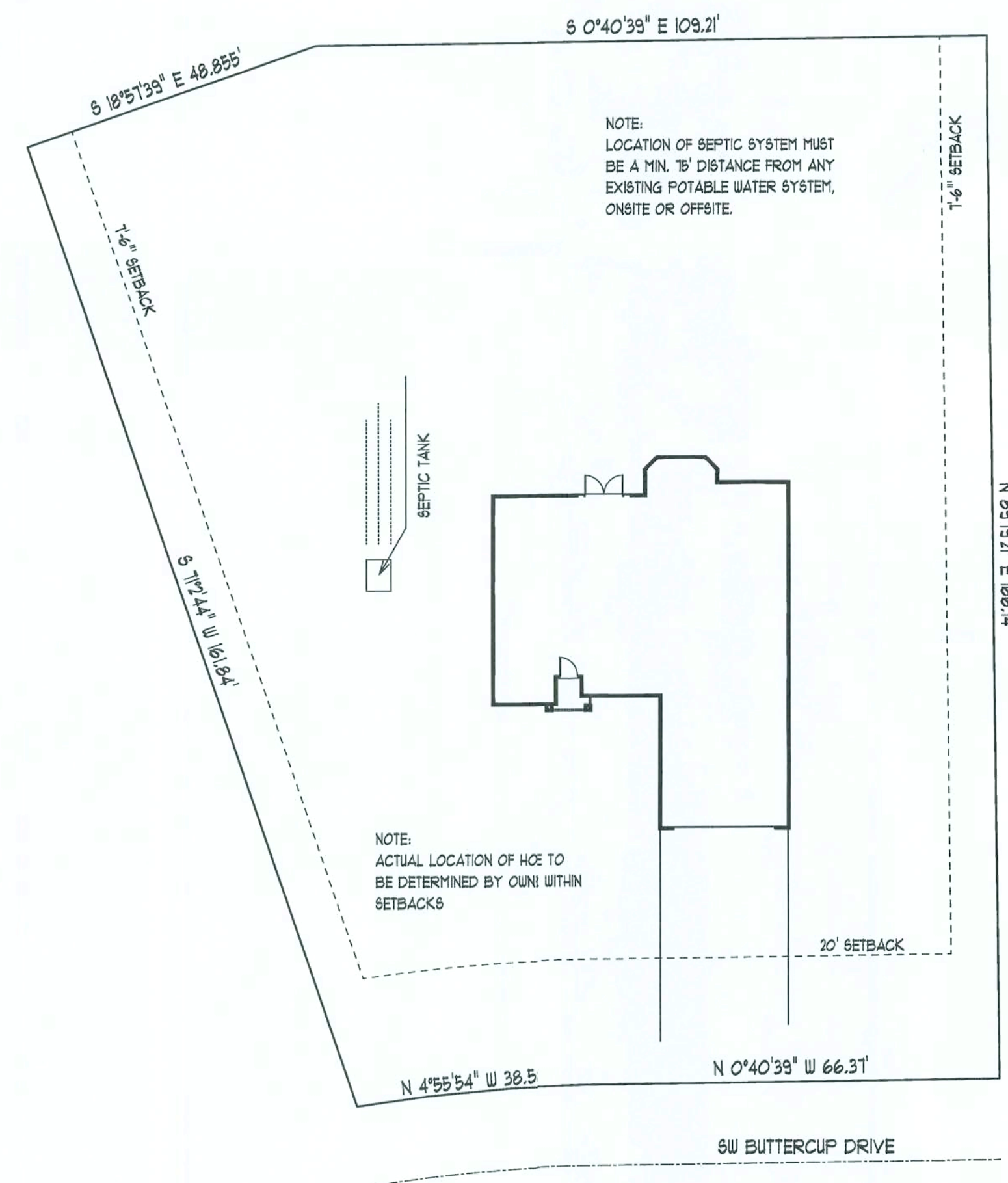


ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



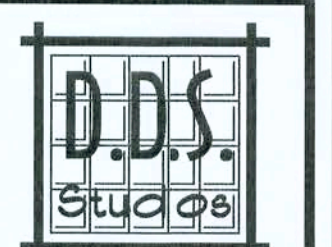
SITE PLAN
SCALE: 1" = 20'
ROLLING MEADOWS
LOT# 41



FILE COPY

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April 06, 2001



D.D.S. STUDIOS
P.O. Box 271
Lake City, FL 32056
(904) 784-081

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EXTERIOR ELEVATIONS

SITE PLAN

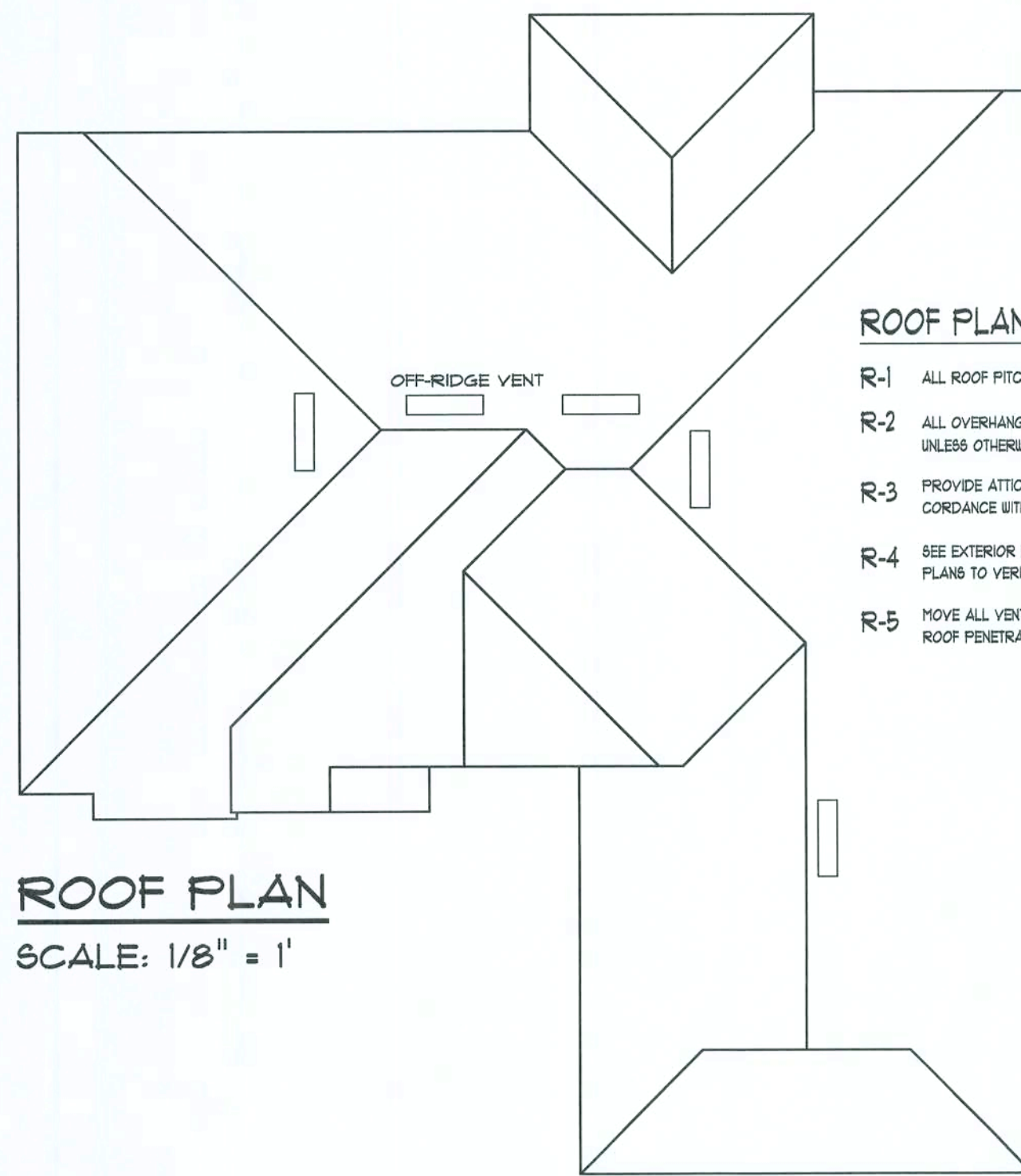
SHEET NUMBER
1 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER
010708

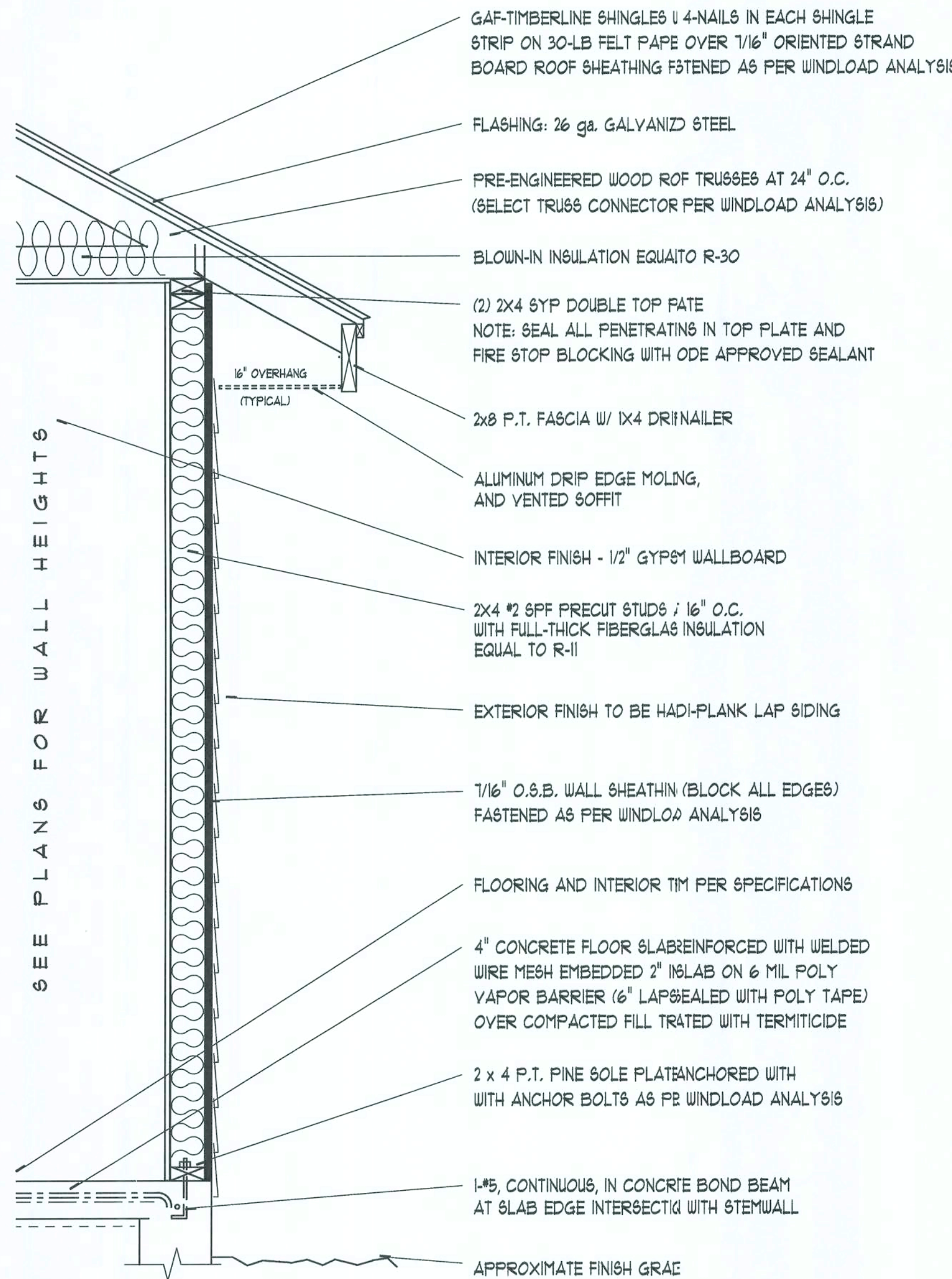
Daniel Shaheen
Daniel Shaheen

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



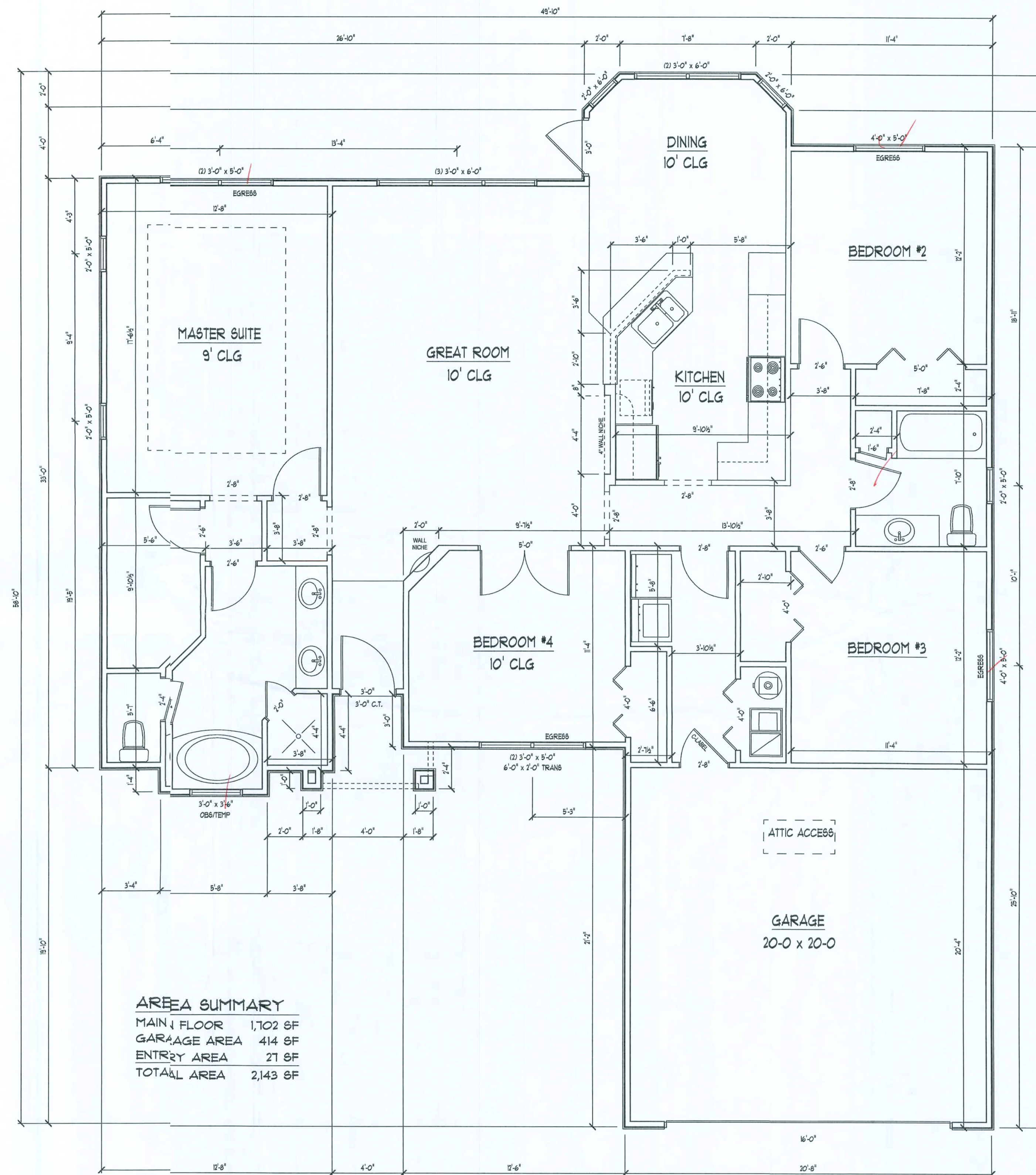
ROOF PLANNOTES

- R-1 ALL ROOF PITCHES UNLESS OTHERWISE NOTED
- R-2 ALL OVERHANGS AND 12" AT GABLES UNLESS OTHERWISE NOTED
- R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
- R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HILL HEIGHTS
- R-5 MOVE ALL VENT AND OTHER ROOF PENETRATIONS TO REAR



TYPICAL WALL SECTION

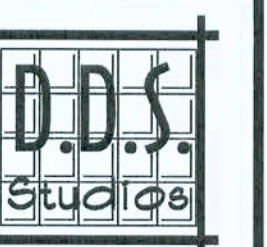
SCALE: 1" = 1'0"



FLOOR PLAN

SCALE: 1/4" = 1'

April 06, 2001



D.D.S. STUDIOS

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FLOOR PLAN

ROOF PLAN

TYPICAL WALL SECTION

SHEET NUMBER

2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

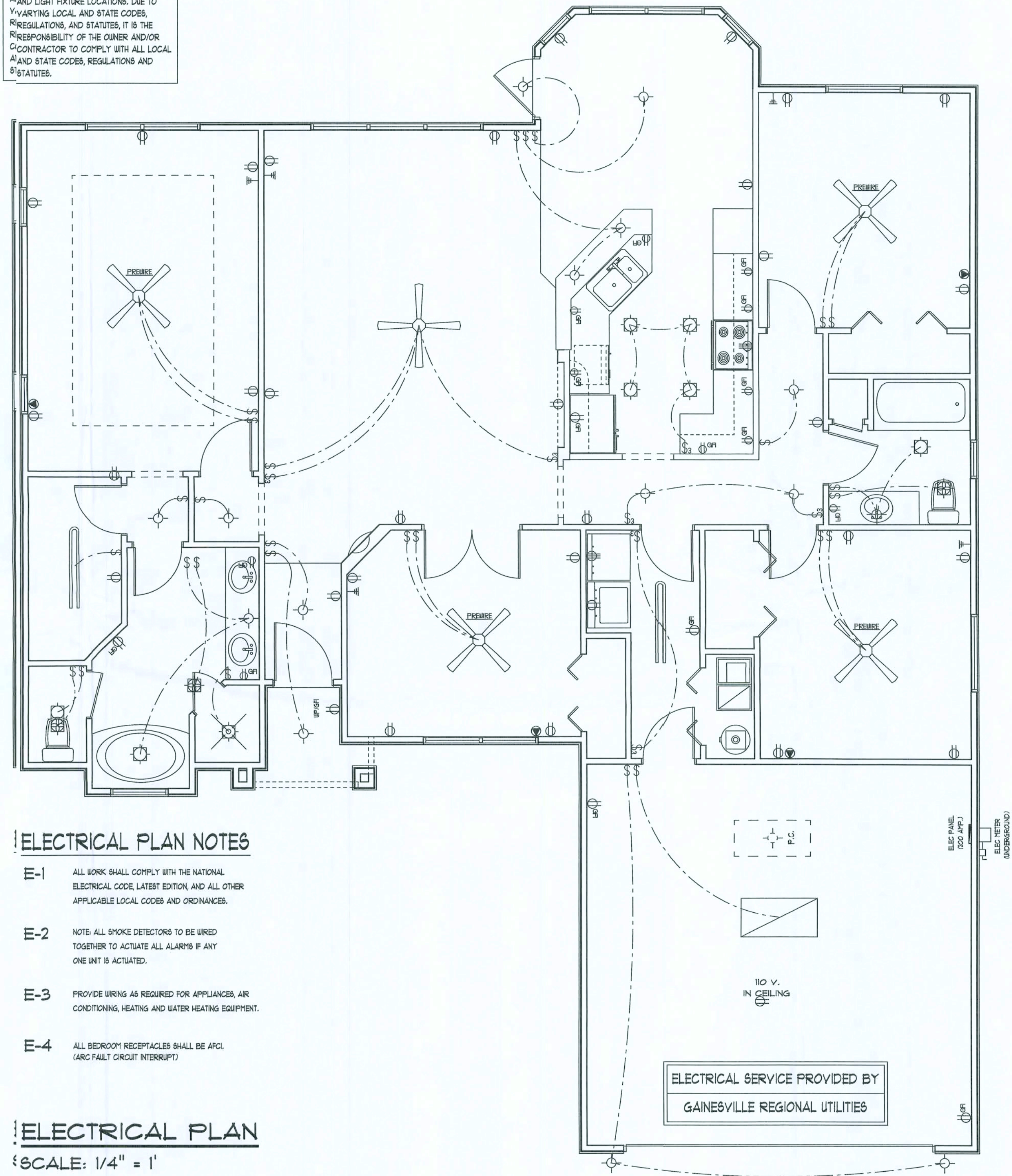
PROJECT NUMBER

010708

Daniel Shansen
Daniel Shansen

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC
WITH SUGGESTED SWITCH, RECEPTACLE
AND LIGHT FIXTURE LOCATIONS. DUE TO
VARYING LOCAL AND STATE CODES,
REGULATIONS, AND STATUTES, IT IS THE
RESPONSIBILITY OF THE OWNER AND/OR
CONTRACTOR TO COMPLY WITH ALL LOCAL
AND STATE CODES, REGULATIONS AND
STATUTES.

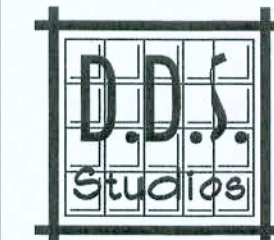


- ELECTRICAL PLAN NOTES**
- E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.
 - E-2 NOTE ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTIVATE ALL ALARMS IF ANY ONE UNIT IS ACTUATED.
 - E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.
 - E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ELECTRICAL PLAN
SCALE: 1/4" = 1'

Daniel Shaheen
Daniel Shaheen

April 06, 2007



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THE BREANNA

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ELECTRICAL PLAN

SHEET NUMBER
3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER
010708

REVISIONS	



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILD AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GULIAM BEAMS: GULIAM BEAM, GLB, 24F-V3SP, $F_b = 2.4ksi$, $E = 1800ksi$, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (12d) FOR PANEL EDGES, 12dC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4dC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 19" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OWNS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

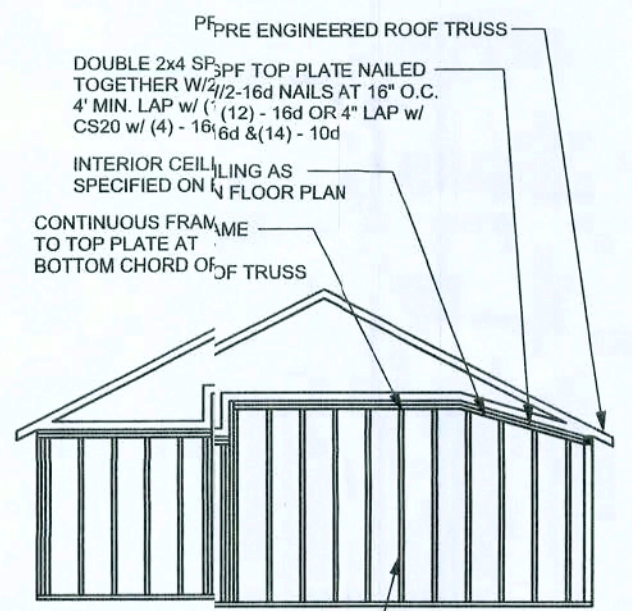
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

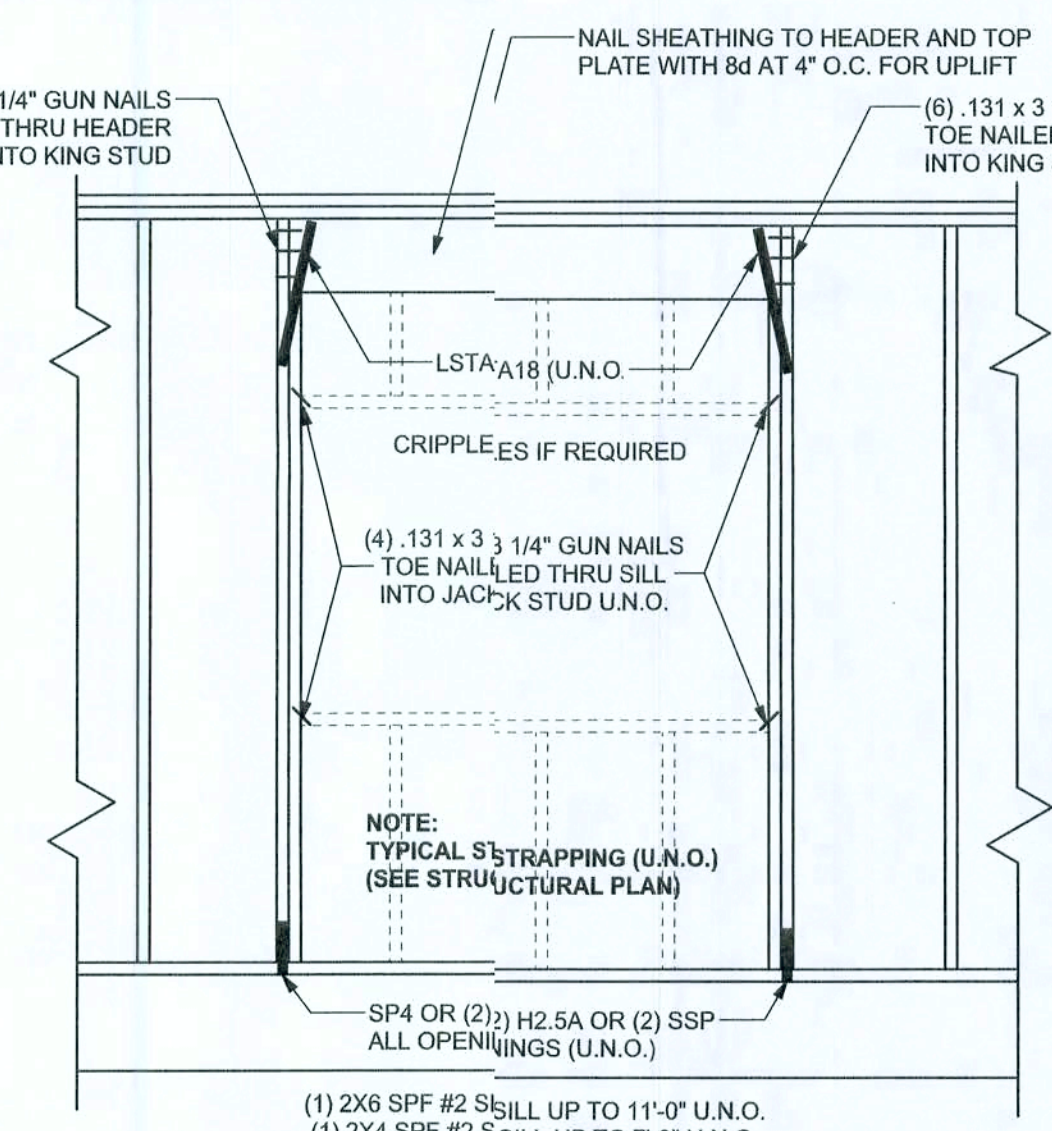
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



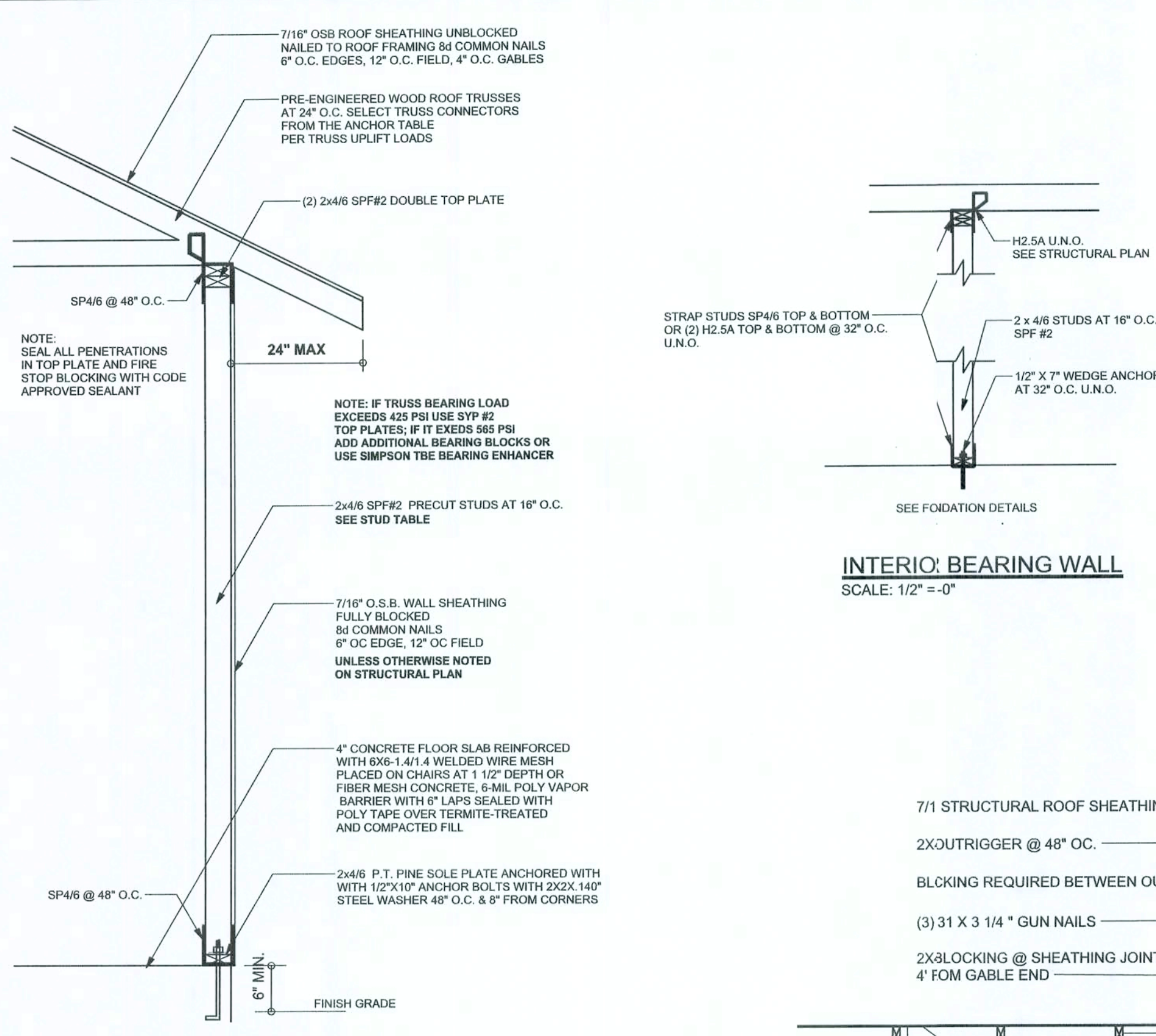
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"



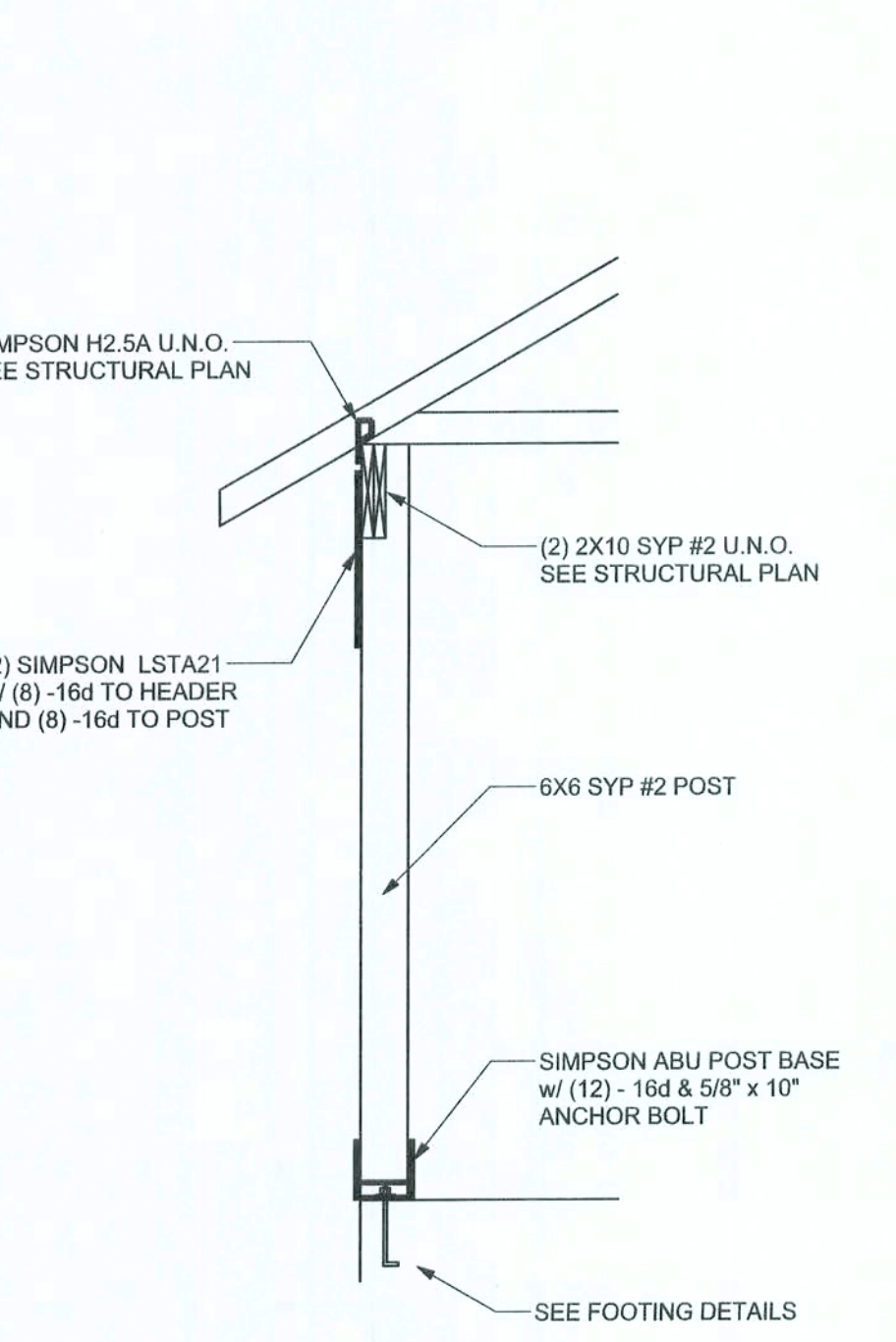
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" OC x 0.85 = 13.6" O.C.

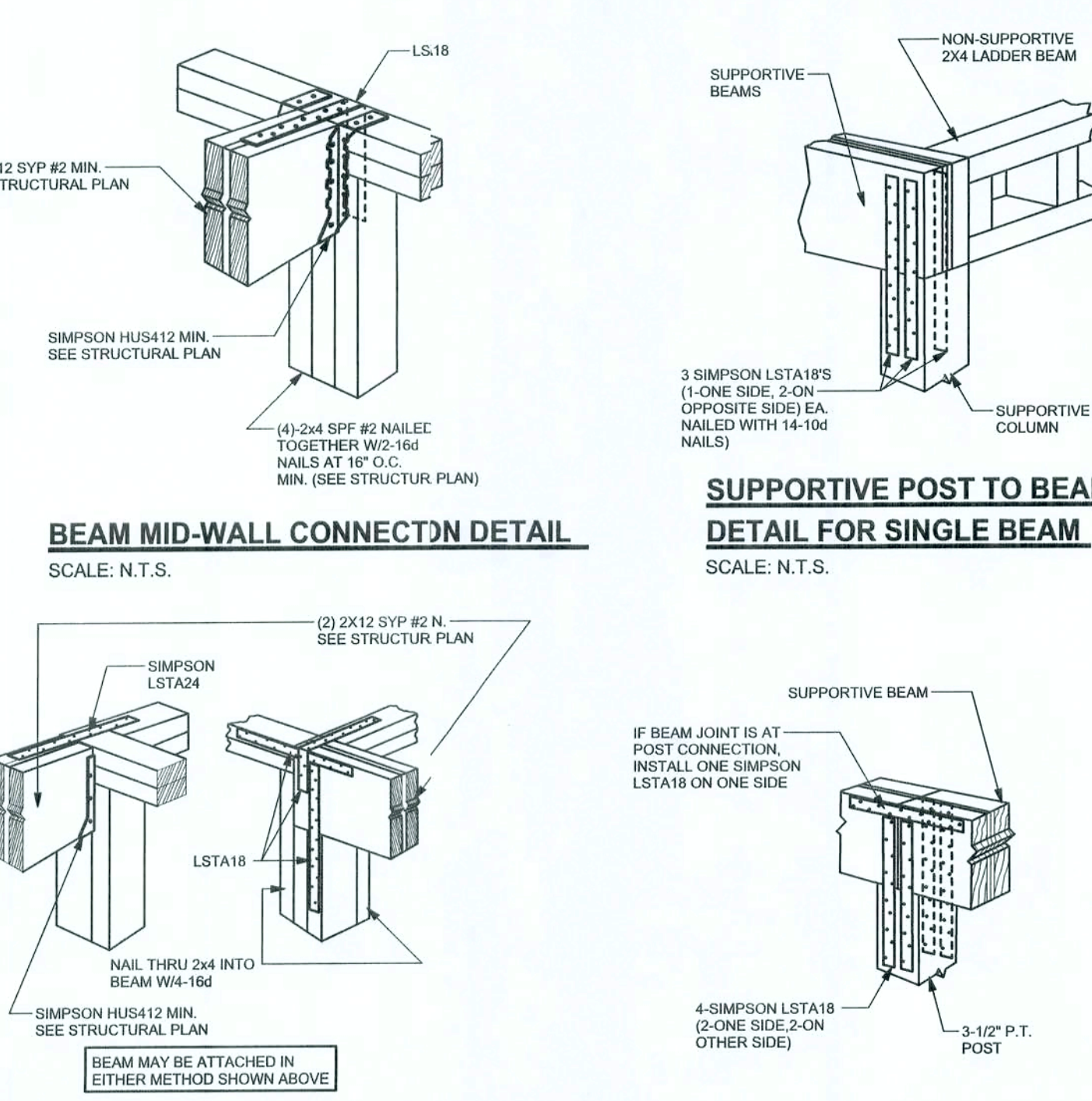


TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

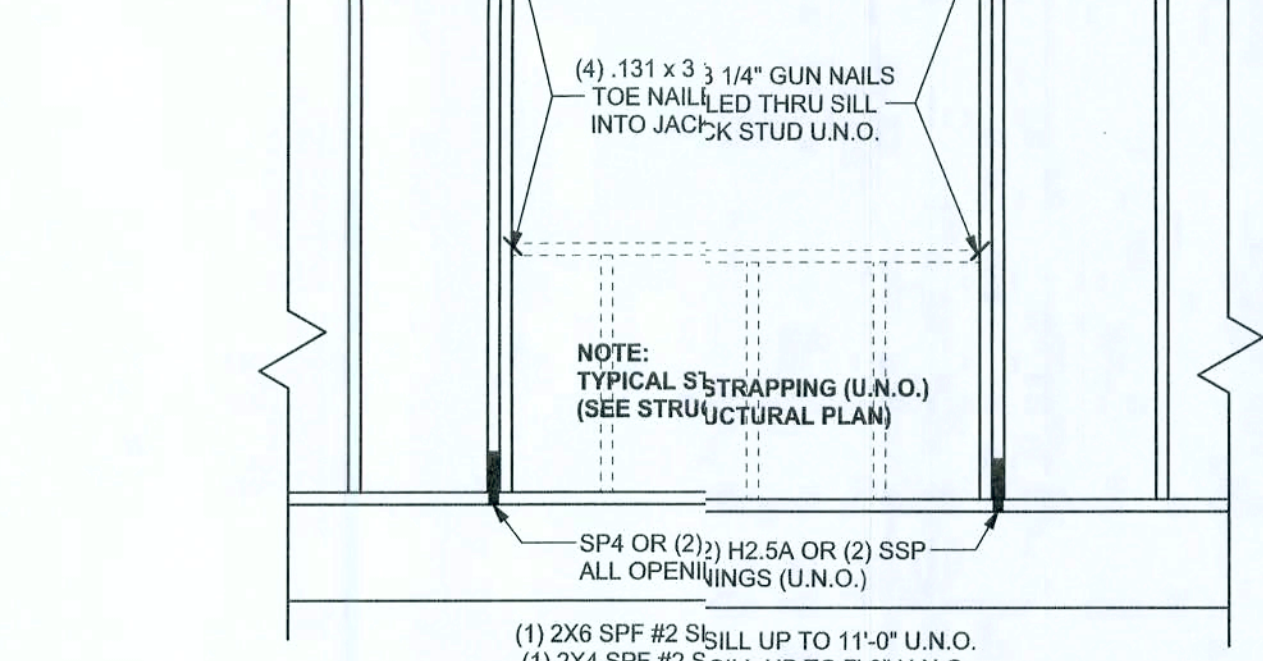


BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG2		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 685	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 685	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*				TO STUDS	TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTD1	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H02A	2-5/8" BOLTS		5/8" AB
< 4175	< 3955	HTT16	16-16d		5/8" AB
< 1400	< 1400	PMD42	16-16d		
< 3335	< 3335	HPMD42	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED =	110 MPH
2.) WIND EXPOSURE =	B
3.) WIND IMPORTANCE FACTOR =	1.0
4.) BUILDING CATEGORY =	II
5.) ROOF ANGLE =	10-45 DEGREES
6.) MEAN ROOF HEIGHT =	<30 FT
7.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	

Zone	Effective Wind Area (ft ²)	
	10	100
1	19.9	21.8
2	19.9	25.5
2 On G	40.6	40.6
3	19.9	25.5
3 On G	48.3	42.4
4	21.8	23.6
5	21.8	29.1
Doors & Windows	21.8	29.1
Worst Case (Zone 5, 10 R2)		
8x7 Garage Door	19.5	22.9
16x7 Garage Door	18.5	21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

WINDLOAD ENGINEER: Mark Discosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relate to wind engineering comply with section R301.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOSWAY
P.E. 53915

164707
SEAL

Compass Builders

Spec House
Lot 47
Rolling Meadows S/D

ADDRESS:
Lot 47 Rolling Meadows S/D
Columbia County, Florida

Mark Discosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 219 - 4871

PRINTED DATE:
April 16, 2007

DRAWN BY: David Discosway

CHECKED BY:

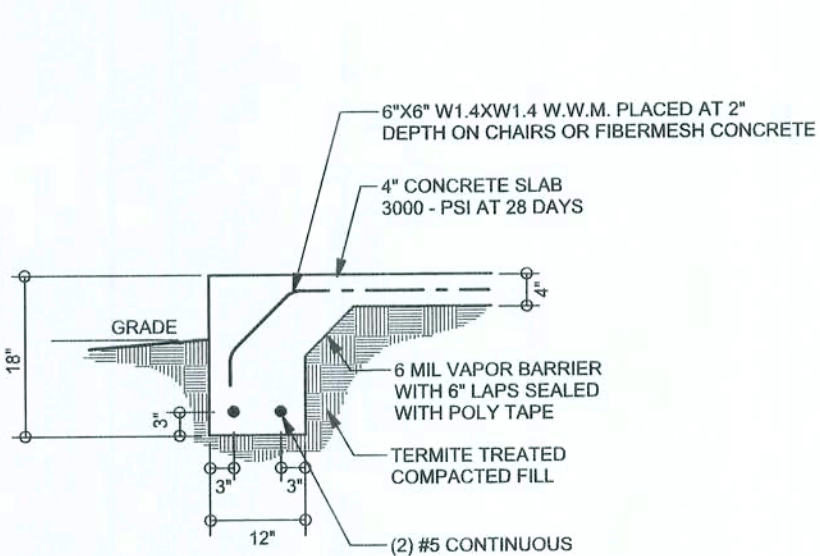
FINAL DATE:
16 / Apr / 07

JOB NUMBER:
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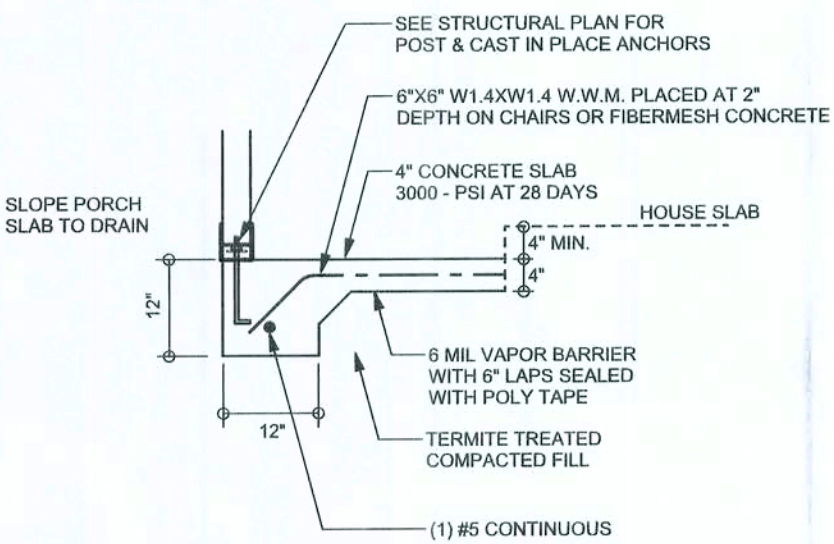
DRAWING NUMBER
S-1
OF 3 SHEETS

REVISIONS	

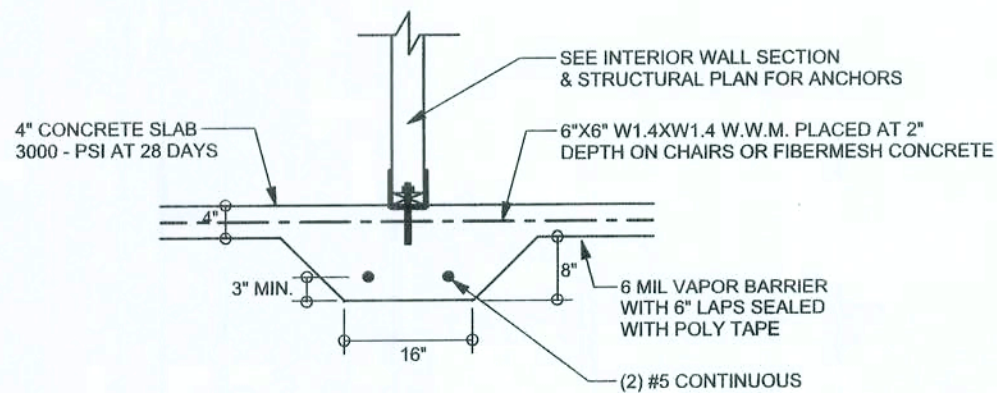
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



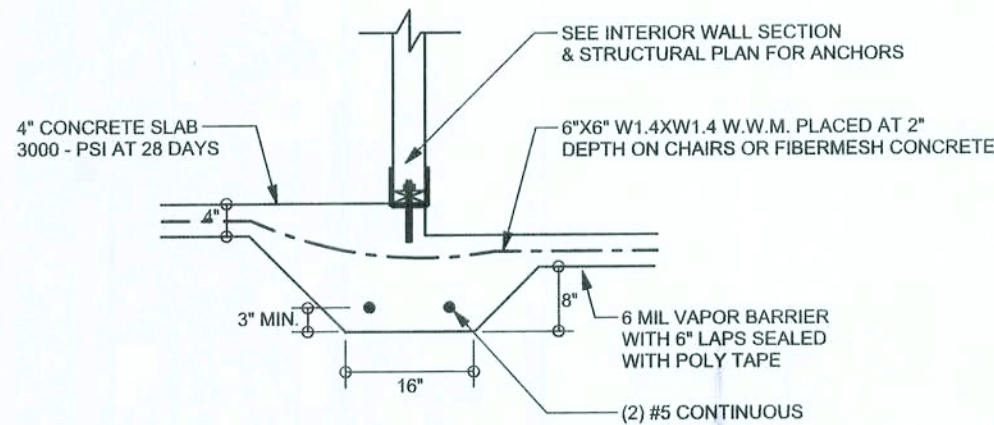
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



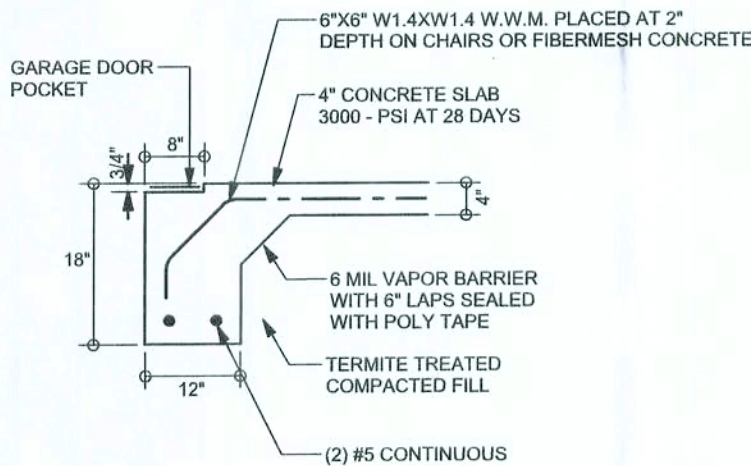
F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



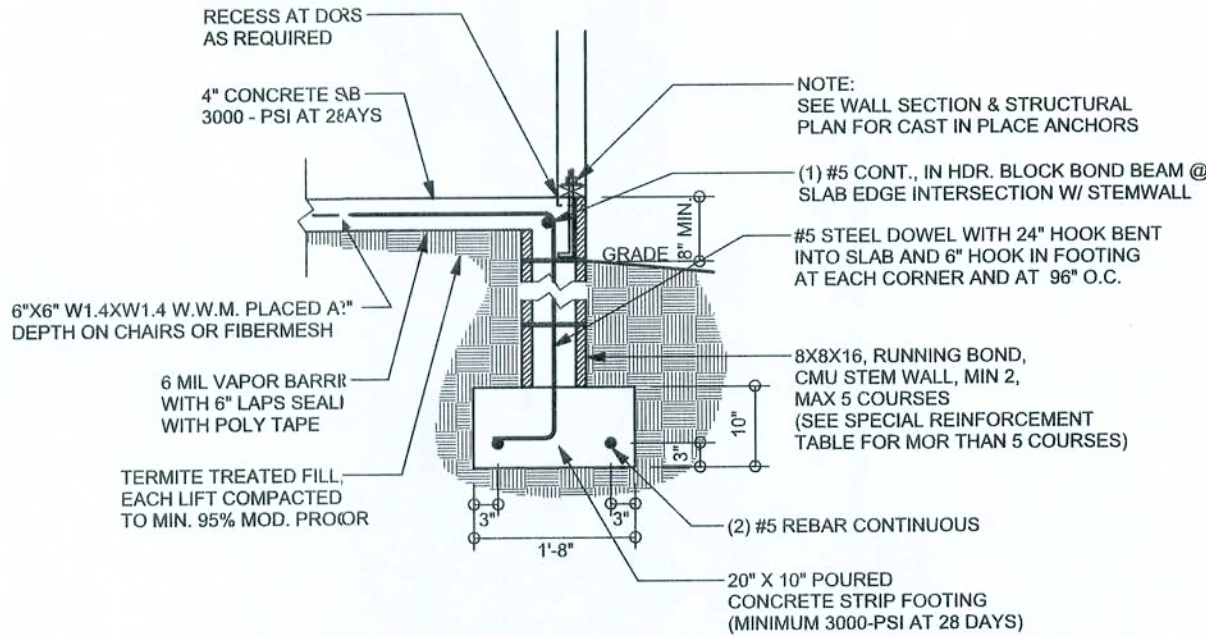
F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



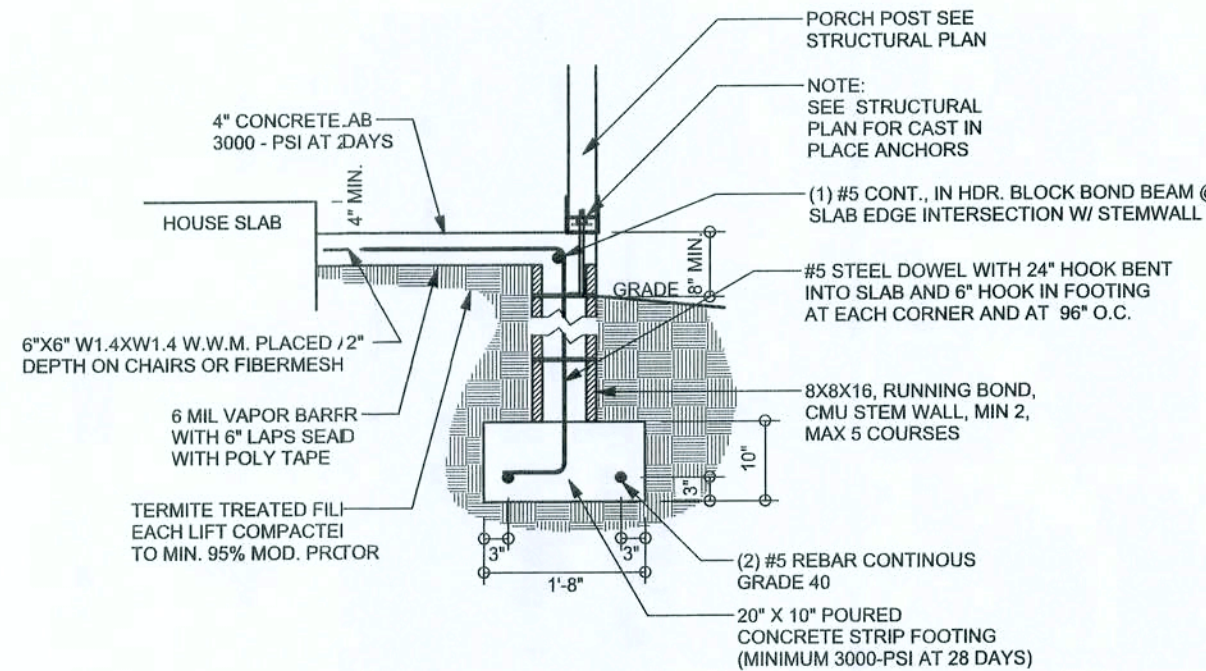
F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F9 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

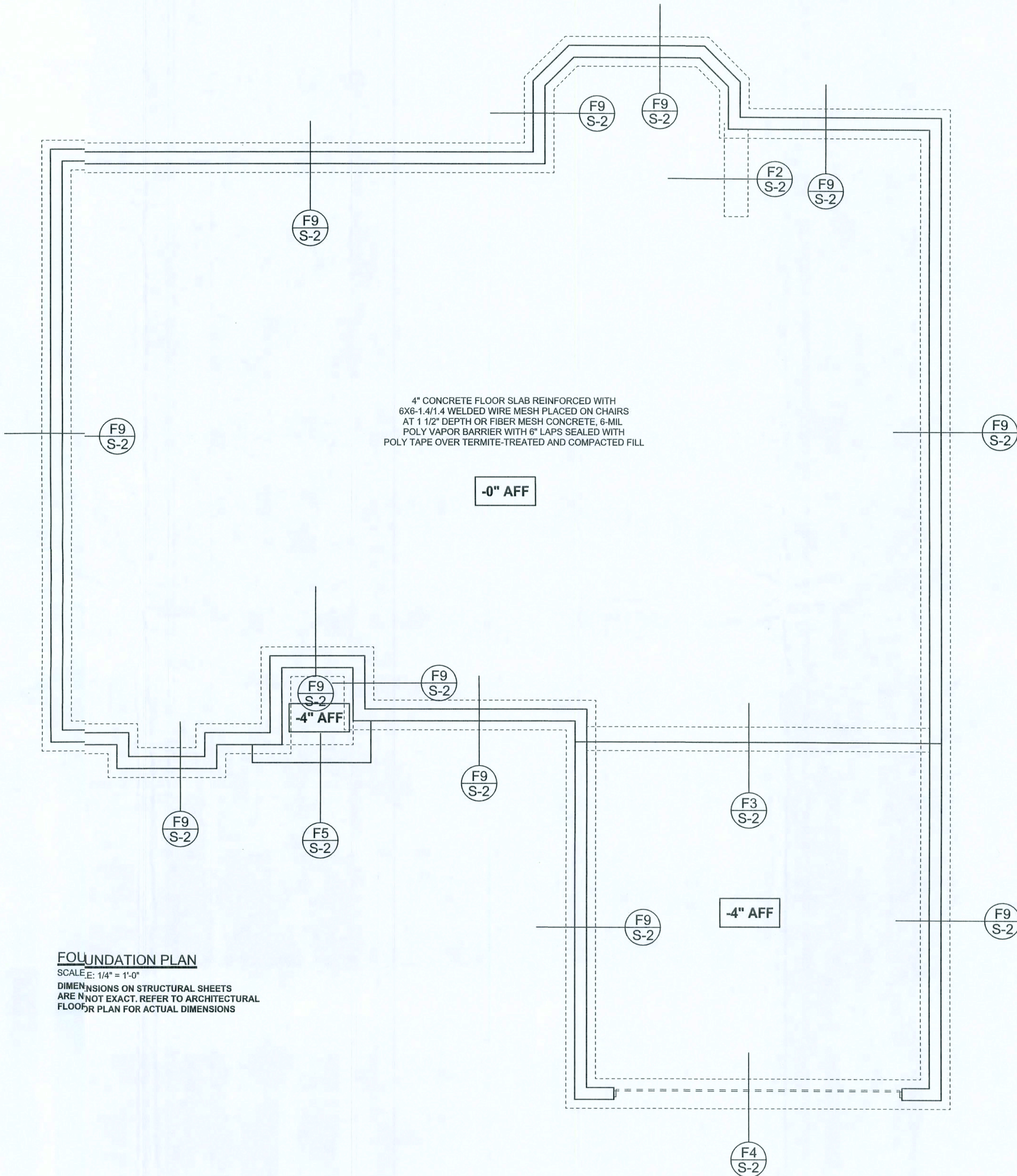


F1 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforcement at the top. The vertical steel is to be placed toward the tension side of the CMU wallway from the soil pressure, within 2" of the exterior side of the wall. If the wall is over 8ft high, add horizontal ladder reinforcement at 16" O.C. vertically or a horizontal bond beam w/ #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.1	3.7	96	96	96	96	96	96
4.1	4.3	88	96	96	96	96	96
5.1	5.0	56	96	96	96	96	96
6.1	5.7	40	80	96	80	96	96
6.1	6.3	32	56	80	56	96	96
7.1	7.0	24	40	56	40	80	96
8.1	7.7	16	32	48	32	64	80
8.1	8.3	8	24	32	24	48	64
9.1	9.0	8	16	24	16	40	48



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Discoway,
P.E. No. 53915, FOS 96, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section FOS 2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
P.E. 53915

Mark Discoway
16 APR 07
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Compass Builders

Spec House
Lot 47
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PRINTED DATE:
April 16, 2007

DRAWN BY: David Discoway
CHECKED BY:

FINALS DATE:
16 / Apr / 07

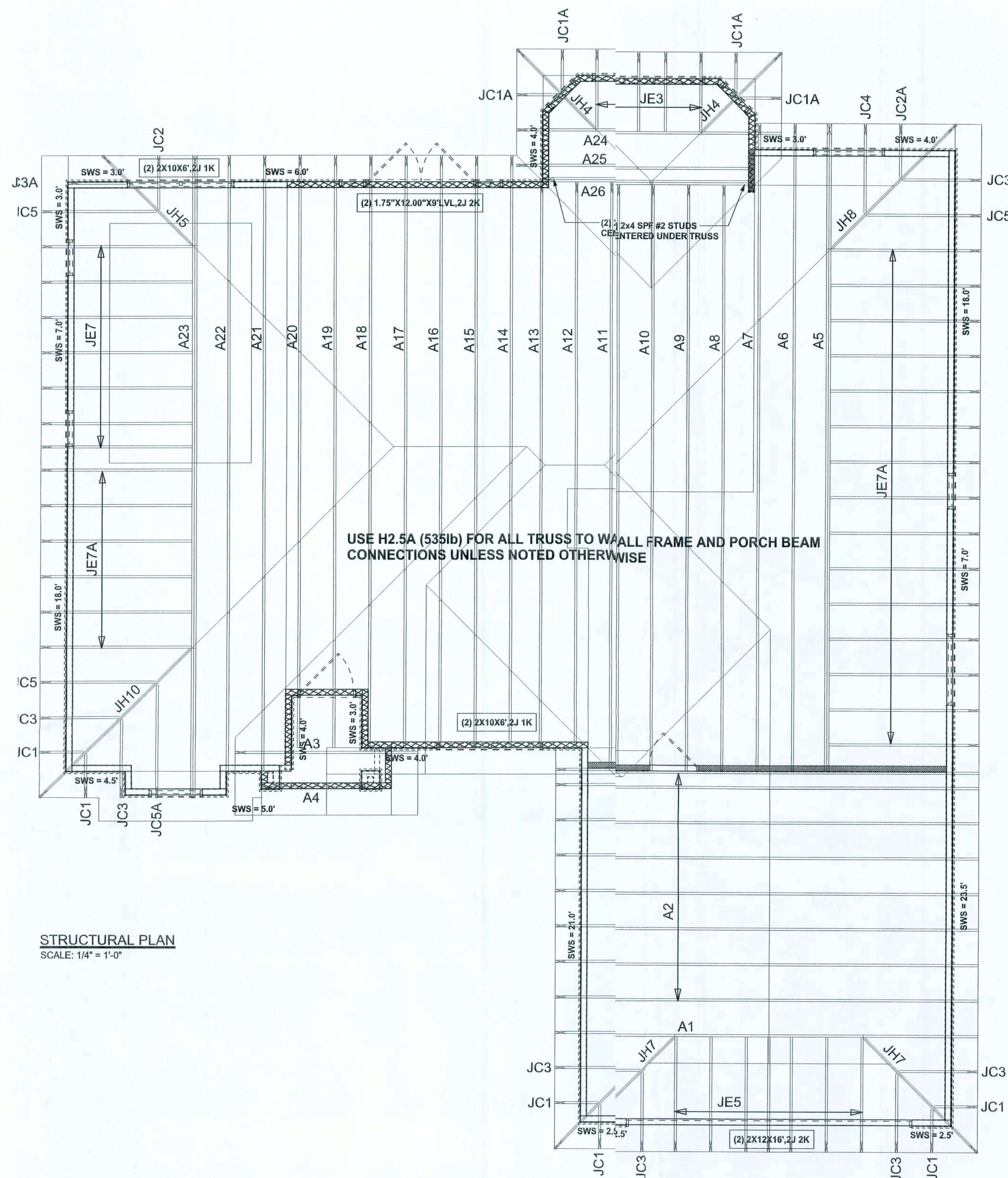
JOB NUMBER:
70-132

DRAWING NUMBER

S-2

OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE


STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND

(2) 2X10X0', 1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
NUMBER OF KING STUDS (FULL LENGTH)	
NUMBER OF JACK STUDS (UNDER HEADER)	
SPAN OF HEADER	
SIZE OF HEADER MATERIAL	
NUMBER OF PLIES IN HEADER	

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	35.4'	87.5'
LONGITUDINAL	29.6'	34.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER: W. B. HOWLAND TRUSS JOB #4494

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede all other dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2 Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
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April 16, 2007

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David Disosway

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FINALS DATE:
16 / Apr / 07

JOB NUMBER:
70413

DRAWING NUMBER

S-3

OF 3 SHEETS