

1/2" GWB UNBLOCKED -

7" OC EDGE 10" OC FIELD

(TYP.) INTERSECTING WALL FRAMING

5d COOLER NAILS

WOOD FRAME

2X\_FULL HEIGHT STUDS (TYP.) -

WOOD FRAME

-8d 3" OC @ PANEL EDGES

INSID RNER

8d 12" OC NOT @ PANEL EDGES

.131"X3 1/4" NAILS 12" OC ---

-8d 3" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

## **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS. TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W 1.4 × W 1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## **BUILDER'S RESPONSIBILITY**

SPECIFICALL	NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	NDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLA	NUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU N OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL NGINEER IMMEDIATELY.
DESIGN, PLACEN	S MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS ENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL ONS.

## **ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW FACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS POOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

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BETWEEN

AND WALL

31.5" MIN. CLEAR WIDTH HANDRAIL ON ONE SIDES

27" MIN. CLEAR WIDTH HANDRAIL ON BOTH SIDES

HANDRAILS SHALL BE CONT. ON AT

LEAST ONE SIDE OF ALL STAIRS

**SECTION VIEW** 

WITH 4 OR MORE RISERS

**PROJECTION** 

THE GREATEST RISER IN ANY

FLIGHT OF STAIRS SHALL NOT

**EXCEED THE SMALLEST** 

THE SUM OF TWO RISERS AND

TYPICAL STAIR AND GUARDRAIL REQUIRMENTS

WINDER TREADS SHALL HAVE A MIN

WITHIN A FLIGHT OF STAIRS THE

GREATEST DEPTH AT 12" SHALL NOT

TREAD DEPTH OF 10" MEASURED AT A

WINDER TREADS SHALL HAVE A MIN. DEPTH OF 6" AT ANY POINT.

**EXCEED THE SMALLEST BY MORE THAN 3/8"** 

A TREAD, EXCLUDING NOSING, IND

SHALL BE BETWEEN 24" AND 25NG.

RISERS + 1 TREAD = 24" - 25") ID 25

9" MIN. TREAD DEP-

EXCEED THE SMALLELL NO

BY MORE THAN 34LLES

IAN 3/8" MAX. TREAD SLOPE OF 2 PERCENT

(1" VERT UNIT IN 48" HORZ, UNITS)

THE GREATEST DEPTH IN AN DEPTH

FLIGHT OF STAIRS SHALL N(IN ANY

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

		A STATE OF THE PARTY OF THE PAR	The State of		1001000
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			
		HEAVY GIRDER HEDOWNS			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
424		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		O/O AD
< 3335	< 3335	HPAHD22	16-16d		
- 10-3480000	- CARAGON -				4/0# AD
< 2200	< 2200	ARI I44	12-16d		
< 2200 < 2300	< 2200 < 2300	ABU44 ABU66	12-16d 12-16d		1/2" AB 1/2" AB

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR\* TO PLATES TO RAFTER/TRUSS

WINDLOACENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 386'54-5419 DIMENSIONS:

REYISIONS

SOFTPIAN

tated dimesions supercede scale dimensionsRefer all questions to Mark Disosay, P.E. for resolution. Do not proced without clarification. COPYRIGH'S AND PROPERTY RIGHTS: Mark Disosay, P.E. hereby expressly reserve s commonaw copyrights and property right i nese instrutents of service. This document is

not to be reroduced, altered or copied in any form or maner without first the express written ermission nd consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined the plan, and that the applicable portions of te plan, relating to wind engineeri comply with ection R301.2.1, florida building

code residetial 2007 and 2009 supplements to the best my knowledge.

LIMITATION This design is valid for one building, at pecified location. P.E. 53915

> Stanley Crawford Construction, Inc.

**Griest Garage** 

ADDRESS: 957 W Indian Springs Drive Lab City, Florida 32055

Mak Disosway P.E. <sup>3</sup>.O. Box 868 Lake City, Florida 32056 Phon: (386) 754 - 5419

Fax (386) 269 - 4871 PRINTED DATE: Jecember 23, 2010 STRUCTURAL BY DRAWN3Y:

FINALS IATE:

2010-2-23 JOB NUMBER:

> 1012064 **PRAWING NUMBER** S-1

OF 2 SHEETS

**Evan Beamsley** 

WIND L	OADS PE	R FLORID	A BUILDIN	G CODE 2007	RESIDENTIA	AL, SI	ECTIO	N R30	1.2.1
(ENCLO MEAN I ON UPI	OSED SIM ROOF HEI PER HALF	PLE DIAPI GHT NOT OF HILL	HRAGM BU EXCEEDIN OR ESCAR	JILDINGS WIT NG LEAST HOI RPMENT 60FT ND FOR 50x H	H FLAT, HIP RIZONTAL D	PED, DIMEN	OR G	ABLE OR 6	ROOF 0 FT; N
				CITY HURRIC					
BUILDI	NG IS NO	T IN THE V	VIND-BOR	NE DEBRIS RE	EGION				
1.) BA	SIC WINE	SPEED =	110 MPH				1800		
2.) W	ND EXPO	SURE = C							
3.) WI	ND IMPO	RTANCE F	ACTOR =	1.0					
4.) BL	ILDING C	ATEGORY	= 11						
5.) RC	OF ANGL	E = 10-45	DEGREES	3		-		ile.	
		HEIGHT	AS TO STORY				-		
7.) IN	TERNAL P	RESSURE	COEFFIC	CIENT = N/A (E	NCLOSED B	IIII D	ING)		
				DESIGN WIND			A STATE OF	R301	2(2))
							,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	11001	-(-//
	×.				Zone	Effective Win		ind Area (ft2)	
_		M				_	10		100
_	1	1	. 4		1 2	_	-30.5 -35.7		-25.3 -30.5
R	< 1				2 O'hg	27.0	-56.8	20.5	-56.8
	2	2 2 1	>77		3	27.8	-35.7	25.3	-30.5
5	1	7	3		3 O'hg		-95.6	1	-59.3
		To a	4		4	_	-33.0		-28.5
2	The same	XX			5	30.5	-40.7	25.9	-31.6
/		2			11 11 11 11 11 11 11 11 11 11	& Windst Cas	е	30.5	-40.7
(3)	1				8x7 Gar			27.3	-32.0
5	2	\	5			16x7 Garage Door		25.9	-29.4
1	2	13/							
	1	A T	4						-
	4	155	4			-			
	4	3 55 55	4						
从	1 LOADS	3 55 2 55	4 17						
DESIGN		(ALL OTH	ER DWELL	LING ROOMS)					
DESIGN	40 PSF	Ø	ER DWELL						
DESIGN	40 PSF 30 PSF	(SLEEPING							
从	40 PSF 30 PSF 30 PSF	(SLEEPING	G ROOMS) /ITH STOR						
DESIGN	40 PSF 30 PSF 30 PSF 10 PSF	(SLEEPING	G ROOMS) /ITH STOR /ITHOUT S	(AGE)					
<b>DESIGN</b> FLOOR	40 PSF 30 PSF 30 PSF 10 PSF 20 PSF	(SLEEPING (ATTICS W	G ROOMS) /ITH STOR /ITHOUT S <4:12)	(AGE)					
<b>DESIGN</b> FLOOR	40 PSF 30 PSF 30 PSF 10 PSF 20 PSF	(SLEEPING (ATTICS W (ATTICS W (FLAT OR (4:12 TO <	G ROOMS) /ITH STOR /ITHOUT S <4:12)	AGE) TORAGE, <3:1					

NOT IN FLOOD ZONE (BUILDER TO VERIFY)