

DESIGN SPECIFICATIONS

DESIGN CODE:
2023 FLORIDA BUILDING CODE – RESIDENTIAL
THIS DESIGN IS VOID ONE YEAR AFTER THE DATE OF THE ORIGINAL PLANS, UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.

DESIGN LOADS: ACTUAL AND UNIFORM

ROOF:
(cd=1.25)
20 psf (ARCH SHINGLES)
10 psf (TILE SHINGLES)
5 psf

FLOOR:
(cd=1.00)
40 psf
10 psf
10 psf
5 psf

ROOF CHORD LIVE LOAD
TOP CHORD DEAD LOAD
TOP CHORD DEAD LOAD
BOTTOM CHORD LIVE LOAD
BOTTOM CHORD DEAD LOAD

DEFLECTION CRITERIA:
ROOF FRAMING: LIVE LOAD L/240 TOTAL LOAD L/180
FLOOR FRAMING: LIVE LOAD L/360 & TOTAL LOAD L/240
0.75" MAX ANY CASE

WIND LOADING:
ASCE 7/22 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD, AND 5 PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED, SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED w/ DEAD LOAD.

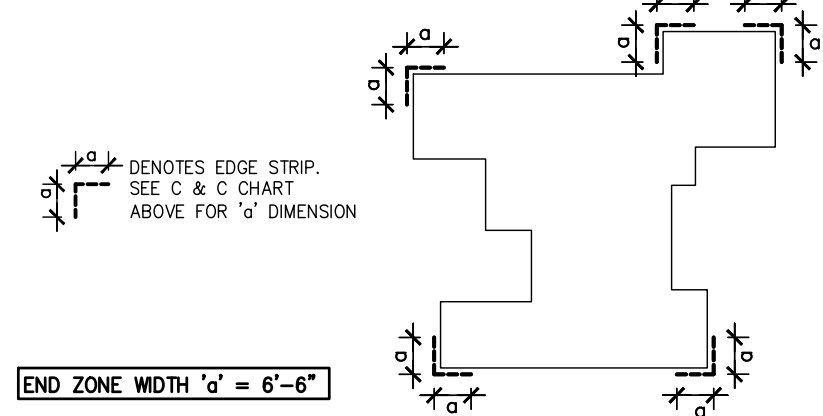
BASIC WIND SPEED (ASCE 7-22) **130 MPH**
IMPORTANCE FACTOR **1.00**
MEAN ROOF HEIGHT **20.0 FT**
ROOF PITCH **8/12**
BUILDING CATEGORY **C**
EXPOSURE CATEGORY **C**
ENCLOSURE CLASSIFICATION **C**
INTERNAL PRESSURE COEFFICIENT **± .18**

MATERIAL SPECIFICATIONS

HARDWARE AND ANCHORS:
ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36.
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX.
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.
REINFORCING REBAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR DOWELS SHALL BE 12 BAR DIAMETER MINIMUM. HOLES SHALL BE 1/4" LARGER THAN REBAR SIX AND 1/2" LARGER THAN THREADED ROD SIZE (0.004).
ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS (DUAL CARTRIDGE INSTALLATION ONLY):
EPOXY: ITW RED HED A7
REINFORCING STEEL SHALL BE ASTM A615, GRADE 60.
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.
WELDED WIRE FABRIC (WWF): SHALL BE ASTM A185.
LUMINATED VENEER LUMBER (LVL): ALL LUMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E), 900ksi, BENDING STRESS (Fb) 2600psi

TRIBUTARY AREA (sf)	COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES		
	INTERIOR ZONE (PSF)	EDGE STRIP (PSF)	GARAGE DOOR PRESSURES (PSF)
10	+25.5 -27.7	+25.5 -34.2	1 CAR GARAGE DOOR (8'x7')
50	+22.9 -25.0	+22.9 -28.8	2 CAR GARAGE DOOR (16'x7')
100	+21.8 -23.2	+21.8 -26.5	

- THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE WIND PRESSURES HAVE BEEN REDUCED BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.
- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



SCOPE OF SERVICE

MEANS AND METHODS:
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED: PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

FLOOR SHEATHING SPECIFICATIONS:
23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD

ROOF SHEATHING SPECIFICATIONS:
SHINGLE – MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.131x2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

TILE – MIN. 15/32", 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

METAL – MIN. 1/2", 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" WITHIN 4'-0" OF ROOF EDGE).

WALL SHEATHING SPECIFICATIONS:
FLEXIBLE FINISH – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/4" STRUCTURAL 1 GRADE SHEATHING OR 5/8" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:
MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/4" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LISTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEMWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, F GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI REA ROOF CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REIN. SHALL BE INCREASED AS NOTED ON 1/51.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:
FOOTING AND FOUNDATIONS:
FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

CONCRETE SLABS ON GRADE:
SLAB SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL. SOIL TREATMENT FOR PREVENTION OF SUBTERANEAN TERMITES. SAWCUTS FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGHOUT SLAB CALL FOR ALTERNATIVE METHODS.

WOOD FRAMING SPECIFICATIONS:
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED WITH A 200 OR 400 DOT BORATE PRESERVATIVE TREATMENT IS USED. ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF ALCCA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AISC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS BRACING PER "COMMENTARY" AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

ROOF COVERING SPECIFICATIONS:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGLES SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE

MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONT. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

BRICK NOTES / LINTEL SCHD

LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
L3 1/2"x3 3/4"x 1/4"	4"	6'-0"
L4 3/4"x3 3/4"x 1/4"	6"	8'-0"
L5 3/4"x3 3/4"x 1/4"	6"	10'-0"
L6 3/4"x3 3/4"x 1/4"	6"	12'-0"
L7 3/4"x3 3/4"x 1/4"	6"	16'-0"

1. STEEL LINTELS TO BE MINIMAL 36" LINTEL MUST HAVE CORROSION RESISTANT COATING OF EPOXY BASED PAINT.

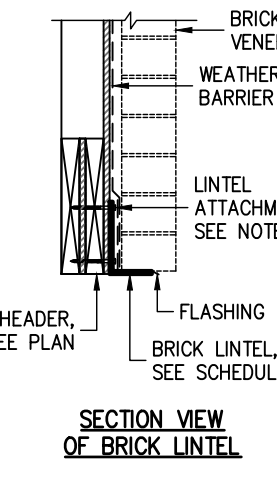
2. LINTEL MORE THAN 8'-0". SHOULD BE LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. w/ 2-1/4"x3" WD. SCREWS INTO HEADER PROVIDE A 1/2" VERTICAL SLOTTED HOLE FOR SCREW.

3. BRICK VENEER ATTACHMENT: HORIZONTAL TIES @ 24" O.C., VERT. TIES @ 12" O.C. (FOR 110mph WIND-ZONE VERT. TIES @ 16" O.C.) AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENING. PROVIDE 3/4" WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.

PLAN LEGEND AND ABBREVIATIONS

INTERIOR LOAD BEARING WALL	BUILT-UP POST IN THE WALL
GABLE X-BRACE, SEE DETAIL 10/20.1	HEADER SIZE, JACK AND KING STUD QUANTITY.
DESIGNATES SHEARWALL. THE HIDDEN LINE DESIGNATES SIZE OF WALL. THE SHEARWALL SHEATHING TO BE APPLIED. 8d @ 3/8" DESIGNATES 8d COMMONS @ 3" O.C. EDGE & 6" O.C. "IN THE FIELD"	
ADJ. - ADJACENT BM - BEAM BOT - BOTTOM BRG - BEARING CMU - CONCRETE MASONRY UNIT DBL - DOUBLE DIA - DIAMETER EA - EACH EE - EACH END EOR - ENGINEER OF RECORD EQ - EQUAL EXT - EXTERIOR FBC - FLORIDA BUILDING CODE FDM - FOUNDATION FT - FOOT THR - THROUGH HDR - HEADER HORIZ - HORIZONTAL LES - POUNDS	LG - Long MANUF - Manufacture MONO - Monolithic OC - On Center OSB - Oriented Strand Board PERP - Perpendicular PRE-ENG - Pre Engineered QTL - Quick Tie REINF - Reinforce SF - Square Foot SPF - Spruce Pine Fir SYP - Southern Yellow Pine THRU - Through TYP - Typical UON - Unless Otherwise Noted VERT - Vertical WWF - Welded Wire Fabric

(2) 2x8-1/2"



BRICK VENEER

WEATHER BARRIER

LINTEL ATTACHMENT

SEE NOTE 2

FLASHING

BRICK LINTEL

SECTION VIEW OF BRICK LINTEL

HEADER, BRICK LINTEL, SEE SCHEDULE

FLASHING

BRICK LINTEL

HEADER, BRICK LINTEL, SEE SCHEDULE

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BRICK LINTEL

SECTION VIEW OF BRICK LINTEL

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SECTION VIEW OF BRICK LINTEL

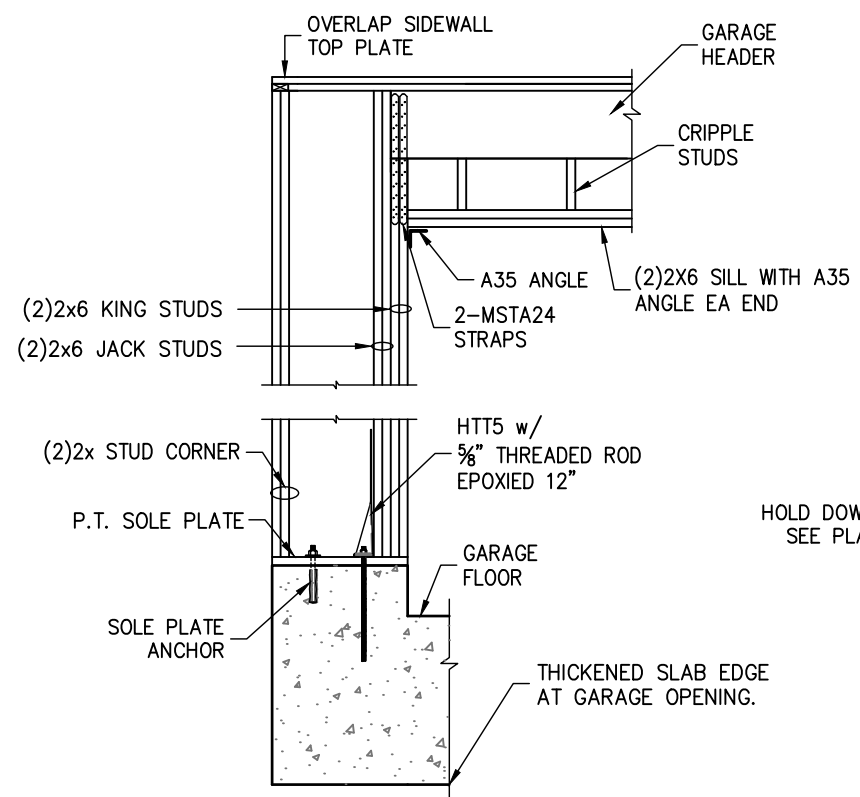
HEADER, BRICK LINTEL, SEE SCHEDULE

FLASHING

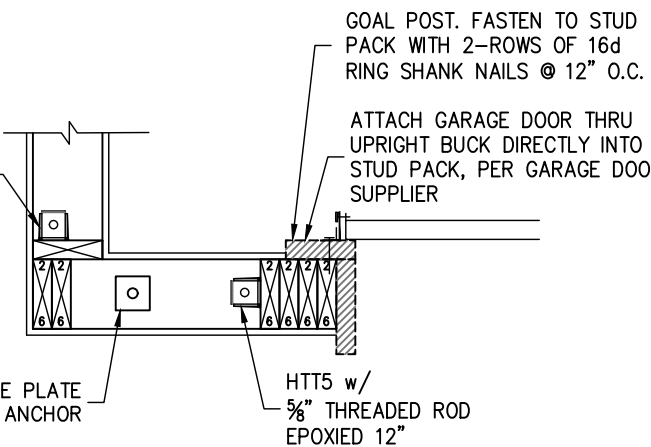
BRICK LINTEL

SECTION VIEW OF BRICK LINTEL

HEADER, BRICK LINTEL, SEE SCHEDULE

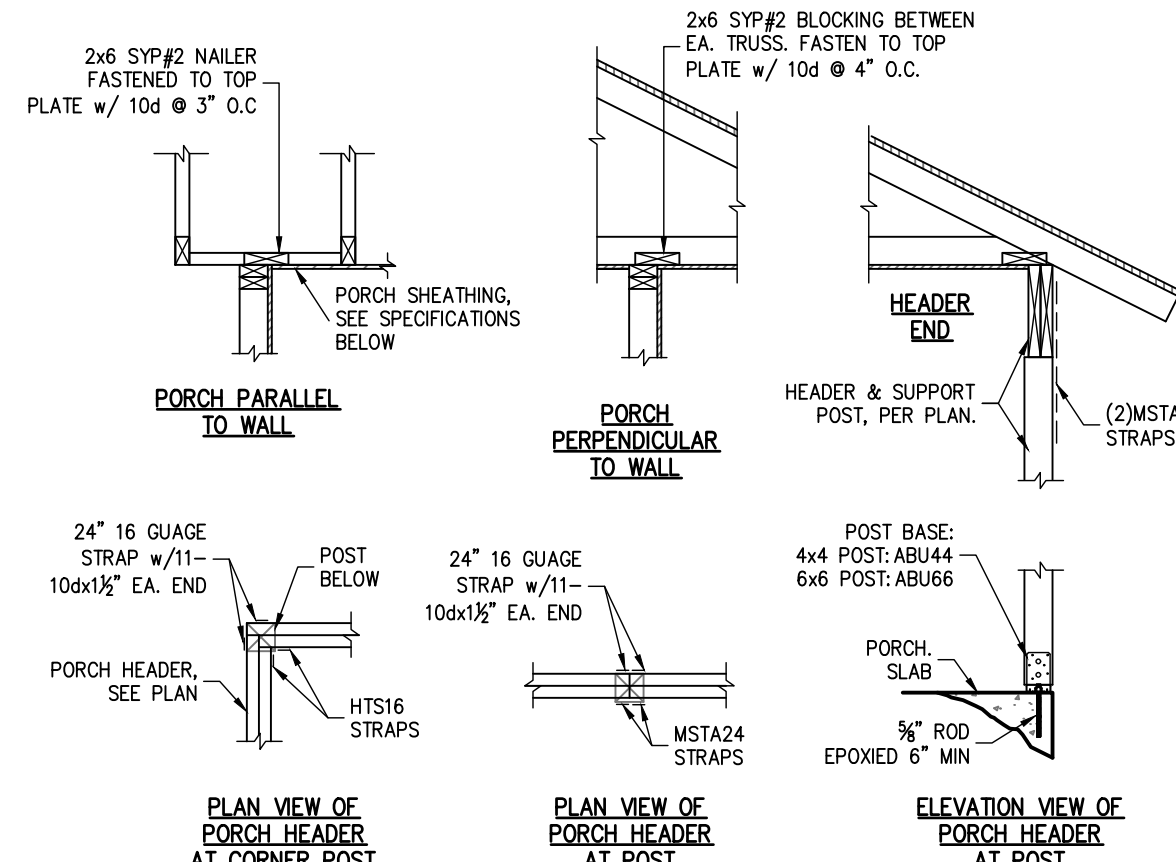


GARAGE WING WALL ELEVATION

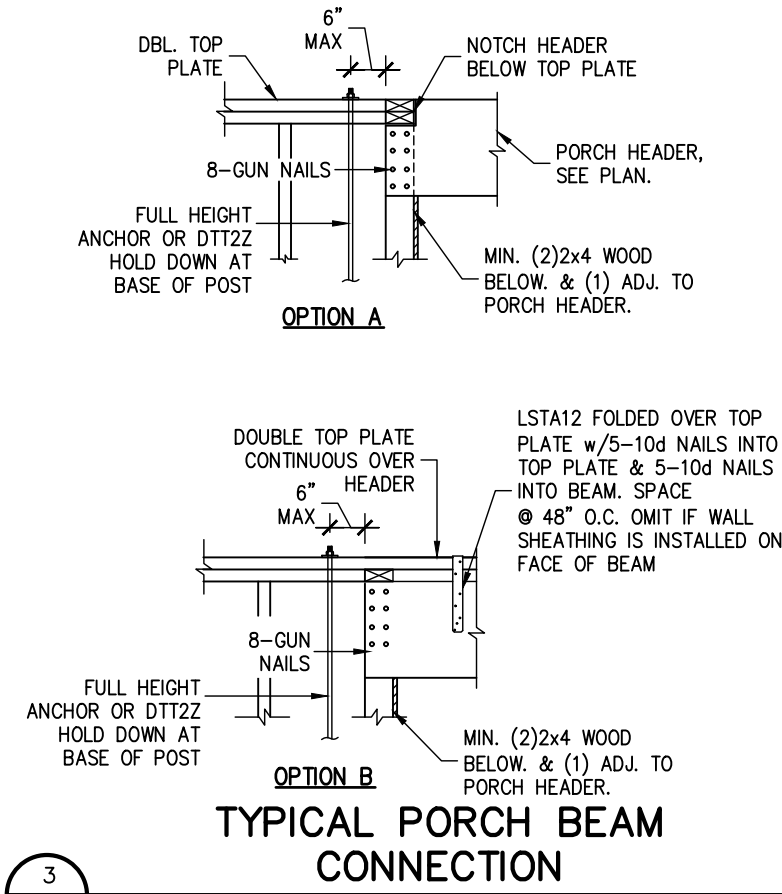


GARAGE WING WALL SECTION

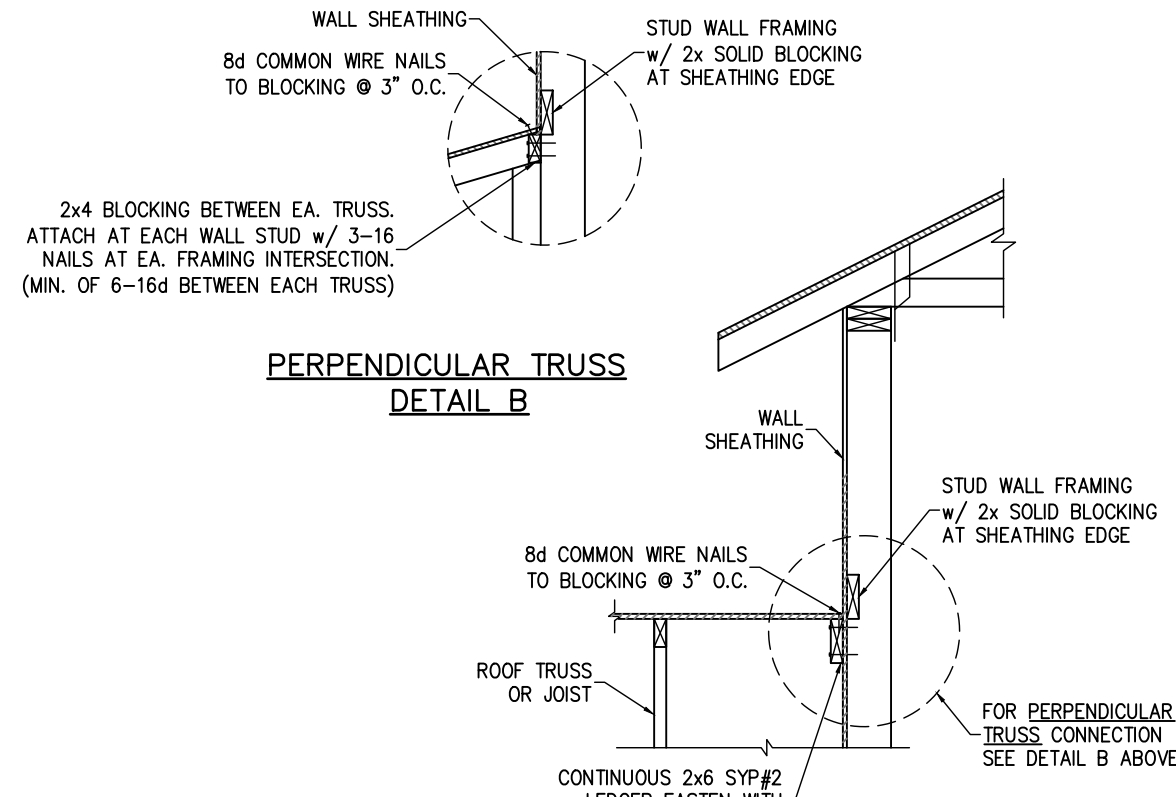
1 GARAGE HEADER FRAMING
SCALE: N.T.S.



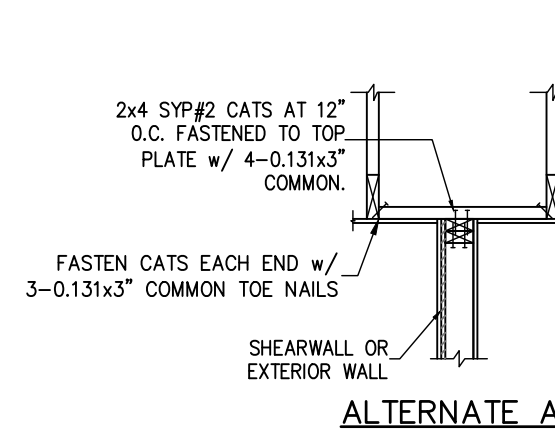
2 TYPICAL PORCH FRAMING DETAILS
SCALE: N.T.S.



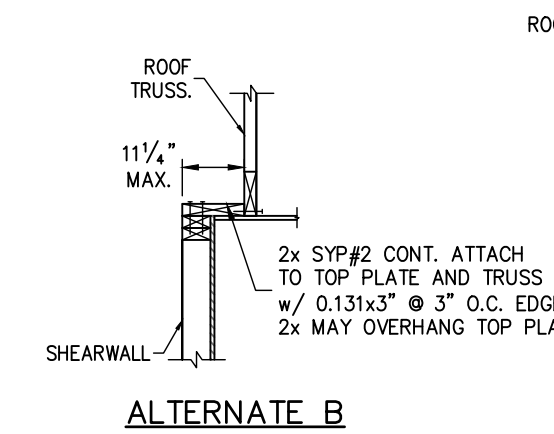
3 TYPICAL PORCH BEAM CONNECTION
SCALE: N.T.S.



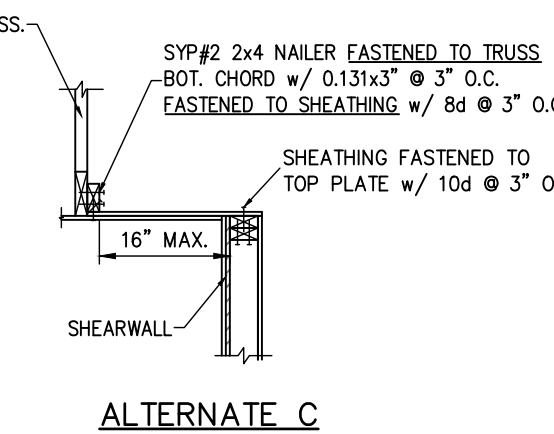
4 WALL ADJ. TO ROOF CONNECTION
SCALE: N.T.S.



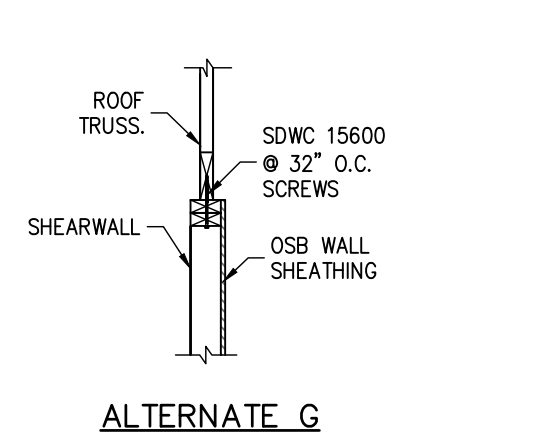
ALTERNATE A



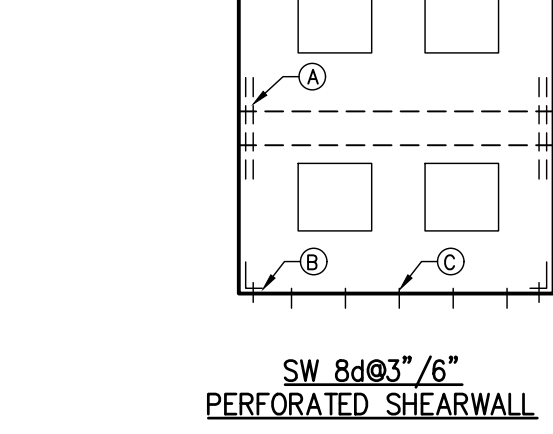
ALTERNATE B



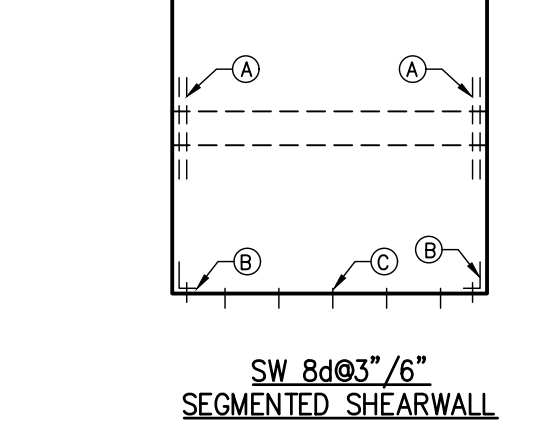
ALTERNATE C



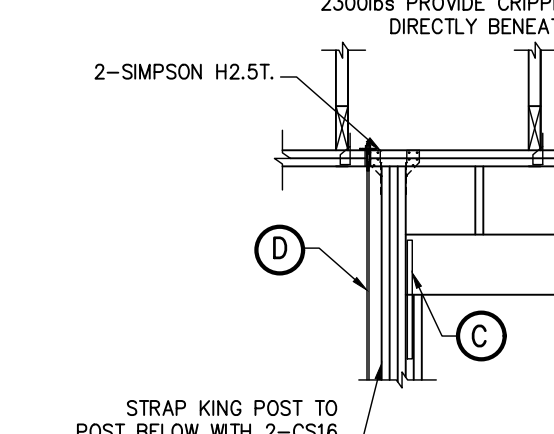
ALTERNATE D



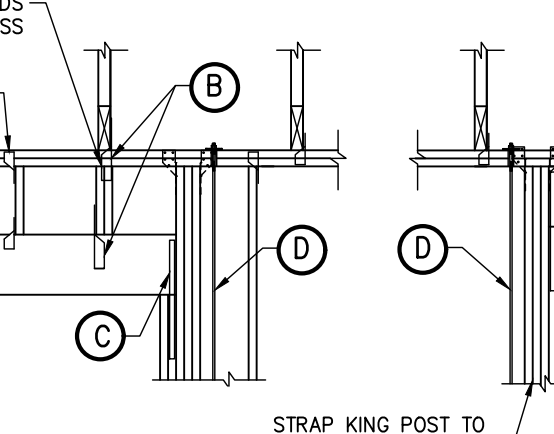
ALTERNATE E



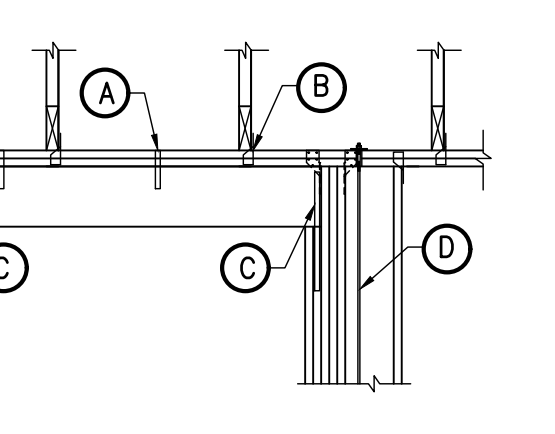
ALTERNATE F



ALTERNATE G

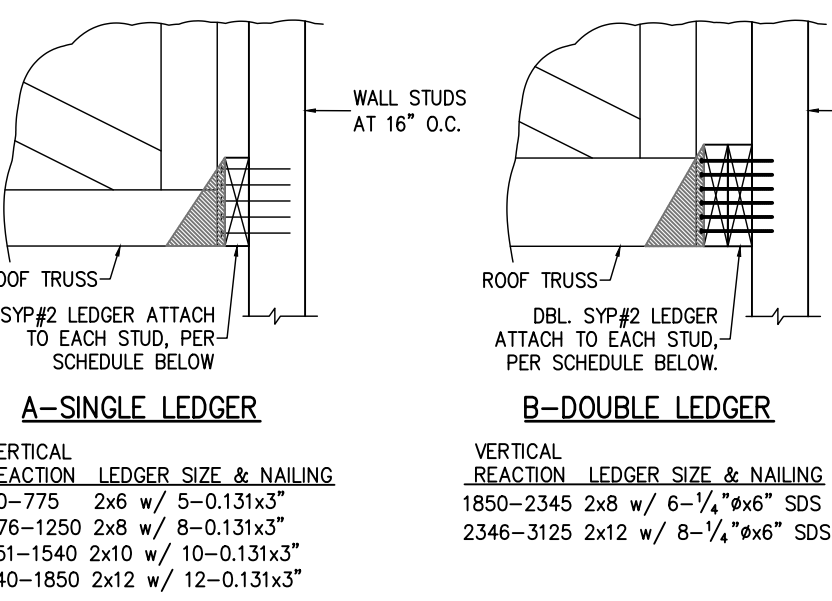


ALTERNATE H

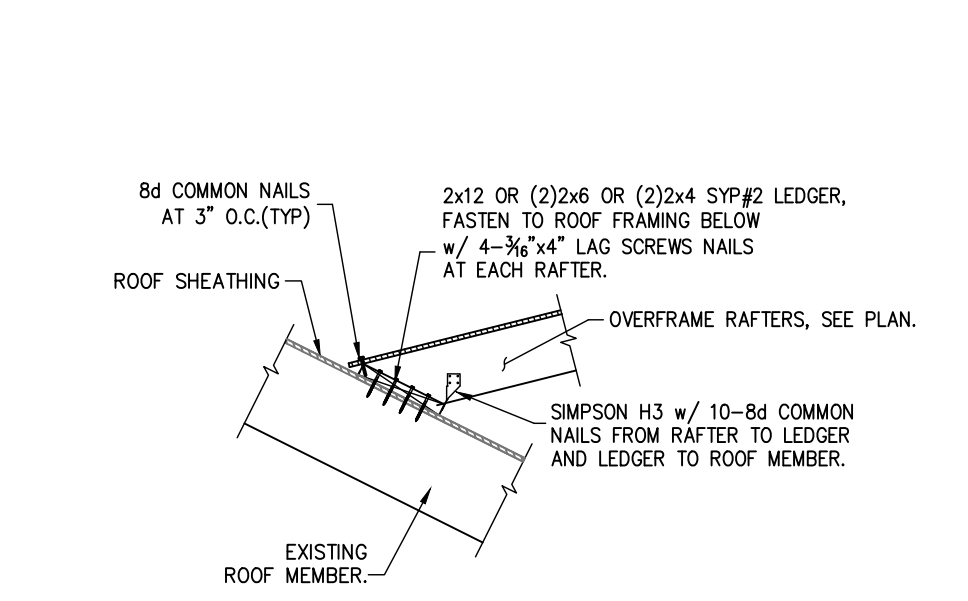


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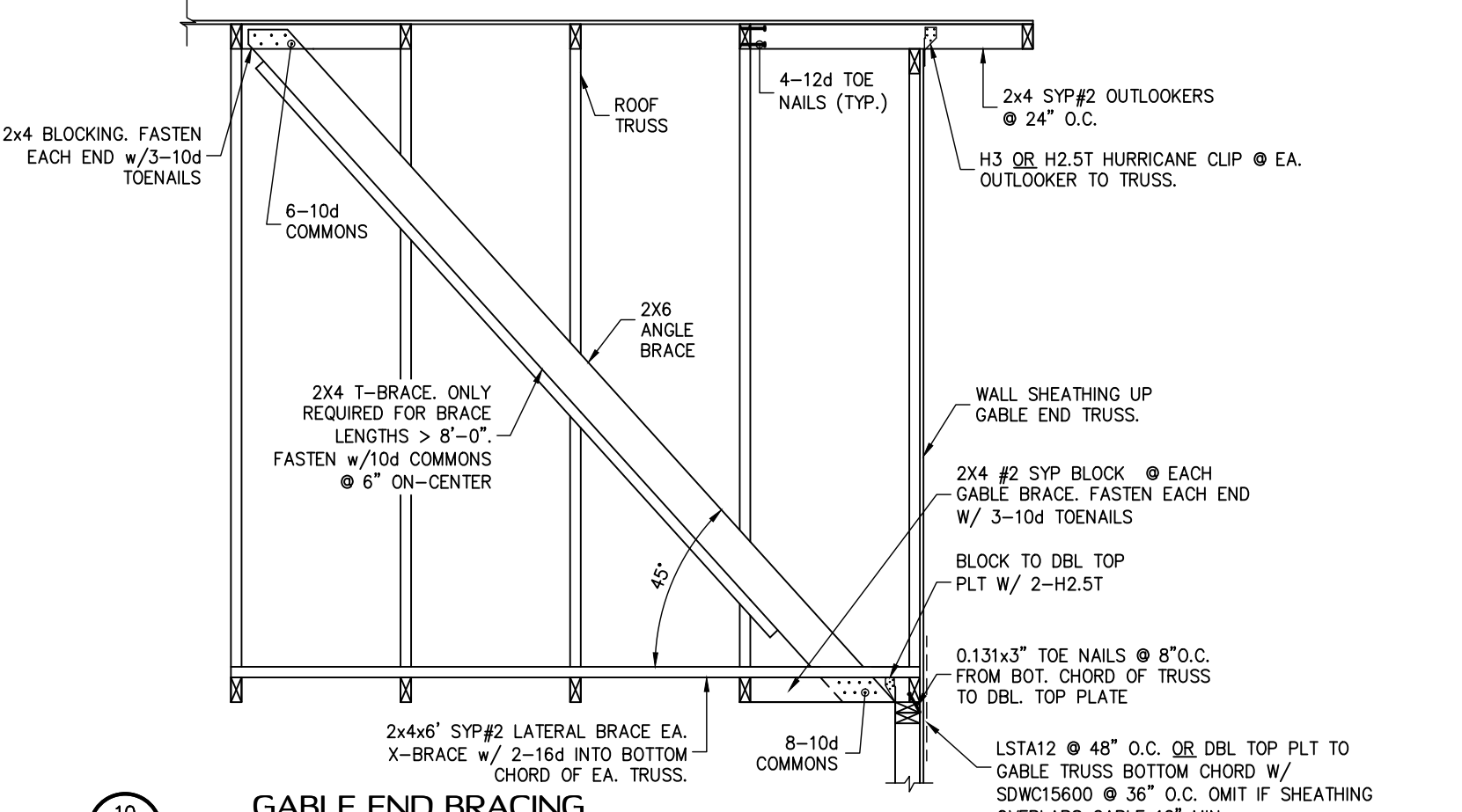
5 SHEARWALL ATTACHMENT AT ROOF & FLOOR
SCALE: N.T.S.



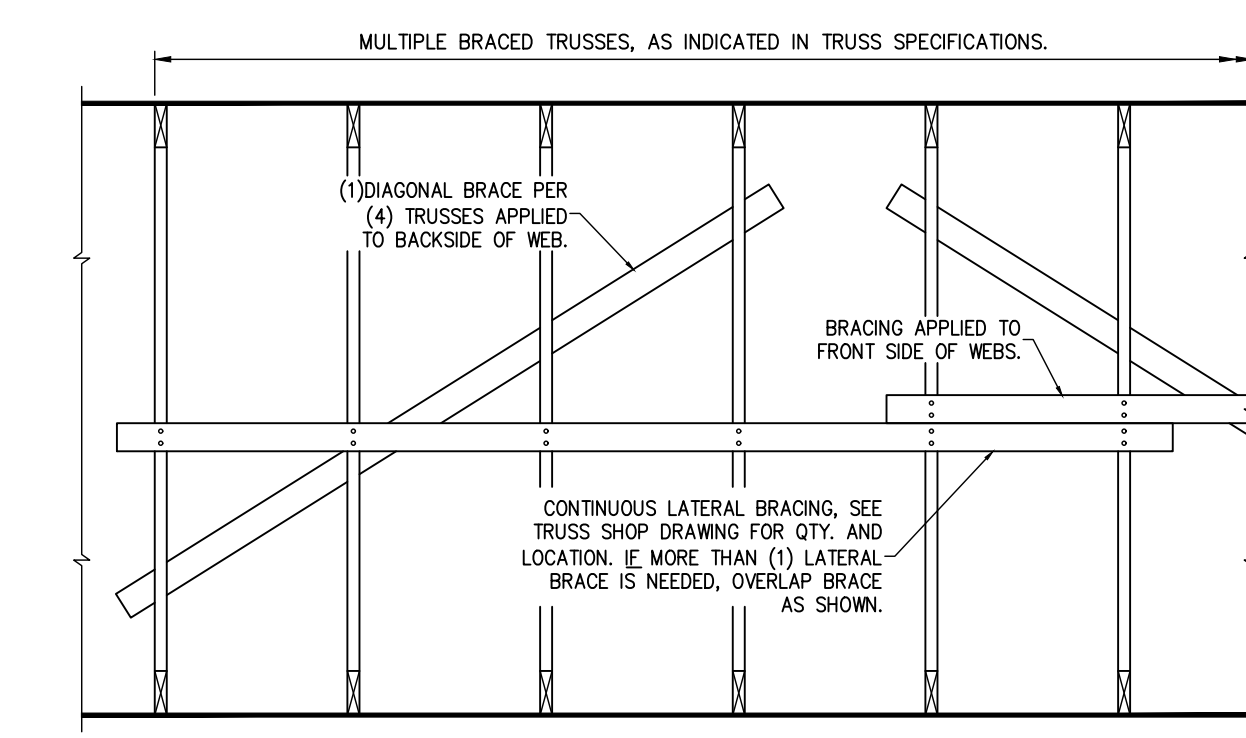
LEDGER CONNECTION



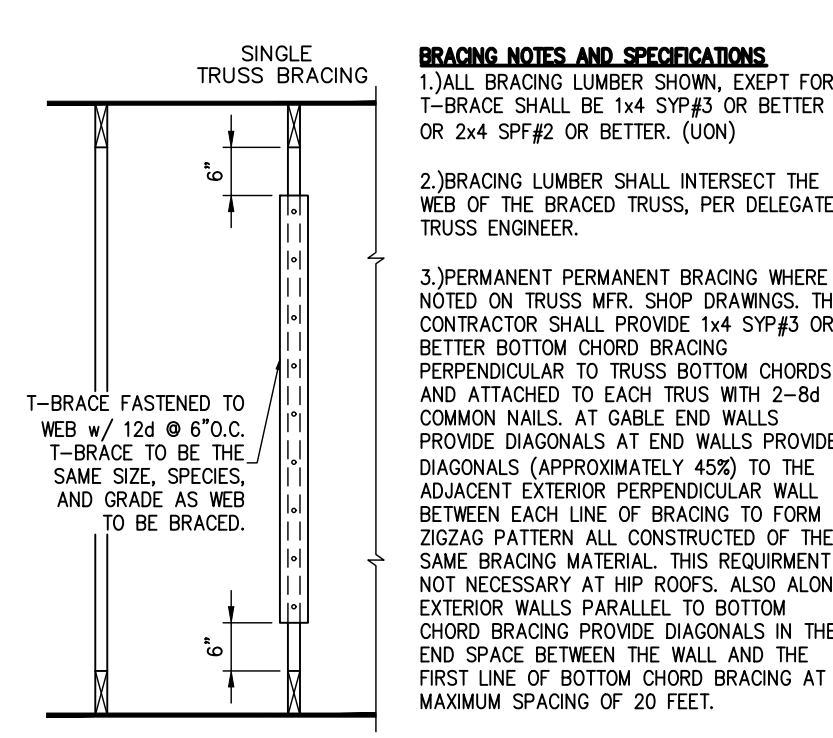
DECK LEDGER AT OVERFRAME RAFTERS



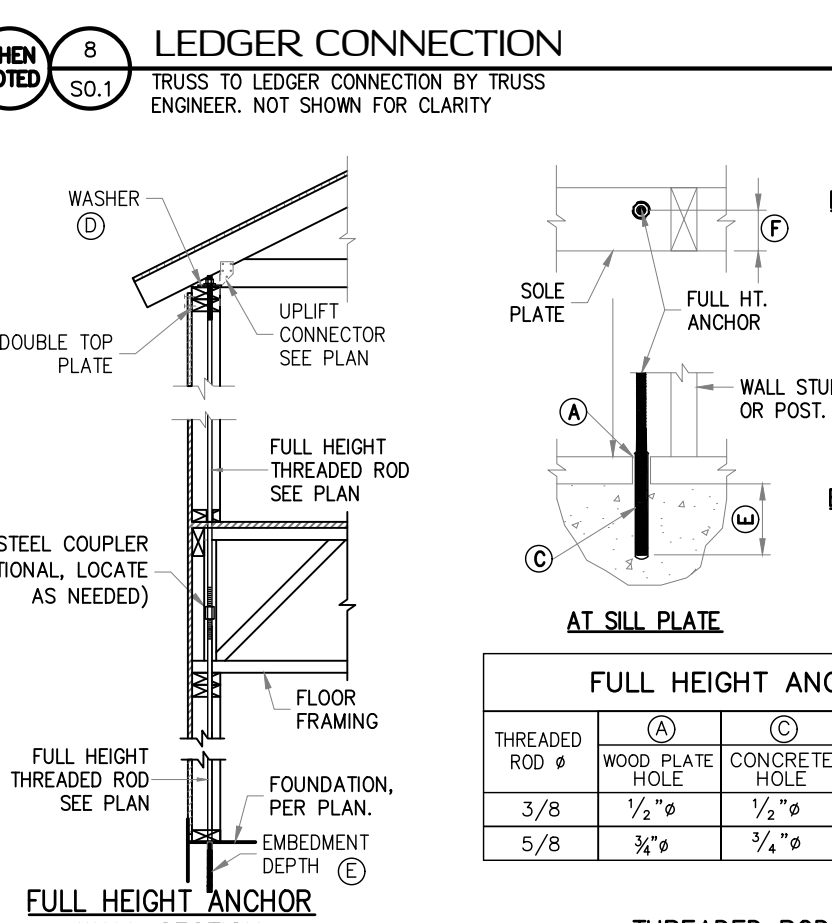
GABLE END BRACING



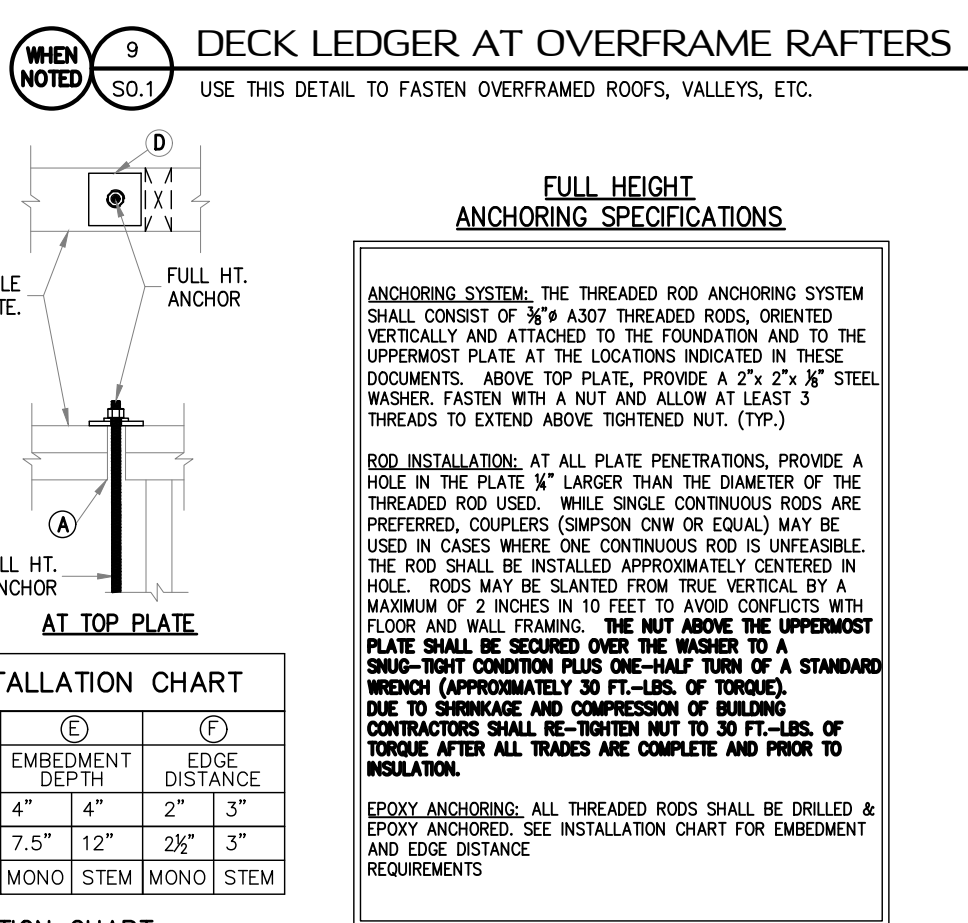
PERMANENT TRUSS BRACING



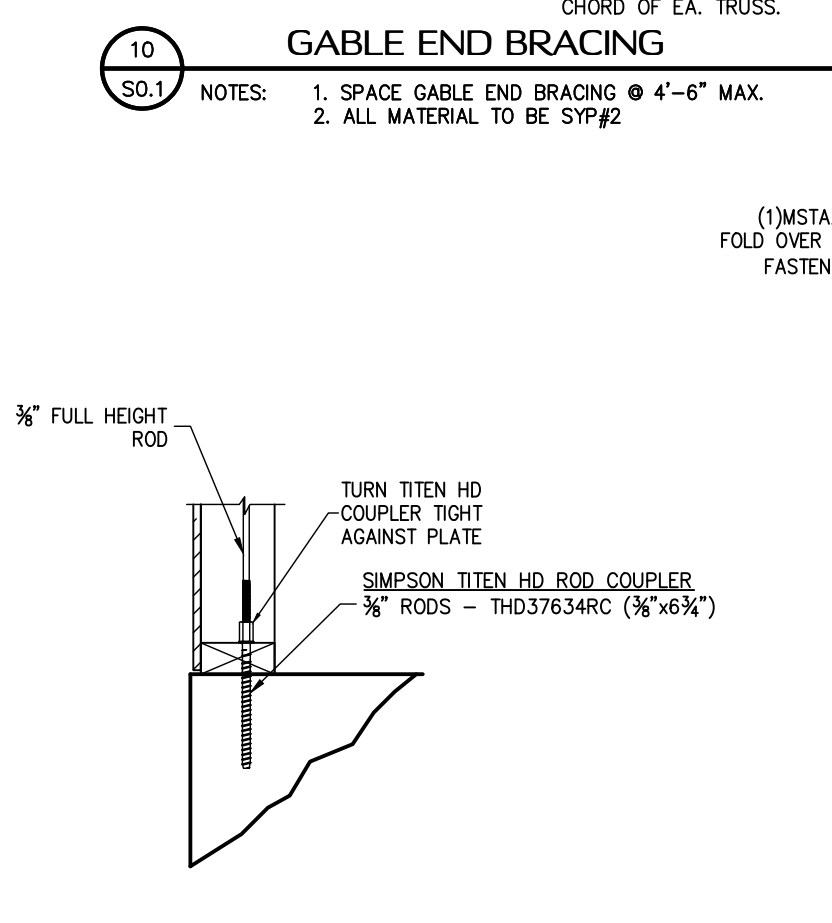
BRACING NOTES AND SPECIFICATIONS



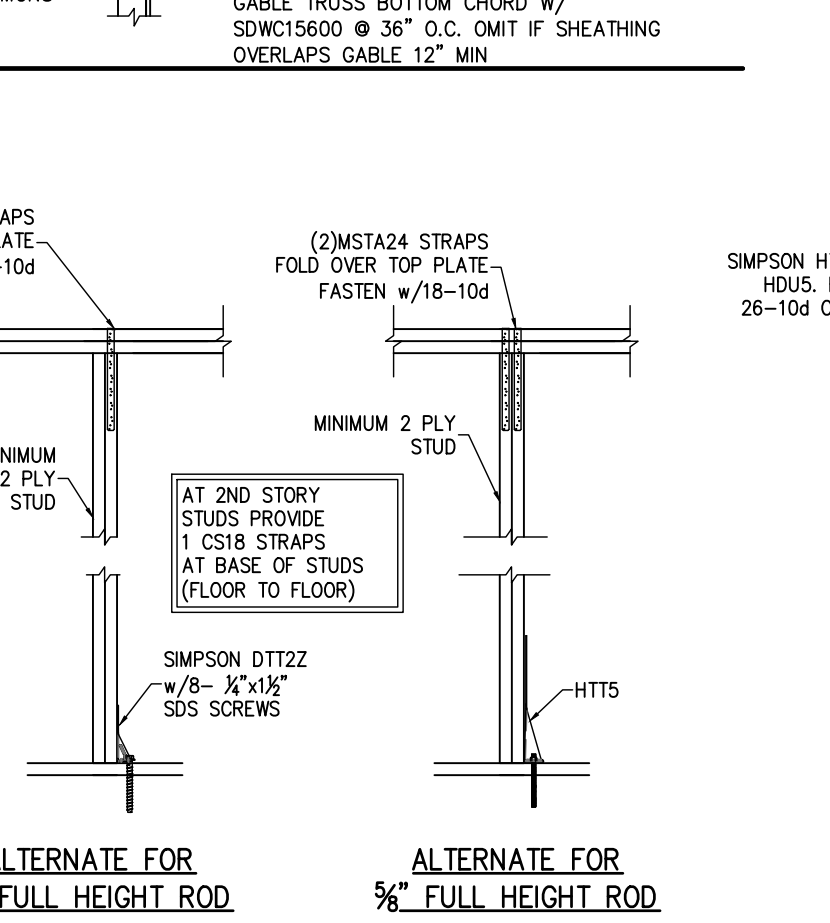
FULL HEIGHT WOOD FRAME WALL ANCHORING SYSTEM



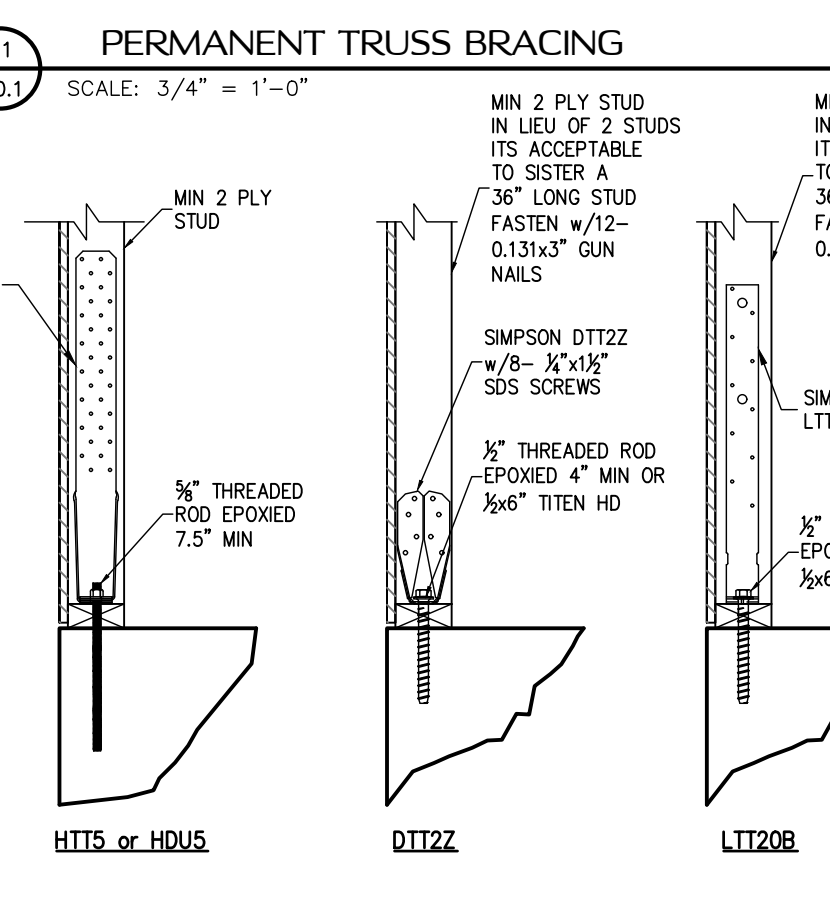
FULL HEIGHT ROD ALTERNATE ATTACHMENT



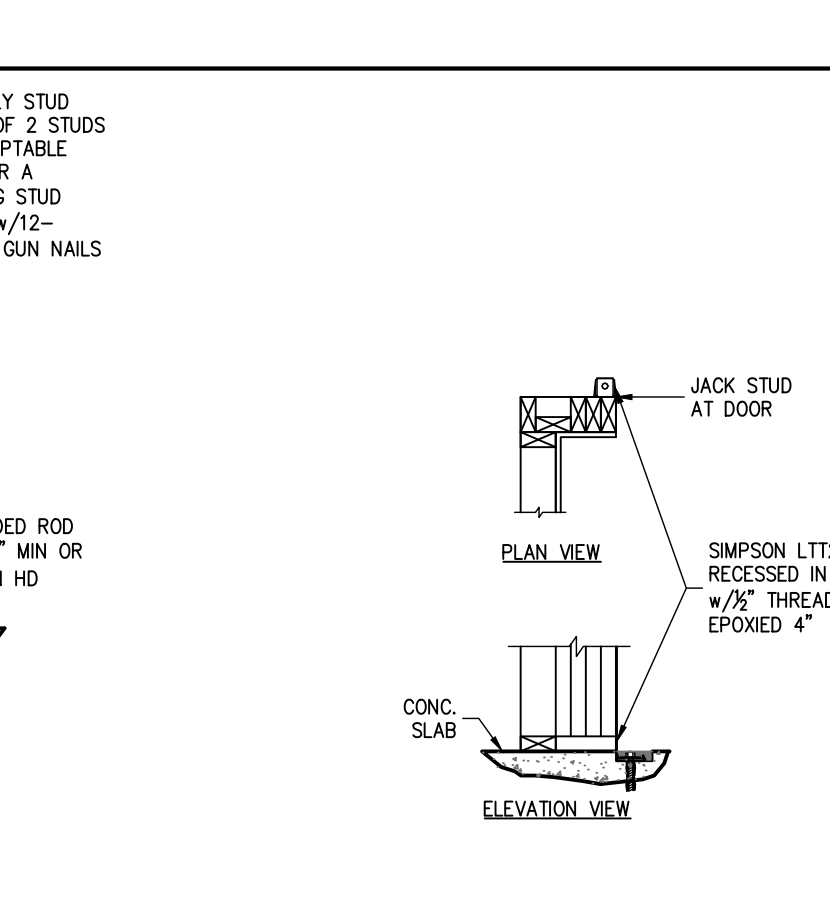
FULL HEIGHT THREADED ROD ALTERNATE



HOLD DOWN ATTACHMENT DETAIL



DOOR JAMB FASTENING



TYPICAL FRAMING DETAILS

CHRISTOPHER J. SABOURIN
No. 71461
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

08.02.24
Christopher J Sabourin
FL PE #71461

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PLAN NAME
BZEC HENDRICK
SSE No.
24-0402

ISSUE
DATE
PERMIT
08.02.24
REVISIONS
DATE

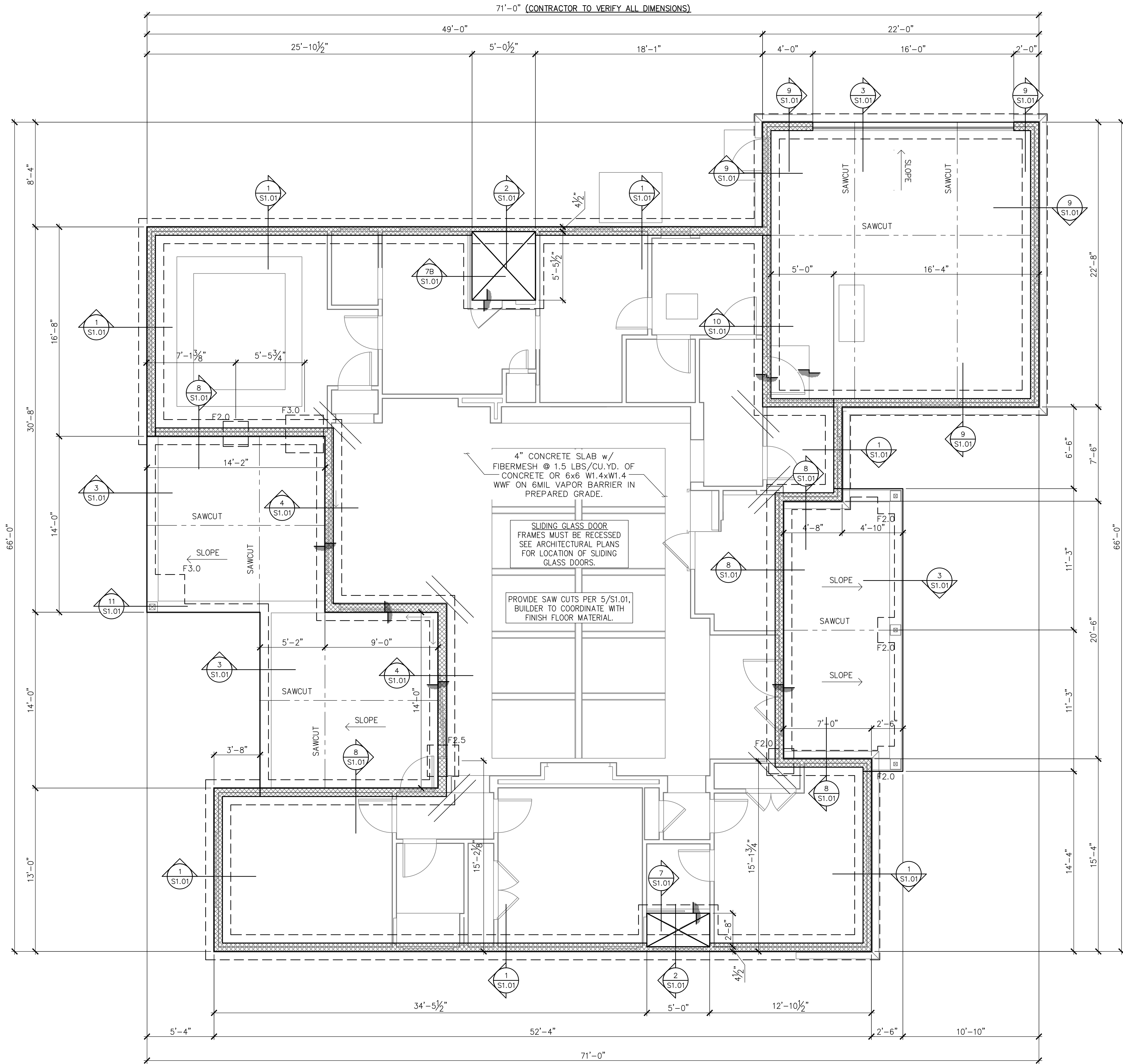
STRUCTURAL ENGINEERING FOR
HENDRICK RESIDENCE AT
279 SW SYDNEY NICOLE CT.
LAKE CITY, FL, 32024

FIELD ALTERATION
CONTRACTOR SHALL CONTACT SABO
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TYPICAL
FRAMING
DETAILS

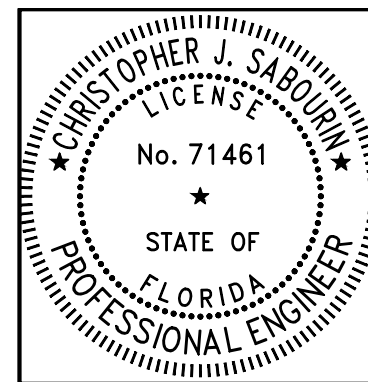
SHEET
S0.1
SHEET 2 OF 7



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION LEGEND			
	DESIGNATES SLAB EDGE LINE		
	DESIGNATES FOOTING LINE		
	DESIGNATES SAWCUT LINE		
	DESIGNATES STEMWALL		
	DESIGNATES SLAB STEP RECESS		
FOOTING SCHEDULE			
TYPE	DEPTH	WIDTH	BOTTOM BARS
F2.0	1'-0"	2'-0"x2'-0"	(3) #5 EW
F2.5	1'-0"	2'-6"x2'-6"	(3) #5 EW
F3.0	1'-0"	3'-0"x3'-0"	(3) #5 EW
F3.5	1'-0"	3'-6"x3'-6"	(4) #5 EW
F4.0	1'-4"	4'-0"x4'-0"	(4) #5 EW
GENERAL FOUNDATION NOTES			
1. THIS FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. SEE ARCH FOR DIMENSIONS			
2. SEE GENERAL NOTES AND SPECIFICATIONS ON S0.0 FOR FEATURES NOT INCLUDED WITHIN THIS PLAN.			
3. FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES			
4. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.			

CONTRACTOR TO VERIFY DIMENSIONS



08.02.24
Christopher J Sabourin
FL PE#71461

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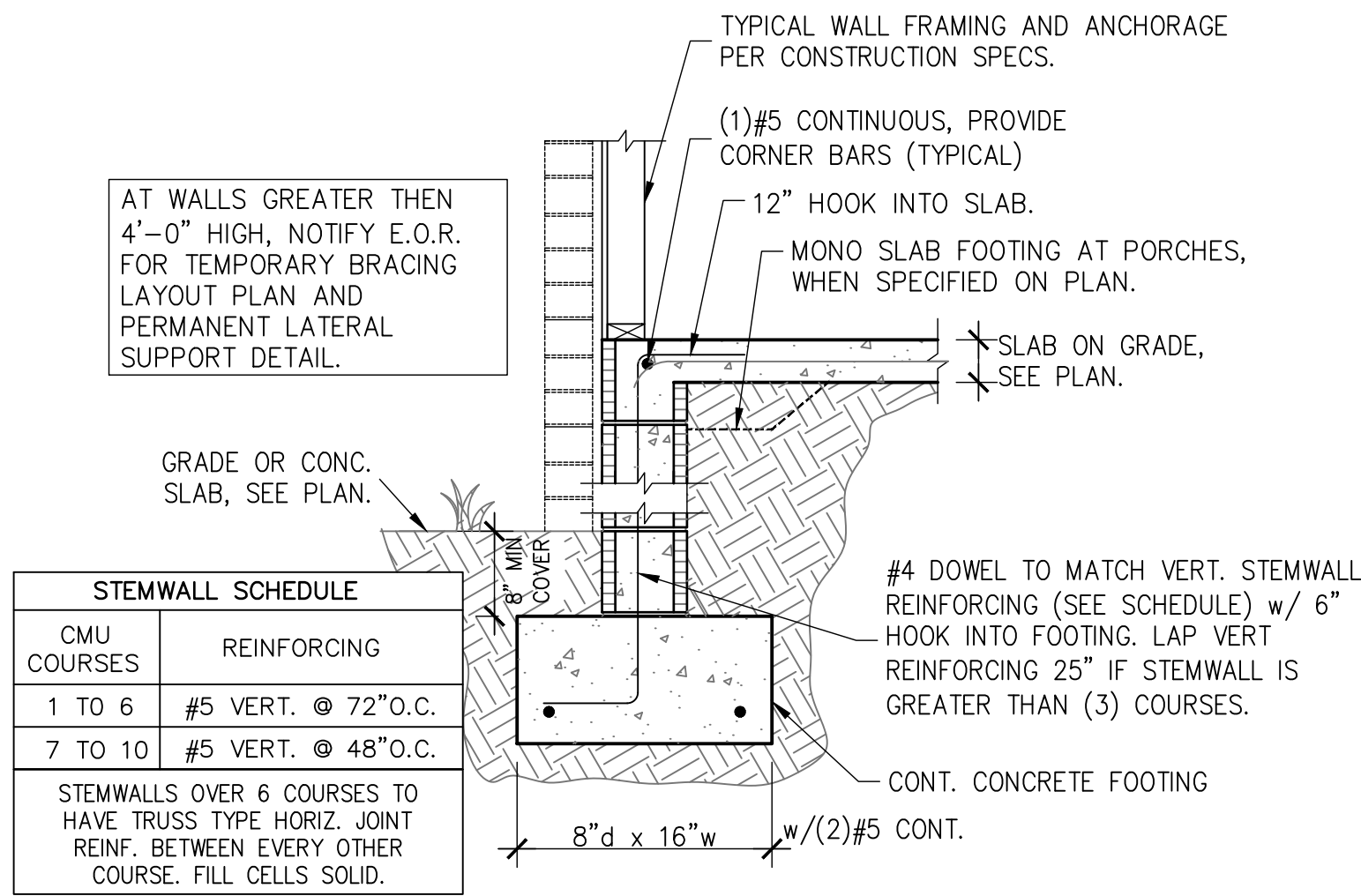
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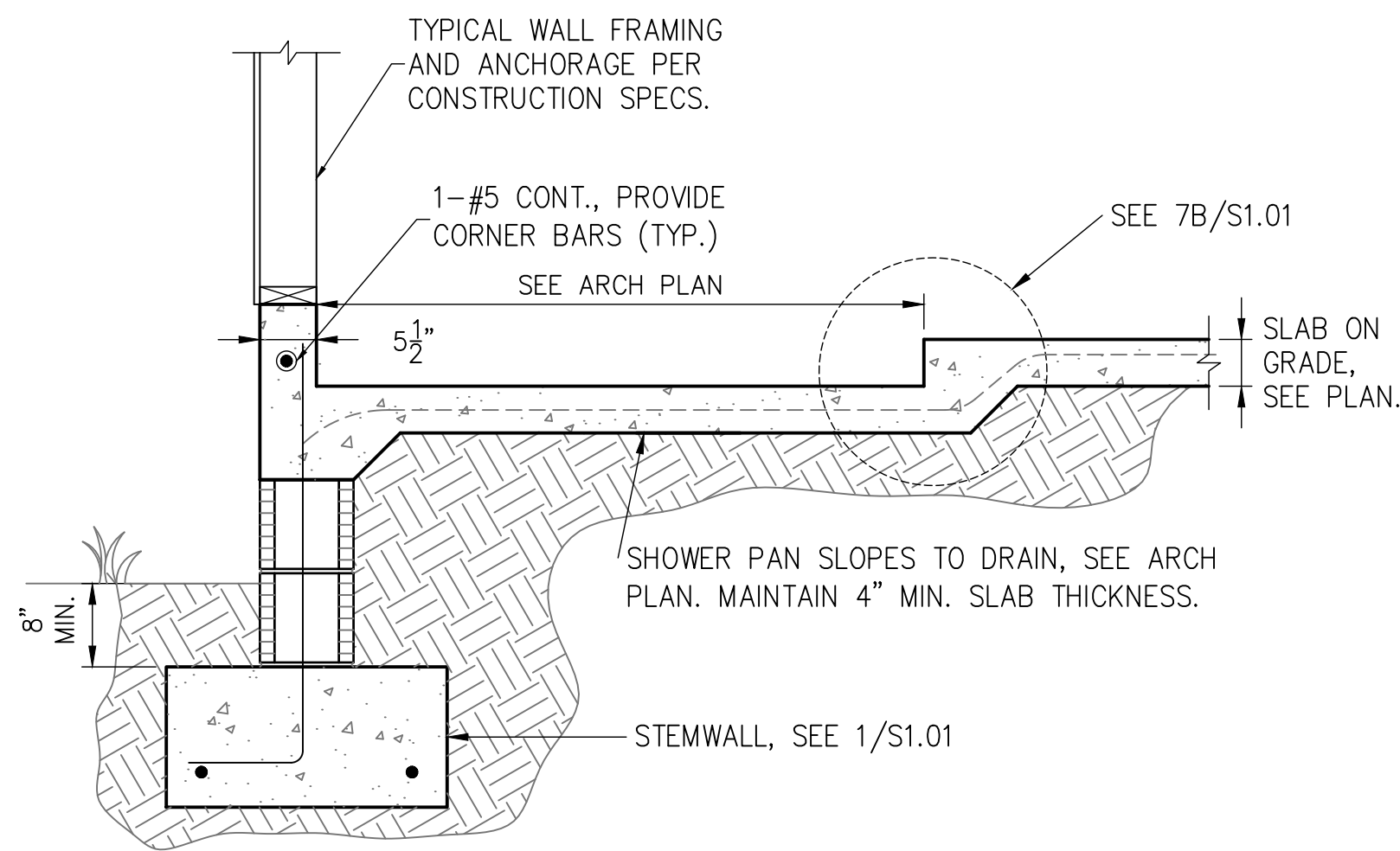
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FOUNDATION
PLAN

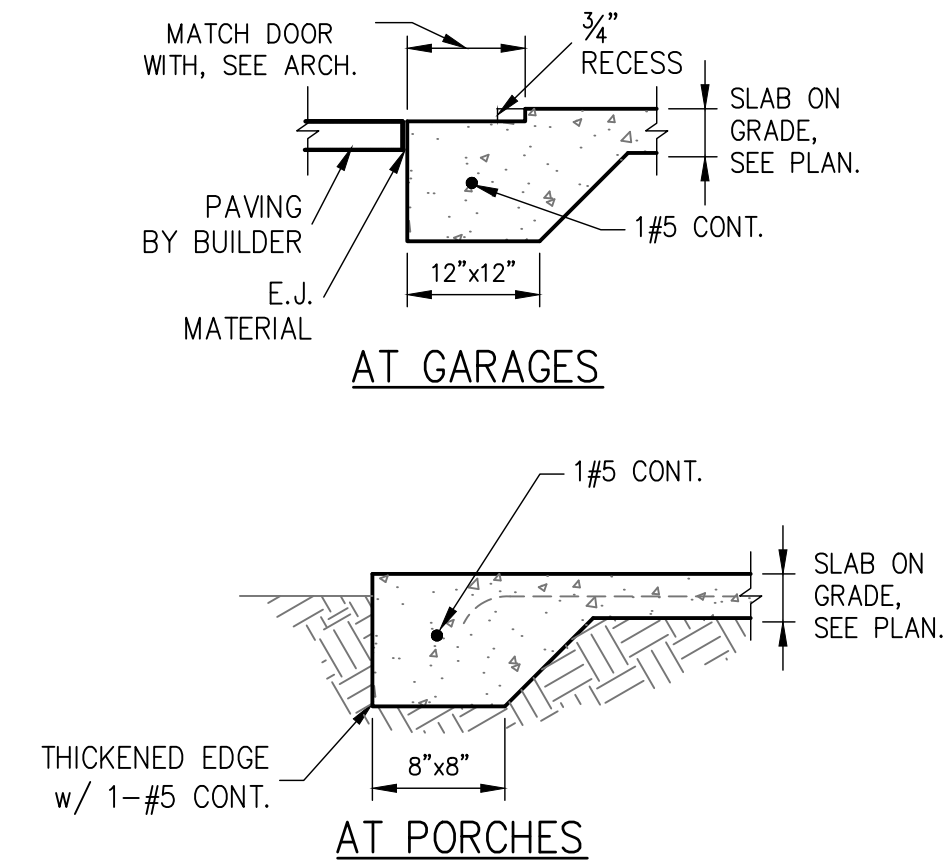
SHEET
S1.0
SHEET 3 OF 7



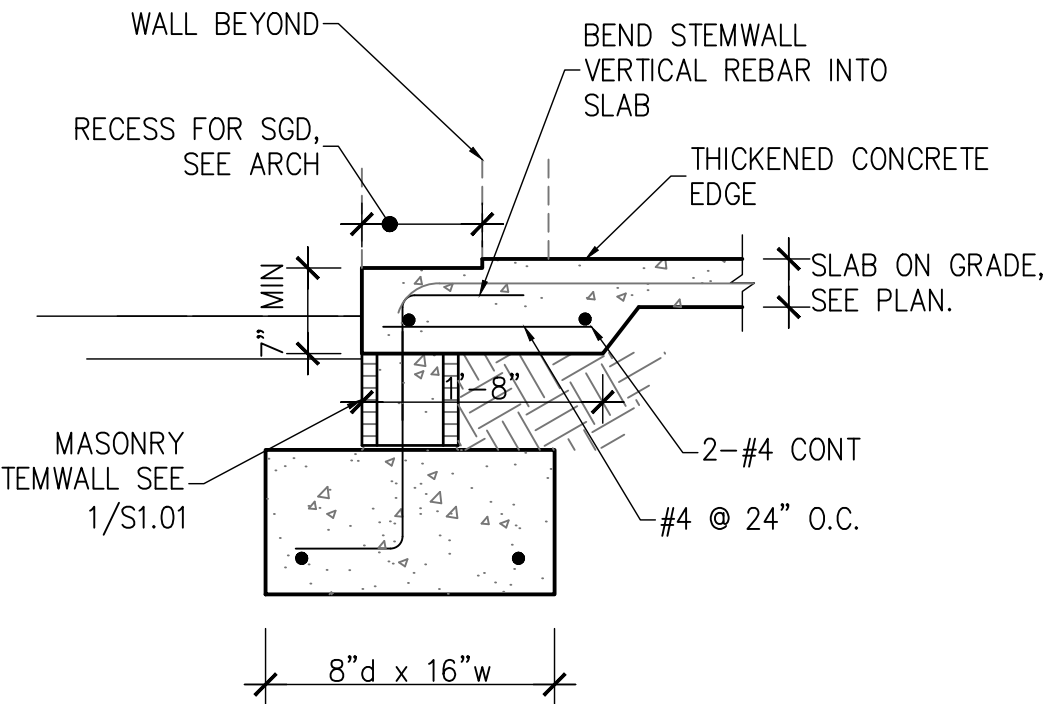
1 STEMWALL FOOTING
S1.01 SCALE: 3/4" = 1'-0"



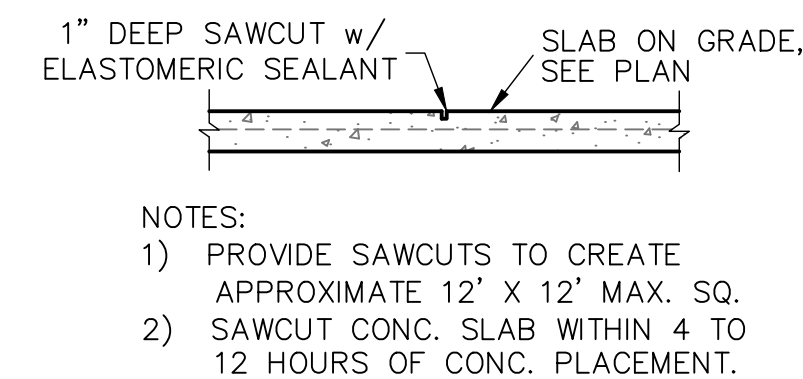
2 FOOTING W/ SHOWER RECESS
S1.01 SCALE: 3/4" = 1'-0"



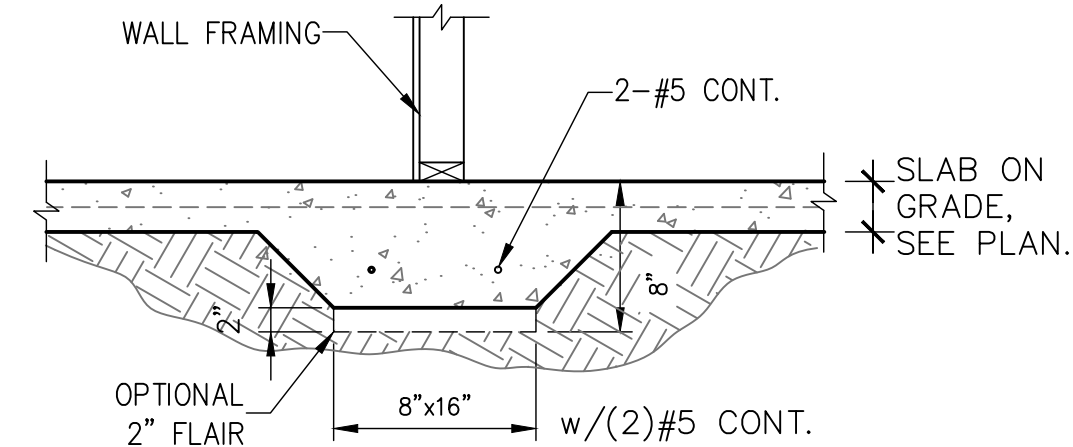
3 THICKENED SLAB
S1.01 SCALE: 3/4" = 1'-0"



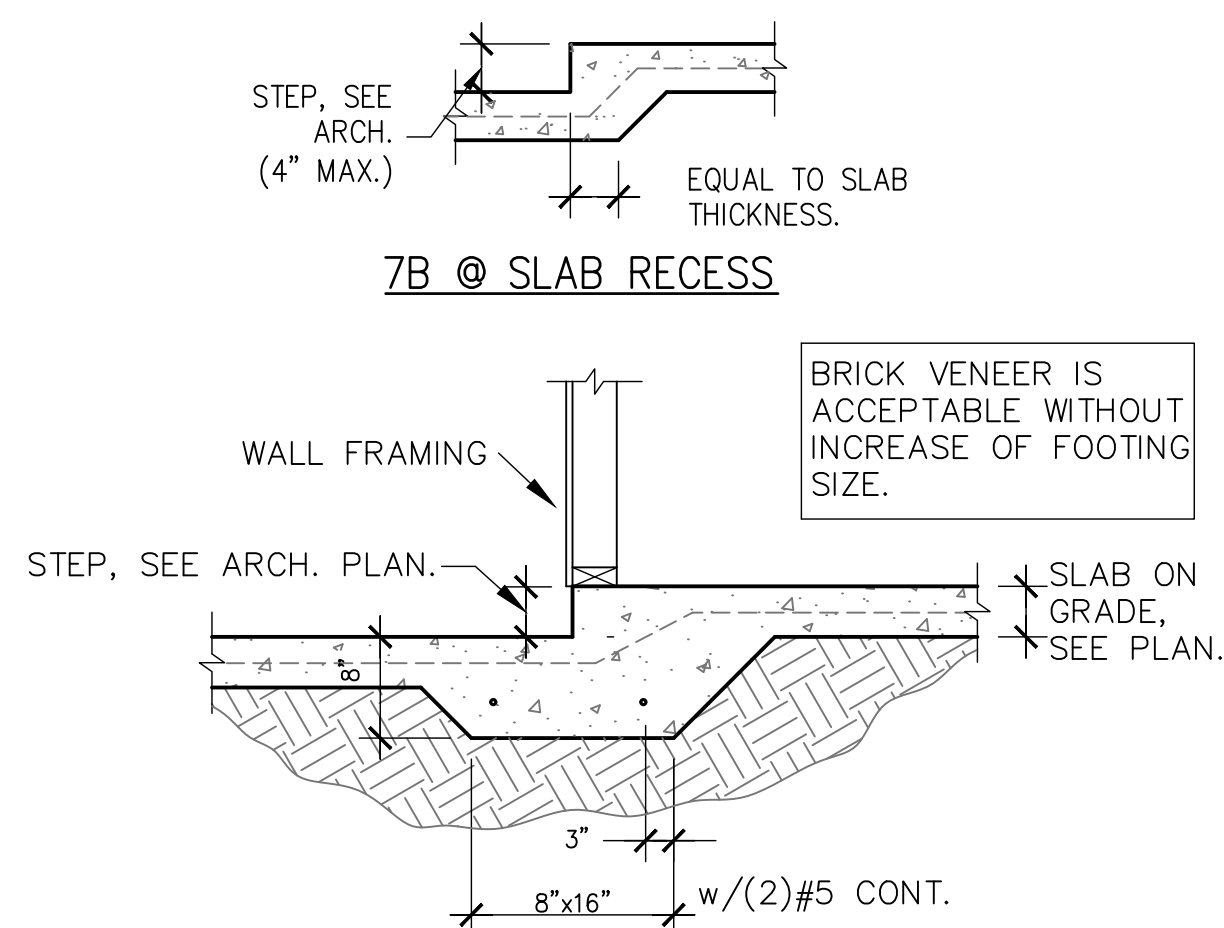
4 STEMWALL FOOTING AT SLIDER
S1.01 SCALE: 3/4" = 1'-0"



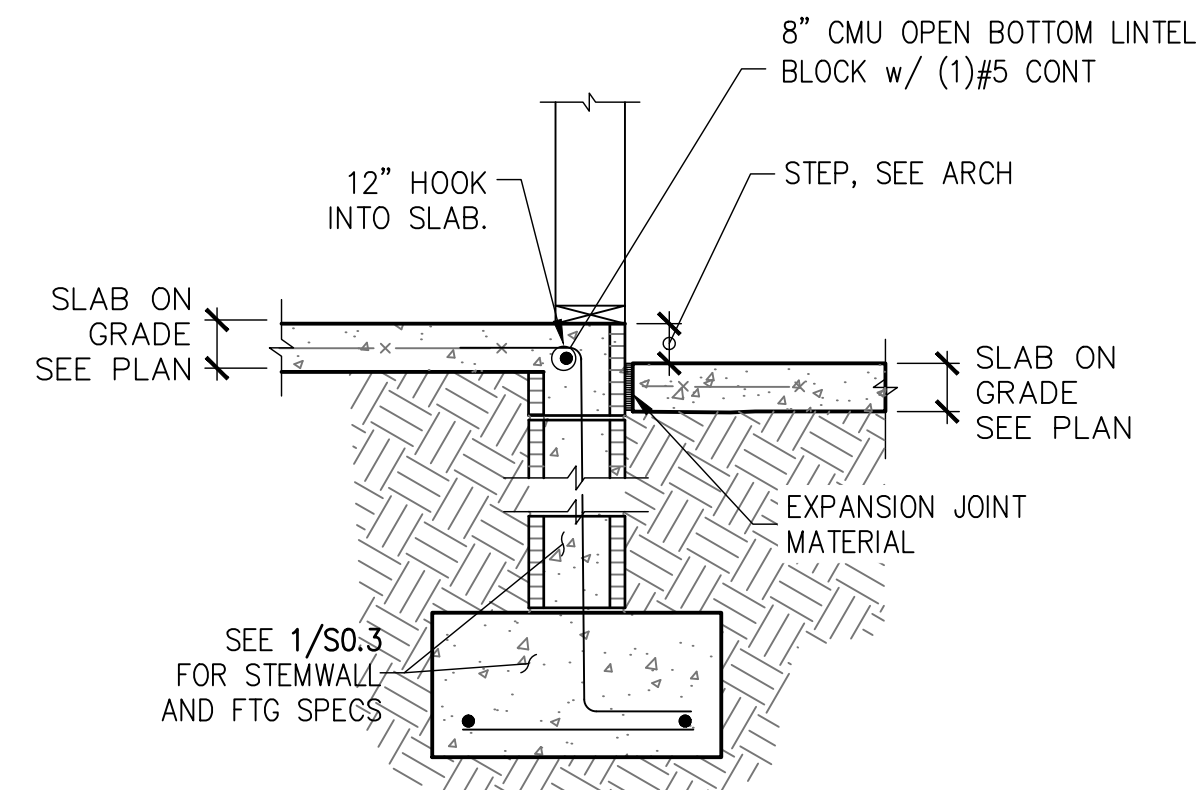
5 SAW CUT DETAIL
S1.01 SCALE: 3/4" = 1'-0"



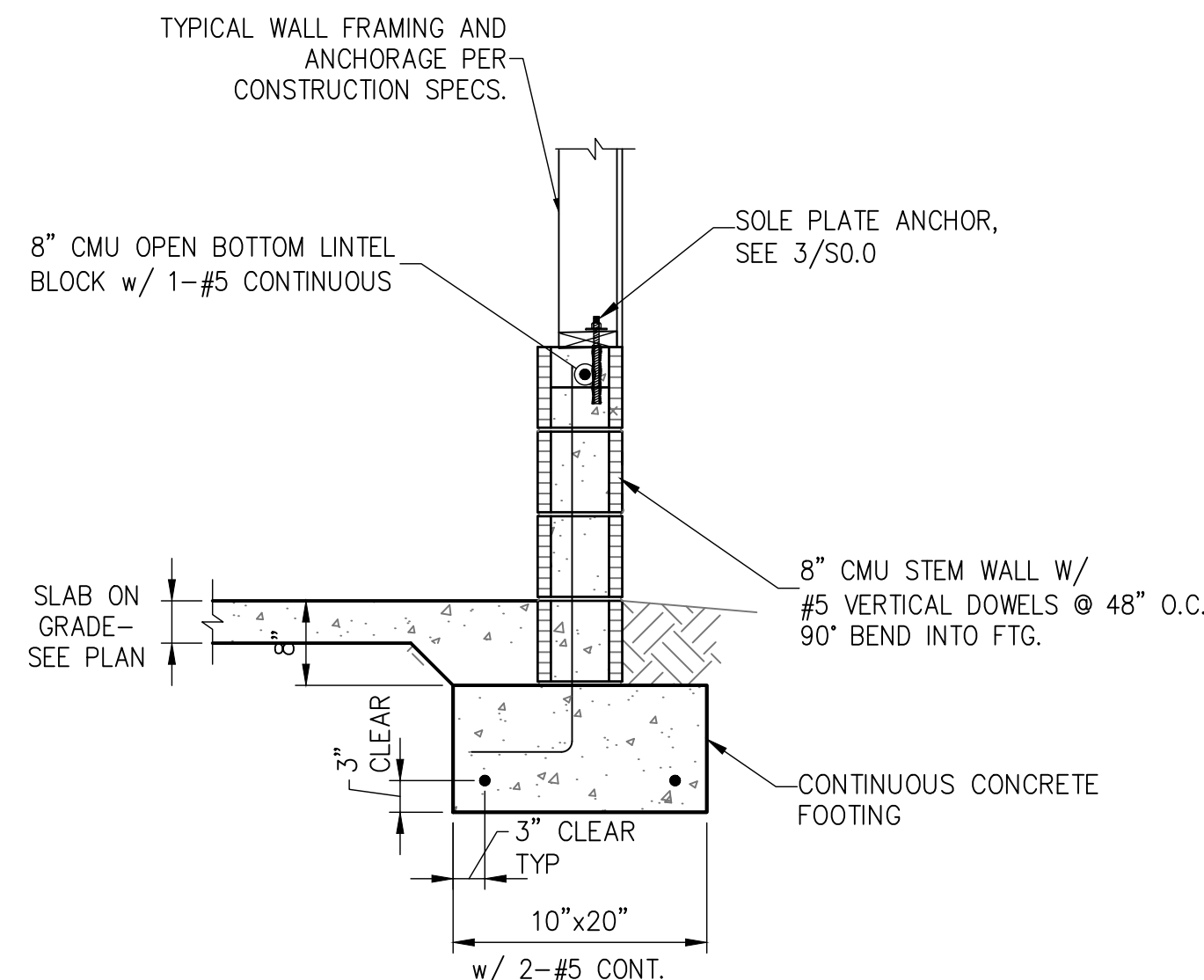
6 BEARING AT INTERIOR
S1.01 SCALE: 3/4" = 1'-0"



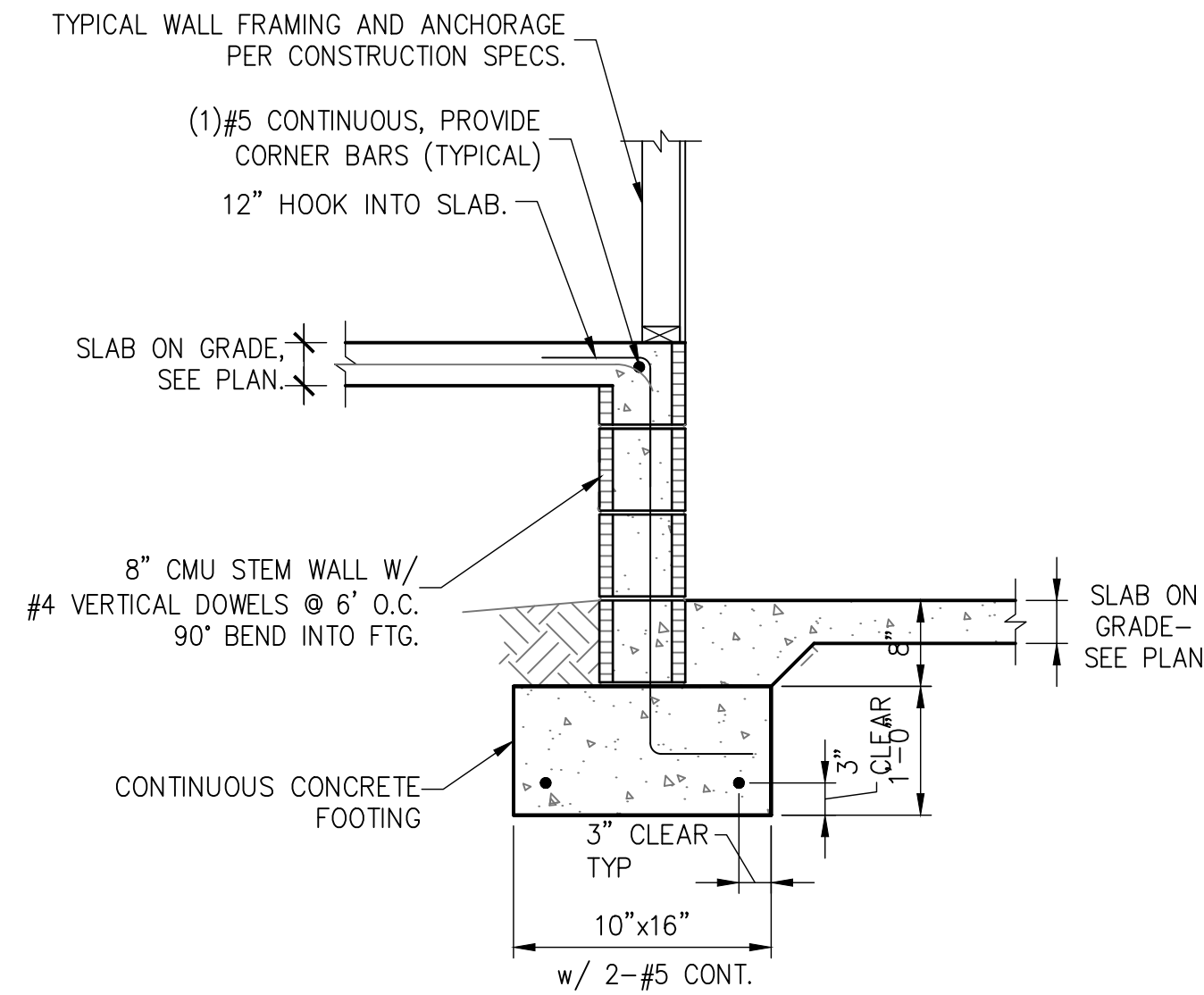
7 MONO. FOOTING AT STEP-DOWN
S1.01 SCALE: 3/4" = 1'-0"



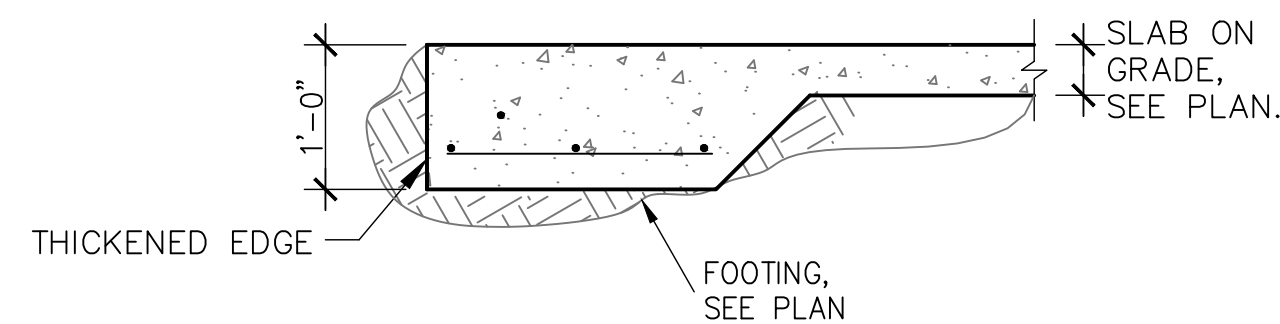
8 STEP AT STEMWALL
S1.01 SCALE: 3/4" = 1'-0"



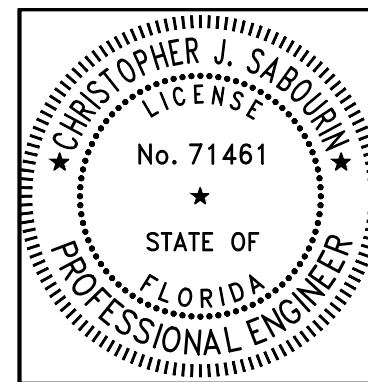
9 GARAGE STEM WALL
S1.01 SCALE: 3/4" = 1'-0"



10 STEMWALL AT GARAGE
S1.01 SCALE: 3/4" = 1'-0"



11 SPOT FOOTING AT PORCH SLAB
S1.01



08.02.24
Christopher J Sabourin
FL PE #71461

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PLAN NAME
BZEC HENDRICK

SSE No.
24-0402

ISSUE	DATE
PERMIT	08.02.24
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STRUCTURAL ENGINEERING FOR
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FIELD ALTERATION

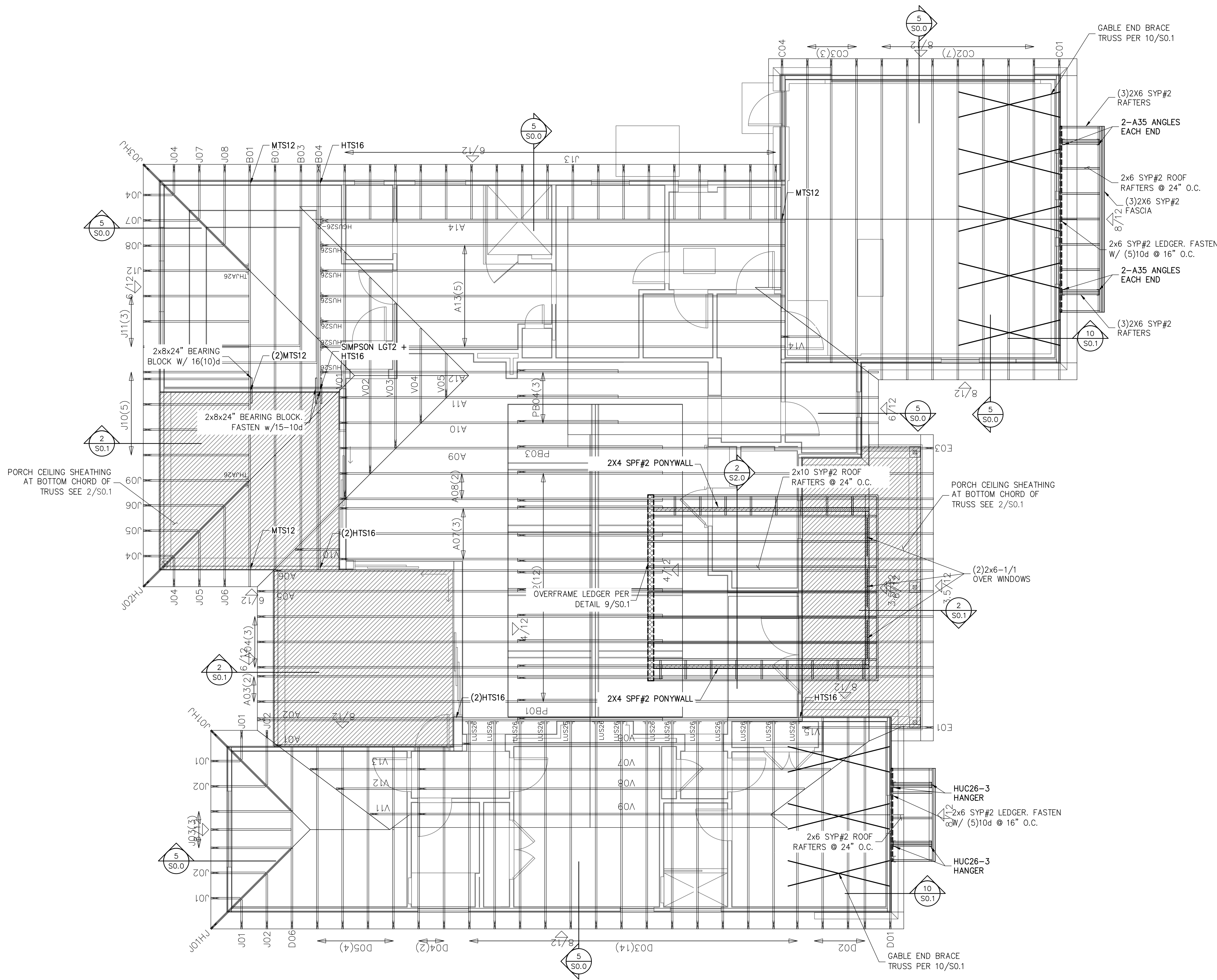
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FOUNDATION
DETAILS

SHEET
S1.01
SHEET 4 OF 7



TRUSS / ROOF RAFTER NOTES: STRAPPING NOTES:
STRAP ROOF TRUSSES AND RAFTERS TO BEARING WITH
(2)12D TOENAILS & (1)SIMPSON SDWC15600 SCREW UNLESS
OTHERWISE NOTED.

ROOF TRUSS PLACEMENT PLAN
SCALE: 1/4" = 1'-0"

SYMBOLS LEGEND

HTS16

DESIGNATES UPLIFT CONNECTION.

FRAMING PLAN NOTES:

1. FOR TYPICAL ROOF SHEATHING AND FRAMING, SEE SHEET S0.0
2. FOR SPECIFIC UPLIFT CONNECTIONS, SEE PLAN MIN. (1)SDWC CONNECTOR.
3. FOR GENERAL DESIGN SPECIFICATIONS SEE SHEET S0.0
4. WHEN USING (2)H2.5T CLIPS ON 1 1/2" WIDE LUMBER, PLACE CLIPS DIAGONALLY ACROSS DOUBLE TOP PLATE FROM EACH OTHER.

TRUSS FASTENING DETAILS

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

TRUSS TIE DOWN WITH SIMPSON SDWC

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

Note:

1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birdsmouth" cut.
2. Reference detail 4 for installation instructions.

SIMPSON SDWC INSTALLATION RANGE

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

Note:

Reference detail 2a for installation angle limit

SDWC INSTALLATION

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

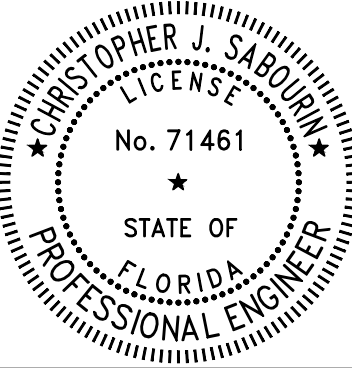
SDWC INSTALLATION RANGE

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

SDWC AT TOP PLATE SPLICE

STUD NOT DIRECTLY BELOW TRUSS



08.02.24
Christopher J Sabourin
FL PE#71461

CHRISTOPHER J SABOURIN ST2 OF
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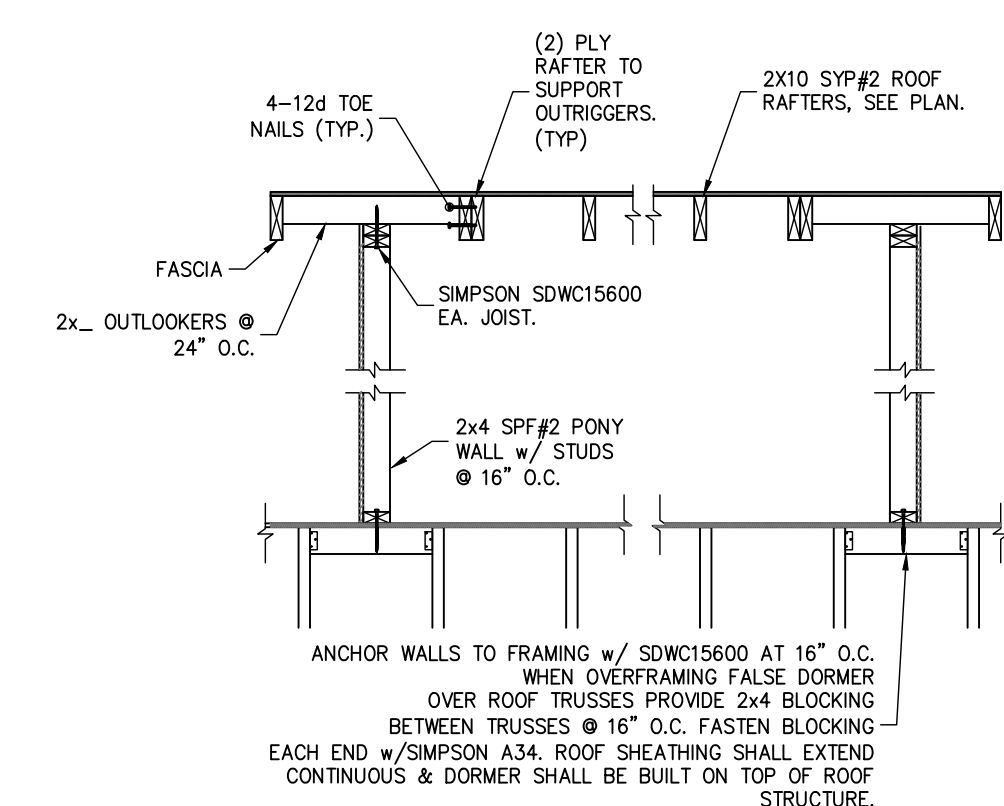
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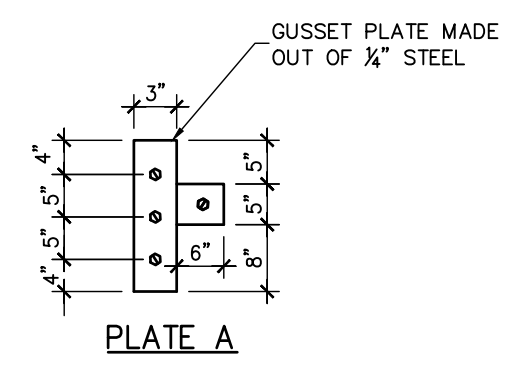
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ROOF TRUSS
PLACEMENT
PLAN



2 DORMER DETAIL
S2.0



4 GARAGE HEADER FRAMING