	1/2009		Ibia County Bu	uilding Permit	nstruction	PERMIT 000027921
APPLICANT	B. TRENT		be Frommently 1 osted	PHONE	386.397.0545	00002/921
ADDRESS	697	SE HOLLY TERRA	ACE	LAKE CITY	300.377.0343	FL 32025
OWNER	PETE GIE		ice	PHONE	386.397.0545	
ADDRESS	170	SW VANN CT		LAKE CITY		FL 32024
CONTRACTO	R B. T	RENT GIEBEIG		PHONE	386.397.0545	
LOCATION O	F PROPER	ΓΥ <u>90-</u> W ΤΟ	SR. 247-S TO MAY-FA	IR LN,TR TO VANN C	Γ,TR & IT'S	
		THE 3RD	LOT ON L.			
TYPE DEVEL	OPMENT	SFD/UTILITY	EST	TIMATED COST OF CO	ONSTRUCTION	115600.00
HEATED FLO	OR AREA	1608.00	TOTAL ARE	EA2312.00	HEIGHT 17	7.60 STORIES 1
FOUNDATION	CONC	WAL	LS FRAMED R	ROOF PITCH 6'12	FLO	OOR CONC
LAND USE &	ZONING	RSF-2		MAX	K. HEIGHT 3:	5
Minimum Set I	Back Requir	ments: STREET	-FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE	x	DEVELOPMENT PER	MIT NO.	
PARCEL ID	11-4S-16-0	02911-313	SUBDIVISION	N MAY-FAIR		
LOT <u>13</u>	BLOCK	PHASE	UNIT _3	3 TOTA	AL ACRES 0.5	51
00000			R282811523	( List	Talka	
Culvert Permit	No.	Culvert Waiver	Contractor's License Num	nber	Applicant/Owner/	Contractor
18"X32'MITER		08-0325	BLK		HD //	N
Driveway Conn	ection	Septic Tank Number	LU & Zonir	ng checked by App	proved for Issuance	e New Resident
COMMENTS:	ELEVATI	ON CONFIRMATIO	N LETTER REQUIRED	OCT AD MEE O MAI	DED DI AT	
		ON CONTINUATIO	N DETTER REQUIRED	@ SLAB. MFE @ 164.	FERFLAI.	
		ON CONTINUATIO	N DE L'IER REQUIREE	@ SLAB. MFE @ 164.		Seb 5028
					Check # or Ca	ash 5028
			UILDING & ZONIN		Check # or Ca	ash 5028 (footer/Slab)
Temporary Pow		FOR BI		IG DEPARTMENT	Check # or Ca	(footer/Slab)
Temporary Pow	ver	FOR BI	UILDING & ZONIN  Foundation		Check # or Ca ONLY Monolithic	(footer/Slab)  date/app. by
	ver	FOR BI	UILDING & ZONIN  Foundation  Slab	IG DEPARTMENT	Check # or Ca ONLY Monolithic	(footer/Slab)
Temporary Pow	ver gh-in plumb	FOR BI  date/app. by  ing  date/a  In	Foundation Slab pp. by sulation	date/app. by	Check # or Ca ONLY Monolithic	(footer/Slab)  date/app. by  Nailing
Temporary Pow Under slab roug	vergh-in plumb	FOR BI  date/app. by  ing  date/a  In	Foundation Slab pp. by sulation	IG DEPARTMENT date/app. by	Check # or Ca ONLY Monolithic	(footer/Slab)  date/app. by  Nailing
Temporary Pow Under slab roug Framing	vergh-in plumb	FOR BI  date/app. by  ing  date/a  In	Foundation  Foundation  Slab  pp. by sulation  date	date/app. by  date/app. by  e/app. by	Check # or Ca ONLY Monolithic	(footer/Slab)  date/app. by  Nailing date/app. by
Temporary Pow Under slab roug Framing Rough-in plumb	date/app	date/app. by ing date/ap . by	Foundation  Foundation  Slab  pp. by sulation  date	date/app. by  date/app. by  e/app. by  Elate/app. by	Check # or Ca ONLY  Monolithic Sheathing/N	(footer/Slab)  date/app. by  Nailing
Temporary Pow Under slab roug Framing	date/appoing above s	date/app. by ing date/ap . by	Foundation  Foundation  Slab  pp. by sulation  date	date/app. by  date/app. by  e/app. by  Elate/app. by	Check # or Ca ONLY  Monolithic  Sheathing/N	(footer/Slab)  date/app. by  Nailing date/app. by
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Temporary Pow Under slab roug Framing Rough-in plumb Heat & Air Duc	date/appoing above set	date/app. by ing date/a date/a  Jack date/app. by  Jack date/app. by  Jack date/app. by	Foundation  Foundation  Slab  pp. by sulation  date floor  Peri. beam (Linte  C.O. Final	date/app. by  date/app. by  e/app. by  E/app. by  ate/app. by  date/app. by  date/app. by	Check # or Ca ONLY  Monolithic  Sheathing/N  ectrical rough-in  Pool  Culvert	(footer/Slab)  date/app. by  Nailing  date/app. by  date/app. by
Temporary Pow Under slab roug Framing Rough-in plumb Heat & Air Duck Permanent powe Pump pole	date/appoing above set	date/app. by ing date/a  date/a  In b. by  lab and below wood a  ate/app. by  te/app. by  Utility Pole	Foundation  Foundation  Slab  pp. by sulation  date  floor  C.O. Final  M/H tie de  tte/app. by	date/app. by  date/app. by  e/app. by  Elate/app. by  date/app. by	Check # or Ca ONLY  Monolithic  Sheathing/N  ectrical rough-in  Pool  Culvert	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Temporary Pow Under slab roug Framing Rough-in plumb Heat & Air Duc Permanent powe	date/appoing above setdate/app. by	date/app. by ing date/ap  b. by lab and below wood to the date/app. by te/app. by Utility Pole date/app.	Foundation  Foundation  Slab  pp. by sulation  date  floor  C.O. Final  M/H tie de	date/app. by  date/app. by  e/app. by  Elate/app. by  date/app. by  date/app. by  late/app. by  date/app. by  date/app. by	Check # or Ca ONLY  Monolithic  Sheathing/N  ectrical rough-in  Pool  Culvert	(footer/Slab)  date/app. by  Nailing  date/app. by  date/app. by  date/app. by  date/app. by
Temporary Pow Under slab roug Framing Rough-in plumb Heat & Air Duc Permanent powe Pump pole d Reconnection	date/appoing above setdate/app. by	date/app. by ing date/a  date/app. by ing date/a  Jack date/app. by ate/app. by Le/app. by Le/app. by date/app. by date/app. by date/app. by	Foundation  Foundation  Slab  pp. by sulation  date  floor  C.O. Final  M/H tie de  te/app. by  RV	date/app. by  date/app. by  e/app. by  e/app. by  ate/app. by  date/app. by  date/app. by  date/app. by  owns, blocking, electricit  date/app. by	Check # or Ca ONLY  Monolithic  Sheathing/N  ectrical rough-in  Pool  Culvert  y and plumbing  Re-roof	(footer/Slab)  date/app. by  Nailing date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
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**PERMIT** 

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

Columbia County Building Permit Application	Colu	umbia	County	Building	Permit	Appl	icatio
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_	
Fo	or Office Use Only Application # 0906-51 Date Received 925 By W Permit # 1935/27921
2	Coning Official BLK Date 01.07 01 Flood Zone Land Use Land Use Zoning RSF-2
F	EMA Map # N/A Elevation N/A MFE 149 STRIVER N/A Plans Examiner HO Date 6-30
	omments Elevation Configuration Letter Required at Slab
	NOC theH Moded or PA Asite Plan - State Road Info - Parent Parcel #
0	Dev Permit # □ In Floodway □ Letter of Auth. from Contractor □ F W Comp. letter
IM	PACT FEES: EMS Fire Corr Road/Code
L	School = TOTAL_Srsperlal
Sep	ofic Permit No. 08-0375
Na	me Authorized Person Signing Permit Trent Greberg Phone 397-0545
Ac	Idress 697 SE Holly Terrace Late City FL 32025
	vners Name Pete Giebeig Phone 752-7968
91	1 Address 170 SW Vann Court, L.C 32 32024
	intractors Name Trent Giebeig Construction Inc Phone 397-0545
	dress 697 St Holly Temace Lake City FL 32025
Fee	Simple Owner Name & Address Pete Gielag
Во	nding Co. Name & Address
Arc	chitect/Engineer Name & Address FREMAN DESIGN
Мо	rtgage Lenders Name & Address
Circ	cle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Prop	perty ID Number 11-45-16-02911-313 Estimated Cost of Construction 100,000
Sub	odivision Name Mayfair Unit 3 Lot 13 Block — Unit 3 Phase —
Driv	ring Directions 247 South Right on Now fair Lane
R	ight on Vann Court 3rd on 1eft
	Number of Existing Dwellings on Property — 0 —
Con	struction of Wood Wame on concrete Floor Total Acreage Lot Size 5/
e p	ou need a <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height 17'6"  PLAN PAGE A-2 OF PLANS  Just Distance of Structure from Property Lines - Front 27 Side 3415 Side Side Rear 908
	mber of Stories Heated Floor Area 1608 Total Floor Area 2312 Roof Pitch 6/12
nsta	lication is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or allation has commenced prior to the issuance of a permit and that all work be performed to meet the standards I laws regulating construction in this jurisdiction. $CV # 5028$

Page 1 of 2 (Both Pages must be submitted together.)

Revised 1-10-08

#### Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature (Permitee)

Contractor's License Number KB 2828 11523

Columbia County

Competency Card Number 000141

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2

Personally known or Produced Identification

SEAL:

Page 2 of 2 (Both Pages must be submitted together.)

ELAINE K. TOLAR MY COMMISSION # DD 436381 EXPIRES: October 2, 2009



Prepared by: Peter W. Giebeig PO Box 1384 Lake City, FL 32056

#### CORPORATE WARRANTY DEED

THIS INDENTURE, Made the Of A O , 2008, by Concept Construction of North Florida, Inc. a Florida Corporation

A corporation existing under the laws of the State of Florida and having its principal place of business at: 2109 W US Highway 90, Suite 170-144, Lake City, FL 32055,

hereinafter called the Grantor,

To:

Peter W. Giebeig,

whose post office address is : P.O. Box 1384, Lake City, FL 32056

hereinafter called the Grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives ad assigns of individuals, and the successors and assigns of corporation)

WITNESSETH: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: Tax ID# R02911-313

Lot 13, May-Fair Unit 3, a subdivision according to the plat thereof filed in Plat Book 8, Pages 84-85, of the Public Records of Columbia County, Florida.

This is an absolute conveyance of the title in consideration of the cancellation of the debt secured by the mortgage recorded in Official Records Book 1085, Page 740, of the Public Records of Columbia County, Florida, and is not intended to be an additional security.

TOGETHER, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land, and hereby warrants the title to said land and will defend the same against the lawful claims of all persons claiming by, through or under the said grantor.

IN WITNESS WHEREOF, the said grantor has hereunto set their hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Concept Construction of North Florida, Inc.

2 President

Jaci Landsy Witness Traci LANDRY

STATE OF FLORIDA COUNTY OF COLUMBIA

I HEREBY CERTIFY that on this day, before me, an officer duly authorized in the State aforesaid and in the county aforesaid, to take acknowledgments, personally appeared Brian S. Crawford, well known to be the PRESIDENT, respectively of Concept Construction of North Florida, Inc., a Florida Corporation named as grantor in the foregoing deed, and that they severally acknowledged executing the same in the presence of two subscribing witnesses freely and voluntarily under authority duly vested.

WITNESS my hand and official seal in the County and State last aforesaid this day of 2008.

NOTARY PUBLIC
My Commission empires

DONNA COX Notary Public, State of Florida My Comm. Expires Jan. 16, 2010 Comm. No. DD 507061

F:\glEBEIG\Deed-Corporate15845.duc

04/21/084:08 PM



# Columbia County Tax Collector

Last Update: 6/25/2009 10:47:28 AM EDT

Site Provided by... governmax.com T1.11

#### Tax Record



Account Number 1 of 1

#### Details

#### Tax Record

» Print View Legal Desc. Appraiser Data Tax Payment Payment History Print Tax Bill New!

#### Searches

#### **Account Number**

GEO Number Owner Name Property Address Certificate NEW! Mailing Address

#### Site Functions

Disclaimer

#### Tax Search

Local Business Tax Tax Sale List Contact Us County Login Home

Ad Valorem Taxes and Non-Ad Valorem Assessments

The information contained herein does not constitute a title search and should not be relied on as such.

Account Number	Tau T	Control of the Contro
	Тах Туре	Tax Year
R02911-313	REAL ESTATE	2008

GIEBEIG PETER W P O BOX 1384

LAKE CITY FL 32056

Property Address 170 VANN CT SW

GEO Number

164511-02911-313

Assessed Value	Exempt Amount	Taxable Value
\$38,000.00	2	Taxable value
100,000.00	See Below	See Below

Exemption Detail

Millage Code

Escrow Code

NO EXEMPTIONS

002 Legal Description (click for full description)

11-4s-16 0000/0000 .51 Acres LOT 13 MAY-FAIR S/D UNIT 3. WD 1085-739, WD 1149-363

Α	d Valoren	Taxes		
Taxing Authority	Rate	Exemption Amount	Taxable Value	Taxes Levied
BOARD OF COUNTY COMMISSIONERS COLUMBIA COUNTY SCHOOL BOARD	7.8910	0	\$38,000	\$299.86
DISCRETIONARY	0.7480	0	\$38,000	\$28.42
LOCAL	5.2220	0	\$38,000	\$198.44
CAPITAL OUTLAY	1.7500	0	\$38,000	\$66.50
SUWANNEE RIVER WATER MGT DIST	0.4399	0	\$38,000	\$16.72
LAKE SHORE HOSPITAL AUTHORITY	2.0160	0	\$38,000	\$76.61
COLUMBIA COUNTY INDUSTRIAL	0.1240	0	\$38,000	\$4.71

Total Millage	W. P		
Total Millage	18.1909	Total Taxes	\$691.26
			4031.20

# Non-Ad Valorem Assessments

Code Levying Authority FFIR FIRE ASSESSMENTS

Amount \$69.58

Total Assessments \$69.58 Taxes & Assessments \$760.84

If Paid By	Amount Due
	\$0.00

Date Paid	Transaction	Receipt	Item	Amount Paid		
3/30/2009	PAYMENT	2604126.0026	2008	The Property of the Park of th		
		2001120.0020	2008	\$760.84		

Prior Years Payment History

	Prior Year Taxes Due
NO DELINQUENT TAXES	

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

# Florida Department of Community Affairs Residential Performance Method A

Project Name: Mayfair Lot 13 St Johns M Street: City, State, Zip: Lake City , FI , 32055- Owner: Design Location: FL, Gainesville	Model	Builder Name: T. Geibeig Permit Office: Permit Number: Jurisdiction:	
A 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		9. Wall Types a. Frame - Wood, Exterior b. Frame - Wood, Adjacent c. N/A d. N/A 10. Ceiling Types a. Under Attic (Vented) b. N/A c. N/A 11. Ducts a. Sup: Attic Ret: Attic AH: Interior Su 12. Cooling systems a. Central Unit 13. Heating systems a. Electric Heat Pump 14. Hot water systems a. Electric b. Conservation features None 15. Credits	Insulation Area R=13.0 1164.00 ft² R=13.0 160.00 ft² R= ft² R= ft² Insulation Area R=30.0 1608.00 ft² R= ft² R= ft² PR= ft² Cap: 36 kBtu/hr SEER: 14  Cap: 36 kBtu/hr HSPF: 7.7  Cap: 50 gallons EF: 0.92
Glass/Floor Area: 0.067	Total As-Built Modified Total Baseline	Loads: 27.88 Loads: 34.16	PASS
I hereby certify that the plans and specificathis calculation are in compliance with the Code.  PREPARED BY: DATE: Local Code.  I hereby certify that this building, as design with the Florida Energy Code.  OWNER/AGENT: DATE:	Florida Energy	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.  BUILDING OFFICIAL: DATE:	

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with N1110.A.3.

*					P	ROJECT	0						
Owner # of Ur Builder	nits: Name: Office: ction: Type: xisting:	Mayfair Lo FLAsBuilt 1 T. Geibeig Single-fan New (Fron	) nily		Bedrooms: Bathrooms: Conditioned A Total Stories: Worst Case: Rotate Angle: Cross Ventilati	1 No 0 ion:	8		Adress Lot # SubDivis PlatBoo Street: County:	sion: k:	Columb Lake C	oia	
					C	CLIMATE							
<b>/</b>	De	sign Location	1	ΓMY Site	IECC Zone	Design 97.5 %	Temp 2.5 %	Int Des Winter	ign Temp Summer	Heati Degree		esign oisture	Daily Tem Range
_	FL	, Gainesville	FL_GAIN	IESVILLE_RE	GI 2	32	92	75	70	1305	.5	51	Mediun
					F	LOORS							
$\sqrt{}$	#	Floor Type		Per	meter	R-Valu	ie	Area		The second	Tile	Wood	Carpet
	1	Slab-On-Gra	ide Edge Insulat	io 20	6 ft	0		1608 ft²			0.2	0.2	0.6
				- ola masana		ROOF	***************************************						
<b>√</b>	#	Туре	Ma	terials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch		
	1	Hip	Composi	tion shingles	1798 ft²	0 ft²	Medium	0.96	No	0	26.6 de	g	
						ATTIC							
$\checkmark$	#	Туре		Ventilation	Ve	ent Ratio (1 ir	1)	Area	RBS	IRCC			
	1	Full attic		Vented		300	10	608 ft²	N	N			
					C	CEILING						- Horado	
$\sqrt{}$	#	Ceiling Typ	е		R-Va	alue	Are	a	Framin	g Frac	Т	russ Ty	pe
1000	1	Under Attic	(Vented)		30		1608	ft²	0.1	1		Wood	
					,	WALLS							
V	#	Ornt	Adjacent To	Wall Type			Cavit R-Val	ty ue Are	Shea a R-V	athing alue	Framing Fraction		Solar Absor.
	1	E	Exterior	Frame - Wo	od		13	421.3			0.23		0.75
	2	N	Exterior	Frame - Wo	od		13	240.6	7 ft²		0.23		0.75
	3	w	Exterior	Frame - Wo	od		13	261.3	3 ft²		0.23		0.75
	4	S	Exterior	Frame - Wo	od		13	240.6	7 ft²		0.23		0.75
	5	W	Garage	Frame - Wo	od		13	160	ft²		0.23		0.01

						D	oors			Children Addisonal		With the second	
$\vee$	#	Ornt	Do	or Type				Storm	s	U-	Value	Area	-
	1	E	Ins	ulated				None	1	C	0.46	40 ft²	-
	2	W	Ins	sulated				None	1	C	).46	33.33 ft <sup>2</sup>	
		Window	orientation	below is a	as entered. A		NDOWS		te angle	shown in "P	roject" section	above.	
<b>/</b>	# (	Ornt Fran	ne Pa	anes	NFRC	U-Factor	SHGC	Storms	Area		hang Separation	Int Shade	Screening
	1	E Met	al Single	(Tinted)	Yes	0.55	0.6	N	30 ft <sup>2</sup>	1 ft 6 in	1 ft 6 in	HERS 2006	None
	2	E Met	al Single	(Tinted)	Yes	1.2	0.7	N	20 ft <sup>2</sup>	1 ft 6 in	1 ft 6 in	HERS 2006	None
	3	N Met	al Single	(Tinted)	Yes	1.2	0.7	N	5 ft²	1 ft 6 in	1 ft 6 in	HERS 2006	None
	4	N Met	al Single	(Tinted)	Yes	1.2	0.7	N	8 ft <sup>2</sup>	1 ft 6 in	1 ft 6 in	HERS 2006	None
	5	W Met	al Single	(Tinted)	Yes	1.2	0.7	N	20 ft <sup>2</sup>	1 ft 6 in	1 ft 6 in	HERS 2006	None
	6	W Meta	al Single	(Tinted)	Yes	1.2	0.7	N	25 ft <sup>2</sup>	1 ft 6 in	1 ft 6 in	HERS 2006	None
					II	NFILTRATI	ON & V	ENTING					
<b>/</b>	Method	d		SLA	CFM 50	ACH 50	ELA	EqLA			Ventilation Exhaust CFM		Fan Watts
	Defaul			0.00036	1518	7.08	83.4	156.8		0 cfm	0 cfm	0	0
						GA	RAGE			ode who we			
$\vee$	#	Floor	Area	C	eiling Area	Exposed	Wall Peri	meter	Avg. V	Vall Height	Exposed	Wall Insulation	
	1	400	ft²		400 ft <sup>2</sup>		64 ft			8 ft		11	
						COOLIN	IG SYS	TEM					
	#	System Typ	e		Subtype			Efficiency		Capacity	Air Flow	SHR	Ductless
	1	Central Uni	t		None			SEER: 14		86 kBtu/hr	1080 cfm	0.75	False
						HEATIN	G SYS	ΓEM					
	#	System Typ	e		Subtype			Efficiency		Capacity	Ductless		
	1	Electric He	at Pump		None		1	HSPF: 7.7	;	36 kBtu/hr	False		
						HOT WAT	ER SY	STEM					
$\sqrt{}$	#	System T	уре			EF	Cap	)	Use	SetPnt		Conservation	
	1	Electric				0.92	50 ga	al 6	0 gal	120 deg		None	
					SOI	AR HOT V	VATER	SYSTEM	Л			-Walana - Walana - I	
V	FSE0 Cert		ny Name			System Mo	del#	Colle	ector M			Storage Volume F	EF
	None	None							311117		ft²		

							DUCTS							
<b>/</b>	#	S Location	upply R-Value Are	ea l	Re _ocation	turn Area	Leaka	ge Type	Air Handler	CFM	25	Percent Leakage	QN	RLF
	1	Attic	6 321.	6 ft	Attic	80.4 ft <sup>2</sup>	Default	Leakage	Interior					
						TEM	PERATU	RES						
Program	able Thern	nostat: Y			C	eiling Fans	:							
Cooling Heating Venting	[X] Jan [X] Jan [X] Jan	X Fe X Fe X Fe	b [X] Mar b [X] Mar b [X] Mar		Apr Apr Apr	[X] May [X] May [X] May	[X] Jun [X] Jun [X] Jun	[X] Jul [X] Jul [X] Jul	[X] Aug [X] Aug [X] Aug	[X] Sep [X] Sep [X] Sep		[X] Oct [X] Oct [X] Oct	X Nov X Nov X Nov	X Dec X Dec X Dec
Thermostat	Schedule	: HERS 2	2006 Reference	e				Hou	ırs					
Schedule T	уре		1	2	3	4	5	6	7	8	9	10	11	12
Cooling (W	D)	AM PM	78 80	78 80	78 78	78 78	78 78	78 78	78 78	78 78	80 78	80 78	80 78	80 78
Cooling (W	EH)	AM PM	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78
Heating (W	(D)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
Heating (W	EH)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66

# **Code Compliance Cheklist**

# Residential Whole Building Performance Method A - Details

ADDRESS:	
----------	--

PERMIT #:

Lake City, FI, 32055-

#### INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	N1106.AB.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	N1106.AB.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	N1106.AB.1.2.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	N1106.AB.1.2.3	Between walls & ceilings; penetrations of ceiling plane to top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	N1106.AB.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	N1106.AB.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	N1106.AB.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

#### OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N112.ABC.3. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	N1112.AB.2.3	Spas & heated pools must have covers (except solar heated).  Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.  Heat pump pool heaters shall have a minimum COP of 4.0.	
Shower heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems N1110.AB		All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB.  Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	N1104.AB.1 N1102.B.1.1	Ceilings-Min. R-19. Common walls-frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

#### ESTIMATED ENERGY PERFORMANCE INDEX\* = 82

The lower the EnergyPerformance Index, the more efficient the home.

2. 3.	New construction or exist Single family or multiple to Number of units, if multip	family	Single	From Plans) -family	9	Wall Types a. Frame - Wood, Exterior b. Frame - Wood, Adjacent c. N/A	Insulation R=13.0 R=13.0	Area 1164.00 ft² 160.00 ft² ft²
335	Number of Bedrooms 3				d. N/A	R=	ft²	
6.	Is this a worst case? Conditioned floor area (ft		No 1608	*.	10	Ceiling Types     a. Under Attic (Vented)     b. N/A	Insulation R=30.0 R=	Area 1608.00 ft <sup>2</sup> ft <sup>2</sup>
7.	Windows** a. U-Factor: SHGC:	Description Sgl, U=1.20 SHGC=0.70		Area 78.00 ft²	1:	c. N/A 1. Ducts	R=	ft²
	b. U-Factor: SHGC:	Sgl, U=0.55 SHGC=0.60		30.00 ft <sup>2</sup>	12	<ul><li>a. Sup: Attic Ret: Attic AH: Interior</li><li>2. Cooling systems</li></ul>	Sup. R= 6, 321.	.6 ft²
	c. U-Factor: SHGC:	N/A		ft²		a. Central Unit	Сар	sEER: 14
	d. U-Factor: SHGC:	N/A		ft²	13	Heating systems     Electric Heat Pump	0	. 26 hDb./b-
	e. U-Factor: SHGC:	N/A		ft²		a. Electric Heat Fullip	Сар	: 36 kBtu/hr HSPF: 7.7
8.	Floor Types a. Slab-On-Grade Edge li b. N/A c. N/A	nsulation	Insulation R=0.0 R= R=	Area 1608.00 ft² ft² ft²	14	Hot water systems     a. Electric      b. Conservation features     None	Сар	: 50 gallons EF: 0.92
					15	5. Credits		Pstat

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature:	Date:	GRE
Address of New Home:	City/FL Zip:	11
		- A



\*Note: The home's estimated Energy Performance Index is only available through the EnergyGauge USA - FlaRes2008 computer program. This is not a Building Energy Rating. If your Index is below 100, your home may qualify for incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at (321) 638-1492 or see the Energy Gauge web site at energygauge.com for information and a list of certified Raters. For information about Florida's Energy Efficiency Code for Building Construction, contact the Department of Community Affairs at (850) 487-1824.

\*\*Label required by Section 13-104.4.5 of the Florida Building Code, Building, or Section B2.1.1 of Appendix G of the Florida Building Code, Residential, if not DEFAULT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: Sw Van Court Project Name: Lot#13 - Mayfair

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Therma-Tru	6'8 Door	NOA 01-0828.0
2. Sliding	Mitomo	Alum. Sliding class	03-1224.01
3. Sectional	Clopay	Sectional PG	05-1212.02
4. Roll up	- COPSO	30,10190	03-1212.02
5. Automatic			
6. Other			
B. WINDOWS			
Single hung	Mitome	Alum SH window	03-1215.02
Horizontal Slider	THITTE	main sh winge	00
3. Casement			
4. Double Hung	1		
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block	<del>                                     </del>		
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
Asphalt Shingles	Tanko	Paul	03-0620.01
2. Underlayments	icarrece	Cool	030020.01
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
Roofing Insulation			
10. Waterproofing	THE OFFICE PARTY SERVICES AND		,
11. Wood shingles /shakes			
12 Roofing Slate			

02/02/04 - 2 of 2

Location

Website: www.tlepermits.org

Effective April 1, 2004

Permit # (FOR STAFF USE ONLY)



STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT
Permit Application Number

Permit Application Number

PART II - SITE PLAN
Scale: Each block represents 5 feet and 1 inch = 50 feet.
Notes: Lot 13 Way face Ph 3
11-45-16-02911-313
Pete Giebeig (Courept Conss.)
Site Plan submitted by: Caluturand Finder
Plan Approved Not Approved Date 3/-28-38
By Columbia County Health Department

Nov. 06 07 12:04p

Lynch Well Drilling

386-752-1477

p.2

Water, Wells Pumps & Service Phone: (386) 752-6677 Fax: (386) 752-1477

# Lynch Well Drilling, Inc.

173 SW Young Place Lake City, FL 32025 www.lynchwelldrilling.com

November 6, 2007

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the above-referenced well:

Size of Pump Motor:

1 Horse Power

Size of Pressure Tank:

81-Gallon Bladder Tank

Cycle Stop Valve Used:

No

Should you require any additional information, please contact us.

Sincerely,

Linda Newcomb

Lynch Well Drilling, Inc.

Linda Newcomb

# **Columbia County Building Department Culvert Permit**

# Culvert Permit No. 000001734

DATE 07/0	01/2009 F	PARCEL ID # 11-4S-1	6-02911-316		
APPLICANT	B. TRENT GIEBEIG		PHONE	386.397.0545	
ADDRESS _	697 SE HOLLY TERRACE	3	LAKE CITY	FL	32025
OWNER PI	ETE GIEBEIG		PHONE	386.752.7968	
ADDRESS _1	23 SW LUCILLE CT		LAKE CITY	FL	32024
CONTRACTO	R B. TRENT GIEBEIG		PHONE	386.752.7968	
LOCATION O	F PROPERTY 90-W TO	SR.247-S,TL TO SW MAY	-FAIR LN,TR TO I	LUCILLE CT,TR	
1ST LOT ON R.					
SUBDIVISION	I/LOT/BLOCK/PHASE/U	NIT MAY-FAIR		16	3
SIGNATURE	/ FINT &	MAR			
	INSTALLATION RE	QUIREMENTS			
X	Culvert size will be 18 in driving surface. Both en thick reinforced concrete	ds will be mitered 4 foot	total lenght of 3 t with a 4 : 1 slop	32 feet, leaving 24 be and poured wit	feet of th a 4 inch
	<ul> <li>b) the driveway to be services</li> <li>b) Turnouts shall be concrete or paved d</li> </ul>	Turnouts will be required and existing drived served will be paved or increte or paved a minimal riveway, whichever is graphy and or concreted turn	vay turnouts are formed with con num of 12 feet v eater. The widtl	crete. wide or the width	of the the
	Culvert installation shall	ll conform to the approv	ved site plan sta	ndards.	
	Department of Transpor	rtation Permit installation	on approved sta	ndards.	
	Other				
	-				

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Inst:200912011031 Cate:7/2/2009 Time:9:15 AM \_\_\_\_\_\_CC, P.DeWitt Cason, Columbia County Page 1 of 1 3:1176 P:1084

NOTICE OF COMMERCE	NOTICE	OF	CON	MIN	EN	CEM	ENT
--------------------	--------	----	-----	-----	----	-----	-----

NOTICE OF COMMENCE	FIAICIAI	Co	unty Clerk's Office Stamp or Seal	Š.
Tax Parcel Identification Numbe	1 <u>1-4s-16-02911-</u>			
THE UNDERSIGNED hereby g Florida Statutes, the following in	ives notice that improvements was a formation is provided in this No	vill be made to certain real propo OTICE OF COMMENCEME	rty, and in accordance with Secti NT.	on 713.13 of the
1. Description of property (legal	description): Lot # 13	May Fair Unit	III	
a) Street (job) Address	170 SW Van Col	ort Lake City. F	1. 32024	
2. General description of improv	ements: Construction	on of Single Fam	nily Residence	
3. Owner Information a) Name and addre₩.	eter W. Giebeig	P.O. Box 1384	Lake City, Fl.	32056
<ul> <li>b) Name and address of</li> </ul>	of fee simple titleholder (if other	r than owner)		
	Fee Simple		The second second	
4. Contractor Information	march Cichoia	Construction Ir	697 SE Holl	v Terrace
a) Name and address:	Trent Greberg	Constituection, in	Opt.) Lake City,	FL. 32025
b) Telephone No.:	386-752-0791	Pax 140. (C	opt.) Lake Cicy	12. 320
5. Surety Information	N/A			(4)
b) Amount of Bond:		Fay No. (	Opt.)	
	(A. 41.47	rax No. (	Opt.)	
6. Lender	:N/A			
b) Phone No				
7 Identity of person within the	State of Florida designated by or	wner upon whom notices or other	r documents may be served:	
7. Identity of person within the s	Male of Florida designated by o	wher apon whom honces or oure.	. adottino may oc our roa	
b) Telephone No:	N/A	Fax No. (C	Opt.)	
o) recommend				
8 In addition to himself owner	designates the following person	to receive a copy of the Lienor's	Notice as provided in Section 7	13.13(1)(b),
Florida Statutes:	designates the lone wing person	to reserve a cop, or an energy		
	N/A			
b) Telephone No:		Fax No. (	Opt.)	
is specified):	Y PAYMENTS MADE BY T	HE OWNER AFTER THE EX	of recording unless a different PIRATION OF THE NOTICE R 713, PART I, SECTION 713.	OF
STATUTES, AND CAN REST COMMENCEMENT MUST I TO OBTAIN FINANCING, C	ULT IN YOUR PAYING TWI BE RECORDED AND POSTE ONSULT YOUR LENDER O	CE FOR IMPROVEMENTS T ED ON THE JOB SITE BEFORE	TO YOUR PROPERTY; A NO RE THE FIRST INSPECTION COMMENCING WORK OR	TICE OF . IF YOU INTEND
YOUR NOTICE OF COMME	INCEMENT.	^		F.S.
STATE OF FLORIDA		1) 1	91	
COUNTY OF COLUMBIA		10. 12 11.	And the second	
		Signature of Owner or Own	ner's Authorized Office/Director/Pr	artner/Manager
		Peter W G	ei-beia	
.5		Print Name	,,,,,,	
	\$20 61 to 400 to 1	المداد	Ti. /.	NG
The foregoing instrument was ack	nowledged before me , a Florida N	Notaty, this day of	July ,20_	0 / by:
	Di	wner	(type of authority, e.g. office	er trustee attorney
fact) for Peter W.	Giebeig		party on behalf of whom instrum	
V	/		T WARE TOLA	. 1
Personally Known X OR Prod	duced Identification Type _		ELAINE K. TOLA	
C1.	1) Tol.	*	EXPIRES: C soper 2,	
Notary Signature Cllud	un. John	Notary Stamp or Seal:	Bonded Thru Notary Public Un	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			Destar con compression property and the control of	1000
		AND		
11. Verification pursuant to Se	ction 92.525, Florida Statutes.	Under penalties of perjury, I d	eclare that I have read the foreg	oing and that the
facts stated in it are true to	the best of my knowledge and	d belief.	· CH-	The second section of the sect
	100 miles	Verte le	. 10	
		Signature of Natural Per	son Signing (in line #10 above.)	
			ur er ur ur ur ur er	



Land Surveyors and Mappers

# BRITT SURVEYING & ASSOCIATES

830 West Duval Street • Lake City, FL 32055 Phone (386) 752-7163 • Fax (386) 752-5573

07/06/09 (09/21/09 revised)

L-19971

To Whom It May Concern:

C/o: Trent Giebieg

Re: Lots 13 in May-Fair Unit 3 / 11-4S-16-02911-313

The elevation of the top of the batter boards for the residence's finished floor is 164.91 feet. The minimum finished floor elevation is 164.00 feet as per the plat of record. The highest adjacent grade is 162.8 feet. The lowest adjacent grade is 162.6 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt PLS #5757

> 0:K. S. Busher 0:a. S. Phrog



# COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST REQUIRMENTS

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE RESIDENTIAL 2007 ONE (1) AND TWO (2) FAMILY DWELLINGS

#### ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current 2007 FLORIDA BUILDING CODES RESIDENTIAL. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the FLORIDA BUILDING CODES RESIDENTIAL (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:

	APPLICANT – PLEASE CH	ECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Applicable	
		35-350-370-3	Yes	No	N/A
1	Two (2) complete sets of plans conta	aining the following:			
2	All drawings must be clear, concise,	drawn to scale, details that are not used shall be marked void			
3	Condition space (Sq. Ft.)	Total (Sq. Ft.) under roof	11111111	HHHHH	IIIII

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

	sions of all building set backs	COST		
6 Locati				
well ar	on of all other structures (include square foots and septic tank and all utility easements.	tage of structures) on parcel, existing	g or proposed	
7 Provid	e a full legal description of property.	STE CO	/	

STAMINER

Items to Include-Each Box shall be

Circled as

# Wind-load Engineering Summary, calculations and any details required

1 2 2 2 2 2 2	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Each C	to Inclui Box shall ircled as olicable	
8	Plans or specifications must show compliance with FBCR Chapter 3	ШШ	IIIII	IIIIII
		YEŞ	NO	N/A
9	Basic wind speed (3-second gust), miles per hour	/		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	/		
11	Wind importance factor and nature of occupancy	1		
12	The applicable internal pressure coefficient, Components and Cladding	1		
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.			

# **Elevations Drawing including:**

14	All side views of the structure	/.	
15	Roof pitch	1	
16	Overhang dimensions and detail with attic ventilation	1	
17	Location, size and height above roof of chimneys	V	
18	Location and size of skylights with Florida Product Approval	1	
18	Number of stories	1	
20A	Building height from the established grade to the roofs highest peak	Ţ	

# Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies		1		
21	Raised floor surfaces located more than 30 inches above the floor or grade				
22	All exterior and interior shear walls indicated				
23	Shear wall opening shown (Windows, Doors and Garage doors)				
24	Emergency escape and rescue opening shown in each bedroom (net clear opening shown)				
25	Safety glazing of glass where needed		r		
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FBCR)	_		_	_
27	Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FBCR SECTION 311)				
28	Identify accessibility of bathroom (see FBCR SECTION 322)				

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plan (see Florida product approval form)

# GENERAL REQUIREMENTS: Items to Include-APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL Each Box shall be Circled as Applicable **FBCR 403: Foundation Plans** YES NO N/A Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing. All posts and/or column footing including size and reinforcing 31 Any special support required by soil analysis such as piling. Pound Per Square Foot 32 Assumed load-bearing valve of soil 33 Location of horizontal and vertical steel, for foundation or walls (include # size and type) FBCR 506: CONCRETE SLAB ON GRADE 34 Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed) 35 | Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports FBCR 320: PROTECTION AGAINST TERMITES Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls) 37 Show all materials making up walls, wall height, and Block size, mortar type 38 Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

# Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers Girder type, size and spacing to load bearing walls, stem wall and/or priers Attachment of joist to girder Wind load requirements where applicable Show required under-floor crawl space Show required amount of ventilation opening for under-floor spaces Show the required access opening to access to under-floor spaces Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &

Floor Framing System: First and/or second story

	intermediate of the areas structural panel sheathing	
49	Show Draftstopping. Fire caulking and Fire blocking	
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 309	
51/	Provide live and dead load rating of floor framing systems (psf).	

# FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Barrier .	ach C	to Inclusion to Inclusion to Include the Included Asserted to Include the Inclusion to Inclusion	ll be
		Yl	ES	NO	N/A
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls				
53	Fastener schedule for structural members per table FBCR 602.3 are to be shown	- /	(2)		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing				
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems				
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCR Table 502.5 (1)				
57	Indicate where pressure treated wood will be placed				
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas				
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail		`		

# **FBCR**:ROOF SYSTEMS:

	<u> </u>		
Include a layout and truss details, signed and sealed by Florida Professional Engineer			
Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters			
Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details			
Provide dead load rating of trusses			
	Truss design drawing shall meet section FBCR 802.10 Wood trusses Include a layout and truss details, signed and sealed by Florida Professional Engineer Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details Provide dead load rating of trusses	Include a layout and truss details, signed and sealed by Florida Professional Engineer  Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters  Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Include a layout and truss details, signed and sealed by Florida Professional Engineer  Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters  Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details

# FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing	)	
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating		
67	Valley framing and support details		
68	Provide dead load rating of rafter system		

# FBCR Table 602,3(2) & FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	/	

# FBCR ROOF ASSEMBLIES FRC Chapter 9

		 	-
71	Include all materials which will make up the roof assembles covering		-
72	Submit Florida Product Approval numbers for each component of the roof assembles covering		

# FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	THE R. P. LEWIS CO., LANSING, MICH., LANSING, MICH., LANSING, LANS	ach E	to Includ Box shall reled as plicable	be
18 18 18 19 19 19 19 19 19 19 19 19 19 19 19 19	YE	S	NO	N/A
73 Show the insulation R value for the following areas of the structure				
74 Attic space				
75 Exterior wall cavity				
76 Crawl space				
77 Submit two copies of a Manual J sizing equipment or equivalent computation study 78 Exhaust fans locations in bathrooms				
	$\rightarrow$	_		
79 Show clothes dryer route and total run of exhaust duct		-		
Plumbing Fixture layout shown  80 All fixtures waste water lines shall be shown on the foundation plan	ľ	71		
	+			
81 Show the location of water heater	-			
not a notation to the second s				
Private Potable Water				
Private Potable Water  82 Pump motor horse power		`		
		`		

# Electrical layout shown including

85	Switches, outlets/receptacles, lighting and all required GFCI outlets identified	
86	Ceiling fans	
87	Smoke detectors & Carbon dioxide detectors	
88	Service panel, sub-panel, location(s) and total ampere ratings	
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	

	4	
90	Appliances and HVAC equipment and disconnects	
91	Arc Fault Circuits (AFCI) in bedrooms	

<u>Disclosure Statement for Owner Builders</u> If you as the applicant will be acting as an owner builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.

# **Notice Of Commencement**

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as
。 11、 ACC 2004 AGE ACC 2004 2004 2004 2004 2004 2004 2004 20	Applicable

# THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

92	Building Permit Application A current Building Permit Application form is to be completed and submitted for all residential projects	1	
93	Parcel Number The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested		
94	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058		
95	City of Lake City A permit showing an approved waste water sewer tap		
96	Toilet facilities shall be provided for all construction sites		
97	<b>Town of Fort White</b> (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.		
98	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations		
99	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the base flood elevation (100 year flood) has been established		
100	A development permit will also be required. Development permit cost is \$50.00		
101	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00).  All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.		
102	911 Address: If the project is located in an area where a 911 address has not been issued, then application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125		

YES

NO

N/A

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code, Building shall govern the administration and enforcement of the Florida Building Code, Residential.

Section 105 of the Florida Building Code defines the:

# Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

# Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

### Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

# If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

# New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became nu and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

# Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

# The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

When the submitted application is approved for permitting the applican will be notified by phone as to the date and time a building permit will b prepared and issued by the Columbia County Building & Zoning Department



Project Information for:

L278347

Builder:

GIEBEIG HOMES

Lot:

Subdivision: County:

**MAYFAIR** COLUMBIA

Truss Count:

28

Building Code:

Design Program: MiTek 20/20 6.3 FBC2004/TPI2002

Truss Design Load Information: Gravity: Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

> Brian T. Giebeig Florida Registered Residential Contractor License No. RR282811523 Address: Trent Giebeig Construction, Inc. 462 Southwest Fairlington Court Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

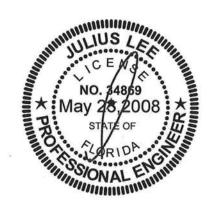
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date		
1	J1966915	CJ1	5/23/08		
2	J1966916	CJ3	5/23/08		
3	J1966917	CJ5	5/23/08		
4	J1966918	EJ3	5/23/08		
5	J1966919	EJ7	5/23/08		
6	J1966920	HJ4	5/23/08		
7	J1966921	HJ9	5/23/08		
8	J1966922	T01	5/23/08		
9	J1966923	T02	5/23/08		
10	J1966924	T03	5/23/08		
11	J1966925	T04	5/23/08		
12	J1966926	T05	5/23/08		
13	J1966927	T06	5/23/08		
14	J1966928	T07	5/23/08		
15	J1966929	T08	5/23/08		
16	J1966930	T09	5/23/08		
17	J1966931	T10	5/23/08		
18	J1966932	T11	5/23/08		
19	J1966933	T12	5/23/08		
20	J1966934	T13	5/23/08		
21	J1966935	T14	5/23/08		
22	J1966936	T15	5/23/08		
23	J1966937	T16	5/23/08		
24	J1966938	T17	5/23/08		
25	J1966939	T18	5/23/08		
26	J1966940	T19	5/23/08		
27	J1966941	T20	5/23/08		
28	J1966942	T21	5/23/08		

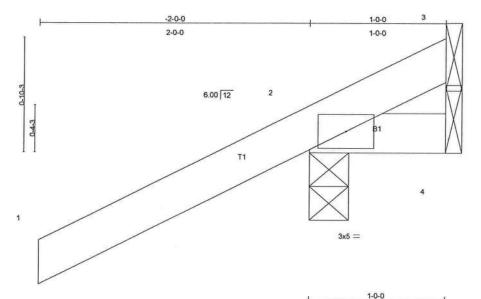




Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	CJ1	ROOF TRUSS	18	1	J1966915
			17955	1300	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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1-0-0 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.28 Vert(LL) -0.002 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.01 Vert(TL) -0.002 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)

Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.17

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3. Continued on page 2

Truse Design Engineer Flonda PE No. 34869 1109 Ceastal Bay Blvd Boynton Beach, FL 33435

May 23,2008

Scale: 1,5"=1"

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HILB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	CJ1	ROOF TRUSS	18	1	J1966915
3555 835535	1 2200				Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:44 2008 Page 2

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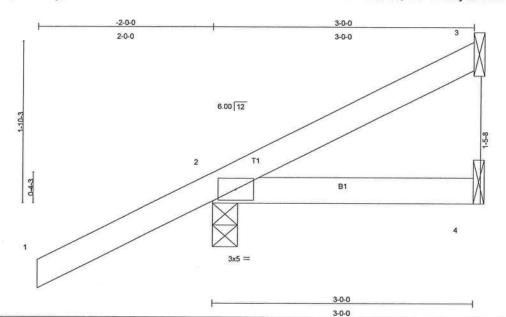
May 23,2008



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	CJ3	ROOF TRUSS	14	1	J1966916
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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LOADING (psf) SPACING 2-0-0 CSI DEFL L/d I/defl **PLATES GRIP** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.29 Vert(LL) 0.01 2-4 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.01 2-4 >999 240 BCLL 10.0 WB Rep Stress Incr YES 0.00 Horz(TL) -0.003 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD Structural wood sheathing directly applied or

3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc **BOT CHORD** 

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)

Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

**BOT CHORD** 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.15

#### **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

May 23,2008

Scale = 1:12.5

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	CJ3	ROOF TRUSS	14	1	J1966916
Taraki is solitar			1,743		Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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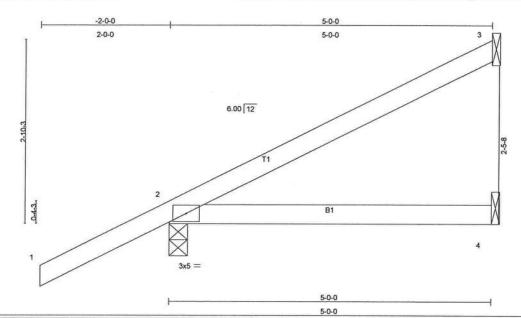
LOAD CASE(S) Standard

May 23,2008



Job Truss Truss Type Qty Ply GIEBEIG HOMES - ST. JOHNS W/RP J1966917 L278347 CJ5 **ROOF TRUSS** 14 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	8 8					Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

**BOT CHORD** 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.17

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

May 23,2008

Scale = 1:16.9

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	CJ5	ROOF TRUSS	14	1	J1966917
	Language Control		Referen	350	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

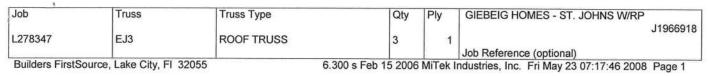
6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:45 2008 Page 2

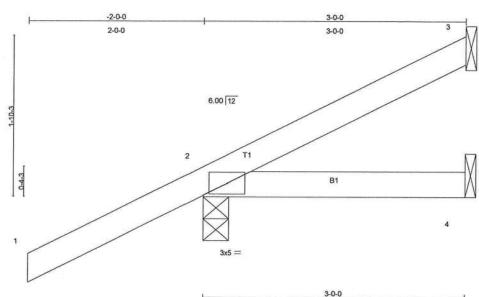
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 2-1895 1400 Caastal Bay Blyd

May 23,2008







LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mati	rix)	, ,					Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)

Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

#### **JOINT STRESS INDEX**

2 = 0.15

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

Truss Design Engineer Flonda FE No. 34868 1100 Geestal Bay Blyd Boynton Beach, FL 33435

May 23,2008

Scale = 1:12.5

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult 8CSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	EJ3	ROOF TRUSS	3	1	J1966918
	1503550				Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOAD CASE(S) Standard

Truse Design Engineer Florida PE No. 24868 1109 Coastal Bay Blyd Boynton Desch. FL 32435

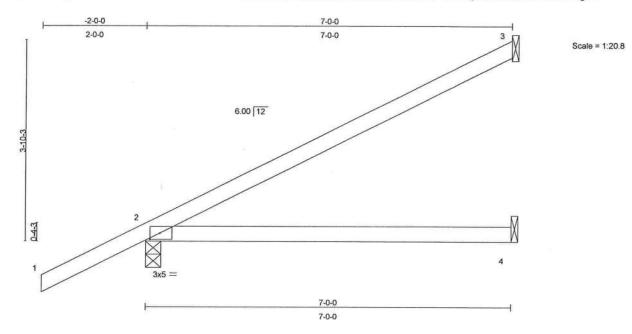
May 23,2008



Job ,	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - ST.JOHNS 4 W/P
L278347	EJ7	ROOF TRUSS	27	1	J1966919
					Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	sets (X,Y):	[2:0-2-4,0-1-8] SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0	Plates Increase Lumber Increase	1.25 1.25	TC BC	0.50 0.45	Vert(LL) Vert(TL)	0.33	2-4 2-4	>250 >501	360 240	MT20	244/190
BCLL 10.0 BCDL 5.0		* Rep Stress Incr YES Code FBC2004/TPI2002		WB 0.00 (Matrix)		Horz(TĹ)	-0.00	3	n/a	n/a	Weight: 26 lb	)
LUMBER TOP CHO BOT CHO	ORD 2X	4 SYP No.2 4 SYP No.2				BRACING TOP CHO	RD	Structu oc purl		sheathing	g directly applied	d or 6-0-0

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-94(load case 6), 2=-224(load case 6), 4=-65(load case 5) Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-131/54

**BOT CHORD** 2-4=0/0

JOINT STRESS INDEX

2 = 0.68

May 23,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job '	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - ST.JOHNS 4 W/P	
L278347	EJ7	ROOF TRUSS	27	1		J1966919
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri May 23 08:53:32 2008 Page 2

### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

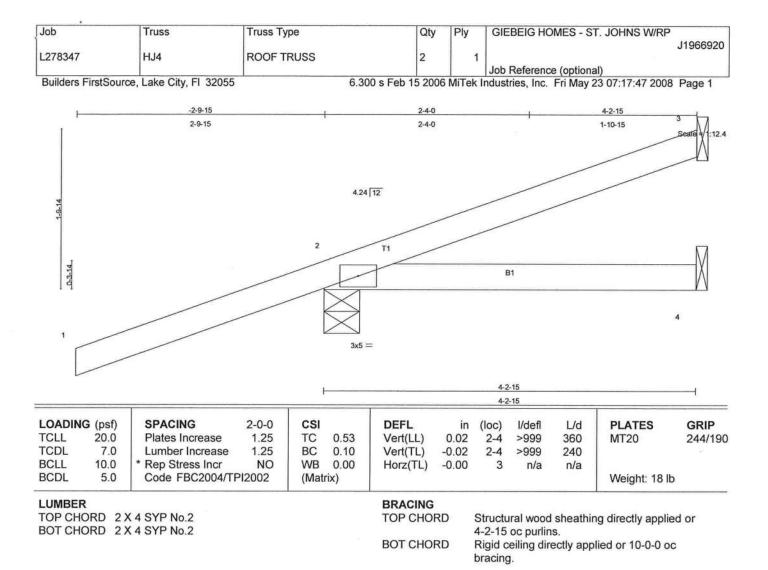
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 224 lb uplift at joint 2 and 65 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 34889 1100 Coastal Bay Blvd.





REACTIONS (lb/size) 3=15/Mechanical, 2=275/0-4-15, 4=14/Mechanical

Max Horz 2=98(load case 3)

Max Uplift 3=-6(load case 6), 2=-302(load case 3), 4=-41(load case 3)

Max Grav 3=32(load case 7), 2=275(load case 1), 4=54(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-37/10

BOT CHORD 2-4=0/0

## JOINT STRESS INDEX

2 = 0.13

# **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3, 302 lb uplift at joint 2 and 41 lb uplift at joint 4.

Julius Les Truse Design Engineer Flonda PE No. 34868 1109 Cassial Bay Blvd Boynton Beach, FL 20436

Continued on page 2

May 23,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component in that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	HJ4	ROOF TRUSS	2	1	J1966920
					Job Reference (optional)

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# **NOTES**

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=-0(F=5, B=5)-to-4=-11(F=-0, B=-0)



Job	Truss	Truss Typ	oe .	Qty	Ply	GI	EBEIG HO	OMES - ST.	JOHNS W/RP	- Caron Sanane - Su - Artan
L278347	НЈ9	ROOF TE	RUSS	7		1				J1966921
	A. 17.75				- 1		Reference	e (optional)	)	
Builders FirstSource	ce, Lake City, FI 32055		6.3	00 s Feb 15 200	06 MiTe	k Indus	stries, Inc.	Fri May 23	3 07:17:47 2008	Page 1
-2	-9-15		4-3-0					9-10-13		1
1 2	9-15		4-3-0					5-7-13	10-1	Scale = 124 2
										W
									//	
			4.2	4 12						
J			71.6	3x5	= _	/				
3-9-14				3						
				TI THE	5		_			
			//	W2		_		W1		
		//								
0.3-14	2				B1					
1.9										YY
1	3x6 =			7 2x4						6 5
				201						3x5 = <sub>0-3-8</sub>
	<del></del>		4-3-0					9-10-13		1
	I		4-3-0					5-7-13		
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in		I/defl	L/d	<b>PLATES</b>	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.60	Vert(LL)	0.09	6-7	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.11	6-7	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.36	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TF	12002	(Matrix)						Weight: 45 lb	
LUMBER				BRACING			-22	o to 550		
TOP CHORD 2				TOP CHOP	RD	Structi	ural wood	sheathing	directly applie	d or
BOT CHORD 2						6-0-0	oc purlins		18 18 18	
WEBS 2)	4 SYP No.3			BOT CHOP	RD	Rigid of bracing	ceiling dir	ectly applie	ed or 7-11-7 oc	•

REACTIONS (lb/size) 4=267/Mechanical, 2=453/0-4-15, 5=220/Mechanical

Max Horz 2=269(load case 3)

Max Uplift 4=-233(load case 3), 2=-399(load case 3), 5=-183(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-650/365, 3-4=-105/65

BOT CHORD

2-7=-538/603, 6-7=-538/603, 5-6=0/0

**WEBS** 

3-7=-89/186, 3-6=-627/559

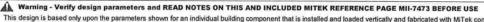
# JOINT STRESS INDEX

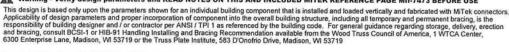
2 = 0.76, 3 = 0.25, 6 = 0.21 and 7 = 0.13

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 Ib uplift at joint 4, 399 lb uplift at joint 2 and 183 lb uplift at joint 5.

Continued on page 2







Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	HJ9	ROOF TRUSS	7	1	J1966921
					Job Reference (optional)

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# **NOTES**

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

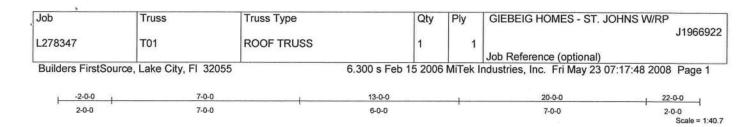
Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Les Truss Design Engineer Flonda PE No. 34869 1109 Coastal Bay Blvd Boynton Beson, FL 22425





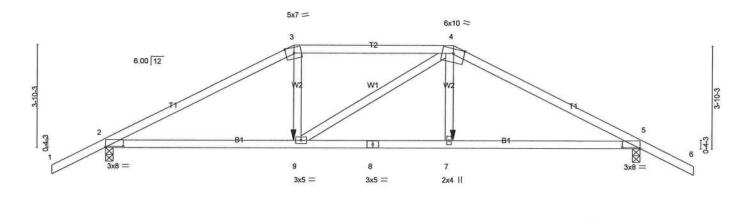


Plate Of	ffsets (X,)	(): [2:0-8-0,0-0-6], [5:	0-8-0,0-0-	6]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.09	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.19	7-9	>999	240	THE STATE OF THE S	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.07	5	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 88 lb		

13-0-0

6-0-0

			-	-	_
L	u	M	в	E	ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

## BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-8-5 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 8-0-10 oc

20-0-0

7-0-0

bracing.

REACTIONS (lb/size) 2=1381/0-3-8, 5=1381/0-3-8

Max Horz 2=77(load case 5)

7-0-0

7-0-0

Max Uplift 2=-474(load case 5), 5=-474(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2416/730, 3-4=-2101/687, 4-5=-2415/730, 5-6=0/47

**BOT CHORD** 2-9=-619/2080, 8-9=-590/2100, 7-8=-590/2100, 5-7=-586/2079

WEBS 3-9=-125/568, 4-9=-124/126, 4-7=-108/516

## JOINT STRESS INDEX

2 = 0.74, 3 = 0.80, 4 = 0.98, 5 = 0.74, 7 = 0.37, 8 = 0.77 and 9 = 0.41

# **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

May 23,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T01	ROOF TRUSS	1	1	J1966922
				1 100	Job Reference (optional)

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## **NOTES**

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 5.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

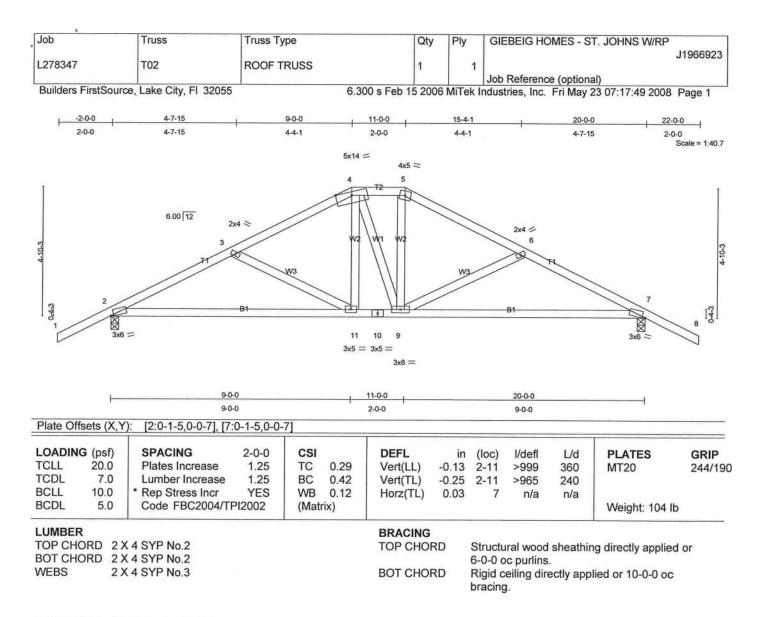
Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-10, 7-9=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 9=-411(F) 7=-411(F)





REACTIONS (lb/size) 2=747/0-3-8, 7=747/0-3-8

Max Horz 2=-89(load case 7)

Max Uplift 2=-229(load case 6), 7=-229(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1067/567, 3-4=-805/447, 4-5=-675/453, 5-6=-806/448,

6-7=-1067/567, 7-8=0/47

BOT CHORD 2-11=-346/898, 10-11=-143/674, 9-10=-143/674, 7-9=-346/898

WEBS 3-11=-258/229, 4-11=-61/210, 5-9=-61/209, 6-9=-257/228, 4-9=-104/109

## JOINT STRESS INDEX

2 = 0.85, 3 = 0.33, 4 = 0.34, 5 = 0.50, 6 = 0.33, 7 = 0.85, 9 = 0.64, 10 = 0.78 and 11 = 0.39

# **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

th any other

3) Provide adequate drainage to prevent water ponding.
4) \*This trues has been designed for a 10.0 per bottom chard live.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collyn leads on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T02	ROOF TRUSS	1	1	J1966923
100000	1.202				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:49 2008 Page 2

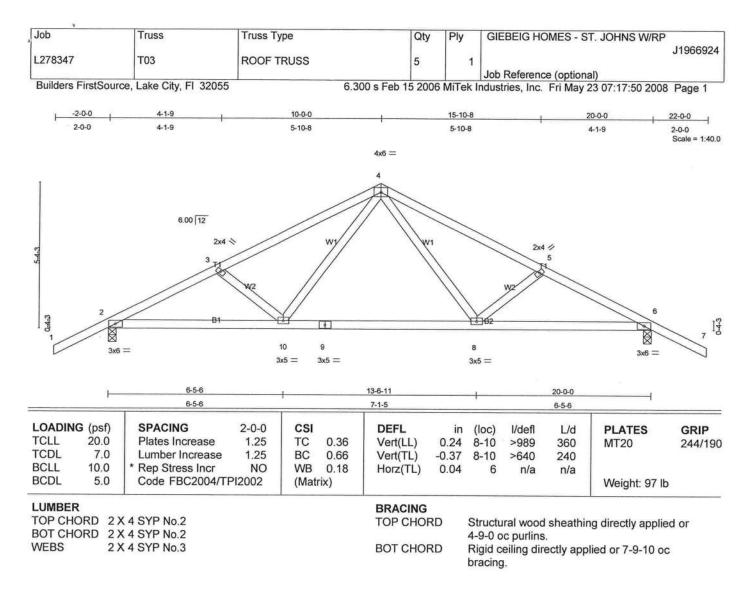
## **NOTES**

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2 and 229 lb

LOAD CASE(S) Standard





**REACTIONS** (lb/size) 2=960/0-3-8, 6=960/0-3-8

Max Horz 2=-95(load case 7)

Max Uplift 2=-292(load case 6), 6=-292(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1609/870, 3-4=-1438/813, 4-5=-1438/813, 5-6=-1609/870, 6-7=0/47

BOT CHORD 2-10=-621/1374, 9-10=-316/925, 8-9=-316/925, 6-8=-621/1374

WEBS 3-10=-216/200, 4-10=-263/547, 4-8=-263/547, 5-8=-216/200

# JOINT STRESS INDEX

2 = 0.69, 3 = 0.33, 4 = 0.82, 5 = 0.33, 6 = 0.69, 8 = 0.40, 9 = 0.64 and 10 = 0.40

# NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 6. Continued on page 2

Trues Design Engineer Florida FE No. 34869 1 100 Crestal Bay Blvd Boynton Beach, FL 33436

May 23,2008

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	тоз	ROOF TRUSS	5	1	J1966924
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:50 2008 Page 2

## **NOTES**

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

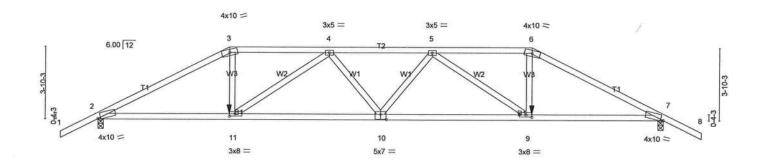
 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee Truse Design Engineer Florida PE No. 34868 1109 Gnastal Bay Sivi



Job	Tru	ISS	Truss Type	Qty	Ply	GIEBEIG HOMES - ST	r. Johns W/	
L278347	то	4	ROOF TRUSS	1	1			J196692
Duildon Fint	Caa 1 a1	- Cit. El 2000		0.000 5.1.45.000		Job Reference (optiona		
bullders First	Source, Lar	e City, FI 3205	00	6.300 s Feb 15 200	6 Milek I	ndustries, Inc. Fri May 2	3 07:17:51 2	008 Page 1
-2-0-0	3-4-3	7-0-0	12-3-11	17-9-5	23-	1-0 26-8-13	30-1-0	32-1-0
2-0-0	3-4-3	3-7-13	5-3-11	5-5-9	5-3	-11 3-7-13	3-4-3	2-0-0



		7-0-0		8-0-8			8-0-8		*	7-	0-0	
Plate Offsets (X,Y): [2:0-1-13,Edge], [7:0-1-13,Edge], [9:0-						1-8], [10:0-3-8	3,0-3-4]	, [11:0-3	3-8,0-1-8	1		
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.31	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.63	10-11	>565	240	5-100008.00008000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.97	Horz(TL)	0.20	7	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)		in suddinary			3.43.44	Weight: 139 lb	

LUMBER	
TOP CHORD	2

2 X 4 SYP No.2

7-0-0

BOT CHORD 2 X 4 SYP No.2

**WEBS** 

2 X 4 SYP No.3

## BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-6-7 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 5-2-2 oc

bracing.

**REACTIONS** (lb/size) 2=2084/0-3-8, 7=2084/0-3-8

Max Horz 2=-77(load case 6)

Max Uplift 2=-660(load case 5), 7=-660(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3937/1255, 3-4=-3479/1162, 4-5=-4602/1496, 5-6=-3479/1162.

6-7=-3937/1255, 7-8=0/47

**BOT CHORD** 2-11=-1083/3426, 10-11=-1482/4531, 9-10=-1464/4531, 7-9=-1049/3426 **WEBS** 

3-11=-388/1308, 4-11=-1374/541, 4-10=0/216, 5-10=0/216, 5-9=-1374/541,

15-0-8

6-9=-388/1308

# JOINT STRESS INDEX

2 = 0.83, 3 = 0.76, 4 = 0.45, 5 = 0.45, 6 = 0.76, 7 = 0.83, 9 = 0.82, 10 = 0.91 and 11 = 0.82

# NOTES

1) Unbalanced roof live loads have been considered for this design.

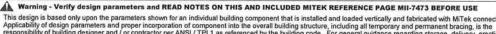
2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coliveden page 2

May 23,2008

Scale = 1:57.7







Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T04	ROOF TRUSS	1	1	J1966925
			12		Job Reference (optional)

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#### NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 660 lb uplift at joint 2 and 660 lb uplift at joint 7.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

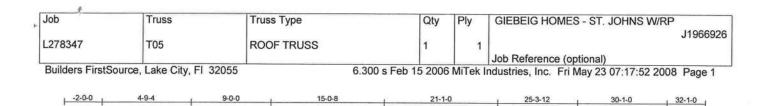
Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-10, 9-11=-22(F=-12), 7-9=-10

Concentrated Loads (lb)

Vert: 11=-411(F) 9=-411(F)

Julius Lee Trues Cesign Engineer Florida PE No. 34888 1109 Ceastal Bay Blvd





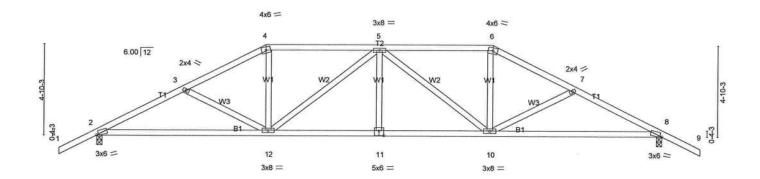
6-0-8

4-2-12

2-0-0 Scale = 1:57.7

6-0-8

15-0-8



		9-0-0		6-0	8-8	6-	-0-8			9-0-0	1	
Plate Of	ffsets (X,)	(): [2:0-1-5,0-0-7], [8:	0-1-5,0-0-	7], [11:0	0-3-0,0-3	-0]						
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.15	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.29	2-12	>999	240	AMMAN AND AND AND AND AND AND AND AND AND A	0000 0000 00 00 00 00
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	and the second of the second				10.000.0000.0	Weight: 154 lb	

L	U	N	1	В	Е	R

2-0-0

4.9.4

4-2-12

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

## BRACING

21-1-0

TOP CHORD

Structural wood sheathing directly applied or

4-8-4 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 7-8-15 oc

30-1-0

bracing.

REACTIONS (lb/size) 2=1069/0-3-8, 8=1069/0-3-8

Max Horz 2=89(load case 6)

9-0-0

Max Uplift 2=-267(load case 6), 8=-267(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1745/913, 3-4=-1515/815, 4-5=-1325/788, 5-6=-1325/788,

6-7=-1515/815, 7-8=-1745/913, 8-9=0/47

**BOT CHORD** 

2-12=-649/1494, 11-12=-617/1570, 10-11=-617/1570, 8-10=-650/1494

**WEBS** 

3-12=-208/198, 4-12=-138/397, 5-12=-401/176, 5-11=0/128, 5-10=-401/176,

6-10=-138/397, 7-10=-208/198

# JOINT STRESS INDEX

2 = 0.82, 3 = 0.33, 4 = 0.60, 5 = 0.56, 6 = 0.60, 7 = 0.33, 8 = 0.82, 10 = 0.56, 11 = 0.37 and 12 = 0.56

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T05	ROOF TRUSS	1	1	J1966926
		Noor mood			Job Reference (optional)

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## **NOTES**

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

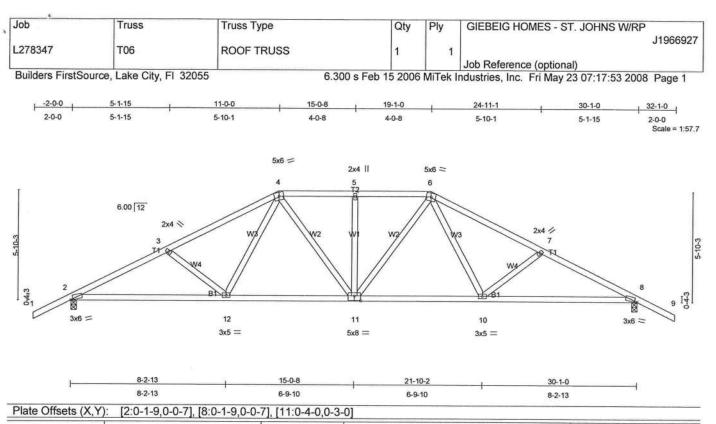
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2 and 267 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 24868 1109 Ceastal Bay Blvd Boynton Beach, Ft. 33435





LOADIN TCLL TCDL BCLL	G (psf) 20.0 7.0 10.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.31 0.38 0.11	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.10 -0.20 0.07	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)			 1110		Weight: 157 lb	

-	1	IN	A	D	D

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 22

2 X 4 SYP No.3

## BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-8-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 7-7-4 oc

bracing.

REACTIONS (lb/size) 2=1069/0-3-8, 8=1069/0-3-8

Max Horz 2=-101(load case 7)

Max Uplift 2=-280(load case 6), 8=-280(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1762/938, 3-4=-1531/851, 4-5=-1268/798, 5-6=-1268/798,

6-7=-1531/851, 7-8=-1762/938, 8-9=0/47

BOT CHORD 2-12=-675/1511, 11-12=-417/1186, 10-11=-417/1186, 8-10=-675/1511

WEBS 3-12=-269/264, 4-12=-116/322, 4-11=-78/263, 5-11=-190/87, 6-11=-78/263,

6-10=-116/322, 7-10=-269/264

# JOINT STRESS INDEX

2 = 0.75, 3 = 0.33, 4 = 0.50, 5 = 0.33, 6 = 0.50, 7 = 0.33, 8 = 0.75, 10 = 0.44, 11 = 0.34 and 12 = 0.44

## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T06	ROOF TRUSS	1	1	J1966927
	10,552			1.535	Job Reference (optional)

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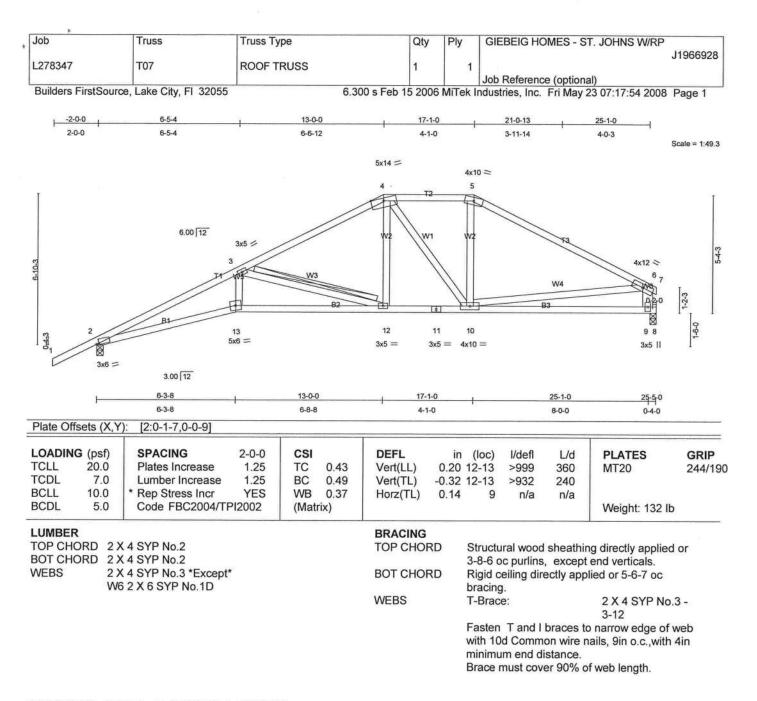
## NOTES

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 280 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Trues Cesian Engineer Flonda PE No. 24869 1109 Ceestal Bay Blvd





REACTIONS (lb/size) 2=917/0-3-8, 9=817/0-3-8

Max Horz 2=175(load case 6) Max Uplift 2=-268(load case 6), 9=-159(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2571/1373, 3-4=-1192/685, 4-5=-891/626, 5-6=-1096/609, 6-7=0/10.

6-9=-766/496

2-13=-1276/2285, 12-13=-1217/2161, 11-12=-468/1004, 10-11=-468/1004,

9-10=-272/372, 8-9=0/0

WEBS 3-13=-249/610, 3-12=-1207/779, 4-12=-182/371, 4-10=-297/99, 5-10=-25/227,

6-10=-136/540

Julius Lee Truss Design Engineer Florida PE No. 34868 1 109 Ceastal Bay Blvd Boymon Beach, FL 23426

# JOINT STRESS INDEX

2 = 0.78, 3 = 0.52, 4 = 0.80, 5 = 0.68, 6 = 0.77, 9 = 0.70, 10 = 0.34, 11 = 0.35, 12 = 0.39 and 13 = 0.72

Continued on page 2

**BOT CHORD** 



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	Т07	ROOF TRUSS	1	1	J1966928
					Job Reference (optional)

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### NOTES

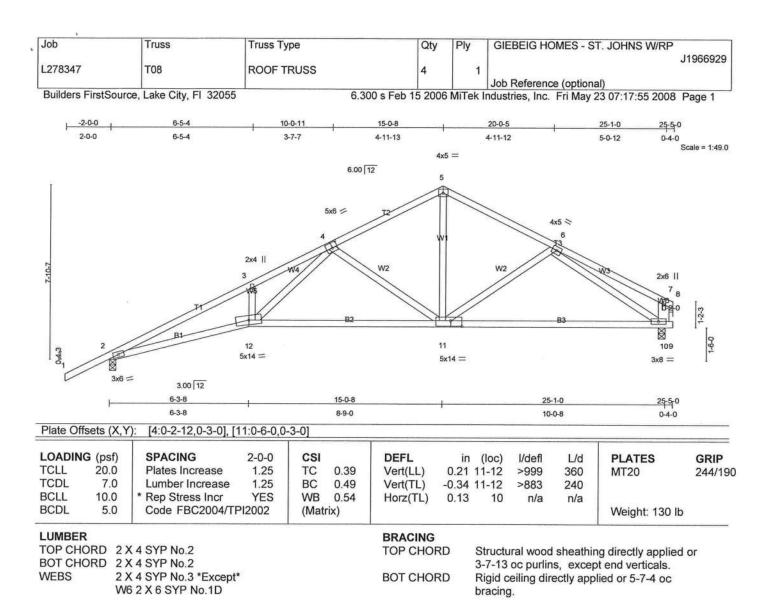
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2 and 159 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Coestal Bay Blvd Devices Design 1994





REACTIONS (lb/size) 2=917/0-3-8, 10=817/0-3-8

Max Horz 2=187(load case 6)

Max Uplift 2=-276(load case 6), 10=-171(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2537/1353, 3-4=-2492/1493, 4-5=-963/614, 5-6=-965/615,

6-7=-317/165, 7-8=0/10, 7-10=-261/204

BOT CHORD 2-12=-1252/2249, 11-12=-720/1325, 10-11=-485/896, 9-10=0/0

WEBS 3-12=-216/240, 4-11=-627/465, 5-11=-311/517, 6-11=-174/176, 6-10=-856/524,

4-12=-708/1197

# JOINT STRESS INDEX

2 = 0.79, 3 = 0.33, 4 = 0.76, 5 = 0.73, 6 = 0.36, 7 = 0.75, 10 = 0.77, 11 = 0.53 and 12 = 0.63

## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truse Design Engineer Flonds FE No. 24868 1 100 Castal Bay Blvd Boynton Beach, FL 33436

Continued on page 2

May 23,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T08	ROOF TRUSS	4	1	J1966929
		1,000	-		Job Reference (optional)

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# **NOTES**

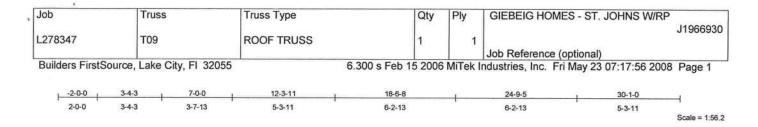
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

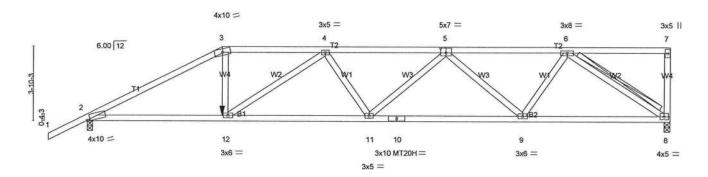
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 2 and 171 lb uplift at joint 10.

LOAD CASE(S) Standard







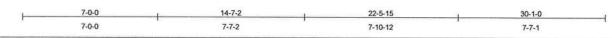


Plate Of	isets (X,	r): [2:0-1-13,Eage], [	5:0-3-8,0-	3-0]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.28	11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.58	9-11	>615	240	MT20H	187/143
RCLL	10.0	* Pan Stress Incr	NO	MAR	0.88	Horz(TI)	0.10	0	2/2	nla	Turkera cattera 4546-3	

BCLL BCDL	10.0 5.0	* Rep Stress Incr NO Code FBC2004/TPI2002	WB 0.88 (Matrix)	Horz(TL) 0.	19	8	n/a	n/a	Weight: 149 lb
LUMBER TOP CH		X 4 SYP No.2		BRACING TOP CHORD	s	Structur	al wood	sheathir	ng directly applied or
BOT CH	ORD 2	X 4 SYP No.2			3.22				end verticals

**BOT CHORD** Rigid ceiling directly applied or 5-0-11 oc bracing.

**WEBS** 

T-Brace: 2 X 4 SYP No.3 - 6-8 Fasten T and I braces to narrow edge of web

with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2108/0-3-8, 2=2051/0-3-8

2 X 4 SYP No.3

Max Horz 2=163(load case 5)

Max Uplift 8=-727(load case 4), 2=-641(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3862/1259, 3-4=-3411/1166, 4-5=-4456/1501, 5-6=-3299/1099,

6-7=-78/14, 7-8=-274/136

**BOT CHORD** 2-12=-1151/3359, 11-12=-1558/4437, 10-11=-1516/4297, 9-10=-1516/4297,

8-9=-929/2619

3-12=-382/1241, 4-12=-1246/526, 4-11=0/190, 5-11=0/246, 5-9=-1342/561,

6-9=-315/1260, 6-8=-3085/1111

## JOINT STRESS INDEX

**WEBS** 

**WEBS** 

2 = 0.81, 3 = 0.73, 4 = 0.41, 5 = 0.79, 6 = 0.92, 7 = 0.59, 8 = 0.89, 9 = 0.92, 10 = 0.86, 11 = 0.41 and 12 = 0.79

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	Т09	ROOF TRUSS	1	1	J1966930
					Job Reference (optional)

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## NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 727 lb uplift at joint 8 and 641 lb uplift at joint 2.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

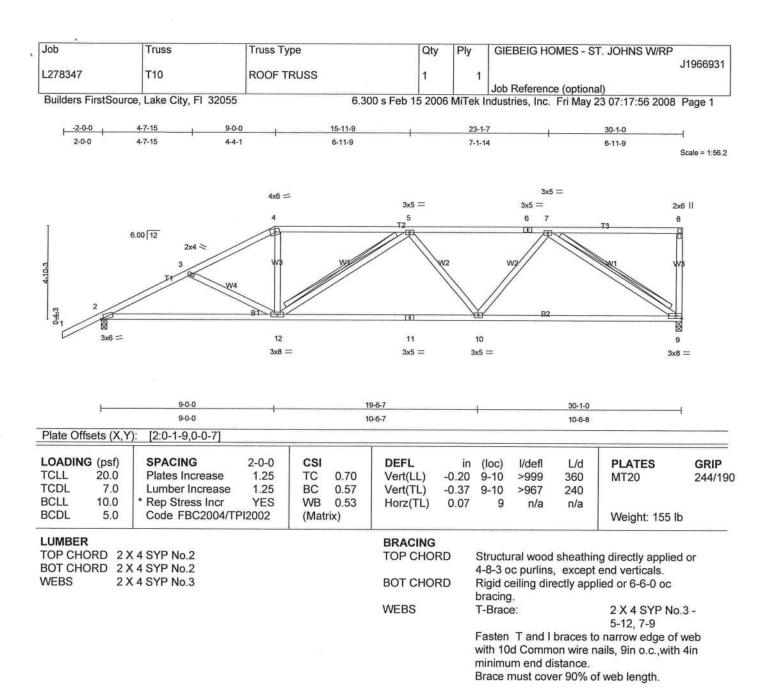
## LOAD CASE(S) Standard

Vert: 12=-411(F)

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12) Concentrated Loads (lb)

> Julius Les Truse Cesian Engineer Flonda PE No. 34889 1109 Coasiel Bay Blyd





**REACTIONS** (lb/size) 9=949/0-3-8, 2=1073/0-3-8

Max Horz 2=195(load case 6)

Max Uplift 9=-259(load case 5), 2=-260(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1758/878, 3-4=-1536/781, 4-5=-1346/760, 5-6=-1400/730,

6-7=-1400/730, 7-8=-58/13, 8-9=-170/118

2-12=-928/1504, 11-12=-881/1581, 10-11=-881/1581, 9-10=-631/1136

WEBS 3-12=-188/191, 4-12=-100/396, 5-12=-283/158, 5-10=-293/245, 7-10=-160/464,

7-9=-1308/743

## JOINT STRESS INDEX

**BOT CHORD** 

2 = 0.77, 3 = 0.33, 4 = 0.71, 5 = 0.40, 6 = 0.32, 7 = 0.46, 8 = 0.76, 9 = 0.58, 10 = 0.40, 11 = 0.59 and 12 = 0.56

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T10	ROOF TRUSS	1	1	J1966931
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:57 2008 Page 2

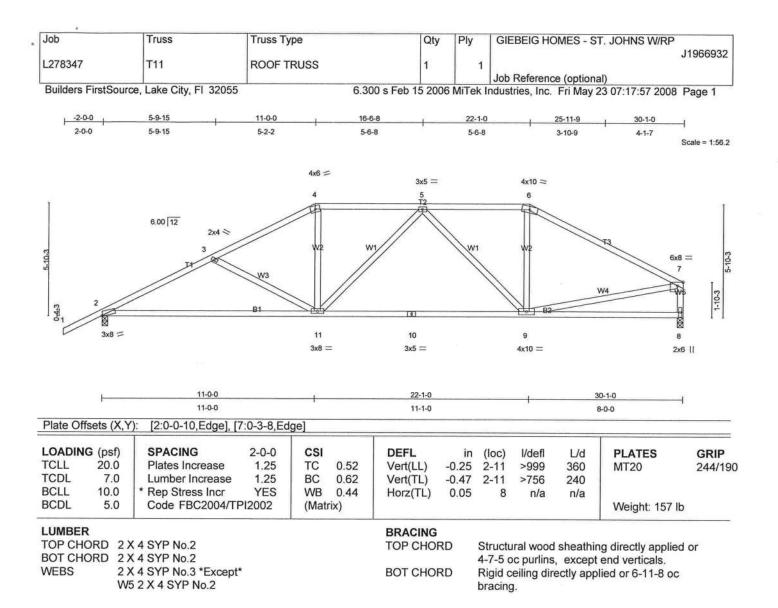
# **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 259 lb uplift at joint 9 and 260 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truee Design Engineer Florida PE No. 2-1969 1109 Costel Bay Blyd





**REACTIONS** (lb/size) 2=1073/0-3-8, 8=949/0-3-8

Max Horz 2=147(load case 6)

Max Uplift 2=-280(load case 6), 8=-163(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1730/929, 3-4=-1419/784, 4-5=-1220/765, 5-6=-1048/691,

6-7=-1265/678, 7-8=-908/533

BOT CHORD 2-11=-814/1478, 10-11=-609/1264, 9-10=-609/1264, 8-9=-171/211

WEBS 3-11=-306/286, 4-11=-107/369, 5-11=-187/118, 5-9=-410/171, 6-9=-16/272,

7-9=-326/850

## JOINT STRESS INDEX

2 = 0.79, 3 = 0.33, 4 = 0.68, 5 = 0.39, 6 = 0.74, 7 = 0.64, 8 = 0.70, 9 = 0.37, 10 = 0.72 and 11 = 0.56

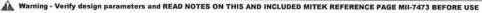
## **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

& Rinkide adequate drainage to prevent water ponding.

Julius Lee Truse Design Engineer Flonda PE No. 24869 1199 Geastal Bay Blvd Dovnton Beach, Ft. 3543







Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T11	ROOF TRUSS	1	1	J1966932
	0.50.50.50	Noor mood			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:17:57 2008 Page 2

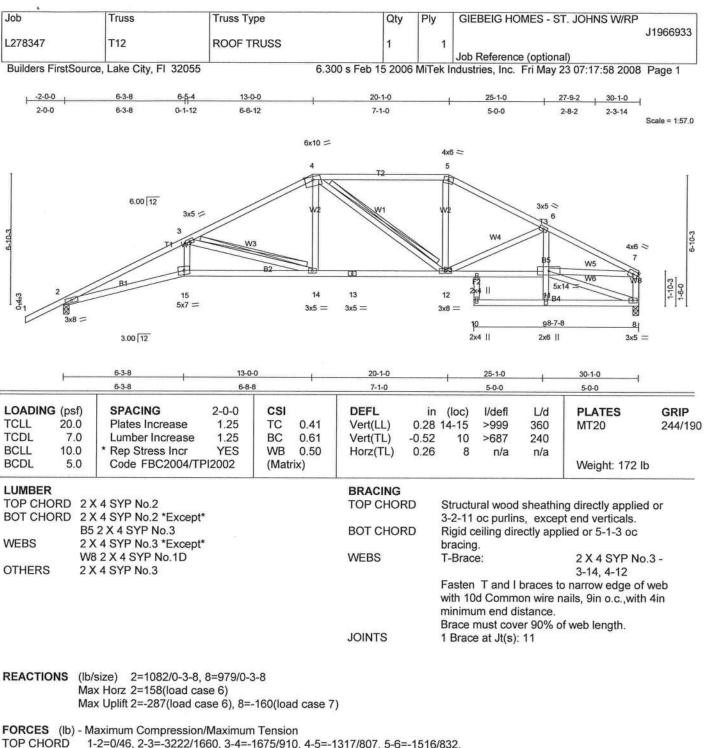
# **NOTES**

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 163 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Trues Cesign Engineer Flonds PE No. 24845 1109 Cessiel Bay Blyd Boynton Besch, FL 20435





1-2=0/46, 2-3=-3222/1660, 3-4=-1675/910, 4-5=-1317/807, 5-6=-1516/832,

6-7=-1919/940, 7-8=-983/522

**BOT CHORD** 2-15=-1503/2876, 14-15=-1432/2723, 13-14=-642/1446, 12-13=-642/1446,

11-12=-783/1677, 9-11=0/197, 6-11=0/309, 9-10=0/0, 8-9=-67/4

**WEBS** 3-15=-307/748, 3-14=-1333/822, 4-14=-175/461, 4-12=-281/107, 5-12=-125/397,

6-12=-419/254, 8-11=-28/129, 7-11=-708/1570

# JOINT STRESS INDEX

Continued on page 2

2 = 0.74, 3 = 0.63, 4 = 0.76, 5 = 0.81, 6 = 0.46, 7 = 0.72, 8 = 0.46, 9 = 0.71, 10 = 0.33, 11 = 0.74, 12 = 0.56, 13 = 0.46, 14 = 0.74

0.43, 15 = 0.84 and 16 = 0.33

May 23,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



3	Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
,	L278347	T12	ROOF TRUSS	1	1	J1966933
		1.1.2				Job Reference (optional)

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## **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

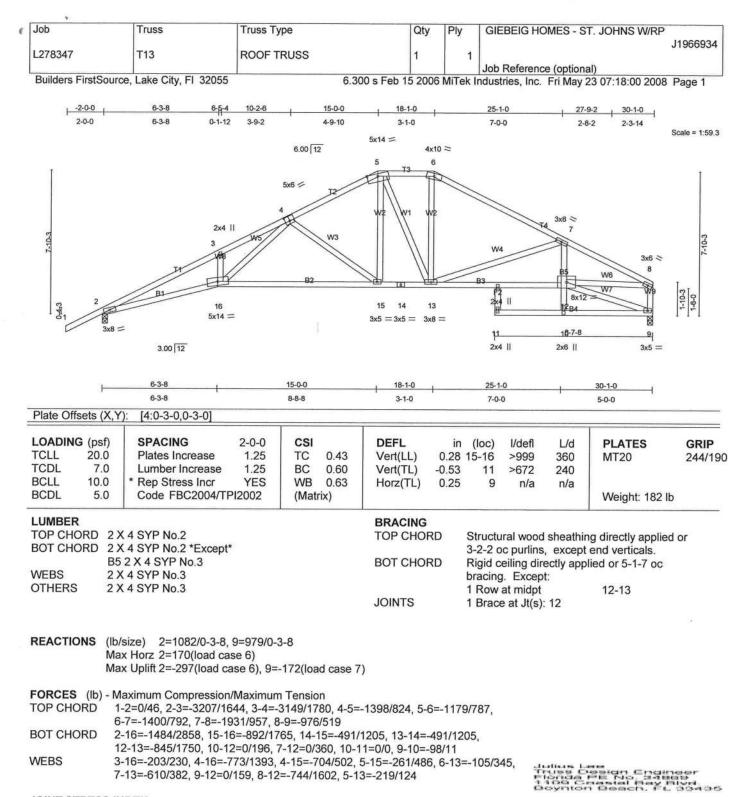
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2 and 160 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Florida HE No. 34899 1109 Ceastal Bay Blvd Boynton Beach, Ft. 32436



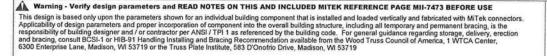


# JOINT STRESS INDEX

2 = 0.74, 3 = 0.33, 4 = 0.62, 5 = 0.41, 6 = 0.61, 7 = 0.85, 8 = 0.82, 9 = 0.44, 10 = 0.75, 11 = 0.33, 12 = 0.64, 13 = 0.62, 14 = 0.59, 15 = 0.39, 16 = 0.80 and 17 = 0.33

## NOTES

Unbalanced roof live loads have been considered for this design.
 Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T13	ROOF TRUSS	1	1	J1966934
			1.		Job Reference (optional)

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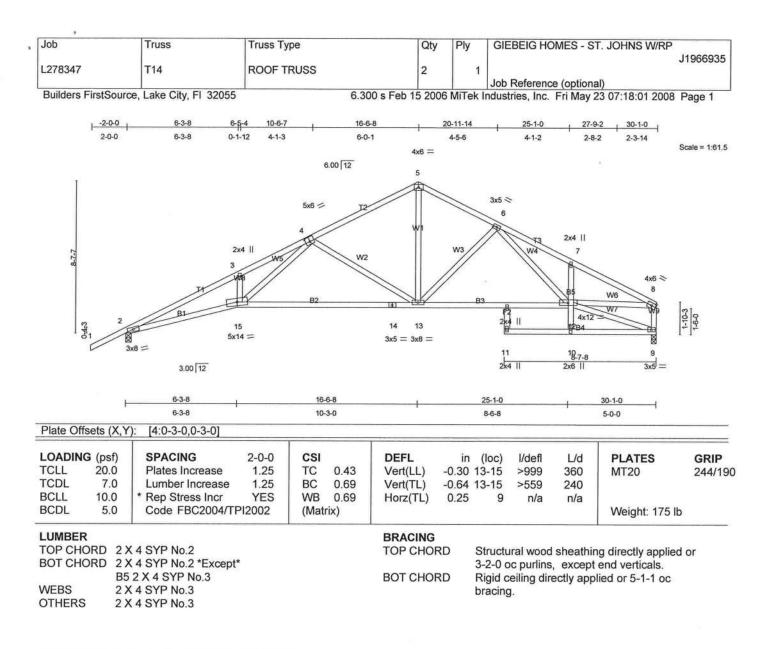
## **NOTES**

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 172 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Trues Cesign Engineer Florida PE No. 34869 1109 Cesstel Bay Blvd Bovnton Besch, FL 33435





REACTIONS (lb/size) 2=1082/0-3-8, 9=979/0-3-8

Max Horz 2=179(load case 6)

Max Uplift 2=-303(load case 6), 9=-180(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3212/1658, 3-4=-3151/1791, 4-5=-1283/770, 5-6=-1254/784,

6-7=-1888/1065, 7-8=-1878/942, 8-9=-985/534

2-15=-1495/2864, 14-15=-897/1741, 13-14=-897/1741, 12-13=-617/1337, **BOT CHORD** 

10-12=0/195, 7-12=-242/248, 10-11=0/0, 9-10=-112/0

WFBS 3-15=-192/224, 4-15=-773/1414, 4-13=-768/559, 5-13=-440/770, 6-13=-398/286,

6-12=-225/538, 9-12=-20/177, 8-12=-711/1537

# JOINT STRESS INDEX

2 = 0.74, 3 = 0.33, 4 = 0.55, 5 = 0.65, 6 = 0.46, 7 = 0.33, 8 = 0.71, 9 = 0.46, 10 = 0.77, 11 = 0.33, 12 = 0.94, 13 = 0.56, 14 = 0.770.61, 15 = 0.87 and 16 = 0.33

1) Unbalanced roof live loads have been considered for this design. Continued on page 2

May 23,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T14	ROOF TRUSS	2	1	J1966935
LETOOTI	114	11001 111000	2		Job Reference (optional)

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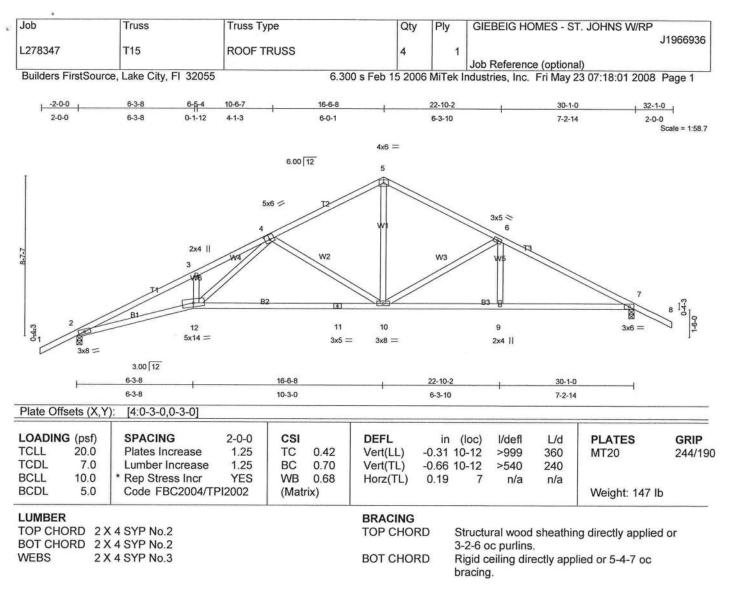
## **NOTES**

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 180 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1100 Coastal Bay Blvd Boydon Description





REACTIONS (lb/size) 2=1069/0-3-8, 7=1069/0-3-8

Max Horz 2=163(load case 6)

Max Uplift 2=-308(load case 6), 7=-289(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3166/1584, 3-4=-3107/1721, 4-5=-1248/759, 5-6=-1255/763,

6-7=-1704/907, 7-8=0/47

BOT CHORD 2-12=-1342/2823, 11-12=-784/1704, 10-11=-784/1704, 9-10=-620/1439,

7-9=-620/1439

WEBS 3-12=-192/230, 4-12=-727/1410, 4-10=-759/533, 5-10=-397/710, 6-10=-474/340,

6-9=0/185

# JOINT STRESS INDEX

2 = 0.73, 3 = 0.33, 4 = 0.52, 5 = 0.75, 6 = 0.46, 7 = 0.74, 9 = 0.33, 10 = 0.56, 11 = 0.60 and 12 = 0.87

# NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee Trues Design Engineer Flonda FE No. 34865 1100 Ceastal Ray Blvri Boynton Besch, FL 3343

May 23,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T15	ROOF TRUSS	4	1	J1966936
LET 0047	110	TOOL TROOP	-	1	Job Reference (optional)

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# **NOTES**

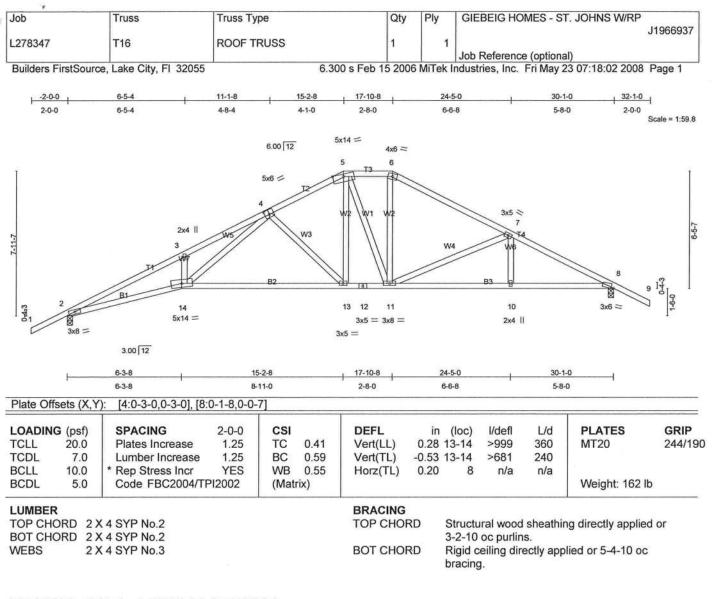
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 289 lb uplift at joint 7.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=1069/0-3-8, 8=1069/0-3-8

Max Horz 2=156(load case 6)

Max Uplift 2=-303(load case 6), 8=-282(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3158/1578, 3-4=-3118/1736, 4-5=-1332/807, 5-6=-1124/768,

6-7=-1330/776, 7-8=-1768/921, 8-9=0/47

BOT CHORD 2-14=-1338/2814, 13-14=-700/1600, 12-13=-375/1151, 11-12=-375/1151,

10-11=-657/1511, 8-10=-657/1511

WEBS 3-14=-235/266, 4-14=-813/1501, 4-13=-635/456, 5-13=-278/521, 6-11=-103/306,

7-11=-437/307, 7-10=0/201, 5-11=-226/106

## JOINT STRESS INDEX

2 = 0.73, 3 = 0.33, 4 = 0.67, 5 = 0.28, 6 = 0.80, 7 = 0.46, 8 = 0.76, 10 = 0.33, 11 = 0.64, 12 = 0.62, 13 = 0.39 and 14 = 0.78

## **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Bonnavide adequate drainage to prevent water ponding.

non Beach, FL 89435

May 23,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



y.					
Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
	F-1613C-2-C65	and a common of the common of		2.504.0047	J1966937
L278347	T16	ROOF TRUSS	1	1	
			100	241	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:18:02 2008 Page 2

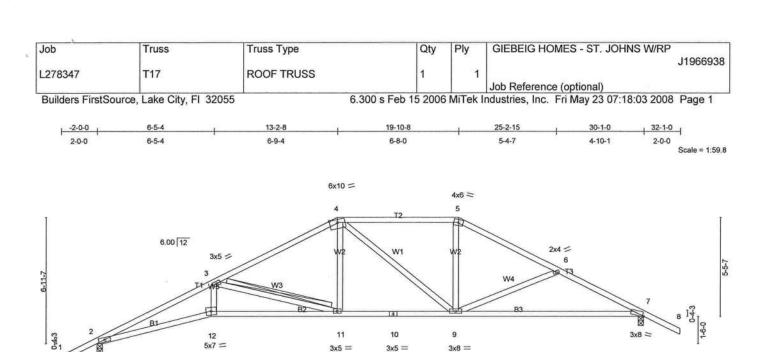
## **NOTES**

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 282 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 24888 1189 Coestel Bay Blyd Baydron Best P. 1944





0-3-6	0110	(E)		
6-3-8	6-11-0	6-8-0	10-2-8	
6-3-8	13-2-8	19-10-8	30-1-0	

Plate Of	fsets (X,Y	'): [7:0-0-10,Edge]		_								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.27	11-12	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.48	7-9	>751	240	127505177555	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.21	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 148 lb	

LUMBER		BRACING			
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood s	heathing directly applied or	
<b>BOT CHORD</b>	2 X 4 SYP No.2		3-3-3 oc purlins.		
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 5-4-5 oc bracing.		
		WEBS	T-Brace:	2 X 4 SYP No.3 -	
				3-11	

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1069/0-3-8, 7=1069/0-3-8

3.00 12

Max Horz 2=144(load case 6)

Max Uplift 2=-293(load case 6), 7=-270(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3177/1592, 3-4=-1614/880, 4-5=-1258/781, 5-6=-1459/797,

6-7=-1749/947, 7-8=0/47

BOT CHORD 2-12=-1355/2836, 11-12=-1293/2685, 10-11=-527/1386, 9-10=-527/1386,

7-9=-681/1502

WEBS 3-12=-267/742, 3-11=-1352/795, 4-11=-175/421, 4-9=-282/110, 5-9=-104/367,

6-9=-274/263

Trues Design Engineer Flonda PE No. 34869 1100 Castal Bay Blvd. Bovoton Besch, FL 33435

JOINT STRESS INDEX

2 = 0.73, 3 = 0.63, 4 = 0.75, 5 = 0.76, 6 = 0.33, 7 = 0.84, 9 = 0.56, 10 = 0.44, 11 = 0.44 and 12 = 0.82

Continued on page 2





Job '	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP	38
L278347	T17	ROOF TRUSS	1	1		,0
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:18:03 2008 Page 2

## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

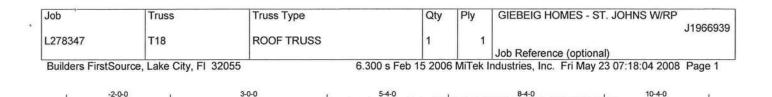
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

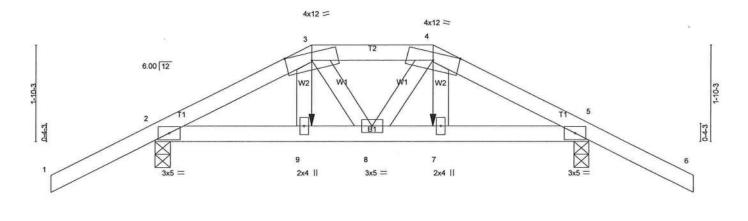
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at joint 2 and 270 lb uplift at joint 7.

LOAD CASE(S) Standard





2-4-0



		3	-0-0		1-2-0	1-2-0			3-0-0			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.01	8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.01	8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.02	Horz(TL)	0.00	5	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	P12002	(Mat	rix)						Weight: 42 lb	

4-2-0

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

2-0-0

BRACING TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

3-0-0

2-0-0

Scale = 1:20.9

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=435/0-3-8, 5=435/0-3-8

Max Horz 2=-54(load case 6)

Max Uplift 2=-278(load case 5), 5=-278(load case 6)

3-0-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-426/230, 3-4=-357/206, 4-5=-429/232, 5-6=0/47

BOT CHORD 2-9=-171/334, 8-9=-166/332, 7-8=-161/336, 5-7=-166/338

WEBS 3-8=-47/42, 4-8=-44/37, 3-9=-56/76, 4-7=-55/76

## JOINT STRESS INDEX

2 = 0.57, 3 = 0.07, 4 = 0.07, 5 = 0.57, 7 = 0.05, 8 = 0.04 and 9 = 0.05

## **NOTES**

LUMBER

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 2 and 278 lb uplift at joint 5. Continued on page 2

Julius Lee Truse Design Engineer Flonda PE No. 34866 1166 Coastal Bay Blvd Boynton Besch, Ft. 33435

May 23,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
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responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enlerprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T18	ROOF TRUSS	1	,	J1966939
L270347	110	ROOF TROOS			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:18:04 2008 Page 2

## NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-64(F=-10), 4-6=-54, 2-9=-10, 7-9=-12(F=-2), 5-7=-10

Concentrated Loads (lb)

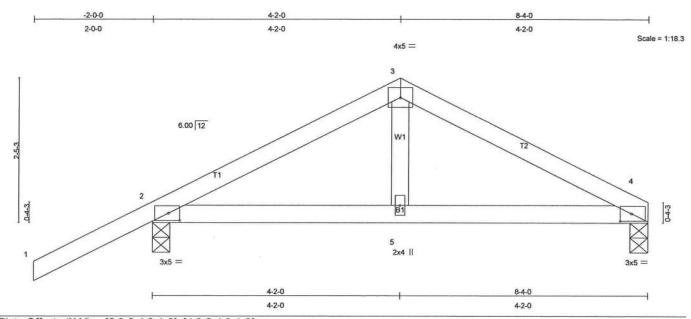
Vert: 9=-48(F) 7=-48(F)

Julius Lee Trues Design Engineer Florida PE No. 34888 1100 Coestal Bay Blvd.



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T19	ROOF TRUSS	2	1	J1966940
THE STATE OF				2	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:18:05 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.03	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 33 lb	

		R

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 9-8-7 oc bracing.

REACTIONS (lb/size) 4=242/0-3-8, 2=389/0-3-8

Max Horz 2=74(load case 6)

Max Uplift 4=-155(load case 7), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-341/525, 3-4=-335/514

BOT CHORD

2-5=-390/255, 4-5=-390/255

**WEBS** 

3-5=-243/134

# JOINT STRESS INDEX

2 = 0.67, 3 = 0.58, 4 = 0.67 and 5 = 0.10

## NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Flonda PE No. 34869 1 109 Coastal Bay Blvd Boynton Beach, FL 33435

May 23,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



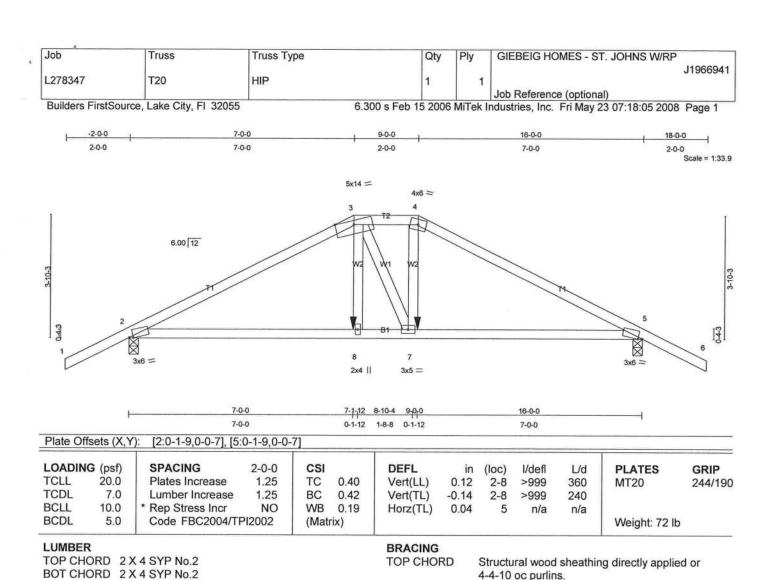
Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP	
L278347	T19	ROOF TRUSS	2	,	J196	66940
L270347	119	ROOF TRUSS	2	1	Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 4 and 262 lb uplift at joint 2.

LOAD CASE(S) Standard





**BOT CHORD** 

bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8

2 X 4 SYP No.3

Max Horz 2=-77(load case 6)

Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/806, 5-6=0/47

BOT CHORD 2-8=-675/1504, 7-8=-684/1523, 5-7=-658/1507

WEBS 3-8=-262/480, 4-7=-303/592, 3-7=-146/159

# JOINT STRESS INDEX

2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.43 and 8 = 0.34

## NOTES

**WEBS** 

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Les Truse Cesign Engineer Floride PE No. 34868 1 100 Caestal Bay Blvd Bovnton Besch, Ft. 33431

Rigid ceiling directly applied or 7-5-5 oc



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T20	HIP	4	4	J1966941
L276347	120	nie	'	'	Job Reference (optional)

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## NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

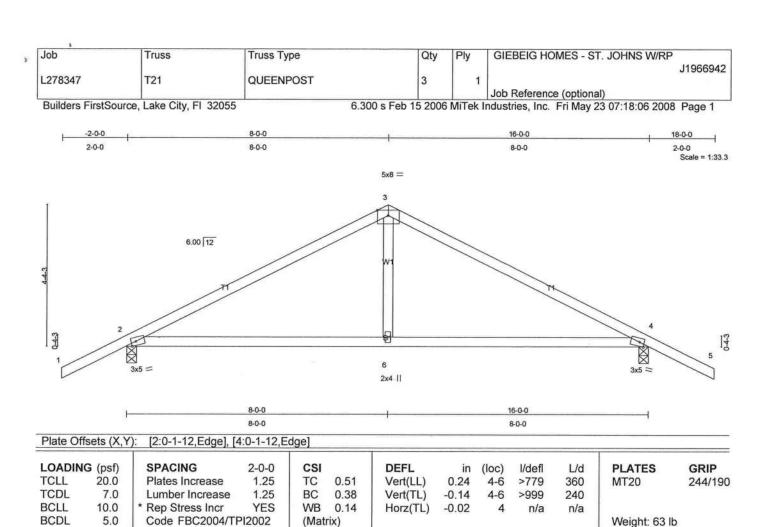
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)

Julius Lee Truss Design Engineer Florida ME No. 34866 1 109 Gestal Bay Blyri





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS** 

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 7-1-0 oc

bracing.

REACTIONS (lb/size) 2=619/0-3-8, 4=619/0-3-8

Max Horz 2=83(load case 6)

Max Uplift 2=-404(load case 6), 4=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-720/981, 3-4=-720/981, 4-5=0/47

**BOT CHORD** 

2-6=-689/562, 4-6=-689/562

**WEBS** 

3-6=-489/273

# JOINT STRESS INDEX

2 = 0.77, 3 = 0.93, 4 = 0.77 and 6 = 0.19

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Champearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

May 23,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - ST. JOHNS W/RP
L278347	T21	QUEENPOST	3	1	J1966942
L270547	121	QUELINI OUT	5		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri May 23 07:18:06 2008 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 4.

LOAD CASE(S) Standard

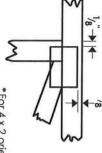


# Symbols

# PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise securely seat.



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates

# PLATE SIZE

4×4

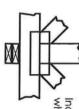
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

# LATERAL BRACING



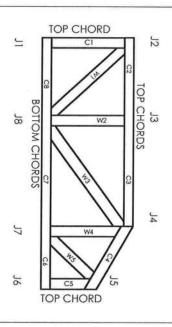
Indicates location of required continuous lateral bracing.

# BEARING



which bearings (supports) occur. Indicates location of joints at

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

# CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922

BOCA

SBCCI

9667, 9432A

WISC/DILHR 960022-W, 970036-N

561

NER



MiTek Engineering Reference Sheet: MII-7473

# General Safety Notes

# Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- N other Cut members to bear tightly against each
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- S Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 0 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- 7. 00 is the responsibility of truss fabricator. General Camber is a non-structural consideration and Plate type, size and location dimensions practice is to camber for dead load deflection
- shown indicate minimum plating requirements
- .9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ff. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- 13. Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- 15. Care should be exercised in handling erection and installation of trusses.

© 1993 MiTek® Holdings, Inc.

# ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, II 1.00, EXPOSURE C

DIAGONAL VENTICAL DOUBLED BRACE RIACONAL AT EACH TOVAL LE TOVAL LE IN VE		N	A A	4.2	ζ	(	j/	A F	31	LE	3	1	VI	ΞF	6.	ľ	C.	A	L		L	E	N	G'	ГΗ	
DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONDET DIAGONAL BRACE FOR SAGI AT EACH EMD. MAY WEB TOTAL LENGTH IS 14°.  VERTICAL LENGTH S. IN TABLE ABOVE.		12	S,	1	O	.(	3.			1 (	6	"	(	).	С		0.120	2	4	91	(	0	. (	· .	SPACING	GABL
AL BRACE OPTION: AL LENCTH MAY BE DE WHEN DIAGONAL BE USED. CONNECT AL HAZE FOR SAGE HE END. MAY WEB LENGTH IS 14*.  VERTICAL LENGTH SHOWN IN TABLE ABOVE.		JFI.	Ç	D	111	I	מלו	CI		JFI.	?	D D		Ή	7	CDT	ļ	J		S		Į		SPF	SPACING SPECIES	CABLE VERTICAL
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CARLO					-	$\vdash$							+	+	-	#2					+	+		#2	$\vee$	BRACE
SBUET THEAD		4.	4. 4.	4 4		4 2	4, 2,"	4.	10"	4.0"	4.	2	4 0	0, 8	3 89	3, 10,		3 6"	3. 6.		- 1			2 4	ACES	S .
2X4 SP 4 2X4 SP 4/4 DIAGON SINGLE 6 CUT (AS	6' 1"			2, A	6, 11,		6' 11"	7' 4"	5' 3"	- 7		- 1	7	, c	6.0			5 0"					- 1	6 10	GROUP A	(1) 1X4 °
ZX4 SP #ZN, DF-L #Z, SPF #1/#Z, DR SETTER DIAGONAL BRACE; SINCLE OR DOUBLE CUT [AS SHOWN] AT UPPER BND.	6' 1"	7	3, 50	3, -	6, 11,	6 11	6' 11"	7. 7.	5' 3"		6, 2,	- 1		y. 0	6.0	6, 10.	4' 3"	٦	6. 0.	ر ع د	- 1	4. 2.	A, 4, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	6.0	GROUP B	(1) 1X4 °L" BRACE •
	B' 0°		1	0 0	7	B' 9,	9, 8,	8.8	6' 11"		7. 11.	٦		8. 10"	7 11	7' 11"	5. 8.		1	6, 11,		- 1	200	1	GROUP	_
SYN ABOUT	B' 0*		0,0		7	8° 9"	8' B"	- 1	6' 11"		00	-1	4	7.11	Ι.	8 1"	5. 8.	1	6.	7' 5"	1	1	n, 0	7' 1"	A CROUP B	"L" BRACE
- # -   - # -   -   -   -   -   -   -	100	- 1	10, 20			10' 5"	10' 6"	10. 6.	-1		9. 6.		מ מ	0 60	1	9 6"	7, 8,		1	- 1	200		0 0		GROUP A	(2) 2X4
	10' 8"		101 101						9, 4,	9 :			0,0			9 8"		- 1			R. 14	7. 5.		0.00	GROUP	L" BRACE .
BETTER O		13.6	12,0		1	13' 8"	13' 8"					12' 5,		12. 4.				10' 3"		10, 10,	10, 10,				B CROUP A	(1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 "L" BRACI
	12' 6"	4.4	24	14 0	12' 3"	13' 8"	13' 8"	14. 0"	10. 10.	12 5	120	3, 0	ما م	12. 4.	12' 4"	12, 8,	8. 10.	10' 3"	10' 4"	11.	100	ماء		11. 5.	GROUP	L" BRACE +
	14. 0*	14.0	140	14.0	14' 0"		14. 0	14. 0.	14.0	14	14.0	14.0	140		14' 0"	14. 0.	12' 0"				101 0		11 21		B GROUP A	(2) 2XB "L" BRACE
	14. 0*	0	140	14 0	14' 0"	14' 0"	14 0	14. 0"	14. 0.	0,0	14.0	14' 0"	14	14, 0,	14' O"	14. 0.	12.0*		13. 7.		12 0				GROUP B	L' BRACE **
ATTACH ATTACH POR N 11 ** FOR N 11 ** FOR N 11 ** FOR N 11 ** FOR	CONT	PROVIDE	LIVE LO	_	CA.					Spr		=	_				I	_  -	סטסט	-	11	SPR	_	BRAC	7	
CABLE END SUPPORTS LOAD FROM 4 0* DUTLIDORERS WITH E 0* DVERHANG, OR 12* PLYWOOD OVERHANG.  ATTACH EACH 'L' BRACE WITH 104 NAILS.  \$ POR (1) 'L' BRACE: SPACE NAILS AT 8* O.C.  IN 18* END ZONES AND 4* O.C. BETWEEN ZONES.  \$ \$ FOR (2) 'L' BRACES: SPACE NAILS AT 3* O.C.  IN 18* END ZONES AND 6* O.C. BETWEEN ZONES.  "L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.  GRADIE VERTICAL LENGTH NO SPIJOR  LESS THAN 4* O*  LESS THAN 4* O*  GREATER THAN 11" 6*  BY THE STACE NAILS AT S	CONTINUOUS BEARING (S PSF TC DEAD LOAD).	PROVIDE UPLATT CONNECTIONS FOR 136 PLF OVER	LIVE LOAD DEPLECTION CRITERIA IS L/240		CARLE TRI			12	£3	BUILD NOCHLINGS	Г	П		0		CHANDAND	CULIS	5	DOUGLAS FIR-LARCH	STUD	#1 / #2 STANDARD	SPRUCE-PINE-FIR	^	BRACING GROUP SPECIES AND GRADES:		
E. CO DERM E. CO DERM E. CO DERM E. SPACE E. SPA	ASA O DNI	NECTIONS F	ON CRATERIA		מת שביים			П		חחוו	11	#1 & BIR	HEM-PIR	GROUP B:			Т	_				25	GROUP A:	SPECIES		
M 4 0"  HANC, DR 12"  ON NAILS AT 8  NAILS AT 8  NAILS AT 3  ON NAILS AT 3  TE SIZES  NO SPICE  INA OR 8X3  ZX4  2.5X4	TC DEAD L	OR 136 PLF	IS L/240.	1011	TRIES DETAIL NOTES			#2	#1	TAS PIR-LA	L	Ц		-2		STANDARD	arun	#3	SOUTHERN PINE	#3 STANDARD	П	HEM-PIR		AND GRA		
2. C.C. V. ZDNES. 3. O.C. V. ZDNES. V. ZDNES.	DAD).	OVER		ç	ņ	3		Ц		ACA.						L	Ш		चि	DARU	B			DES:		

VERTICAL LENGTH

LESS THAN 4' 0"

GREATER THAN 4' D', BUT

LESS THAN 11' B" REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. GABLE VERTICAL PLATE SIZES NO SPLICE 2.5X4 224

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH CONTINUOUS BEARING

\*\*\*WARRING\*\*\* TRUSSES REQUIRE EXTRENE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRADING. REFER TO IESS 1-63 (BUILING COMPONENT SAFETY INFORMATION, PUBLISED BY TRY TRINGS PLATE (INSTITUTE). SAID DUTCH OFFORTO BR. SUITE 200, MAIDION, VI. 23759 AND VICE A COZO TRUSS COLUCILIO BRACIFICA, 6300 ENTERPRISE UN, MAIDION, VI 33719) TRE SAFETY PAULTICES PRIBE TO PERFORMING THEXE TUNCTIONS. UNICES OFFERVISE INGIDIATED, 109 CORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORGO SMALL HAVE A PROPERLY ATTACHED RIGID CITINGS. ULIUS LEE'S cons. Engineers P.A. DELRAY BEACH, PL 33444-2161 STATE OF FLORIDA MAX MAX TOT SPACING

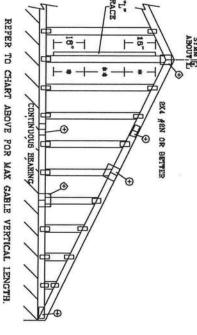
CONNECT DIAGONAL AT AUTOPOINT OF VERTICAL WEB.

E. 60 PSF DRWG DATE REF NITEX SID GABLE 15 E HI 11/26/03 ASCB7-02-GAB13015

24.0"

# ASCE 7-02: 130 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, H 1.00, EXPOSURE a

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		LHL	ן ן	<i>ن</i> ار	) j	TII	Į.	JIL	なせば	2000 2000 2000	LFL	1	\(\frac{1}{2}\)	ì	TIT	Į,	OF F	CDI		L H L	1	υ. Τ	)	1111	ij	בלק	נותנו	SPACING SPECIES	CARLE VERTICAL
	STANDARD	STUD	<b>‡</b> 3	#2	<b>+</b> 3	STANDARD	STUD	*	\$1 / #2	STANDARD	STUD	<b>£</b> 3	#2	<b>#</b> 3	STANDARD	STUD	#3	<b>\$1 / #2</b>	STANDARD	STUD	<b>#</b> 3	Z#2	1.1	STANDARD	STUD	<b>\$</b> 3	\$1 / #2	GRADE	BRACE
	4. 0.		4.	4. 4.	4. 5.	3' 11"	3' 11"	3' 11"	4.0,	3. 8.		3. 8.	3' 11"	4.0	3. 7.	3' 7'	3' 7"	3. 8.		3 3	3. 3.	3' 6"	3' 6"		3' 1"	3' 1"	3.	BRACES	1971
	5 6	6.4	و. و.	6' 11"	5' 11 <b>"</b>	5, 4,	6' 3"	6. G	6 11"	4. 9.		5. 7.	6' 4"	B' 4"	4. 8.	5' 6"	5. 5.	6. 4.	3' 10"	. 7	4. 6.	5' 6'	5, 6,		4' 6"		5' 6"	GROUP A	(1) 1X4 °L°
	5, 6,	6' 4'	6, 2,	7' 6"	7' 6"	5' 4"		6 3°	7' 2'	4' 9"	5 6	6. 7.	8' 10"	6' 10"	4' 8"	6. 5.	5' 5"	6. 6.	3' 10"	4 6	4. 6.	5' 11"	100	3. 9.	4, 5,	4. 5.	6' 8"	GROUP B	BRACE .
	7' 3"	8.3	e, 3,		8 3	7' 1"	8° 3"	8' 3"	8' 3'	6" 3"	7' 3"	7. 4.	7' 8"		6, 5,	7. 2.	7' 2"	7' 6"	6, 1,		6, 0,	6, 6,	&' B'	6.0.	5' 10"	6. 10	6. 6.	GROUP A	(1) 2X4 "L" BRACE
CI MIXAS	7' 3"	8. 5.	8' 6"	.8' 11"	B' 11.	7' 1"	a' 3"	8' 3"	8' 6'	6' 3"	7' 3"	7' 4"	B' 1"	B' 1"	6. 2.	7. 2.	7' 2"	7' 8"	5. 1.		6.0"	7' 0"		5. 0.	- 1	5' 10"	- 1	GROUP B	
5			9' 10"	100	8, 10,	9, 8,		9, 10,		B) 5"	8' 11"	8. 11	8' 11"	8' 11"	8. 3.	8' 11"	8' 11"	8. 11	8' 11"	7' 10"	7' 10"	7' 10"		6. 9.	7' 10"	7' 10"	7 10	GROUP A	(2) 2X4 "L"
	8, 8,	10' 4"	10' 4"	10' 7"	10' 7"	g, 6,	9' 10"	9' 10"	10' 1'	B' 5°	8, 2,	8, 6,	8, 3,	7	8.3.		8' 11"	9 2;	6, 11,	8 0	8 1.	-1	7	6. 9.	7' 10"	- 1	8. 0.	GROUP B	BRACE **
	11' 4"	1 1	11, 21	12, 11,	12' 11"	11' 1"	12. 10.	12' 11"		8, 8,	11' 4"			11, 8,	8. 3.	11, 1,,		11. 9.	8' 0"		9. 4.	10′ 3″	10' 3"	7' 10"	9' 1"		10. 3.	GROUP A	(1) 2X6 "L" BRACE
	11' 4"	13' 1"	13' 3"	13' 11°	13' 11"	11' 1"	12' 10"	12' 11'	13' 4"	9' 9"	11' 4"	11. 6.	12' B"	12' B"	9. 7.	11' 1"	11' 2"	12' 1"	8.0.	8 3	9. 4.	11, 1	11.	7. 10.	9( 1.	9.17	10' 7"	GROUP	" BRACE .
	14' 0"	14. 0.	14' 0"	14' 0"	14, 0,	14' 0"	14' 0"	14 0	14. 0.	13' 3"	14. 0	14. 0.	14' 0"	14.00	12. 11.	14. 0.	14' 0"	14. 0.	10, 10,	12 3	12. 3.	12, 3,	12' 3"	10. 7.	12' 3"	12' 3"	12' 3"	B GROUP A GROUP	(2) ZXB "L" BRACE
	14' 0"	14 0"	14' 0"	14' 0"	14.0	14. 0*	14' 0"	14.0				14. 0.	- 1	14' D"		- 1		14. 0.								12' 3"	12' 7"	GROUP B	BRACE **
DUTILIDATES WITH E' O' DVERHANG DR 12"	CARL BUR STREAMS (O FOR IC STALL LIAD).	CONTINUING BEARING (& DOE TO DEAD TOWN	Designation of the second seco	DIST I SI NIBALINA MULTALISAU UVOI ANII	CABLE IKUSS DETAIL NOTES:				#2	_	SOUTHERN PINE DOUGLAS FIR-LARCH	-	#1 & BTR	HEM-FIR	GROUP B:			STANDARD	I	_	DOUGLAS FIR-LARCH SOUTHERN PINE	Chydnyte of Contents of	AZ STANDARD AZ	-PINE-FIR HEM-FI	GROUP A:		BRACING GROUP SPECIES AND GRADES:		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WINN DIAGONAL
BRACE IS USSED. CONNECT
MACONAL BRACE FOR BEG
AT EACH YED. MAY WEB
TOTAL LENGTH IS 14\*.

GABLE TRUSS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DIF-L #Z OR
BETTER DIAGONAL
BRACE, SNGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

CONNECT DIAGONAL AT WEB.

E	
TRUSS	
DETAIL	
NOTES:	

PLYWOOD OVERHANG. BLE END SUPPORTS LOAD FROM 4' 0"
DUTLOCKERS WITH 2' 0" DVERHANC, DR 12" AD DEPLECTION CRITERIA IS L/240. HUDLIS BEARING (6 PSF TC DEAD LOAD).

ATTACH EACH 'L' BRACE WITH 10d NAILS.

\* POR (1) 'L' BRACE: SPACE NAILS AT 8' O.C.

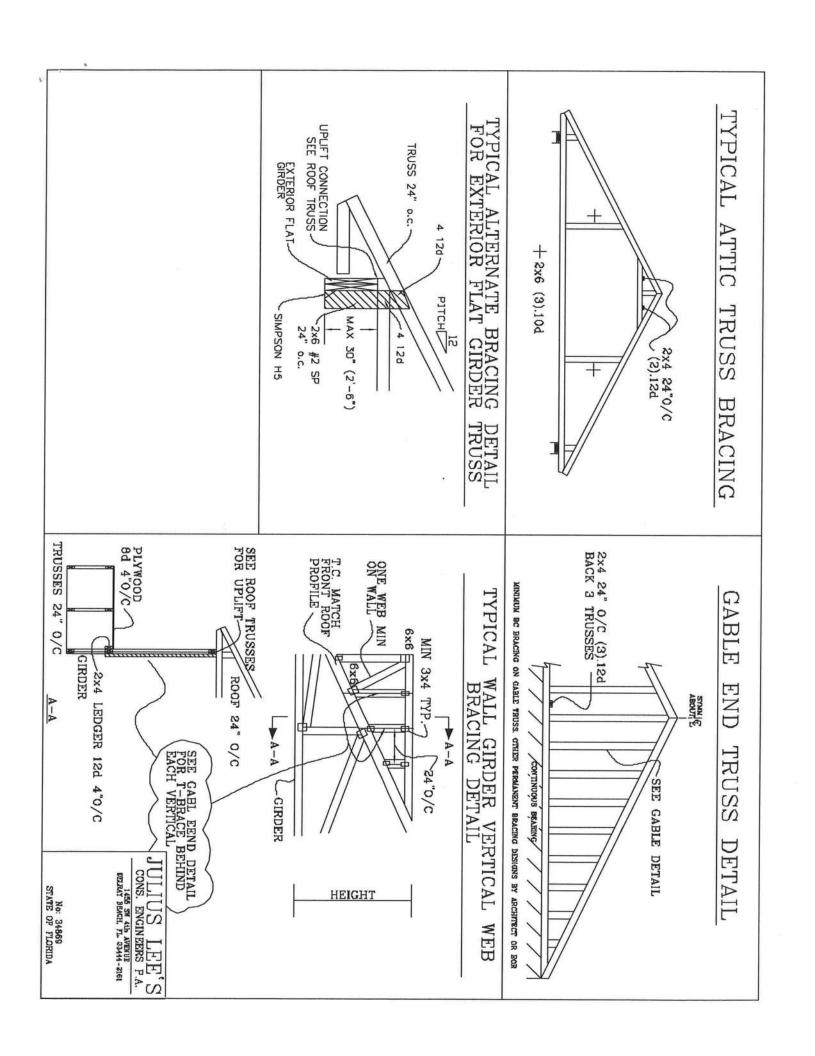
\* POR (2) 'L' BRACES AND 4" O.C. BETWEEN ZONES.

\* FUR (2) 'L' BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

PLATES.	REFER TO COMMON TRUES PEAK, SPLICE, AND HEEL I
2.5%4	GREATER THAN 11' 6"
17/2	GREATER THAN 4' D', BUT
1X4 DR 2X3	LESS THAN 4' 0"
ND SPLICE	VERTICAL LENGTH
E SIZES	GABLE VERTICAL PLAT

			LS AND BOTTON CHOI	THESE CONTIDES. UNLESS CITERAISE INDICATED, TOP CHOOL SHALL HAVE PROPERLY ATTACHED.	ANING ** TRUSSES REBUIRE EXTREME CARE IN FADRU	
STATE OF FLORIDA	1			DELRAY BEACH, PL. 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
MAX. SPACING 24.0"		MAX. TOT. LD. 60 PSF				
			-ENG	DWG MITEK STD GABLE SO' E HT	DATE 11/26/03	REF ASCE7-02-CAB13030



TOP CHORD 2X4 2X4 300 222

# PIGGYBACK DETAIL

TYPE

SPANS

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5

30,

34

88

52

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. TO SEALED DESIGN FOR DASHED PLATES.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF PLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

HIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND HC DL=6 PSP

U Ħ

> 5X6 .5X4

5X5

4XB 5X4 EXG.

OR 3X6 TRULOX AT 4' HOTATED VERTICALLY

DC,

0

1.5X4

1.5X4 5X6

B >

**4X8** 2X4

5X8

5X8

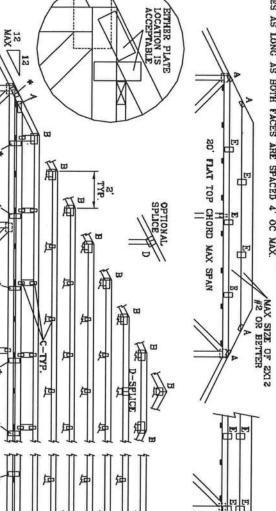
5X6

2.5X4

2.6X4

335

FRONT FACE (5,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. 中 带 #2 OR BETTER 



o' To 7'9"   NO BRACING	-71	WEB BRACING	TYACH TRUOX PLATES WITH QUAL, PER FACE PER PLY. E CONNECTED. REFER TO DIVERNATION.
	REQUIRED BRACING	IG CHART	ATTACH THULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 84 NAILS AT 4" OC.
10' TO 14'	2x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

\* PICCYBACK SPECIAL PLATE

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o '	*	
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SIHI
DRAWING
REPLACES
DRAWINGS
634,016
634,017
₺ 847.04
4

\*ATTACH

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

	8	WAVARHINGS: TRUSCS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING.  BACING: REFER TO JUST I-DO GUILLING COMPONENT SAFETY INFORMATION, PILE (SHIEL BY THE CRUSCS AND THE ACTION FOR THE SAST CAPITAL BY THE CONTROL BY THE CONTROL BY THE CRUSCS CAPITAL BY THE CRUSCS AND SHIPPING. LIVERS CONTROL BY THE CONTROL SAFETY PROCEDURE. LIVERS CONTROL FOR THE STENDED SHIPPING. LIVERS CONTROL MICHAEL THE CRUSC SAFETY PROCEDERY ATTACHED STRUCTURAL PANCLS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING.	. WELLACK THE SAN ENDED ON ALTINE FIGUREACH SECURE FLAIR.
STATE OF FLORIDA		JULIUS LEE'S cons. engineers p.a.	THIS DRAWII
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	NG REPLACES DRAWINGS
		REF PIGGYBACK  DATE 09/12/07  DRWGMITEK STD PIGGY  -ENG JL	THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

# VALLEY TRUSS DETAIL

TOP CHORD BOT CHORD WEBS 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. ZX4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- \*\* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCI BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR OR (3) 16d ENCLOSED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"—BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

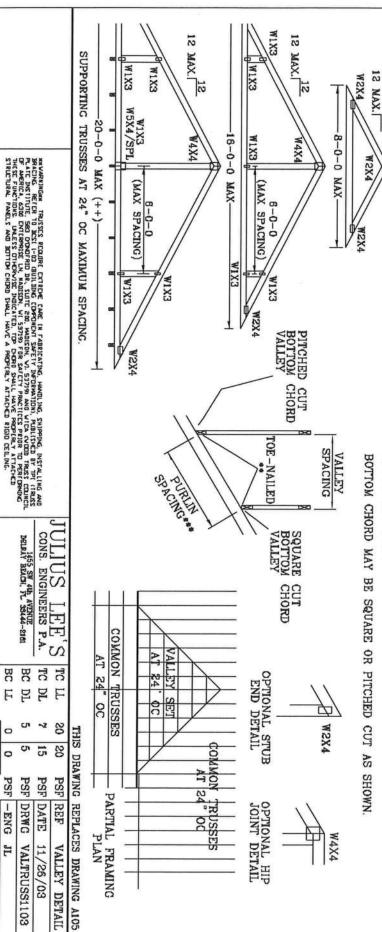
\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



No: 34868 STATE OF FLORIDA

SPACING **DUR.FAC. 1.25** 

24

1.25

BC

F

PSF PSF

-ENG

VALTRUSS1103

TOT. LD.

32 0

40

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE. END DISTANCE. SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

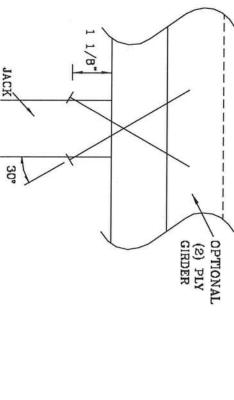
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

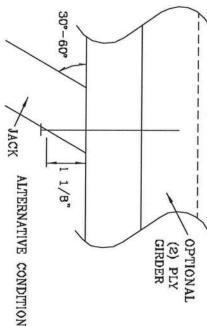
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

# MAXINUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

""" """ """ """ """ """ """ """ """ ""	5 49	4 39	3	22	NUMBER OF TOE-NAILS 1	
11	493#	394#	296#	187#	PLY	SOUTHERN PINE
11	639#	511#	383#	256#	2 PLIES 1 PLY	
11000	452#	361#	271#	181#	1 PLY	DOUGLAS
#coc	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
noon.	390#	312#	234#	156#	1 PLY	HEM-FIR
70.4	スロマル	406#	304#	203#	2 PLIES	-FIR
14.00	#18E	307#	230#	154#	1 PLY	SPRUCE
#00#	1084	397#	298#	189#	2 PLIES	SPRUCE PINE FIR

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





THIS DRAWING REPLACES DRAWING 784040

			***WARRING*** TRUSSES REDURE EXTRENE CARE IN FABRICATING, HANDLING, SUPPINIE, INSTALLING AND BRACINE. RETER TO BEST 1-43 CUMLING CORPORENT SAFETY (NETDWATION, PUBLISHED BY FP] CTRUSS PARTE INSTITUTE, 383 D'ONCHRED DR., SUITE 280, MADISON, M. 30719) AND VICA K-UDD TRUSS COLNEY OF AREELY, 6300 ENTERPRISE DR. MATISON, M. 130719) TOR SAFETY PRACTICES PRIET IN PREPERBY ATTACHED RIGHT ATTACHED RIGHT ATTACHED SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING.					
STATE OF FLORIDA	No: 34869 STATE OF FLORIDA			DELEAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S		
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL		
	1.00	PSF	PSF	PSF	PSF	PSF		
			-ENG JL	DRWG	DATE	PSF REF		
			л	CNTONAIL1103	09/12/07	TOE-NAIL		

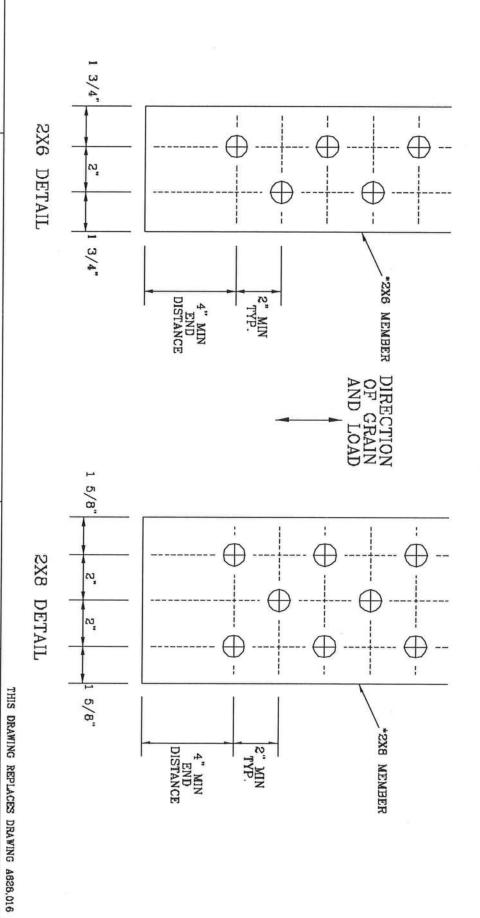
# DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL T0GRAIN

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



CONS. ENGINEERS P.A.

DELRAY BEACH, PL. 39444-2161

TC DL BC DL

PSF

BOLT SPACING 11/26/03 CNBOLTSP1103

PSF

DATE DRWG -ENG

PSF

No: 34869 STATE OF FLORIDA

TOT. LD.
DUR. FAC
SPACING

# TRULOX CONNECTION DETAI

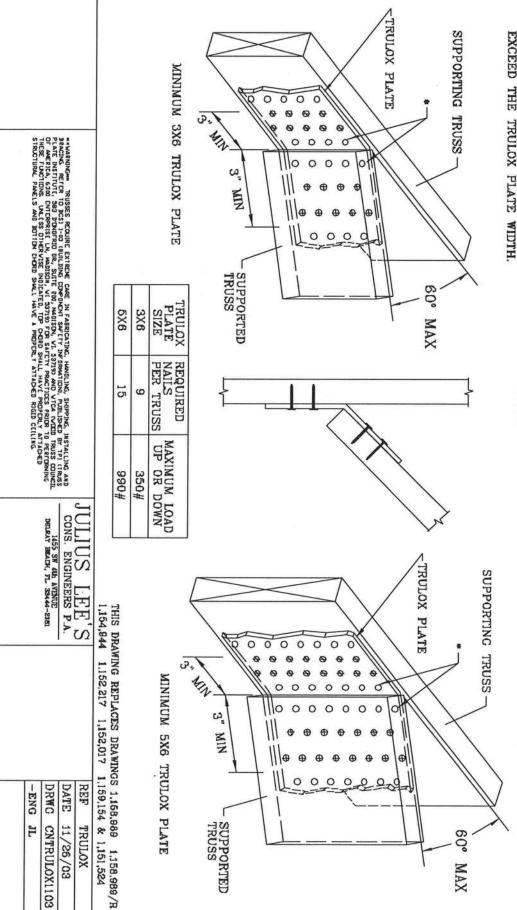
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\( \phi \)).

\* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE. DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

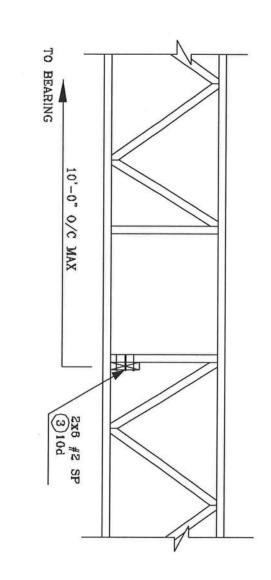
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

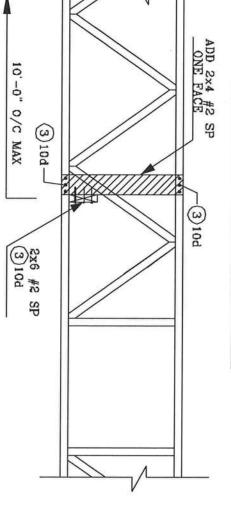


No: 34869 STATE OF FLORIDA

# STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



# ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. engineers p.a.

No: 34869 STATE OF FLORIDA TO BEARING



# Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in

Building permit No. 000027921

12.84

Fire:

33.50

Waste:

Total:

accordance with the Columbia County Building Code.

Parcel Number 11-4S-16-02911-313

Use Classification SFD/UTILITY

Permit Holder B. TRENT GIEBEIG

Owner of Building PETE GIEBEIG

170 SW VANN COURT., LAKE CITY, FL Location:

Date: 08/16/2010

**Building Inspector** 

POST IN A CONSPICUOUS PLACE Business Places Only)

