

- 2 X 4 BLOCK NAILED TO EA. BRACE

NOTE: EXTEND RAT RUN BRACES GABLE TO GABLE

OR A LENGTH EQUAL TO 1/2 OF GABLE WIDTH FOR

ROOFS UP TO 7:12 AND 5/8 OF GABLE WIDTH OVER

W23 - GYPSUM CEILING DIAPHRAGM OPTION - GABLE END WALL

GABLE END TRUSS

- SIMPSON LSTA30 STRAP W/

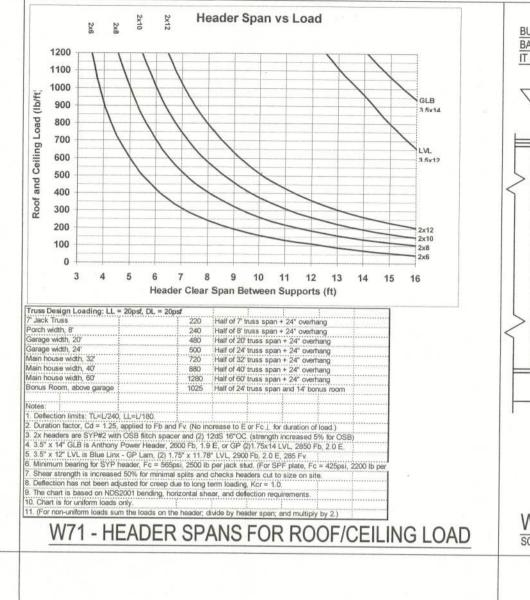
10-8d NAILS IN 2 X 4 BRACE &

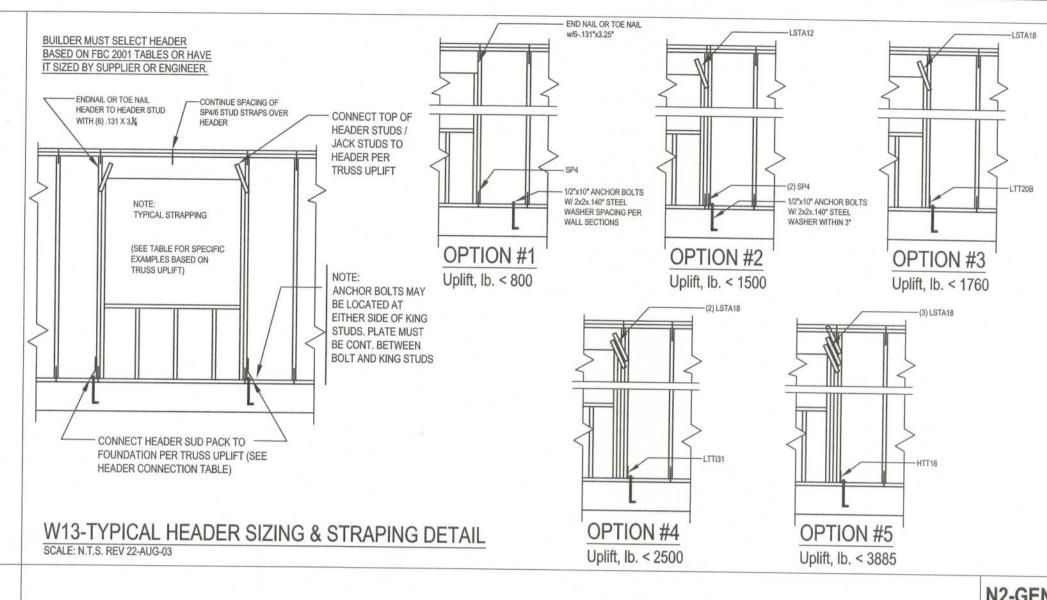
10-8 d NAILS IN ENDWALL STUD

W/4-10d NAILS

1/2 GYP. BOARD W/ 5d COOLER NAILS

5d COOLER NAILS AT 7 IN. O.C.





STEPS PER OWNER

COVERED PORCH

PORCH BEAMS PER ENGINEER

3. For engineered lumber beams have suppliers engineer size beam. Jack Studs and King Studs (BY BUILDER) Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5. 5. Use one jack stud for every 3000 lb vertical load. 6. Total king plus jack studs = studs needed to be there if no opening was there. Header Uplift Connections (BY BUILDER) Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header. 8. Select header connections from table below or mfg. catalog to connect header to stud (top connection) and stud to foundation (bottom connection). Option # Uplift, lb. Top Connector **Bottom Connector** < 800 End nail or toe nail P4, 6-10dx11/2" w/6-.131"x3.25" < 1500 LSTA12, 10-10d 2) SP4, 6-10dx11/5", 1/5" AB 1380 #3 < 1750 LSTA18, 14-10d #4 < 2500 (2) LSTA18, 14-10d LTT20B, 10-16d 1/2" AB #5 < 3885 (3) LSTA18, 14-10d 3480 HTT16, 18-16d, %"x10" AB 417 olift greater than 3885 lb requires engineering design Header Spans Building Width / Truss Span (ft) Supporting Roof+Ceiling (20psf+20psf) NOTES: NJ = Number of jack studs required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on

_oad Bearing Header Sizing Methods (BY BUILDER)

Use supplier pubished data or Southern pine span tables.

Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.

N2-GENERAL NOTES:

FOUNDATION: FOR POINT LOADS GRATER THAN 5000 Ib OR REPETITIVE TRUSS LOADS GRATER THAN 2000 Ib PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 Ib OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAPS SPLICES 48 * db (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

MASTER BEDROOM BEDROOM 9-8 x 11-7 8'-2"----SEE OWNER FOR ALL FLOOR FINISH SELECTIONS LIVING ROOM 18-7 x 23-5 COVERED PORCH STEPS PER OWNER FLOOR PLAN

WINDLOAD ENGINEERING

REV-27-Jul-04

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT" Mark Disosway P.E.

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Location: Lot 5 Ichetuckhee Forest Tract "B" Phase I Columbia County, Florida

Ron Preston Residence

Designer: Tim Delbene Job #05035

Approved: FLPE#53915

Sheet S-1 of 1 Sheet Windload Engineering Job # 507046

N4-WIND LOAD DESIGN DATA

Basic Wind Speed

Wind Exposure

Wind Importance Factor

SST catalog to meet truss uplift. Use fasteners as specified. (Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with Uplift Uplift To Truss / Rafter SPF SYP Connector 535 600 H2.5/ 620 720 H10 10-10dor 12-10dx1 1/2" | 10-10d or 12-10dx 1 10-10dx13/5" 14-10d Sinker 16-16d Sinker %" Thd. Rod To One Member To Other Member 9-8d or 7-10d 9-8d or 7-10d 12-10dx11/5" 3-10d to double plate or 1-10d to single 6-10d to double plate or 2-10d to single 14-8d or 11-10d 14-8d or 11-10d To Foundation %"x 16"AB

N5 - TRUSS UPLIFT CONNECTOR TABLE

All connectors are Simpson Strongtie, uno. Select top and bottom connections from this table or

1085 1265 LSTA18-24 %"x 16" AB 18-10dx 11/3 %"x 16" AB 2-5/8" Bolts %"x 16" AB %"x 16" AB Ib of reaction. Check the minimum bearing requirements of the truss and top plate (SPF, Fc=425psi=2230lb/ply). anufacturer and product number are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed achieve rated loads. All connections exposed directly to the weather shall be not dipped galvanized after fabrication. Loads are increased for win duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=.84*16d, 10dx1½*=.80*10d, 10d=12d=16d sinker). SPF=.86*SYP

REV-18-NOV-04

Building Category Internal pressure Coefficien N/A (Enclosed Building not in the high velocity hurricane zone Building not in the wind-borne debris region * Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions; Mean Roof Height Roof Angle Components And Cladding Wind Pressures (FBC Table 1606.2 B8 Zone Effective Wind Area (ft2) Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder. **Total Shear Wall Segments** Transverse Longitudinal Il exterior walls are type II shear walls CTUAL SHEAR WALL length is the total f all wall segments with full height sheathing and width to height ratio greater than 1: 3.5 (plus special shear wall egments if noted.) REQUIRED SHEAR ALL length is from WFCM-2001, table 7A & 3.17B with table 3.17E adjustment for type II shear wall (or equivalent calculation) REV-27-Jun-03

110 MPH

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of necord for the trusses and did not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work. * Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise

* Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures. * Provide a ciontinuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately. * Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.

including interior bearing walls. * Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads. OCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control.

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Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. s drawing is not valid for construction unless raised seal is affixed.

REV-06-OCT-03