

Project Information for:

L251203

Address:

16665 CR 138

LAKE CITY, FL

County:

COLUMBIA COUNTY

Truss Count:

Design Program: MiTek 20/20 6.3 **Building Code:** FBC2004/TPI2002

Truss Design Load Information: Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

WILLIAM CASON Florida License No. CBC060151

Address: 10 NW 15 ST, ALACHUA COUNTY,

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Drwg.#	Truss ID	Date
J1891891	EJ4	9/17/07
J1891892	EJ6	9/17/07
J1891893	T01	9/17/07
J1891894	T01G	9/17/07
J1891895	T02	9/17/07
J1891896	T02G	9/17/07
J1891897	T04G	9/17/07
J1891898	T05G	9/17/07
	J1891891 J1891892 J1891893 J1891894 J1891895 J1891896 J1891897	J1891891 EJ4 J1891892 EJ6 J1891893 T01 J1891894 T01G J1891895 T02 J1891896 T02G J1891897 T04G

September 17,2007

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Wind Exposure: B

September 17,2007

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

WILLIAM CASON Florida License No. CBC060151

Address: 10 NW 15 ST, ALACHUA COUNTY.

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

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Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1891891	EJ4	9/17/07
2	J1891892	EJ6	9/17/07
3	J1891893	T01	9/17/07
4	J1891894	T01G	9/17/07
5	J1891895	T02	9/17/07
6	J1891896	T02G	9/17/07
7	J1891897	T04G	9/17/07
8	J1891898	T05G	9/17/07

CASON / PLAN 9916 Truss Type Qty Ply Job Truss J1891891 **GABLE** L251203 EJ4 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 17 11:22:25 2007 Page 1 Builders FirstSource, Lake City, FI 32055

4-4-0 -1-4-0 1-4-0 4-4-0 3 Scale = 1:9.8 4.00 12 2 0-3-1 4-4-0 3x6 H 4-4-0

LOADING TCLL TCDL BCLL	G (psf) 20.0 7.0 10.0	Plat Lun	ACING tes Increase ober Increase o Stress Incr	2-0-0 1.25 1.25 NO	CSI TC BC WB	0.14 0.17 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.04 -0.02 0.00	(loc) 2-6 2-6	l/defi >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	1	le FBC2004/TI	–	(Matr		11012(12)	0.00		11/4	100	Weight: 17 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-4-0

oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=258/0-4-0, 6=135/0-4-0

Max Horz 2=94(load case 4)

Max Uplift 2=-232(load case 4), 6=-130(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4/26, 2-3=-48/27, 3-4=-2/1, 3-6=-113/145

BOT CHORD 2-6=0/0, 5-6=0/0

JOINT STRESS INDEX

2 = 0.11, 3 = 0.08 and 6 = 0.05

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2 and 130 lb uplift at joint 6.

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Continued on page 2 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of besign parameters and proper incorporation of component that is instated and reduced verucally and nationated with MITER Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916	
L251203	EJ4	GABLE	1	1		J1891891
L231200	L.04	S/ WEE	'	'	Job Reference (optional)	

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NOTES

- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) Gable truss supports 12" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

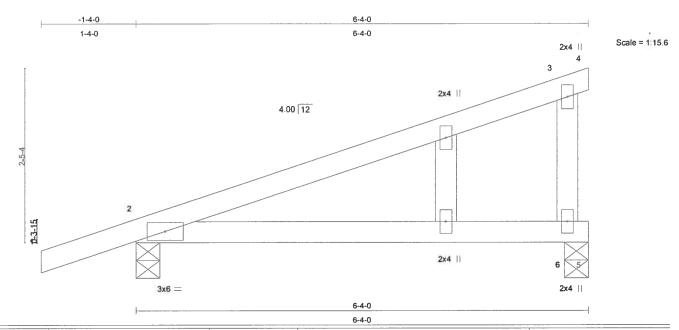
Vert: 1-3=-64(F=-10), 3-4=-24(F=-10), 2-5=-10

Junton Leer Truck (Operon Endencer Plands Pie No. 3-1800) 1100 Chastal Pay Nort Boymon (Joseph - L. 22-126



Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916	
L251203	EJ6	GABLE	2	1		J1891892
L231203	200	O/NOEE	_	'	Job Reference (optional)	

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LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.51	Vert(LL)	0.19	2-6	>362	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.11	2-6	>648	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Matr	rix)						Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

2 X 4 SYP No.3 **OTHERS**

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=325/0-4-0, 6=215/0-4-0

Max Horz 2=125(load case 4)

Max Uplift 2=-286(load case 4), 6=-207(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-4/26, 2-3=-74/45, 3-4=-2/1

BOT CHORD WEBS

2-6=0/0, 5-6=0/0 3-6=-183/233

JOINT STRESS INDEX

2 = 0.13, 3 = 0.13, 6 = 0.13, 7 = 0.00 and 8 = 0.00

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

September 17,2007



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Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916 J1891892
L251203	EJ6	GABLE	2	1	Job Reference (optional)

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2 and 207 lb uplift at
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) Gable truss supports 12" max. rake gable overhang.

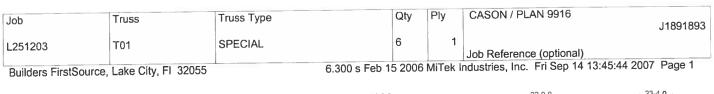
LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-64(F=-10), 3-4=-24(F=-10), 2-5=-10





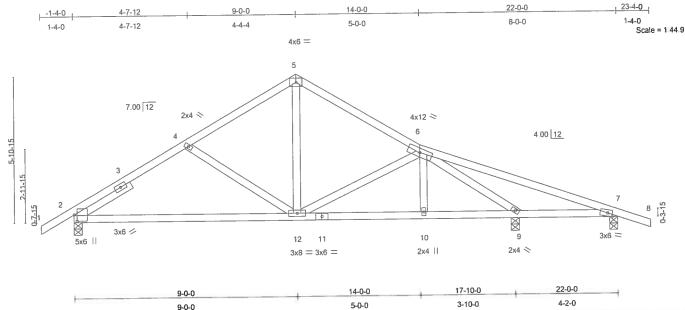


Plate Of	fsets (X,Y): [2:0-3-2,0-1-6], [7:	0-2-4,Edg	e]		,					
LOADIN TCLL TCDL BCLL BCDL	20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mat	0.42 0.39 0.37 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.11 -0.19 0.02	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 112 lb	GRIP 244/190

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

Left 2 X 4 SYP No.3 2-7-8 **SLIDER**

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 6-0-0 oc bracing: 7-9.

REACTIONS

(lb/size) 2=652/0-4-0, 7=216/0-4-0, 9=683/0-4-0

Max Horz 2=-160(load case 4)

Max Uplift 2=-181(load case 6), 7=-211(load case 5), 9=-180(load case 7) Max Grav 2=652(load case 1), 7=226(load case 11), 9=683(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-828/389, 3-4=-722/407, 4-5=-621/344, 5-6=-643/352, 6-7=-25/155, TOP CHORD

7-8=0/22

2-12=-205/652, 11-12=-289/743, 10-11=-289/743, 9-10=-286/744, 7-9=-83/115 **BOT CHORD**

4-12=-222/187, 5-12=-192/346, 6-12=-325/272, 6-10=0/91, 6-9=-991/481 **WEBS**

JOINT STRESS INDEX

2 = 0.84, 2 = 0.31, 3 = 0.00, 4 = 0.33, 5 = 0.28, 6 = 0.75, 7 = 0.82, 9 = 0.53, 10 = 0.33, 11 = 0.30 and 12 = 0.56

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for Committeeners Basified.

September 17,2007

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Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916
			_		J1891893
L251203	T01	SPECIAL	6	1	
					Job Reference (optional)

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NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2, 211 lb uplift at joint 7 and 180 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Less Truss Coston Chomper Plurida Pia Ido Sissell 1400 Sinsalai Ray Rivel Beynton Beach, FL 10406



Job	Truss	Truss Typ	е	Qty	Ply	CAS	ON / PLA	N 9916	J1	1891894
.251203	T01G	GABLE		1	1					
				1	3 A 4777 - 1.	Job R	Reference	(optional)) 12:45:46 2007 P	age 1
Builders FirstSc	ource, Lake City, FI 32	2055	6.300 s F	eb 15 2006	мпек	Industr	ies, inc. i	·n sep 14	13:45:46 2007 P	age i
		9-0-0	9				18-0-0		19-4	-0
1-4-0		9-0-0					9-0-0		1-4- Sc	-0 cale = 1 35.5
5.65.14	3x6 = 4 $3x6 = 3$	00 12	6		8		9	10	3x6 \\ 11 3x6 \\ 22 12	<u> 51</u>
21-7-20	3x6 20	19	18 17 18-0 18-0 18-0		16	15		14	3x6	13 2-7-0
	10.0.0.40.0.0	01 140.0 2 42								
BCLL 10	sf) SPACING .0 Plates Increa .0 Lumber Incre	ease 1.25 ncr YES	CSI	DEFL /ert(LL) /ert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 13 13 12	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 101 lb	GRIP 244/19
LUMBER	2 X 4 SYP No.2 2 X 4 SYP No.2			BRACING OP CHOI	RD RD	6-0-0	oc purlins		ng directly applied	

19=122/18-0-0, 20=159/18-0-0, 16=129/18-0-0, 15=122/18-0-0,

14=159/18-0-0

Max Horz 2=-185(load case 4)

Max Uplift 2=-89(load case 6), 12=-108(load case 7), 18=-91(load case 6),

19=-96(load case 6), 20=-106(load case 6), 16=-89(load case 7),

15=-97(load case 7), 14=-106(load case 7)

Max Grav 2=183(load case 1), 12=183(load case 1), 17=110(load case 1),

18=134(load case 10), 19=122(load case 1), 20=159(load case 10),

16=134(load case 11), 15=122(load case 1), 14=159(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-146/113, 3-4=-136/122, 4-5=-92/108, 5-6=-52/119, 6-7=-45/158, TOP CHORD 7-8=-45/153, 8-9=-45/98, 9-10=-46/39, 10-11=-59/45, 11-12=-69/37, 12-13=0/22

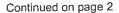
2-20=-26/116, 19-20=-26/116, 18-19=-26/116, 17-18=-26/116, 16-17=-26/116,

BOT CHORD 15-16=-26/116, 14-15=-26/116, 12-14=-26/116

7-17=-90/4, 6-18=-113/103, 5-19=-104/107, 4-20=-132/122, 8-16=-113/101, **WEBS**

9-15=-104/107, 10-14=-132/122

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Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916	
						J1891894
L251203	T01G	GABLE	1	1		
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Sep 14 13:45:46 2007 Page 2

JOINT STRESS INDEX

2 = 0.75, 3 = 0.00, 3 = 0.15, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.26, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.00, 11 = 0.15, 11 = 0.15, 12 = 0.75, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

NOTES

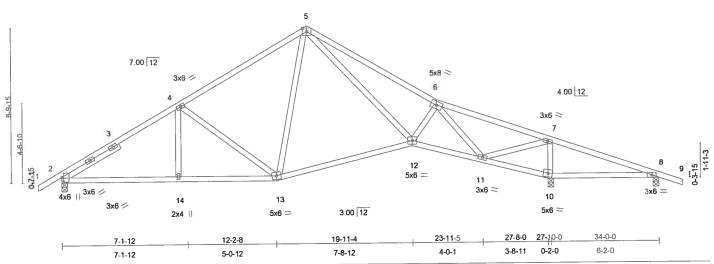
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2, 108 lb uplift at joint 12, 91 lb uplift at joint 18, 96 lb uplift at joint 19, 106 lb uplift at joint 20, 89 lb uplift at joint 16, 97 lb uplift at joint 15 and 106 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truss Coston Engineer Hibrids ME No. 3-1990 Hibrids Meson, H. Sivrbs Bownion Beson, H. Sivrbs



Job	Truss	Truss Type		Qty	Ply	CASON / PLAN 9916		J1891895
_251203	T02	SPECIAL		13	1	Job Reference (option		
Builders FirstSo	ource, Lake City, FI	32055	6.300 s Feb 1	5 2006	MiTek I	Industries, Inc. Fri Sep	14 13:45:47 2007	Page 1
-1-4-0	6-7-9	14-0-0	21-4-0			27-8-0	34-0-0	35-4-0
1-4-0	6-7-9	7-4-7	7-4-0			6-4-0	6-4-0	1-4-0 Scale = 1:63
		,	5x6 =					



TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES Pl2002	CSI TC BC WB (Mat	0.43 0.36 0.44 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)		4 7	l/defl >922 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 178 lb	GRIP 244/190
---------------------------------------	---	--	-------------------------------	------------------------------	--	--	-----	-------------------------------	--------------------------	----------------------------------	---------------------

LUMBER

TOP CHORD 2 X 4 SYP No.2

Plata Officate (Y V): [2:0-3-2 0-1-2]

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.3 3-9-4

Len 2 X 4 3 11 140.0 0-5 4

BRACING

TOP CHORD S

Structural wood sheathing directly applied or

5-1-4 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS

(lb/size) 2=882/0-4-0, 10=1494/0-4-0, 8=-57/0-4-0

Max Horz 2=-243(load case 4)

Max Uplift 2=-234(load case 6), 10=-392(load case 7), 8=-226(load case 5) Max Grav 2=882(load case 1), 10=1494(load case 1), 8=11(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-3

1-2=0/22, 2-3=-1225/549, 3-4=-1073/568, 4-5=-884/502, 5-6=-1350/680,

6-7=-452/222, 7-8=-356/961, 8-9=0/22

BOT CHORD

2-14=-311/977, 13-14=-311/977, 12-13=-68/684, 11-12=-330/1114, 10-11=-973/476

, 8-10=-852/408

WEBS

4-14=0/167, 4-13=-397/284, 5-13=-116/248, 5-12=-263/641, 6-12=-70/176,

6-11=-1128/500, 7-11=-478/1366, 7-10=-1173/606

Julium Less Trups Casagn Chamast Florida pti No. 3-1888 1166 Casasial Bay Missi Geynton Geson, 1-50-165

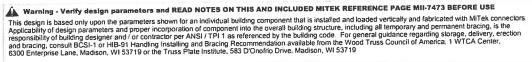
JOINT STRESS INDEX

2 = 0.90, 2 = 0.22, 2 = 0.22, 3 = 0.00, 4 = 0.40, 5 = 0.67, 6 = 0.61, 7 = 0.70, 8 = 0.27, 10 = 0.61, 11 = 0.71, 12 = 0.73, 13 = 0.66 and 14 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2





Job	Truss	Truss Type	Qty Ply		CASON / PLAN 9916 J1891	895
L251203	T02	SPECIAL	13	1	Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 2, 392 lb uplift at joint 10 and 226 lb uplift at joint 8.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty Ply		CASON / PLAN 9916
					J1891896
L251203	T02G	GABLE	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Sep 14 13:45:48 2007 Page 1

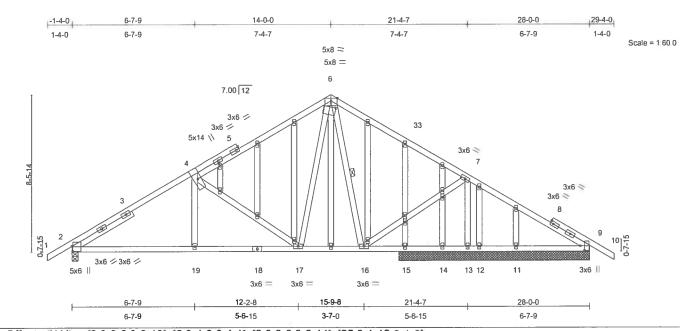


Plate Offsets (X,Y): [2:0-3-6,0-0-10], [6:0-4-0,0-4-1], [9:0-3-0,0-2-14], [25:0-1-12,0-1-0] LOADING (psf) **GRIP SPACING** 2-0-0 CSI **DEFL** L/d **PLATES** I/defl in (loc) TC 244/190 TCLL 20.0 Plates Increase 1.25 0.69 Vert(LL) -0.04 17-19 >999 360 MT20 **TCDL** 7.0 Lumber Increase 1.25 BC 0.32 Vert(TL) -0.08 17-19 >999 240 **BCLL** 10.0 Rep Stress Incr NO **WB** 0.39 Horz(TL) 0.02 15 n/a n/a Code FBC2004/TPI2002 Weight: 225 lb **BCDL** 5.0 (Matrix)

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3

SLIDER Left 2 X 4 SYP No.3 3-9-4

BRACING

TOP CHORD Structural wood sheathing directly applied or

5-7-15 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS 1 Row at midpt

6-16

REACTIONS (lb/size) 9=121/10-4-0, 2=821/0-4-0, 13=1225/10-4-0, 15=118/10-4-0, 14=-54/10-4-0, 12=-58/10-4-0, 11=93/10-4-0

Max Horz 2=286(load case 5)

Max Uplift 9=-141(load case 7), 2=-389(load case 6), 13=-511(load case 6),

15=-29(load case 6), 14=-54(load case 1), 12=-58(load case 1),

11=-30(load case 7)

Max Grav 9=183(load case 11), 2=821(load case 1), 13=1225(load case 1),

15=125(load case 2), 14=42(load case 6), 12=52(load case 7), 11=126(load

case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-3=-1147/509, 3-4=-1000/530, 4-5=-766/424, 5-6=-666/423,

6-33=-357/333, 7-33=-583/347, 7-8=-82/294, 8-9=-96/252, 9-10=0/22

BOT CHORD 2-19=-393/929, 18-19=-394/929, 17-18=-394/929, 16-17=-109/504, 15-16=-222/166

. 14-15=-222/166, 13-14=-222/166, 12-13=-222/166, 11-12=-222/166,

9-11=-222/166

4-19=0/181, 4-17=-466/341, 6-17=-184/348, 6-16=-447/173, 7-16=-225/702,

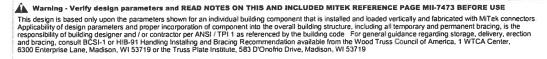
7-13=-1151/588

Julium Lee Fruge (1994) on Chamesr Flance, Mis Flan Selbett 1400 (Imasia) May (North Boynton Wasson, 12 2041)

September 17,2007

Continued on page 2

WEBS





Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916
1.254202	T02G	GABLE	4	4	J1891896
L251203	102G	GABLE	'	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Sep 14 13:45:48 2007 Page 2

JOINT STRESS INDEX

2 = 0.76, 2 = 0.21, 2 = 0.21, 3 = 0.00, 4 = 0.82, 5 = 0.00, 5 = 0.40, 5 = 0.40, 6 = 0.63, 6 = 0.22, 7 = 0.46, 8 = 0.00, 8 = 0.23, 8 = 0.20, 8 == 0.23, 9 = 0.80, 11 = 0.33, 12 = 0.33, 13 = 0.41, 14 = 0.33, 15 = 0.33, 16 = 0.40, 16 = 0.33, 17 = 0.36, 17 = 0.33, 18 = 0.33, 18 = 0.3319 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.00, 25 = 0.39, 26 = 0.33, 27 = 0.33, 280.33, 29 = 0.33, 30 = 0.33, 30 = 0.33, 31 = 0.33 and 32 = 0.33

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 9, 389 lb uplift at joint 2, 511 lb uplift at joint 13, 29 lb uplift at joint 15, 54 lb uplift at joint 14, 58 lb uplift at joint 12 and 30 lb uplift at joint 11.
- 9) Gable truss supports 18" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-6=-91, 6-33=-91, 10-33=-54, 2-9=-10

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Job	Truss	Truss Type	Truss Type Qty Ply		CASON / PLAN 9916
					J1891897
L251203	T04G	GABLE	1	1	
					Job Reference (optional)

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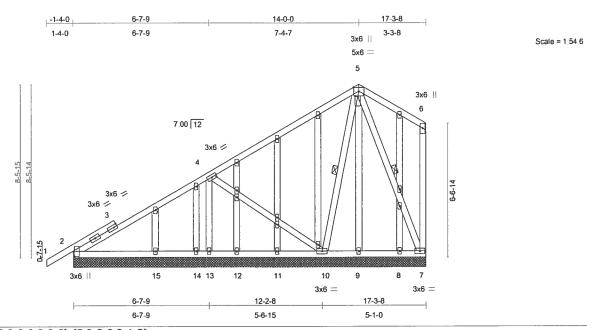


Plate Of	fsets (X,Y	'): [2:0-3-0,0-0-6], [5:	0-0-8,0-1-	8]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	0.01	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 164 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood shear	thing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins, exc	ept end verticals.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.	
		WEBS	1 Row at midpt	5-10, 5-7

REACTIONS (lb/size) 2=262/17-3-8, 6=20/17-3-8, 13=369/17-3-8, 10=251/17-3-8, 7=141/17-3-8, 9=17/17-3-8, 11=23/17-3-8, 12=14/17-3-8, 14=-44/17-3-8, 15=93/17-3-8, 8=23/17-3-8

Max Horz 2=346(load case 6)

Max Uplift 2=-80(load case 6), 6=-71(load case 4), 13=-288(load case 6), 10=-168(load case 6), 7=-109(load case 6), 14=-44(load case 1), 15=-30(load case 6), 8=-1(load case 5)

Max Grav 2=262(load case 1), 6=72(load case 11), 13=369(load case 1), 10=251(load case 1), 7=141(load case 1), 9=53(load case 2), 11=69(load case 2), 12=50(load case 2), 14=47(load case 6), 15=128(load case 2), 8=56(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-3=-164/46, 3-4=-123/65, 4-5=-120/89, 5-6=-39/77, 6-7=0/0

BOT CHORD 2-15=-119/71, 14-15=-119/71, 13-14=-119/71, 12-13=-119/71, 11-12=-119/71,

10-11=-119/71, 9-10=-42/52, 8-9=-42/52, 7-8=-42/52

WEBS 4-13=-331/270, 4-10=-75/121, 5-10=-195/114, 5-7=-151/127 s. Lagre g (142) 영(2) 45 (175) (175) Ser Lagrer (175) 15 (175) (175) 4 (175) 15 (175) 15 (175) 15 (175) 15 (175) 15 (175) 15 (175) 15 (175)

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	CASON / PLAN 9916
L251203	T04G	GABLE	1	1	J1891897
L251205	1040	OABEL	'	'	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Sep 14 13:45:49 2007 Page 2

JOINT STRESS INDEX

2 = 0.88, 3 = 0.00, 3 = 0.23, 3 = 0.23, 4 = 0.40, 5 = 0.67, 5 = 0.38, 6 = 0.15, 7 = 0.48, 8 = 0.33, 9 = 0.33, 10 = 0.36, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 18 = 0.33, 19 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33 and 24 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 2, 71 lb uplift at joint 6, 288 lb uplift at joint 13, 168 lb uplift at joint 10, 109 lb uplift at joint 7, 44 lb uplift at joint 14, 30 lb uplift at joint 15 and 1 lb uplift at joint 8.

LOAD CASE(S) Standard

Simble Coefficient (Indender Planks (Design Challes) Planks Parks Toelle Parks (Design Challes) Parks (Design Challes)



Job	Truss	Truss Type		Ply	CASON / PLAN 9916
1 251202	T05G	GABLE	4	1	J1891898
L251203	105G	GABLE	'	'	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Sep 14 13:45:50 2007 Page 1

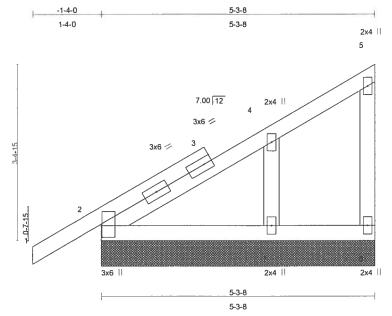


Plate Of	fsets (X,Y): [2:0-2-12,0-0-2]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber increase	1.25	BC	0.06	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 30 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-3-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=180/5-3-8, 6=37/5-3-8, 7=185/5-3-8

Max Horz 2=167(load case 6)

Max Uplift 2=-82(load case 6), 6=-32(load case 6), 7=-125(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/22, 2-3=-122/11, 3-4=-113/42, 4-5=-34/12, 5-6=-34/38

BOT CHORD

2-7=0/0, 6-7=0/0

WEBS

4-7=-150/161

JOINT STRESS INDEX

2 = 0.63, 3 = 0.00, 3 = 0.15, 3 = 0.15, 4 = 0.08, 5 = 0.02, 6 = 0.02 and 7 = 0.09

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Continued on page 2

Truss Cosign Engineer Historia PR No. 34888 1466 Chastal May Rivel Woyned Leach, FL 20495

September 17,2007

Scale = 1 21.5

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty Ply		CASON / PLAN 9916	
L251203	T05G	GABLE	1	1	J1891898	3
2201200	1000	0,1322	ľ		Job Reference (optional)	

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NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 2, 32 lb uplift at joint 6 and 125 lb uplift at joint 7.

LOAD CASE(S) Standard

Julium Emm Prum (Demign Engirmer Flands Michael Mey Mori 1100 Chammin Mey Mori Lacynton Wesen, 1-2020

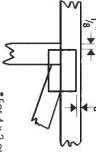


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless Dimensions are in inches. Apply plates to both sides of truss and dimensions indicate otherwise securely seat



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



required direction of slots in This symbol indicates the connector plates.

PLATE SIZE

4 × 4

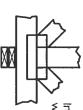
perpendicular to slots. Second to slots dimension is the length parallel The first dimension is the width

LATERAL BRACING



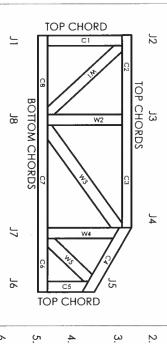
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

IC_BO

BOCA

96-31, 96-67

3907, 4922

SBCCI

WISC/DILHR 960022-W, 970036-N 9667, 9432A

NER R

561



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- ٥.

Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- 7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- œ shown indicate minimum plating requirements Plate type, size and location dimensions
- % Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- 10. Top chords must be sheathed or purlins provided at spacing shown on design
- Bottom chords require lateral bracing at 10 unless otherwise noted tt. spacing, or less, it no ceiling is installed
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- 13. Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or alter truss member or plate engineer. without prior approval of a professional
- 15. Care should be exercised in handling erection and installation of trusses
- © 1993 MiTek® Holdings, Inc.

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHICH DEGONAL HRACE IS USED, CONNUCT HRACENAL BRACE FOR SAGE AT EACH END. MAX WEB VERTICAL TOTAL LIZYCTH IS 14". MAX **GABLE** LENGTH DI TABLE ABOVE. SPACING SPECIES 12" 16 24" O.C. O.C. O.C. MILLIAGONAL AT CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF SPF DFL DFL S S SP HH AS S HH)FL ASCE NAOHB GRADE STANDARD STANDARD STANDARD STANDARD STANDARD 13 to 12 STUD **STUD** HARW 7-02: BRACE ***WARDIGO*** TRUSSES REQUIRE EXTREME CARE IN FABRICATINE, MAIDLING, SAIPPING, INSTALLING AND BRADING, REFER TO JESS 1-60 (BUILDING COPPIDENT SAFETY PATEMATION, PUBLISED BY TPJ (TRUSS PLATE (NSTITUL), 593 INDIGETED BY, SAITE 200, MAIDSIN, V.J. SATIS) AND VITA, VIZID TRUSS CILIAGA. OF AMERICA, 6300 ENTERPRISE LM, MADISIN, V.J. SATIS) FOR SAFETY PACTICES PRIZE TO PETERBURG THESE TURCTIONS. UNICES OFFEREYES INDIGETED CARE TO FOREIGN ATTACHED STRUCTURAL PARELS AND JUTTUM CHERO SMALL HAVE A PROPERLY ATTACHED RIGID CELLING. SOURT TRUBS 3 3 3 S NO BRACES 130 ZX4 EF #ZW, DT-L #Z, SPF #L/#Z, DR BETTER DIAGONAL BRACT; ENVICES OR DOUBLE CUT (AS SEEDWN) AT GROUP A (1) 1X4 "L" BRACE • N 4 8 8 B' 0" MPH UPPER RND. GROUP B 6. 0. 10. 4, 5, 3, 0, WIND GROUP A (1) 2X4 "L" BRACE . 6. 10<u>-</u> 5' B" SPEED, GROUP B 40 Ø. REFER TO 15 PUTAS 18 - 6 (2) 2X4 "L" BRACE ** GROUP MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH EXA #EN OR BRITISH CONTINUOUS HEARING GROUP B ඩ ක ස ස ස ක ක ක ස ස HEIGHT, • Ð c CONS. (1) 2X6 "L" BRACE * (2) ZXB "L" HRACE *** GROUP A 1455 GY 481 AVINUB DELRAY BEACH, FL 33444-2161 10' 3" 000000 No: 34889 STATE OF FLORIDA 10' IUS LEI ENCLOSED, GROUP 12 9 ₩ PET S GROUP A ์ מל Н MAX. MAX. GROUP Ш 13' 3' 12' 11" 14. 0. 14' D" Φ 1.00, TOT. SPACING Ħ Ę ATTACH EACH "L" ERACE WITH 104 NAILS & \$ FOR (1) "L" BRACE; SPACE NAILS AF 2" O.C. \$ FOR (2) "L" BRACES; SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLAT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC BEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MINUSE LENGTH. T. BRACING MUST BE A MINIMUM OF BOX OF WEB DOUGLAS FIR-LARCH 43 9TUD STANDARD SPROCE-PINE-INR #1 / #2 STANDARD #3 STUD PLYWOOD OVERHANG. BRACING GROUP SPECIES VERTINGAL LENGTH LESS THAN 4. D. BUT LIZSB THAN 11. B. GREATER THAN 11. B. EXPOSURE CABLE TRUSS DETAIL NOTES: SOUTHEASEN CO. 60 REFER TO COMMON TRUBS DESIGN FOR PEAK, SPLICE, AND SEEL PLATES. 24.0" GABLE VERTICAL PLATE SIZES PSF N DATE DRWG REF GROUP GROUP HEM-PIR 11 & BIR DOUGLAS FIR-LARCH \Box MILES SID CYBITS 19 E HI SOUTHERN PINE #3 9TUD STANDARD 11/26/03 ASCB7-02-CAB13015 Ä A #3 NO BPLICE HIM-PIR STUD STANDARD 2.5X4 AND GRADES: X

ASCE 7-02: 130 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, I || 1.00, EXPOSURE Ω

BRACING GROUP SPECIES AND GRADES:

GROUP A:

HEM-PIR

#2 STUD

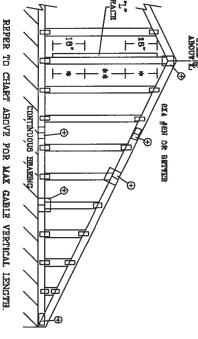
#3 STANDARD

SOUTHERN PINE

#3
STUD

STANDARD

							_			_	_	_							_		_		_	_	_				
]	M	A	X		G	i A	ΔĒ	31	J	3	1	V	E	R	Т	'I	C.	A	L		L	E	N		Γ	Ή	
		1	2	23	-	0	. (ζ.		1.0	1	6	3 3	(0	. (ζ.			2	4	23		O	. (ζ.	,	SPACING	2
		F.	1	(/ <u>)</u>)	TTT	I I	בי בי	משו	1				}	TTT,	5	UK!	משו				\(\frac{1}{2}\))	TTT.	ij	ひてュ	E E	SPECIES GRADE	2X4
	STANDARD	CUIS	‡ 3	#2	#1	STANDARD	STUD	£4	2# / IF	STANDARD	CUTE	E.	2#	41	STANDARD	CUTS	靐	2# / L ‡	STANDARD	CUIS	E#	2#	1.4	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 0"	4. 2,	4' 2"	4' 4"	4' 5"	3' 11"		3' 11"	٠.	3' B"	3'8"	3' 8"	3' 11"	4' 0"	8. %	3' 7"	3' 7"	3' 8"	3' 0"	3' 3"		-	3 6"	2' 11"	3' 1"	3' 1"	ın' αi	BRACES	ž
	5 6	6 4	6 6	6' 11"	B' 11°	- 1	B' 3"	B 3*	6' 11"	4' 9°	5' 6"	B' 7°	B' 4"	B' 4"		5, 6,	5, 5,	6' 4"	3' 10"	4' 8"			5, 8,		4' 6"	4' 5"	5. 6.	GROUP A	(1) 1X4 °
	5, 8,	6' 4"	6' 5"	7' B"	7, 8,	Б' 4°	6. 3.	8 3*	ે. ક.		5 B"	6. 7.	6' 10"		4. B.	6, 6,		6. 6.	3' 10"	4' B"	4' 6"	5' 11"	5' 11"		4' 5"		6° B°	GROUP B	"L" BRACE •
	7' 3"	1 1	B' 3"	i .	8, 3,		A3 3"	8' 3"	6. 3.	6' 3"	7' 3"	7' 4"	7º 8"		8.8	7' 2"	-3i -2i		6" 1"	5' 11"	6. 0,	6' 6"	6, 13		b' 10*	5' 10"	8° 8°	GROUP	(1) 2X4
TAS .	7' 3"	8' 6"	8' 6"	l	B' 11°	- ۱		8° 3°	8. 8.	8, 3,	7' 3"	7' 4"	B' 1"	В' 1°	8.	2 2	7' 2"	7' 8"		5' 11"	6. 0.	7' 0"	7' 0"	5.0	5' 10"	5' 10"	6' 9"	A GROUP B	"L" BRACE .
30 PUTAS	B. 8	9. 10.	9' 10"	8° 10°	8° 10°		9' 10"	9' 10"		8° 5"	8' 11"	B* 11"	B' 11"	B' 11"	a. 3.	B' 11"	B' 11"	8" 11"	6' 11"	7' 10"	7' 10"	7' 10"	7' 10"		7' 10"	7' 10"	7' 10"	GROUP A	(2) 2X4 "L"
	9, 8,	10' 4"	10' 4"	10' 7"	10' 7"		9' 10"	9' 10"	10. 1.	B' 5"	ר ו	9. 6.	9, 2,	B' 7"	6. 3.		6' 11"	- 1	ł I	8' D"	8' 1"	e' 5°	B) 5	T	7' 10"	7' 10"	8' 0"	GROUP B GROUP	BRACE **
	11' 4"	12' 11"	12' 11°	18' 11"	12' 11"		12" 10"	12' 11"	12' 11"				11' 9"	11' 9"			1 1	11. 9.		8,3		10' 3"	10' 3"	7' 10"	9' 1"	9' 1"	10° 3"	GROUP A	(1) 2X6
	11' 4"	13. 1.	13' 3"	13' 11"	13' 11"	11' 1"	12' 10"	12' 11"	15' 4"	9, 9,	11' 4"	11. 6.	12' B"	12° B"	9. 7.	11, 1,	11' 2"	12' 1"	8, 0,	B' 3*	9. 4.	11' 1"	11' 1"	7' 10"	9' 1"	9' 1"	10′ ፖ		"L" BRACE +
	14' 0"	14. 0	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12. 11.	14' 0"	14' 0"	14. 0-	10' 10"	12' 3"	12. 3°	12' 3"	12' 3"	10' 7"	12' 3"	12' 3"	12' 3"	GROUP B GROUP A GROUP B	(z) ZZB 'L'
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0*	14' 0"	14' 0"	14' 0"	13' 3"	14 0	14' 0"	14' D"	14' D"	18' 11"	14" D"	14' 0"	14' 0"	10' 10"	12' 6"		13' 2"	13' 2"	10' 7"	12' 3"	12' 3"	12' 7"	GROUP B	HRACK
DULIDONERS MILE S. O.		9) DIEMARE STOUNTHOOD	Partition of the partit	TABL NOTACETERS OF THE	Control Anno Co.	CARIE TRIES DI			R	i is	SOUTHING PINE		#1 R L	10:4-3	arcot.	CBOILE			STANDARD	9	DOUGLAS FIR-LARCH		ت ا	JI / LZ STANDARD	Spation Single Circuit	illuan	BRACING GROUP SPEC		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL.
BRACE IS USED. CONNECT
IMAGONAL BRACE FOR SROG
AT RACH WID. MAY WEB

CABIL TEUBS

TOTAL LEXICIH IS 14".

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DT-L #2 CH
REFTEL DIAGONAL
BRACT, SNGAL
CUT (AS SHOWN)
AT UPPER END

CONNECT DIAGONAL AT

EPER	\	
To	/	
EPER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.	\	- * - + * - / \(\rightarrow \)
ABOVE	гуже впослугия	exa ∦en or betwee
FOR	, g	₩ 3
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CABLE	
TRUSS	
DETAIL	
NOTES:	

HEM-PIR H & HIR GROUP B:

PROVIDE UPILIT CONNECTIONS FUR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC BEAD LOAD). ABAL END EUPPORIS LOAD FROM 4' 0" IVE LOAD DEPLECTION CRITERIA IS L/240. PLYMOOD OVERHANG.

ATTACE EACH 'L' ERACE WITH 10d NAILS &

* POR (1) 'L' ERACE; SPACE NAILS AF 2' O.C.

* PUR (2) 'L' ERACES; EFACE NAILS AT 3' O.C.

EN 18' END ZONES AND 6" O.C. ELYMEN ZONES.

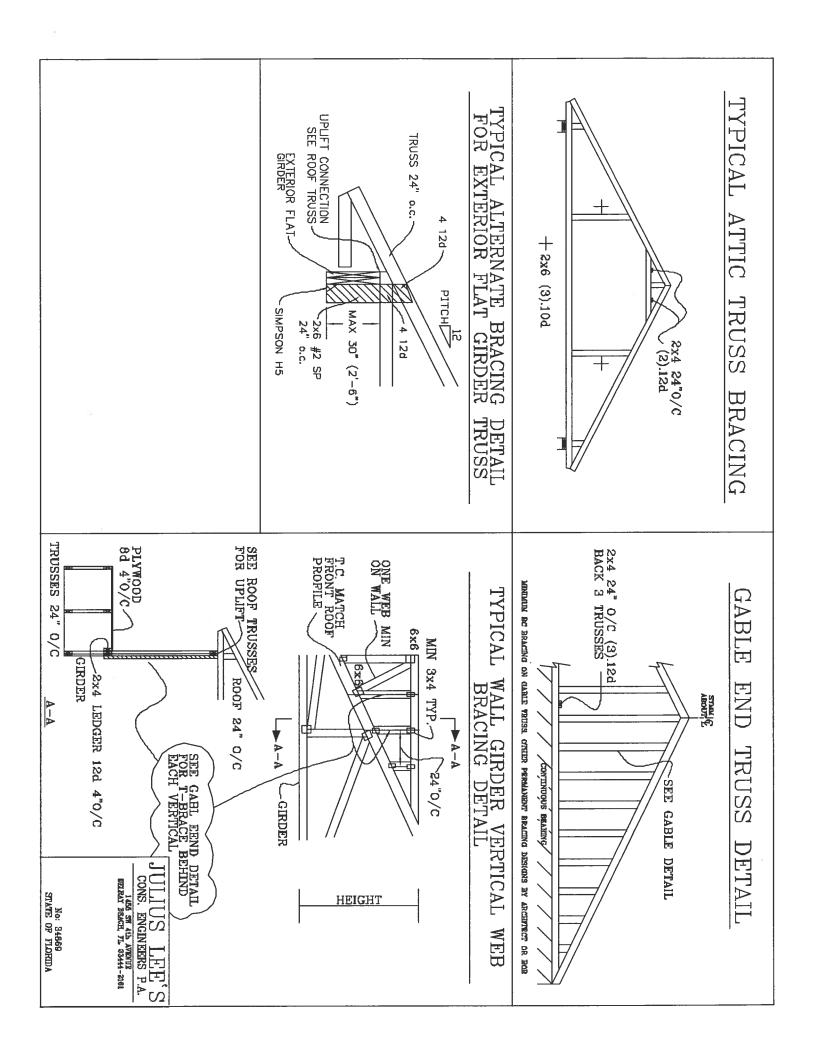
** PUR (2) 'L' ERACES; I. BRACING MUST BE A MINIMUM OF BOX OF WEB

MEMBER LENGTH.

1 171
GABLE VERTICAL PLATE SIZES

		STRUCTURAL PANELS AND BETTEN CHORD SHALL HAVE A PROPERTY ATTACHED ROLD CELLING.	PLATE INSTITUTE, 383 D'INCERCO DR. SUTTE 200, HADISON, VI. 53719) AND VICA (MODO TRUSS COLACIL. OF ALECCIA, 6430 ENTERPRISE, LM. MODON, VI. 53739) FOR SMETT, PRACTICES PRISE TO PROPERTING	ANANAMANIA TRUSSES REBUIRE EXTREME CARE IN FARRICATING, HANDLING, SUPPRIES INSTALLING AND RAYCOM. REFER TO BISS 19-03 COULDING COMPINENT SAFETY (MYDRANTION), PUBLISHED BY TRY CRUSS	
No: 34869 STATE OF FLORIDA			1456 SW 4th AVERUE Delbay binach, Pl. 33444-2161	CONS. ENGINEERS P.A.	STEEL SILLILL
MAX. SPACING 24.0"	MAX. TOT. LD. 60 PSF				
		-ENG	DWG meek sed gable 30' e he	DATE 11/26/09	REF ASCEY-02-GABI3030

	ō	2	HNG	MAX. SPACING 24.0"	NAX.
	PSF	60	Ë	MAX. TOT. LD. 60 PSF	MAX.
-ENG					
DWG Meek sed gable so, e hu					
DATE 11/26/03					
REF ASCET-02-GAB13030					



TOP CHORD CHORD WEBS 2X4 2X4 2X4 \$ 17 to BETTER BETTER BETTER

PIGGYBACK DETAIL

TAKE

SPANS

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REFER TO SEALED DESIGN FOR DASHED PLATES

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER. SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS ö

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY HE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO BUCINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MBAN HGT, ASCE 7-02, CLOSED BIDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DI=5 PSF, WIND BC DI=5 PSF

110 MPH WIND, 50' MEAN HGT, FEC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF

130 MFH WIND, 30' MFAN BGT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TO DI=6 PSF, WIND BC DI=6 PSF CLOSED EXP. C,

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84 **5X4** EXd.

OR SX6 TRULOX AT 4' ROTATED VERTICALLY

50

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989

90

C

1.5X4

1.6X4

1.5X4 ВΧЭ

H >

4X8

5X8

8

9X9

2**X**4

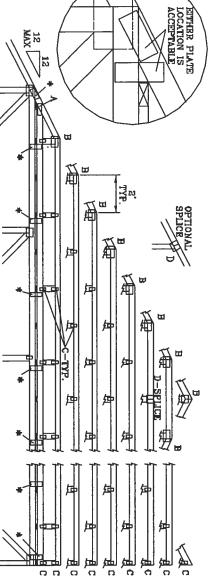
2.5X4

2.6X4

3X6

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. E 占 MAX SIZE OF ZXIZ #2 OR HETTER

20' FLAT TOP CHORD MAX SPAN



-					
	10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEED LENGTH	
	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 18d NAILS AT 4° OC.	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

ATTACH TRULOX PLATES WITH (8) 0.120° X 1.975" EQUAL, PER FACE PER PLY. (4) NAILS IN EACH | BE CONNECTED. REFER TO DRAWING 160 TL FOR INFORMATION.

R TRULOX

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ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS WITHE (4) 0.120 % I 1.375 NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4 OC OR LISS. * PIGGYBACK SPECIAL PLATE

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a Ω n C a ິດ

DRAWING REPLACES DRAWINGS 634,016 634.017 & B47,045

B

*ATTACH

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WAVARHIGM TRAISES REQUIRE CYTROPE EME IN FABRICATING, HANDING, SHIPPING, DRITALLING AND BACHNA SEFER TO EXEL I-DA QUILLING COMPONENT SAFETT INFORMATION, PUBLICADE BY THE CRIMES PLANTE INCTITINE, SEQ CONDICTION DE SUDIE EME, MANUEN, VI. 323759 AND AFDA CHOID TRAISE COUNCIL OF AMERICA, COID CHERPISE IN HANDSON, VI. 33739 FIRE SAFETY FRACTICES PERE TO POPERHOUS THESSES PROGETTO. TOT CHERD SHALL HAVE FOR PEREY ATTACHED STRUCTURAL PANELS AND BOTTOM CHERD SHALL HAVE A PROFERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHERD SHALL HAVE A PROFERLY ATTACHED RIGHD CELLING.

J U LLI

HOVER AVERTO

STATE OF FLORIDA			EMBRICA ATTRIBUTED AND TO THE TAXABLE PARTY.	1466 SW 4th AVENUE	ONS. ENGINEERS P.A.		
SPACING	47 PSF AT 1.15 DUR. FAC	T.ZD DUK. FAC.	50 P	1.33 DC	55 PSF AT	MAX L	4
24.0*	SF AT	JR. PAC.	PSF AT	JR. FAC.	SF AT	MAX LOADING	0 4 1 1 1 1 1 1 1
			-ENG JL	DRWG	DATE	REF	4 10 10 0
			JL.	DRWGMITEK STD PIGGY	DATE 09/12/07	PIGGYBACK	Appears and the state of the st

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS ZX4 SP #3 OR BETTER.

- * ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

 (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

 FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR

 ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED

 BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

LARGER AS REQ'D

4-0-0

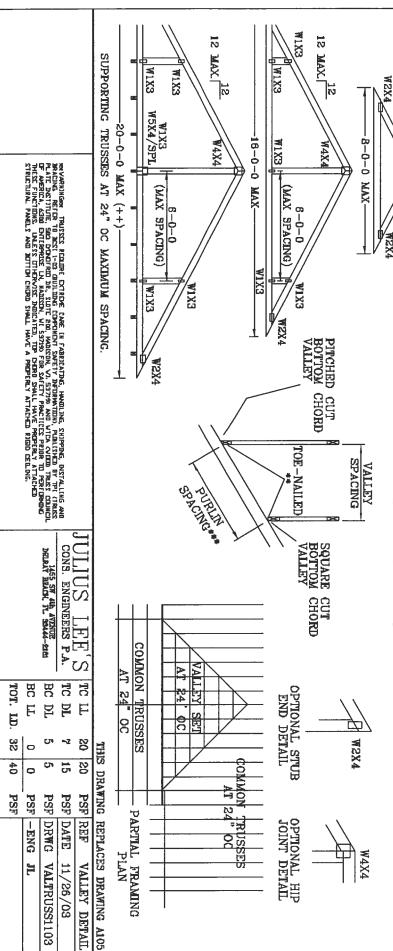
XAM

12 MAX.

W2X4

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



No: 34868 STATE OF FLORIDA

SPACING

1.25

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

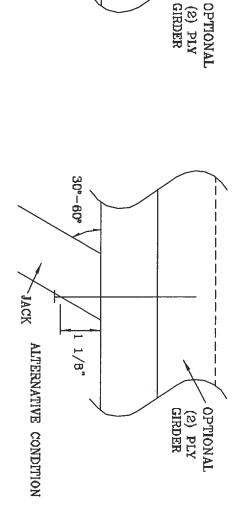
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 PLY	1 PLY 2 PLIES	1 PLY	2 PLIES
ಸ	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALU	ES MAY BE	MULTIPLE	D BY APP	ROPRIATE	DURATION	ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	CTOR.	



1/B"

JACK

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANCES AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHD CELLING	PLATE INSTITUTE, 383 PONCERED DR., SUITE 289, MADISON, ME 33719) AND VICA MODD TRUSS COLNCIL OF AMERICA, 5800 ENTERPRISE LM, MARIEDM, ME 33719) TRE SMETTY PRACTICES PRIDE TO PREPERVING	BRACING. RETER TO BISS 1-03 CHEMING CORPORET IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BISS 1-03 CHEMING CORPORET IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND	
STATE OF FLORIDA	No: 34889		75	SS COLUNCIA DELIZAY BEACH, FL S3444—2161		FET SULTUL
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	A TC DL	S T ST
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG CNTONAIL1103	DATE 09/12/07	REF TOE-NAIL

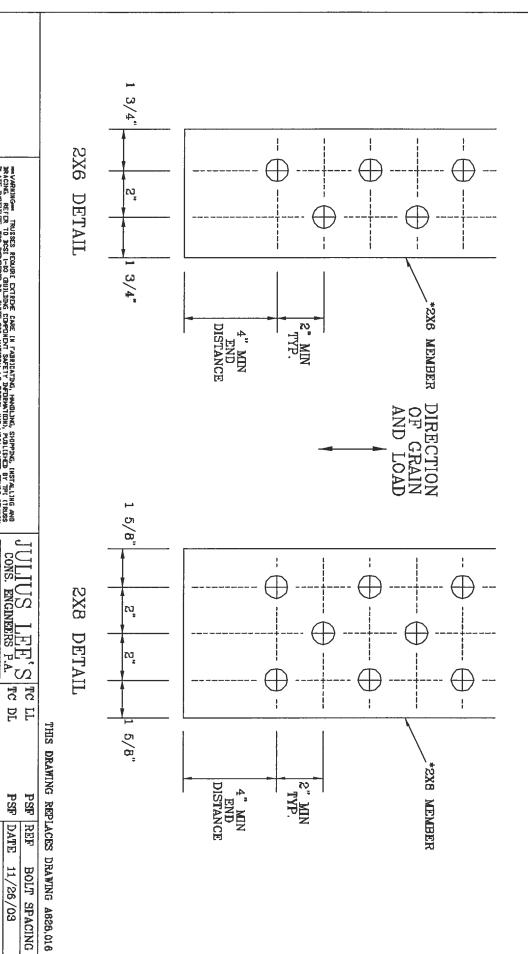
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/S2" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLIS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PAITERNS SHOWN BELOW. BOLT APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



INVERSING TRUSSES REQUIRE EXTREME IN FABRICATING, HANDLING, SHIPPING, INST.

BRAINEN REFER TO DOS! 1-60 GBILLING EXPRINCINT SAFETY EXPERMINDH, PULICIPED BY
PLATE INSTITUTE, 500 CHOUNTED BR, SUITE 28H, MURISON, V. 32759 AND LYLO AVEID TR
FERENCA, GOID ONTERPOISE LM, MAINSON, VI 33739 FIR SAFETY PRACTICES PRIBE TO PR
THESE FINCTIONS UNLESS ONTERPOISE TO EXPENSE AND LYLO EXPENSE AND AND CELLING
THESE FINCTIONS UNLESS ONTERPOISE DESCRIPTION DORD ONTERPOISE AND AND CONTROLLED AND CELLING
STRUCTURAL PARELS AND BUTTON CHURD SHALL HAVE A PROPERLY ATTACHED ROOD CELLING

DELRAY STACE, FL 33444-2161

BC LL BC DL TC

PE

PSF

DATE

TOT. LD

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> -ENG DRWG

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CNB0LTSP1103 11/26/03

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC.

TRULOX CONNECTION DETAIL

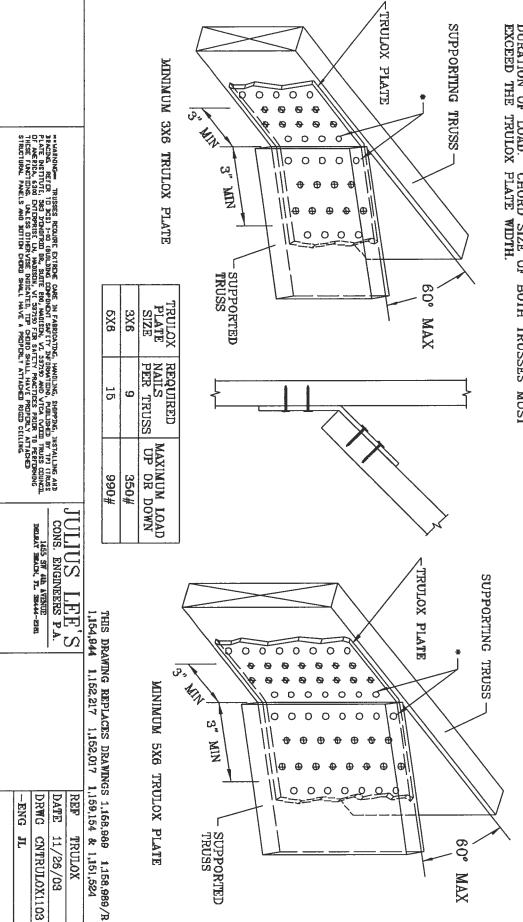
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi \)).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO, PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

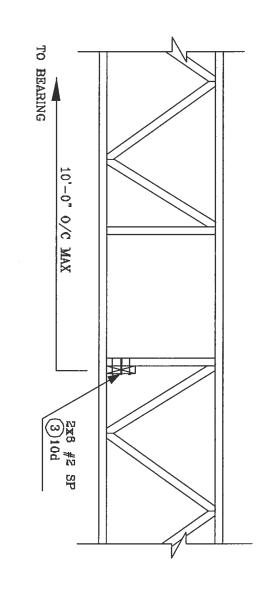
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

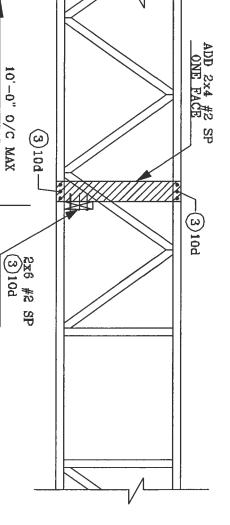


No: 34869 STATE OF PLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. engineers p.a.

No: 34869 STATE OF FLORIDA TO BEARING