

OPENING FORCE TRANSFER

(TYP.) CORNER FRAMING

WOOD FRAME

- MIN. 1/2" ANCHOR

WITHIN 6" EACH SIDE

(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

LL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER ULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOLINDATION: CONFIRM THAT THE FOLINDATION DESIGN & SITE CONDITIONS MEET

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4 FB = 85KSL WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLI

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT

(25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN

UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING

ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd

9x7 GARAGE DOOR

16x7 GARAGE DOOR

Stated dimensions supercede scale dimensions. Refer all questions to Mark Disosway, P.E. for resolution Do not proceed without clarification

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examined this plan, and that the applicable portions of the plan, relating to wind engineering

comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge. LIMITATION: This design is valid for one

ouilding, at specified location.

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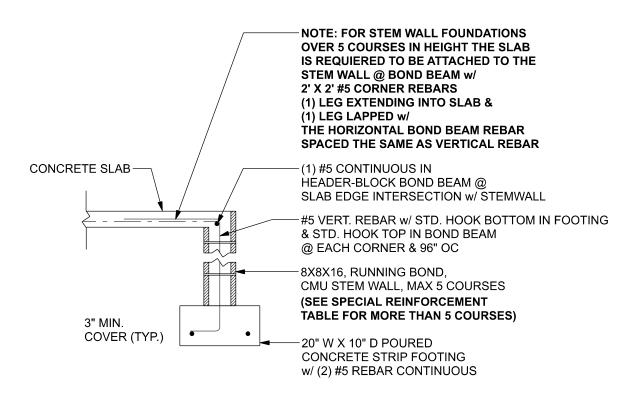
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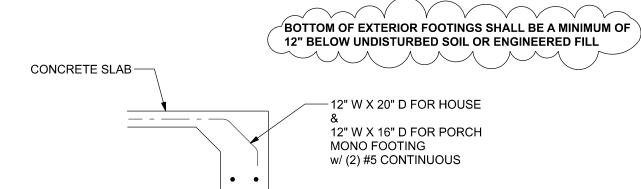
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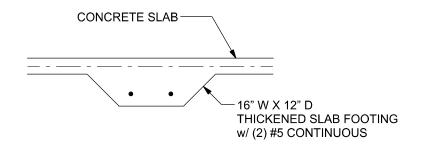
JOB NUMBER: 220567 **S-1** OF 3 SHEETS



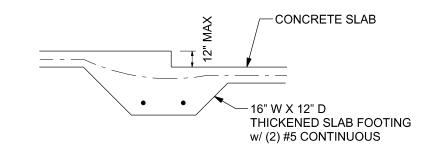
STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



OPTIONAL MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



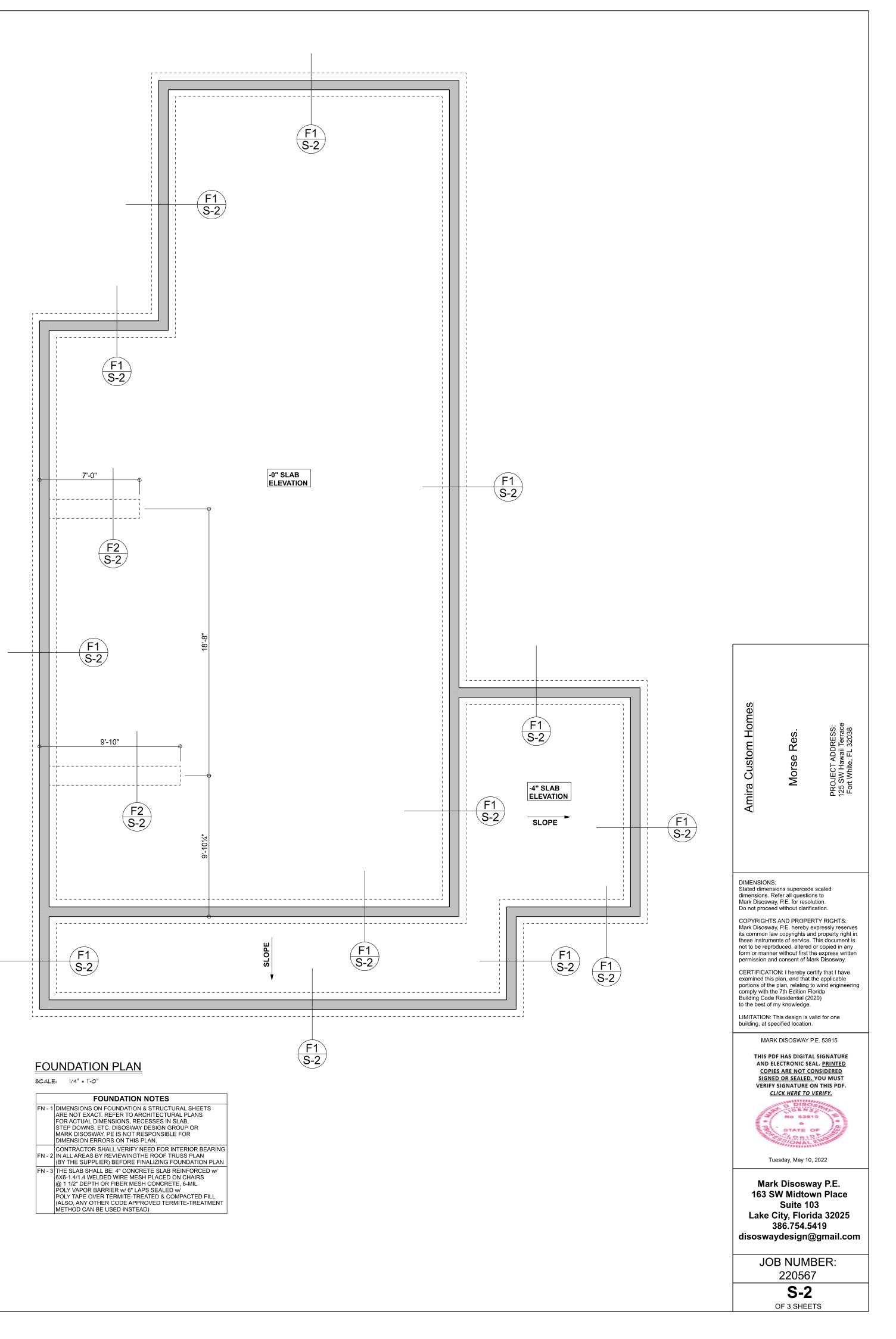
INTERIOR BEARING STEP FOOTING

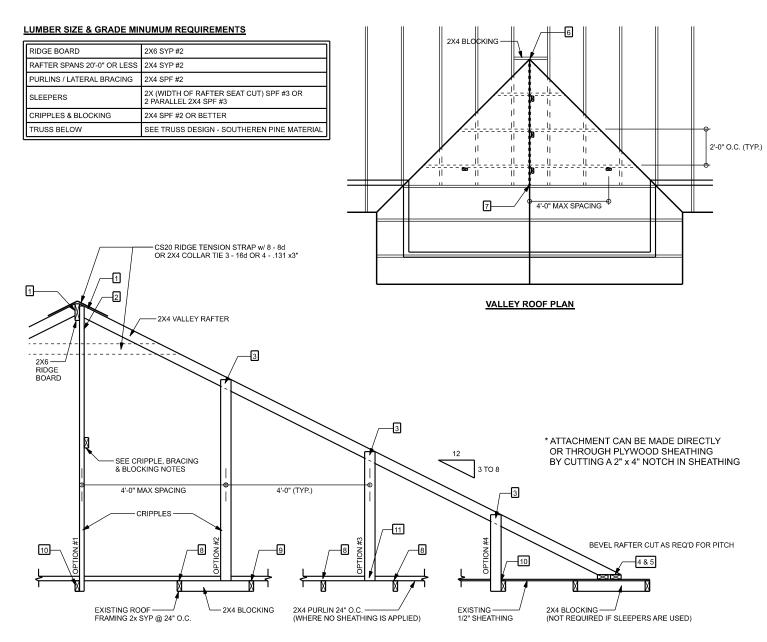
S-2 SCALE: 1/2" = 1'-0"

| TALL STEM WALL TABLE: | | | | | | | | |
|---|--|---------------------|----|----------------------|---------------|----|-----------|--|
| | The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the | | | | | | | |
| footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed | | | | | | | | |
| toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior | | | | | | | | |
| | side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of | | | | | | | |
| | | | | | | | Jai la Oi | |
| the wall 12" CMU may be used with reinforcement as shown in the table below. STEMWALL UNBALANCED VERTICAL REINFORCEMENT VERTICAL REINFORCEMENT | | | | | | | | |
| HEIGHT | BACKFILL | FOR 8" CMU STEMWALL | | FOR 12" CMU STEMWALL | | | | |
| (FEET) | HEIGHT | (INCHES O.C.) | | | (INCHES O.C.) | | | |
| | | #5 | #7 | #8 | #5 | #7 | #8 | |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 | |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 | |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 | |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 | |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 | |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 | |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 | |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 | |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 | |

9.3 9.0 8 16 24 16 40 48

| MASO SHALI FOR M THE C PROC | L CONFORM TO ALL REQU MASONRY STRUCTURES" (ONTRACTOR AND MASON | MATERIALS FOR THIS PROJECT IIREMENTS OF "SPECIFICATION (ACI 530.1/ASCE 6/TMS 602). I MUST IMMEDIATELY, BEFORE SINEER OF ANY CONFLICTS ESE DESIGN DRAWINGS. | | |
|---|---|--|--|--|
| | XCEPTIONS TO ACI 530.1- NGINEER IN WRITING. | 02 MUST BE APPROVED BY | | |
| | ACI530.1-02 Section | Specific Requirements | | |
| 1.4A | Compressive strength | 8" block bearing walls F'm = 1500 psi | | |
| 2.1 | Mortar | ASTM C 270, Type N, UNO | | |
| 2.2 | Grout | ASTM C 476, admixtures require approval | | |
| 2.3 | CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block | | |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" | | |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5) | | |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS | | |
| 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS | | |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. | | |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. | | |





SECTION CUT PARALLEL TO VALLEY RAFTER

VALLEY ROOF PLAN MEMBER LEGEND

TRUSS = = = TRUSS UNDER VALLEY FRAMING

:===:: VALLEY RAFTER OR RIDGE CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

| CON | CONNECTION REQUIREMENT NOTES | | | | | |
|-----|---|--|--|--|--|--|
| 1 | 2X4 RAFTERS TO RIDGE | 3 -16d OR 6131 x 3" TOE NAILS | | | | |
| 2 | CRIPPLE TO RIDGE | 3 - 16d OR 6131 x 3" FACE NAILS | | | | |
| 3 | CRIPPLE TO RAFTERS | 3 - 16d OR 6131 x 3" FACE NAILS | | | | |
| 4 | RAFTER TO SLEEPER OR BLOCKING | 6 -16d OR 12131 x 3" TOE NAILS | | | | |
| 5 | SLEEPER TO TRUSS | 4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS | | | | |
| 6 | RIDGE BOARD TO ROOF BLOCK | 3 -16d OR 6131 x 3" TOE NAILS | | | | |
| 7 | RIDGE BOARD TO TRUSS | 3 -16d OR 6131 x 3" TOE NAILS | | | | |
| 8 | PURLIN TO TRUSS (TYP.) | 3 -16d OR 6131 x 3" NAILS | | | | |
| 8 | PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN) | 4 -16d OR 8131 x 3" NAILS | | | | |
| 9 | TRUSS TO BLOCKING | 3 -16d OR 6131 x 3" END NAILS | | | | |
| 10 | CRIPPLE TO TRUSS | 3 -16d OR 6131 x 3" FACE NAILS | | | | |
| 11 | CRIPPLE TO PURLIN | 3 -16d OR 6131 x 3" FACE NAILS | | | | |

GENERAL NOTES

GENERAL NOTES

MAXIMUM RAFTER SPANS
6-0° FOR 2X4, 9°-0° FOR 2X6 SPF #2 OR SYP #2.

MAXIMUM ROOF AREA PER SUPPORT
16ft2 IN ZONES 2 & 3, 24ft2 IN ZONE 1. (EXAMPLE: 4°-0" O.C. X 4°-0" SPAN = 16ft2 OR 2°-0" X 8°-0" SPAN = 16ft2)

PURLINS REQUIRED 2°-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM.
OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
-SPANS (DISTANCS BETWEEN HEELS) 40°-0" OR LESS
-MAXIMUM WIND SPEED: 130 MPH
-MAXIMUM WIND SPEED: 130 MPH
-MAXIMUM MEAN ROOF HEIGHT: 30 FEET
-MAXIMUM MEAN ROOF HEIGHT: 30 FEET
-MAXIMUM TOTAL LOADING: 40 psf
-MEETS FEC I / ASCE 7:16 WIND REQUIREMENTS
- EXPOSURE CATEGORY 1°C", I = 1.0, Kzt = 1.0
-ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5-0" TO 10'-0" LONG NAILED W 2 - 104 NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB's OR BOTH FACES W "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

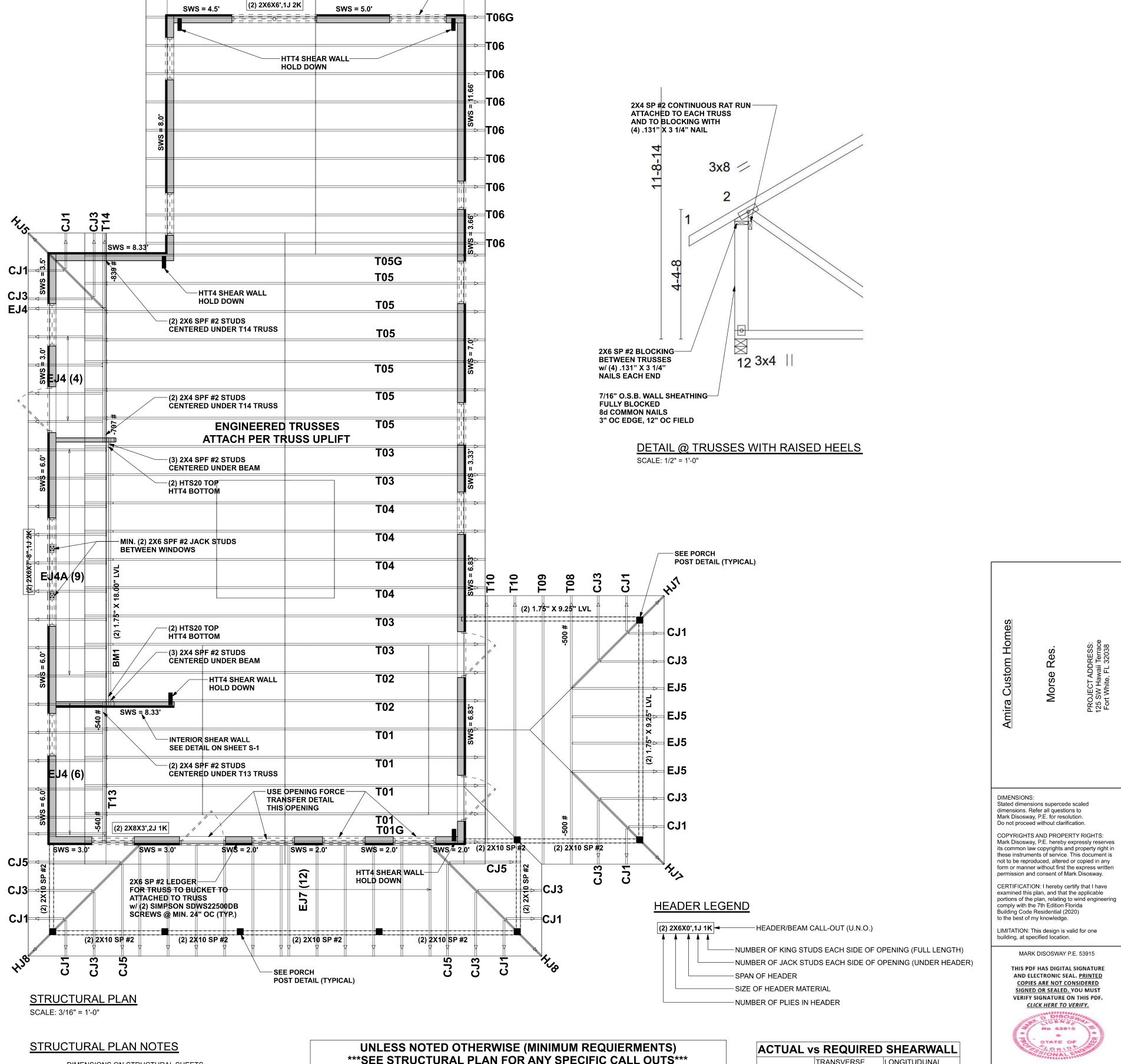
-NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.

-INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

-INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.

-APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

ROOF OVER FRAMING & BRACING DETAIL



USE OPENING FORCE TRANSFER DETAIL THIS OPENING

DIMENSIONS ON STRUCTURAL SHEETS SN-1 ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

| BEAM / HEADERS (SIZE) | ALL LOAD BEARING FRAME WALL & PORCH HEADERS |
|------------------------------|--|
| | SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO) |
| HEADERS (JACK & KING STUDS) | ALL LOAD BEARING FRAME WALL HEADERS |
| | SHALL HAVE (1) JACK STUD & (1) KING STUD |
| | EACH SIDE (UNO) |
| HEADERS (STRAPING) | ALL HEADERS W/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE |
| | WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL |
| | WRAP UNDER BOTTOM PLATE & OVER TOP PLATE |
| | 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER |
| | MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.) |
| JACK STUDS UNDER GIRDER TRUS | SS USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD |

| ACTUAL vs REQUIRED SHEARWALL | | | | | |
|------------------------------|------------|--------------|--|--|--|
| | TRANSVERSE | LONGITUDUNAL | | | |
| ACTUAL | 16064 LBF | 17234 LBF | | | |
| REQUIRED | 15926 LBF | 8603 LBF | | | |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #3153732

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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> JOB NUMBER: 220567

S-3 OF 3 SHEETS