

DIAGONAL AND THE CEILING IS APPROX. 4!

40 MIN, 55 MAX, NAILED AT EACH END WITH

SEVEN (7) 10d NAILS.

MANUFACTURER'S ENGINEERING TRUSS CONNECTOR UPLIFT SYP UPLIFT SPF | F1F1 SYP | F2 SYP | F1 SPF | F2 SPF | TO RAFTER/TRUSS | TO PLATES 125 160 105 140 4-8d x 1 1/2" 4-8d x 1 1/2 150 150 130 130 5-8d x 1 1/2" H2.5 365 5-8d x 1 1/2 H2.5A 5-8d x 1 1/2 5-8d x 1 1/2 H6 950 820 8-8d 5-10d x 1 1/2" 5-10d x 1 1/2 H14-1 1050 12-8d x 1 1/2 13-8d H14-2 1465 1050 515 265 480 245 12-8d x 1 1/2" 15-8d H10 850 585 525 505 450 8-8d x 1 1/2 8-8d x 1 1/2' H10-2 655 6-10d 6-10d 1470 1265 H16 2-10d x 1 1/2" 10-10d x 1 1/ H16-2 1470 1265 2-10d x 1 1/2" 10-10d x 1 1/ 620 1000 LTS12 - LTS20 6-10d x 1 1/2" 6-10d x 1 1/ 1000 7-10d x 1 1/2" 7-10d x 1 1/ HTS16 - HTS30 1450 1245 12-10d x 1 1/2" | 12-10d x 1 1/2 HEAVY GIRDER TIEDOWN TO FOUNDATION LGT2 14-16d 14-16d LGT3-SDS2.5 2655 795 410 795 410 12-SDS 1/4" x 2 1/2" 26-16dS 3860 LGT4-SDS3 4060 12-SDS 1/4" x 3" 36-16dS 22 -10d 5/8" ANCHOR HGT-2 10980 6485 16 -10d 2-5/8" ANCHOR 10530 9035 HGT-3 16 -10d 2-5/8" ANCHOR 9250 9250 HGT-4 16 -10d 2-5/8" ANCHOR STUD STRAP CONNECTOR TO STUDS SSP DOUBLE TOP PLATE 435 3-10d 4 -10d SSP SINGLE SILL PLATE 455 420 4 -10d 1-10d DSP DOUBLE TOP PLATE 6 -10d 8 -10d DSP SINGLE SILL PLATE 825 600 2 -10d 8 -10d 535 4 -10d 6 -10d 1065 605 SP2 6 -10d 6 -10d 885 6-10d x 1 1/2" SPH4 1240 1065 10-10d x 1 1/2 885 SP6 6-10d x 1 1/2 1240 SPH6 10-10d x 1 1/2" LSTA18 1235 1110 14-10d 1235 1235 LSTA21 16-10d CS20 1030 1030 14-10d 1705 1705 CS16 22-10d STUD ANCHORS TO FOUNDATION TO STUDS LTT19 1350 1/2" ANCHOR 8-16d 2310 2310 18-10d x 1 1/2" 5/8" ANCHOR HD2A 2775 2570 2-5/8" BOLTS 5/8" ANCHOR 4175 HTT16 3695 5/8" ANCHOR 18-16d 5260 5250 HTT22 32-16d 5/8" ANCHOR 2200 2200 ABU44 12-16d 5/8" ANCHOR 2300 2300 ABU66 12-16d 5/8" ANCHOR

> MASONRY TRUSS ANCHOR TABLE OBTAIN UUPLIFT REQUIREMENTS FROM TRUSS MANUFACACTURER'S ENGINEERING

ABU88

2320 2320

UPLIFT L LBS.	TRUSS CONNECTOR MASONRY *		
< 1205:05	TA22	10-10d x 1 1/2"	
< 160505	TA22	11-10d	
< 860 ₆₀	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS	
< 117 [£] 75	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS	
< 104940	META20	7-10d, 1 1/2"	
< 149(90	META20	10-10d, 1 1/2"	
< 178680	HETA20	7-16d	
< 1786 ₈₀	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER	
< 213(30	HHETA20	17-10d, 1 1/2"	
< 231910	HHETA24	21-10d, 1 1/2"	
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT	
< 1098 ₉₈₀	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT	
< 1053530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT	

18-16d

2-5/8" ANCHOR

MASONNRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURIRES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMME IEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFL-LICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEP: PTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
moisture or wire ties, anch ties not completely embed		Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints			

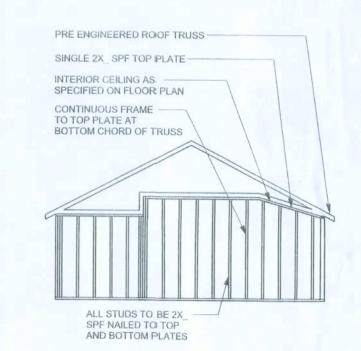
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP#2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL WOOD FRAME

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT 3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C. 1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C 1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W

3/16" TAPCONS @ 12" O.C 1/4" TAPCONS @ 18" O.C.

GARAGE DOOR UP TO 10'W (2) 3/16" TAPCONS & 16" O.C.

(2) 1/4" TAPCONS & 24" O.C. GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C. (2) 1/4" TAPCONS & 12" O.C.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116

CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS; WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNO

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTE FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRE LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

ERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT I THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRE OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES. RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

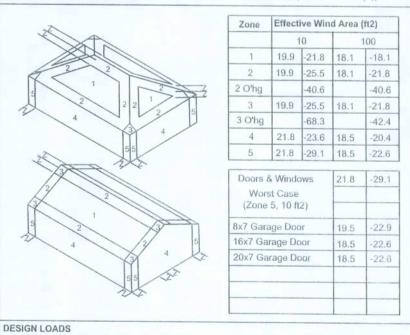
DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B. 30FT IN EXP. C. AND >105 SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LES BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION BASIC WIND SPEED = 110 MPH

WIND EXPOSURE = B WIND IMPORTANCE FACTOR = 1.0

BUILDING CATEGORY =) ROOF ANGLE = 10-45 DEGREE MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) .) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) OF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) TAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN

o.53915, POB 868, Lke City, FL 32056, 386-754-5419 DIMENSIONS: ensions. Refer all questions to Mark Disosway, P.E. fo resolution not proceed withoutclarification OPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. heeby expressly serves its common lay copyrights and operty right in these istruments of service is document is not tobe reproduced, alter copied in any form o manner without firs e express written pemission and consent of Mark Disosway. CERTIFICATION: I her/by certify that I have xamined this plan, anothat the applicable portions of the plan, relating to wind engineering compl with section R301.2.1, florida buildir; code to the best of my knowledge. IMITATION: This design is valid for one ouilding, at specified loation. P.E. 51915 Restoration Specialists

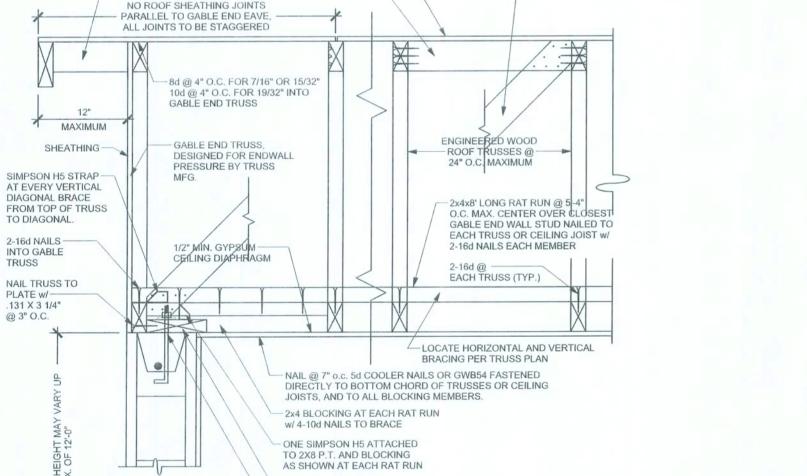
Abels Residence Regair ADDIESS: BranfordHighway Columbia Conty, Florida Mark Disosway P.E. P.O. Box 868 Lake City, Fbrida 32056 Phone: (386 754 - 5419 Fax: (386) 269 - 4871 PRINTE) DATE: June 04, :009

STRUCTURAL B DRAWN BY: David Disosway

FINALS DATE JOB NUMBER:

> 906014 DRAWINGNUMBER S-1

OF 3 SHEETS



2X8 PT SYP #2 PLATE

- 1/2"X10" ANCHOR BOLTS WITH 2X2X.140" STEEL WASHER 8" FROM CORNERS & 48" O.C.

GABLE END DETAIL (CMU WALL)

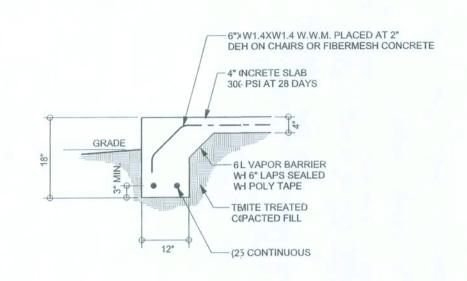
FRAMING @

LOOR SLAB

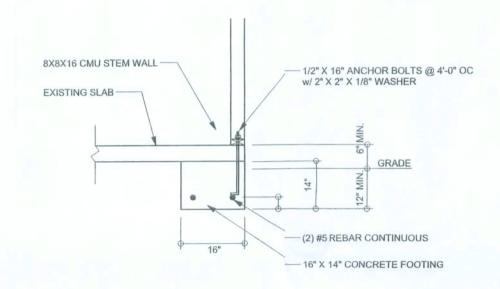
3-16d NAILS EACH END

24" O.C.

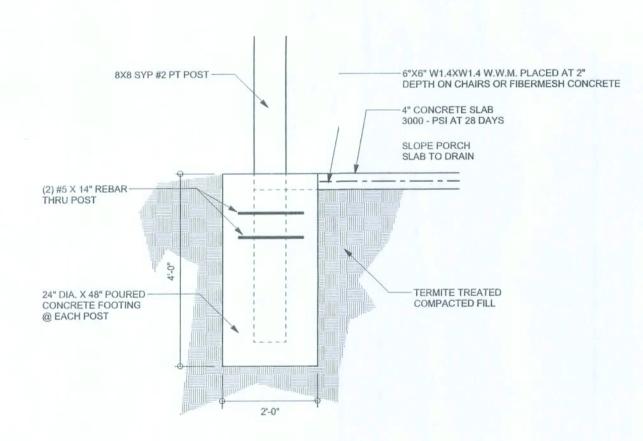
TO TOP PLATE NAILS 12d @ 6" O.C. SIMPSON LSTA2 -2X4X8' RAT RUN NAIL EACH w/ (8) -16d TO TRUSS & (8) -16d TO WALL @ 48" O.C. U.N.O. CONNECTION w/ (4) .131"X3 1/4" NAILS (4) .131"X3 1/4" NAILS - (8) .131"X3 1/4" NAILS - 2X4 SPF#2 BLOCKING H3 INSTALLED HORIZONTALLY SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED (TYP.) GABLE BRACING DETAIL WOOD FRAME



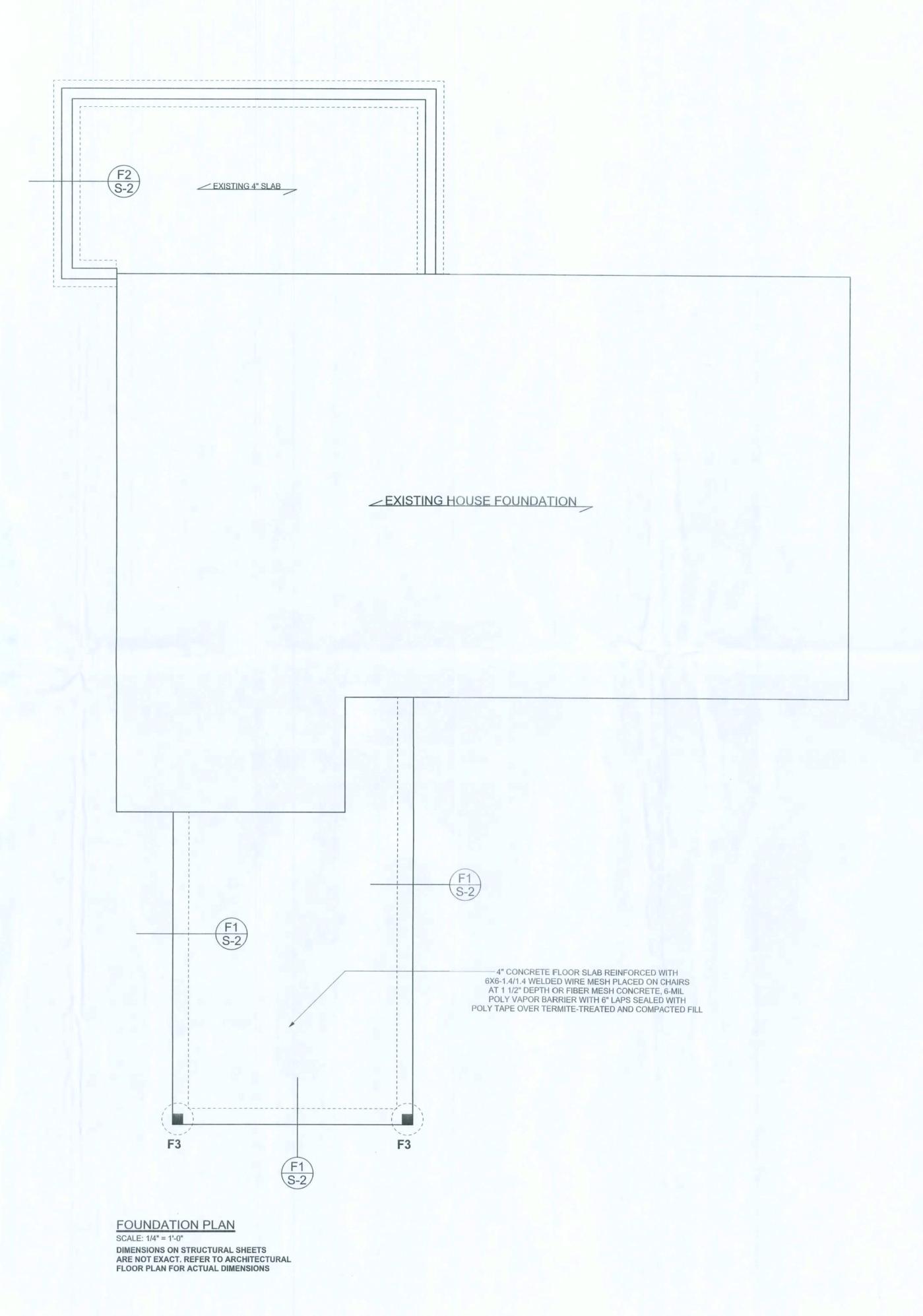
MONOLITHI FOOTING S-2 SCALE: 1/2" = 1'-0"



F2 FOOTING R'PAIR S-2 SCALE: 1/2" = 1'-0'



F3 REAR PORCIPOST FOUNDATION
S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN ARCHITECURAL DESIGN SOFTWARE

WINDLOAD ENGIFEER: Mark Disosway, PE No.53915, POB 86i, Lake City, FL 32056, dimensions. Refer ill questions to Mark Disosway, P.i. for resolution. Do not proceed wittout clarification. COPYRIGHTS ANI PROPERTY RIGHTS: COPYRIGHTS ANI PROPERTY RIGHTS:
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reserves its common law copyrights and
property right in thee instruments of service.
This document is not to be reproduced, altered
or copied in any fom or manner without first
the express writtencermission and consent
of Mark Disosway. CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plar relating to wind engineering cmply with section R301.2.1, florida bilding code residential 2007, to the best of my knowledge.

LIMITATION: This lesign is valid for one building, at specifie location. MARI DISOSWAY PE. 53915

Restoration Specialists

AbelsResidence Repair

ADDRESS: Branfrd Highway Columbia County, Florida

Mark Disosway P.E. P.O.Box 868 Lake City Florida 32056 Phone: (336) 754 - 5419 Fax: (383) 269 - 4871

PRIITED DATE: June (4, 2009 DRAWN BY: STRUCTURAL BY

David Disosway

FINALS DATE: 3Jun09

JOB NUMBER: 9)6014 DRAWNG NUMBER

> 5-2 OF3 SHEETS

