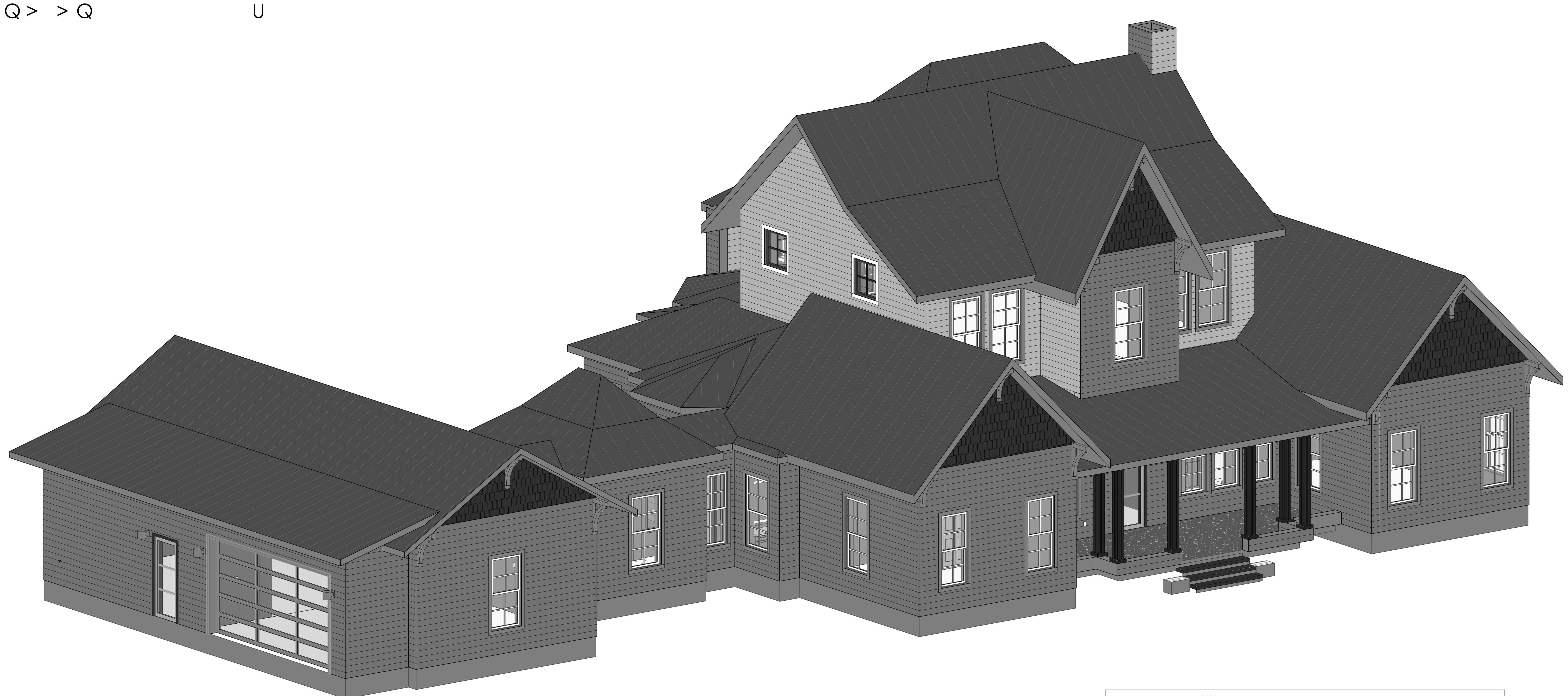

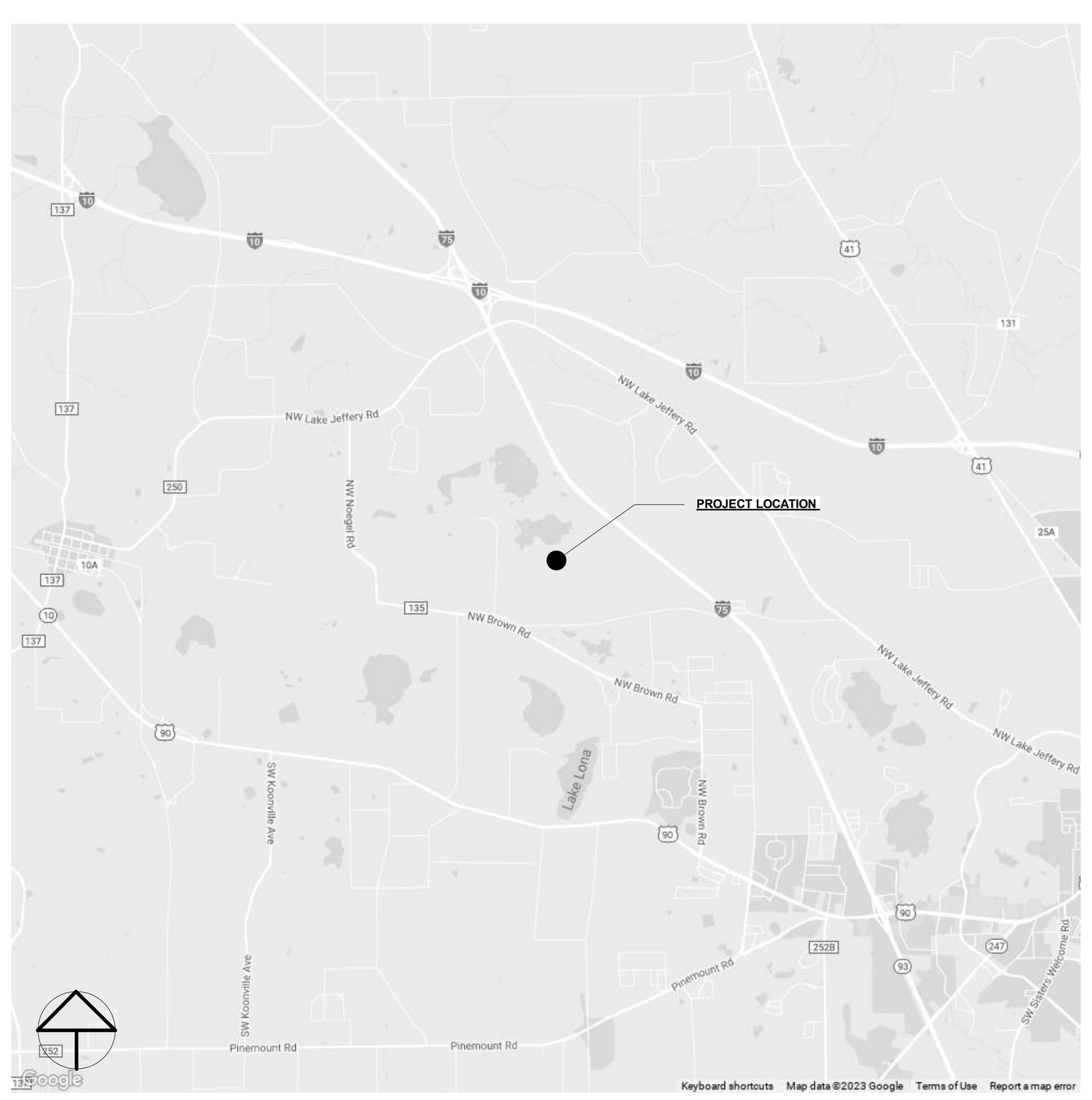

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A101	FLOOR PLAN - LEVEL 1	09/16/23
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A103	REFLECTED CEILING PLAN - LEVEL 1	09/16/23
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CLINTON RESIDENCE

889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

COVER SHEET

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A000

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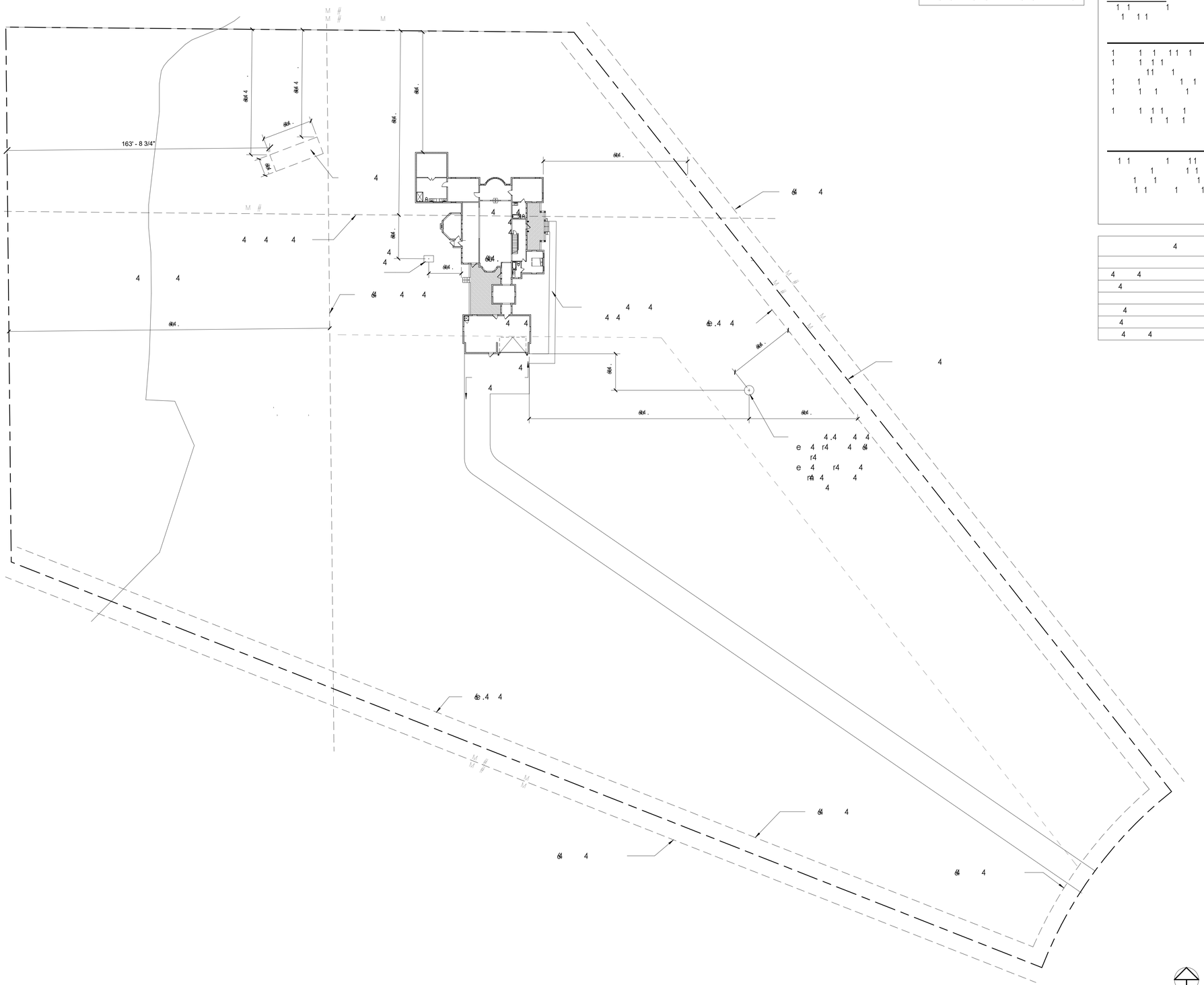

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CLINTON RESIDENCE
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LAKE CITY, FLORIDA 32055

SITE PLAN

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A100

Scale	As indicated

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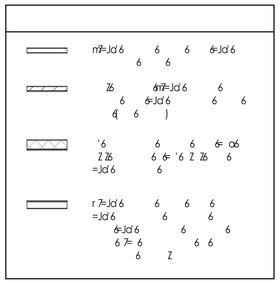

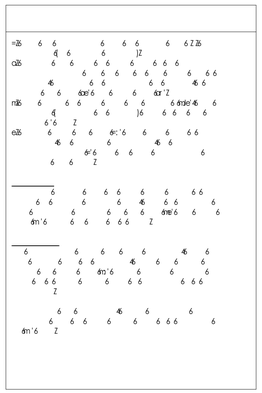

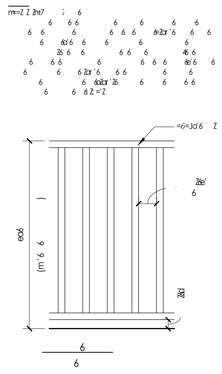

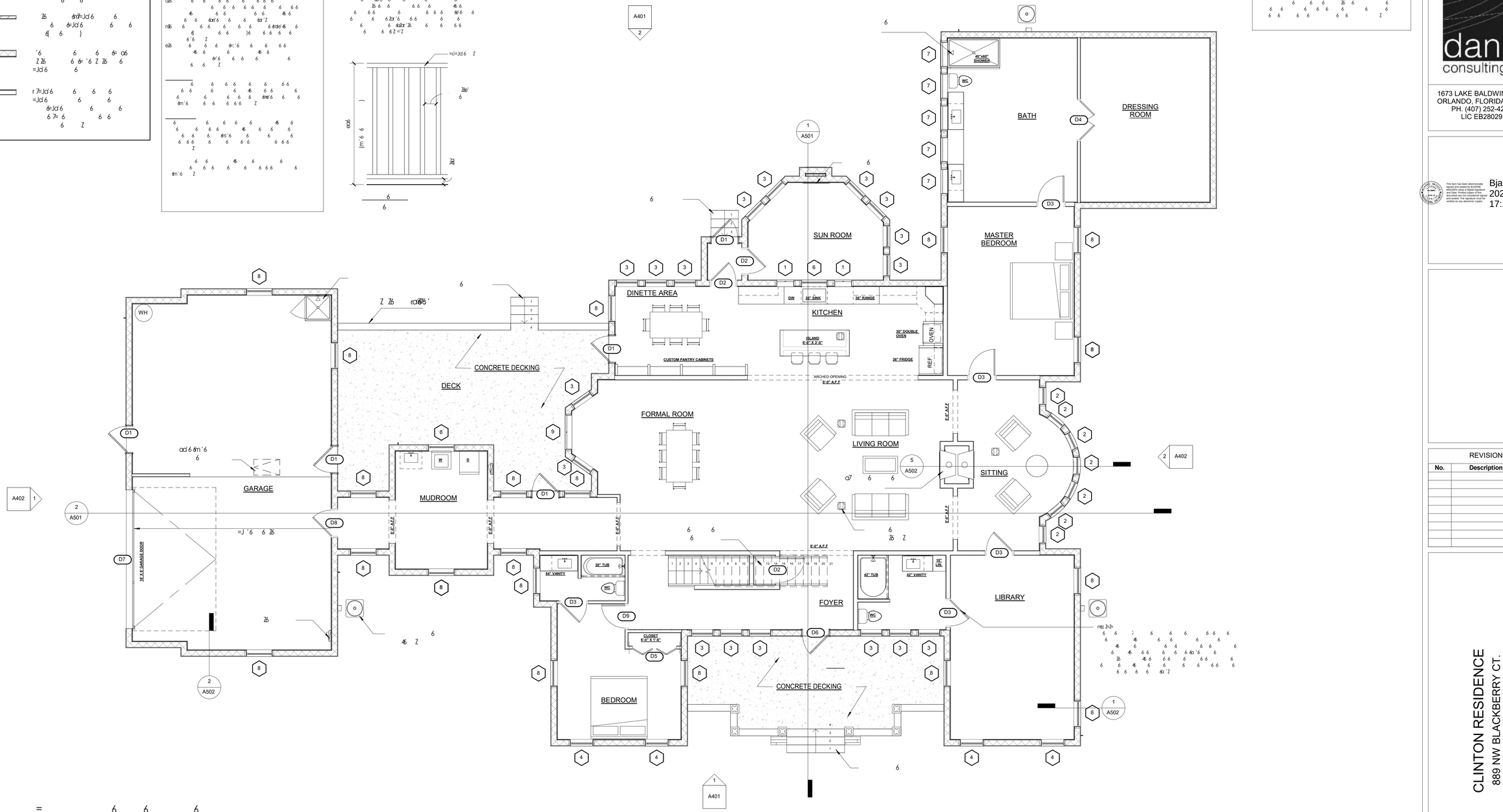

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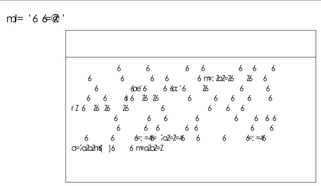

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FLOOR PLAN - LEVEL 1

Project number	05
Date	09-16-23
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Checked by	bM

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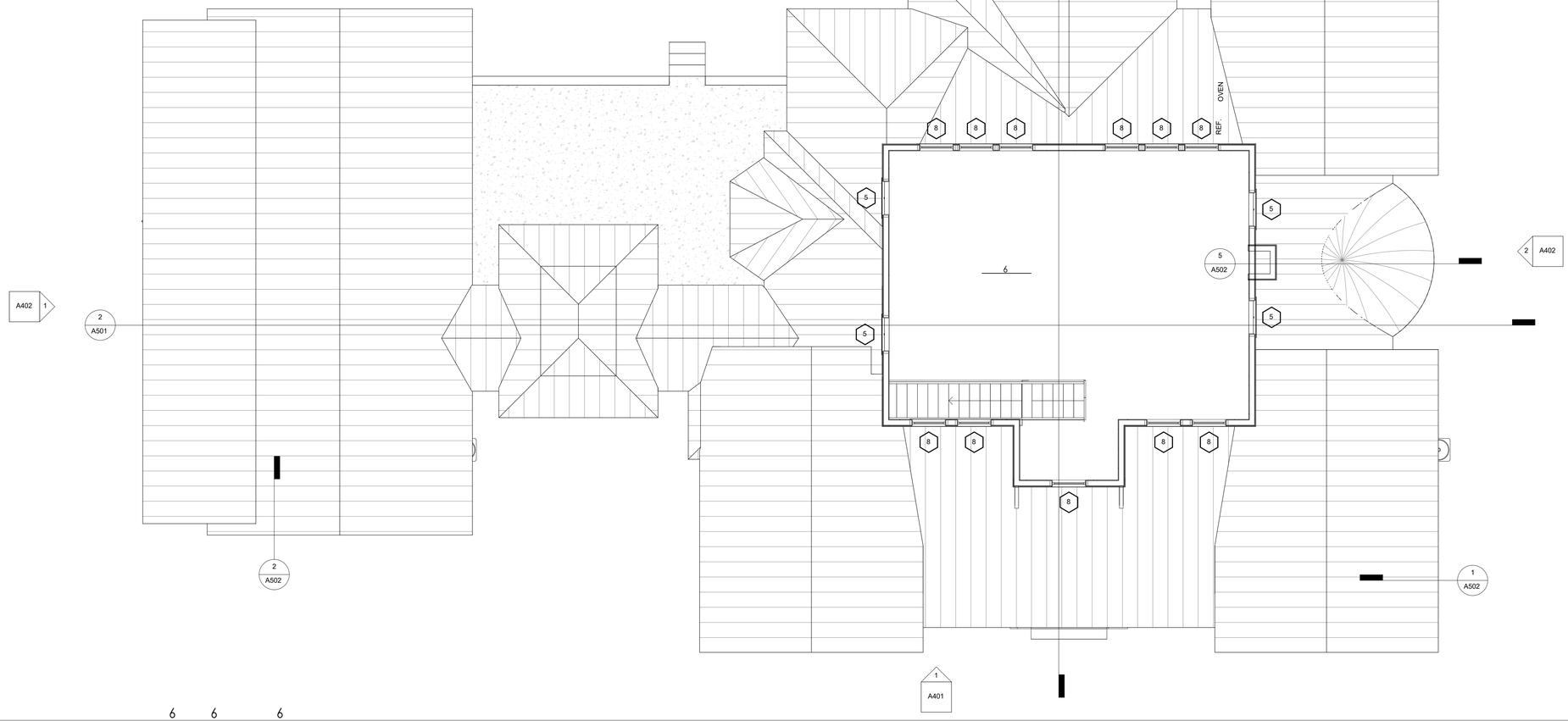

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FLOOR PLAN - LEVEL 2

Project number	05
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A102

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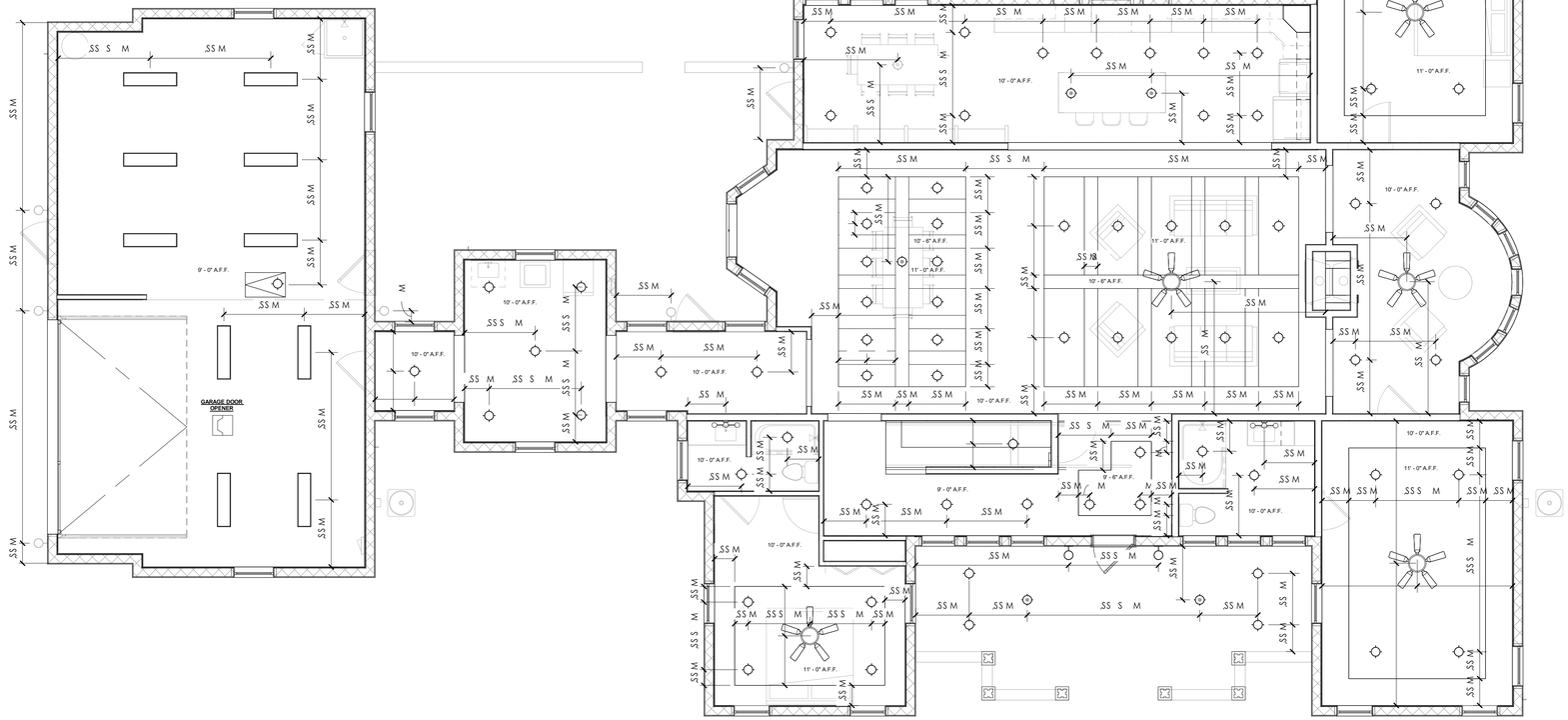

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REFLECTED CEILING PLAN - LEVEL 1

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

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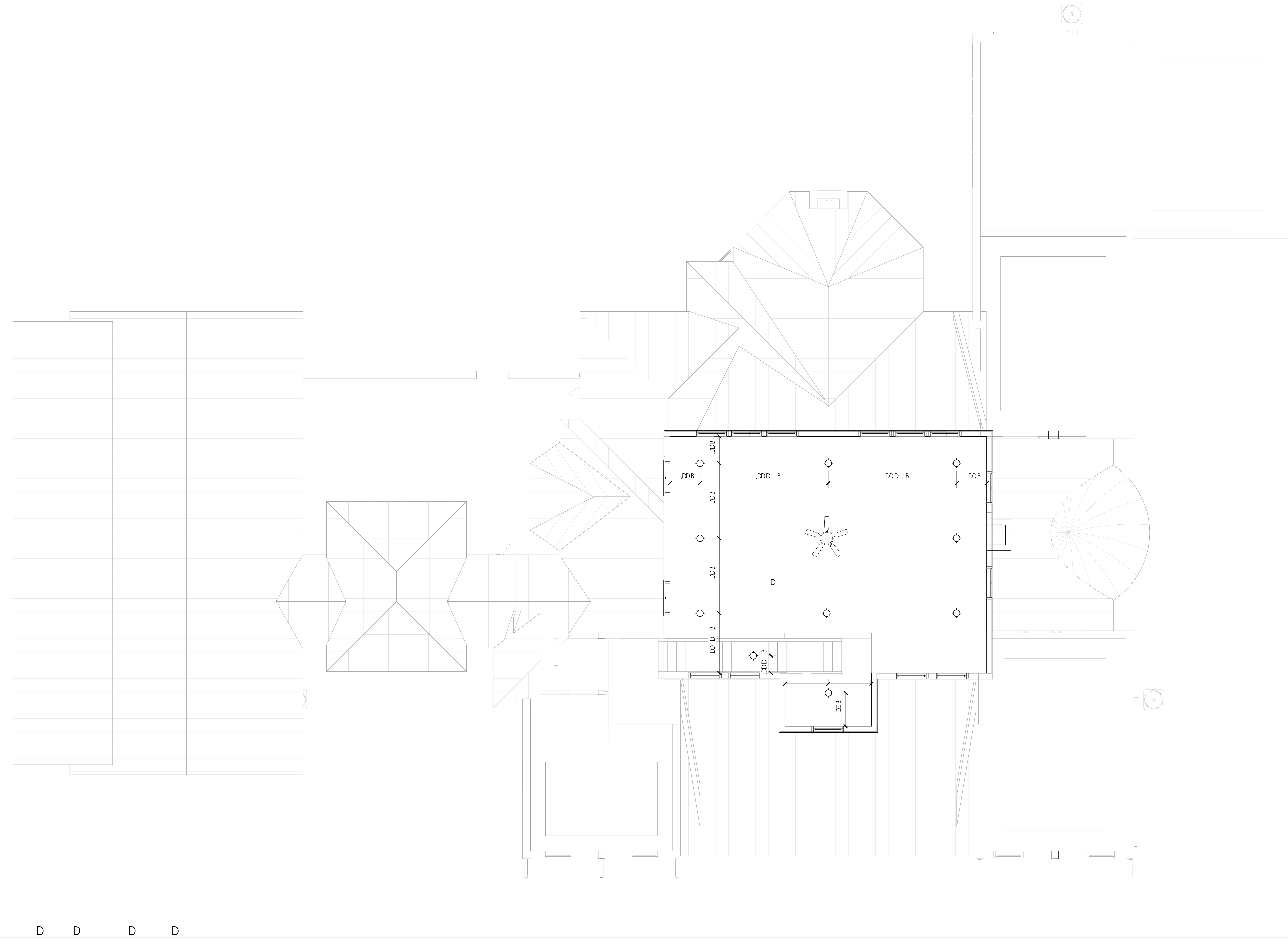

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REFLECTED CEILING PLAN - LEVEL 2

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

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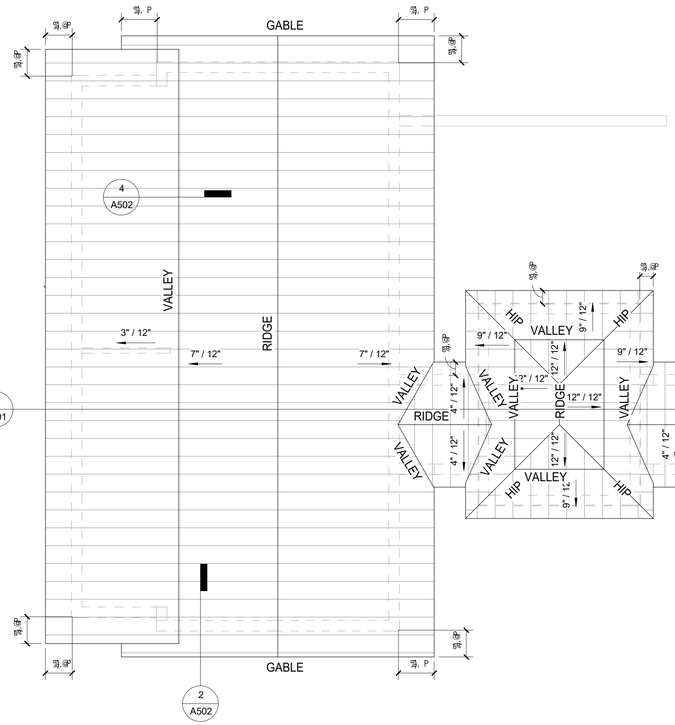

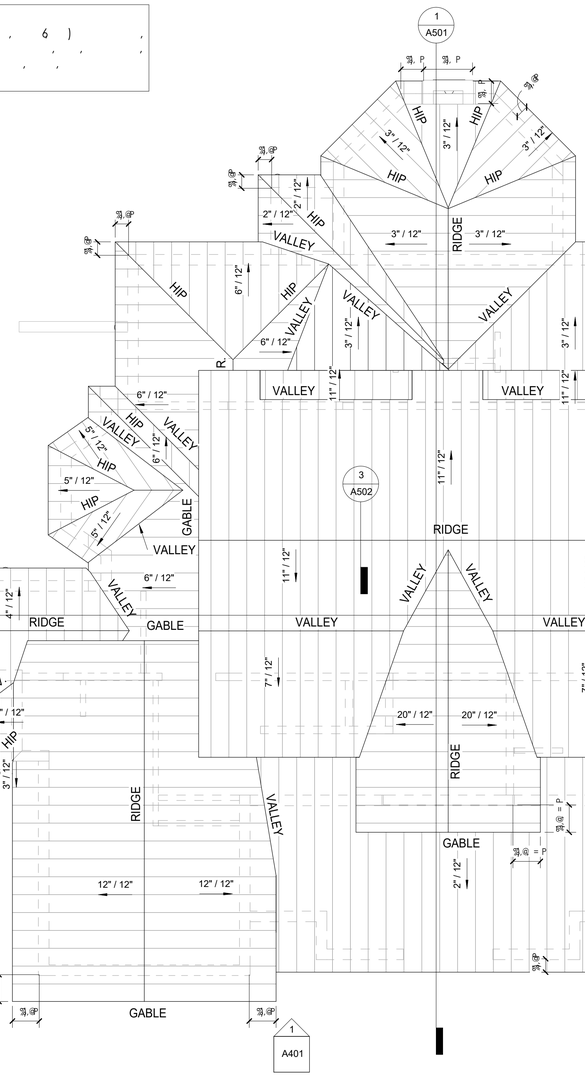

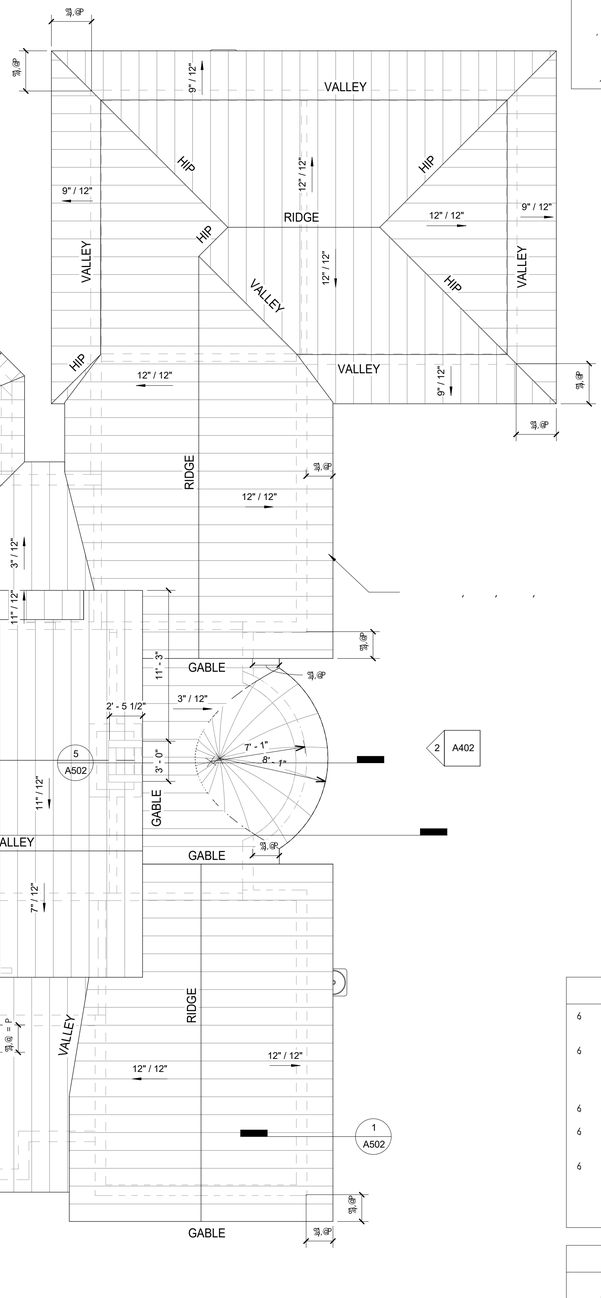

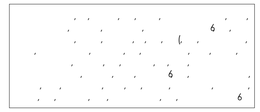

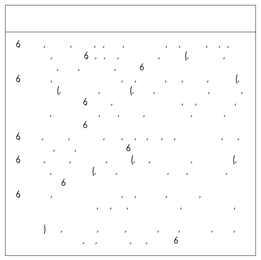

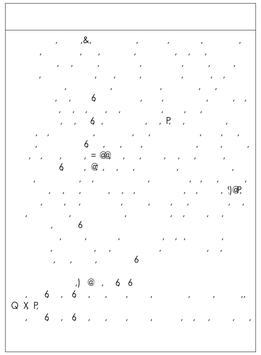

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ROOF PLAN

Project number	05
Date	09-16-23
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Checked by	bM

A105

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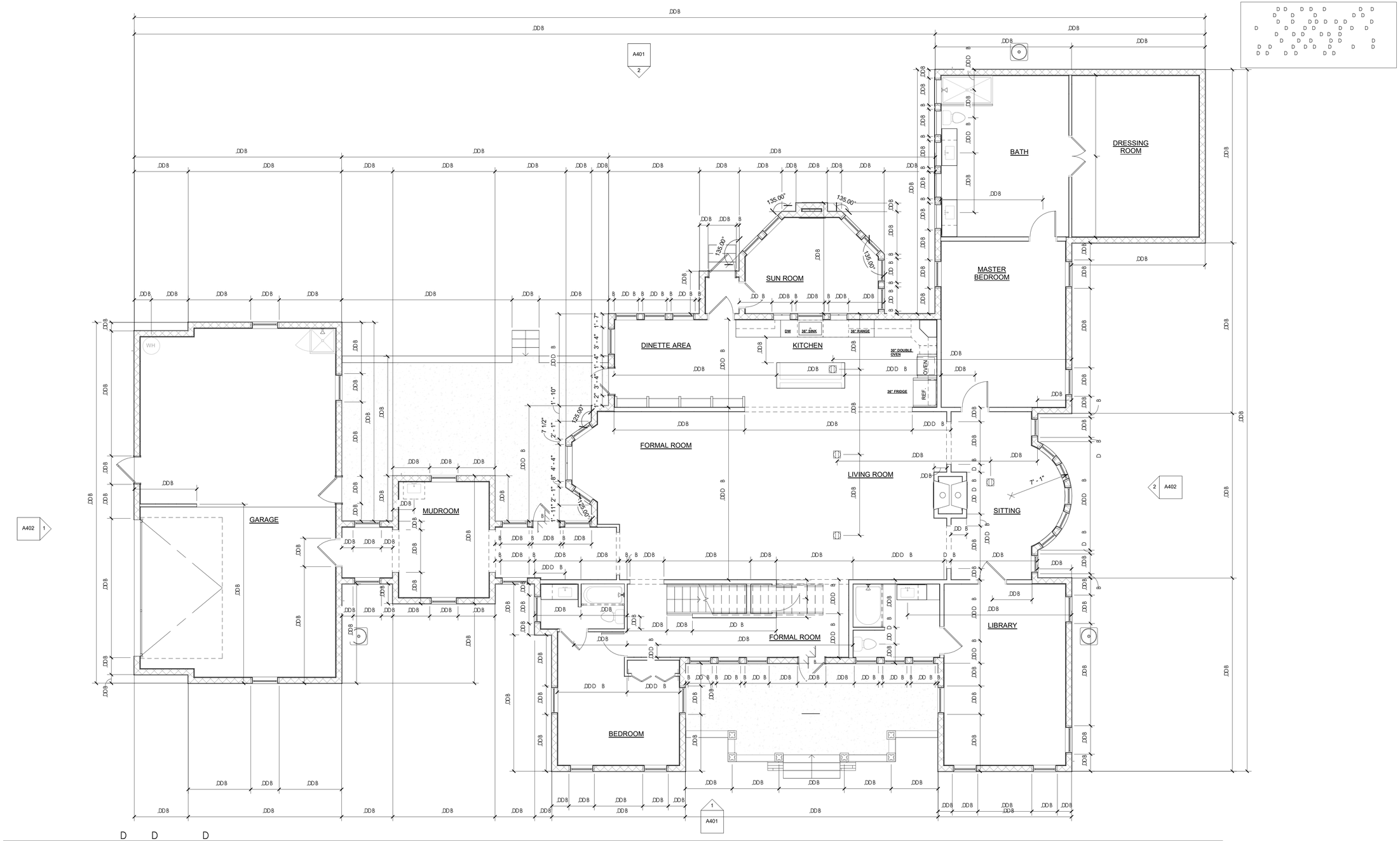

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DIMENSION PLAN - LEVEL 1

Project number	05
Date	09-16-23
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Checked by	bM

A201

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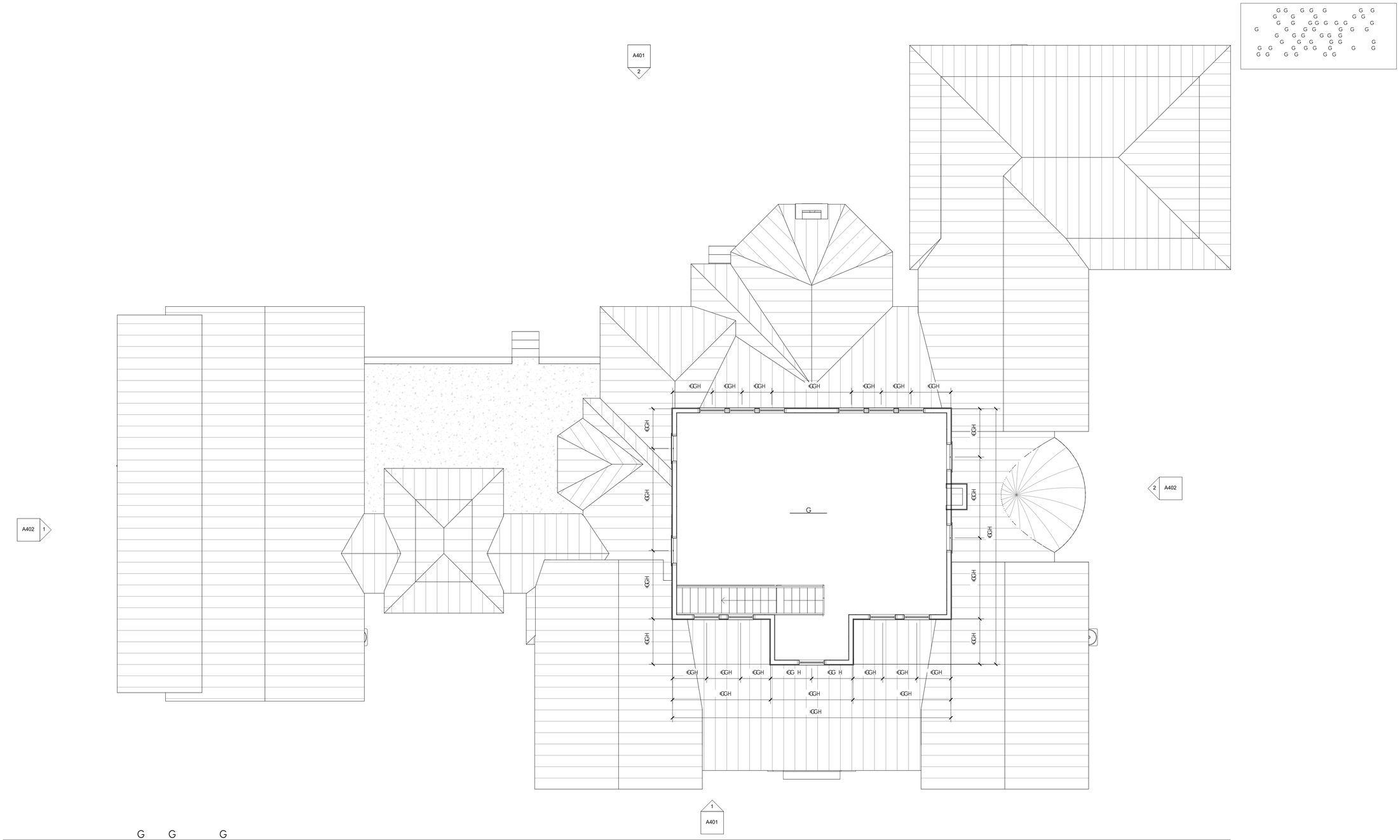

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CLINTON RESIDENCE
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LAKE CITY, FLORIDA 32055

DIMENSION PLAN - LEVEL 2

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A202

Scale: As indicated

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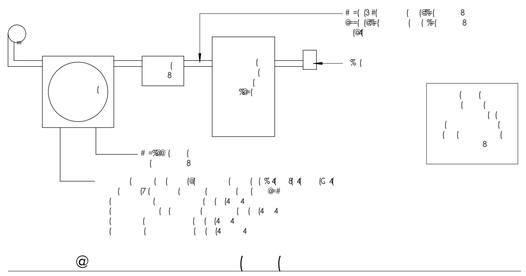

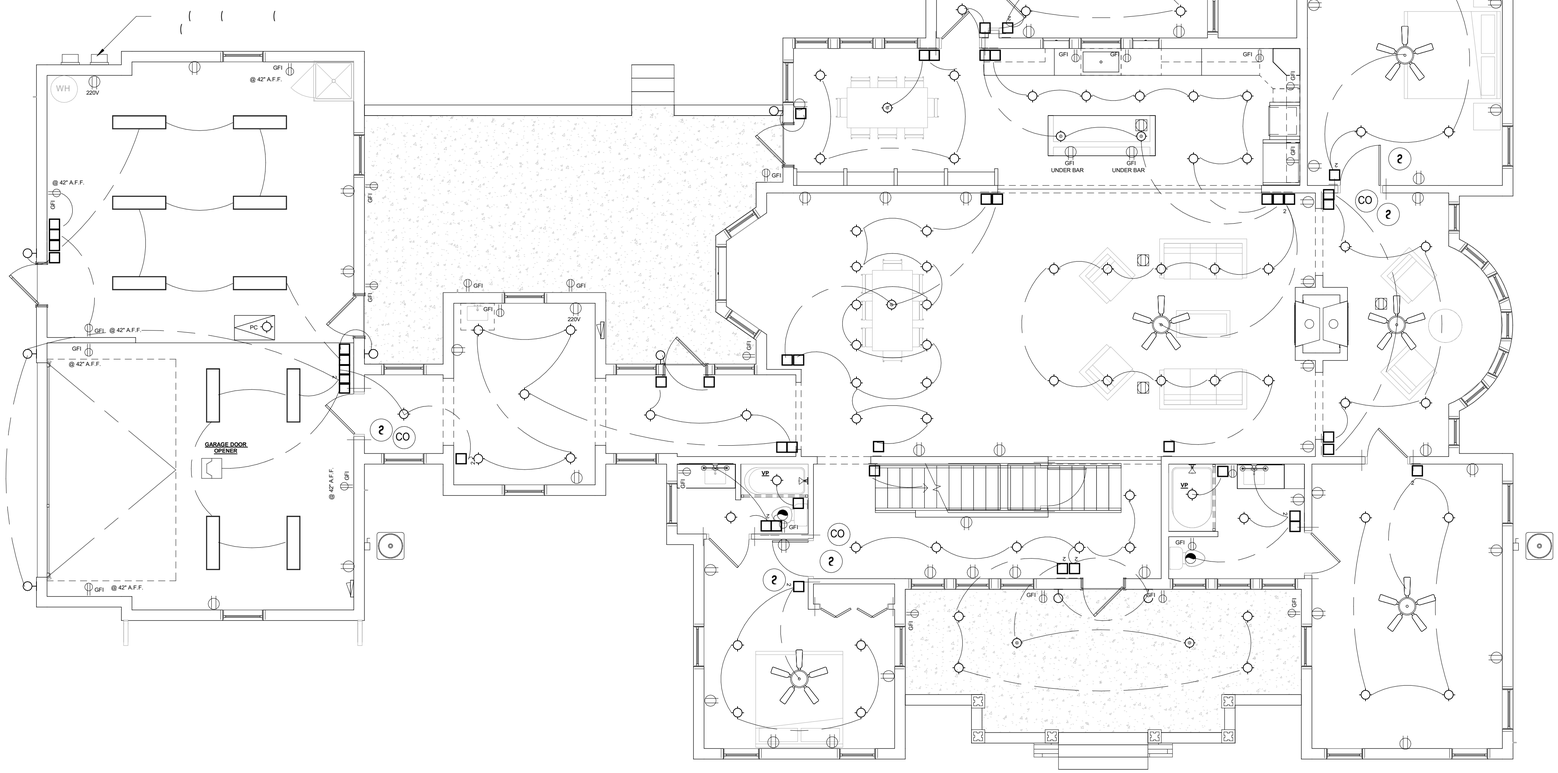

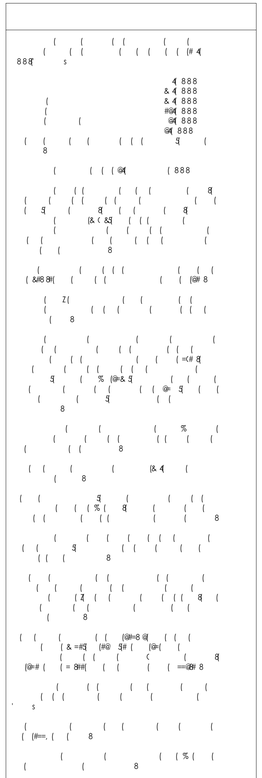

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No.	Description	Date

CLINTON RESIDENCE
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LAKE CITY, FLORIDA 32055

ELECTRICAL PLAN - LEVEL 1

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM
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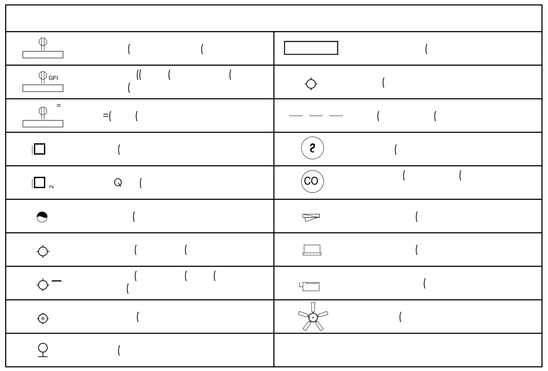

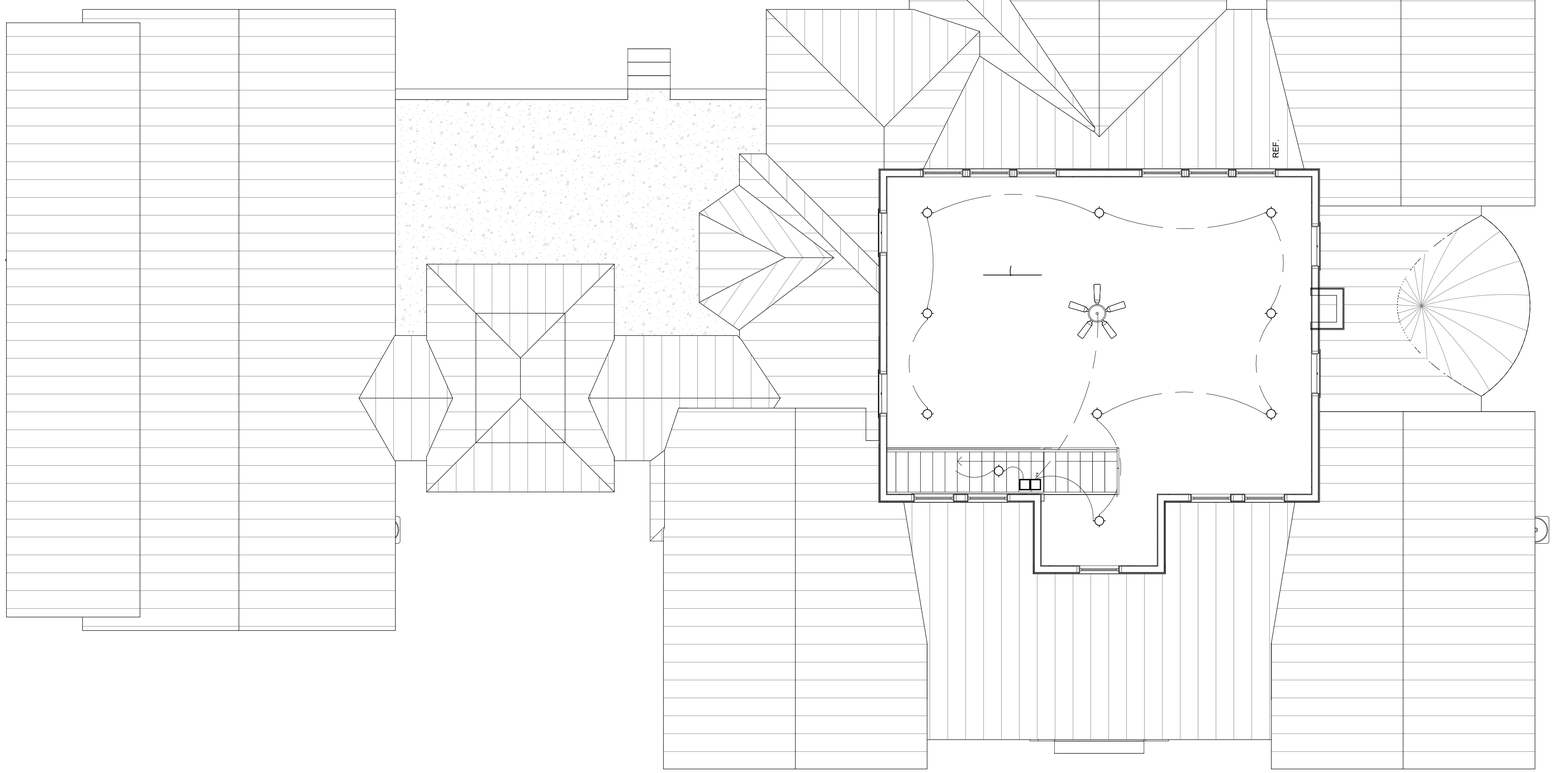

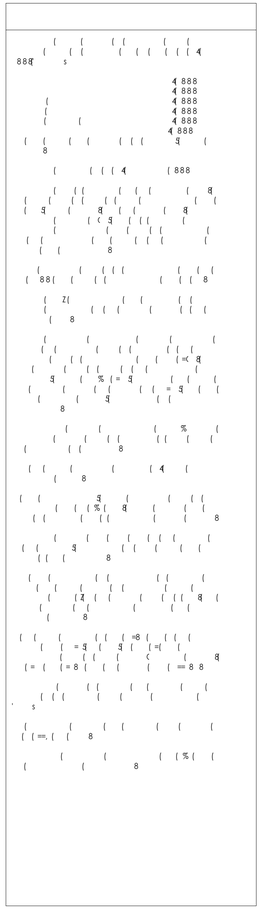

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CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

ELECTRICAL PLAN - LEVEL 2

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A302

Scale As indicated

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No.	Description	Date

CLINTON RESIDENCE
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LAKE CITY, FLORIDA 32055

EXTERIOR ELEVATION

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

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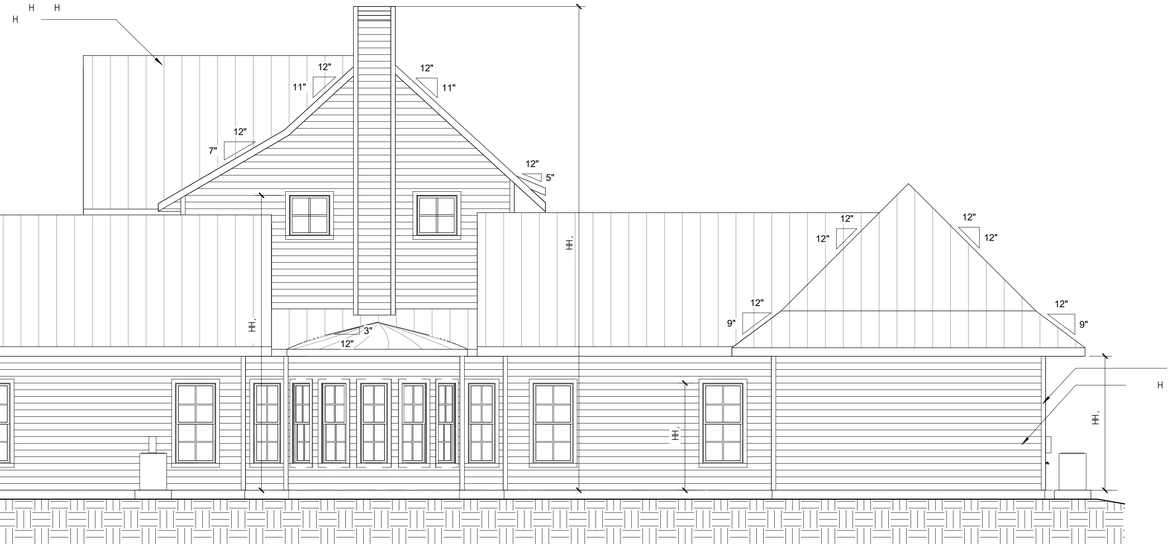

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CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

EXTERIOR ELEVATION

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A402

Scale: As indicated

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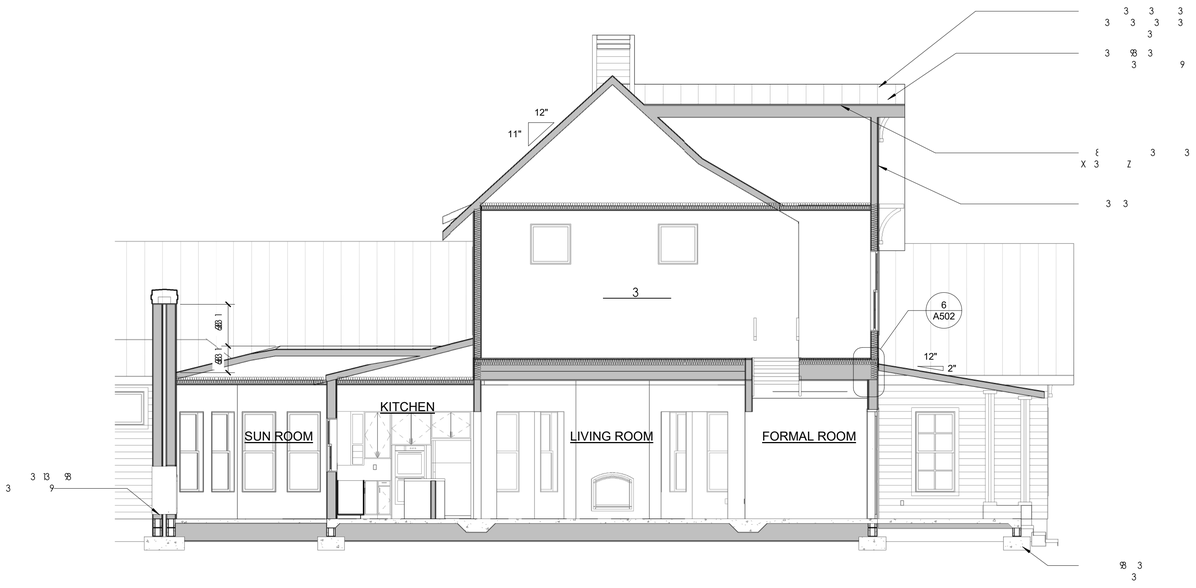

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CLINTON RESIDENCE
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LAKE CITY, FLORIDA 32055

BUILDING SECTIONS

Project number	05
Date	09-16-23
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Checked by	Checker

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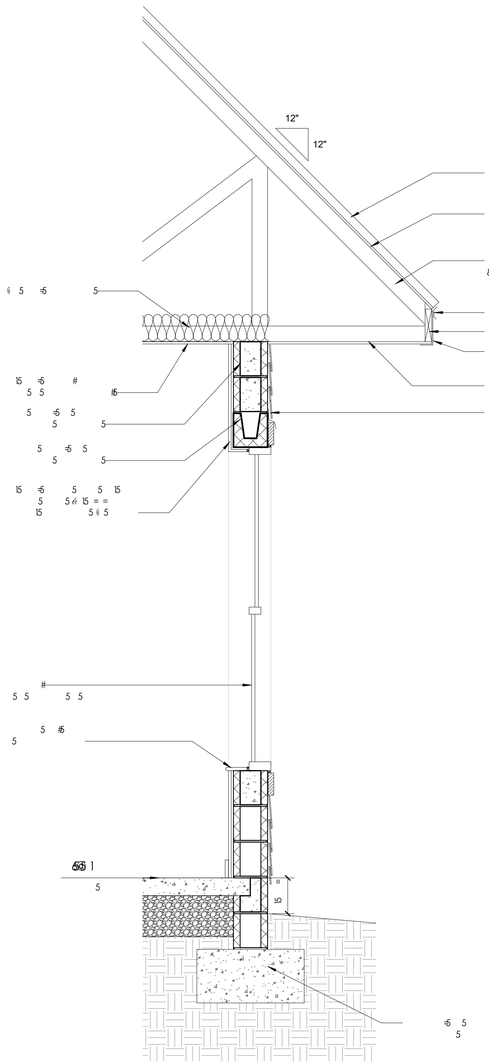

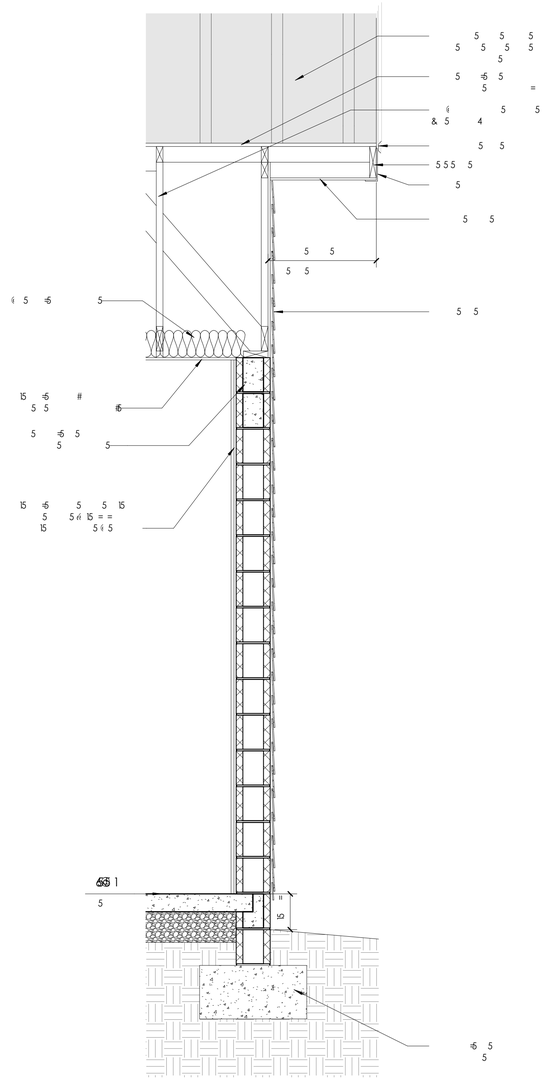

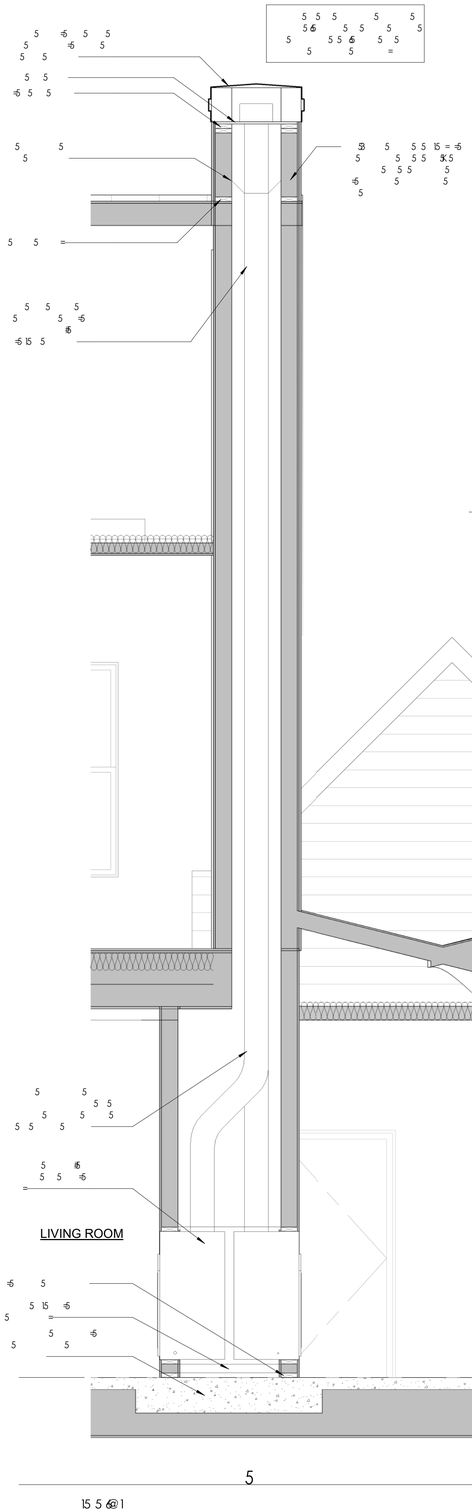

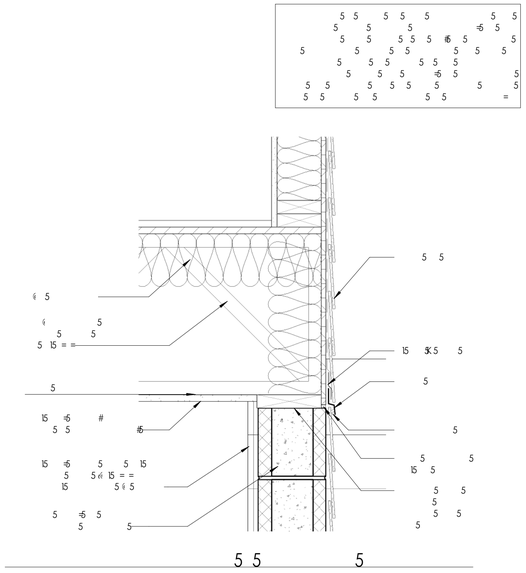

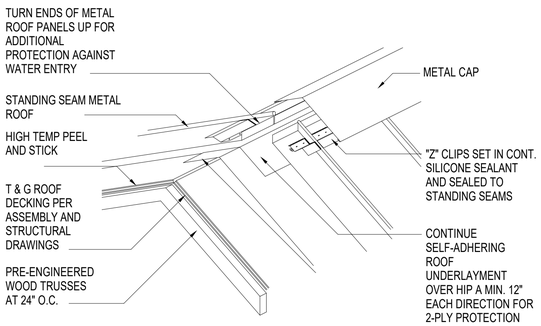

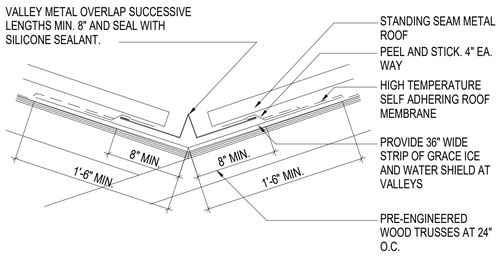

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LIC EB28029

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Bjarne Madsen
2023.09.18
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REVISIONS

No.	Description	Date

CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

SECTION DETAILS

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

A502

Scale As indicated

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STRUCTURAL NOTES

CODES AND STANDARDS

1. WIND LOADS AS PER:
   - A. FLORIDA BUILDING CODE 2020 EDITION, FOR A 139 MPH(ULT)/108 MPH(ASD) WIND SPEED, EXPOSURE C, +/-0.18 INTERNAL PRESSURE COEFFICIENT, 1.0 IMPORTANCE FACTOR, AND RISK CATEGORY II.
   - B. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.
2. THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
   - A. FLORIDA BUILDING CODE 2020 EDITION.
   - B. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ 2014 EDITION).
   - C. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).
   - D. MANUAL OF STANDARD PRACTICE FOR WELDING REINFORCING STEEL, INSERTS & CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION. AWS. D1.4/ 2017 EDITION.
   - E. NATIONAL DESIGN SPECIFICATION, WOOD CONSTRUCTION NDS/2018 EDITION.
   - F. FLORIDA BUILDING CODE - ALLOWABLE STRESS DESIGN FOR WOOD PER SECTIONS 2304, 2305 AND 2306.
   - G. BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, &/TMS 402, 602/2016 EDITIONS).
3. ARCHITECTURAL AND MECHANICAL DRAWINGS:
   - A. THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
   - B. REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, INSERTS, BOLTS SETTINGS, DRAINS, REGLETS, ETC.
   - C. BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
   - D. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
   - E. ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO INSURE THAT ITS OPERATIONS AND PROCEDURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.
4. SECTIONS AND DETAILS:

   ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

SPECIALTY ENGINEERED PRODUCTS

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
   - A. LIGHT GAUGE METAL INCLUDING, BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.
   - B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.
   - C. MISCELLANEOUS HANGERS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, METAL RAILINGS, SAFETY RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

FOUNDATION

1. ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE:
   - A. RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY AN APPROVED TESTING LABORATORY PRIOR TO FOUNDATION WORK.
2. THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
3. BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2000 PSF.
4. SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
5. TOP OF ALL EXTERIOR FOOTINGS SHALL BE MINIMUM 12" BELOW EXTERIOR FINISH GRADE.
6. EXCAVATION & BACKFILL:
   - A. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
   - B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
7. CENTERLINE OF FOOTINGS: SHALL COINCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.
8. DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

1. CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:

   A. FOUNDATIONS 3000 PSI
   B. SLAB-ON-GRADE 3000 PSI

   ALL OTHER CONCRETE TO BE 3000 PSI UNLESS NOTED OTHERWISE.
2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:
   - A. A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS.
   - B. SLUMPS SHALL BE 4" MINIMUM AND 6" MAXIMUM.
   - C. CONCRETE SHALL HAVE 2 PERCENT AIR ENTRAINMENT.
   - D. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
   - E. JOBSITE WATER SHALL NOT BE ADDED.
3. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ 2008 EDITION), THE ACI DETAILING MANUAL (ACI 315/ LATEST EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).
4. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.
5. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
6. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 185, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.
7. REQUIREMENTS:
   - A. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.
   - B. WWF SHALL COMPLY WITH ASTM A 185.
8. LAP ALL BARS MINIMUM 48 DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 12 INCHES (UNLESS OTHERWISE NOTED).
9. REINFORCING BARS:
   - A. ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.

WOOD

1. ALL STRUCTURAL WOOD MEMBERS ARE DESIGNED AS "DRY-USE". MOISTURE CONTENT MUST BE 19 % OR LESS. STORE WOOD FRAMING ABOVE GROUND AND UNDER TARPS WITH PROPER AIR CIRCULATION.
2. ALL LUMBER SHALL BE SOUTHERN PINE SPECIES #2 GRADE OR APPROVED EQUAL. ALLOWABLE DESIGN STRESSES SHALL FOLLOW NATIONAL DESIGN SPECIFICATION (NDS) (LATEST EDITION).
3. HEADERS AT NON BEARING CONDITIONS SHALL BE AS FOLLOWS:

OPENING SIZE	HEADER
UP TO 4'-0"	(2) 2"X 4" (SIDEWAYS)
4'-0" TO 6'-0"	(2) 2" X 8"
6'-0" TO 9'-0"	(2) 2" X 10"

4. PROVIDE SP ACQ PRESSURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREADED LUMBER SHALL BE GALVANIZED WITH A RATING OF G-185 AND CONFORM TO ASTM A653. ALL NAILS AND SCREWS USED WITH PRESSURE TREATED LUMBER ARE TO BE HOT-DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.
5. PLYWOOD SHEATHING:
   - A. FLOOR: USE 3/4" T&G APA 24oc STURD-I-FLOOR, EXP. 1, PLYWOOD SUB-FLOOR SHEATHING.
   - B. WALL: Use 1/2" O.S.B. SHEATHING & 1/2" P.T. PLYWOOD SHEATHING FOR FIRST 4'-0" OF BUILDING HEIGHT ABOVE FIN. FLOOR.
   - C. ROOF: Use 19/32" PLYWOOD SHEATHING
   - D. SEE FRAMING PLANS FOR NAILING AND/OR BLOCKING REQUIREMENTS. USE 8'- 0" LONG X 4'-0" WIDE SHEETS WITH LENGTH ACROSS FRAMING. STAGGER PANEL END JOINTS 4'-0" TYP., ALLOW 1/8" SPACE ALONG PANEL EDGES AND END JOINTS.
   - E. FLOOR SHEATHING TO BE NAILED WITH 10d NAILS AT 6" O.C. AT EDGES & AT 12" O.C. AT INTERMEDIATE SUPPORTS AND GLUED FOR PARTIAL PARTIAL COMPOSITE ACTION. SELECT ADHESIVE WITH APA AFG-01 SPECIFICATION AND FOLLOW APA RECOMMENDATIONS.
   - F. SEE FRAMING PLANS FOR DIAPHRAGM NAILING TYPE, SIZE, SPACING AND LOCATIONS.
6. WOOD CONNECTIONS - ALL NAILS USED FOR STRUCTURAL FRAMING MEMBERS SHALL BE COMMON WIRE, U.N.O. ALL NAILS, TRUSS HANGERS, TRUSS ANCHORS AND STRAPS SHALL BE GALVANIZED FOR CORROSIVE RESISTANCE. ALL METAL STRAPS MUST BE INSTALLED WITH EQUAL LENGTHS ABOUT THE JOINT LINE. USE SIMPSON STRONG-TIE CONNECTOR PRODUCTS OR APPROVED EQUAL. TOE NAILING WILL NOT BE PERMITTED.
7. FINGER JOINT STUDS MAY BE USED IF PROPERLY STORED AND OWNER APPROVED

TIMBER

1. ALL PARALLAM PSL BEAMS TO:
   - A. BE ENGINEERED AND MANUFACTURED BY TRUS JOIST WEYERHAEUSER (TJW) OR APPROVED EQUAL. TEMPORARY BRACING TO BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS. PROVIDE CONTINUOUS SUPPORT OF THE COMPRESSION EDGE AND PROVIDE LATERAL SUPPORT AT ALL BEARINGS. THE MINIMUM ALLOWABLE STRESSES FOR PARALLAM BEAMS ARE AS FOLLOWS:

     Fb = 2,900 PSI Fv = 290 PSI E = 2,000,000 PSI
   - B. ALL EXPOSED EXTERIOR PARALLAM BEAMS ARE TO BE WOLMANIZED PRESSURE TREATED FOR A SERVICE LEVEL 2 EXPOSURE. ALL OTHER PARALLAM BEAMS ARE TO BE WOLMANIZED PRESSURE TREATED FOR A SERVICE LEVEL 1 EXPOSURE.
2. ALL STRUCTURAL TIMBER TO:
   - A. SOUTHERN PINE SPECIES, #2 GRADE (MINIMUM) OR APPROVED EQUAL.
   - B. BE DESIGNED PER THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION'S (AITC)"TIMBER CONSTRUCTION MANUAL" AND AMERICAN FOREST & PAPER ASSOCIATIONS'S (AFPA) NATIONAL DESIGN SPECIFICATION".
   - C. BE CCA PRESSURE TREATED FOR EXTERIOR USE OR WHEN IN CONTACT WITH CONCRETE OR MASONRY PER AMERICAN WOOD PRESERVERS ASSOCIATION STANDARDS.
3. ALL TIMBER AND GLUE LAMINATED BEAM CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND CODES AS SPECIFIED BELOW:
   - A. NATIONAL LUMBER MANUFACTURERS ASSOCIATION: NATIONAL DESIGN SPECIFICATION FOR STRESS GRADE LUMBER AND ITS FASTENINGS.

WOOD TRUSSES

1. WOOD
   - A. ROOF TRUSSES
   - B. FLOOR TRUSSES

   ARE TO BE DESIGNED FOR THE WOOD FABRICATOR BY A PROFESSIONAL SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA. SEALED CALCULATIONS AND LAYOUT DRAWINGS ARE TO BE SUBMITTED FOR APPROVAL. TRUSS FABRICATOR TO PROVIDE ALL TRUSS-TO-TRUSS HANGERS AS REQUIRED TO RESIST GRAVITY AND UPLIFT REACTION. (UPLIFT LOADING SHALL USE COMPONENTS & CLADDING WIND FORCES.)
2. WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE" HANDLING, INSTALLING AND BRACING OF WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS, HIB (1991 EDITION). BRACING IN THE PLANE OF THE WEB MEMBERS:
   - A. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.
3. DO NOT CUT, DRILL OR NOTCH ROOF OR FLOOR TRUSSES WITHOUT WRITTEN APPROVAL FROM TRUSS ENGINEER. COORDINATE MECHANICAL, ELECTRICAL, PLUMBING, ETC. SIZES AND LOCATIONS WITH TRUSS LAYOUT PRIOR TO ERECTION.
4. TRUSSES SHALL BE MANUFACTURED & DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATION(S) FOR WOOD CONSTRUCTION, AF & PA, AND NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION, ANSI/TPI 1-1995, AND THE LOCAL CODE JURISDICTIONS.
5. DO NOT OVERLOAD FLOOR OR ROOF TRUSSES WITH BUILDING MATERIALS.
6. CONNECTOR PLATES SHALL BE MANUFACTURED BY A WTCA MEMBER PLATE SUPPLIER AND SHALL MEET OR EXCEED ASTM A653/A653M REQUIREMENTS FOR STRUCTURAL STEEL.
7. WOOD TRUSS MANUFACTURER TO DESIGN BOTTOM CHORDS OF WOOD ROOF TRUSSES FOR A MINIMUM 10 PSF LIVE LOAD. BOTTOM CHORDS OF WOOD ATTIC TRUSSES TO BE DESIGNED FOR 30 PSF MINIMUM LIVE LOAD.

MASONRY

1. MASONRY UNITS SHALL BE
   - A. LOAD BEARING ASTM C 90
   - B. TYPE II NON-MOISTURE CONTROLLED
   - C. NORMAL WEIGHT
   - D. ALL CMU SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND (U.N.O.).
2. THE COMPRESSIVE STRENGTH OF MASONRY (F'M) SHALL BE 1,500 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C1314.
3. ALL MORTAR SHALL BE IN ACCORDANCE WITH ASTM SPECIFICATION C270
   - A. FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
4. GROUT SHALL BE A HIGH SLUMP MIX
   - A. IN ACCORDANCE WITH ASTM SPECIFICATION C476
   - B. HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI
   - C. FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
5. ALL CONCRETE MASONRY BEARING AND SHEAR WALLS SHALL BE
   - B. INSPECTED BY A CERTIFIED INSPECTION COMPANY AND CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/TMS 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602)/ 2016 EDITIONS.
6. PROVIDE 8" X 8" MASONRY BEAM WITH 2 #5 CONT. AT EVERY WINDOW SILL. EXTEND BEAM 8" BEYOND EDGE OF OPENING.
7. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS.

   FOR JOINT REINFORCEMENT, WALL TIES, ANCHORS AND INSERTS, APPLY A MINIMUM COAT OF 1.5 OUNCES PER SQUARE FOOT (PSF) (458/G/M2) COMPLY WITH THE REQUIREMENTS OF ASTM A153, CLASS B.
8. EPOXY GROUT SHALL BE NON-SHRINK HIGH CREEP RESISTANT, AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE PROPERTIES:

   TENSILE STRENGTH, ASTM C 30: 1,500 PSI
   FLEXURAL STRENGTH, ASTM C 580: 4,000 PSI
   COMPRESSIVE STRENGTH, ASTM C 579: 1,600 PSI/7 DAYS.
9. MINIMUM LAP SPLICES FOR REINFORCED CMU (WITH F'M = 1,500 PSI):

	BAR SIZE				
CMU SIZE	#4	#5	#6	#7	#8
8"	25"	31"	36"	42"	48"

SHOP DRAWINGS

1. THE SHOP DRAWINGS SHALL BE SUBMITTED IN COMPLETE PACKAGES FOR THE FOLLOWING:
   - A. CONCRETE MIX DESIGNS
   - B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
   - C. PRE-ENGINEERED WOOD TRUSSES
   - D. CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
2. PRE-ENGINEERED ITEMS SHALL BE SUBMITTED SIGNED AND SEALED BY A SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA.

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LIC EB28029

REVISIONS

No.	Description	Date

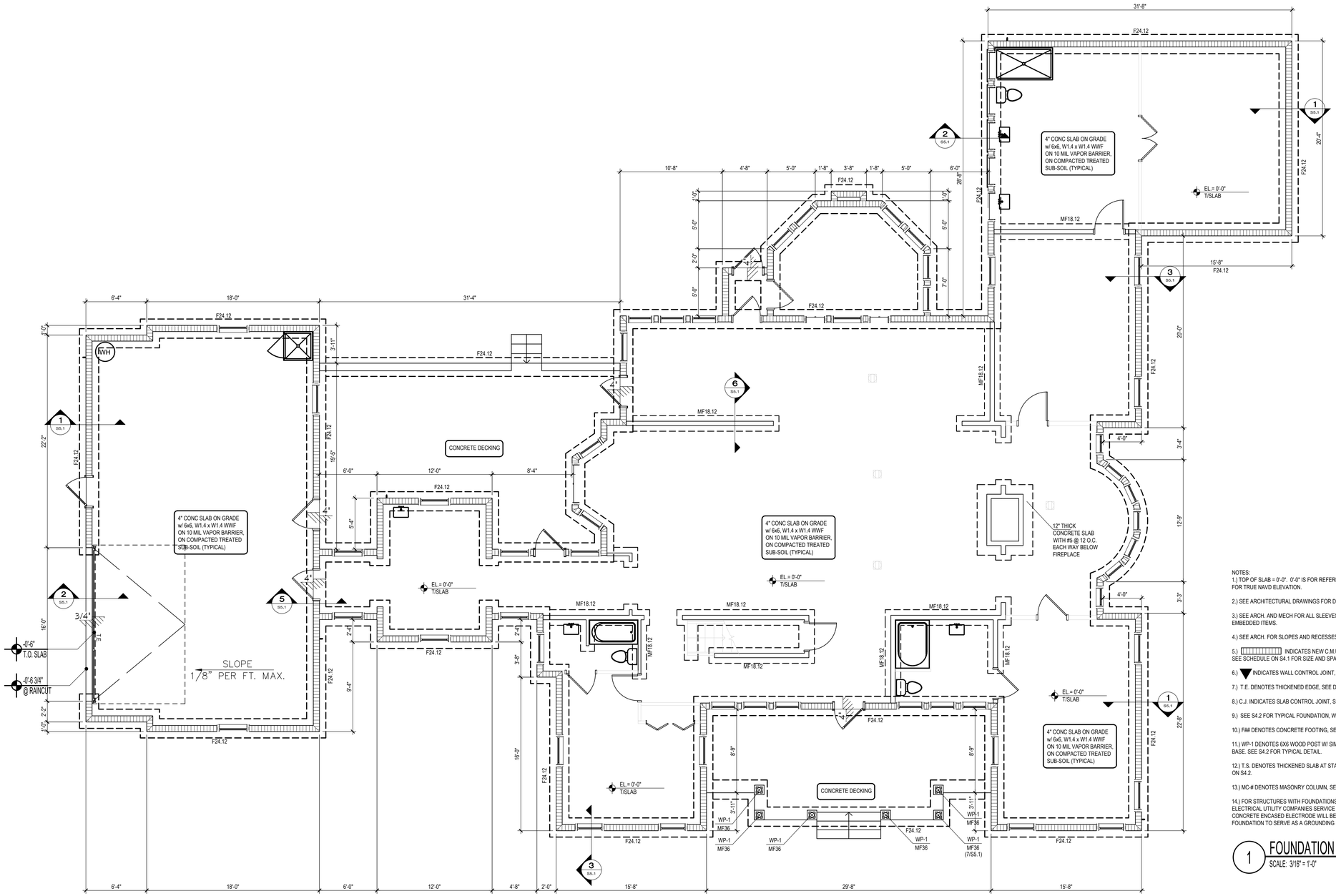
CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

STRUCTURAL NOTES

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S0.1

Scale AS NOTED



NOTES:
1.) TOP OF SLAB = 0'-0". 0'-0" IS FOR REFERENCE ONLY, SEE CIVIL FOR TRUE NAVD ELEVATION.

2.) SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.

3.) SEE ARCH. AND MECH FOR ALL SLEEVES, PIPES, INSERTS AND EMBEDDED ITEMS.

4.) SEE ARCH. FOR SLOPES AND RECESSES.

5.) ▭▭▭▭ INDICATES NEW C.M.U. WALLS (MW-1), SEE SCHEDULE ON S4.1 FOR SIZE AND SPACING OF REBAR.

6.) ▼ INDICATES WALL CONTROL JOINT, SEE DETAIL ON S4.2.

7.) T.E. DENOTES THICKENED EDGE, SEE DETAIL ON S4.2.

8.) C.J. INDICATES SLAB CONTROL JOINT, SEE S4.2 FOR DETAILS.

9.) SEE S4.2 FOR TYPICAL FOUNDATION, WALL AND SLAB DETAILS.

10.) F## DENOTES CONCRETE FOOTING, SEE SCHEDULE ON S3.1.

11.) WP-1 DENOTES 6X6 WOOD POST W/ SIMPSON ABU66 POST BASE. SEE S4.2 FOR TYPICAL DETAIL.

12.) T.S. DENOTES THICKENED SLAB AT STAIRS, SEE TYPICAL DETAIL ON S4.2.

13.) MC-# DENOTES MASONRY COLUMN, SEE SCHEDULE ON S4.1.

14.) FOR STRUCTURES WITH FOUNDATIONS WHICH ESTABLISH NEW ELECTRICAL UTILITY COMPANIES SERVICE CONNECTION A CONCRETE ENCASED ELECTRODE WILL BE REQUIRED WITHIN THE FOUNDATION TO SERVE AS A GROUNDING ELECTRODE SYSTEM.

1 FOUNDATION PLAN
SCALE: 3/16" = 1'-0"

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This item has been electronically signed and sealed by Bjarne Madsen using a Digital Signature and Date. Printed copies of this document are not considered signed and sealed. The signature must be verified on any electronic copies.

Bjarne Madsen
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REVISIONS

No.	Description	Date

CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

FOUNDATION PLAN

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S1.1

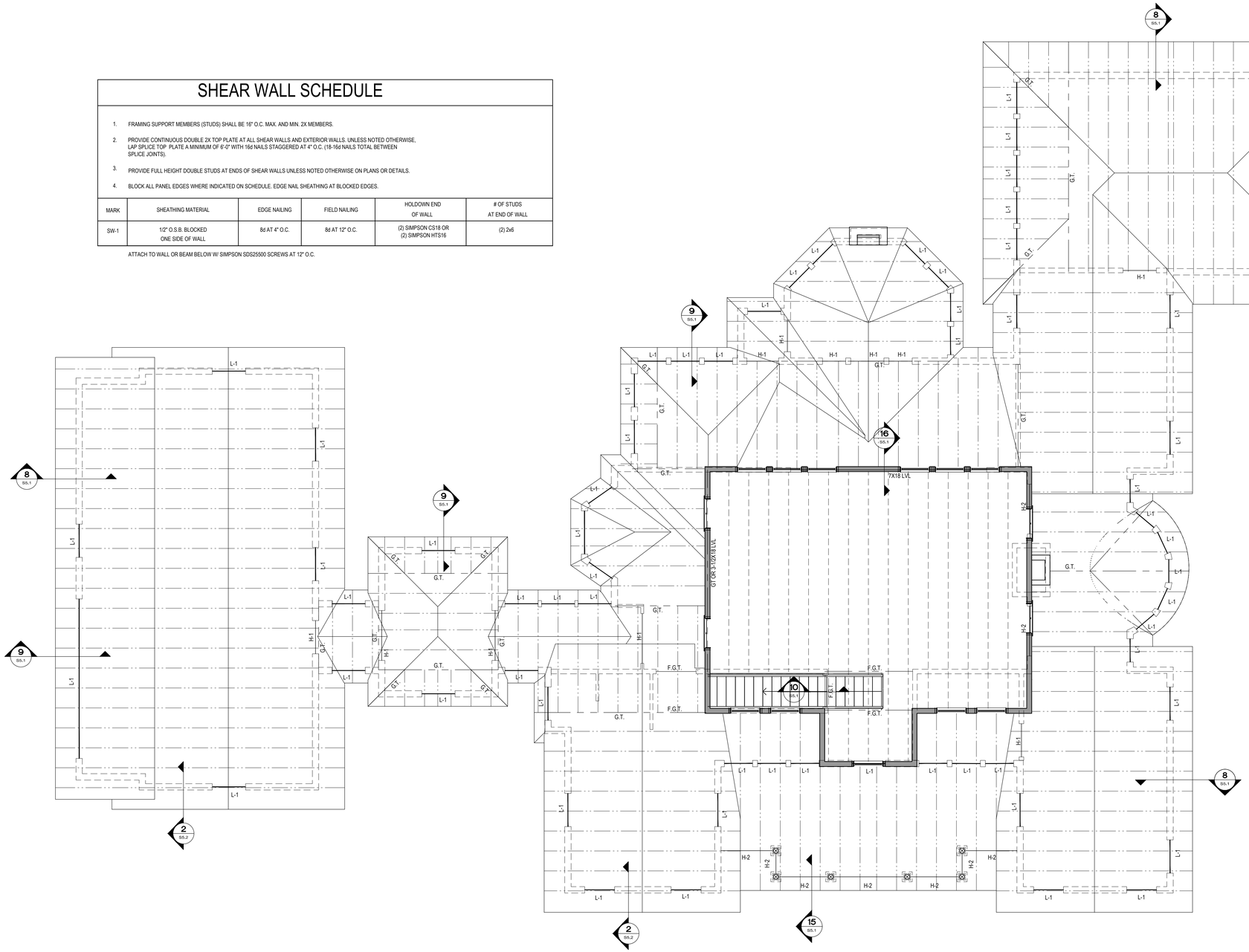
Scale AS NOTED

SHEAR WALL SCHEDULE

1. FRAMING SUPPORT MEMBERS (STUDS) SHALL BE 16" O.C. MAX. AND MIN. 2X MEMBERS.
2. PROVIDE CONTINUOUS DOUBLE 2X TOP PLATE AT ALL SHEAR WALLS AND EXTERIOR WALLS. UNLESS NOTED OTHERWISE, LAP SPLICE TOP PLATE A MINIMUM OF 6'-0" WITH 16d NAILS STAGGERED AT 4" O.C. (18-16d NAILS TOTAL BETWEEN SPLICE JOINTS).
3. PROVIDE FULL HEIGHT DOUBLE STUDS AT ENDS OF SHEAR WALLS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.
4. BLOCK ALL PANEL EDGES WHERE INDICATED ON SCHEDULE. EDGE NAIL SHEATHING AT BLOCKED EDGES.

MARK	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	HOLDOWN END OF WALL	# OF STUDS AT END OF WALL
SW-1	1/2" O.S.B. BLOCKED ONE SIDE OF WALL	8d AT 4" O.C.	8d AT 12" O.C.	(2) SIMPSON CS18 OR (2) SIMPSON HTS16	(2) 2x6

ATTACH TO WALL OR BEAM BELOW W/ SIMPSON SDS25500 SCREWS AT 12" O.C.



2ND FLOOR FRAMING NOTES:

1. BEARING ELEVATION - SEE ARCH
2. G.T. INDICATES ROOF GIRDER TRUSS BY TRUSS MANUFACTURER. O.B. INDICATES OVER-BUILT TRUSS BY TRUSS MANUFACTURER.
3. ROOF TRUSS AND MISC. TRUSSES SHALL BE SPACED AT 2'-0" O.C. MAX.
4. WOOD TRUSSES SHALL BE SPACED 2'-0" O.C. MAX.
5. — — — — — INDICATES 18" DEEP PRE-ENGINEERED WOOD FLOOR TRUSSES AT 24" O.C. MAX. DEPTH VARIES SEE ARCH FOR LOCATION.
6. F.G.T. INDICATES FLOOR GIRDER TRUSS BY TRUSS MANUFACTURER. CONNECT TO WOOD POSTS OR MASONRY WALLS W/ SIMPSON HUC HANGERS. USE A SIMPSON PC TO CONNECT TOP OF POST FOR CONTINUOS F.G.T. CONDITION.
7. SEE S4.1 FOR FLOOR, NAILING/FASTENING AND SHEATHING REQUIREMENTS.
8. WALL SHEATHING SHALL BE 1/2" CDX PLYWOOD FASTENED PER PLYWOOD SHEATHING NAILING SCHEDULE ON S4.1.
9. ▭ INDICATES LOAD BEARING 2X6 WOOD STUD WALLS AT 16" O.C.
10. ▬ INDICATES SHEAR WALL (SW-1, TYP.). SEE SCHEDULE THIS SHEET FOR REQUIREMENTS AND DETAILS.
11. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHT. S4.1.
12. EXTERIOR STUD WALLS SHALL BE 2X6 @ 16" O.C. INTERIOR LOAD BEARING WALLS SHALL BE 2X4 @ 16" O.C.
13. INTERIOR NON-LOAD BEARING STUDS SHALL BE 2X4 @ 24" O.C. (SEE ARCH'L DRAWINGS FOR LOCATIONS).
14. USE (2) SIMPSON HTS20 HOLDOWNS FOR ALL GIRDERS AND SIMPSON HCP FOR HIPS AND SIMPSON H10 FOR ALL OTHER TRUSSES. SEE HOLD DOWN SCHEDULE ON S4.1.
15. GIRDER TRUSSES SHALL BE DESIGN FOR AN ADDITIONAL POINT LOAD OF 1,000 LBS AT THE END OF ALL SHEARWALLS. FORCE SHALL BE APPLIED BOTH UPWARDS AND DOWNWARDS IN SEPARATE LOAD CASES.
16. ATTACH HEADERS AND FGT'S TO WOOD POSTS WITH (2) SIMPSON CS18 OR HTS16.
17. L- INDICATES PRECAST LINTEL, SEE SCHEDULE ON S4.1. FOR OPN'S LESS THAN 4' USE 8F8-1T.
18. USE SIMPSON HETAL 16 (#) STRAPS @ TRUSS BEARING ON MASONRY WALL CONNECTIONS, TYP. SEE HOLD DOWN SCHEDULE ON S4.1.
19. CONNECT WOOD HEADERS TO MASONRY W/ SIMPSON HUS HANGERS. USE 3/16"Ø TAPCONS 1-1/2" LONG.
20. PLACE A DBL. BOND BEAM ON TOP OF ALL MASONRY WALLS W/(1)#5 TOP AND BOTTOM.

1 2ND FLOOR FRAMING PLAN
SCALE: 3/16" = 1'-0"

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No.	Description	Date

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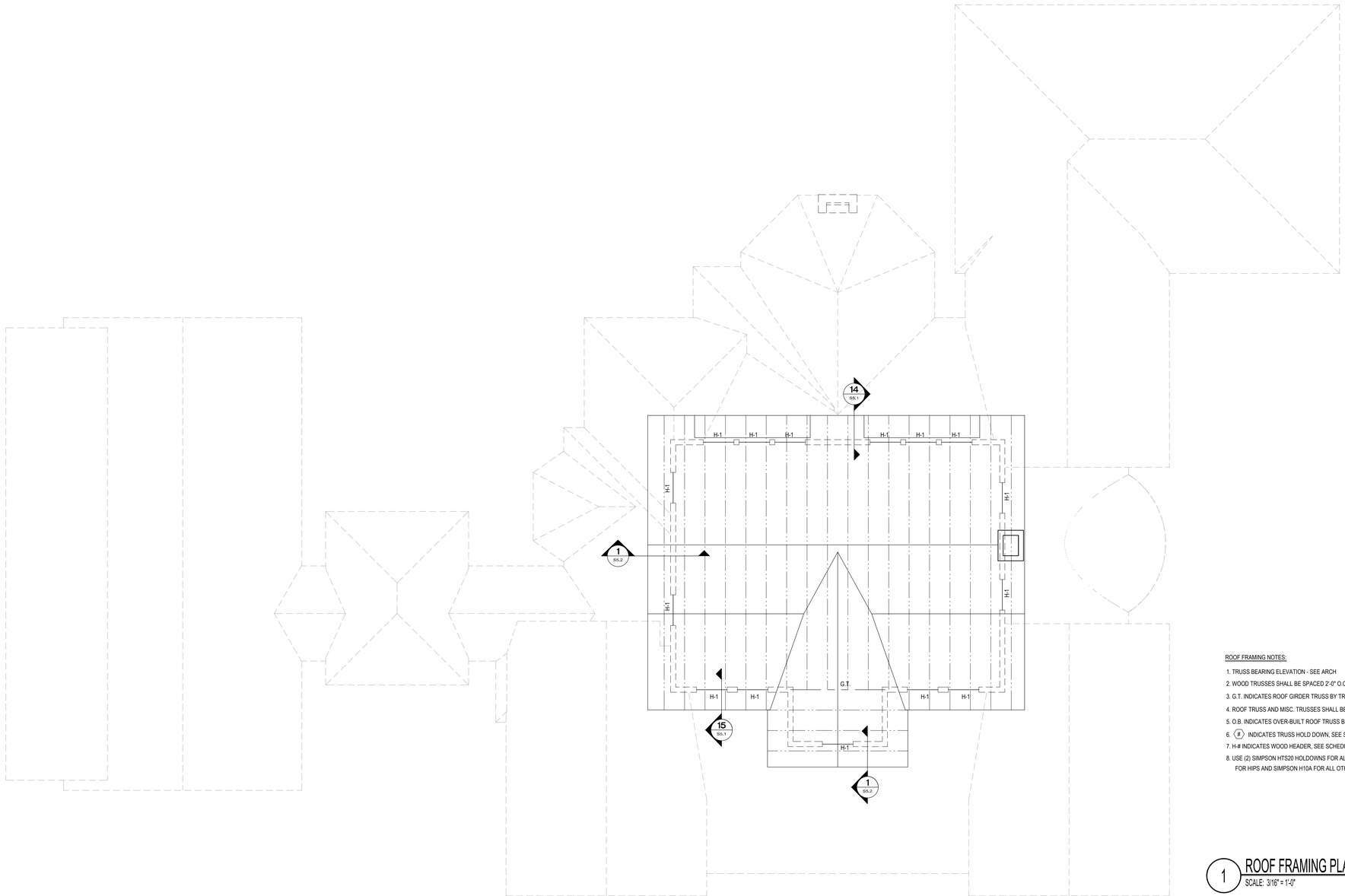
LAKE CITY, FLORIDA 32055

2ND FLOOR FRAMING PLAN

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM
	S2.1
Scale	AS NOTED



ROOF FRAMING NOTES:

1. TRUSS BEARING ELEVATION - SEE ARCH
2. WOOD TRUSSES SHALL BE SPACED 2'-0" O.C. MAX.
3. G.T. INDICATES ROOF GIRDER TRUSS BY TRUSS MANUFACTURER.
4. ROOF TRUSS AND MISC. TRUSSES SHALL BE SPACED AT 2'-0" O.C. MAX.
5. O.B. INDICATES OVER-BUILT ROOF TRUSS BY TRUSS MANUFACTURER.
6. (#) INDICATES TRUSS HOLD DOWN, SEE SCHEDULE THIS SHEET.
7. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHEET S4.1.
8. USE (2) SIMPSON HTS20 HOLDOWNS FOR ALL GIRDERS, SIMPSON HCP FOR HIPS AND SIMPSON H10A FOR ALL OTHER TRUSSES. SEE HOLD DOWN SCHEDULE ON S4.1.

1 ROOF FRAMING PLAN
SCALE: 3/16" = 1'-0"

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No.	Description	Date

CLINTON RESIDENCE
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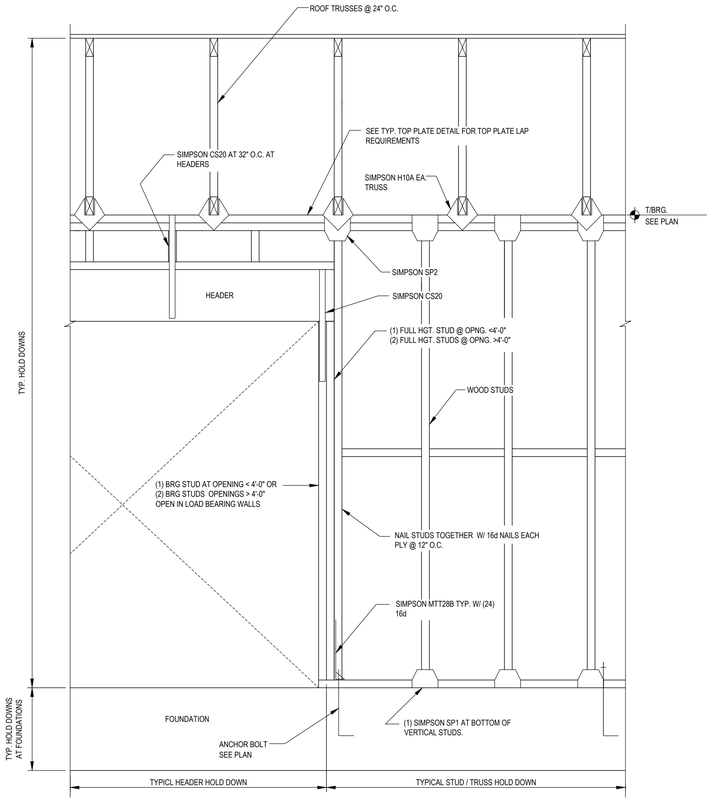
LAKE CITY, FLORIDA 32055

ROOF FRAMING PLAN

Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S3.1

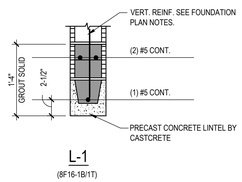
Scale AS NOTED



TYPICAL WOOD STUD BEARING WALL ELEVATION

N.T.S.

APPLIES TO ALL STUD WALLS SUPPORTING ROOF TRUSSES. ANCHORAGE/STRAPPING FOR ENDS OF SHEAR WALLS IS NOT SHOWN AND IS IN ADDITION TO THAT SHOWN HERE.



L-1

(8F16-1B/1T)

NOTES:

1. 8" PRECAST LINTELS BY CASTCRETE CORPORATION.
2. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
3. SEE THE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
4. PROVIDE 8" MINIMUM BEARING EACH END.

8" C.M.U. PRECAST LINTEL SCHEDULE

N.T.S.

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
MF18.12	1'-6" X 12" X CONT.	(2) #5 CONT. BOTTOM
F24.12	2'-0" X 12" X CONT.	(2) #5 CONT. BOTTOM
F36.12	3'-0" X 12" X CONT.	(3) #5 CONT. BOTTOM
F36	3'-0" X 3'-0" X 12"	(3) #5 E.W. BOTTOM
MF36	3'-0" X 3'-0" X 18"	(4) #5 E.W. BOTTOM

FASTENER SUBSTITUTIONS:

ALL NAILS ARE COMMON NAILS, UNLESS NOTED OTHERWISE. THE FOLLOWING FASTENERS ARE ACCEPTABLE SUBSTITUTIONS. THE ALTERNATE FASTENERS SHALL BE SPACED AT THE SAME SPACING AS THE SCHEDULED FASTENERS.

SCHEDULED FASTENER	ALTERNATE FASTENER
8d COMMON NAIL	8d RING SHANK NAIL 8d SCREW SHANK NAIL 0.131 P–NAIL
10d COMMON NAIL	10d RING SHANK NAIL 10d SCREW SHANK NAIL 0.148 P–NAIL
6d COOLER NAIL	#6 x 1 1/4" TYPE S OR W DRYWALL SCREW

PLYWOOD SHEATHING NAILING SCHEDULE
(WALLS AND ROOF)

NAIL SIZE	NAIL SPACING	ZONE
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	ROOF (1)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	ROOF (2)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	ROOF (3)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	WALL (4)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	WALL (5)

CORNER DISTANCE, A = 7 FEET

PROVIDE ANCHOR STRAP FROM TABLE BELOW AT EACH BEARING POINT FOR EACH WOOD TRUSS AND EACH GIRDER TRUSS ADEQUATE TO RESIST UPLIFT AS SPECIFIED BY THE WOOD TRUSS MANUFACTURER.

"SIMPSON" TRUSS TIE DOWN (U.N.O.)

MARK	ANCHOR TYPE	NAILS TO TRUSS	NAILS TO SEAT	BOLTS	ALLOWABLE UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPEND. TO WALL
A	SIMPSON H3	4-8d x 1 1/2"	4-8d	-	455 # (*)	125 #	160 #
B	SIMPSON H10A	9-10d x 1 1/2"	9-10d	-	1140 # (*)	585 #	525 #
C	SIMPSON HCP	6-10d x 1 1/2"	6-10d	-	605 # (*)	-	-
D	SIMPSON MGT	22-10d x 1 1/2"	NOTE 5	5/8"Ø	3965 # (**)	-	-
E	HETAL 16	14-10d x 1 1/2"	-	-	1810 # (*)	415 #	1100 #
F	SIMPSON HGT-2	16-10d	NOTE 5	(2) 3/4"Ø	10880 # (**)	-	-

NOTES:
1. (*) - ONE PLY MEMBER (**) - TWO PLY (MIN.) MEMBER
2. T.B. - THRU-BOLT
3. U.N.O. - UNLESS NOTED OTHERWISE
4. APPROVED EQUAL OR BETTER TIE DOWNS FOR THE SAME LATERAL & UPLIFT LOADS ARE ACCEPTABLE.
5. USE SIMPSON "SET" EPOXY W/ 12" MIN. EMBED.

HEADER SCHEDULE

MARK	SIZE (IN.)	BEARING STUDS
H-1	(2) 2 X 10 / (3) 2 X 8	(1) B & (1) FULL HEIGHT
H-2	(2) 2 X 12 / (3) 2 X 10	(2) B & (1) FULL HEIGHT
H-3	(3) 2 X 12	(2) B & (1) FULL HEIGHT
H-4	3-1/2 X 9-1/4 P.S.L.	(2) B & (1) FULL HEIGHT
H-5	3-1/2 X 11-1/4 P.S.L.	(2) B & (2) FULL HEIGHT
H-6	5-1/4 X 14 P.S.L.	(2) B & (3) FULL HEIGHT

NOTES:
1. B INDICATES BEARING.
2. USE (2) FULL HEIGHT STUDS AT TRUSS BEARING WALLS PER TYPICAL STUD WALL ELEVATION

DESIGN LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT	LIVING	ROOF							
SYSTEM	15	15							
MISC.	10	5							
TOTAL DEAD LOAD	25	20							
TOTAL LIVE LOAD	40	20							
TOTAL LOAD	65	40							

MASONRY WALL SCHEDULE

MARK	THICKNESS	REINFORCING
MW-1	8" CMU	#5 @ 48" O.C.
MW-2	8" CMU	#5 @ 24" O.C.

MASONRY WALL NOTES:

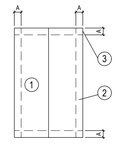
1. WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 6" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
2. ADJACENT TO ANY EXTERIOR/INTERIOR WALL OPENING, PLACE 1 #5 VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT.
3. ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.

ROOF WIND PRESSURE (PSF)
COMPONENTS AND CLADDING-EXP C-108 MPH(ASD) WIND SPEED

EFFECTIVE WIND AREA (SQ. FT.)	ROOF AREA 1	2	3
10	+18.1/-28.9	+18.1/-61.0	+18.1/-61.0
20	+16.5/-28.1	+16.5/-55.4	+16.5/-55.4
50	+14.5/-27.0	+14.5/-47.9	+14.5/-47.9
100	+14.5/-26.2	+14.5/-42.2	+14.5/-42.2

NOTES:

1. WIND DESIGN PER FBC-2020
2. + INDICATES WIND PRESSURE
   - INDICATES WIND SUCTION
3. WALL DISTANCE A = 7 FT. (COMPONENTS AND CLADDING)
4. FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE WIND AREA
5. MULTIPLY VALUES WITH 1.67 FOR ULT.



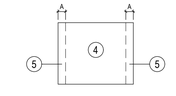
PLAN

DOOR & WINDOW WIND PRESSURE (PSF)
COMPONENTS AND CLADDING-EXP C-108 MPH(ASD) WIND SPEED

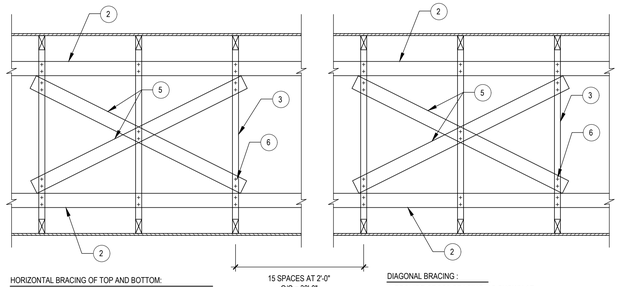
SIZE OF WALL OPENING (SQ. FT.)	WALL AREA 4	5
10	+31.6/-34.2	+31.6/-42.2
20	+30.2/-32.8	+30.2/-39.4
50	+28.3/-30.9	+28.3/-35.7
100	+26.8/-29.6	+26.8/-32.8

NOTES:

1. WIND DESIGN PER FBC-2020
2. + INDICATES WIND PRESSURE
   - INDICATES WIND SUCTION
3. WALL DISTANCE A = 7 FT. (COMPONENTS AND CLADDING)
4. FOR WALL OPENINGS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER WALL OPENING AREA.
5. MULTIPLY VALUES WITH 1.67 FOR ULT.



WALL ELEVATION



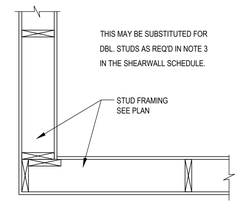
HORIZONTAL BRACING OF TOP AND BOTTOM:

1. PROVIDE ONE ROW OF HORIZONTAL BRACING AS SHOWN EVERY 10'-0" PERPENDICULAR TO SPAN OF TRUSS.
2. BRACING MEMBERS TO BE CONTINUOS 2X4 WOOD MEMBERS. AT SPLICE LOCATIONS, LAP BRACING MEMBERS OVER AT LEAST 2 TRUSSES.
3. WOOD TRUSSES AT 24" O.C.

DIAGONAL BRACING :

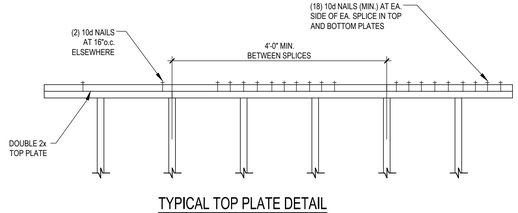
4. PROVIDE CROSS BRACING, AS SHOWN AT EACH END OF EACH ROW OF PERMANENT HORIZONTAL BRACING, AND AT NOT MORE THAN 30'-0" SPACING IN DIRECTION OF HORIZONTAL BRACING.
5. BRACING MEMBERS TO BE 2X4 WOOD MEMBERS.
6. (2) 10d NAILS (TYP.)

TYPICAL BRACING DETAIL



TYPICAL CORNER PLAN DETAIL

SCALE: N.T.S.



TYPICAL TOP PLATE DETAIL

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REVISIONS

No.	Description	Date

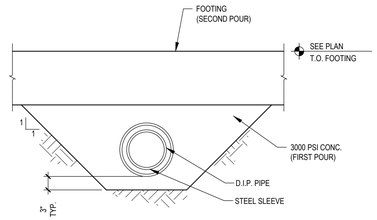
CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

STRUCTURAL SCHEDULES

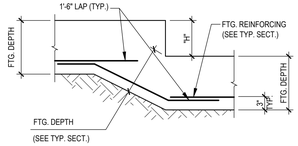
Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S4.1

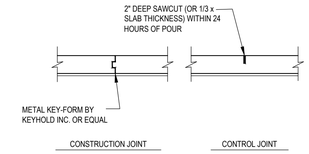
Scale AS NOTED



TYP. PIPE UNDER FTG.

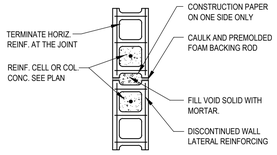


TYP. STEPPED FTG.



TYPICAL SLAB-ON-GRADE

NOTE: CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 400 SQ. FEET



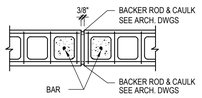
PLAN VIEW

NOTES :

1.- SAW CUT BOND BEAMS, TIE BEAMS 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL.

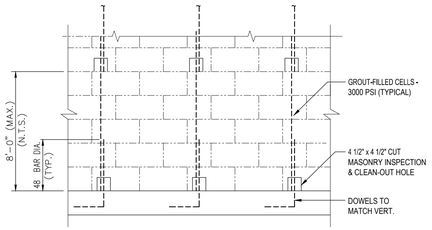
2.- CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0"o.c. IN WALLS WITH MORE THAN 25'-0" OF UNINTERRUPTED MASONRY. REFER TO DWG'S. FOR ADDITIONAL SPECIFIED LOCATIONS AS NOTED THUS (WCJ).

3.- CONTINUE ALL BOND BEAMS, TIE BEAMS REINF. THROUGH THE JOINT.



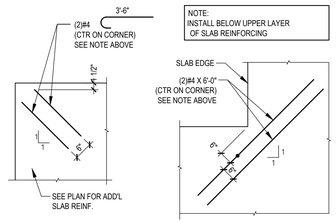
ALTERNATE METHOD

CMU WALL CONTROL JOINT (WCJ) DETAIL

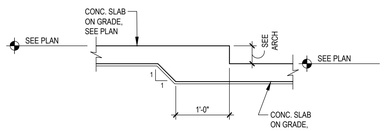


TYPICAL MASONRY FILLED CELL DETAIL

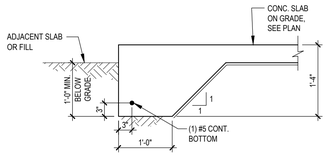
N.T.S.



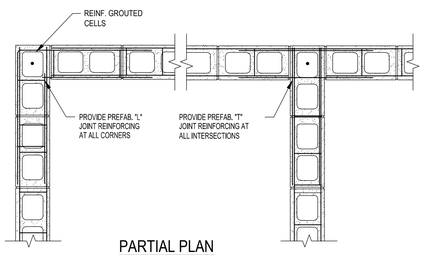
TYP. SLAB CORNER REINF.



TYPICAL SLAB RECESS



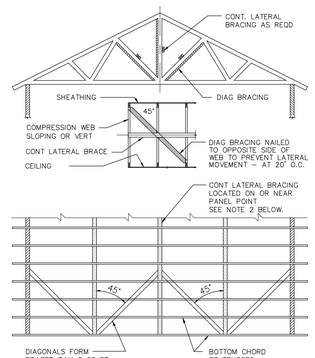
THICKENED EDGE (T.E.)



PARTIAL PLAN

TYPICAL CMU WALL DETAILS

N.T.S.



1. WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE" BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS, HIB-91, BRACING IN THE PLAN OF THE WEB MEMBERS :

   a. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.

   b. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER). THIS BRACING IS TO BE CONTINUOUS.

   c. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.

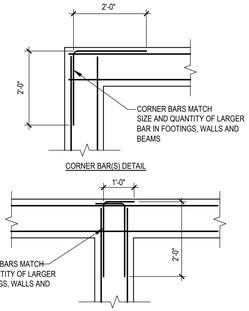
2. THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8'-0" O. C. WITH A CEILING ATTACHED TO BOTTOM OF TRUSSES. OR IF NO CEILING IS ATTACHED TO BOTTOM OF TRUSSES BRACING SHALL BE MIN. 2 x 4 @ 36" O.C. NAILED TO THE TOP OF THE BOTTOM CHORD. DIAGONALS PLACED AT 45° TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END. IF BUILDING EXCEEDS 80 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

3. TOP CHORD BRACING :

   a. IF PLYWOOD DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND NAILED TO DEVELOP DIAPRAGHM ACTION, BRACING IS NOT REQUIRED.

   b. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 80 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

WOOD TRUSS BRACING DETAIL



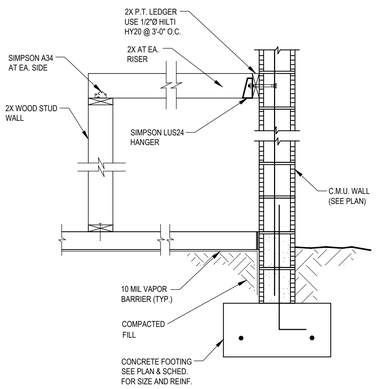
TYPICAL BAR DETAILS

BUCKS TO BE FASTENED HORIZONTALLY AND VERTICALLY TO CONCRETE BEAMS AND COLUMNS OR CONCRETE FILLED MASONRY.

G.C. TO COORDINATE OPENING DIMENSIONS.

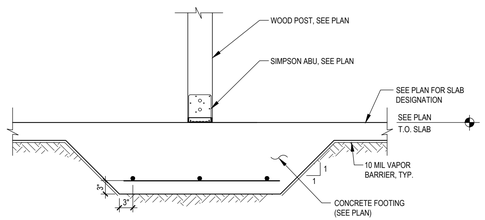
TYPICAL WOOD BUCK TO CONCRETE CONNECTION DETAIL

1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH 2004 FLORIDA BUILDING CODE BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.



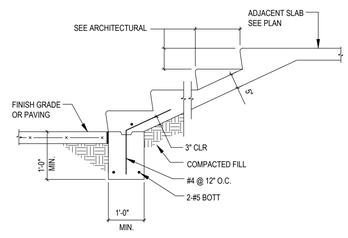
TYPICAL STAIR LANDING DETAIL

N.T.S.



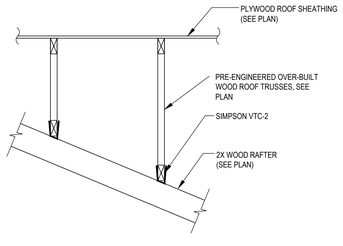
TYPICAL WOOD POST FOOTING DETAIL

N.T.S.



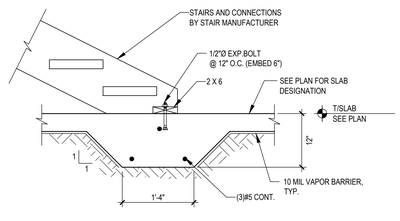
TYPICAL CONCRETE STAIR ON GRADE

N.T.S.



TYPICAL OVER-BUILT DETAIL

N.T.S.



TYPICAL STAIR (T.S.) DETAIL

N.T.S.

dansk consulting LLC
1673 LAKE BALDWIN LANE
ORLANDO, FLORIDA 32814
PH. (407) 252-4225
LIC EB28029

Bjarne Madsen
2023.09.18
17:20:18-04'00'

This item has been electronically signed and sealed by BJARNE MADSEN using a Digital Signature and Date. Printed copies of this document are not considered signed and sealed. The signature must be verified on any electronic copies.

REVISIONS

No.	Description	Date

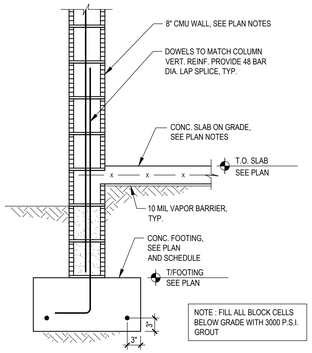
CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

TYPICAL DETAILS

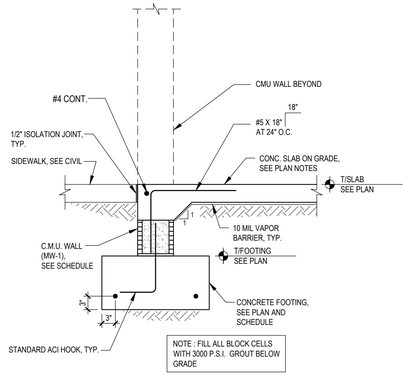
Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S4.2

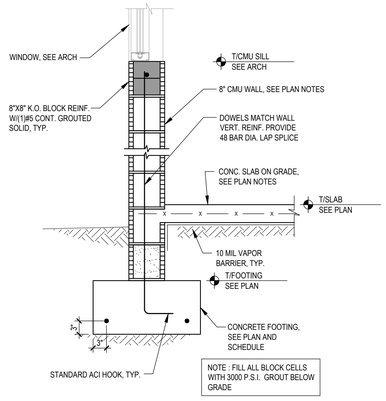
Scale AS NOTED



SECTION 1
3/4" = 1'-0"
S5.1

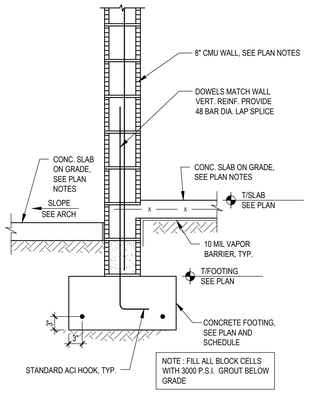


SECTION 2
3/4"=1'-0"
S5.1

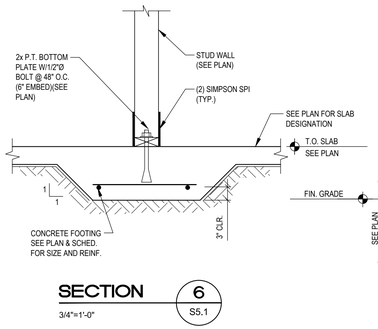


SECTION 3
3/4"=1'-0"
S5.1

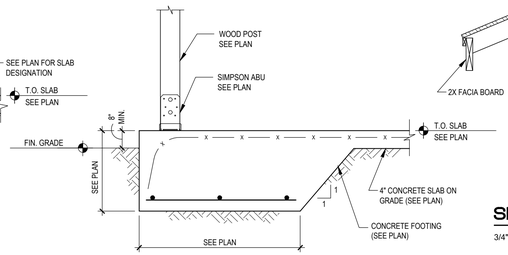
SECTION 4
NOT USED
S5.1



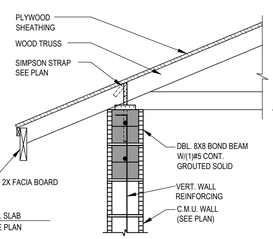
SECTION 5
3/4"=1'-0"
S5.1



SECTION 6
3/4"=1'-0"
S5.1

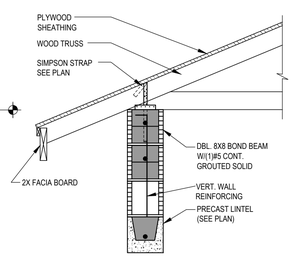


SECTION 7
3/4"=1'-0"
S5.1

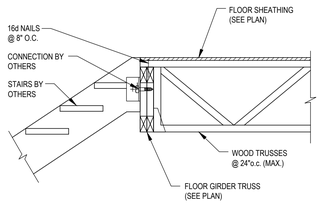


SECTION 8
3/4" = 1'-0"
S5.1

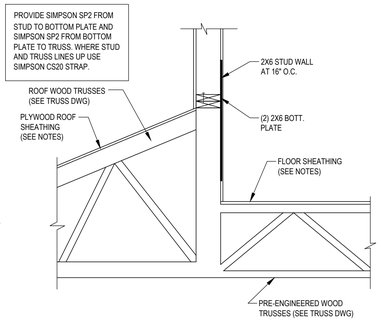
SEE PLAN ROOF BRG.



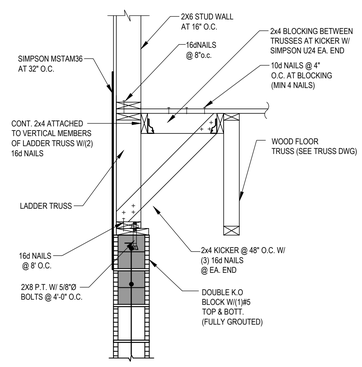
SECTION 9
3/4" = 1'-0"
S5.1



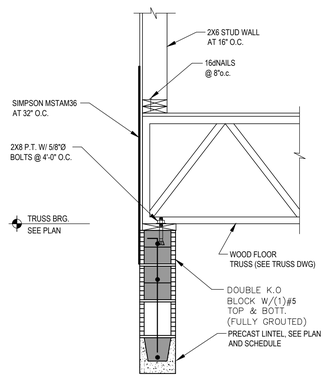
SECTION 10
3/4" = 1'-0"
S5.1



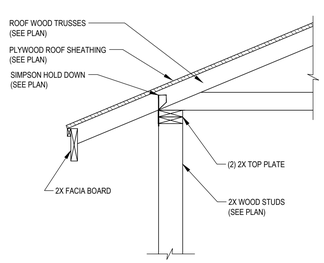
SECTION 11
3/4"=1'-0"
S5.1



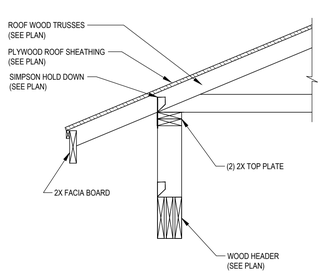
SECTION 12
3/4"=1'-0"
S5.1



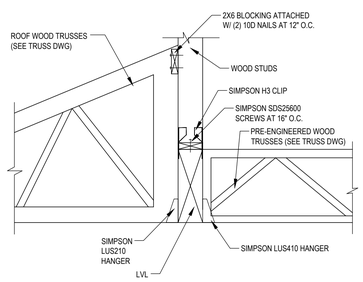
SECTION 13
3/4"=1'-0"
S5.1



SECTION 14
3/4"=1'-0"
S5.1



SECTION 15
3/4"=1'-0"
S5.1



SECTION 16
3/4"=1'-0"
S5.1

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REVISIONS

No.	Description	Date

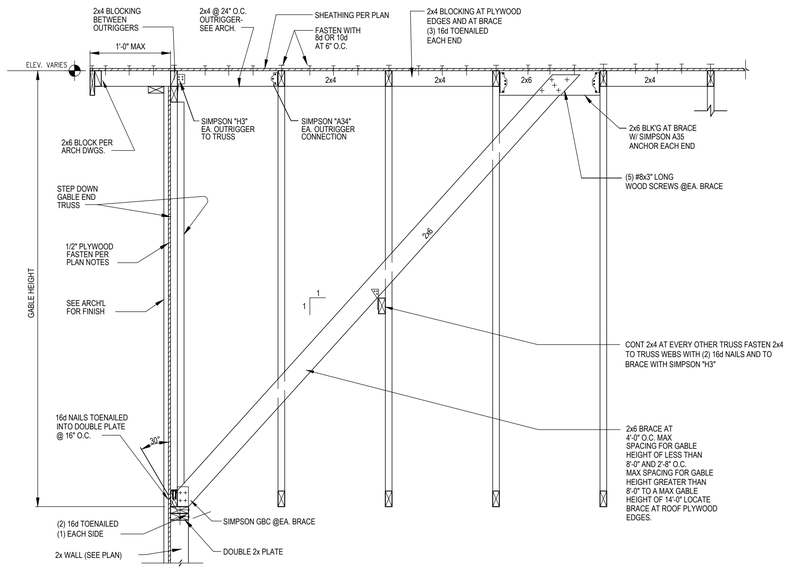
CLINTON RESIDENCE
889 NW BLACKBERRY CT.
LAKE CITY, FLORIDA 32055

STRUCTURAL SECTIONS

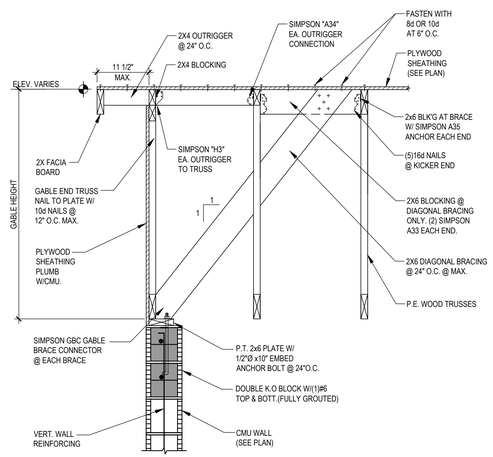
Project number	05
Date	09-16-23
Drawn by	rC
Checked by	bM

S5.1

Scale AS NOTED



SECTION 1
3/4"=1'-0"
S5.2



SECTION 2
3/4" = 1'-0"
S5.2

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STRUCTURAL SECTIONS

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S5.2

Scale AS NOTED