DATE 12/21/2010

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000029092

APPLICANT FRED J. HATFIELD III PHONE 386-364-1234

ADDRESS 8588 US HWY 90 LIVE OAK FL 32060

OWNER COLUMBIA GAIN & INGREDIETS, INC. PHONE 755-7700

ADDRESS 3830 NW BROWN RD LAKE CITY FL 32055

CONTRACTOR FRED J. HATFIELD III PHONE 386-364-1234

LOCATION OF PROPERTY 90 W, R BROWN RD, AT CORNER OF BROWN AND NOGEL RD

TYPE DEVELOPMENT STORAGE GRAIN BIN ESTIMATED COST OF CONSTRUCTION 225000.00

HEATED FLOOR AREA _____ TOTAL AREA 30000.00 HEIGHT 18.00 STORIES _____

FOUNDATION CONCRETE WALLS METAL ROOF PITCH 2/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. _____ FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 19-3S-16-02181-001 SUBDIVISION _____

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES _____

_____	_____	CBC058310	x Frank J. Hatfield III
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

EXISTING	NA	BK	TC	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: SE 0507 APPROVED

AT FINAL INSPECTION ZONING IS REQUIRED TO ALSO SIGN OFF ON INSPECTION

Check # or Cash 27607

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 1125.00 CERTIFICATION FEE $ 150.00 SURCHARGE FEE $ 150.00

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ _____ **TOTAL FEE** 1500.00

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE CAA AVE
City LAKE CITY Phone 386-752-1705

Site Location: Subdivision COLUMBIA GRAIN
Lot # _____ Block# _____ Permit # 29092
Address BEAUN RD.

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
MAIN BODY	30,000	700'	1537

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

1-14-11 Date

2:30 PM Time

TERRY MARTIN Print Technician's Name

F-304

Remarks: _____

Columbia County Building Permit Application

No E.H. Needed Revised 9-23-

For Office Use Only Application # 1012-20 Date Received 12/14/10 By LH Permit # 29092

Application Approved by - Zoning Official BLK Date 12.17.10 Plans Examiner T.C. Date 12.17.10

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments SE0507 Zoning Needs to be present for final Inspection.

☐ NOC Not Recorded ☑ Site plan in zoning file ☑ Only One set (of Plans) ☑ Authorization Letter in file (Applicant Needs to sign - if not fred) ☑ App fee paid ☑ VF form

Applicants Name Fred J. Hatfield III - Gamble + Associates Phone 386/364-1234

Address 8588 US Hwy 90 Live Oak FL 32060

Owners Name Darryl Ward - Columbia Grain & Ingredients, Inc. Phone 386/755-7700

911 Address 3830 NW Brown Rd. Lake City, FL 32055

Contractors Name Gamble + Associates Construction, Inc. Phone 386/364-1234

Address 8588 U.S. Hwy 90 Live Oak, FL 32060

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address Curtis Keen

Mortgage Lenders Name & Address NA

Circle the correct power company - (FL Power & Light) - Clay Elec. - Suwannee Valley Elec. - Progressive Ener

Property ID Number 19-3S-16-02181-001 Estimated Cost of Construction 225,000.00

Subdivision Name _____ Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 90 out of Lake City to Brown Rd. Turn right. GO 3 miles. Project at corner of Brown + Hoagle.

Type of Construction New Storage (Grain) Building Number of Existing Dwellings on Property Ø

Total Acreage 18.53 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing D

Actual Distance of Structure from Property Lines - Front See site plan Side _____ Side _____ Rear _____

Total Building Height Eve 18' Number of Stories 1 Heated Floor Area Ø Roof Pitch 1:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CBC058310

Competency Card Number 573

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 29 day of September 2010.

Personally known ✓ or Produced Identification ____

NOTARY STAMP/SEAL

LORI A. DAVIS MY COMMISSION EXPIRES August 22, 2012 #DD 781488 Bonded thru Underwriters STATE OF FLORIDA

Notary Signature

Spoke to Secretary 12-21-10

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

Matthew Ward dc WH (Owners Must Sign All Applications Before Permit Issuance.)

Owners Signature **"OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20___.

Personally known_____ or Produced Identification_____

SEAL:

State of Florida Notary Signature (For the Contractor)

FLORIDA DEPARTMENT OF STATE
DIVISION OF CORPORATIONS

Detail by Entity Name

Florida Profit Corporation

COLUMBIA GRAIN & INGREDIENTS, INC.

Filing Information

Document Number	J83738
FEI/EIN Number	592826416
Date Filed	07/17/1987
State	FL
Status	ACTIVE
Last Event	AMENDMENT
Event Date Filed	03/18/1999
Event Effective Date	NONE

Principal Address

3830 BROWN ROAD NW
LAKE CITY FL 32055

Changed 01/26/2009

Mailing Address

P.O. BOX 315
WELLBORN FL 32094

Changed 04/04/1989

Registered Agent Name & Address

WARD, H. MATTOX, JR.
3830 BROWN ROAD NW
LAKE CITY FL 32055 US

Address Changed: 01/26/2009

Officer/Director Detail

Name & Address

Title P

WARD, H. MATTOX, JR.
3830 BROWN ROAD NW
LAKE CITY FL 32055

Title V

HAZEN, JACK E., SR.
13870 SW 175TH AVE
BROOKER FL 32622

Title COO

HAZEN, JACK E JR
17057 CR 49
WELLBORN FL 32094

Annual Reports

Report Year	Filed Date
2008	03/10/2008
2009	01/26/2009
2010	03/31/2010

Document Images

03/31/2010 -- ANNUAL REPORT	View image in PDF format
01/26/2009 -- ANNUAL REPORT	View image in PDF format
03/10/2008 -- ANNUAL REPORT	View image in PDF format
03/15/2007 -- ANNUAL REPORT	View image in PDF format
04/27/2006 -- ANNUAL REPORT	View image in PDF format
02/11/2005 -- ANNUAL REPORT	View image in PDF format
02/19/2004 -- ANNUAL REPORT	View image in PDF format
01/23/2003 -- ANNUAL REPORT	View image in PDF format
02/07/2002 -- ANNUAL REPORT	View image in PDF format
01/19/2001 -- ANNUAL REPORT	View image in PDF format
01/21/2000 -- ANNUAL REPORT	View image in PDF format
03/18/1999 -- Amendment	View image in PDF format
01/30/1999 -- ANNUAL REPORT	View image in PDF format
01/29/1998 -- ANNUAL REPORT	View image in PDF format
09/26/1997 -- AMENDMENT	View image in PDF format
02/13/1997 -- ANNUAL REPORT	View image in PDF format
01/26/1996 -- ANNUAL REPORT	View image in PDF format
01/26/1995 -- ANNUAL REPORT	View image in PDF format

Note: This is not official record. See documents if question or conflict.

Previous on List Next on List Return To List Entity Name Search
Events **No Name History** Submit

| Home | Contact us | Document Searches | E-Filing Services | Forms | Help |

Columbia County Property Appraiser

DB Last Updated: 8/5/2010

2009 Tax Roll Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator | Interactive GIS Map | Print

Parcel: 19-3S-16-02181-001

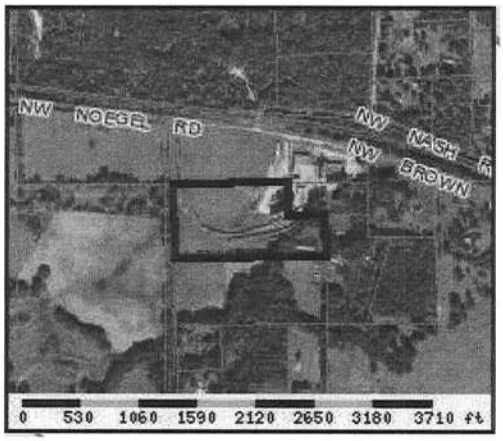
<< Next Lower Parcel | Next Higher Parcel >>

Search Result: 1 of 1

Owner & Property Info

Owner's Name	COLUMBIA GRAIN & INGREDIENTS		
Mailing Address	INC P O BOX 315 WELLBORN, FL 32094		
Site Address			
Use Desc. (code)	IMP AG/BAR (005020)		
Tax District	3 (County)	**Neighborhood**	19316
Land Area	19.210 ACRES	**Market Area**	01
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		

NE1/4 OF NW1/4 & NW1/4 OF NE1/4, ALL AS LIES SOUTH OF FAIRFIELD RD EX 18.53 AC DESC IN ORB 541-660 & EX 7.73 AC DESC IN ORB 561-624 & 477-371 & 523-193 EX E 349.24 FT & EX 19.74 AC & EX A PART OF 9.89 AC DESC ORB 633-181. ORB 638-602 & WD 1065-1130, WD 1140-935,



Property & Assessment Values

2009 Certified Values		
Mkt Land Value	cnt: (1)	$0.00
Ag Land Value	cnt: (0)	$3,842.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (1)	$6,430.00
Total Appraised Value		$10,272.00
Just Value		$109,904.00
Class Value		$10,272.00
Assessed Value		$10,272.00
Exempt Value		$0.00
Total Taxable Value		Cnty: $10,272 Other: $10,272 | Schl: $10,272

2010 Working Values

NOTE:
2010 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
12/13/2007	1140/935	WD	I	U	01	$785,000.00
11/2/2005	1065/1130	WD	I	U	03	$1,240,200.00
11/23/1987	638/602	WD	I	U		$75,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0020	BARN,FR	0	$6,430.00	0000001.000	0 x 0 x 0	(000.00)

Land Breakdown

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER _____ CONTRACTOR Fred Watkins / G+A PHONE 386-364-123

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #:	Signature _____ Phone #:
MECHANICAL/ A/C _____	Print Name _____ License #:	Signature _____ Phone #:
PLUMBING/ GAS	Print Name _____ License #:	Signature _____ Phone #:
ROOFING	Print Name _____ License #:	Signature _____ Phone #:
SHEET METAL	Print Name _____ License #:	Signature _____ Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name _____ License#:	Signature _____ Phone #:
SOLAR	Print Name _____ License #:	Signature _____ Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	N/A		
CONCRETE FINISHER		Gamble & Associates	Fred Watkins
FRAMING	N/A		
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR	N/A		
METAL BLDG ERECTOR		Gamble & Associates	Fred Watkins

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Columbia County Property Appraiser

J. Doyle Crews - Lake City, Florida 32055 | 386-758-1083

PARCEL: 19-3S-16-02181-001 - IMP AG/BAR (005020)

NE1/4 OF NW1/4 & NW1/4 OF NE1/4, ALL AS LIES SOUTH OF FAIRFIELD RD EX 18.53 AC DESC IN ORB 541-660 & EX 7.73 AC DESC IN ORB 561-624 & 477-371 & 523-19

Name: COLUMBIA GRAIN & INGREDIENTS

Site:

Mail: INC
P O BOX 315
WELLBORN, FL 32094

Sales	12/13/2007	$785,000.00	I / U
Info	11/2/2005	$1,240,200.00	I / U

2009 Certified Values

Land	$0.00
Bldg	$0.00
Assd	$10,272.00
Exmpt	$0.00
Taxbl	Cnty: $10,272
	Other: $10,272 | Schl: $10,272

NOTES:

Gamble & Associates Construction Inc.

8588 U.S. Hwy. 90•Live Oak, FL 32060•
386/364-1234 386/364-3514 Fax
RB 0047309 CB C058310 QB 32066

September 29, 2010

Agent Authorization for Columbia Grain & Ingredients-Columbia County

Attention: Permitting Department

Please be advised that I hereby authorize Fred J. Hatfield III of Gamble & Associates Construction Company, Inc. and/or Curtis Keen, P.E., to act as my agents/representatives in obtaining the necessary permits for my project.

Thank you,

Mattox Ward

Mattox Ward
Columbia Grain & Ingredients

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness Name: Ruth H. Davis

Witness Name: James J. Taylor Jr.

COLUMBIA GRAIN & INGREDIENTS LLC, a Florida limited liability company

By _____
Jack E. Hazen
Its Vice President

State of Florida
County of Bradford

The foregoing instrument was acknowledged before me this 13th day of December, 2007, by Jack E. Hazen, as vice president of Columbia Grain & Ingredients LLC, for an on behalf of said ~~limited~~ liability company, who [X] is personally known or [ ] has shown a driver license as identification.

[Notary Seal] James J. Taylor, Jr.
Commission # DD582501
Expires October 1, 2010
Bonded Troy Fain - Insurance, Inc 800-385-7019

_____
Notary Public

Printed Name: _____

My Commission Expires: _____

EXHIBIT A

Parcel 1

TOWNSHIP 3 SOUTH - RANGE 16 EAST

Sections 18 & 19: Commence at the Southeast Corner of the SW¼ of Section 18, Township 3 South, Range 16 East, Columbia County, Florida and run N89°23'30"W along the South line of the SE¼ of SW¼, Section 18 a distance of 330.00 feet for a POINT OF BEGINNING; thence N0°21'22"E, 276.77 feet to the South Right of Way of Fairfield Road; thence S81°42'30"E along said South Right of Way line of Fairfield Road 93.66 feet to the Point of curve of a curve concave to the right having a radius of 5897.58 feet; thence Southeasterly along the arc of said curve 621.52 feet; thence S0°21'22"W, 548.81 feet; thence N89°21'30"W, 702.96 feet; thence N0°21'22"E 300.00 feet to the POINT OF BEGINNING.

Parcel 2

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTIONS 18 & 19: Part of the SE¼ and part of the SW¼ of Section 18, and part of the NW¼ of the NE¼ and part of the NE¼ of the NW¼ of Section 19, all in Township 3 South, Range 16 East, Columbia County, Florida for POINT OF BEGINNING; commence at the Southeast corner of said NE¼ of the NW¼, thence run S 88°48'45"W along the South line of said NE¼ of the NW¼, a distance of 1245.88 feet to the East right of way line of County Road No. 135 (Noegels Road), a 66 foot right of way; thence run N 00°18'35"E along said East right of way line, a distance of 1260.35 feet; thence continue along said East right of way line, North 01°58'22"W, a distance of 532.33 feet to the South right of way line of Fairfield Road, a 50 foot right of way; thence run S 81°42'30"E along said South right of way line, a distance of 1141.30 feet to the Point of Curve of a curve concaved Southwesterly have a radius of 5897.58 feet; thence run along and around said curve, a chord bearing and distance of S 73°29'15"E, 1686.37 feet to the East line of said NW¼ of the NE¼; thence run S 00°54'22"E along said East line a distance of 1180.50 feet; thence run N 87°31'02"W along the South line of said NW¼ of the NE¼, a distance of 1409.69 feet to the POINT OF BEGINNING.

LESS AND EXCEPT:

SECTIONS 18 & 19: Commence at the Southeast corner of the SW¼ of Section 18, Township 3 South, Range 16 East, Columbia County, Florida, and run N 89°23'30"W along the South line of the SE¼ of SW¼, Section 18 a distance of 330.00 feet for a POINT OF BEGINNING; thence N 0°21'22"E, 276.77 feet to the South right of way of Fairfield Road; thence S 81°42'30"E along said South right of way line, Fairfield Road 93.66 feet to the Point of Curve of a curve concave to the right having a radius of 5897.58 feet; thence Southeasterly along the arc of said curve 621.52 feet; thence S 0°21'22"W, 548.81 feet; thence N 89°23'30"W, 702.96 feet; thence N 0°21'22"E, 300 feet to the POINT OF BEGINNING.

Parcel 2 continued

ALSO LESS AND EXCEPT:

Part of the SE¼ of Section 18 and part of the NW¼ of the NE¼ of Section 19, Township 3 South, Range 16 East, Columbia County, Florida, being more particularly described as follows: For POINT OF BEGINNING, commence at the Southeast Corner of said NW¼ of the NE¼, thence run N 87°31'09"W along the South Line of said NW¼ of the NE¼, a distance of 349.24 feet; thence run N 00°54'22"W, a distance of 1313.94 feet to the Southerly right-of-way line of Fairfield Road, said right-of-way line being in a curve concaved Southwesterly, having a radius of 5897.58 feet; thence run along and around said curve, a chord bearing and distance of S 87°07'04"E, 381.00 feet; thence run S 00°54'22"E along the East line of said NW¼ of the NE¼, a distance of 1180.88 feet to the POINT OF BEGINNING.

ALSO LESS AND EXCEPT:

Part of the Southeast 1/4 of Section 18 and part of the Northwest 1/4 of the Northeast 1/4 of Section 19, Township 3 South, Range 16 East, Columbia County, Florida, being more particularly described as follows: For Point of Reference commence at the Southwest Corner of said NW¼ of the NE¼, thence run S 87°31'09"E, a distance of 373.20 feet; thence run N 00°21'22"E, a distance of 742.03 feet to the POINT OF BEGINNING; thence continue N 00°21'22"E, a distance of 751.34 feet to the South right-of-way line of Fairfield Road, said South right-of-way line being in a curve concaved Southwesterly having a radius of 5897.58 feet; thence run along and around said curve, a chord bearing and distance of S 72° [redacted] feet; thence run S 00°54'22"E, a distance of 549.20 feet; thence run N 89°23'30"W, a distance of 669.94 feet to the POINT OF BEGINNING.

ALSO LESS AND EXCEPT:

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 19: Part of the NE¼ of the NW¼ of Section 19, Township 3 South, Range 16 East, Columbia County, Florida, being more particularly described as follows: For POINT OF BEGINNING commence at the Southeast corner of said NE¼ of the NW¼; thence run S 88°46'45"W along the South line of said NE¼ of the NW¼, a distance of 1245.48 feet to the East right-of-way line of County Road No. 135; thence run N 00°18'35"E along said East right-of-way line, a distance of 600.00 feet; thence run N 88°46'45"E, a distance of 1245.97 feet to the East line of said NE¼ of the NW¼; thence run S 00°21'22"W along said East line, a distance of 600.00 feet to the POINT OF BEGINNING.

ALSO LESS AND EXCEPT:

TOWNSHIP 3 SOUTH - RANGE 16 EAST

SECTION 19: Part of the NW¼ of the NE¼ of Section 19, Township 3 South, Range 16 East, Columbia County, Florida, being more particularly described as follows: For POINT OF BEGINNING Commence at the Southwest corner of said NW¼ of the NE¼; thence run N 00°21'22"E along the West line of said NW¼ of the NE¼ a distance of 932.35 feet; thence run S 89°23'30"E a distance of 372.94 feet; thence run S 00°21'22"W a distance of 202.53 feet; thence run S 89°23'30"E a distance of 669.94 feet; thence run S 00°54'22"E a distance of 764.73 feet; thence run N 87°31'09"W along the South line of said NW¼ of the NE¼ a distance of 1060.48 feet to the POINT OF BEGINNING.

Prepared by and return to:
James J. Taylor Jr.
Taylor & Taylor P.A.
Post Office Box 2000
Keystone Heights, FL 32656
352-473-8088

Inst:200812000433 Date:1/9/2008 Time:1:50 PM
Doc Stamp-Deed:5495.00
DC,P.DeWitt Cason,Columbia County Page 1 of 4

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this 13th day of **December, 2007,** between **Columbia Grain & Ingredients LLC, a Florida limited liability company,** whose post office address is **Post Office Box 315, Wellborn, Florida 32094,** grantor, and **Columbia Grain & Ingredients, Inc., a Florida corporation,** whose post office address is **Post Office Box 315, Wellborn, Florida 32094,** grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable consideration to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Columbia County, Florida,** to-wit:

See attached Exhibit A which by this reference is incorporated herein and made a part hereof.

Parcel ID Nos. R02181-001, R02180-003, R02181-013 and R02180-000

Grantor hereby also assigns, sets over, transfers and conveys to grantee all of grantor's interests as lessor of the above described property under that certain Commercial Lease Agreement made effective as of July 30, 2005, by and between grantor, as lessor, and Hillandale, LLC, as lessee, subject to the terms and conditions thereof.

Subject to covenants, restrictions, reservations and easements of record, if any, and taxes for 2008 and subsequent years.

This instrument prepared without benefit of title examination, title insurance or attorney's opinion of title.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

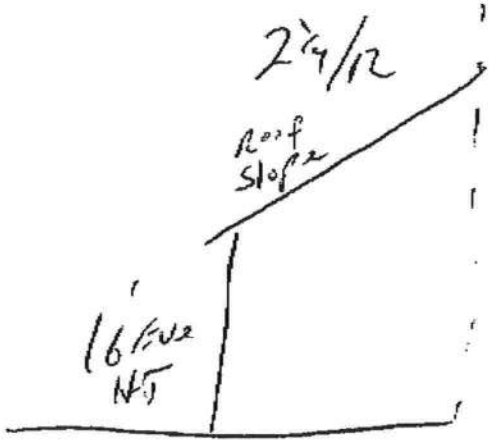
And the grantor hereby covenants with grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; and that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever.

Gamble Construction

- Job - Columbia Grain

150

200



A&S Building Systems

LETTER OF TRANSMITTAL

Date: 11/08/2010 Customer #: **43119**

Attn: JAY HATFIELD
Company: GAMBLE & ASSOCIATES CONST
Address: 8588 US HWY 90
LIVE OAK, FL 32060
Phone: 386 364-1234

RECEIVED
BY DATE
NOV 16 2010

Job Number: **2251897**
Project Name COLUMBIA GRAIN
Jobsite: LAKE CITY, FL

Project Manager:: BUTLER, PHILIP W.
Phone #: 865-425-2010
Email: pbutler@a-s.com

The enclosed package will include drawings and/or documentation as listed below:

Quantity		Description
With Seal	**Without Seal**	
1	0	Letter Of Certification
		Bldg A
1	0	Set(s) Of Calculations
		Bldg A

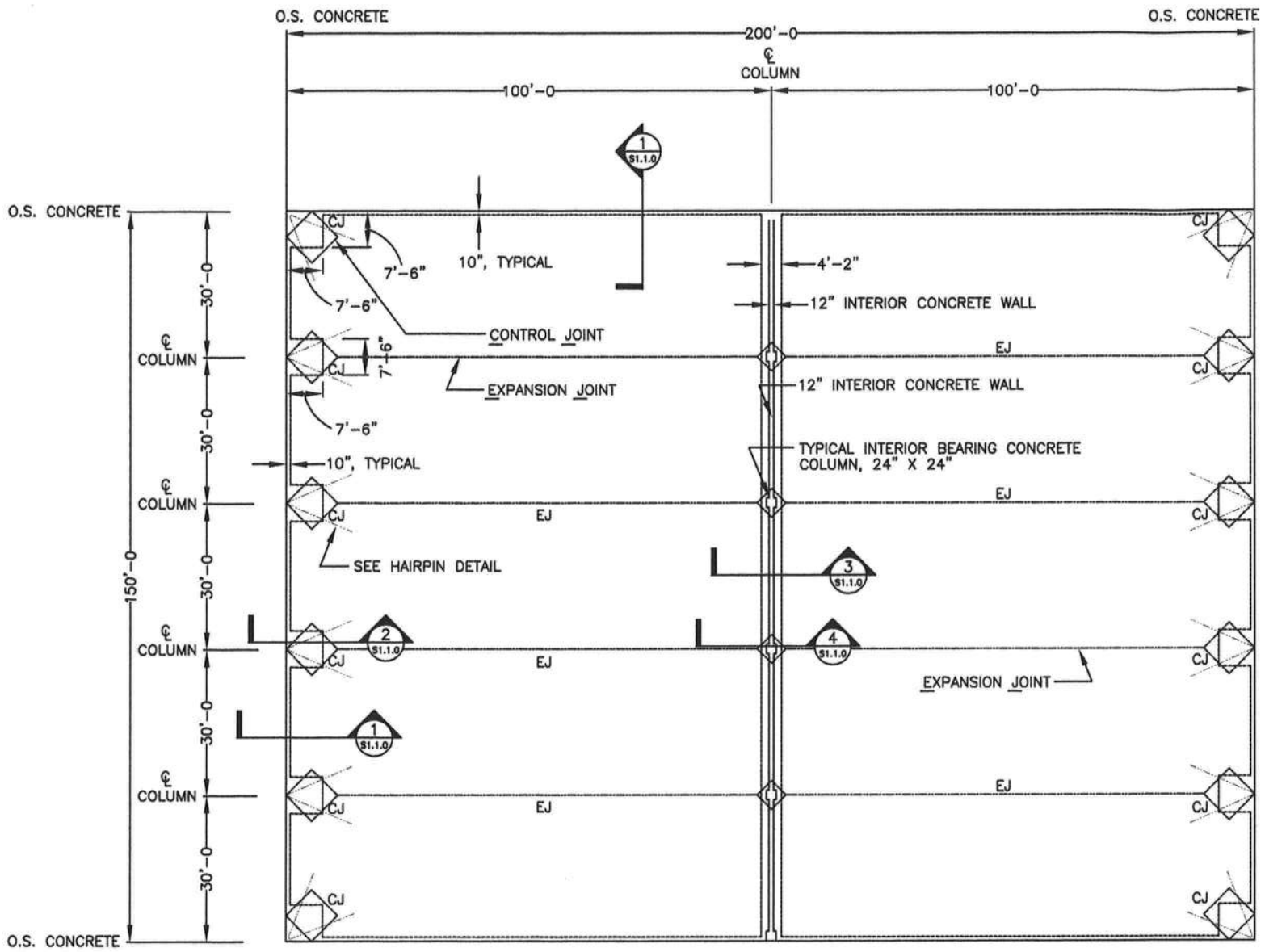
If you have any questions or fail to receive any of the above, please contact us immediately.

Drawing Issue: 0
Ship Code: FedEx ESaver

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

NOTE: CONTRACTOR SHALL REFER TO STEEL BUILDING MANUFACTURER'S COLUMN LOCATIONS, RELATED DIMENSIONS & LIST(S) OF REQUIRED COMPONENTS FOR PROPER COORDINATION W/ THESE PLANS

SCALE NOTE:
PLAN VIEW: N.T.S.



1 S1.0.0 FOUNDATION PLAN VIEW
SCALE: N.T.S.

CONCRETE & RELATED REQUIREMENTS

CONCRETE

1. CONCRETE CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318," & "MANUAL CONCRETE PRACTICE, PART 1 ACI 305 & ACI 306," & MANUAL OF CONCRETE PRACTICE, PART 1 ACI 305 & 306" LATEST EDITION
2. CEMENT FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 150
3. AGGREGATES FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C 33
4. WATER FOR CONCRETE SHALL BE POTABLE WATER FROM MUNICIPAL MAINS OR PRIVATE WELL.
5. OPTIONAL: TEST CONCRETE FOR COMPRESSION WITH 1 SET OF 3 CYLINDERS FOR EACH 50 CUBIC YARDS OF CONCRETE PLACED ON A GIVEN DAY. BREAK 1 CYLINDER @ 7 DAYS AND THE OTHERS @ 28 DAYS. TESTING WILL BE PAID FOR BY OWNER.
6. CONCRETE SHALL HAVE STRENGTHS AND CHARACTERISTICS AS INDICATED ELSEWHERE THESE PLANS
7. SAWED JOINTS MUST BE SAWED WITHIN 24 HOURS OF PLACEMENT OF CONCRETE
8. REINFORCING STEEL SHALL MEET THE REQUIREMENTS OF ASTM A615 GR 60 UNLESS OTHERWISE NOTED
9. NOT USED
10. NOT USED
11. SLAB REINFORCING SHALL BE IN TOP 1/2 OF SLAB OR AS ILLUSTRATED
12. VIBRATE OR SCREEN ALL CONCRETE THOROUGHLY INTO PLACE
13. MINIMUM COVER OF REINFORCEMENT SHALL BE AS REQUIRED BY CODE
14. MOIST CURE CONCRETE FOR 7 DAYS AFTER PLACING
15. PROVIDE VAPOR BARRIER OF POLYETHYLENE UNDER SLAB(S)
16. PLACE CONTROL JOINTS IN SLAB TO PROVIDE MAXIMUM SLAB SIZE OF 600 SQUARE FEET
17. CONCRETE TEMPERATURE SHALL NOT EXCEED 90 DEGREES F DURING PLACEMENT
18. CONCRETE SHALL BE PLACED IN A MANNER TO PREVENT SEGREGATION
19. CONCRETE SHALL NOT BE ALLOWED TO FREE FALL MORE THAN 60 INCHES
20. AREAS TO RECEIVE CONCRETE SHALL BE CLEAR OF ANY DEBRIS AND SHALL HAVE REINFORCING STEEL PROPERLY POSITIONED PRIOR TO CONCRETE PLACEMENT
21. FOR LOCATION OF CONTROL OR CONSTRUCTION JOINTS OTHER THAN THOSE ILLUSTRATED VERIFY W/ ENGINEER
22. NOT USED
23. ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A 307
24. ANCHOR BOLTS AND DOWELS SHALL BE SET IN SUCH A MANNER THAT THEIR FULL EMBEDDED LENGTH SHALL BE COVERED WITH CONCRETE
25. LAP SPLICES SHALL BE 40 BAR DIAMETERS OR AS SHOWN OR NOTED ELSEWHERE THESE PLANS
26. DETAILING, FABRICATION AND PLACEMENT OF REINFORCEMENT STEEL SHALL CONFORM TO CURRENT CRSI AND ACI SPECIFICATIONS
27. REINFORCING STEEL SHALL BE FREE OF LOOSE RUST, MIL SCALE AND COATINGS THAT WOULD REDUCE OR DESTROY BOND
28. REINFORCING BARS SHALL NOT BE REDUCED IN SECTION, KINKED OR BENT OTHER THAN INDICATED
29. NOT USED
30. SUPPORT REINFORCING STEEL IN CHAIRS
31. KEEP ONE SET OF CONCRETE CYLINDERS ON SITE AT ALL TIMES TO MAKE SAMPLES IN CASE CONCRETE CHARACTER CHANGES

REINFORCING STEEL

1. REINFORCING STEEL SHALL BE #5 UNLESS OTHERWISE NOTED
2. ALL REINFORCING STEEL SHALL BE A MINIMUM OF GRADE 40 AND IDENTIFIED IN ACCORDANCE W/ ASTM A615, A616, A617 OR A 706
3. SPLICES SHALL BE LAP SPLICES W/ A MINIMUM OF 25" FOR #5 BARS
4. FOR MINIMUM COVER OVER REINFORCEMENT – SEE DETAILS & SECTIONS ELSEWHERE THESE PLANS
5. ALL REINFORCEMENT IN CMU'S SHALL EXTEND A MINIMUM OF 6" INTO ALL FOOTINGS W/ A 6" STANDARD BEND

METAL ACCESSORIES

1. ALL JOINT REINFORCEMENT & ANCHOR TIES SHALL CONFORM TO ASTM A36 & A366 AS REQUIRED
2. LONGITUDINAL WIRES OF JOINT REINFORCEMENT SHALL BE FULLY EMBEDDED IN MORTAR OR GROUT W/ A MINIMUM COVER OF 5/8" WHEN EXPOSED TO EARTH OR WEATHER AND A MINIMUM OF 1/2" WHEN NOT EXPOSED TO EARTH OR WEATHER
3. METAL ACCESSORIES USED IN EXTERIOR WALL CONSTRUCTION SHALL BE GALVANIZED IN ACCORDANCE W/ ASTM A153 CLASS B–2
4. METAL ACCESSORIES USED IN INTERIOR WALL CONSTRUCTION SHALL BE MILL GALVANIZED IN ACCORDANCE W/ ASTM A641, CLASS 1

FILL COMPACTION

1. PRIOR TO GRADING OPERATIONS ALL SOIL, ORGANIC LITTER AND FILL SHALL BE STRIPPED FROM BUILDING AREA
2. COMPACTION SHALL NOT BE LESS THAN 98% OF THE STANDARD PROCTOR DENSITY
3. ALL FILL MATERIAL SHALL BE INORGANIC W/ NOT MORE THAN 30% BY WEIGHT FINER THAN 200 U.S. STANDARD SIEVE CONFORMING TO
   A. LIQUID LIMIT, LW.....30 MAXIMUM
   B. ELASTICITY, LW.....15, MAXIMUM
   C. DRY UNIT WEIGHT.....100 LBS. PER CU. FT.
4. ALL FILL MATERIAL SHALL BE UNIFORMLY PLACED @ OPTIMUM MOISTURE CONTENT IN 6" UNIFORM LAYERS AND COMPACTED TO A DENSITY OF 98% OF THE STANDARD PROCTOR IN ACCORDANCE W/ ASTM D698T
5. FOOTINGS EXCAVATIONS SHALL BE INSPECTED PRIOR TO PLACING ANY CONCRETE TO ENSURE THAT FOOTINGS REST UPON SOUND EARTH
6. ALL SUBGRADES MUST BE LEVEL, SMOOTH AND UNIFORMLY COMPACTED
7. SUB GRADE MUST BE ACCURATE WITHIN 1/4" OF THE DESIGNATED LEVEL
8. ANY WALL WHICH IS TO RECEIVE BACK FILL ON BOTH SIDES SHALL HAVE THE BACK FILL PLACED SIMULTANEOUSLY ON BOTH SIDES IN EVEN LAYERS AS PREVIOUSLY DESCRIBED SO AS NOT TO APPLY UNEVEN LOADS

GENERAL

1. FOOTINGS SHALL BE LEVEL OR STEPPED AS INDICATED ON PLAN VIEWS & DETAILS OR SECTIONS
2. SOIL, WASTE PIPES OR BUILDING DRAINS PASSING UNDER A FOOTING OR THROUGH A FOUNDATION SHALL BE PROVIDED W/ A RELIEVING ARCH OR AN IRON PIPE SLEEVE A MINIMUM OF 2 – PIPE SIZES GREATER THAN THE PIPE PASSING THROUGH
3. STEM WALLS SHALL EXTEND NO GREATER THAN 3 FEET ABOVE THE FINISH GRADE AND CONSTRUCTED W/ THE PREVIOUSLY DESCRIBED MASONRY UNITS
4. ALL STATE AND LOCAL CODES SHALL BE COMPLIED WITH BY THE CONTRACTOR
5. 2,000 P.S.F. SOLID BEARING PRESSURE SHALL BE OBTAINED UNDER ALL FOOTINGS & SLABS

CERTIFICATION:

THESE FOUNDATION PLANS FOR COLUMBIA GRAIN & INGREDIENTS WILL COMPLY WITH SECTION 1600 OF THE FLORIDA BUILDING CODE, 2007 EDITION FOR A 110 MPH WIND LOAD, 3 SECOND GUST, EXPOSURE B, WITH THE INTERNAL PRESSURE OF + 0.18 AND – 0.18 INCLUDED IN THESE LOADS.
BASED ON REACTIONS BY A & S BUILDING SYSTEMS JOB #2251897.

Curtis E. Keen 12/07/10
CURTIS E. KEEN, PE #23836

PROJECT No. GAMBLE–S1.0.0.DWG	DATE 12/03/10	DIMENSIONED FOUNDATION PLAN VIEW MISC. NOTES, REFERENCES & INSTRUCTIONS	Curtis E. Keen, PE #23836 Certification of Authorization #3761 DATE:	COLUMBIA GRAIN & INGREDIENTS COLUMBIA COUNTY, FLORIDA
SHEET No. S1.0.0				

KEEN ENGINEERING & SURVEYING, INC.
9263 CR 417
LIVE OAK, FLORIDA 32060
386–362–4787
ENG. LIC. EB 3761

ENGINEERING DATA

Gamble and Associates Contruction
Job # 2251897
Pages 1 - 24
1 of 1
11/8/10

A&S BUILDING SYSTEMS

CARYVILLE, TENNESSEE 37714

A&S Building Systems
An NCI Company

IAS ACCREDITED

November 08, 2010

Gamble & Associates Construction
8588 US Highway 90 West
Live Oak, FL 32060

22-T-51897
Columbia Grain
Lake City, FL
200'0" x 150'0" x 16'0"

To Whom It May Concern:

This is to certify that materials for the subject structure have been designed in accordance with the order documents, specifically as shown per the attached Engineering Design Criteria Sheet.

Aspects of code compliance as related to use or occupancy, such as sprinkler requirements, are not addressed by these documents.

These materials, when properly erected on an adequate foundation in accordance with the erection drawings as supplied and using the components as furnished, will meet the attached loading requirements.

This certification does not cover field modifications or the design of materials not furnished by A&S Building Systems.

The attached design criteria information is to remain with and form part of this Letter of Certification.

The calculations and the metal building they represent are the product of A&S Building Systems or a division of its affiliate NCI Building Systems. The engineer whose seal appears hereon is employed by either A&S Building Systems or a division of its affiliate NCI Building Systems and is not the engineer of record for this project.

Cordially,

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

A&S Building Systems
Materials for Metal Buildings
An NCI Company

AARON K. BATCHELLOR
LICENSE
No. 67187
STATE OF FLORIDA
PROFESSIONAL ENGINEER
11-11-10

Aaron K. Batchellor, P.E.
Manager of Design

Real People. Real Products. Real Solutions.
Phone: 865-426-2141 Fax: 865-426-2011 - 1880 Hwy 116 Caryville, Tennessee 37714

Building Code	2007 Florida with 2009 Amendments
Occupancy Category	Normal (Category II)
Roof Dead Load	
Superimposed	2.59 psf
Collateral	2.00 psf
(0.00 psf Ceiling 2.00 psf Other)	
Roof Live Load	20.00 psf reduction allowed

Wind

Basic Wind Speed	110.00 mph
Wind Importance Factor (I) ...	1.00
Wind Exposure Category	B
Internal Pressure Coef (GCpi)	0.18/-0.18

Loads for components not provided by building manufacturer

Corner Areas (within 10.15' of corner)	21.76 psf pressure -29.14 psf suction
Other Areas	21.76 psf pressure -23.61 psf suction

These values are the maximum values required based on a 10 sq ft area.
Components with larger areas may have lower wind loads.

Real People. Real Products. Real Solutions.
Phone: 865-426-2141 Fax: 865-426-2011 - 1880 Hwy 116 Caryville, Tennessee 37714

A&S BUILDING SYSTEMS

STRUCTURAL CALCULATIONS

BUILDER: **Gamble & Associates Construction**
CUSTOMER: **Columbia Grain**
JOB NUMBER: **22-T-51897**

TABLE OF CONTENTS

Original Design Completed thru Change Order # 0
Revision History

Revision	Reason for Revision	Pages Revised	Date Revised

Project Engineer: JOHN FRENCH
Checking Engineer: AARON BATCHELLOR
Signing Engineer:

Job Number 22-T-51897
Builder ..... Gamble & Associates Construction
Jobsite Location ..... LAKE CITY, FL

Building Code ..... 2007 Florida with 2009 Amendments
Occupancy Category ..... Normal (Category II)
Roof Dead Load
Superimposed ..... 2.59 psf
Collateral ..... 2.00 psf
(0.00 psf Ceiling 2.00 psf Other)
Roof Live Load ..... 20.00 psf reduction allowed

Wind
Basic Wind Speed ..... 110.00 mph
Wind Importance Factor (I) ... 1.00
Wind Exposure Category ..... B
Internal Pressure Coef (GCpi) 0.18/-0.18
Loads for components not provided by building manufacturer
Corner Areas (within 10.15' of corner) 21.76 psf pressure -29.14 psf suction
Other Areas 21.76 psf pressure -23.61 psf suction
These values are the maximum values required based on a 10 sq ft area.
Components with larger areas may have lower wind loads.

"Bracing size" as noted on Engineering documents and Erection drawings denotes thread diameter for rod bracing and wire strand cable diameter for wire strand cable bracing.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the "Specification for Structural Joints Using ASTM A325 or A490 Bolts, June 30, 2004". Pretensioning methods, including turn-of-nut and calibrated wrench are NOT required.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

X-Bracing is to be installed to a taut condition with all slack removed. Do not tighten beyond this state.

The cold form members designed by the building manufacturer do not provide structural support at the top of the wall system not furnished by the building manufacturer. Applies to building A wall SWA, SWC, and EWD.

Builder :
Gamble & Associates Cons
Job No: 51897A run01
Version: ver01-jfrench
Fri Nov 5 08:45:59 2010

key Struts x=double Z,
xx=triple Z,
o=pipe(FM)

Owner :
Columbia Grain
LAKE CITY FL 32055
P.O. 2251897

M A I N B U I L D I N G D E S I G N S U M M A R Y R E P O R T

ROOF PLANE ------- RPA
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AroofRPA_01.edf

Panel PBR26
Purlins ..... 55.0 ksi Yield Strength
Eave Struts ..... 55.0 ksi Yield Strength

PURLIN SPACING : 2@3'0 4'0 15@5'0 2@4'9 4'3-9/16 1'2-7/16

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	26.000	8X2.5Z12	None	S 0.000	2.479 C
2	32.667	8X2.5Z13	None	C 2.479	3.146 C
3	32.667	8X2.5Z13	None	C 3.146	3.146 C
4	32.667	8X2.5Z13	None	C 3.146	2.479 C
5	26.000	8X2.5Z12	None	C 2.479	0.000 S

Purlin Clip Use 4 A307 Bolts @ Level 2-22 @ Frame Lines:1-6

Purlin Stiffened Clips @ Level 22 @ Frame Lines:1-6

ROOF PLANE ------- RPC
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AroofRPC_01.edf

Panel PBR26
Purlins ..... 55.0 ksi Yield Strength
Eave Struts ..... 55.0 ksi Yield Strength

PURLIN SPACING : 2@3'0 4'0 15@5'0 2@4'9 4'3-9/16 1'2-7/16

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	26.000	8X2.5Z12	None	S 0.000	2.479 C
2	32.667	8X2.5Z13	None	C 2.479	3.146 C
3	32.667	8X2.5Z13	None	C 3.146	3.146 C
4	32.667	8X2.5Z13	None	C 3.146	2.479 C
5	26.000	8X2.5Z12	None	C 2.479	0.000 S

Purlin Clip Use 4 A307 Bolts @ Level 2-22 @ Frame Lines:1-6

Purlin Stiffened Clips @ Level 22 @ Frame Lines:1-6

SWC Eave Strut @ 16.000 (ft) : 8X3.5E14 @ Bays 1-5;
SWA Eave Strut @ 16.000 (ft) : 8X3.5E14 @ Bays 1-5;

Eave strut interior connection at SWA use (4)-1/2" A307 bolts w/SC-18 plate.
Eave strut interior connection at SWC use (4)-1/2" A307 bolts w/SC-18 plate.

Eave strut connection at end-frame use (4)-1/2" A307 bolts.

SIDEWALL PLANE SWA -- (8.250" Inset columns)
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AwallSWA_01.edf

Panel PBR26
Girts ..... 55.0 ksi Yield Strength

GIRTS SPACINGS : 11'6

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	8'0	26.000	8X2.5C16	None	S 0.000	0.000 S
2	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
3	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
4	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
5	8'0	26.000	8X2.5C16	None	S 0.000	0.000 S
1	11'6	26.000	8X2.5Z16	None	S 0.000	2.479 C
2	11'6	32.667	8X2.5Z14	None	C 2.479	1.479 C
3	11'6	32.667	8X2.5Z14	None	C 1.479	1.479 C
4	11'6	32.667	8X2.5Z14	None	C 1.479	2.479 C
5	11'6	26.000	8X2.5Z16	None	C 2.479	0.000 S

OPEN AREAS:

Size	Wall	Distance
150'0 x 8'0	SWA	0'0

SIDEWALL PLANE SWC -- (8.250" Inset columns)
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AwallSWC_01.edf

Panel PBR26
Girts ..... 55.0 ksi Yield Strength

GIRTS SPACINGS : 11'6

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	8'0	26.000	8X2.5C16	None	S 0.000	0.000 S
2	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
3	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
4	8'0	32.667	8X2.5C13	None	S 0.000	0.000 S
5	8'0	26.000	8X2.5C16	None	S 0.000	0.000 S
1	11'6	26.000	8X2.5Z16	None	S 0.000	2.479 C
2	11'6	32.667	8X2.5Z14	None	C 2.479	1.479 C
3	11'6	32.667	8X2.5Z14	None	C 1.479	1.479 C
4	11'6	32.667	8X2.5Z14	None	C 1.479	2.479 C
5	11'6	26.000	8X2.5Z16	None	C 2.479	0.000 S

OPEN AREAS:

Size	Wall	Distance
150'0 x 8'0	SWC	0'0

Endwall Plane EWB Design ..... Bearing Frame (BF)
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AwallEWB_01.edf

Panel PBR26

RAFTERS -----

Mem #	Description Member Size Identification	Length (ft)	Start (ft)	End (ft)
1	W10X12 50.0 ksi	27.280	0.000	27.280
	connections... Left: Type-V SEP		Right: Type-II MEP	
2	W10X12 50.0 ksi	25.436	27.280	52.715
	connections... Left: Type-II MEP		Right: Type-II MEP	
3	W10X12 50.0 ksi	47.819	52.715	100.534
	connections... Left: Type-II MEP		Right: Type-III	
4	W10X12 50.0 ksi	47.819	100.534	148.353
	connections... Left: Type-III		Right: Type-II MEP	
5	W10X12 50.0 ksi	25.436	148.353	173.789
	connections... Left: Type-II MEP		Right: Type-II MEP	
6	W10X12 50.0 ksi	27.280	173.789	201.069
	connections... Left: Type-II MEP		Right: Type-V SEP	

Type-II MEP = (4)-1/2" A325N bolts w/ 3/8" Moment End Plate
Type-V SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate
Type-III = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWA: **ALL, except omit at 3.0, 6.0, 35.0, 40.0, 60.0, 65.0**
PLANE SWC: **ALL, except omit at 3.0, 6.0, 35.0, 40.0, 60.0, 65.0**

(omit any rafter flange brace that fouls with column extension)

Girts ..... 55.0 ksi Yield Strength

Girts Spacings : 3'6 3'10 4'2 2@5'9 5'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten	
1	3'6	25.000	8X2.5Z13	None	S 0.000	2.479	C
2	3'6	25.000	8X2.5Z16	None	C 2.479	1.479	C
3	3'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
4	3'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
5	3'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
6	3'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
7	3'6	25.000	8X2.5Z16	None	C 1.479	2.479	C
8	3'6	25.000	8X2.5Z13	None	C 2.479	0.000	S

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	7'4	25.000	8X2.5Z13	None	S 0.000	2.479 C
2	7'4	25.000	8X2.5Z16	None	C 2.479	1.479 C
3	7'4	25.000	8X2.5Z16	None	C 1.479	1.479 C
4	7'4	25.000	8X2.5Z16	None	C 1.479	1.479 C
5	7'4	25.000	8X2.5Z16	None	C 1.479	1.479 C
6	7'4	25.000	8X2.5Z16	None	C 1.479	1.479 C
7	7'4	25.000	8X2.5Z16	None	C 1.479	2.479 C
8	7'4	25.000	8X2.5Z13	None	C 2.479	0.000 S
1	11'6	25.000	8X2.5Z13	None	S 0.000	2.479 C
2	11'6	25.000	8X2.5Z16	None	C 2.479	1.479 C
3	11'6	25.000	8X2.5Z16	None	C 1.479	1.479 C
4	11'6	25.000	8X2.5Z16	None	C 1.479	1.479 C
5	11'6	25.000	8X2.5Z16	None	C 1.479	1.479 C
6	11'6	25.000	8X2.5Z16	None	C 1.479	1.479 C
7	11'6	25.000	8X2.5Z16	None	C 1.479	2.479 C
8	11'6	25.000	8X2.5Z13	None	C 2.479	0.000 S
1	17'3	25.000	8X2.5Z16	None	S 0.000	1.479 C
2	17'3	25.000	8X2.5Z16	None	C 1.479	2.479 C
3	17'3	25.000	8X2.5Z16	None	C 2.479	1.479 C
4	17'3	25.000	8X2.5Z16	None	C 1.479	1.479 C
5	17'3	25.000	8X2.5Z16	None	C 1.479	1.479 C
6	17'3	25.000	8X2.5Z16	None	C 1.479	2.479 C
7	17'3	25.000	8X2.5Z16	None	C 2.479	1.479 C
8	17'3	25.000	8X2.5Z16	None	C 1.479	0.000 S
2	23'0	25.000	8X2.5Z16	None	S 0.146	0.146 S
3	23'0	25.000	8X2.5Z12	None	S 0.146	2.479 C
4	23'0	25.000	**8X2.5Z12**	None	C 2.479	1.479 C
5	23'0	25.000	**8X2.5Z12**	None	C 1.479	2.479 C
6	23'0	25.000	8X2.5Z12	None	C 2.479	0.146 S
7	23'0	25.000	8X2.5Z16	None	S 0.146	0.146 S
4	28'0	25.000	8X2.5Z13	None	S 0.146	1.479 C
5	28'0	25.000	8X2.5Z13	None	C 1.479	0.146 S

Note : Maximum distance to extend girt from 'adjacent bay is 36.00 inches.

FRAMED OPENINGS:

Qty	Size	Jamb & Header	Wall	Bay	Distance
1	20'0 x 20'0	C8X11.5	EWB	4	2'6
1	20'0 x 20'0	C8X11.5	EWB	5	2'6
~~1~~	~~3'0 x 6'0~~	~~8X3.5C14~~	~~EWB~~	~~3~~	~~20'0~~

Note to Drafting: Delete the 3'0 x 6'0 framed opening. It was entered to model a 3070 door in the wall in order to get the girts designed correctly.

At 23'0 row, girt clips adjacent to the 20'x20' FO's get 4 bolts at clip (see sketch).

COLUMNS ----- (8.250" Inset columns)

Col #	Dist. from left	Description Member Size Ident.	Base Elev (ft)	Base plate design information Thickness & bolts
1	0.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.50 elev.		
2	25.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.50 elev.		
3	50.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.5, 17.25 elev.		
4	75.000'	W10X22 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.5, 17.25 elev.		
5	100.000'	BU 10x.375/.185x12	0.0000'	0.500" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 28.00 elev.		
6	125.000'	W10X22 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.5, 17.25 elev.		
7	150.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.5, 17.25 elev.		
8	175.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.50 elev.		
9	200.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.50 elev.		

ENDWALL COLUMN TO BRIDGE CHANNEL CONNECTIONS:

COL. NO.	STRUT-TO-COLUMN CLIP ENDWALL FRAME LINE 1
PLANE SWC:	
1	
2	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
3	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
4	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
5	AT PEAK, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A325N W8X18 COLUMN EXTENSION w/ 12.000 " LAP LENGTH; 8X2.5C12 BRIDGE CHANNEL
6	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
7	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
8	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
9	
PLANE SWA:	

Endwall Plane EWD Design ..... Bearing Frame (BF)
S:\Jobs\Active\ENG\22-T-51897\ver01-jfrench\Bldg-A\run01\AwallEWD_01.edf

Panel PBR26

RAFTERS -----

Mem #	Description Member Size Identification	Length (ft)	Start (ft)	End (ft)
1	W10X12 50.0 ksi	27.280	0.000	27.280
	connections... Left: Type-V SEP		Right: Type-II MEP	
2	W10X12 50.0 ksi	25.436	27.280	52.715
	connections... Left: Type-II MEP		Right: Type-II MEP	
3	W10X12 50.0 ksi	47.819	52.715	100.534
	connections... Left: Type-II MEP		Right: Type-III	
4	W10X12 50.0 ksi	47.819	100.534	148.353
	connections... Left: Type-III		Right: Type-II MEP	
5	W10X12 50.0 ksi	25.436	148.353	173.789
	connections... Left: Type-II MEP		Right: Type-II MEP	
6	W10X12 50.0 ksi	27.280	173.789	201.069
	connections... Left: Type-II MEP		Right: Type-V SEP	

Type-II MEP = (4)-1/2" A325N bolts w/ 3/8" Moment End Plate
Type-V SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate
Type-III = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWA: **ALL, except omit at 3.0, 6.0, 35.0, 40.0, 60.0, 65.0**
PLANE SWC: **ALL, except omit at 3.0, 6.0, 35.0, 40.0, 60.0, 65.0**

(omit any rafter flange brace that fouls with column extension)

Girts ..... 55.0 ksi Yield Strength

Girts Spacings : 11'6 2@5'9 5'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten	
1	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
2	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
3	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
4	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
5	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
6	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
7	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
8	8'0	25.000	8X2.5C16	None	S 0.000	0.000	S
1	11'6	25.000	8X2.5Z13	None	S 0.000	1.479	C
2	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
3	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
4	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
5	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
6	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
7	11'6	25.000	8X2.5Z16	None	C 1.479	1.479	C
8	11'6	25.000	8X2.5Z13	None	C 1.479	0.000	S

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten	
1	17'3	25.000	8X2.5Z16	None	S 0.000	1.479	C
2	17'3	25.000	8X2.5Z16	None	C 1.479	2.479	C
3	17'3	25.000	8X2.5Z16	None	C 2.479	1.479	C
4	17'3	25.000	8X2.5Z16	None	C 1.479	1.479	C
5	17'3	25.000	8X2.5Z16	None	C 1.479	1.479	C
6	17'3	25.000	8X2.5Z16	None	C 1.479	2.479	C
7	17'3	25.000	8X2.5Z16	None	C 2.479	1.479	C
8	17'3	25.000	8X2.5Z16	None	C 1.479	0.000	S
2	23'0	25.000	8X2.5Z16	None	S 0.146	0.146	S
3	23'0	25.000	8X2.5Z12	None	S 0.146	2.479	C
4	23'0	25.000	8X2.5Z14	None	C 2.479	1.479	C
5	23'0	25.000	8X2.5Z14	None	C 1.479	2.479	C
6	23'0	25.000	8X2.5Z12	None	C 2.479	0.146	S
7	23'0	25.000	8X2.5Z16	None	S 0.146	0.146	S
4	28'0	25.000	8X2.5Z13	None	S 0.146	1.479	C
5	28'0	25.000	8X2.5Z13	None	C 1.479	0.146	S

Note : Maximum distance to extend girt from 'adjacent bay is 36.00 inches.

OPEN AREAS:

Size	Wall	Distance
200'0 x 8'0	EWD	0'0

COLUMNS ----- (8.250" Inset columns)

Col #	Dist. from left	Description Member Size Ident.	Base Elev (ft)	Base plate design information Thickness & bolts
1	0.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.50 elev.		
2	25.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.50 elev.		
3	50.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.5, 17.25 elev.		
4	75.000'	W10X22 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.5, 17.25 elev.		
5	100.000'	W12X26 50.0 ksi	0.0000'	0.500" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.5, 23.00 elev.		
6	125.000'	W10X22 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.5, 17.25 elev.		
7	150.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.5, 17.25 elev.		
8	175.000'	W8X18 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.625" A36
		Flange Brace @ 11.50 elev.		
9	200.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.**750**" A36
		Flange Brace @ 11.50 elev.		

ENDWALL COLUMN TO BRIDGE CHANNEL CONNECTIONS:

COL. NO.	STRUT-TO-COLUMN CLIP ENDWALL FRAME LINE 6
PLANE SWA:	
1	
2	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
3	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
4	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
5	AT PEAK, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A325N W8X18 COLUMN EXTENSION w/ 12.000 " LAP LENGTH; 8X2.5C12 BRIDGE CHANNEL
6	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
7	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
8	ALIGNS WITH PURLIN, TYPE 2 CONN., ALL SLOPES PC75 (0.3750") (4)-1/2" A307N W8X13 COLUMN EXTENSION BEVELED w/ 12.000 " LAP LENGTH; NO BRIDGE CHANNEL
9	
PLANE SWC:	

A & S Building Systems

FRAME DESCRIPTION: ms1 200./16./28.823 20./110./0.
USER NAME:jfrench JOB NAME:51897A
DATE:11/ 5/10 TIME:10:16:13 FILE:ml-1.fra
PAGE: 1-1

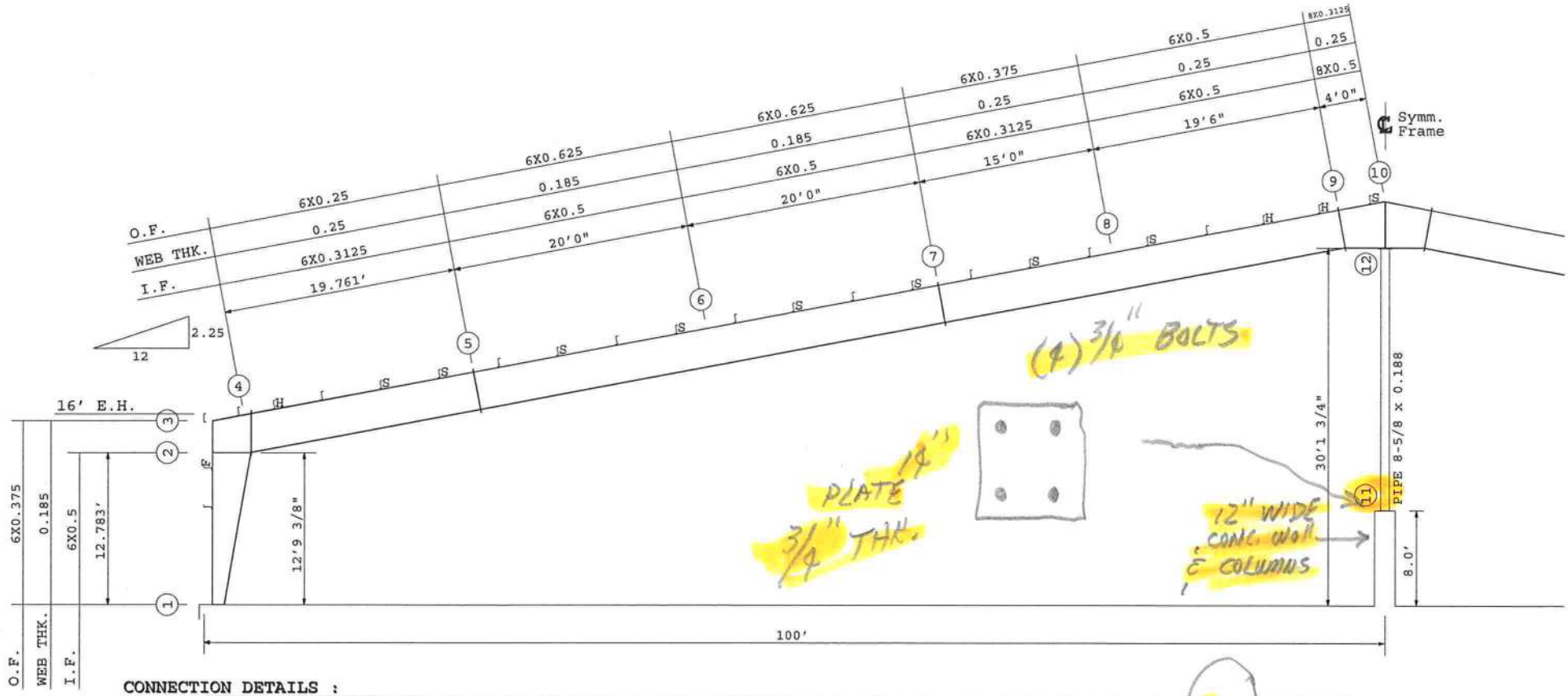
DESIGN AND ESTIMATION INFORMATION

WEIGHT: 12040 lbs
BOLTS:A325 SNUG TIGHT
PURLINS(horz. from eave) :8"-Z 2@3',4',15@5',2@4'9",4'3 9/16"
GIRTS (vert. from floor): 8"-Z 8'2 1/2",3'3 1/2"[8.25"]

FRAME ID #01 LOCATION: frame lines 2,5
DETAIL FILE: ng\22-T-51897\ver01-jfrench\Bldg-A\Drftg\x01L.12

STANDARD NOTES:

(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.



CONNECTION DETAILS :

Location	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)
Web Dep.	12.0	39.0	N/A	38.0	38.0	38.0	38.0	38.0	38.0	45.375	8.625	8.625
Type	BASE	HORZ STF	CAP (EXT)	3E/3E	2E/2E	SPLICE	2E/4E	SPLICE	4E/3E	SPLICE	BASE	CAP/STF
Plate(DN)	8.0X0.625	2.75X0.5	6.0X0.25	6.0X0.625	6.0X0.5	N/A	6.0X0.5	N/A	6.0X0.75	N/A	14.0X0.75	10.0X0.5
Plate(UP)	N/A	N/A	N/A	6.0X0.5	6.0X0.5	N/A	6.0X0.5	N/A	8.0X0.75	N/A	N/A	3.75X0.5
Bolts	(4)-1	N/A	N/A	(12)-3/4	(8)-3/4	N/A	(12)-3/4	N/A	(14)-3/4	N/A	(4)-3/4	(4)-1/2

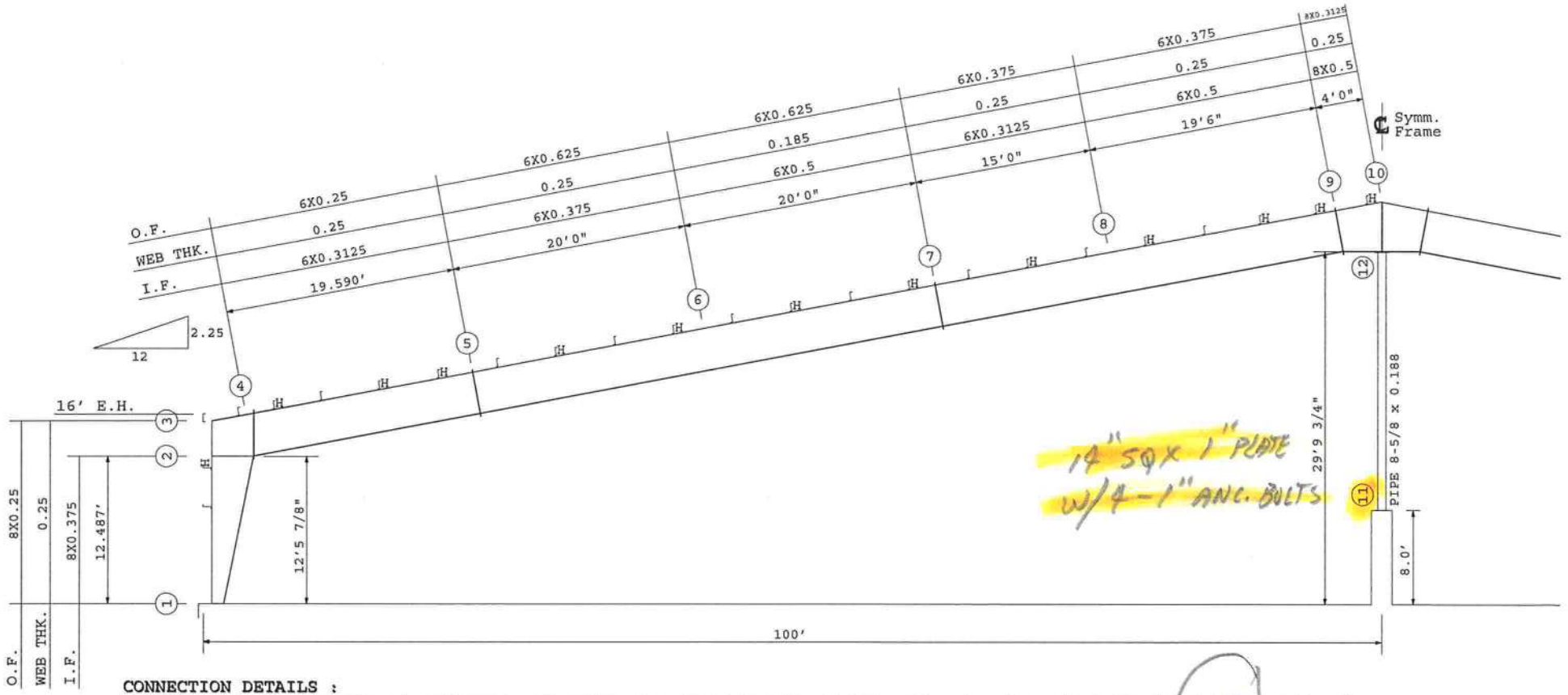
A & S Building Systems

FRAME DESCRIPTION: ms1 200./16./32.667 20./110./0.
USER NAME:jfrench JOB NAME:51897A
DATE:11/ 5/10 TIME:10:18:02 PAGE: 2-1
FILE:m2-1.fra

DESIGN AND ESTIMATION INFORMATION FRAME ID #02 LOCATION: frame lines 3,4
DETAIL FILE: ng\22-T-51897\ver01-jfrench\Bldg-A\Drftg\x02L.12

WEIGHT: 13184 lbs
BOLTS:A325 SNUG TIGHT
PURLINS(horz. from eave) :8"-Z 2@3',4',15@5',2@4'9",4'3 9/16"
GIRTS (vert. from floor): 8"-Z 8'2 1/2",3'3 1/2"[8.25"]

STANDARD NOTES:
(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.



CONNECTION DETAILS :

Location	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)
Web Dep.	12.0	42.0	N/A	42.0	42.0	42.0	42.0	42.0	42.0	49.25	8.625	8.625
Type	BASE	HORZ STF	CAP (EXT)	4E/4E	2E/2E	SPLICE	2E/4E	SPLICE	4E/3E	SPLICE	BASE	CAP/STF
Plate(DN)	8.0X0.625	3.75X0.375	8.0X0.25	8.0X0.5	6.0X0.5	N/A	6.0X0.625	N/A	6.0X0.75	N/A	14.0X1.0	10.0X0.5
Plate(UP)	N/A	N/A	N/A	6.0X0.5	6.0X0.5	N/A	6.0X0.625	N/A	8.0X0.75	N/A	N/A	3.75X0.5
Bolts	(4)-1	N/A	N/A	(16)-3/4	(8)-3/4	N/A	(12)-3/4	N/A	(14)-3/4	N/A	(4)-1	(4)-1/2

A&S BUILDING SYSTEMS

REACTIONS

BUILDER: Gamble & Associates Construction
CUSTOMER: Columbia Grain
JOB NUMBER: 22-T-51897

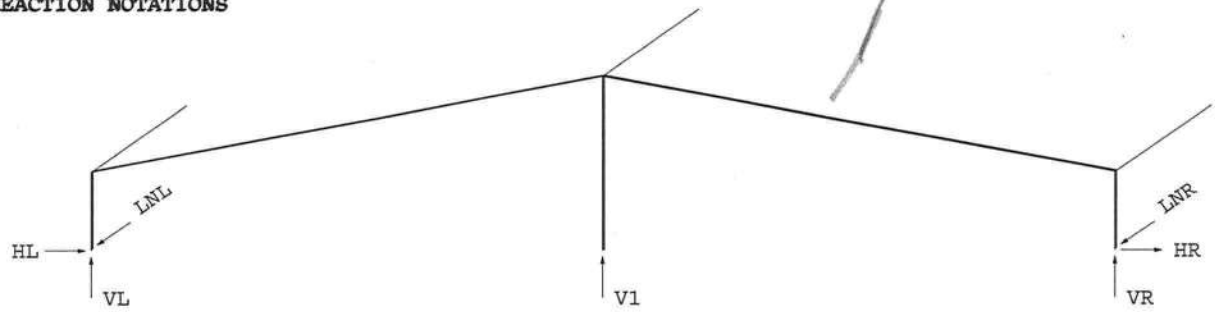
Notes

1) The reactions provided are based on the Order Documents at the time of mailing. Any changes to building loads or dimensions may change the reactions. The reactions will be superseded and voided by any future mailing.
2) The reactions provided have been created with the following layout (unless noted otherwise).
   a) A reaction table is provided with the reactions for each load group.
   b) Rigid Frames
      (1) Gabled Buildings
         (a) Left and Right columns are determined as if viewing the left side of the building, as shown on the anchor rod drawing, from the outside of the building.
         (b) Interior columns are spaced from left side to right side.
      (2) Single Slope Buildings
         (a) Left column is the low side column.
         (b) Right column is the high side column.
         (c) Interior columns are spaced from low side to high side.
   c) Endwalls
      (1) Left and Right columns are determined as if viewing the wall from the outside.
      (2) Interior columns are spaced from left to right.
   d) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer.
   e) Anchor rods are A36 or A307 material unless noted otherwise on the anchor rod layout drawing (F1 sheet).
   f) X-Bracing
      (1) Rod Bracing reactions have been included in values shown in the reaction tables.
      (2) For IBC and UBC based building codes, when x-bracing is present in the sidewall, individual longitudinal seismic loads (RBUPEQ and RBDWEQ) do not include the amplification factor, Ω_0.
      (3) For IBC and UBC based building codes, when x-bracing is present in the endwall, individual transverse seismic loads (EQ) do not include the amplification factor, Ω_0.
3) Reactions are provided as un-factored for each load group applied to the column. The foundation engineer will apply the appropriate load factors and combine the reactions in accordance with the building code and design specifications to determine bearing pressures and concrete design. The factors applied to load groups for the steel column design may be different than the factors used in the foundation design.
   Maximum reactions are <u>not</u> provided by the manufacturer to allow the foundation engineer to determine the correct values for his design procedures and allow for an economical foundation design.

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #01 LOCATION:frame lines 2,5

NOTES:(1) All reactions are in kips and kip-ft.
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1		
BASE PLATE	8.0X13.0X0.625			8.0X13.0X0.625			14.0X14.0X0.75		
ANC. RODS	(4)-1			(4)-1			(4)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR	H1	V1	LN1
DL	5.3	7.2	0.0	-5.3	7.2	0.0	0.0	11.4	0.0
COLL	2.5	3.2	0.0	-2.5	3.2	0.0	0.0	5.2	0.0
PLL1	7.4	18.3	0.0	-7.4	0.8	0.0	0.0	15.5	0.0
PLL2	7.4	0.8	0.0	-7.4	18.3	0.0	0.0	15.5	0.0
LL	14.9	19.1	0.0	-14.9	19.1	0.0	0.0	31.1	0.0
RBDWEQ	0.0	0.2	0.0	0.0	0.2	0.0	0.0	0.0	0.0
EQ	-0.3	-0.1	0.0	-0.3	0.1	0.0	0.0	0.0	0.0
RBUPEQ	0.0	-0.3	-0.5	0.0	-0.3	-0.5	0.0	0.0	0.0
WL1	-18.6	-25.1	0.0	13.6	-18.5	0.0	0.0	-28.2	0.0
WL2	-11.5	-13.9	0.0	6.4	-7.2	0.0	0.0	-12.4	0.0
WL3	-13.6	-18.5	0.0	18.6	-25.1	0.0	0.0	-28.2	0.0
WL4	-6.5	-7.2	0.0	11.5	-13.9	0.0	0.0	-12.4	0.0
LWL1	-18.5	-30.5	0.0	19.2	-27.2	0.0	0.0	-38.3	0.0
RBUPLW	0.1	-3.6	-5.9	-0.1	-3.6	-5.9	0.0	0.0	0.0
LWL2	-19.2	-27.2	0.0	18.5	-30.5	0.0	0.0	-38.3	0.0
LWL3	-10.8	-18.7	0.0	11.1	-17.4	0.0	0.0	-23.8	0.0
LWL4	-11.1	-17.4	0.0	10.8	-18.7	0.0	0.0	-23.8	0.0
RBDWLW	0.0	3.7	0.0	0.0	3.7	0.0	0.0	0.0	0.0

LOAD GROUP DESCRIPTION

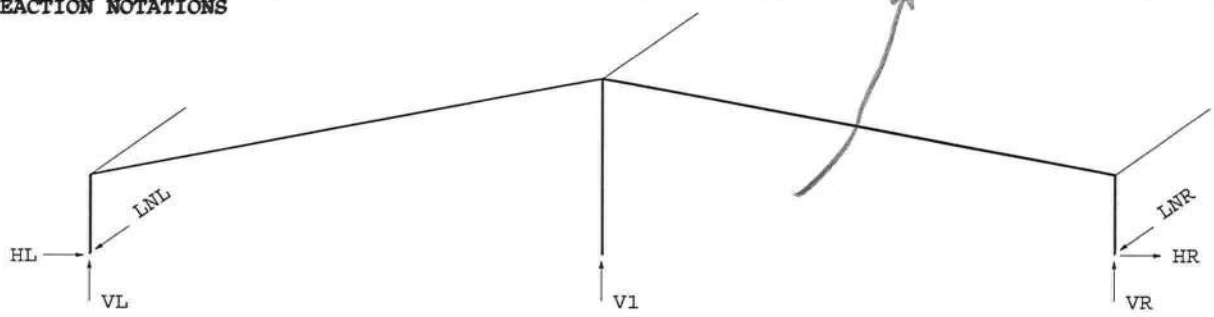
DL : Roof Dead Load
COLL : Roof Collateral Load
PLL1 : Pattern Live Load [PLLxx]
PLL2 : Pattern Live Load [PLLxx]
LL : Roof Live Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load
WL3 : Lateral Primary Wind Load
WL4 : Lateral Primary Wind Load
LWL1 : Longitudinal Primary Wind Load
RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind
LWL2 : Longitudinal Primary Wind Load
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #02 LOCATION:frame lines 3,4

NOTES:(1) All reactions are in kips and kip-ft. TIME:10:18:02

(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1		
BASE PLATE	8.0X13.0X0.625			8.0X13.0X0.625			14.0X14.0X1.0		
ANC. RODS	(4)-1			(4)-1			(4)-1		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR	H1	V1	LN1
DL	6.1	8.1	0.0	-6.1	8.1	0.0	0.0	12.5	0.0
COLL	2.9	3.6	0.0	-2.9	3.6	0.0	0.0	5.8	0.0
PLL1	8.7	20.8	0.0	-8.7	1.0	0.0	0.0	17.4	0.0
PLL2	8.7	1.0	0.0	-8.7	20.8	0.0	0.0	17.4	0.0
LL	17.4	21.8	0.0	-17.4	21.8	0.0	0.0	34.7	0.0
RBDWEQ	0.0	0.2	0.0	0.0	0.2	0.0	0.0	0.0	0.0
EQ	-0.3	-0.1	0.0	-0.3	0.1	0.0	0.0	0.0	0.0
RBUPEQ	0.0	-0.2	-0.5	0.0	-0.2	-0.5	0.0	0.0	0.0
WL1	-21.7	-28.7	0.0	16.0	-21.2	0.0	0.0	-31.4	0.0
WL2	-13.3	-15.8	0.0	7.6	-8.3	0.0	0.0	-13.8	0.0
WL3	-16.0	-21.2	0.0	21.7	-28.7	0.0	0.0	-31.4	0.0
WL4	-7.6	-8.3	0.0	13.3	-15.8	0.0	0.0	-13.8	0.0
LWL1	-21.8	-34.8	0.0	22.5	-31.2	0.0	0.0	-42.8	0.0
RBUPLW	0.1	-2.9	-5.9	-0.1	-2.9	-5.9	0.0	0.0	0.0
LWL2	-22.5	-31.2	0.0	21.8	-34.8	0.0	0.0	-42.8	0.0
LWL3	-12.8	-21.4	0.0	13.1	-19.9	0.0	0.0	-26.6	0.0
LWL4	-13.1	-19.9	0.0	12.8	-21.4	0.0	0.0	-26.6	0.0
RBDWLW	0.0	2.9	0.0	0.0	2.9	0.0	0.0	0.0	0.0

LOAD GROUP DESCRIPTION

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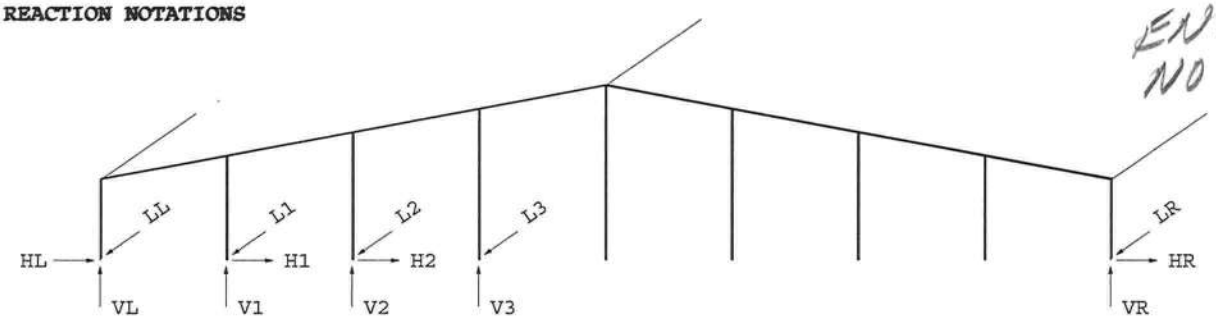
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME:08:45:47

REACTION NOTATIONS

END 5
NO COLUMNS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2			INTERIOR COLUMN 3		
BASE PLATE	6.0X10.0X0.375			6.0X10.0X0.375			6.0X8.5X0.375			6.0X8.5X0.375			6.0X10.5X0.375		
ANC. BOLTS	(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3
D	0.0	0.7	0.	0.0	0.7	0.	0.	1.7	0.	0.	1.6	0.	0.	2.0	0.0
C	0.0	0.3	0.	0.0	0.3	0.	0.	0.7	0.	0.	0.6	0.	0.	0.8	0.0
L	0.1	2.4	0.	-0.1	2.4	0.	0.	6.5	0.0	0.	5.5	0.0	0.	6.6	0.0
W+	-0.1	-8.5	1.3	0.1	-8.5	1.3	0.	-9.3	4.1	0.	-7.2	5.0	0.	-9.6	5.9
W-	-0.1	-1.2	4.4	0.1	-1.2	4.4	0.	-9.3	-4.5	0.	-7.2	-5.6	0.	-9.6	-6.7
WR	-0.1	-4.8	0.	0.1	-4.8	0.	0.	-8.5	0.0	0.9	-8.0	0.0	0.	-9.6	0.0
WL	-0.1	-4.8	0.	0.1	-4.8	0.	-0.9	-10.3	0.0	0.	-6.3	0.0	0.	-9.6	0.0
E+	0.	-0.2	0.4	0.	-0.2	0.4	0.	0.	0.	0.	0.	0.	0.	0.	0.
E-	0.	0.3	-0.1	0.	0.3	-0.1	0.	0.	0.	0.	0.	0.	0.	0.	0.
ER	0.	0.	0.	0.	0.	0.	0.	0.2	0.	0.2	-0.2	0.	0.	0.	0.
EL	0.	0.	0.	0.	0.	0.	-0.2	-0.2	0.	0.	0.2	0.	0.	0.	0.

LOAD GROUP DESCRIPTION

D : DEAD LOAD
C : COLLATERAL LOAD
L : LIVE LOAD
W+ : WIND LOAD AS AN INWARD ACTING PRESSURE
W- : WIND LOAD AS AN OUTWARD ACTING SUCTION
WR : WIND FORCE FROM THE RIGHT
WL : WIND FORCE FROM THE LEFT
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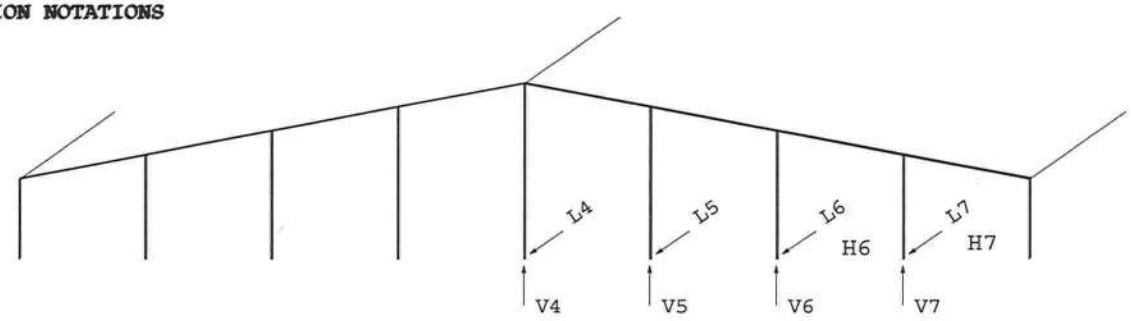
E- : EARTHQUAKE FORCE ACTING OUTWARD
ER : EARTHQUAKE FORCE FROM RIGHT
EL : EARTHQUAKE FORCE FROM LEFT

SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME:08:45:47

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	INTERIOR COLUMN 4			INTERIOR COLUMN 5			INTERIOR COLUMN 6			INTERIOR COLUMN 7		
BASE PLATE	10.0X13.0X0.5			6.0X10.5X0.375			6.0X8.5X0.375			6.0X8.5X0.375		
ANC. BOLTS	(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8		
LOAD GROUP	H4	V4	L4	H5	V5	L5	H6	V6	L6	H 7	V 7	L 7
D	0.	2.1	0.0	0.	2.0	0.0	0.	1.6	0.	0.	1.7	0.
C	0.	0.5	0.0	0.	0.8	0.0	0.	0.6	0.	0.	0.7	0.
L	0.	4.6	0.0	0.	6.6	0.0	0.	5.5	0.0	0.	6.5	0.0
W+	0.	-8.8	6.9	0.	-9.6	5.9	0.	-7.2	5.0	0.	-9.3	4.1
W-	0.	-8.8	-7.8	0.	-9.6	-6.7	0.	-7.2	-5.6	0.	-9.3	-4.5
WR	0.	-8.8	-0.1	0.	-9.6	0.0	0.	-7.2	0.0	0.	-9.3	0.0
WL	0.	-8.8	-0.1	0.	-9.6	0.0	0.	-7.2	0.0	0.	-9.3	0.0
E+	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
E-	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
ER	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
EL	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

LOAD GROUP DESCRIPTION

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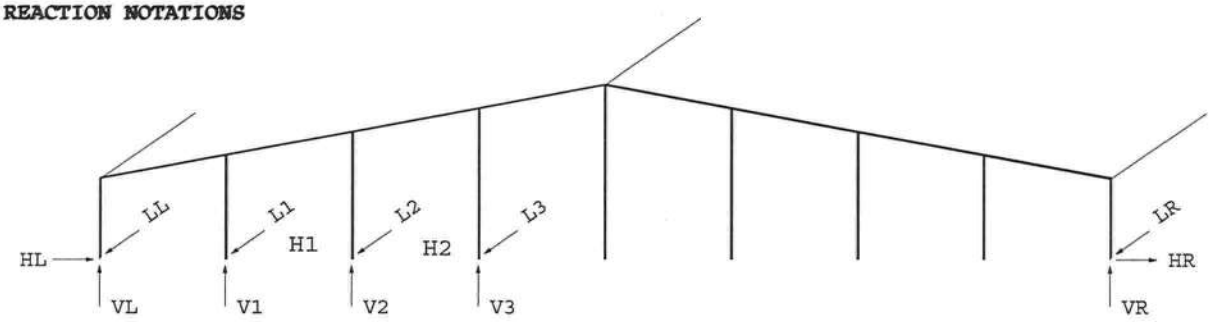
SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME:08:45:47

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2			INTERIOR COLUMN 3		
BASE PLATE	6.0X10.0X0.375			6.0X10.0X0.375			6.0X8.5X0.375			6.0X8.5X0.375			6.0X10.5X0.375		
ANC. BOLTS	(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3
D	0.0	0.7	0.	0.0	0.7	0.	0.	1.7	0.	0.	1.6	0.	0.	2.0	0.0
C	0.0	0.3	0.	0.0	0.3	0.	0.	0.7	0.	0.	0.6	0.	0.	0.8	0.0
L	0.1	2.4	0.	-0.1	2.4	0.	0.	6.5	0.0	0.	5.5	0.0	0.	6.6	0.0
W+	-0.1	-1.2	1.3	0.1	-1.2	1.3	0.	-9.3	4.1	0.	-7.2	5.0	0.	-9.6	5.9
W-	-0.1	-8.5	-7.5	0.1	-8.5	-7.5	0.	-9.3	-4.5	0.	-7.2	-5.6	0.	-9.6	-6.7
WR	1.5	-4.8	0.	1.7	-4.8	0.	0.	-9.3	0.0	0.	-7.2	0.0	0.	-9.6	0.0
WL	-1.5	-4.8	0.	-1.2	-4.8	0.	0.	-9.3	0.0	0.	-7.2	0.0	0.	-9.6	0.0
E+	0.	0.3	0.8	0.	0.3	0.8	0.	0.	0.9	0.	0.	0.9	0.	0.	0.9
E-	0.	-0.2	-1.2	0.	-0.2	-1.2	0.	0.	-0.9	0.	0.	-0.9	0.	0.	-0.9
ER	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
EL	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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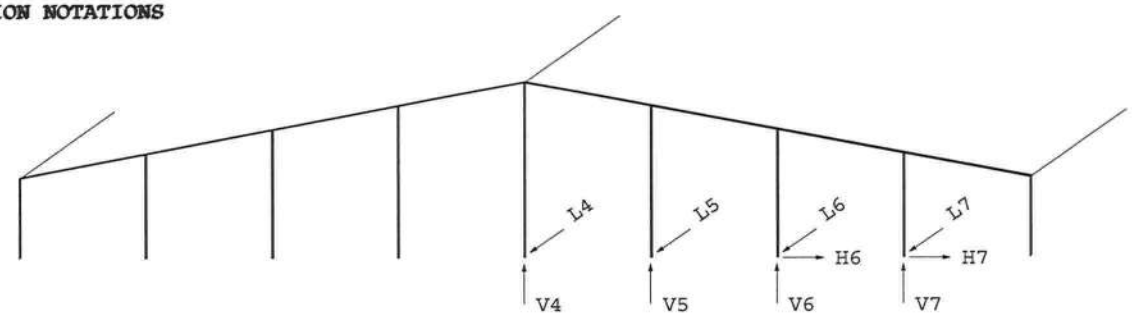
SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME:08:45:47

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	INTERIOR COLUMN 4			INTERIOR COLUMN 5			INTERIOR COLUMN 6			INTERIOR COLUMN 7		
BASE PLATE	8.0X12.5X0.5			6.0X10.5X0.375			6.0X8.5X0.375			6.0X8.5X0.375		
ANC. BOLTS	(4)-5/8			(4)-5/8			(4)-5/8			(4)-5/8		
LOAD GROUP	H4	V4	L4	H5	V5	L5	H6	V6	L6	H 7	V 7	L 7
D	0.	1.8	0.0	0.	2.0	0.0	0.	1.6	0.	0.	1.7	0.
C	0.	0.5	0.0	0.	0.8	0.0	0.	0.6	0.	0.	0.7	0.
L	0.	4.6	0.0	0.	6.6	0.0	0.	5.5	0.0	0.	6.5	0.0
W+	0.	-8.8	6.9	0.	-9.6	5.9	0.	-7.2	5.0	0.	-9.3	4.1
W-	0.	-8.8	-7.7	0.	-9.6	-6.7	0.	-7.2	-5.6	0.	-9.3	-4.5
WR	0.	-8.8	0.0	0.	-9.6	0.0	0.	-6.4	0.0	0.9	-10.1	0.0
WL	0.	-8.8	0.0	0.	-9.6	0.0	-0.9	-8.2	0.0	0.	-8.3	0.0
E+	0.	0.	1.0	0.	0.	0.9	0.	0.	0.9	0.	0.	0.9
E-	0.	0.	-1.0	0.	0.	-0.9	0.	0.	-0.9	0.	0.	-0.9
ER	0.	0.	0.	0.	0.	0.	0.	0.2	0.	0.2	-0.2	0.
EL	0.	0.	0.	0.	0.	0.	-0.2	-0.2	0.	0.	0.2	0.

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