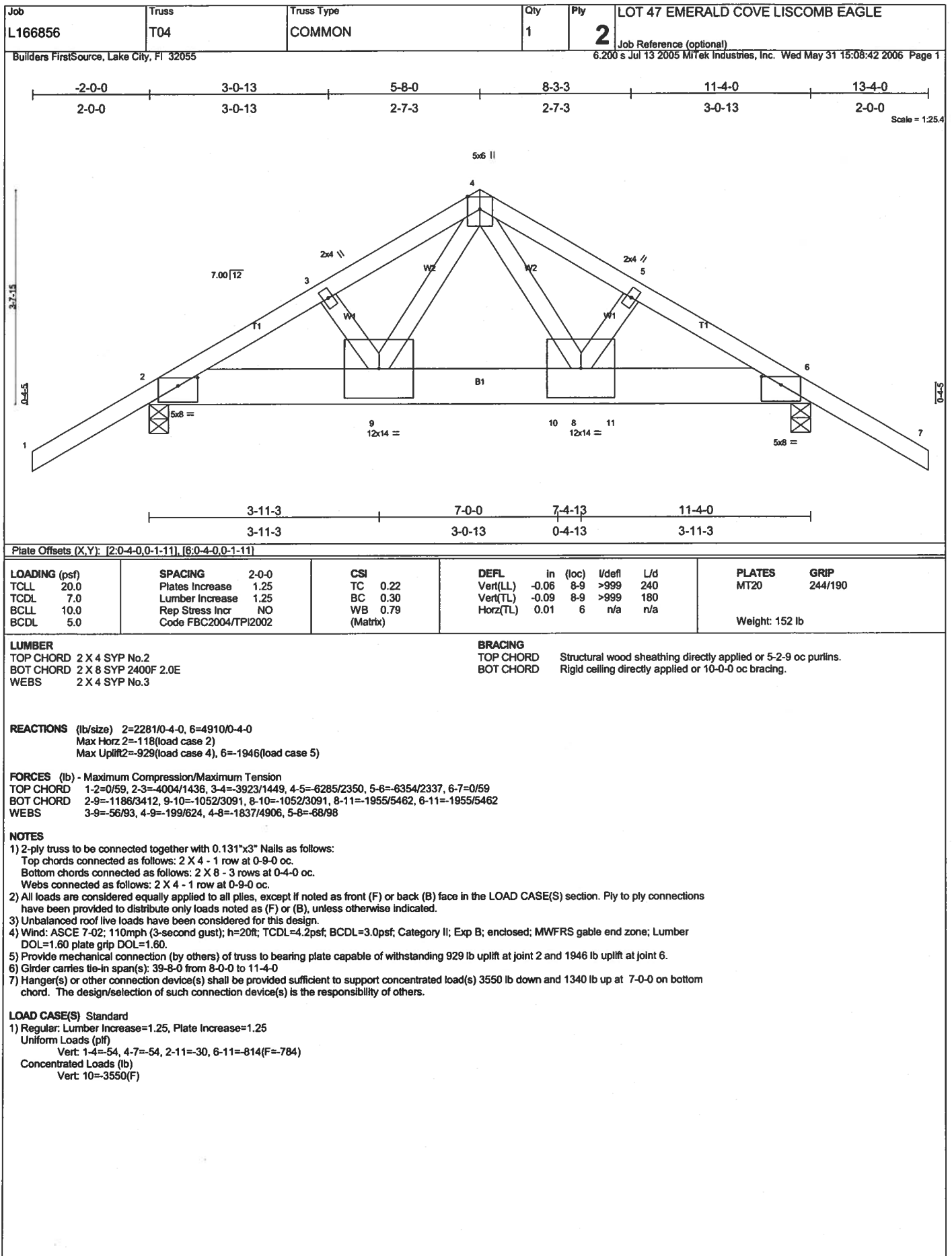


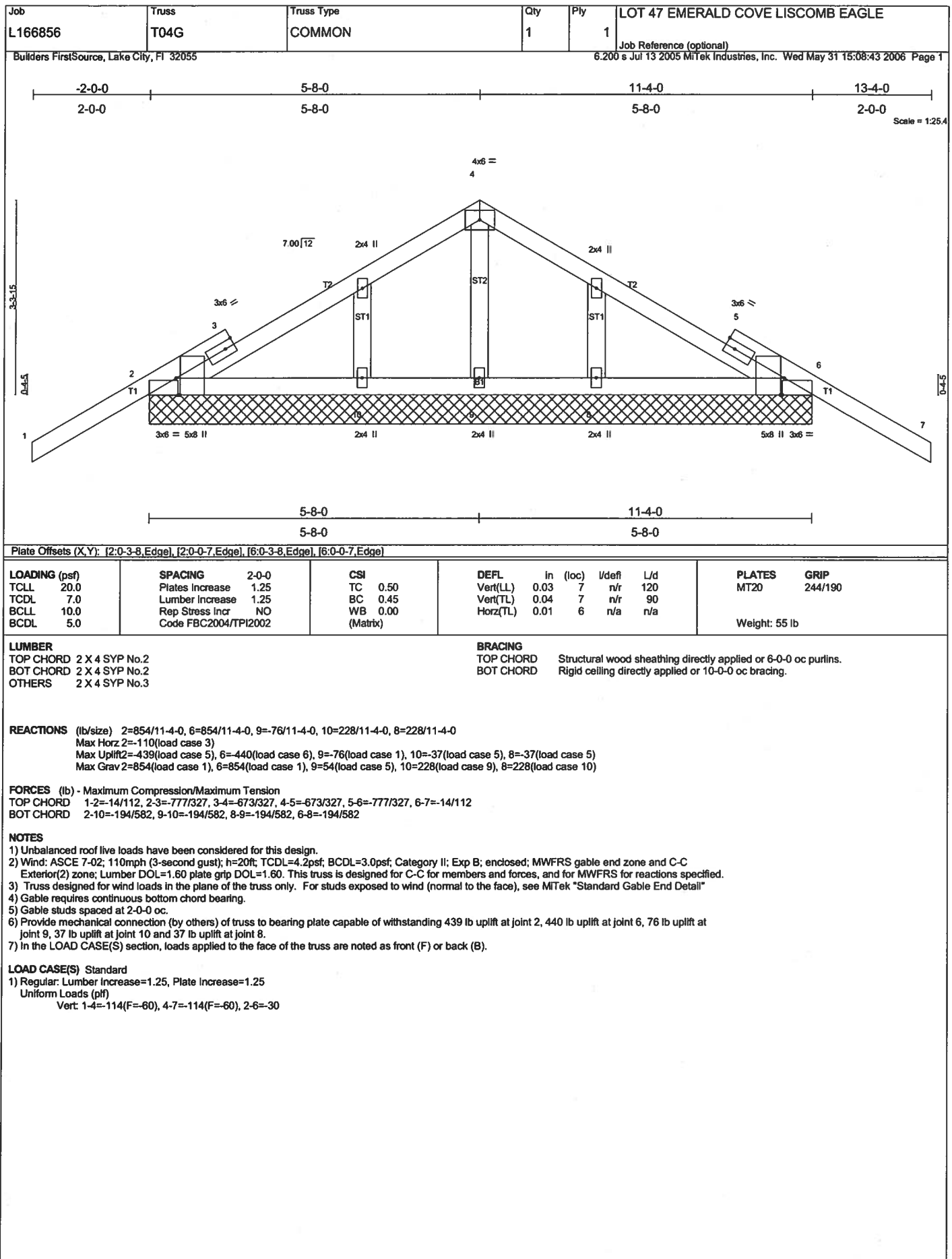


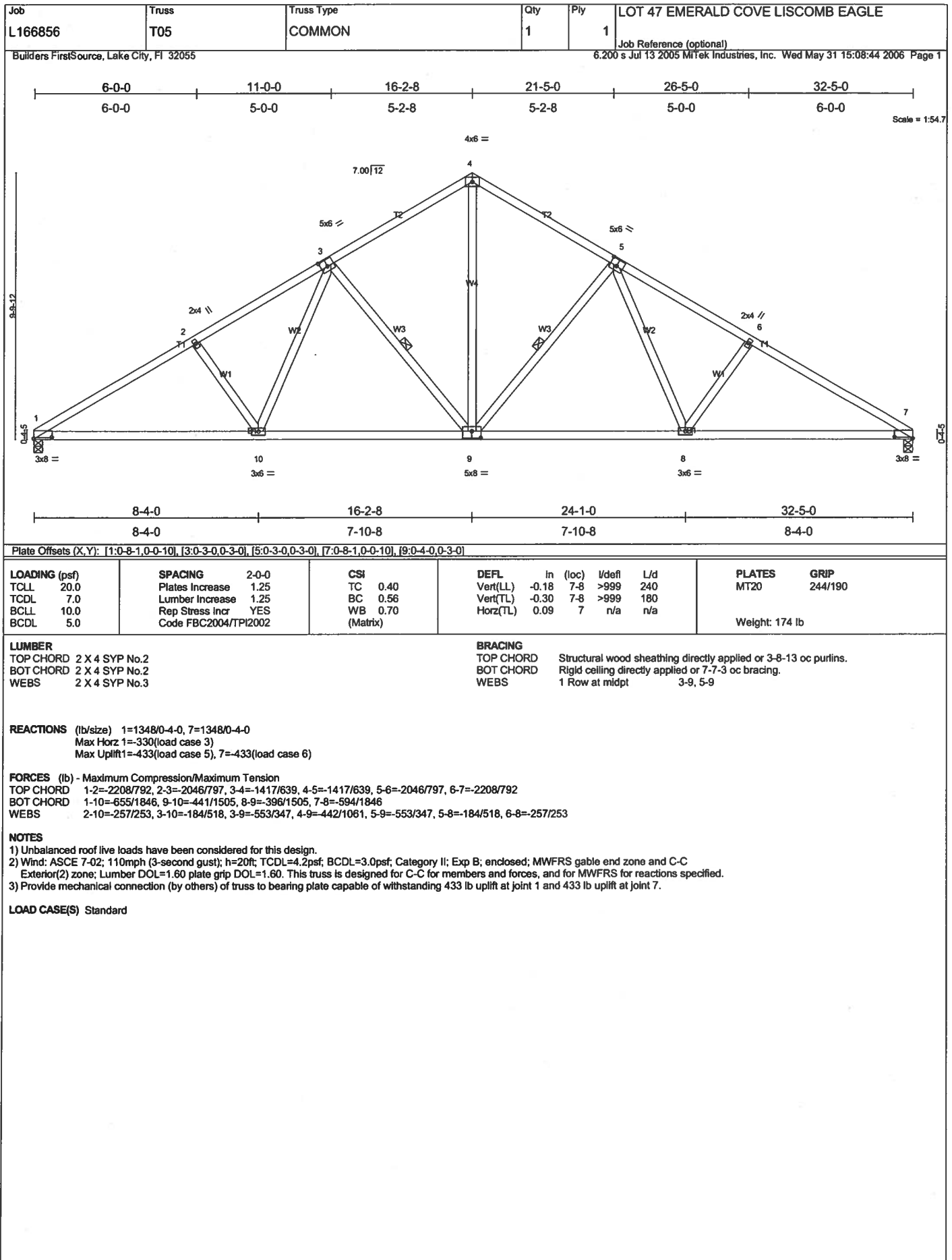












Job L166856	Truss T05A	Truss Type SPECIAL	Qty 2	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Wed May 31 15:08:44 2006 Page 1		

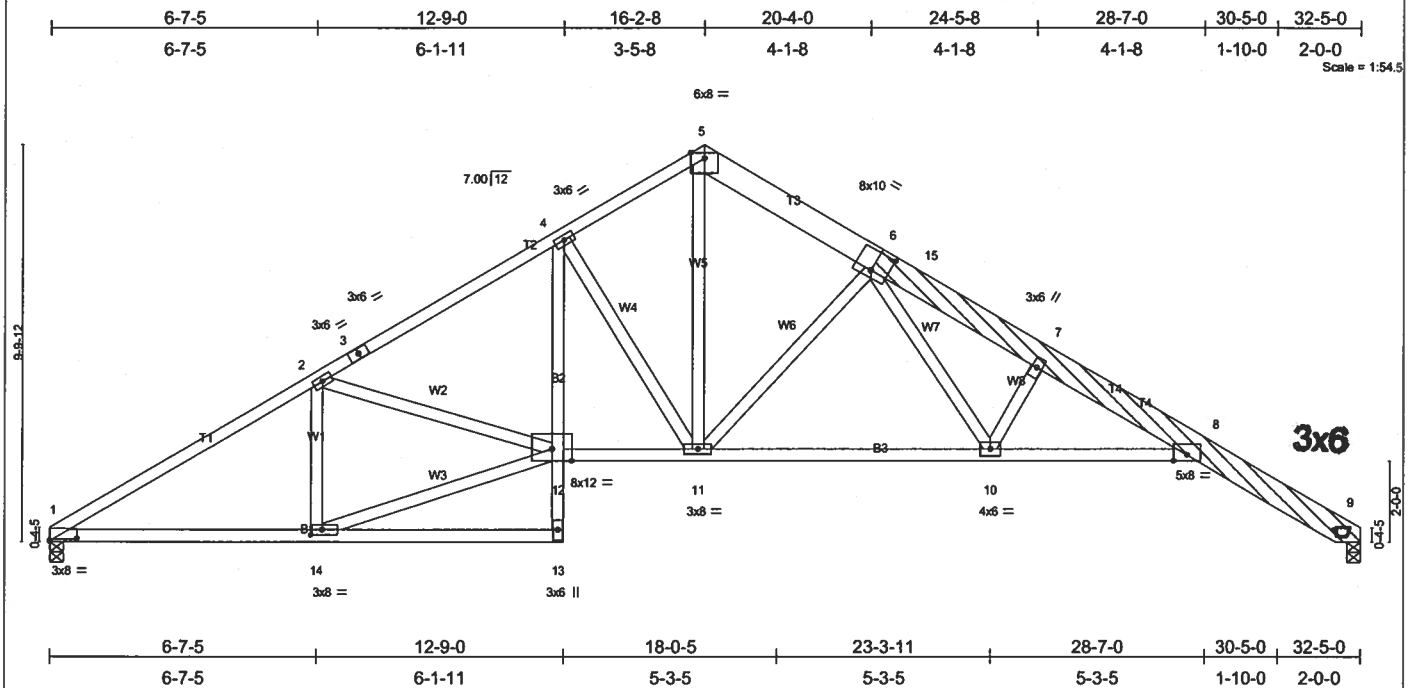


Plate Offsets (X,Y): [1:0-8-1,0-0-10], [6:0-5-0,0-6-0], [14:0-3-8,0-1-8]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES
TCLL 20.0	Plates Increase	1.25	TC 0.53	in (loc) l/defl L/d	GRIP
TCDL 7.0	Lumber Increase	1.25	BC 0.85	Vert(LL) -0.42 8-10 >926 240	MT20 244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.60	Vert(TL) -0.68 8-10 >569 180	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)	Horz(TL) 0.47 9 n/a n/a	
Weight: 245 lb					

LUMBER	BRACING
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*	TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.
BOT CHORD 2 X 4 SYP No.2 *Except*	
B2 2 X 4 SYP No.3	
WEBS 2 X 4 SYP No.3	
LBR SCAB 6-9 2 X 8 SYP 2400F 2.0E one side	

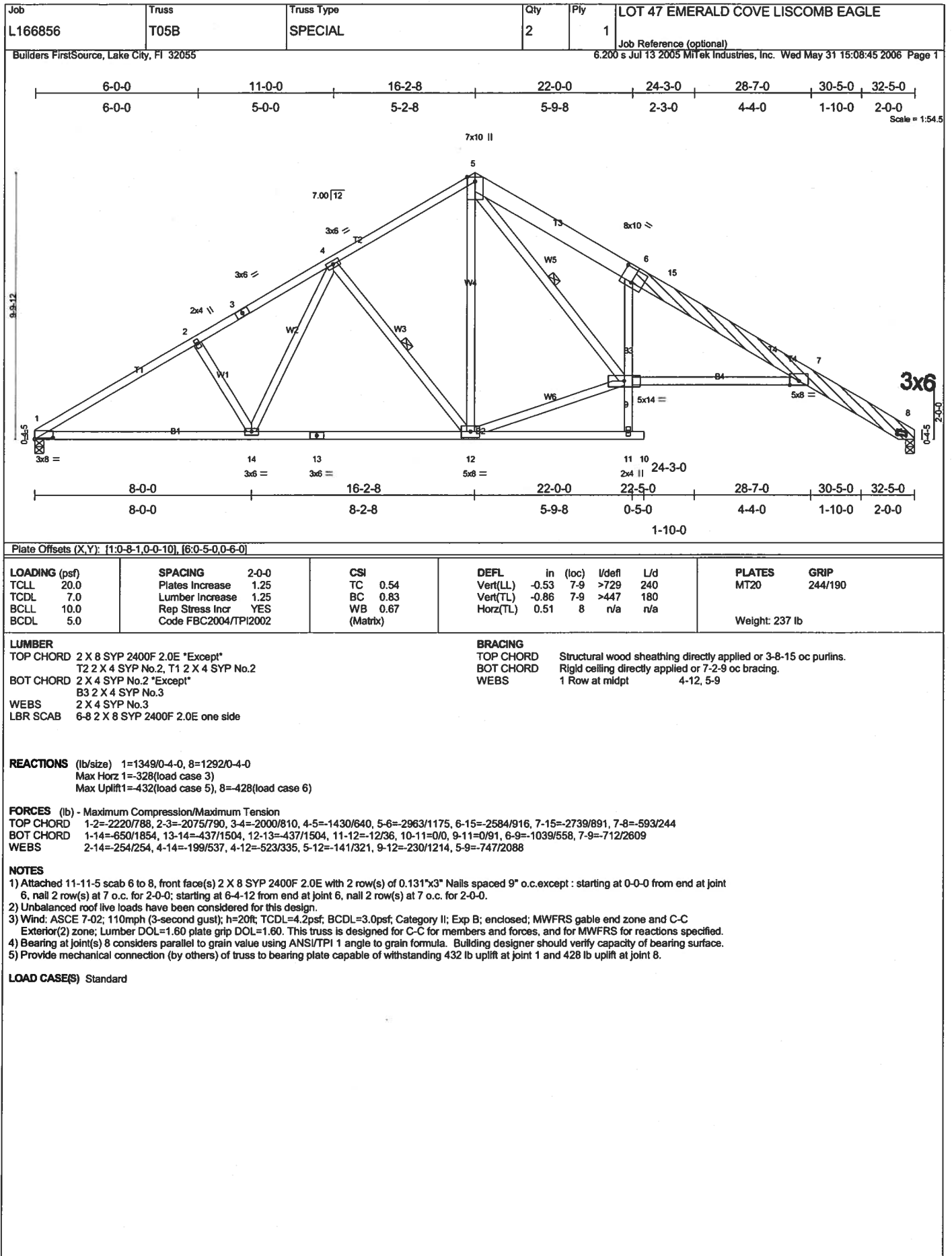
REACTIONS (lb/size) 1=1343/0-4-0, 9=1280/0-4-0
 Max Horz 1=-328(load case 3)
 Max Uplift 1=-433(load case 5), 9=-430(load case 6)

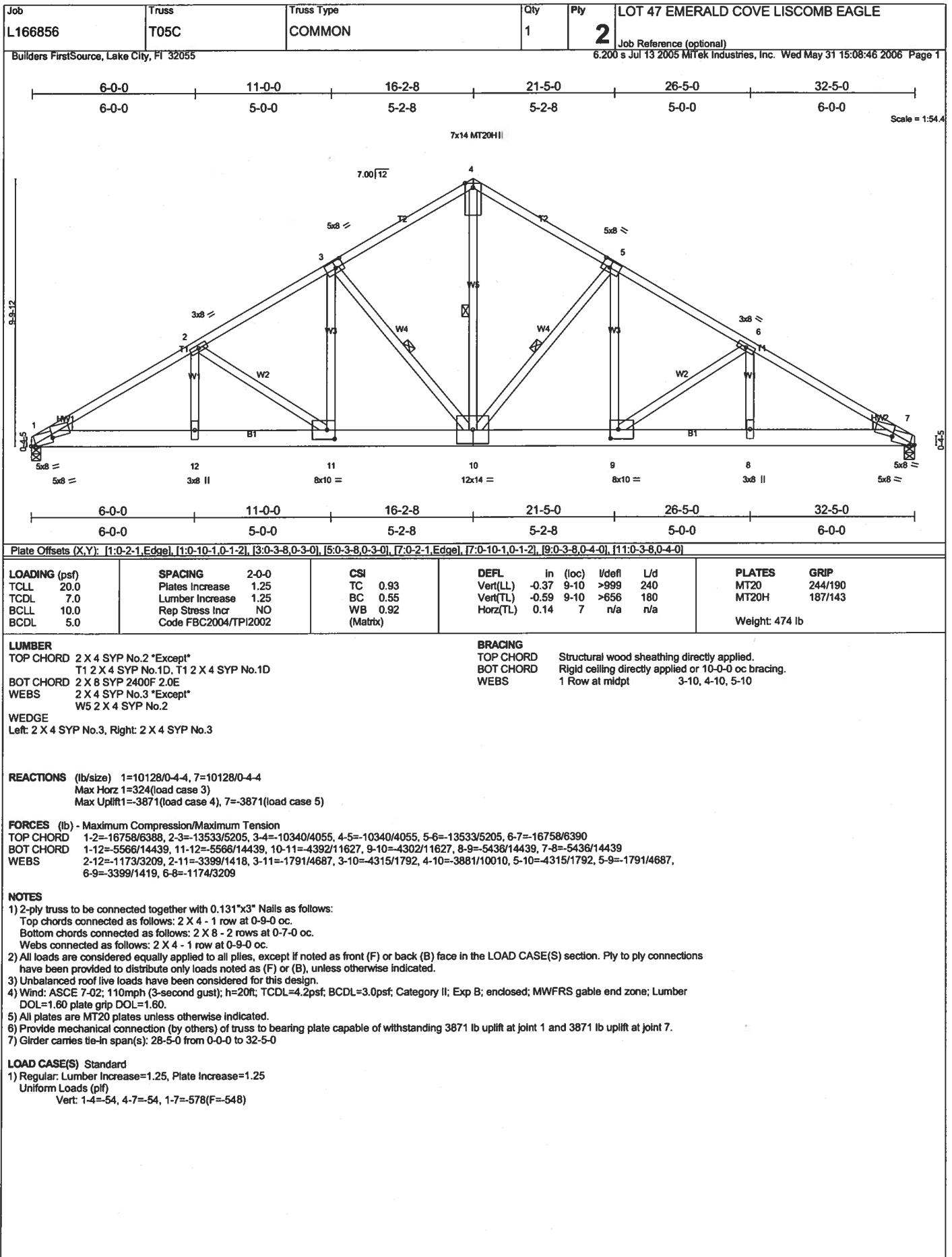
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2247/783, 2-3=-2300/812, 3-4=-2209/835, 4-5=-1752/722, 5-6=-1733/702, 6-15=-3218/1191, 7-15=-3317/1177, 7-8=-3348/1142, 8-9=-588/245
 BOT CHORD 1-14=-637/1856, 13-14=-1/82, 12-13=0/85, 4-12=-231/687, 11-12=-504/1902, 10-11=-514/2026, 8-10=-1002/3328
 WEBS 2-14=-385/236, 12-14=-672/1875, 2-12=-56/132, 4-11=-756/397, 5-11=-561/1444, 6-11=-829/411, 6-10=-488/1475, 7-10=-1176/510

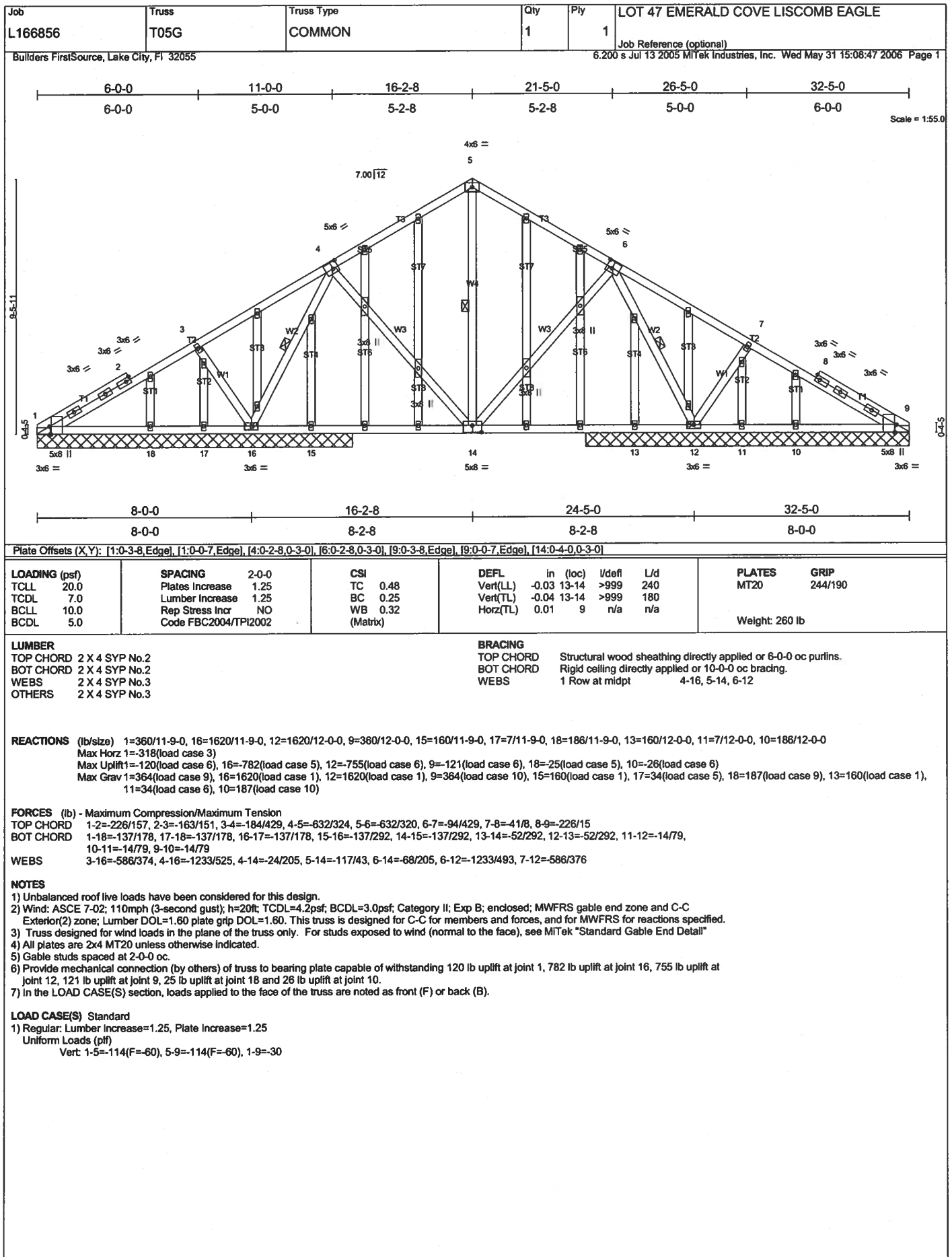
NOTES

- 1) Attached 13-9-13 scab 6 to 9, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c. except : starting at 3-9-4 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 8-3-4 from end at joint 6, nail 2 row(s) at 4 o.c. for 2-0-0.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 430 lb uplift at joint 9.

LOAD CASE(S) Standard







Job L166856	Truss T06	Truss Type SPECIAL	Qty 1	Ply 2	LOT 47 EMERALD COVE LISCOMB EAGLE Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Wed May 31 15:08:48 2006 Page 1
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Builders FirstSource, Lake City, FL 32055

Scale = 1:72.6

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.92	Vert(LL)	0.67	19	>704	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-1.03	17-18	>461		
BCLL 10.0	Rep Stress Incr	NO	WB 0.98	Horz(TL)	0.30	13	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)					Weight: 530 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3 *Except*
 W9 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-11-5 oc bracing.

REACTIONS (lb/size) 13=4067/Mechanical, 24=4345/0-4-0
 Max Horz 24=-239(load case 2)
 Max Uplift 13=-2294(load case 2), 24=-2267(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/58, 2-3=-4368/2366, 3-4=-5188/2851, 4-5=-4601/2567, 5-6=-10768/6091, 6-7=-14782/8685, 7-8=-14670/8626, 8-9=-10647/6292, 9-10=-6827/4112, 10-11=-6839/4123, 11-12=-3847/2226, 2-24=-4231/2230, 12-13=-3955/2300
 BOT CHORD 23-24=-3876, 21-22=-1871/3587, 20-21=-4393/8029, 19-20=-7409/13068, 17-18=-207/377, 16-17=-5309/9222, 15-16=-5309/9222, 14-15=-1768/3188, 13-14=-66/87
 WEBS 22-23=-11/46, 3-22=-1781/995, 3-21=-949/1600, 4-21=-1273/2316, 5-21=-4713/2771, 5-20=-2186/3848, 6-20=-3232/2114, 6-19=-1594/2410, 18-19=-7/181, 7-19=-301/313, 8-17=-3107/2071, 9-17=-1076/1883, 9-15=-0/299, 9-15=-3178/1868, 10-15=-575/600, 11-15=-2781/4720, 11-14=-1601/1113, 2-22=-2114/4022, 12-14=-2044/3676, 8-19=-2673/4626, 17-19=-6131/10619, 22-24=-40/250

NOTES
 1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 3) Unbalanced roof live loads have been considered for this design.
 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
 5) Provide adequate drainage to prevent water ponding.
 6) Refer to girder(s) for truss to truss connections.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2294 lb uplift at joint 13 and 2267 lb uplift at joint 24.
 8) Girder carries tie-in span(s): 4-7-9 from 2-4-0 to 16-4-0; 4-2-2 from 2-4-0 to 16-4-0
 9) Girder carries hip end with 0-0-0 right side setback, 16-4-0 left side setback, and 7-0-0 end setback.
 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1258 lb down and 873 lb up at 16-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert 1-2=-54, 2-3=-117(F=-63), 3-4=-104(F=-50), 4-7=-104(F=-50), 7-11=-117(F=-63), 11-12=-117(F=-63), 23-24=-65(F=-35), 21-22=-71(F=-41), 19-21=-71(F=-41), 13-18=-65(F=-35)
 Concentrated Loads (lb)
 Vert 19=-1258(F)

Job L166856	Truss T07	Truss Type SPECIAL	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Wed May 31 15:08:49 2006 Page 1		

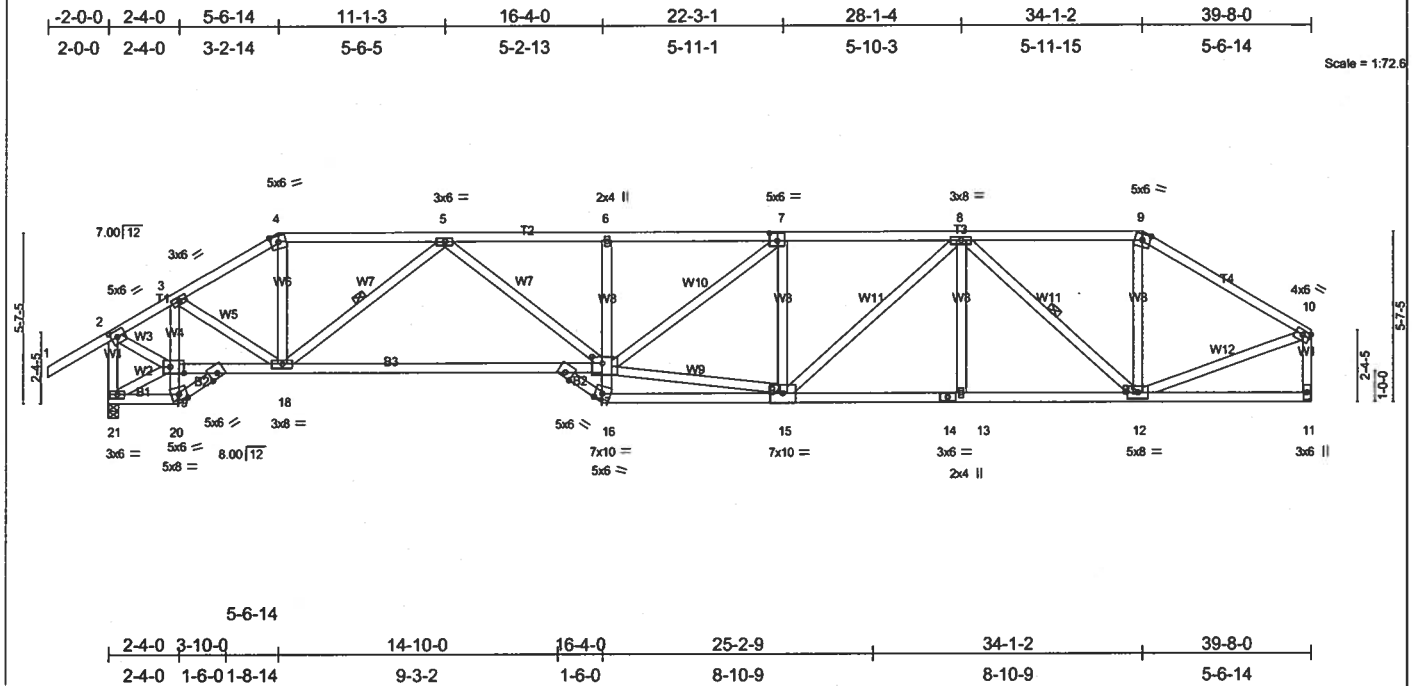


Plate Offsets (X,Y): [2:0-2-9,0-2-8], [7:0-3-0,0-3-0], [10:Edge,0-1-12], [17:0-4-4,0-2-8], [19:0-5-8,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.46	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.83	Vert(LL) -0.56 17-18 >847 240		
BCCL 10.0	Rep Stress Incr YES	WB 0.96	Vert(TL) -0.94 17-18 >503 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.18 11 n/a n/a		
				Weight: 256 lb	

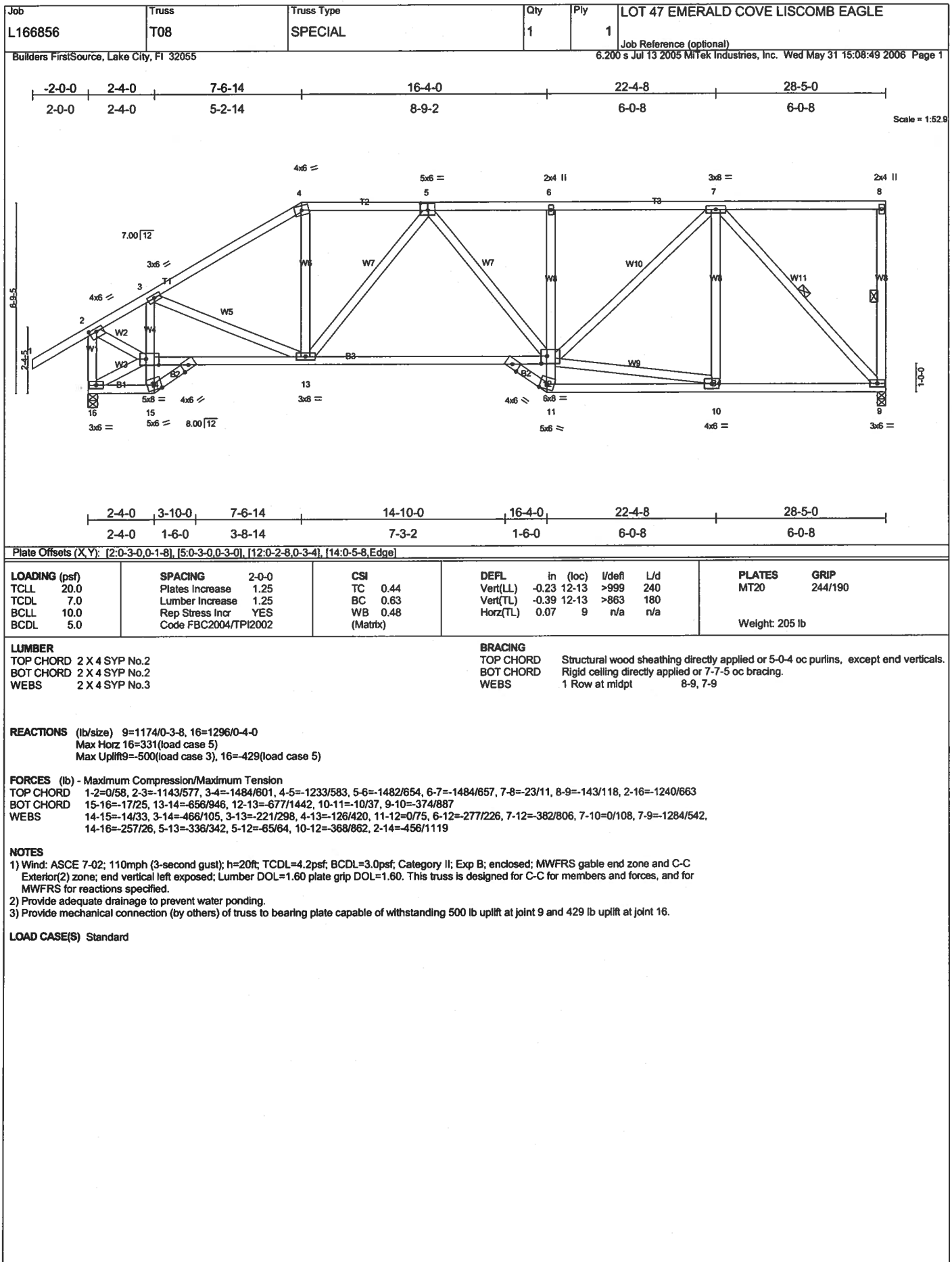
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-1-8 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2 "Except"	BOT CHORD Rigid ceiling directly applied or 5-5-10 oc bracing.
B3 2 X 4 SYP No.1D	WEBS 1 Row at midpt 5-18, 8-12
WEBS 2 X 4 SYP No.3	

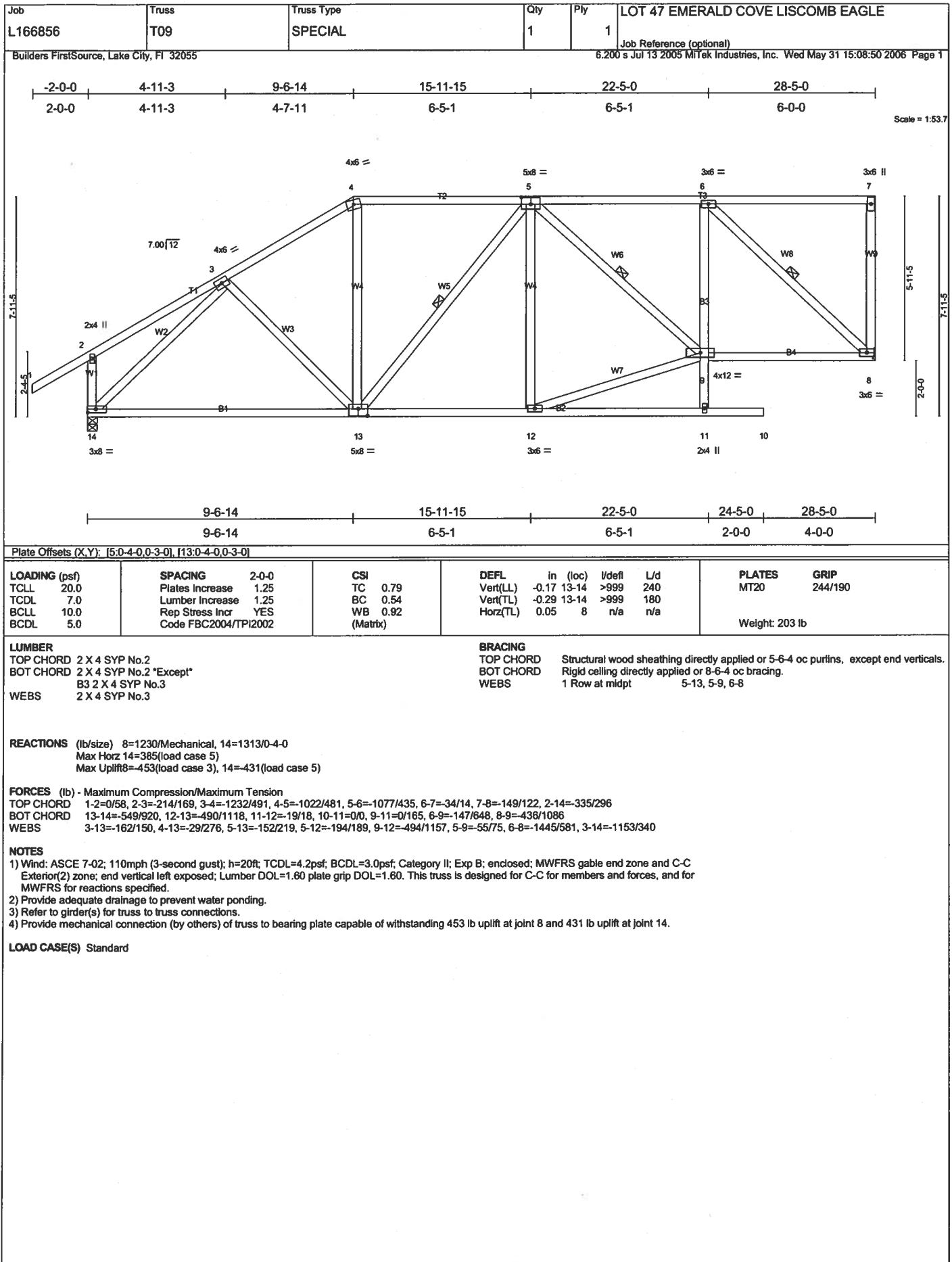
REACTIONS (lb/size) 11=1649/Mechanical, 21=1766/0-4-0
 Max Horz 21=200(load case 4)
 Max Uplift 11=634(load case 3), 21=665(load case 4)

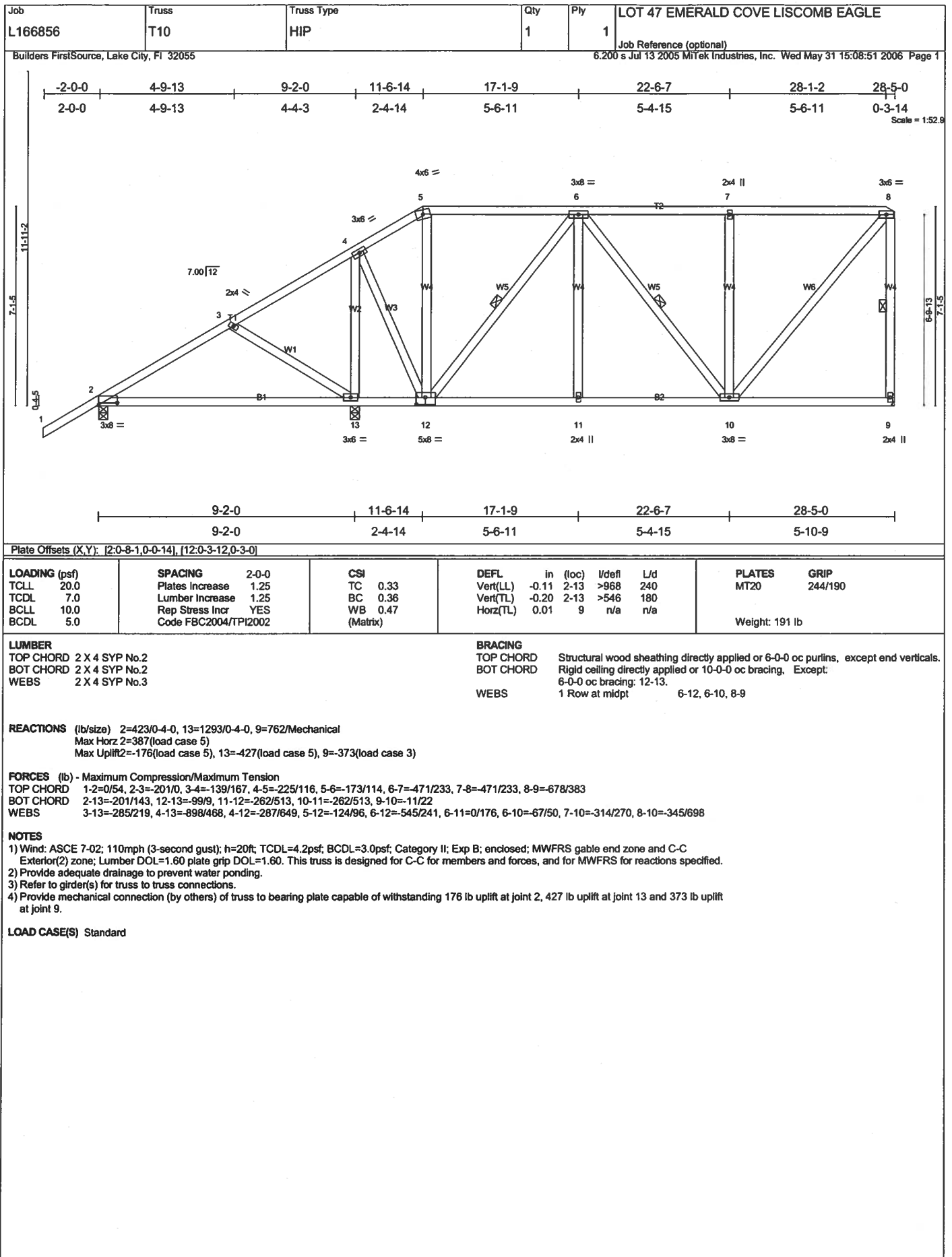
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/58, 2-3=-1615/751, 3-4=-2192/948, 4-5=-1904/866, 5-6=-3646/1686, 6-7=-3634/1684, 7-8=-3044/1376, 8-9=-1525/700,
 9-10=-1818/754, 2-21=-1710/797, 10-11=-1563/645
 BOT CHORD 20-21=-15/32, 18-19=-719/1314, 17-18=-1449/2968, 15-16=-43/106, 14-15=-1119/2552, 13-14=-1119/2552, 12-13=-1119/2552,
 11-12=-54/31
 WEBS 19-20=-9/28, 3-19=-739/253, 3-18=-351/688, 4-18=-291/828, 5-18=-1391/820, 5-17=-398/898, 16-17=0/85, 6-17=-295/252, 7-17=-397/739,
 7-15=-750/498, 8-15=-346/683, 8-12=-1427/729, 9-12=-225/616, 2-19=-633/1526, 10-12=-645/1518, 8-13=0/170, 15-17=-1351/2980,
 19-21=-163/129

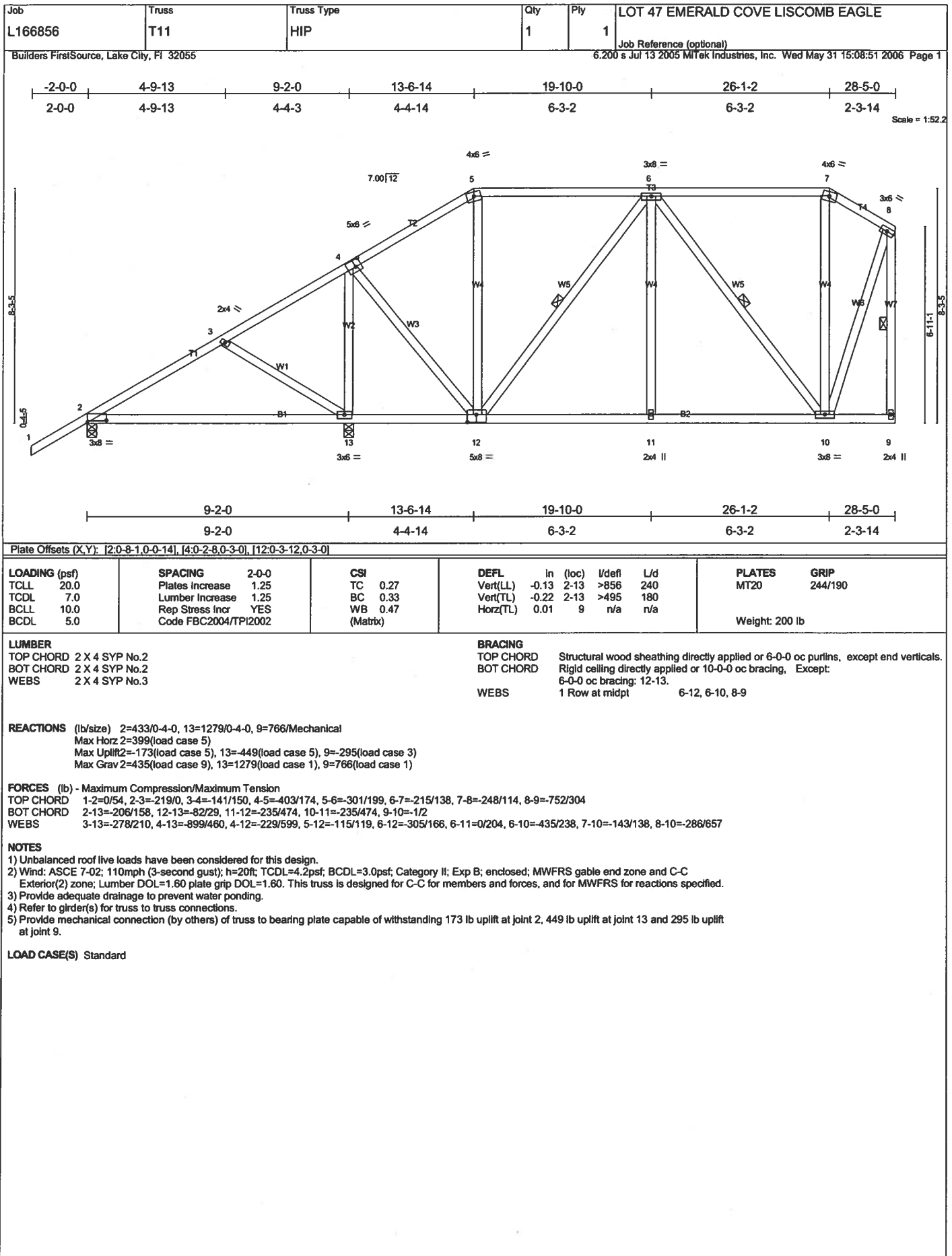
- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 634 lb uplift at joint 11 and 665 lb uplift at joint 21.

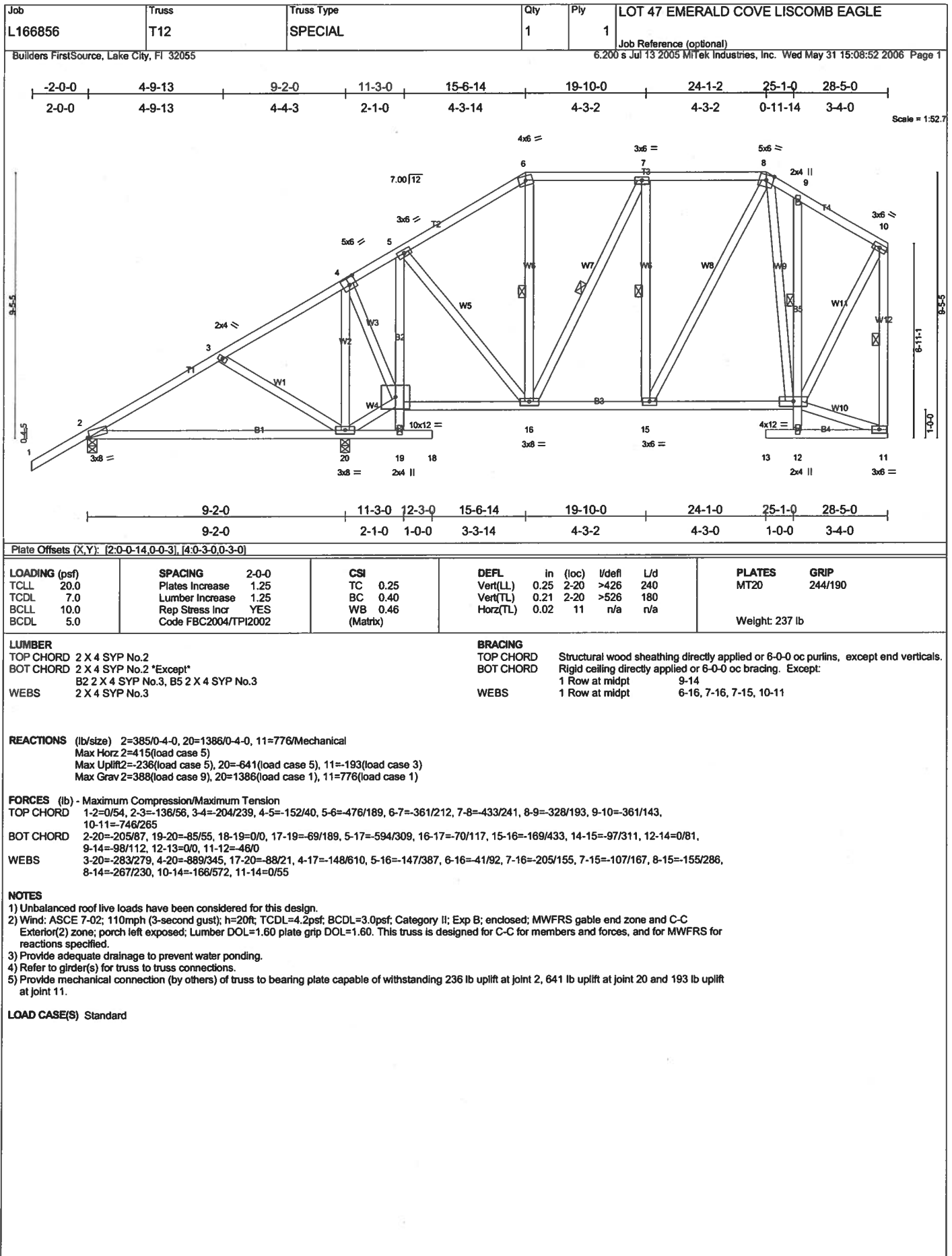
LOAD CASE(S) Standard

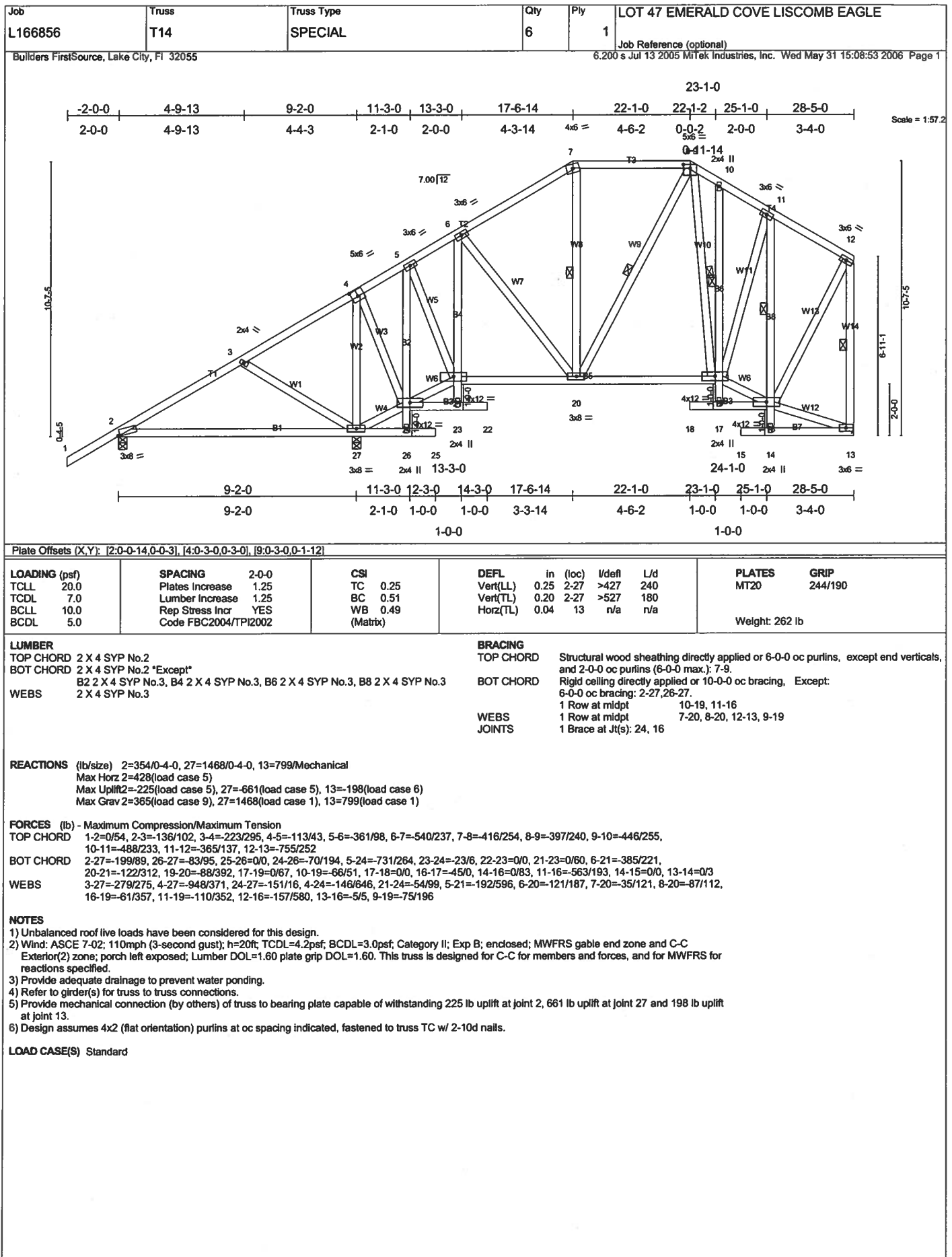












Job L166856	Truss T15	Truss Type SPECIAL	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6,200 s Jul 13 2005 MITek Industries, Inc. Wed May 31 15:08:53 2006 Page 1		

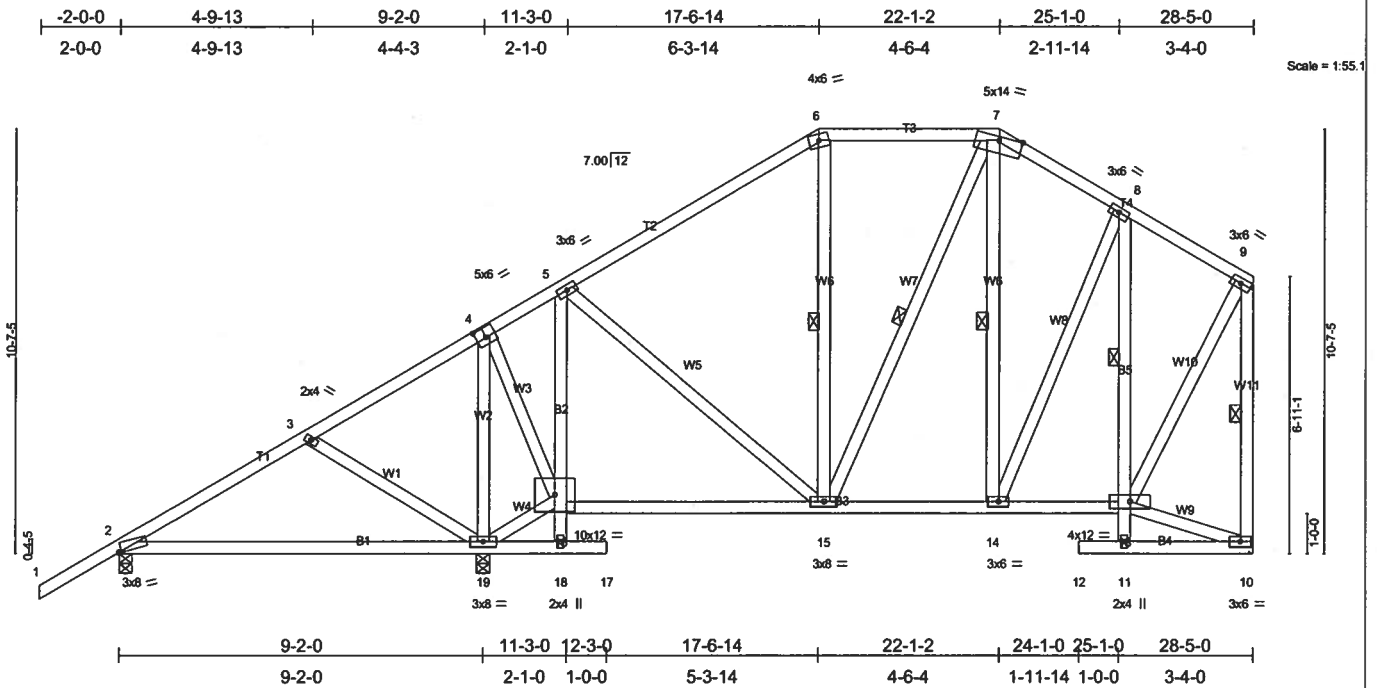


Plate Offsets (X,Y): [2-0-0-14-0-3], [4-0-3-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.32	Vert(LL)	0.25	2-19	>425	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(TL)	0.21	2-19	>526	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.47	Horz(TL)	0.02	10	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 232 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
 WEBS 1 Row at midpt 8-13
 1 Row at midpt 6-15, 7-15, 7-14, 9-10

REACTIONS (lb/size) 2=385/0-4-0, 19=1386/0-4-0, 10=776/Mechanical

Max Horz 2=432(load case 5)
 Max Uplift 2=230(load case 5), 19=658(load case 5), 10=207(load case 6)
 Max Grav 2=393(load case 9), 19=1386(load case 1), 10=776(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-146/81, 3-4=-214/237, 4-5=-153/51, 5-6=-526/218, 6-7=-376/256, 7-8=-440/243, 8-9=-356/141, 9-10=-734/261
 BOT CHORD 2-19=-213/95, 18-19=-124/36, 17-18=0/0, 16-18=-73/187, 5-16=-598/343, 15-16=-94/146, 14-15=-85/342, 13-14=-83/277, 11-13=0/83,
 8-13=-364/177, 11-12=0/0, 10-11=-4/1
 WEBS 3-19=-284/280, 4-19=-894/351, 16-19=-69/50, 4-16=-196/660, 5-15=-103/307, 6-15=-124/166, 7-15=-106/132, 7-14=-75/138, 8-14=-130/191,
 9-13=-167/564, 10-13=-3/11

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 658 lb uplift at joint 19 and 207 lb uplift at joint 10.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L166856	Truss T16	Truss Type HIP	Qty 2	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mitek Industries, Inc. Wed May 31 15:08:54 2006 Page 1		

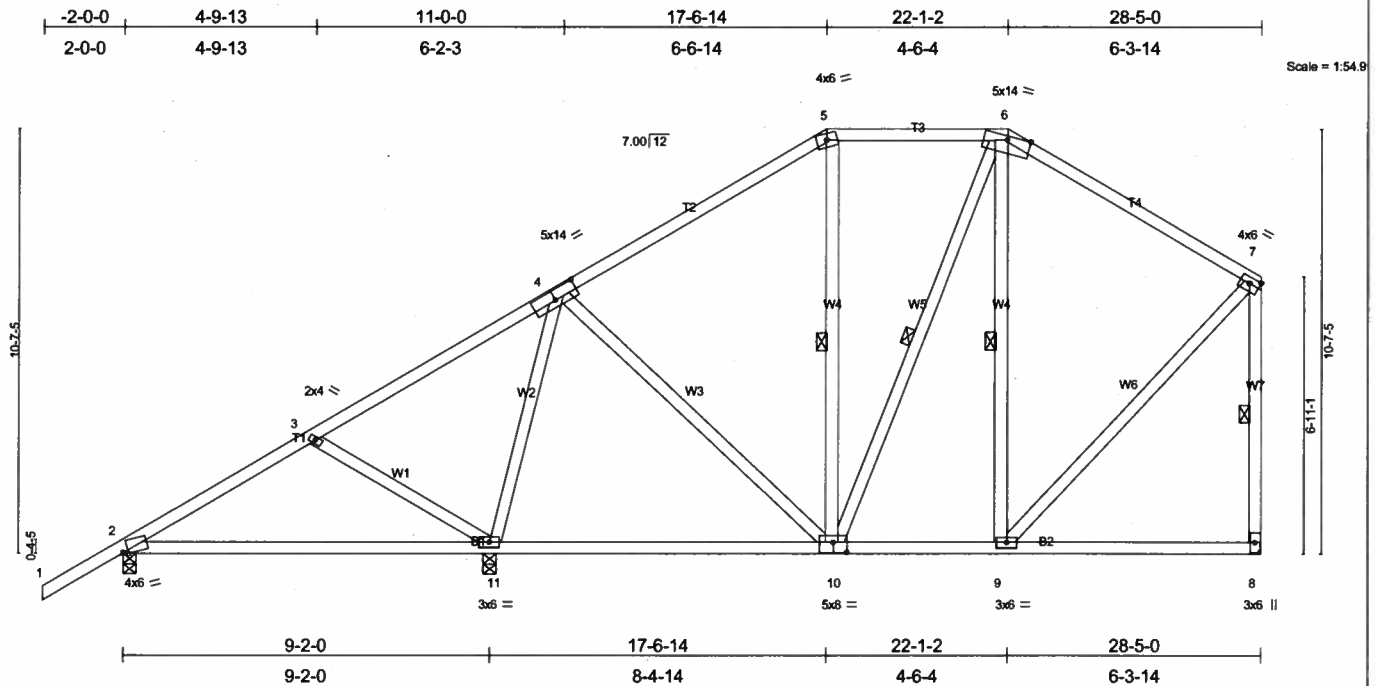


Plate Offsets (X,Y): [2:0-1-10,0-0-11], [4:0-7-0,0-3-0], [7:Edge,0-1-12], [10:0-4-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	Vert(LL) 0.35	2-11	>316	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.34	Vert(TL) 0.30	2-11	>364	180			
BCLL 10.0	Rep Stress Incr YES	WB 0.65	Horz(TL) 0.01	8	n/a	n/a			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)							
									Weight: 193 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10, 6-10, 6-9, 7-8

REACTIONS (lb/size) 2=439/0-4-0, 11=1277/0-4-0, 8=762/Mechanical
Max Horz 2=432(load case 5)
Max Uplift 2=253(load case 5), 11=639(load case 5), 8=216(load case 6)
Max Grav 2=445(load case 9), 11=1277(load case 1), 8=762(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=251/57, 3-4=192/181, 4-5=505/207, 5-6=350/248, 6-7=458/209, 7-8=664/285
BOT CHORD 2-11=250/187, 10-11=106/112, 9-10=92/322, 8-9=17/30
WEBS 3-11=302/308, 4-11=857/444, 4-10=63/322, 5-10=141/187, 6-10=110/144, 6-9=168/101, 7-9=122/429

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2, 639 lb uplift at joint 11 and 216 lb uplift at joint 8.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	LOT 47 EMERALD COVE LISCOMB EAGLE
L166856	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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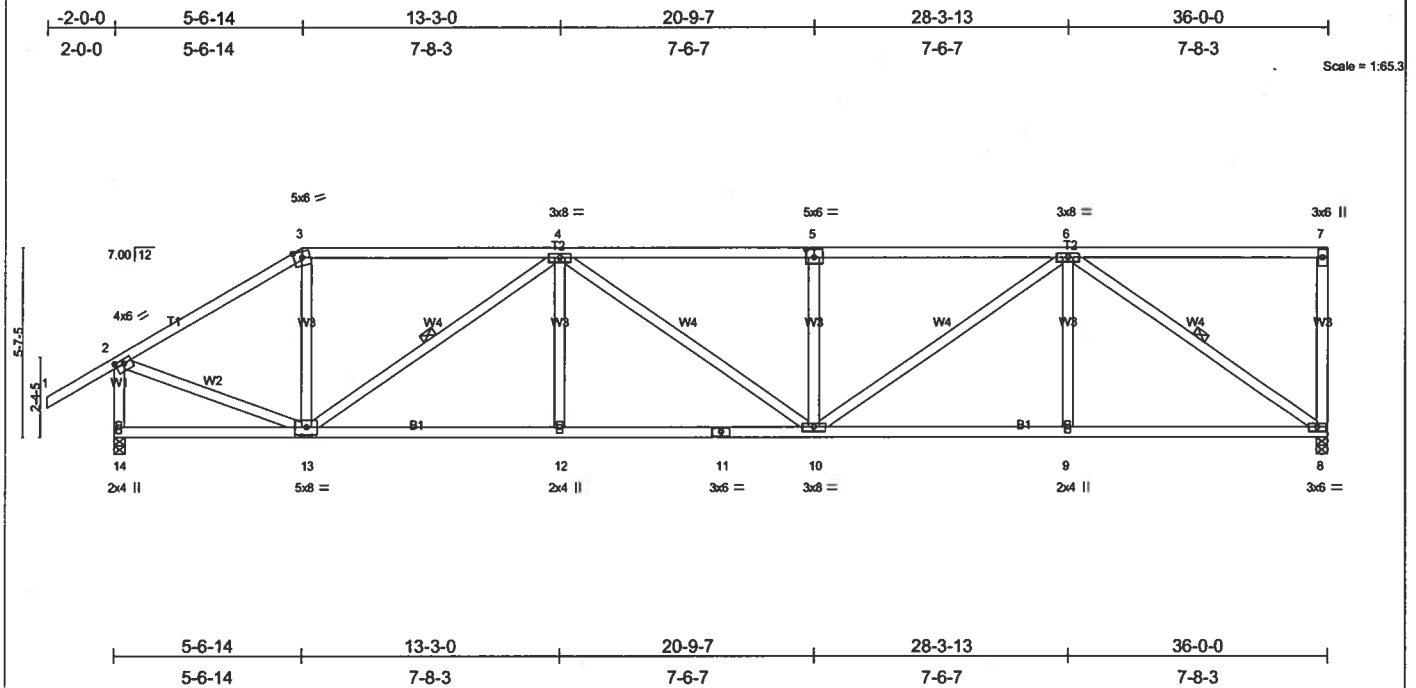


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.65	Vert(LL)	-0.22 10-12	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.59	Vert(TL)	-0.36 10-12	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.84	Horz(TL)	0.10 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)					Weight: 213 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-8-3 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-2-13 oc bracing.
WEBS	1 Row at midpt 4-13, 6-8

REACTIONS

(lb/size) 8=1496/0-4-0, 14=1619/0-4-0
Max Horz 14=277(load case 5)
Max Uplift 8=-685(load case 3), 14=-490(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/58, 2-3=1633/651, 3-4=1365/626, 4-5=2516/1123, 5-6=2516/1123, 6-7=56/26, 7-8=190/157, 2-14=1546/692
 BOT CHORD 13-14=231/67, 12-13=1369/2401, 11-12=1039/2401, 10-11=1039/2401, 9-10=795/1749, 8-9=795/1749
 WEBS 3-13=1343/429, 4-13=1269/667, 4-12=0/230, 4-10=102/140, 5-10=397/332, 6-10=140/1939, 6-9=0/228, 6-8=207/1941, 2-13=597/1422

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; and vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 685 lb uplift at joint 8 and 490 lb uplift at joint 14.

LOAD CASE(S) Standard

Job L166856	Truss T19	Truss Type MONO HIP	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mitek Industries, Inc. Wed May 31 15:08:56 2006 Page 1		

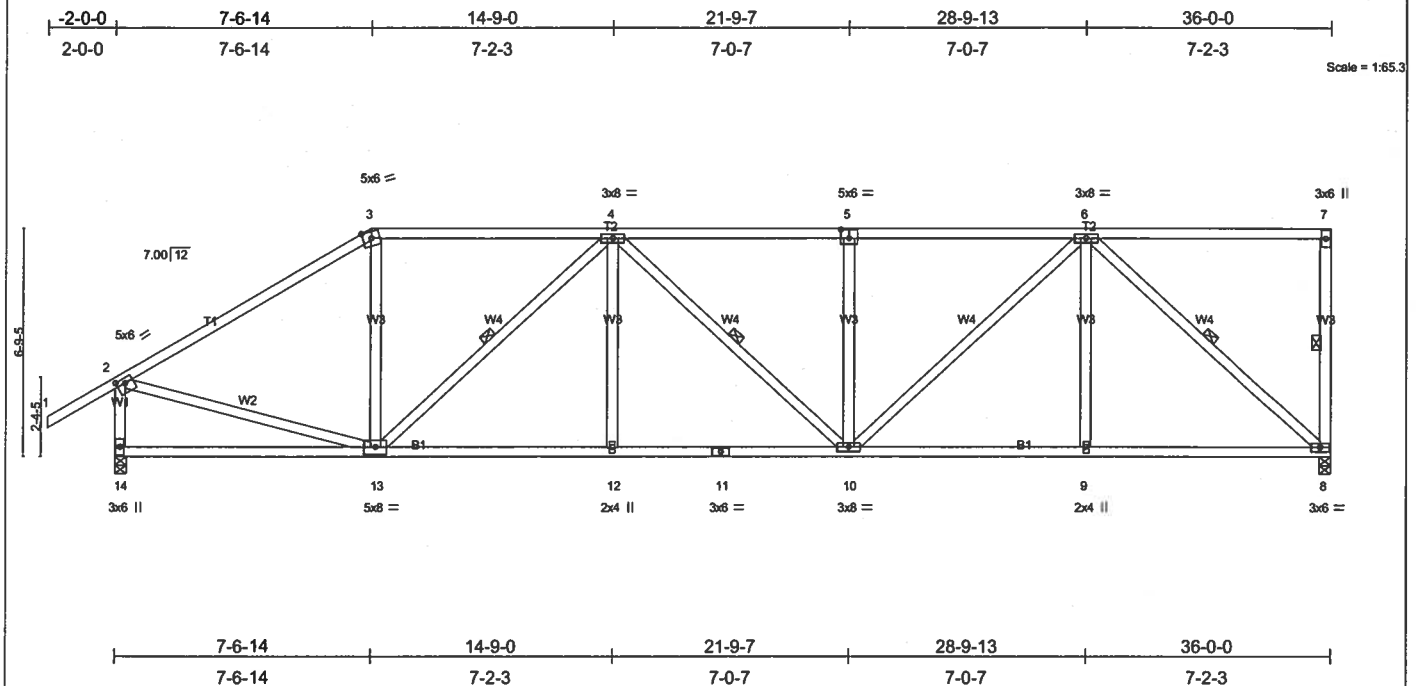


Plate Offsets (X,Y): [2-0-3-0-0-1-12], [5-0-3-0-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.58	Vert(LL) -0.16	10-12	>999	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.52	Vert(TL) -0.27	10-12	>999	180			
BCLL 10.0	Rep Stress Incr YES	WB 0.78	Horz(TL) 0.08	8	n/a	n/a			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)							
								Weight: 227 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing.
 WEBS 1 Row at midpt 7-8, 4-13, 4-10, 6-8

REACTIONS (lb/size) 8=1498/0-4-0, 14=1619/0-4-0
 Max Horz 14=331(load case 5)
 Max Uplift 8=664(load case 3), 14=504(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/58, 2-3=1736/670, 3-4=1421/659, 4-5=2015/854, 5-6=2015/854, 6-7=37/17, 7-8=175/144, 2-14=-1504/682
 BOT CHORD 13-14=-335/124, 12-13=-872/2037, 11-12=-872/2037, 10-11=-872/2037, 9-10=-595/1359, 8-9=-595/1359
 WEBS 3-13=-151/427, 4-13=-838/509, 4-12=0/208, 4-10=-42/56, 5-10=-374/306, 6-10=-383/892, 6-9=0/215, 6-8=-1798/787, 2-13=-555/1337

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 664 lb uplift at joint 8 and 504 lb uplift at joint 14.

LOAD CASE(S) Standard

Job L166856	Truss T20	Truss Type HIP	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mitek Industries, Inc. Wed May 31 15:08:57 2006 Page 1		

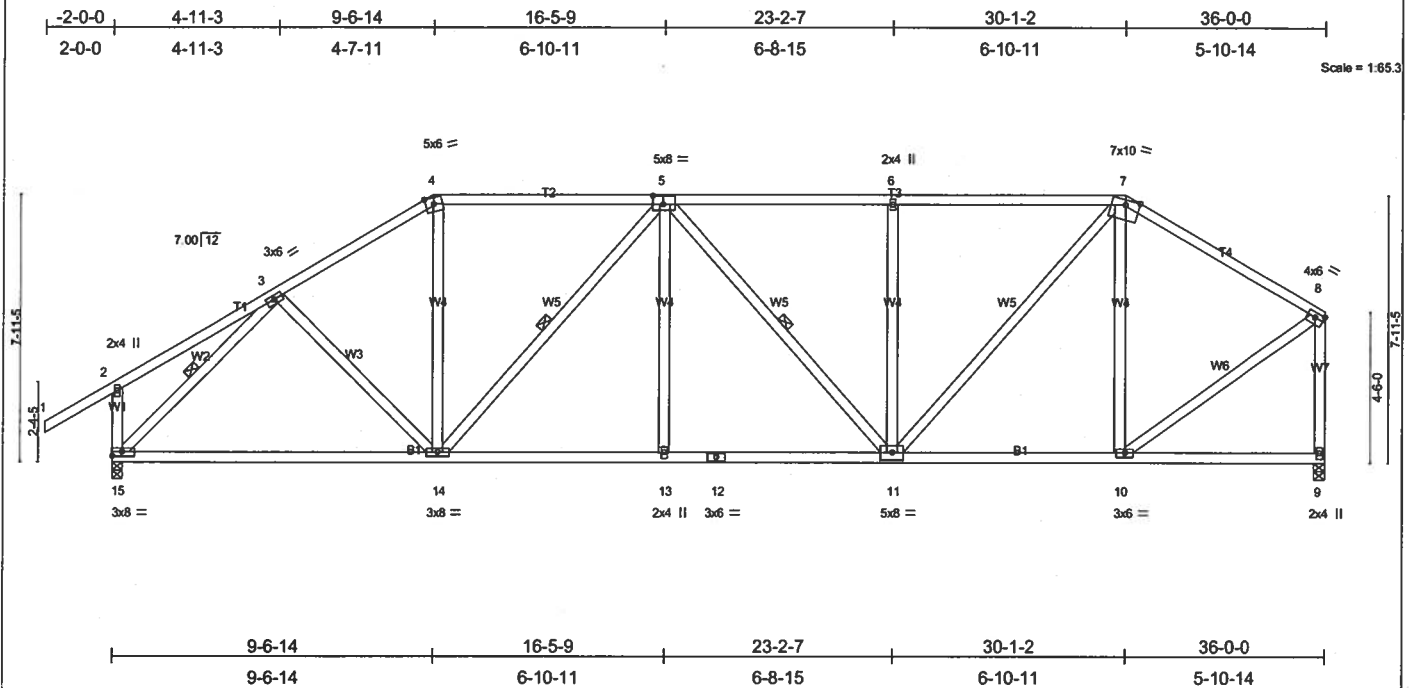


Plate Offsets (X,Y): [5:0-3-12,0-3-0], [8:Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.95	Vert(LL)	-0.18 14-15	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.65	Vert(TL)	-0.30 14-15	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.94	Horz(TL)	0.07 9	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002						Weight: 240 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-7-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-1-5 oc bracing.
 WEBS 1 Row at midpt 5-14, 5-11, 3-15

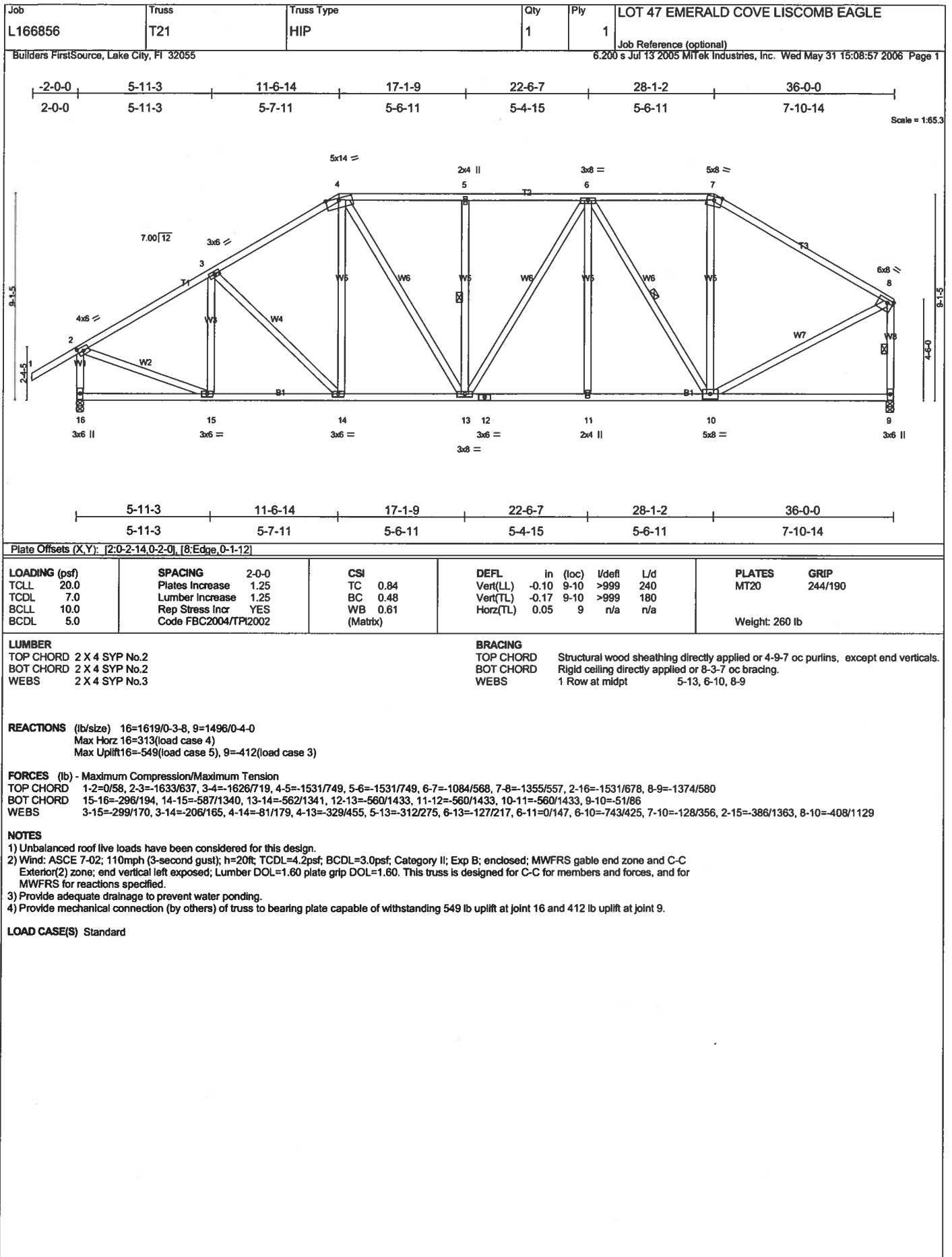
REACTIONS (lb/size) 15=1619/0-3-8, 9=1496/0-4-0
 Max Horz 15=276(load case 5)
 Max Uplift 15=533(load case 5), 9=494(load case 3)

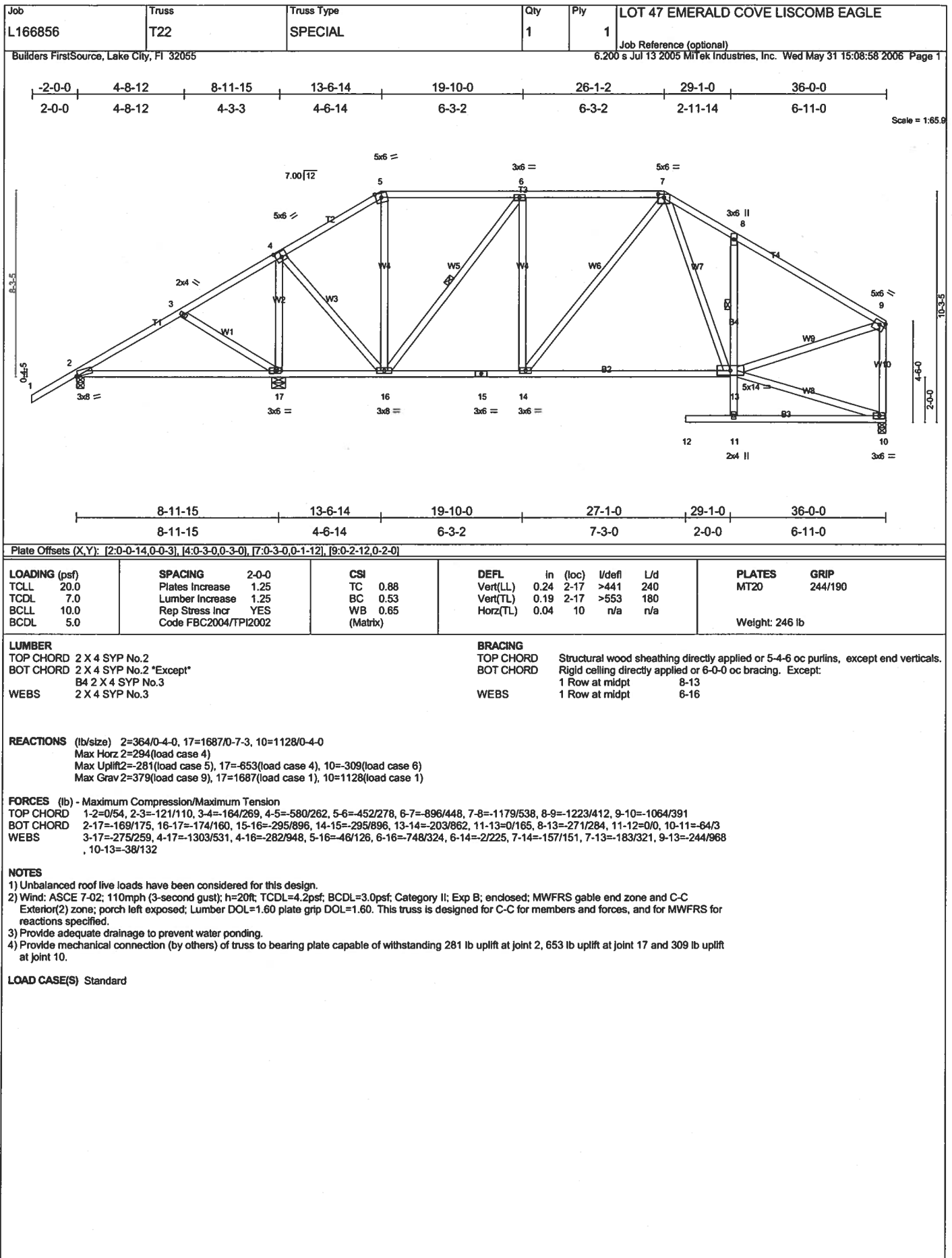
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/58, 2-3=222/173, 3-4=-1672/717, 4-5=-1408/680, 5-6=-1638/760, 6-7=-1638/760, 7-8=-1224/505, 2-15=-342/299, 8-9=-1413/583
 BOT CHORD 14-15=-599/1213, 13-14=-786/1766, 12-13=-786/1767, 11-12=-786/1767, 10-11=-353/987, 9-10=-26/39
 WEBS 3-14=-221/344, 4-14=-134/483, 5-14=-616/400, 5-13=0/184, 5-11=-216/128, 6-11=-383/334, 7-11=-520/1012, 7-10=-499/308, 3-15=-1569/553, 8-10=-464/1181

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 533 lb uplift at joint 15 and 494 lb uplift at joint 9.

LOAD CASE(S) Standard





Job L166856	Truss T23	Truss Type SPECIAL	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Wed May 31 15:08:59 2006 Page 1		

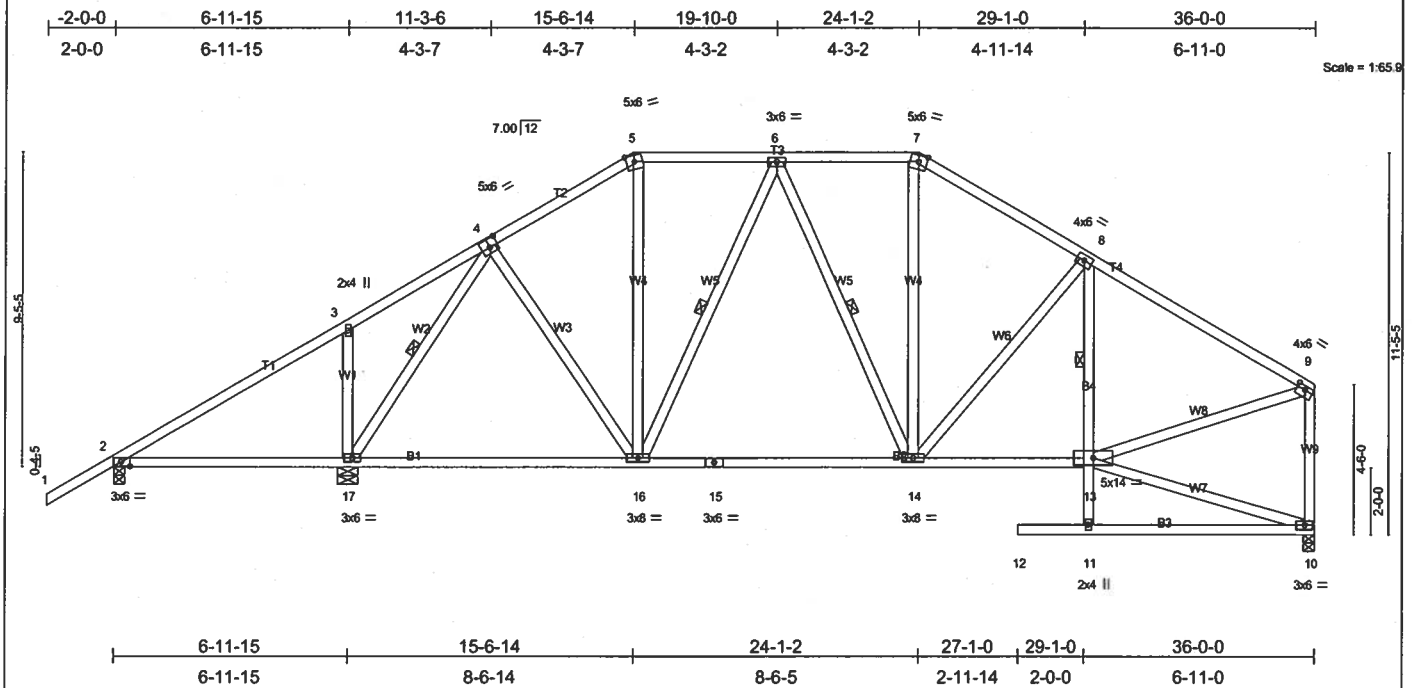


Plate Offsets (X,Y): [2:0-3:3,0-1-8], [4:0-3:0,0-3-0], [9:0-3:0,0-1-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.90	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.44	Vert(LL) 0.14 2-17 >586 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.39	Vert(TL) 0.12 2-17 >672 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.05 10 n/a n/a		
	Code FBC2004/TP12002			Weight: 253 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
B4 2 X 4 SYP No.3	6-0-0 oc bracing: 2-17.
WEBS 2 X 4 SYP No.3	1 Row at midpt 8-13
	1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=293/0-4-0, 17=1658/0-7-3, 10=1229/0-4-0
 Max Horz 2=334(load case 4)
 Max Uplift 2=228(load case 5), 17=639(load case 5), 10=349(load case 6)
 Max Grav 2=312(load case 9), 17=1658(load case 1), 10=1229(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-193/239, 3-4=-47/178, 4-5=-958/445, 5-6=-785/436, 6-7=-943/500, 7-8=-1163/519, 8-9=-1332/465, 9-10=-1142/429
 BOT CHORD 2-17=-119/168, 16-17=-231/557, 15-16=-255/911, 14-15=-255/911, 13-14=-302/1071, 11-13=0/169, 8-13=-103/136, 11-12=0/0, 10-11=0/20
 WEBS 3-17=-348/335, 4-17=-1245/400, 4-16=-135/436, 5-16=-55/238, 6-16=-371/252, 6-14=-60/197, 7-14=-86/298, 8-14=-220/203,
 9-13=-296/1074, 10-13=-45/45

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2, 639 lb uplift at joint 17 and 349 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L166856	Truss T24	Truss Type SPECIAL	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
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Builders FirstSource, Lake City, FL 32055

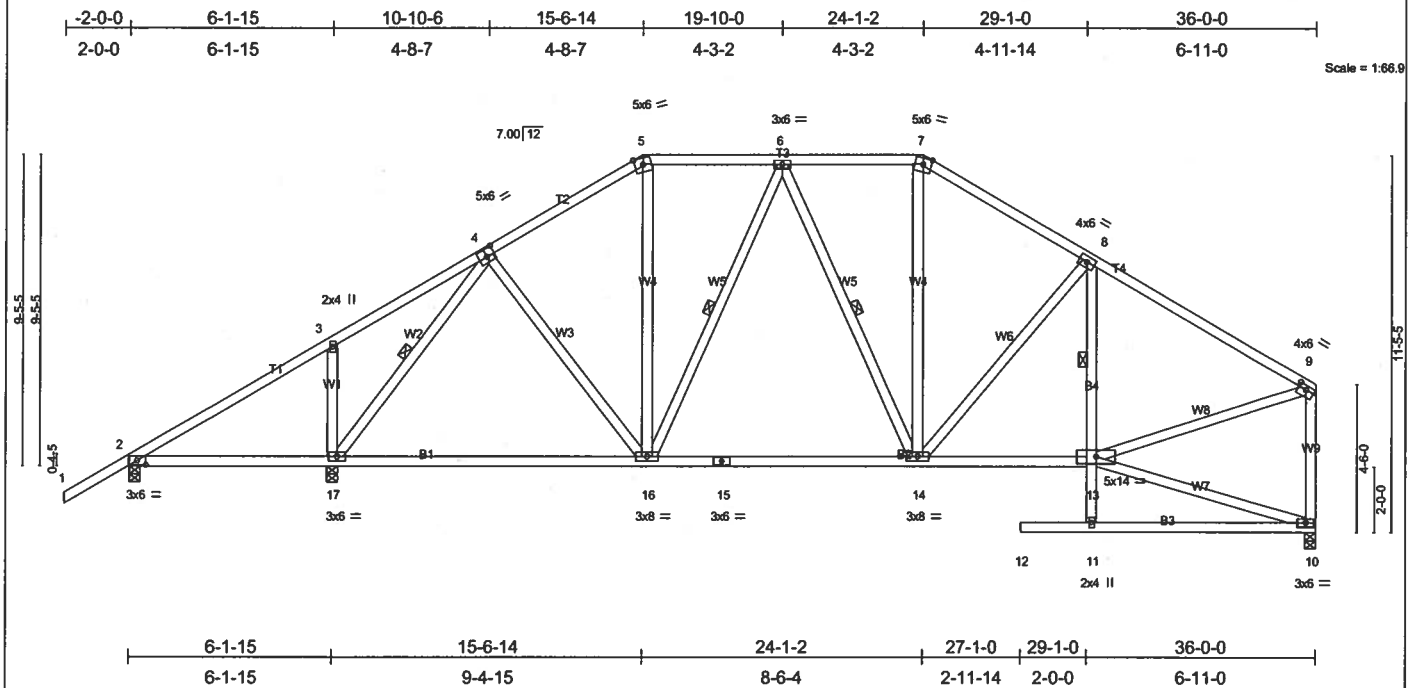
Job Reference (optional)
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Plate Offsets (X,Y): [2-0-3-4,0-1-8], [4-0-3-0,0-3-0], [9-0-3-0,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.93	Vert(LL) 0.08	2-17	>943	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.46	Vert(TL) -0.24	16-17	>999	180		
BCCL 10.0	Rep Stress Incr YES	WB 0.42	Horz(TL) 0.06	10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 252 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 B4 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-13 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 6-0-0 oc bracing: 2-17.
 1 Row at midpt 8-13
 1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=264/0-4-0, 17=1646/0-4-0, 10=1269/0-4-0

Max Horz 2=334(load case 4)
 Max Uplift 2=-213(load case 5), 17=-618(load case 5), 10=-360(load case 6)
 Max Grav 2=282(load case 9), 17=1646(load case 1), 10=1269(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-191/212, 3-4=-59/180, 4-5=-1075/488, 5-6=-879/475, 6-7=-995/522, 7-8=-1223/545, 8-9=-1383/487, 9-10=-1184/447
 BOT CHORD 2-17=-110/169, 16-17=-271/677, 15-16=-275/983, 14-15=-275/983, 13-14=-321/1116, 11-13=0/169, 8-13=-115/142, 11-12=0/0, 10-11=0/19
 WEBS 3-17=313/306, 4-17=-1323/453, 4-16=-130/382, 5-16=-65/279, 6-16=-332/250, 6-14=-93/209, 7-14=-98/327, 8-14=-212/199, 9-13=-316/1120, 10-13=-45/46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 618 lb uplift at joint 17 and 360 lb uplift at joint 10.
- 5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L166856	Truss T25	Truss Type HIP	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed May 31 15:09:00 2006 Page 1		

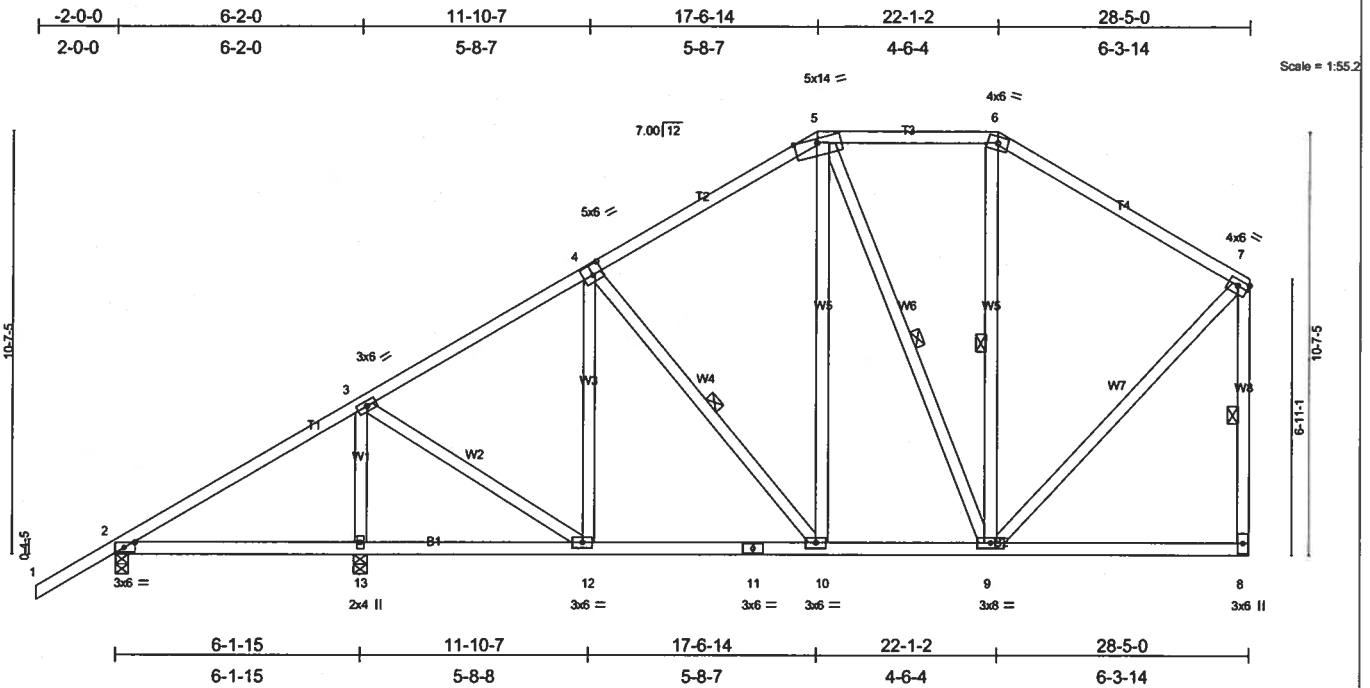


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [4:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	In	(loc)	I/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	Vert(LL) 0.08	2-13	>924	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL) -0.07	10-12	>999	180			
BCLL 10.0	Rep Stress Incr YES	WB 0.27	Horz(TL) 0.01	8	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 200 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.); 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=332/0-4-0, 13=1234/0-4-0, 8=912/Mechanical

Max Horz 2=432(load case 5)
Max Uplift 2=-187(load case 5), 13=-560(load case 5), 8=-267(load case 5)
Max Grav 2=332(load case 9), 13=1234(load case 1), 8=912(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-211/115, 3-4=-726/225, 4-5=-654/314, 5-6=-418/294, 6-7=-565/260, 7-8=-817/356
BOT CHORD 2-13=-120/57, 12-13=-120/57, 11-12=-259/558, 10-11=-259/558, 9-10=-173/498, 8-9=-17/29
WEBS 3-13=-1037/506, 3-12=-188/684, 4-12=-197/164, 4-10=-111/147, 5-10=-91/226, 5-9=-269/171, 6-9=-120/165, 7-9=-178/567

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 560 lb uplift at joint 13 and 267 lb uplift at joint 8.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L166856	Truss T26	Truss Type HIP	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6,200 s Jul 13 2005 Mitek Industries, Inc. Wed May 31 15:09:01 2006 Page 1		

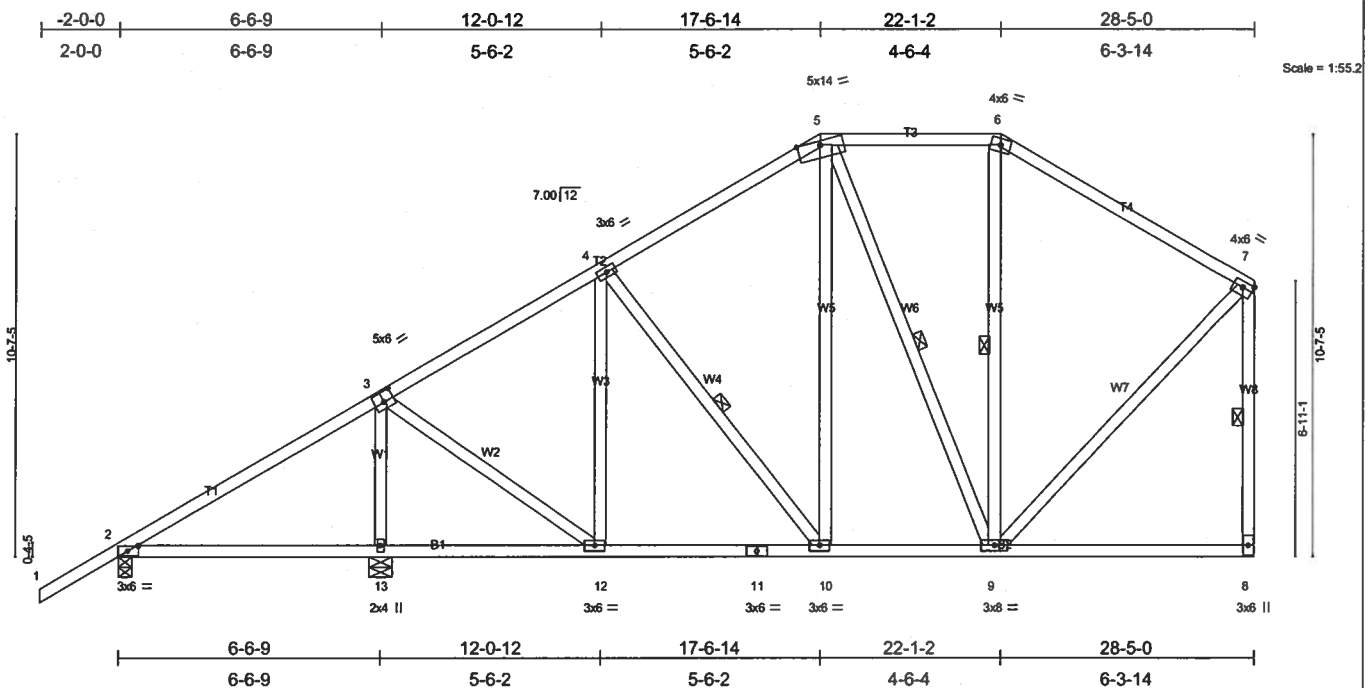


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(LL) -0.04 2-13 >999 240		
BCCL 10.0	Lumber Increase 1.25	WB 0.30	Vert(TL) -0.07 2-13 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 8 n/a n/a		
	Code FBC2004/TP12002			Weight: 201 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=351/0-4-0, 13=1230/0-7-3, 8=897/Mechanical

Max Horz 2=432(load case 5)
Max Uplift 2=-128(load case 5), 13=472(load case 5), 8=263(load case 6)
Max Grav 2=352(load case 9), 13=1230(load case 1), 8=897(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-203/115, 3-4=-684/231, 4-5=-631/312, 5-6=-408/293, 6-7=-554/259, 7-8=-801/354
BOT CHORD 2-13=-117/61, 12-13=-118/62, 11-12=-250/521, 10-11=-250/521, 9-10=-170/481, 8-9=-17/29
WEBS 3-13=-1025/505, 3-12=-197/646, 4-12=-212/149, 4-10=-89/144, 5-10=-87/203, 5-9=-251/168, 6-9=-124/163, 7-9=-177/553

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 2, 472 lb uplift at joint 13 and 263 lb uplift at joint 8.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L166856	Truss T27	Truss Type HIP	Qty 1	Ply 1	LOT 47 EMERALD COVE LISCOMB EAGLE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Wed May 31 15:09:01 2006 Page 1		

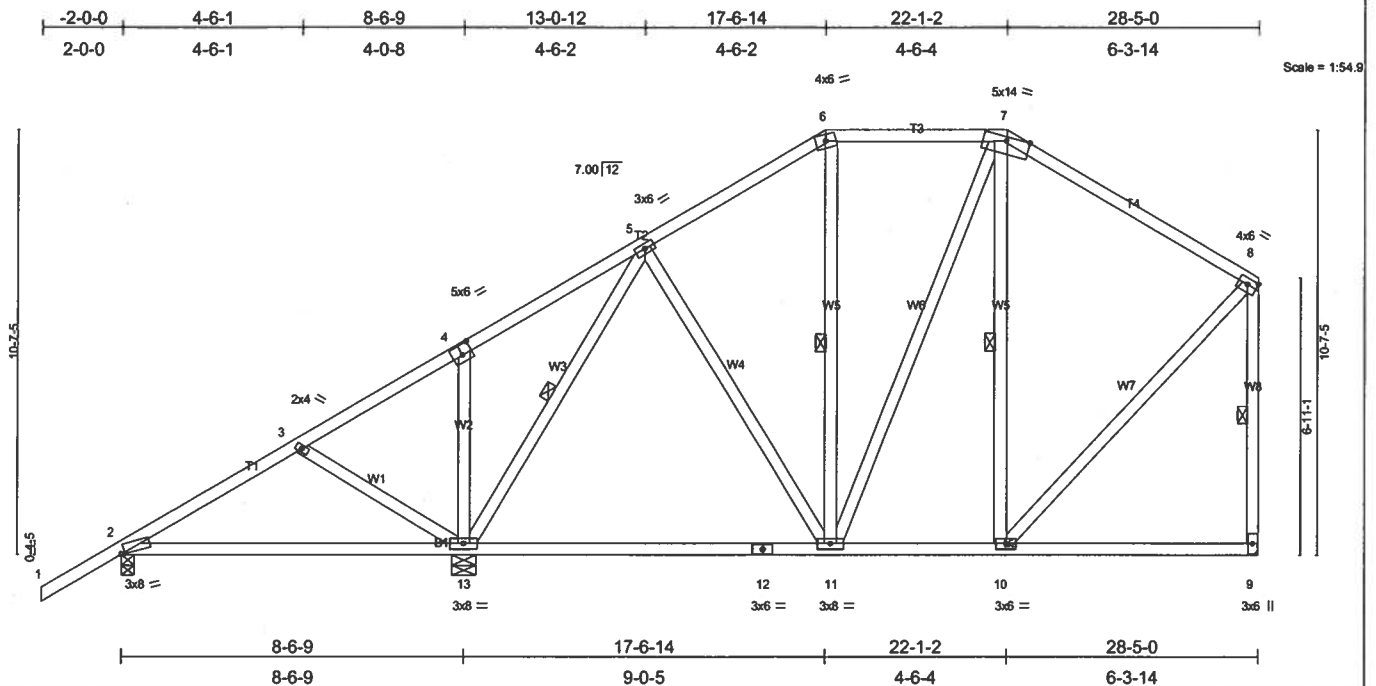


Plate Offsets (X,Y): [2-0-1-2,0-0-3], [4-0-3-0,0-3-0], [8-Edge,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	0.25	2-13	>399	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.36	Vert(TL)	0.23	2-13	>445	180		
BCCL 10.0	Rep Stress Incr	YES	WB 0.28	Horz(TL)	0.01	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 202 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 6-11, 7-10, 8-9

REACTIONS (lb/size) 2=414/0-4-0, 13=1265/0-7-3, 9=799/Mechanical
Max Horz 2=432(load case 5)
Max Uplift 2=230(load case 5), 13=629(load case 5), 9=226(load case 6)
Max Grav 2=418(load case 9), 13=1265(load case 1), 9=799(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=216/61, 3-4=201/115, 4-5=101/130, 5-6=509/247, 6-7=384/258, 7-8=483/220, 8-9=699/301
BOT CHORD 2-13=215/151, 12-13=151/313, 11-12=151/313, 10-11=103/345, 9-10=16/29
WEBS 3-13=231/252, 4-13=232/224, 5-13=729/305, 5-11=72/178, 6-11=45/137, 7-11=110/169, 7-10=204/114, 8-10=134/464

NOTES

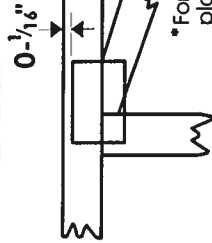
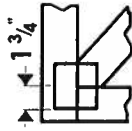
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 629 lb uplift at joint 13 and 226 lb uplift at joint 9.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

* This symbol indicates the required direction of slots in connector plates.



* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 X 4

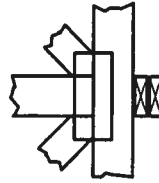
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



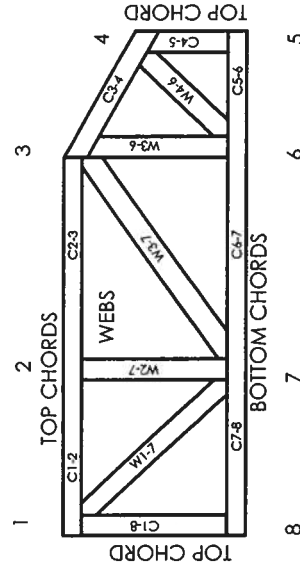
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths

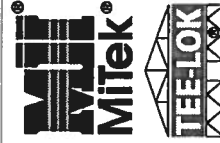


JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 96048, 9511, 9432A



MiTek Engineering Reference Sheet: MII-7473



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPMENT – EMERALD COVE
PROJECT NUMBER: PF05-034

**WIND LOAD AND STRUCTURAL CALCULATIONS
FOR**

**GATEWAY DEVELOPMENT
EMERALD COVE MODEL
LOT 47
EMERALD COVE SUBDIVISION**



GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: EMERALD COVE
Project: PF05-034
Client GATEWAY I
Calculations: Gary Gill, PE
Date: 5/26/2006

Design Basis

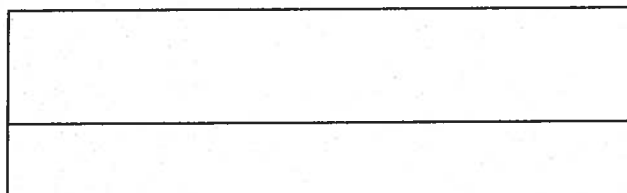
Design Loads

Wind Load 110
Floor Live Load
 Sleep Areas = 30 psf
 All Others = 40 psf
Floor Dead Load 10 psf
Wall Dead Load 10 psf
Roof Live Load 20 psf
Roof Dead Load 10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape Rectangle
Length 66.9 ft
Width 45.708 ft
Type 1 storey sog

References

2001 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: Gateway Development - Emerald Cove House**Analysis by:** Gary Gill**User Input Data**

Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.256	Deg
Type of Roof	Hipped	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	21.33	ft
Mean Roof Height (Ht)	14.65	ft
Width Perp. To Wind Dir (B)	66.90	ft
Width Paral. To Wind Dir (L)	45.71	ft
Damping Ratio (beta)	0.02	

*Red values should be changed only through "Main Menu"***Calculated Parameters****Type of Structure**

Height/Least Horizontal Dim	0.32
Flexible Structure	No

Calculated Parameters

Importance Factor	1
<i>Hurricane Prone Region (V>100 mph)</i>	
Table C6-4 Values	
Alpha =	7.000
zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
I =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method

Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
-------	---	------

Gust Factor Category II: Rigid Structures - Complete Analysis

Zm	Zmin	30.00 ft
Izm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$I*(zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^{0.63}))^{0.5}$	0.9036
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8681

Gust Factor Summary

G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85
---	--	------

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev ft	Kz	Kzt	qz lb/ft ²	Pressure (lb/ft ²)	
				Windward Wall*	
				+GCpi	-GCpi
21.33	0.64	1.00	16.73	8.65	14.10
20	0.62	1.00	16.43	8.45	13.90
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

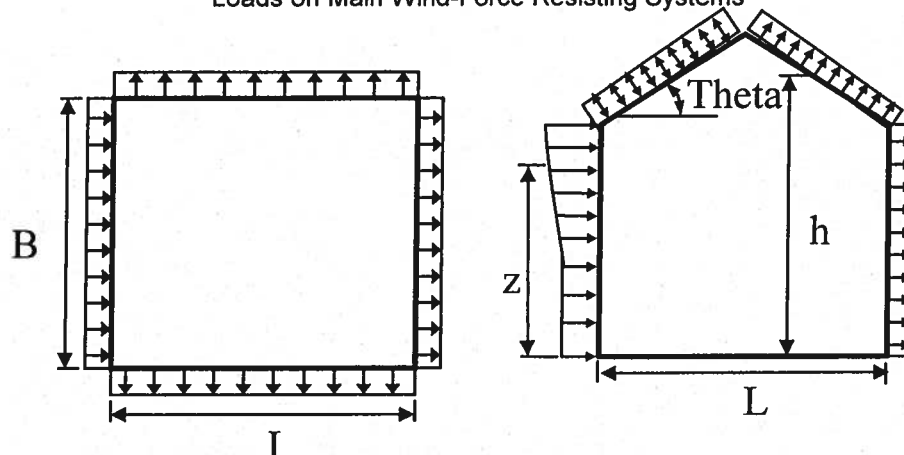
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, C_p

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
K_h	$2.01 \cdot (15/z_g)^{2/\alpha}$	0.57	
K_{ht}	Topographic factor (Fig 6-2)	1.00	
Q_h	$.00256 \cdot (V)^2 \cdot I \cdot K_h \cdot K_{ht} \cdot K_d$	15.13	psf
K_{hcc}	Comp & Clad: Table 6-5 Case 2	0.70	
Q_{hcc}	$.00256 \cdot V^2 \cdot I \cdot K_{hcc} \cdot K_{ht} \cdot K_d$	18.45	psf

Wall Pressure Coefficients, C_p	
Surface	C_p
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, C_p	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 66.9 ft Face	C_p	Pressure (psf)	
<i>Additional Runs may be req'd for other wind directions</i>		+GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 66.9 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge ($\theta \geq 10$) - for Wind Normal to 66.9 ft face			
Windward - Max Negative	-0.19	-5.20	0.25
Windward - Max Positive	0.28	0.84	6.29
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.48	-2.48
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All θ) - for Wind Normal to 66.9 ft face			
Dist from Windward Edge: 0 ft to 7.325 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.325 ft to 14.65 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 14.65 ft to 29.3 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 29.3 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

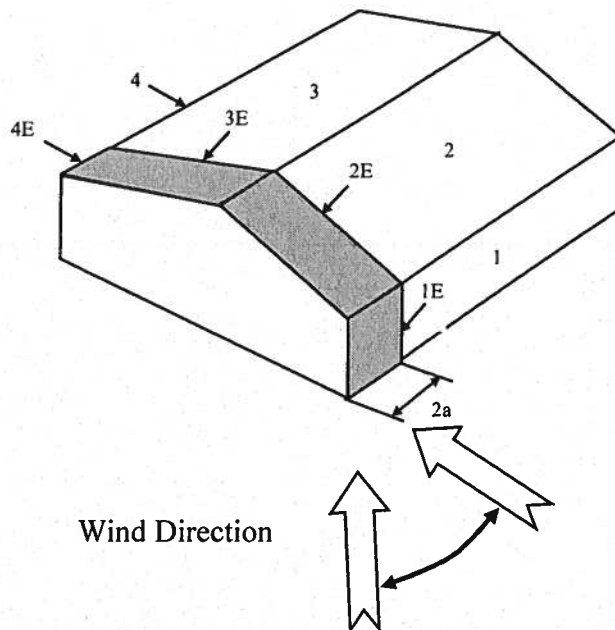
WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf
Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{2/\alpha} &= & 0.57 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13
 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

$$* p = q_h \cdot (GC_{pf} - GC_{pi})$$



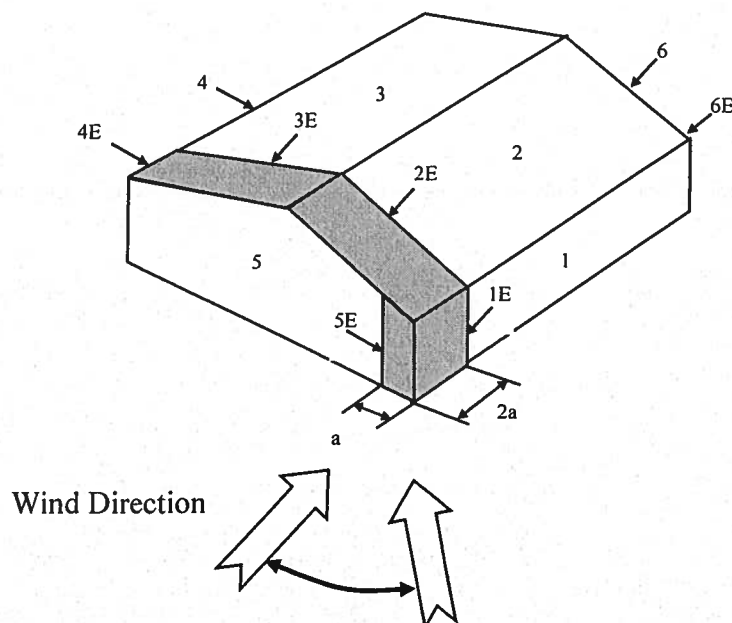
WIND98 v3-02
Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf
Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (15/z_g)^{(2/\alpha)} &= & 0.57 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 15.13 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

$$* p = q_h * (GCpf - GCpi)$$



WIND98 v3-02

Wind Load Design per ASCE 7-98
v

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7 Roof Dia	13.89
		Length of	
Vertical Roof height		13.33 Building	66.9
		Width of	
2nd Floor height	0	Building	45.708
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall -	11.2 psf
			Leeward	
Leeward Roof - Surface 3	-3.78	psf	Wall -	
			Surface 4	-2.88 psf
			Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz. (psf)	2.97
Leeward Roof Horz. (psf)	-1.90
Total	4.88
Tributary area (roof)	434.85
Roof shear values	2120.97

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	0.00

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	41.58	60.8	
Shearwall #1 length	28.58	35.8	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.28	0.22	
Rigidities of shearwalls	11.61	14.67	
Lateral load on shearwall column (lbs) based on rigidity	2600.71	3288.07	
Percent Full-Height Sheathing Shearwall #1	68.73%	58.88%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	13804.14	17291.40	
Stress Ratio	0.69	0.63	
uplift at shear ends	727.98	734.76	
shear and uplift between holddown, v and u	91.00	91.85	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7 Roof Dia	13.89
		Length of	
Vertical Roof height		13.33 Building	66.9
		Width of	
2nd Floor height	0	Building	45.708
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz. (psf)	8.78
Leeward Roof Horz. (psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	304.64
Roof shear values to each shearwall	3180.44

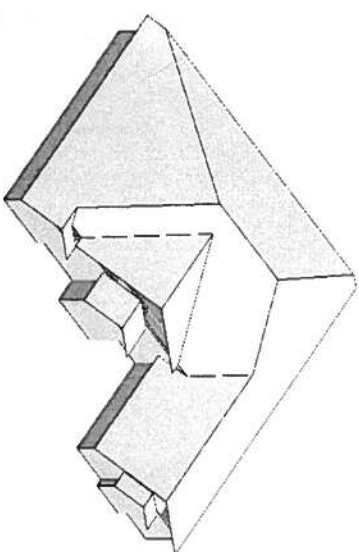
Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	0.00

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	
Full wall length	66.9	66.9	
Shearwall #1 length	36.9	36.9	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.22	0.22	
Rigidities of shearwalls	15.14	15.14	
Lateral load on shearwall column (lbs) based on rigidity	3727.92	3727.92	
Percent Full-Height Sheathing			
Shearwall #1	55.16%	55.16%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	17822.70	17822.70	
Stress Ratio	0.63	0.63	
uplift at shear ends	808.22	808.22	
shear and uplift between holddown, v and u	101.03	101.03	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465



BEARING HEIGHT SCHEDULE

	8'-0"
	10'-0"

NOTES:

- 1) REFER TO HD 91 RECOMMENDATIONS FOR HANDING INSTALLATION AND TEMPORARY BRACING. REFER TO ENGINEER DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VDS FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5.) ALL WALLS SHOWN ON PLACEMENT PLANS ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) 5/42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BEARING ADVERT. (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VDS. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVISED AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONTAINING TO MAKE MAJOR CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU!

Issued Drawing No. : _____

Approved by: _____ Date: _____



Bunnell

PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville

PHONE: 904-772-6100 FAX: 904-772-1973

Lake City

PHONE: 904-755-6694 FAX: 904-755-7973

Sanford

PHONE: 407-322-0059 FAX: 407-322-9553

BUILDER:

LIPSCOMB EAGLE

LOT 47 EMERALD COVE

EMERALD COVE

SCALE: NTS

DATE: 05-30-06

JRD L166856