FILE COPY

ELEVATION

SHEET NUMBER

A-2

OF 4 SHEETS

AREA SUMMARY

LIVING AREA GARAGE PORCHES TOTAL AREA

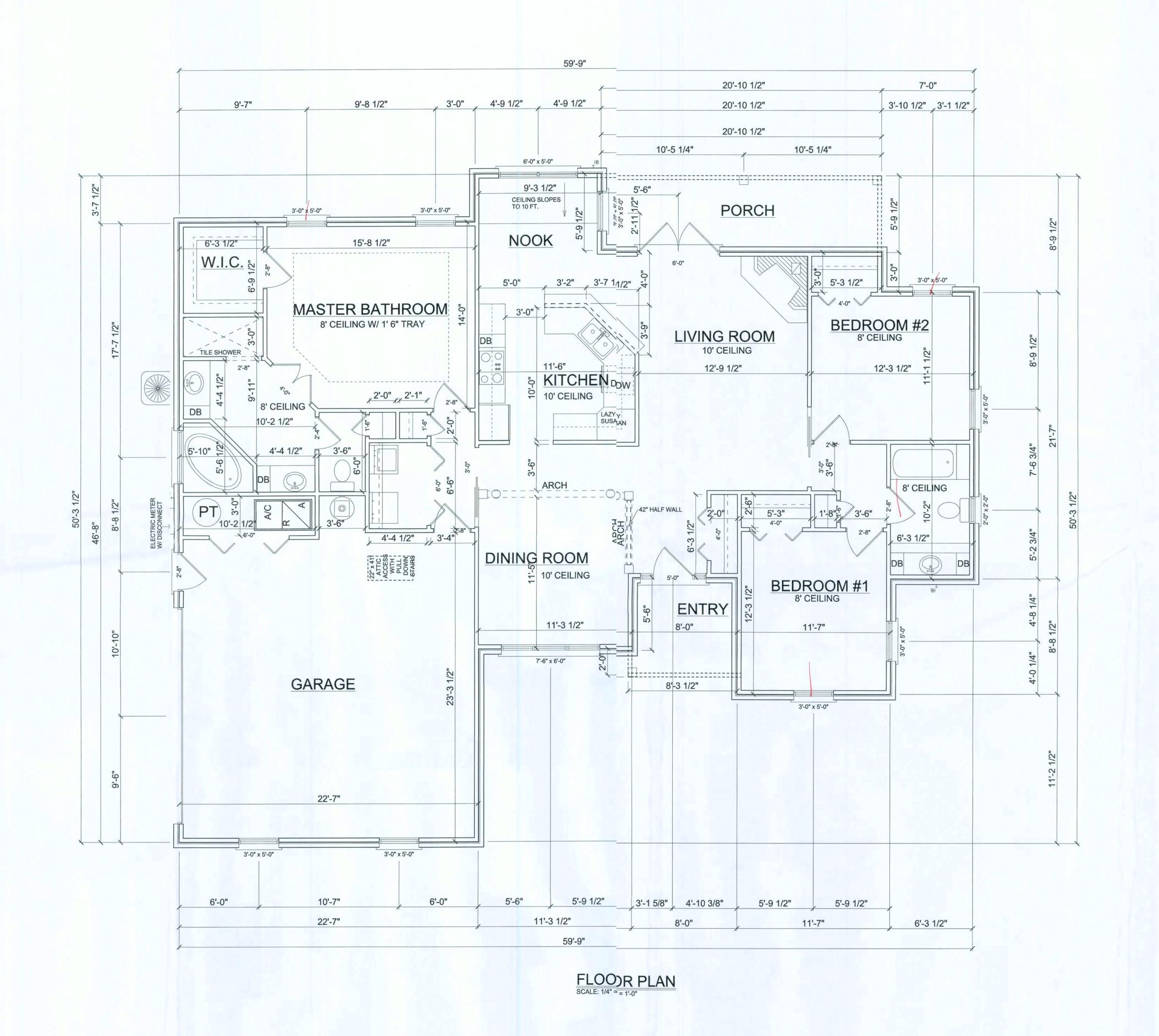
2344 SF

1600 SF

181 SF

563 SF

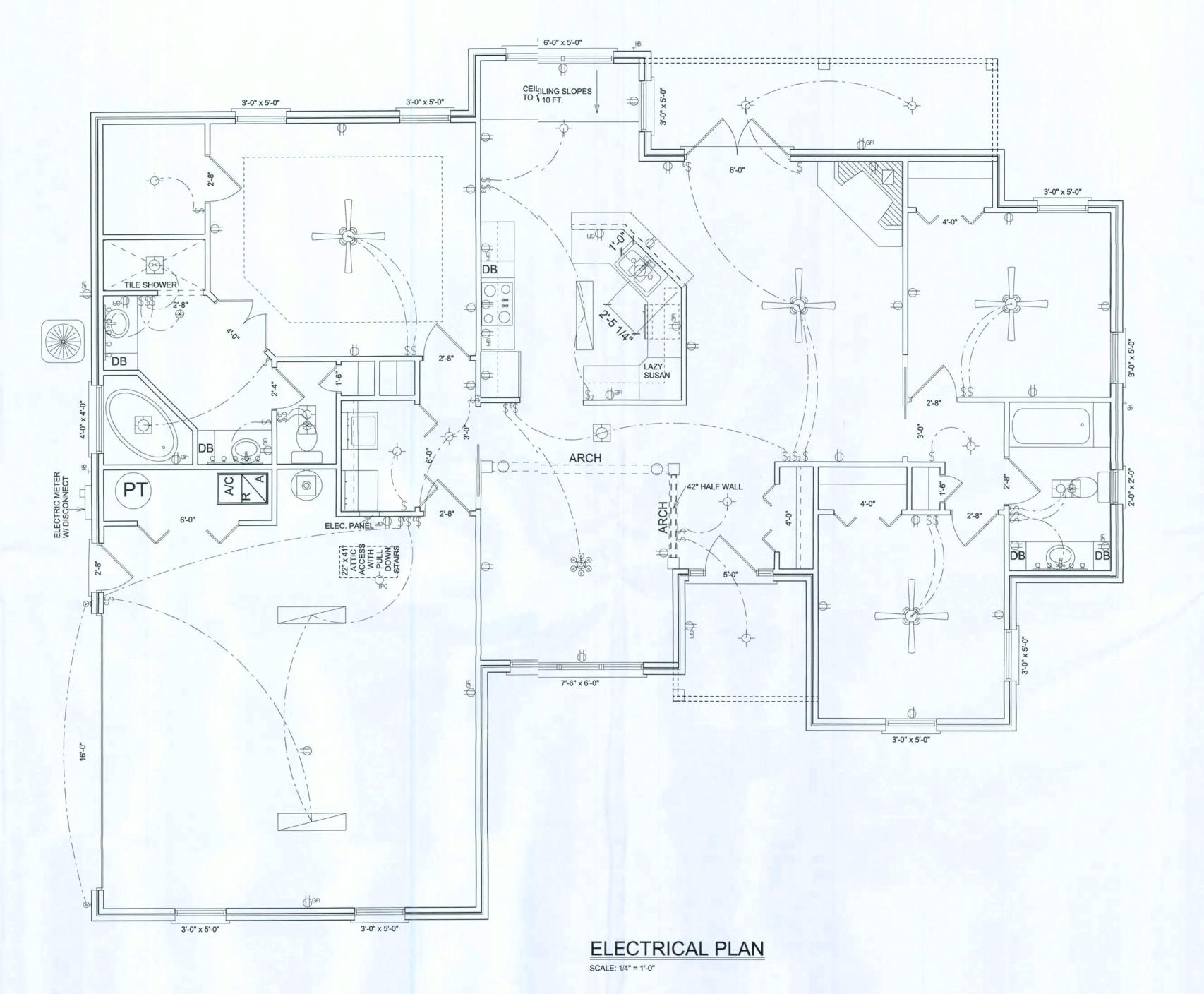
FLOOR PLAN



LIVING AREA **GARAGE**

1600 SF 563 SF 181 SF 2344 SF

ELECTRICAL PLAN



COUNT SYMBOL

3

3

28

16

26

10

99999

-

DGF1

ELECTRICAL

ceiling fan spotlights 1

Can Light

chandelier

fluorescent fixture

vanity bar light

Fan Bathroom

Fan Light - bathroom

wall sconce

light

outlet

outlet 220v

pull chain light

switch 3 way

outlet gfi

switch

ELECTRICAL PLAN NOTES

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFCI CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

#

CHECKED BY:

SHEET NUMBER

A-3

OF4 SHEETS

AREA SUMMARY

PORCHES TOTAL AREA

DATE:
CHECKED BY:

A-4
OF 4SHEETS

SHEE NUMBER

1600 SF 563 SF 181 SF

2344 SF

ROOF PLAN TYPICAL SECTION

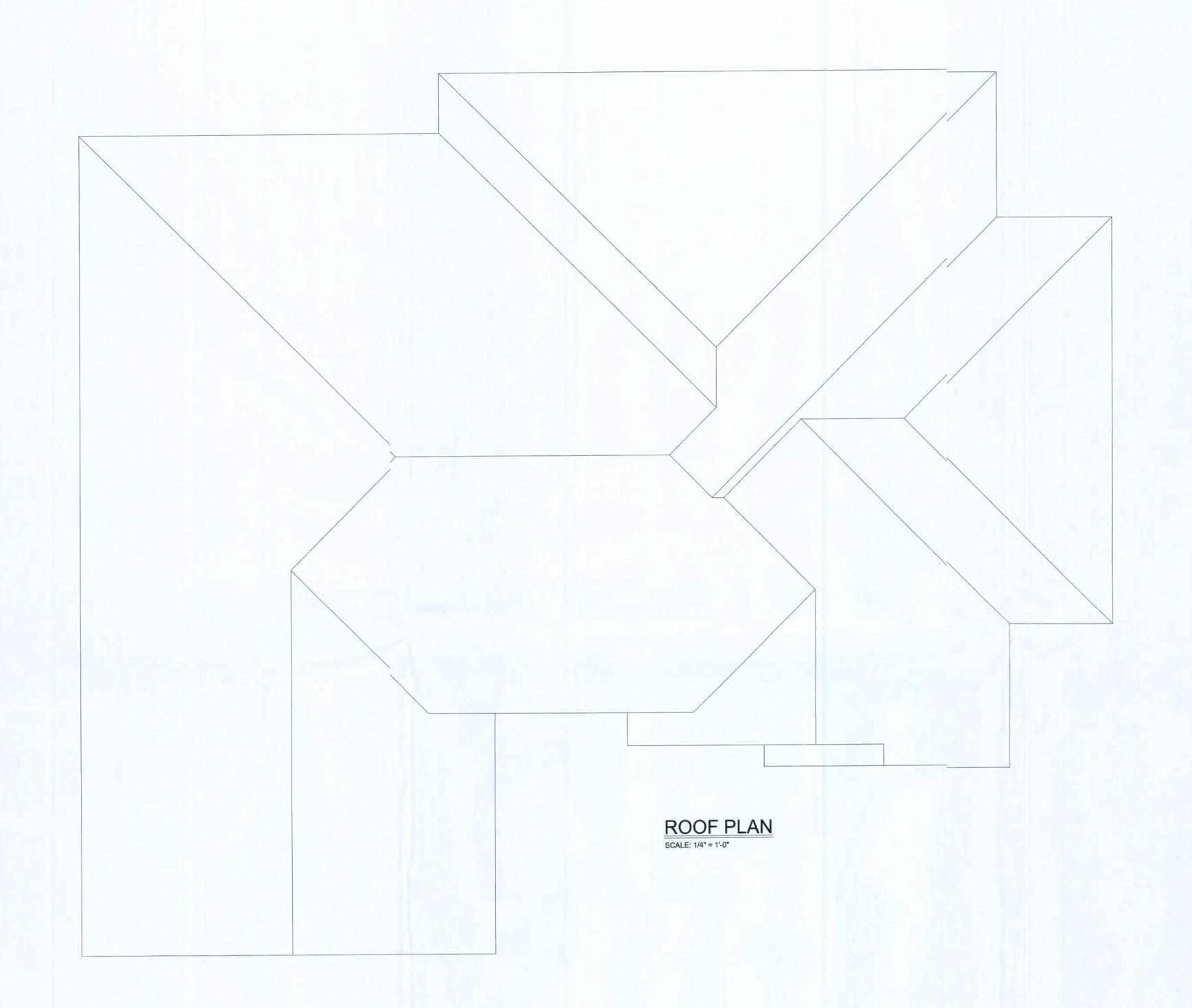
AREA SUMMARY

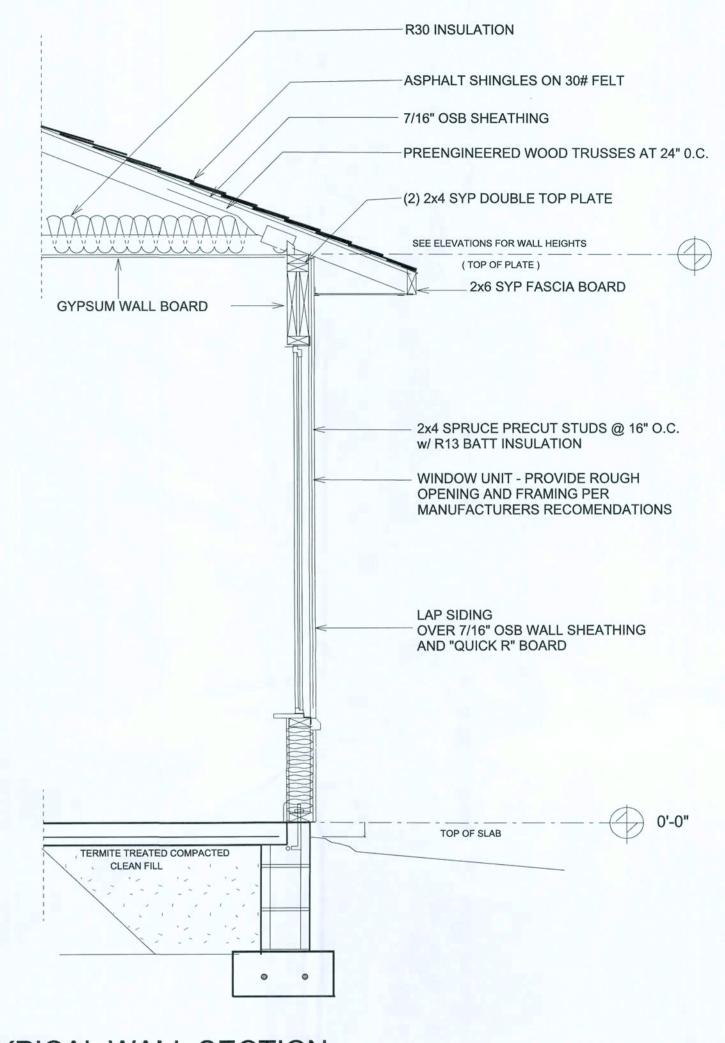
LIVING AREA

TOTAL AREA

GARAGE

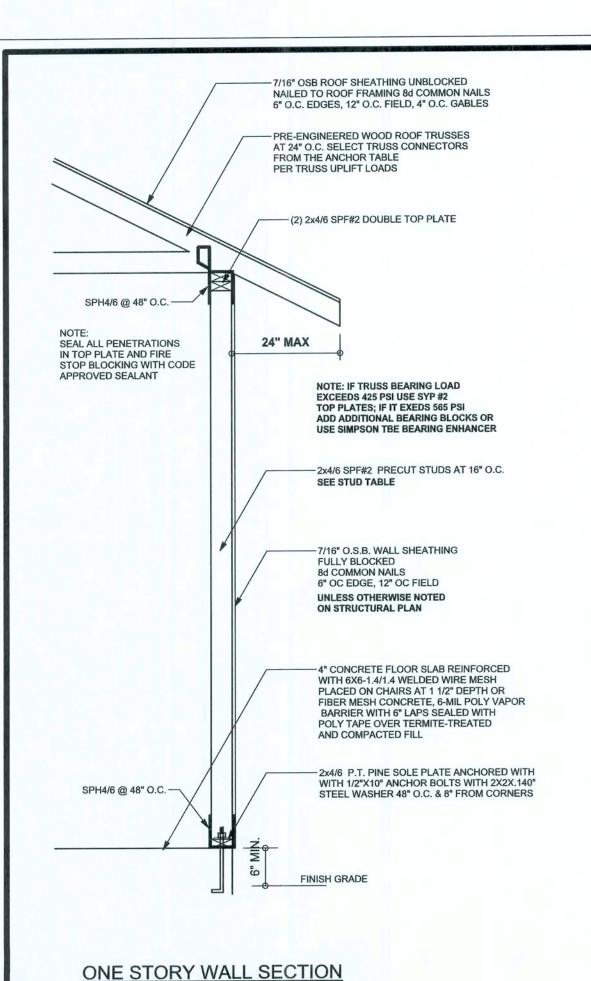
PORCHES





TYPICAL WALL SECTION

2 X 4 STUD WALL



EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

SCALE: 3/4" = 1'-0"

SIMPSON H2.5A U.N.O. -SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

		Ī
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT	
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT	
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT	
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT	

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTH STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

-(2) 2X12 SYP #2 U.N.O.

-6X6 SYP #2 POST

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

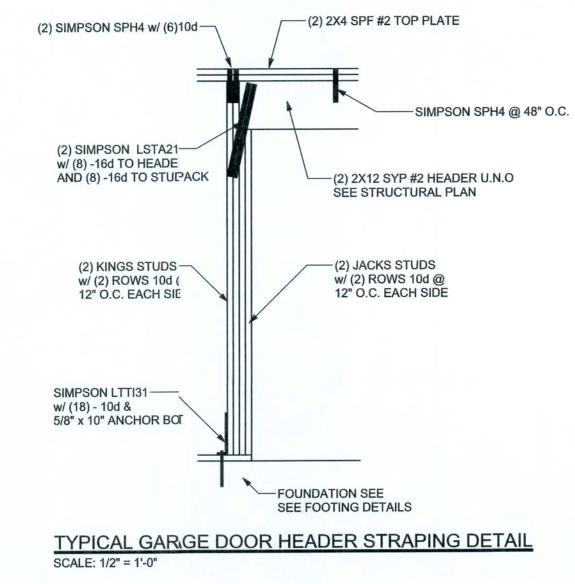
SEE FOOTING DETAILS

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

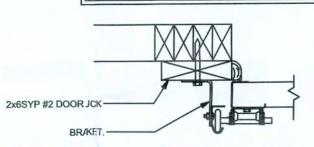
SCALE: 1/2" = 1'-0"

SÉE STRUCTURAL PLAN

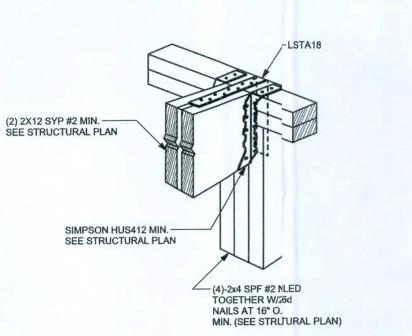


25 SYP #2 GARAGE DOOR BUCK ATTACHMENT AFACH GARAGE DOOR BUCK TO STUD PACK AT EH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SREWS w/ 1" WASHER LAG SCREWS MAY BE CUNTERSUNK. HORIZONTAL JAMBS DO NO TANSFER LOAD. CENTER LAG SCREWS OR SAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

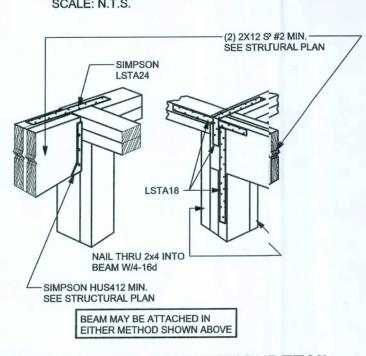
OOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



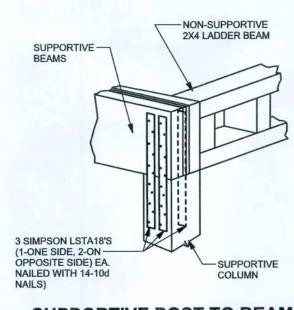
GARAGEDOOR BUCK INSTALLATION DETAIL SCALE: N.T.S.



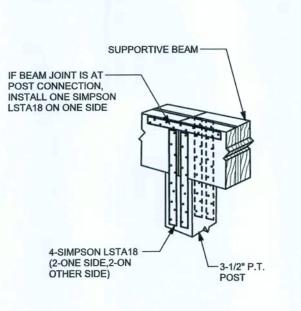
BEAM MID-WALL CONNETION DETAIL



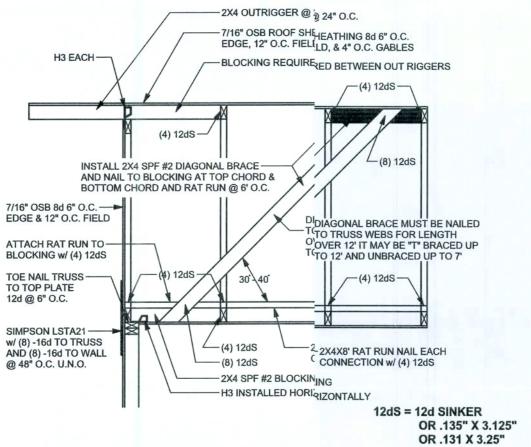
BEAM CORNER CONNECTIONDETAIL SCALE: N.T.S.



SUPPORTIVE POST TO BEAM **DETAIL FOR SINGLE BEAM** SCALE: N.T.S.



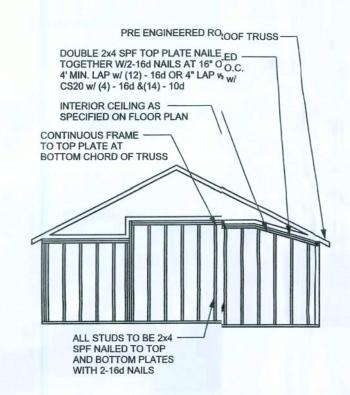
SUPPORTIVE CENTER POST TO BEAM DETAIL



SPACE RAT RUN & DIAGONAL BRARACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 1 110 MPH, EXP. C, ENCLOSED GABLE BRACING DETTAIL

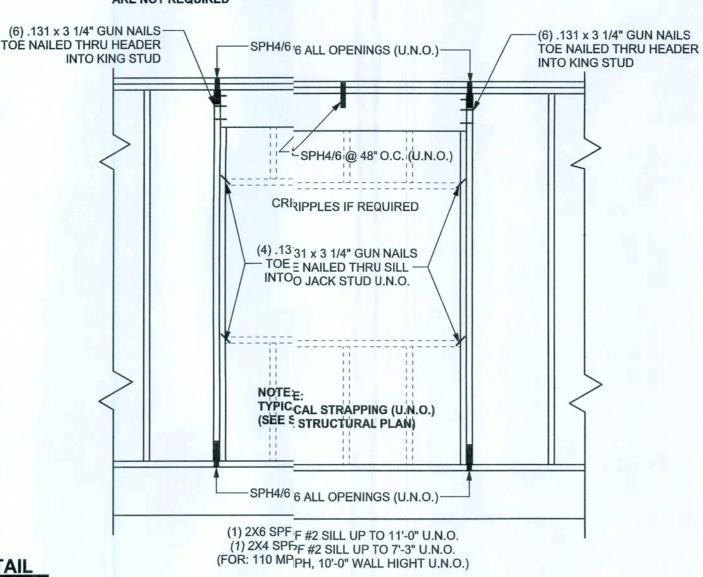
GRADE & SPECIES; TABLE

		Fb (r _(psi)	E (10 ⁶ psi)
2x8	SYP #2	120200	1.6
2x10	SYP #2	105)50	1.6
2x12	SYP #2	97:75	1.6
GLB	24F-V3 SP	240,100	1.8
LSL	TIMBERSTRAND	170,00	1.7
LVL	MICROLAM	160,00	1.9
PSL	PARALAM	290,00	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAAGM DETAIL

IF TRUSS TO WALL STRAPS ARIRE NAILED TO THE HEADER THE SPH4/6 @@ 48" O.C. ARE NOT REQUIRED



TYPICAL HEAADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	ND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	NDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	LS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 OR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLA	NUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU I OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL NGINEER IMMEDIATELY.
DESIGN, PLACEM	S MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS NT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL INS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

ACI530.1-02 Section

1.4A Compressive strength

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

2.4F Coating for corrosion protection

3.3.E.7 | Movement joints

IN WRITING.

2.1 Mortar

2.2 Grout

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

or 304SS

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 476, admixtures require approva

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

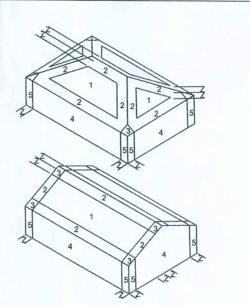
detailed on project drawings.

ANCHOR	TABLE				
OBTAIN UPLIFT RE MANUFACTURER'S	QUIREMENTS FROM ENGINEERING	M TRUSS			
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	=
< 420	< 245	H5A	3-8d	3-8d	_

				TO TOTAL PEROTITIONS	10 01000
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD
< 10980	< 6485	HGT-2		16 -10d	12" EMBEDMENT 2-5/8" THREADED ROD
< 10530	< 9035	HGT-3		16 -10d	12" EMBEDMENT 2-5/8" THREADED ROD
< 9250	< 9250	HGT-4		16 -10d	12" EMBEDMENT 2-5/8" THREADED ROD
	0200	STUD STRAP CONNECTOR*		10 - 100	12" EMBEDMENT
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		TO STUDS
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		4 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4	2-10u		8 -10d
< 1240	< 1065	SPH4			6-10d, 1 1/2"
< 885	< 760	SP6			10-10d, 1 1/2"
< 1240	< 1065	SPH6			6-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 104		10-10d, 1 1/2"
< 1235	< 1235	LSTA16	14-10d		
< 1030	< 1030	W1-2-1-1-1-1	16-10d		
< 1705	< 1705	CS20	18-8d		
1705	1705	CS16	28-8d		
- 12E0	4205	STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320					

DESIGN DATA

77-77-77	D LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



	Zone	Effect	tive Wi	nd Are	ea (ft2)
		1	0		100
	1	19.9	-21.8	18.1	-18.1
	2	19.9	-25.5	18.1	-21.8
	2 O'hg		-40.6		-40.6
	3	19.9	-25.5	18.1	-21.8
	3 O'hg		-68.3		-42.4
	4	21.8	-23.6	18.5	-20.4
	5	21.8	-29.1	18.5	-22.6
		& Wind st Cas 5, 10	е	21.8	-29.1
	8x7 Gar	age D	oor	19.5	-22.9
	16x7 Ga	rage [Door	18.5	-21.0
-					

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12)	
30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)	
10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)	
ROOF 20 PSF (FLAT OR <4:12)	
16 PSF (4:12 TO <12:12)	
101 01 (4.12 10 12.12)	
12 PSF (12:12 AND GREATER)	
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	

SOFTPLAN

REVISIONS

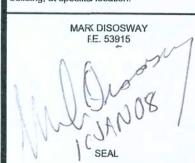
/INDLOAD ENGIFEER: Mark Disosway, PE No.53915, POE868, Lake City, FL 32056, 386-754-549 IMENSIONS Stated dimensions upercede scaled

dimensions. Refer II questions to Mark Disosway, P.i. for resolution.

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CERTIFICATION: hereby certify that I have camined this plan and that the applicable ortions of the plan relating to wind enginee comply with sectior R301.2.1, florida building code residential 204, to the best of my

LIMITATION: This esign is valid for one building, at specifiel location.



Stanley Crawford Corstruction

Spec House Lot 7 Mayfair S/D

ADDRESS: Lot 7Mayfair S/D Columbi County, Florida

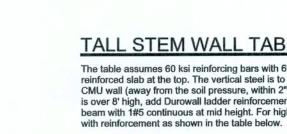
Mark Dsosway P.E. P.O Box 868 Lake City Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRIITED DATE: Januay 10, 2008 STRUCTURAL BY

FINALS DATE: 09 / Jan / 08

> JOB NUMBER: 801093 DRAWNG NUMBER

> > **S-1** OF3 SHEETS



-#5 STEEL DOWEL WITH 24" HOOK BENT INTO SLAB AND 6" HOOK IN FOOTING AT EACH CORNER AND AT 96" O.C. 6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" — DEPTH ON CHAIRS OR FIBERMESH -8X8X16, RUNNING BOND, CMU STEM WALL, MIN 2, 6 MIL VAPOR BARRIER ---MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MOR THAN 5 COURSES) TO MIN. 95% MOD. PROCTOR -(2) #5 REBAR CONTINUOUS CONCRETE STRIP FOOTING

SEE WALL SECTION & STRUCTURAL PLAN FOR CAST IN PLACE ANCHORS

(MINIMUM 3000-PSI AT 28 DAYS)

-(1) #5 CONT., IN HDR. BLOCK BOND BEAM @

SLAB EDGE INTERSECTION W/ STEMWALL

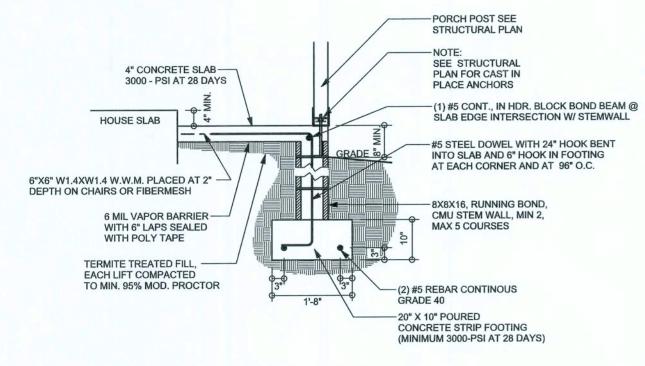
STEM WALL FOOTING SCALE: 1/2" = 1'-0"

RECESS AT DOORS -

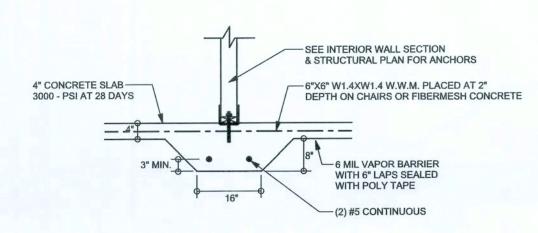
4" CONCRETE SLAB —— 3000 - PSI AT 28 DAYS

AS REQUIRED

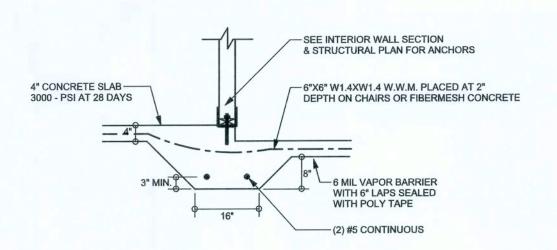
WITH 6" LAPS SEALED WITH POLY TAPE TERMITE TREATED FILL. EACH LIFT COMPACTED



ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"

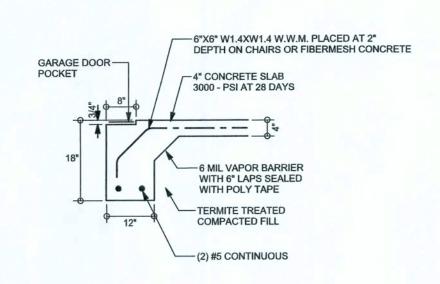


F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"

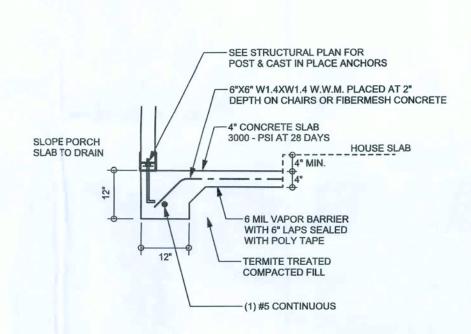
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR	AL REINFOR 3" CMU STEI (INCHES O.C	MWALL	FOR 1	AL REINFOR 2" CMU STEI INCHES O.C	MWALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

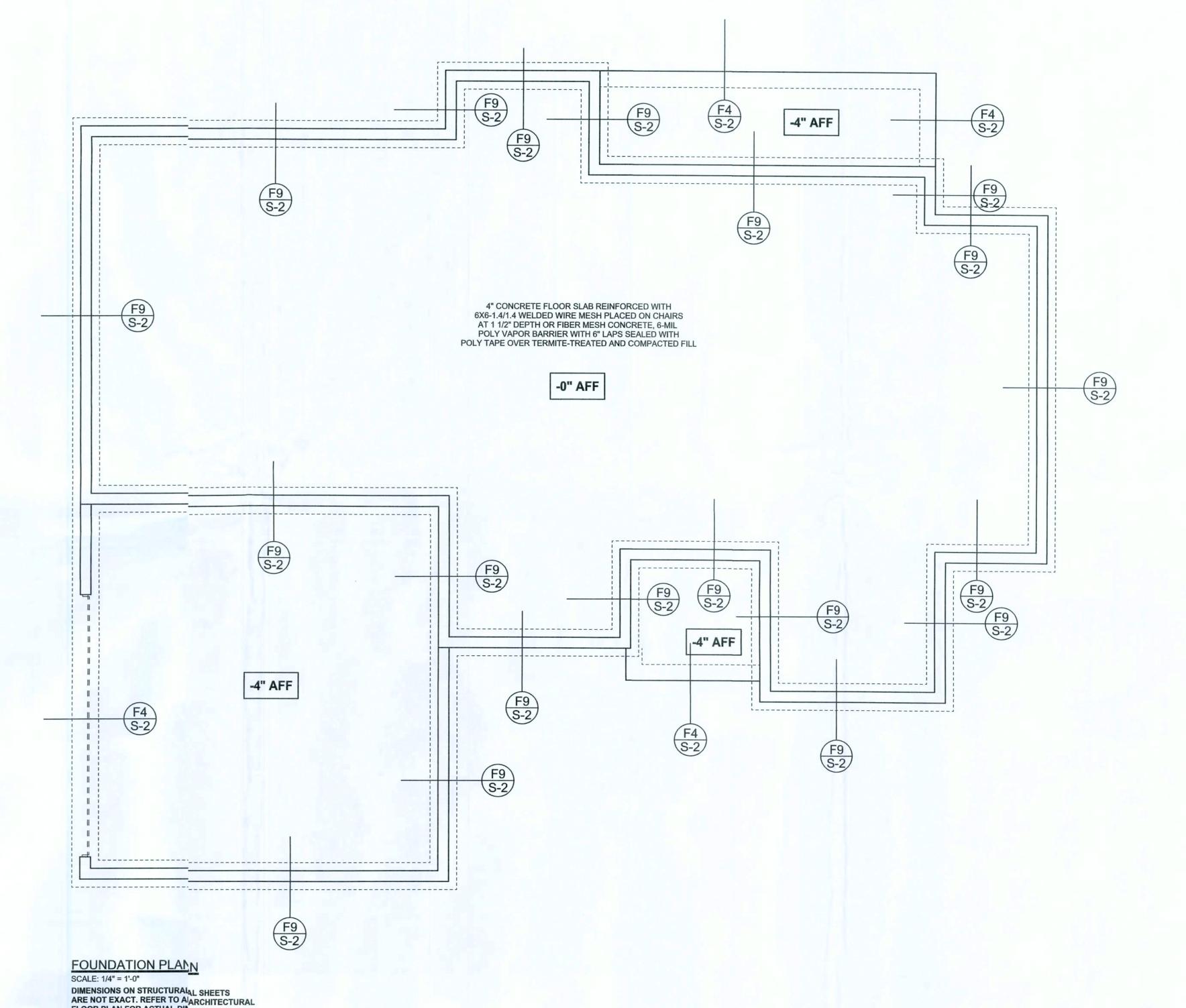


F4 GARAGE DOOR FOOTING SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

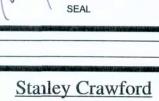
FLOOR PLAN FOR ACTUAL DINIMENSIONS



SOFTPIAN

REVSIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915,POB 868, Lake City, FL 32056, 386-75-5419 DIMENSIONS Stated dimensons supercede scaled dimensions. Refer all questions to Mark Disoswa, P.E. for resolution. Do not proceel without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoswa, P.E. hereby expressly reserves its common lav copyrights and property right in these instruments of service. This document is not to be reprduced, altered or copied in any form or manne without first the express writte permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined thisolan, and that the applicable portions of theplan, relating to wind engineerin comply with section R301.2.1, florida building code residentil 2004, to the best of my LIMITATION: 'his design is valid for one building, at spicified location. MARK DISOSWAY P.E. 53915



Spec House Lot 7 Mayfair S/D

Construction

ADDRESS: Lot 7 Mayfair S/D Colunbia County, Florida

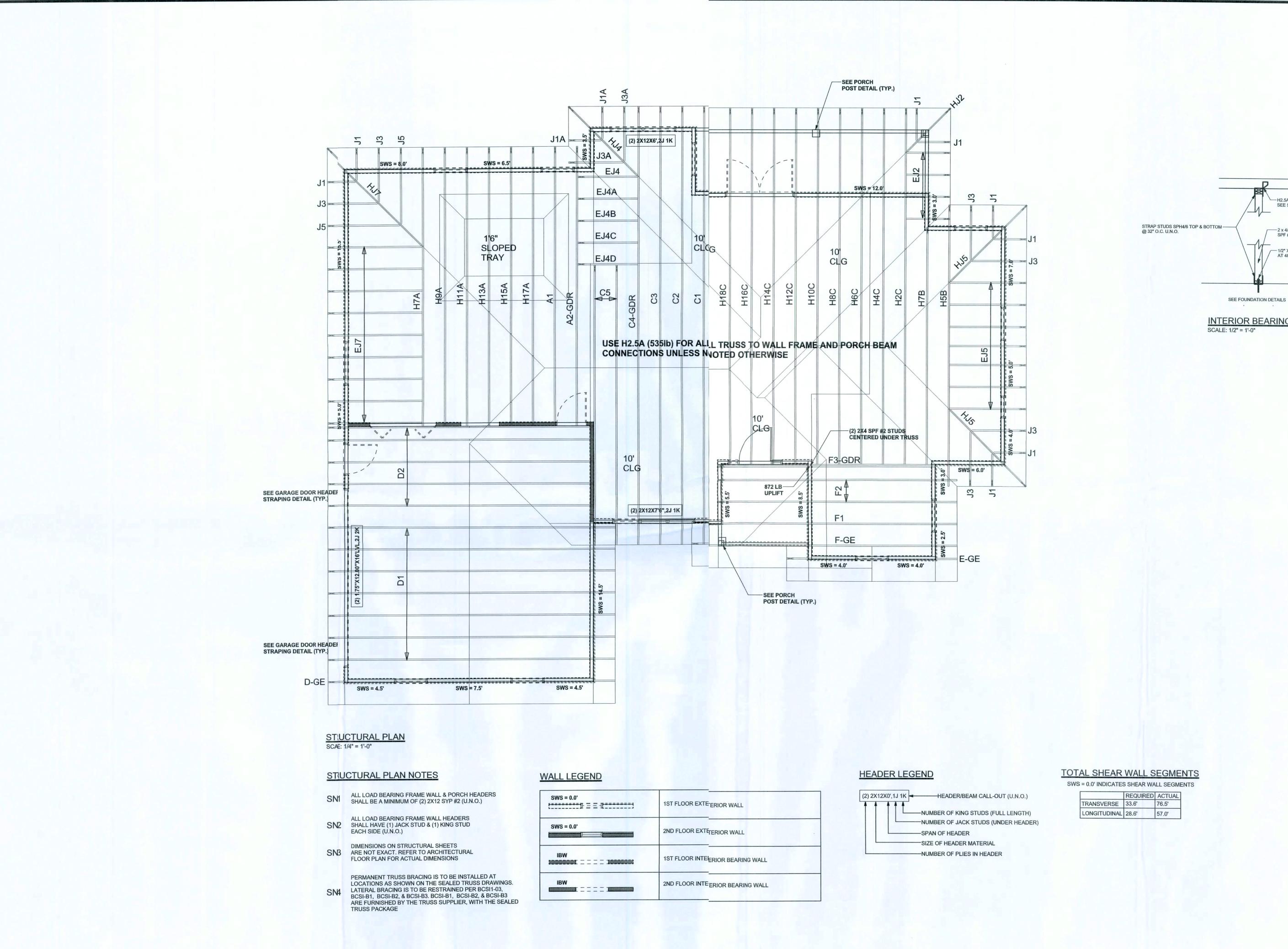
Marł Disosway P.E. F.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: 386) 269 - 4871

> PRINTED DATE: Jauary 10, 2008 STRUCTURAL BY:

FINALS DATE: 09 / Jan /08

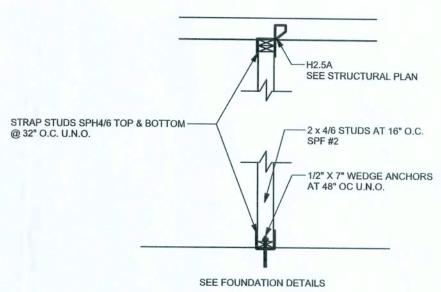
> JO3 NUMBER: 801093 DRAWING NUMBER

> > **S-2** OF 3 SHEETS



REVISIONS

SOFTPIAN



INTERIOR BEARING WALL

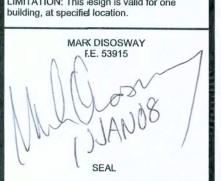
WINDLOAD ENGIFEER: Mark Disosway, PE No.53915, POE868, Lake City, FL 32056, 386-754-549

DIMENSIONS: dimensions. Refer II questions to Mark Disosway, P.I. for resolution. Do not proceed wittout clarification.

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CERTIFICATION: Ihereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with sectior R301.2.1, florida building code residential 20/4, to the best of my knowledge.

LIMITATION: This esign is valid for one



Stanley Crawford Construction

Spec House Lot 7 Mayfair S/D

ADDRESS: Lot 7Mayfair S/D Columbia County, Florida

Mark Dsosway P.E. P.O Box 868 Lake City Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> January 10, 2008 STRUCTURAL BY

> > **S-3**

OF3 SHEETS

FINALS DATE: 09 / Jan / 08

JOB NUMBER: 801093 DRAWNG NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #8-007