

DESCRIPTION

PART OF TAX ID. PARCEL NO. 21-68-17-09100-000.

THAT PART OF THE FOLLOWING DESCRIBED PROPERTY LYING NORTH OF SW MINTER ROAD 4 EAST OF SW CARL WILSON ROAD:

THE SW1/4 OF NW1/4 LESS THE EAST 6.66 ACRES AND THE NW1/4 OF SW1/4 LESS THE EAST 6.66 ACRES, IN SECTION 21, TWP 6 SOUTH, RGE 17 EAST, COLUMBIA CO., FLORIDA.

NOTES:

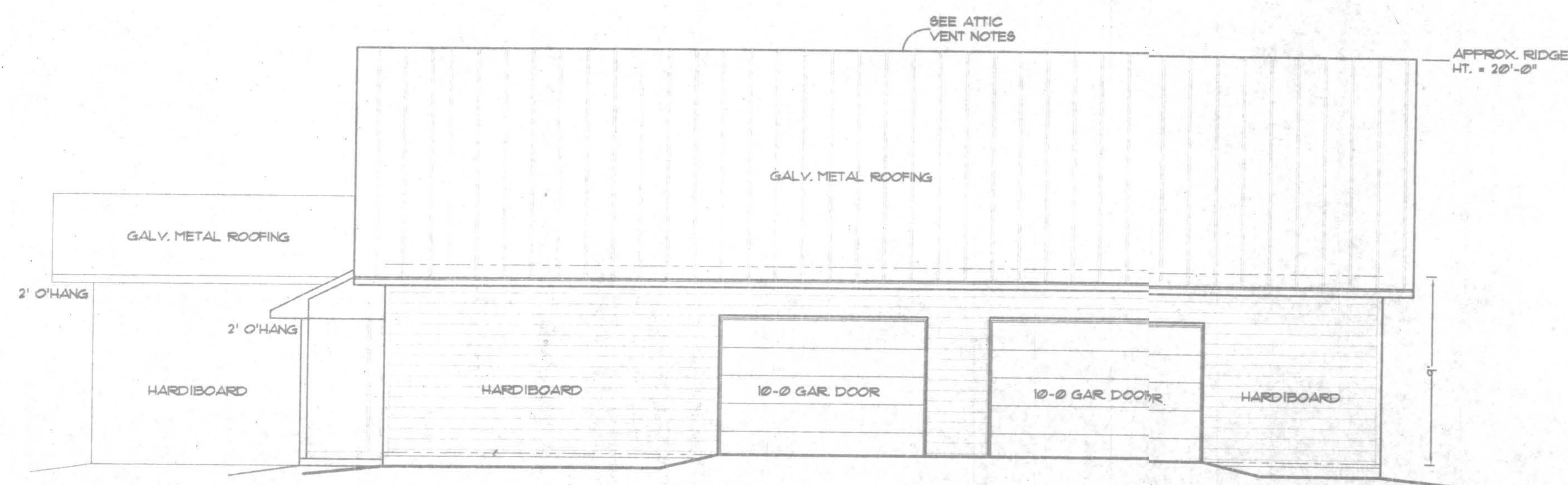
- 1.) BUILDING LOCATION PER OWNER OR CONTRACTOR
- 2.) LOT DIMENSIONS TAKEN FROM DATA FURNISHED BY OWNER
- 3.) BUILDER SHALL VERIFY ALL APPLICABLE SETBACKS, REGULATIONS AND DEED RESTRICTIONS.

SITE PLAN

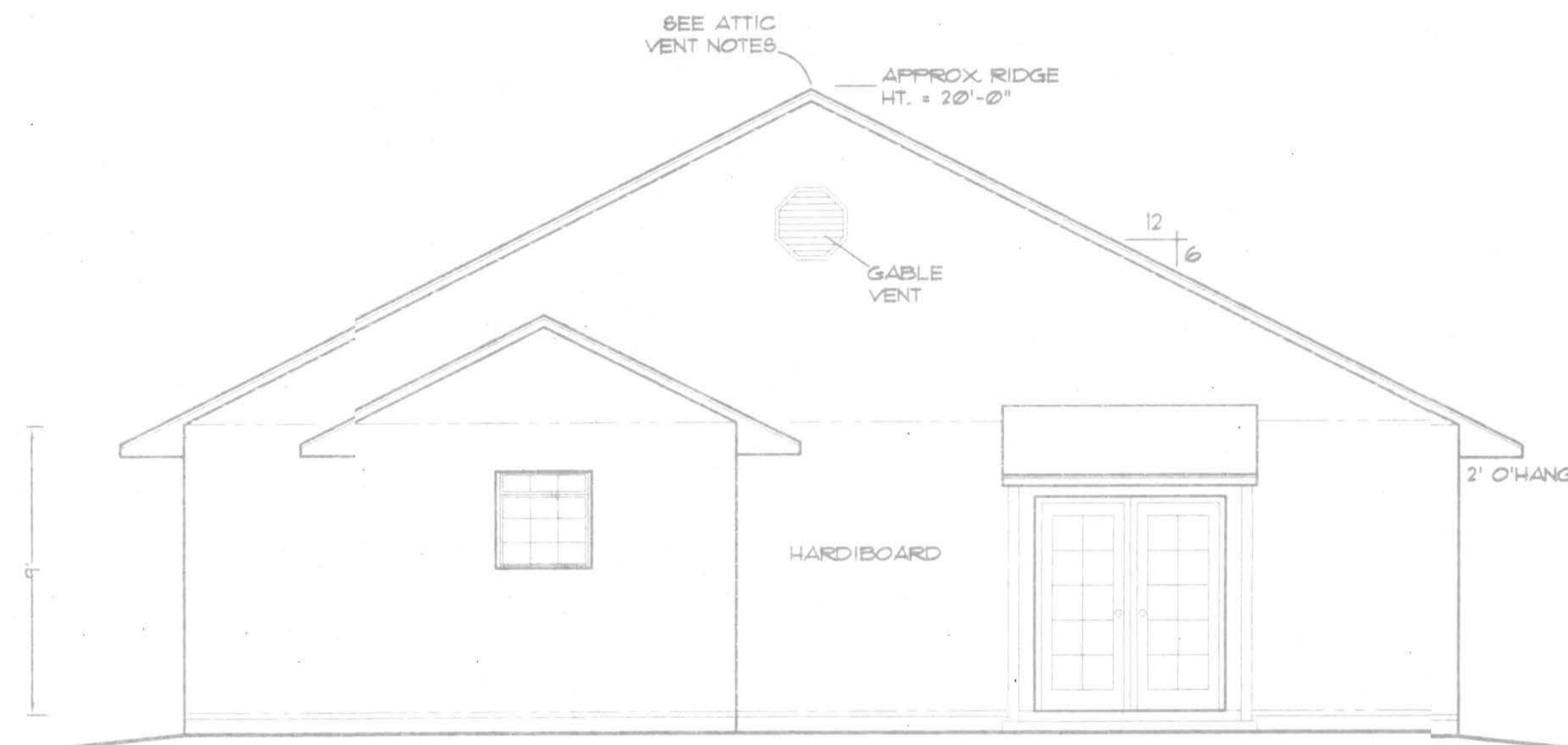
SCALE: 1 IN. = 150 FT.

A-1

FILE: OG-022	DUNHAM-BLACK RESIDENCE	SHEET: 1 OF 4
DATE: G-15-OG		CAD FILE: OGO22
DRAWN: T A D		REV: 9-2-OG
CHECK: T A D		REV:
PREPARED BY: TIM DELBENE Drafting & Technical Services 192 SW Snowbird Ct. Lake City, FL 32624 Phone (386) 755-5891		

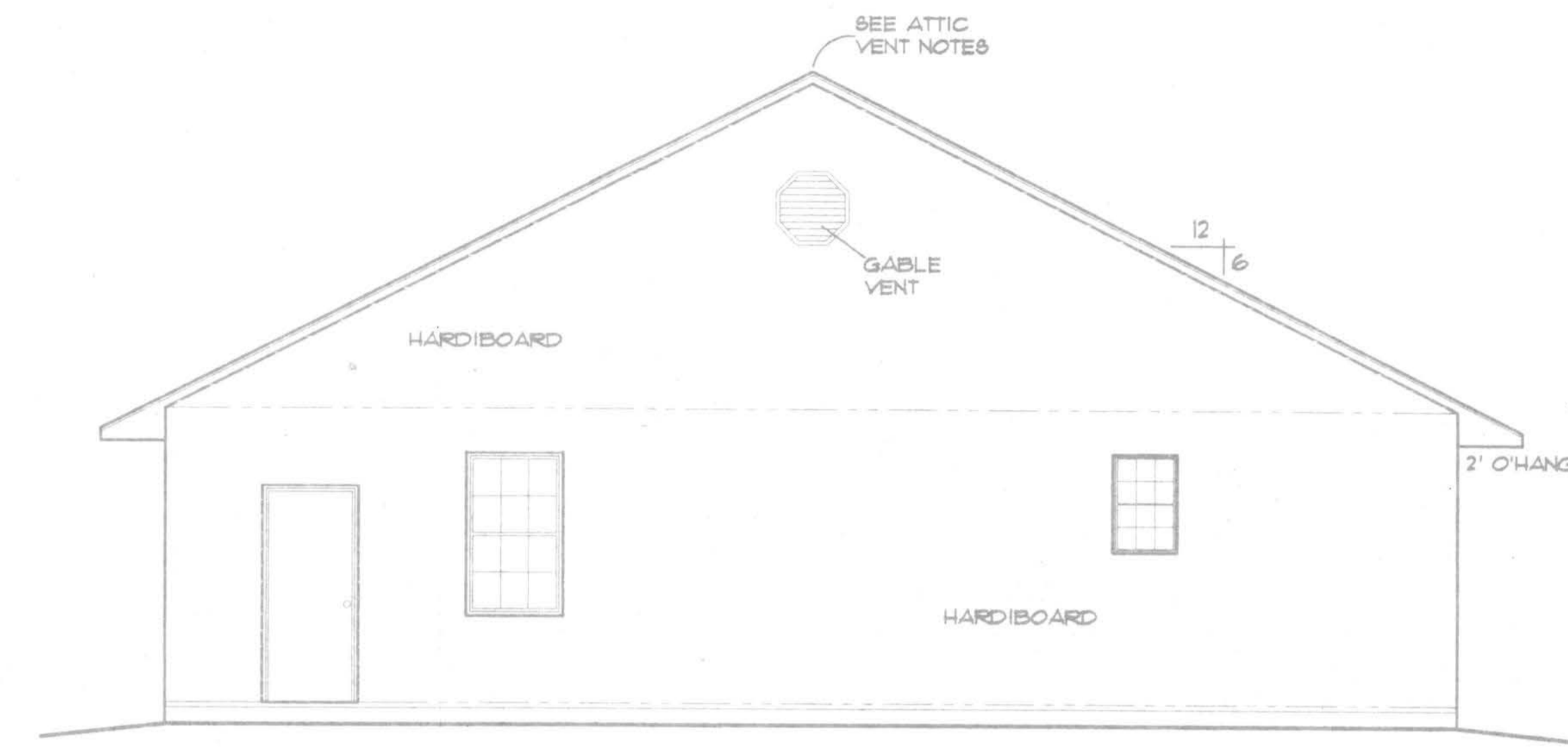


SCALE: 1/4 IN. = 1 FT.



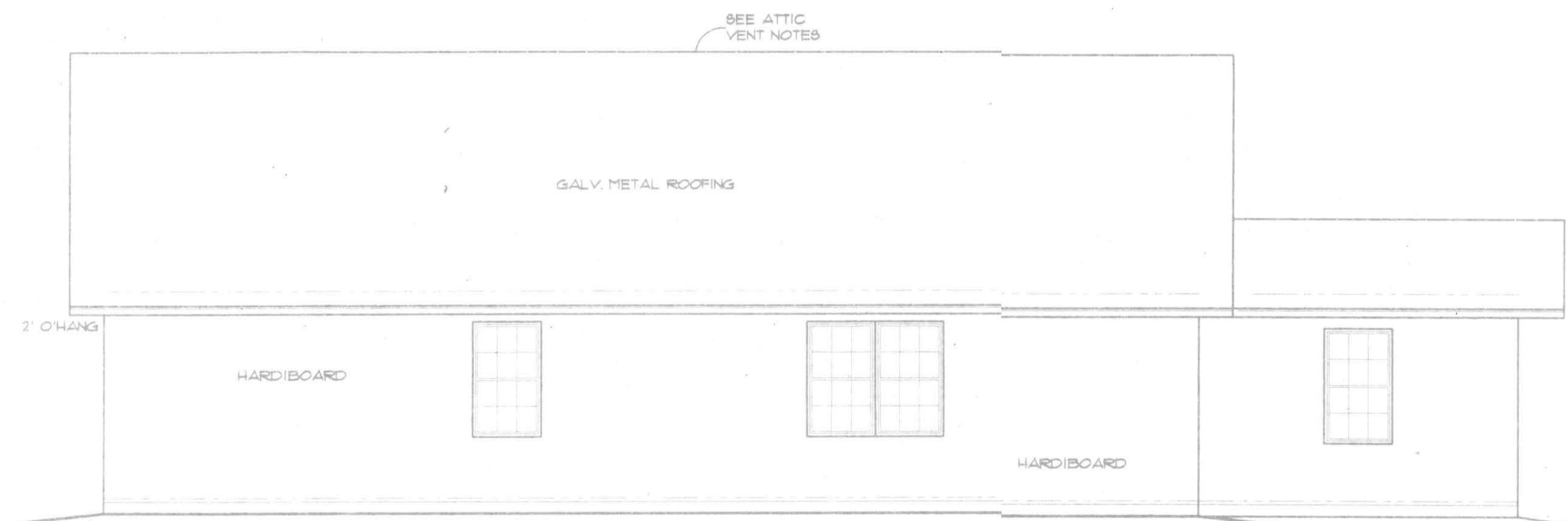
LEFT ELEVATION

SCALE: 1/4" IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4" IN. = 1 FT.



REAR ELEVATION

SCALE: 1/4" IN. = 1 FT.

ATTIC VENTILATION

Enclosed attic and roof space with no eave barge where ceiling are applied direct to the underside of roof rafters and barge barge ventilation through separate ridge cap venting. Ridge caps protected against the entrance of rain, venting to be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center. Ridge cap venting to be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center. Ridge cap venting to be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center.

The ridge cap venting shall be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center. Ridge cap venting to be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center. Ridge cap venting to be made of 1/2 inch plywood with 1/2 inch square holes spaced 12 inches on center.

GENERAL NOTES

1. See "Wind Load Detail Sheet S-1" and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
2. All concrete used to be 2500 PSI strength or greater.
3. HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
4. Windows to be aluminum framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
5. Roof Truss design is the responsibility of the supplier.
6. The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Order locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 200 lbs for End Modification.
7. Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
8. Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

A-2

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

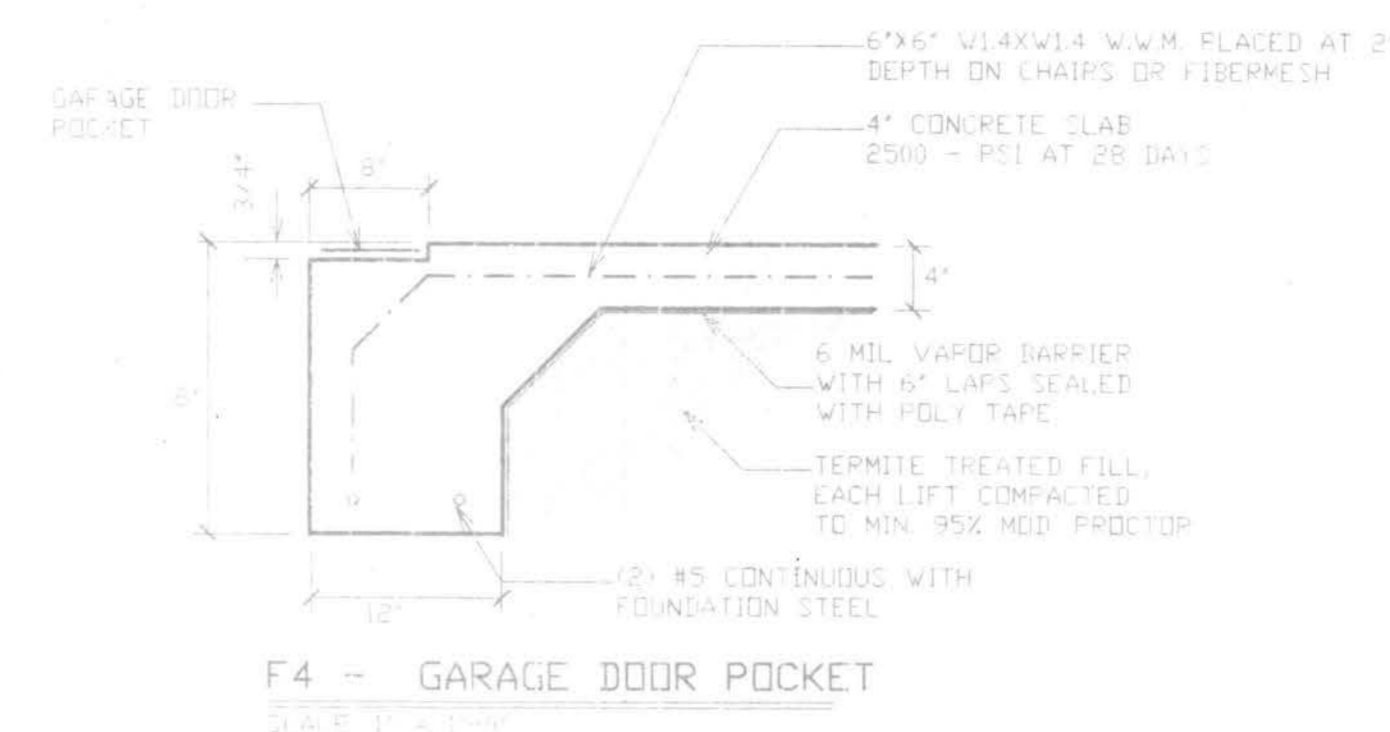
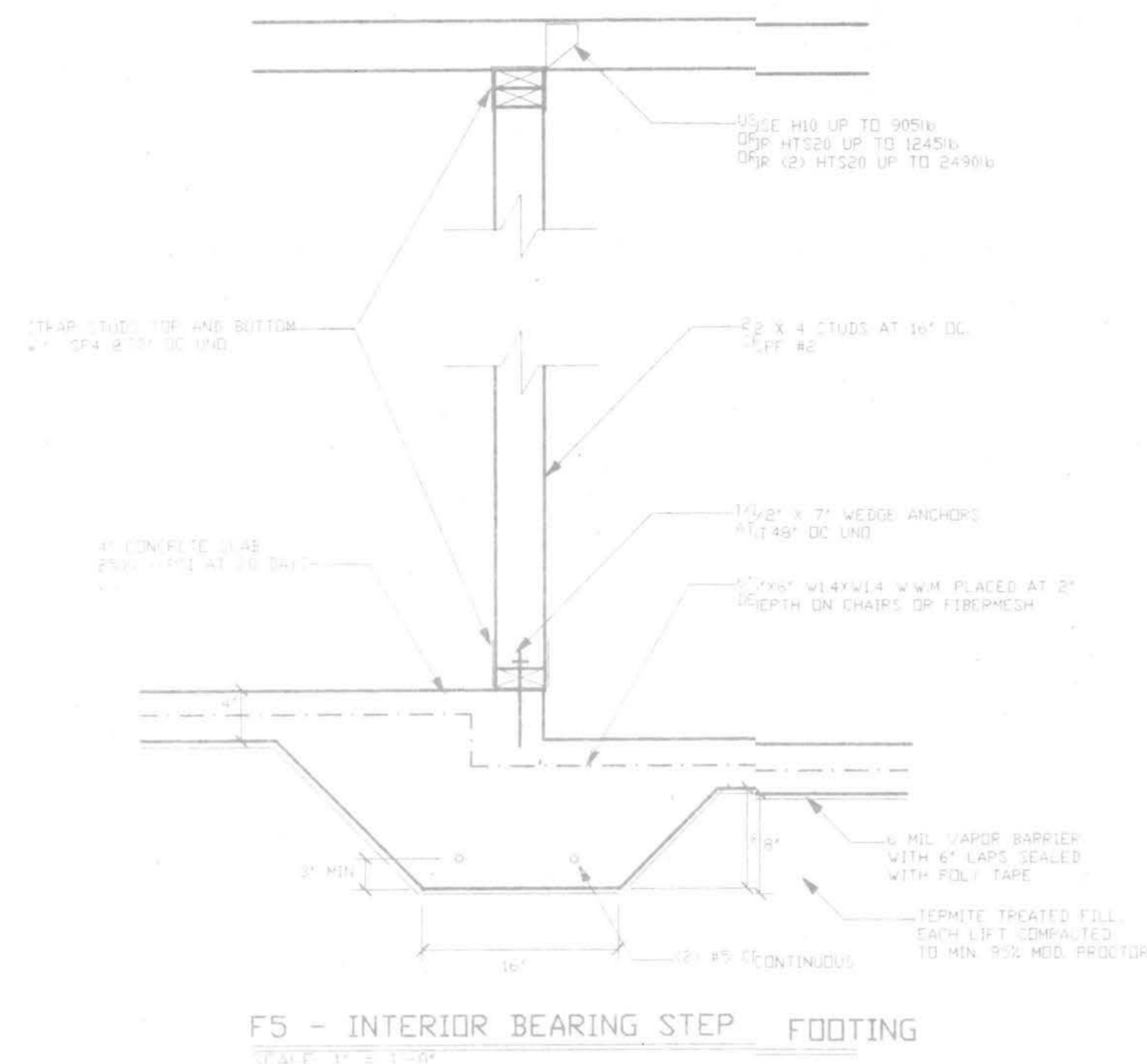
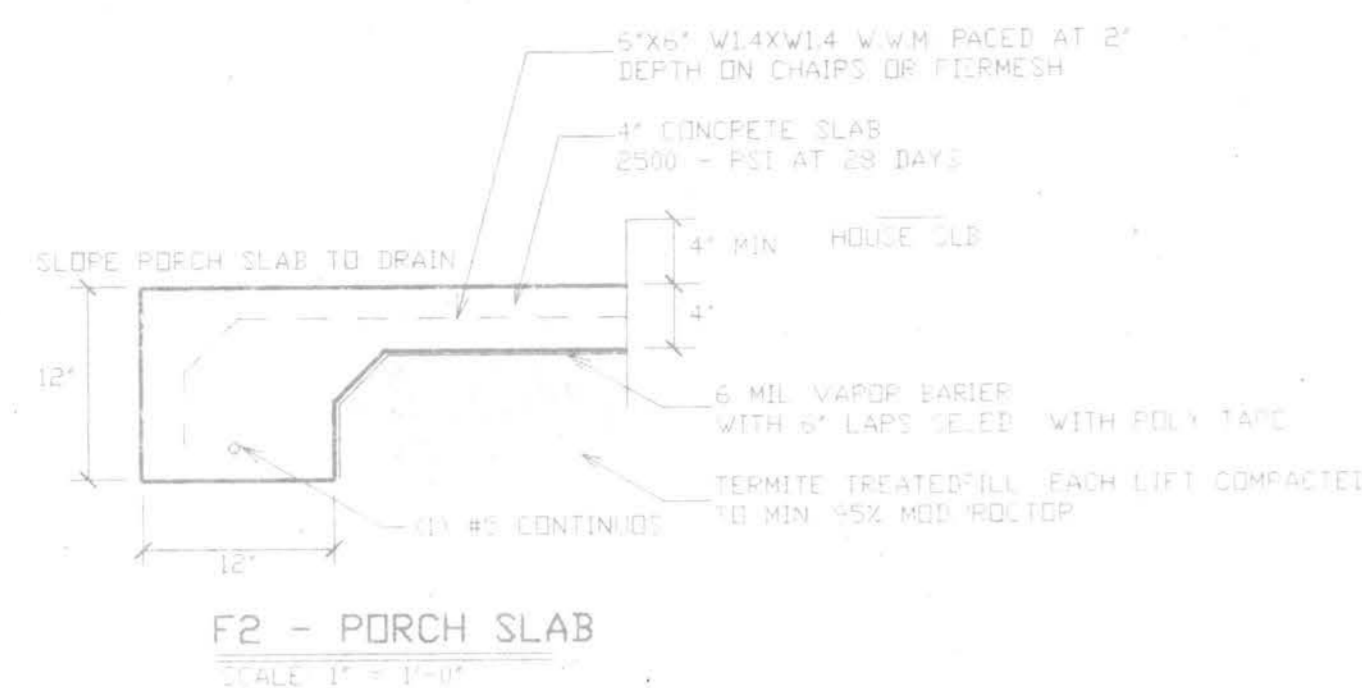
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

CARL WILSON, RD. 1
Location: WINTER RD., COLUMBIA CO. Job No. _____

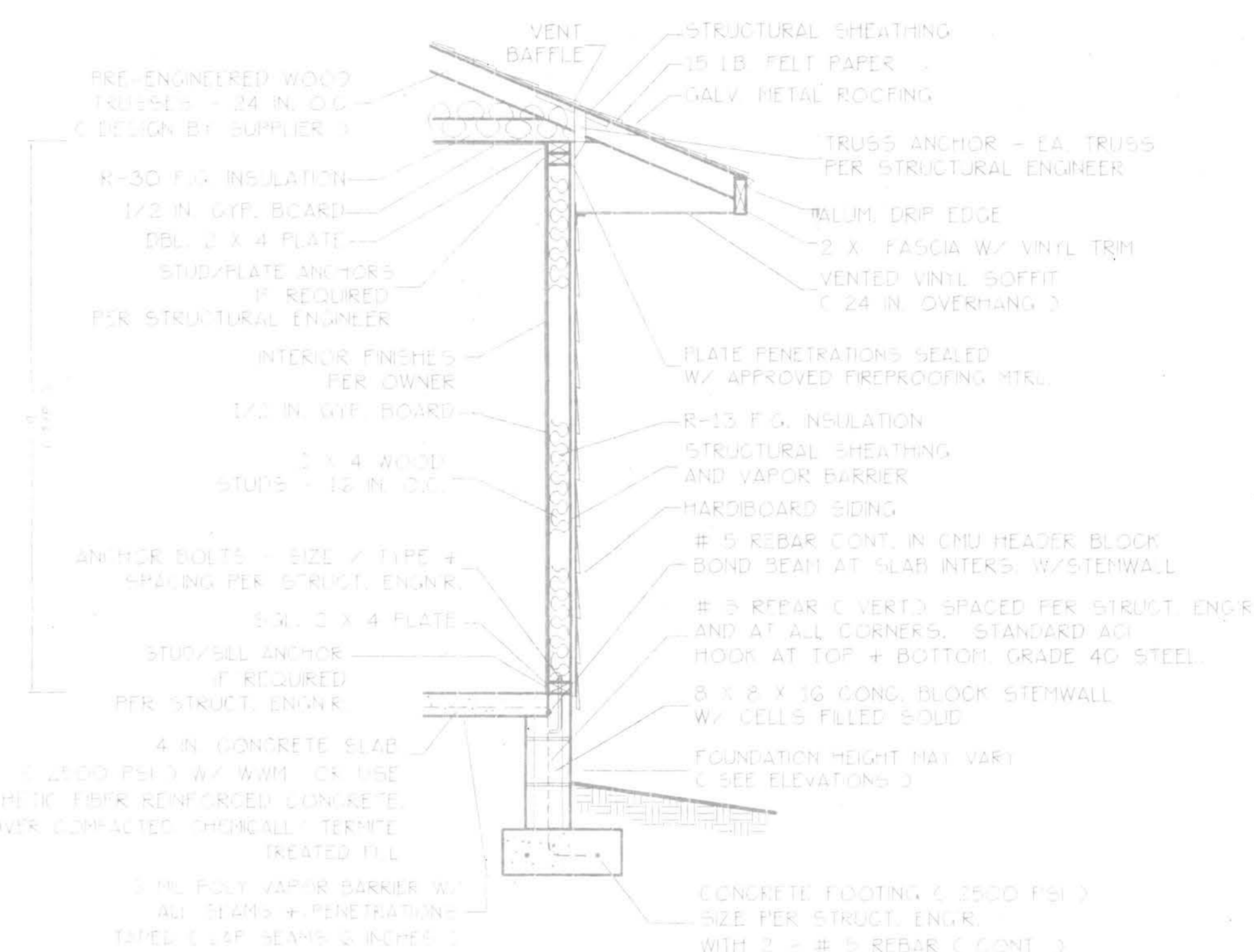
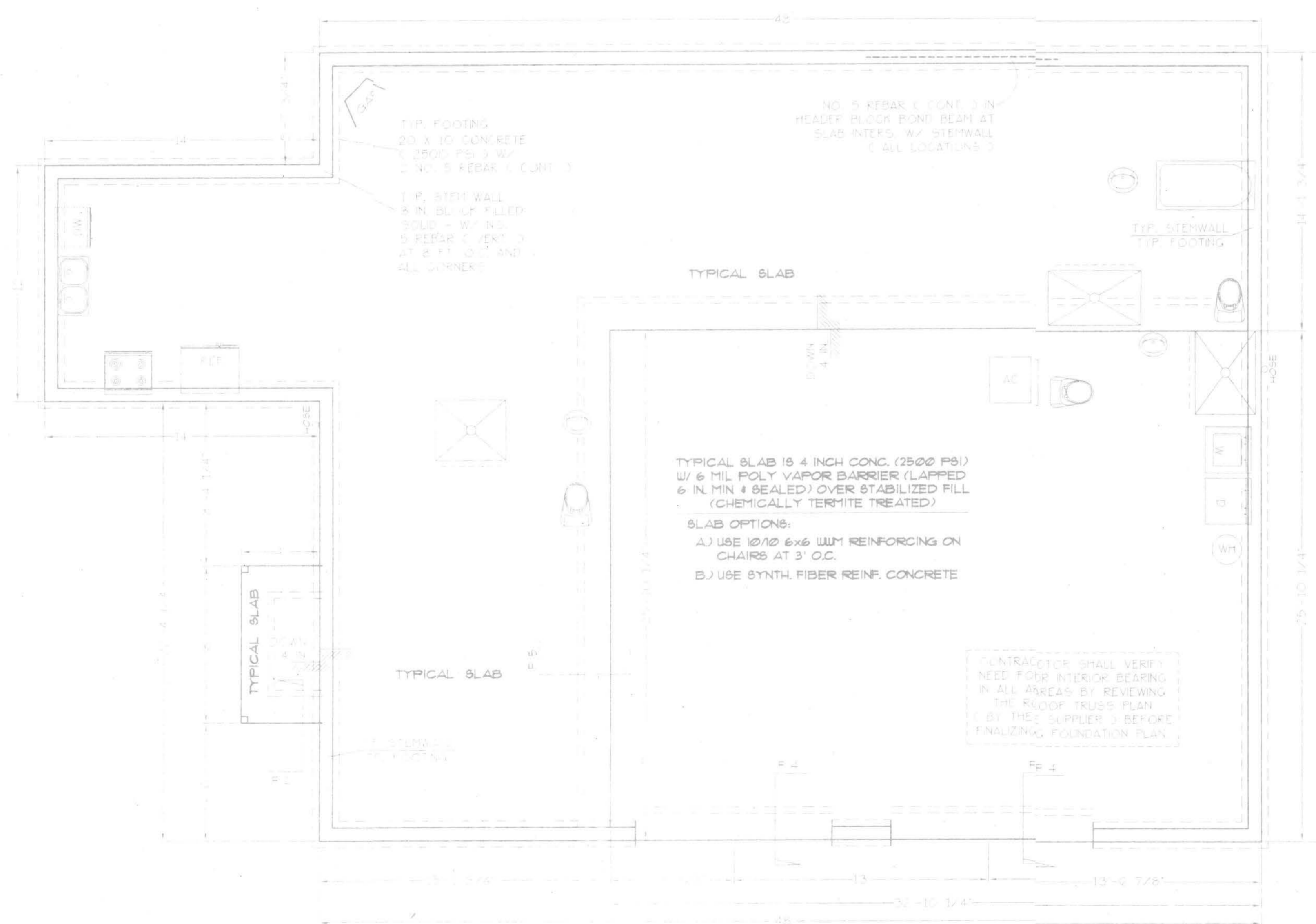
FILE 06-022	DUNHAM-BLACK RESIDENCE	SHEET 2 OF 4
DATE 6-15-06		CAD FILE 06022
DRAWN T A D		REV. 9-2-06
CHECK T A D		REV.

PREPARED BY
TIM DELBENE
Drafting + Technical Services
192 SW Sagewood Cir. Lake City, FL 32024
Phone: (386) 755-5891



FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.



WALL SECTION NOTES:

- The Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

A-3

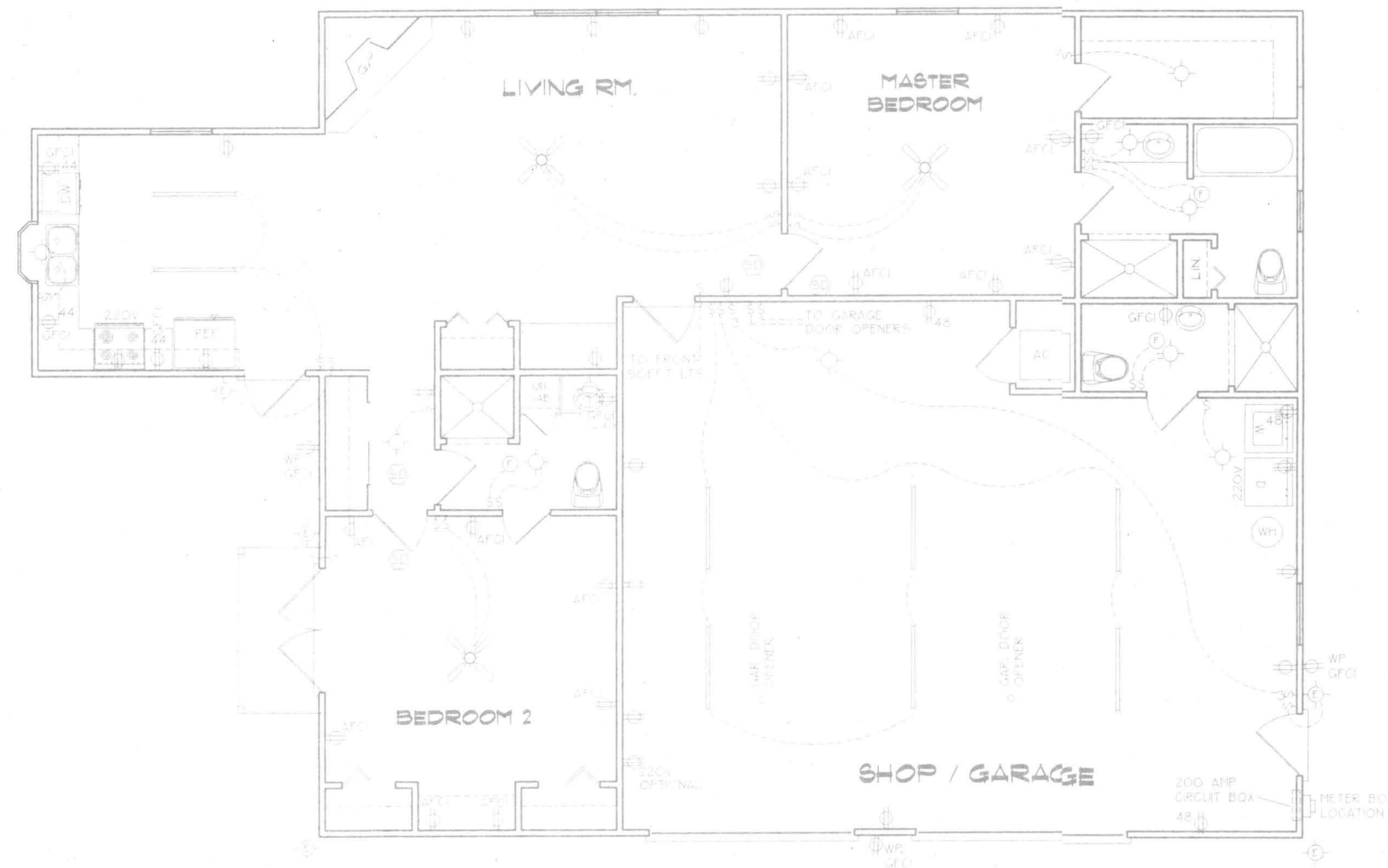
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 863, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: WINTER RD. COLUMBIA CO. Job No.:

FILE: 06-012	DUNHAM-BLACK RESIDENCE	SHEET: 3 OF 4
DATE: 2-15-06		DATE: 03-02-06
DRAWN: T A D	PREPARED BY: TIM DELBENE	REV: 4-2-06
CHECK: T A D	Drafting + Technical Services	REV:
	110 SW Sagewood Dr. Lake City, FL 32024	
	Phone: (386) 755-5841	



ELECTRICAL PLAN
NOT TO SCALE

ELECTRICAL SYMBOL LEGEND

	= FLOURESCENT LIGHTING FIXTURE
	= CEILING LIGHT FIXTURE
	= EXTERIOR LIGHTING FIXTURE
	= LIGHT SWITCH
	= THREE-WAY SWITCH
	= 110 V. DUPLEX OUTLET
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
	= GROUND FAULT CIRC. OUTLET
	= ARC FAULT CIRC. OUTLET
	= 110 V. SINGLE RECEPTACLE OUTLET
	= 220 VOLT OUTLET (4 WIRE)
	= FAN LOCATION (CEILING)
	= FAN LOCATION (EXHAUST)
	= SMOKE DETECTOR

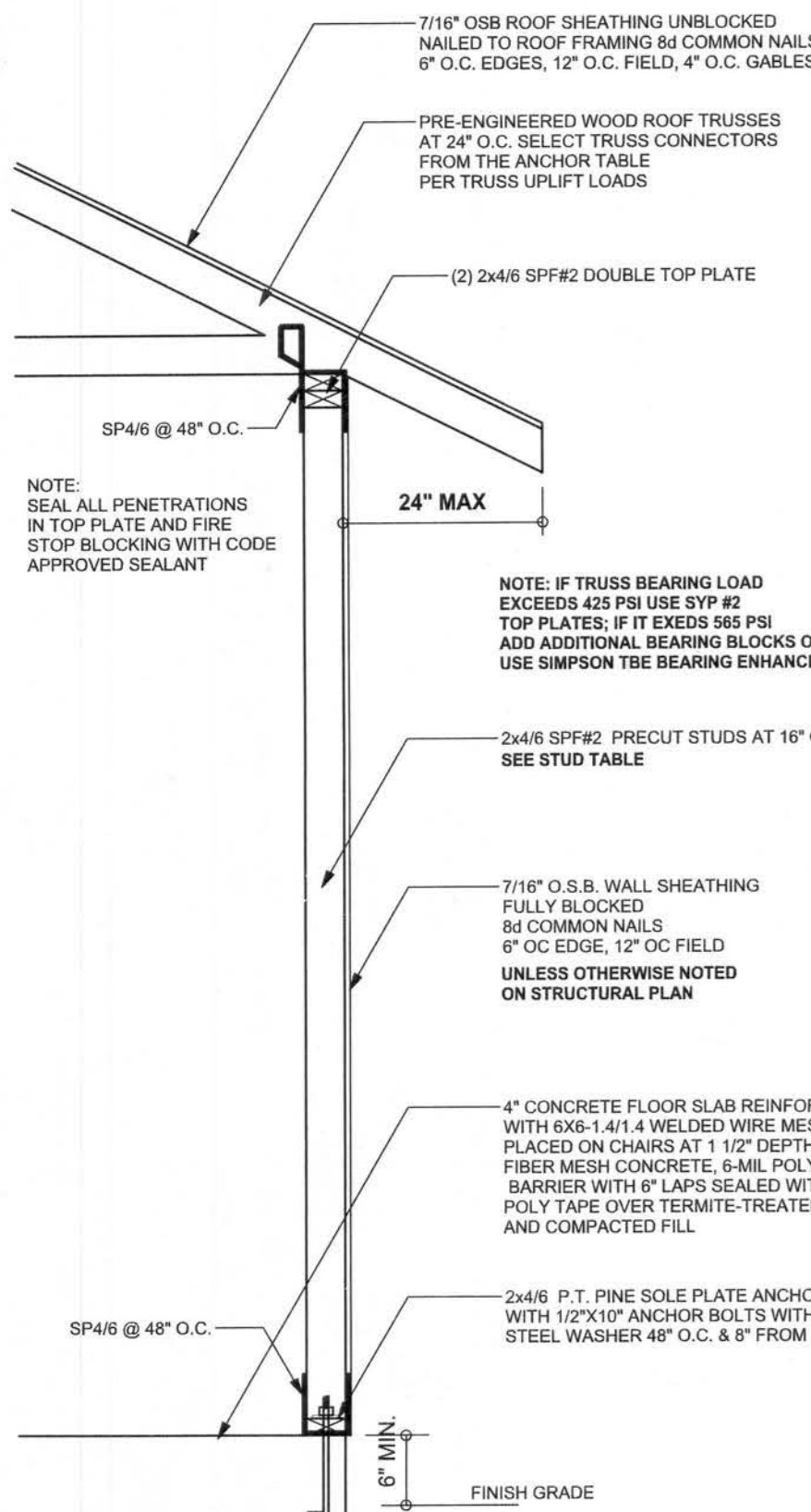
ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

A-4

CARL WILSON RD. 4
MINTER RD. COLUMBIA CO.

FILE 04-002	DUNHAM-BLACK RESIDENCE	SHEET 4 OF 4
DATE 6-15-06		CAD FILE 06002
DRAWN T & D	PREPARED BY TIM DELBENE Drafting + Technical Services	REV. 4-3-06
CHECK T & D	142 SW Sagewood Dr., Lake City, FL 32024 Phone: (386) 755-5891	REV.



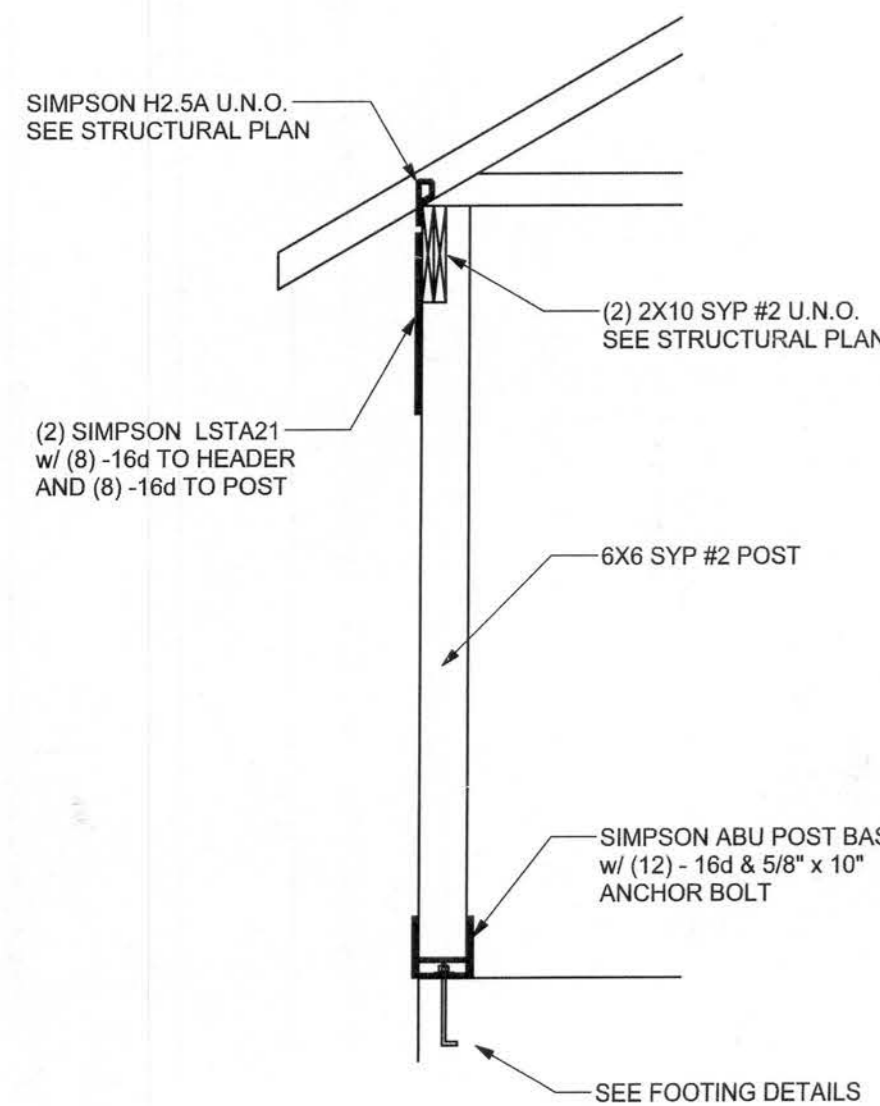
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

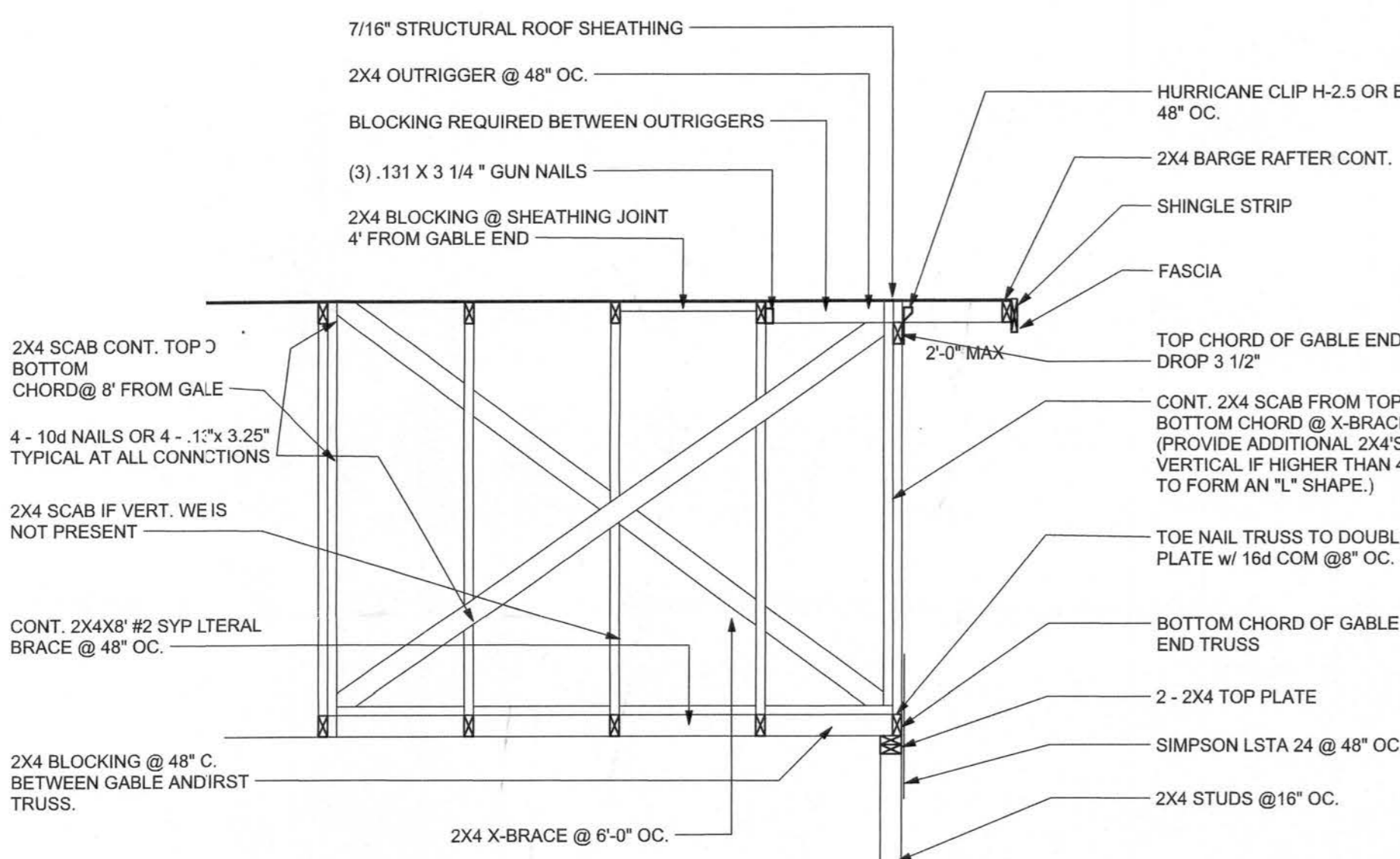
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B.
EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WIND LOADS. EXTERIOR STUDS
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



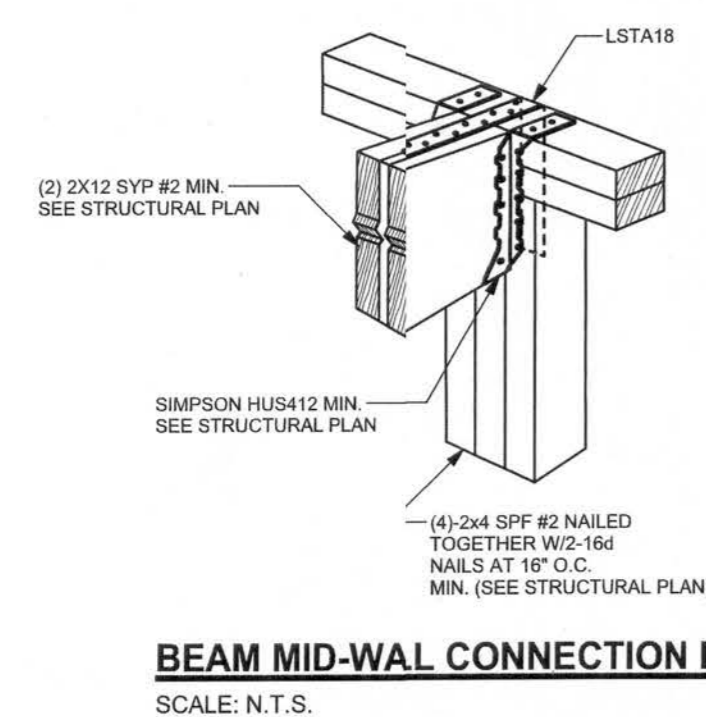
TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



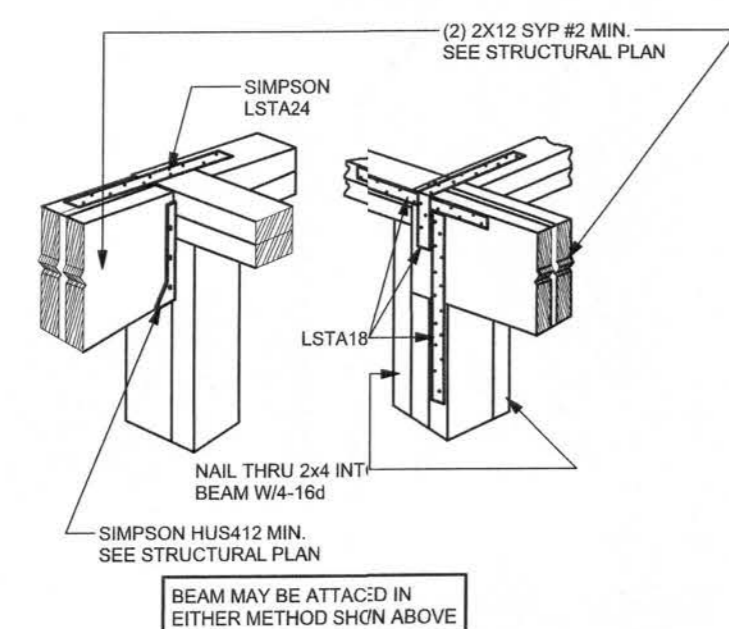
TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



BEAM MID-WAL CONNECTION DETAIL

SCALE: N.T.S.

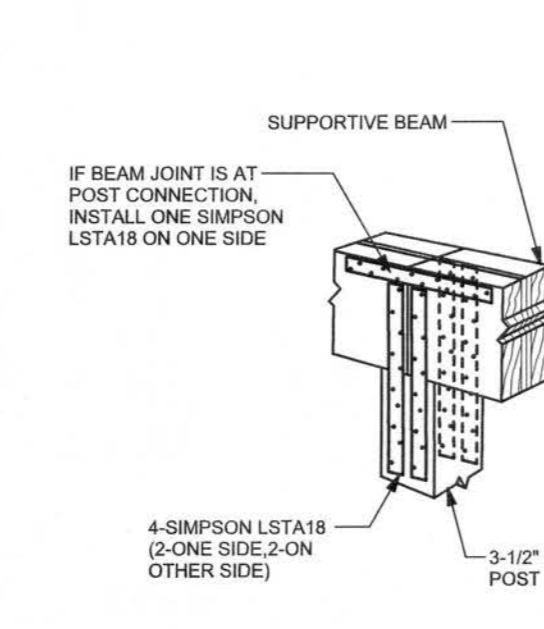


BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM
DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

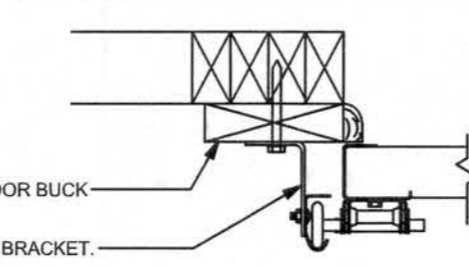


SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"
ON PER TABLE BELOW.

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.

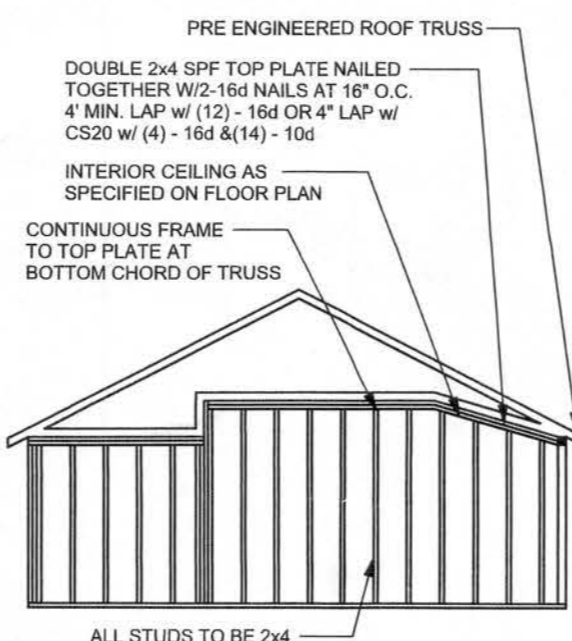


GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: 1/2" = 1'-0"

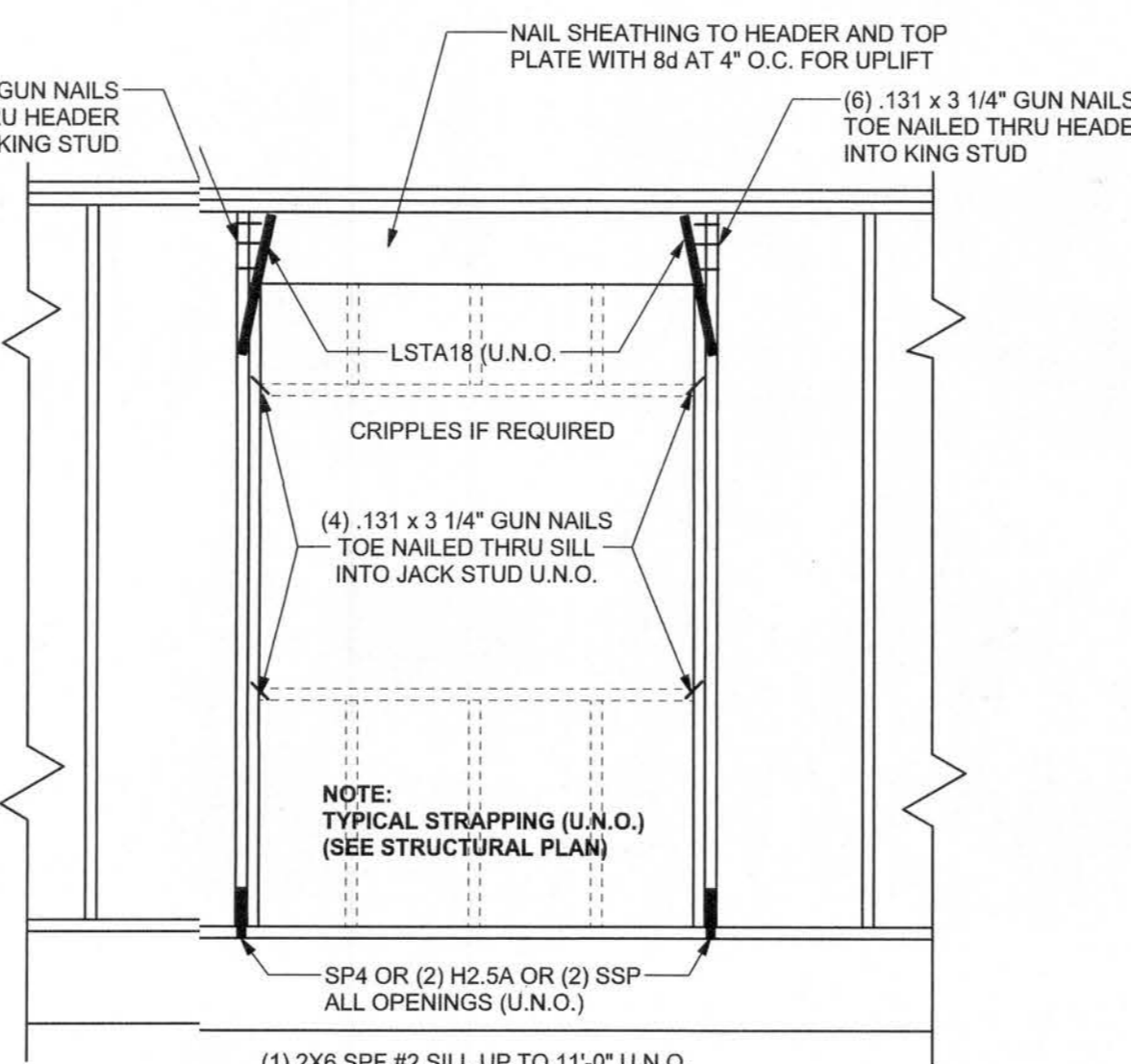
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT
CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVIDE OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 8" W1.4 x W1.4, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC
(W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD.
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 309. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT
CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS);
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,400, E = 1,800ksi, UNO. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131) 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH
3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL
CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY
STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON
MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF
ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER
IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F _m = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	H6	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 880	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 8035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 8250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			6-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			6-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 -16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 -16d		2-5/8" AB

DESIGN DATA

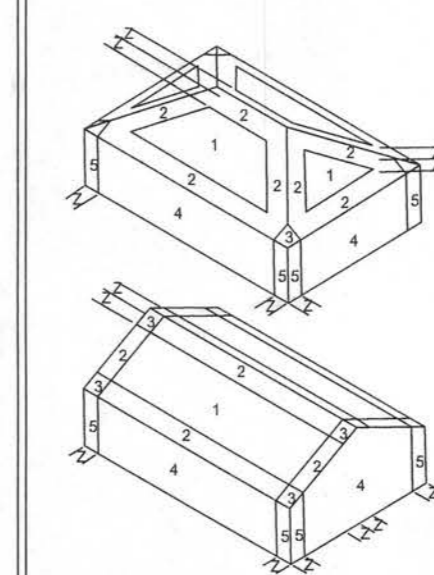
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF SLOPE NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%
SLOPE AND UNOBSERVED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

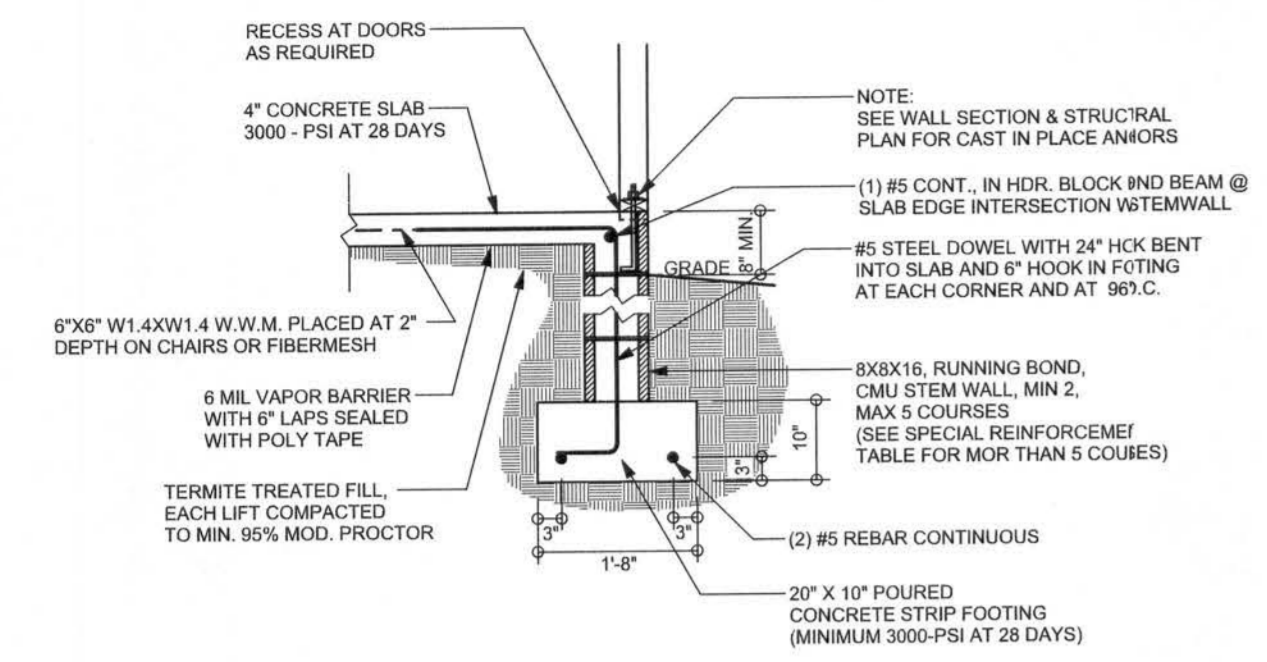
- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



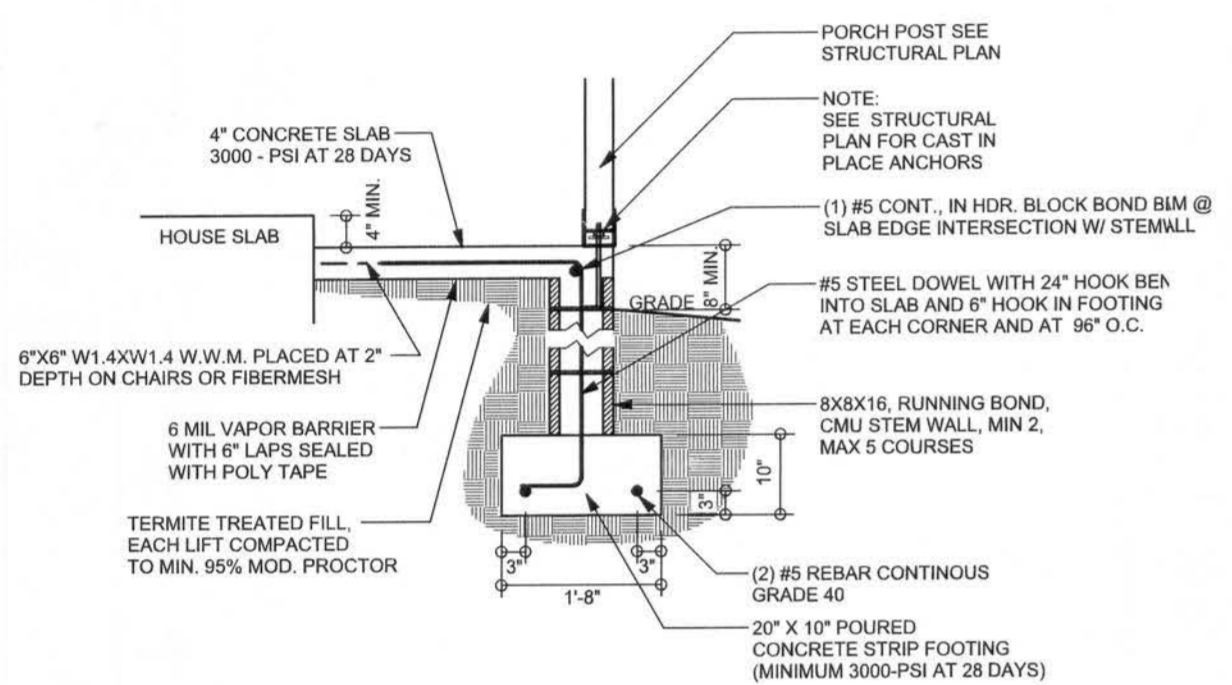
Zone	Effective Wind Area (ft ²)			
	10	100		
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 0'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 0'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	-29.1	
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0	

REVISIONS	

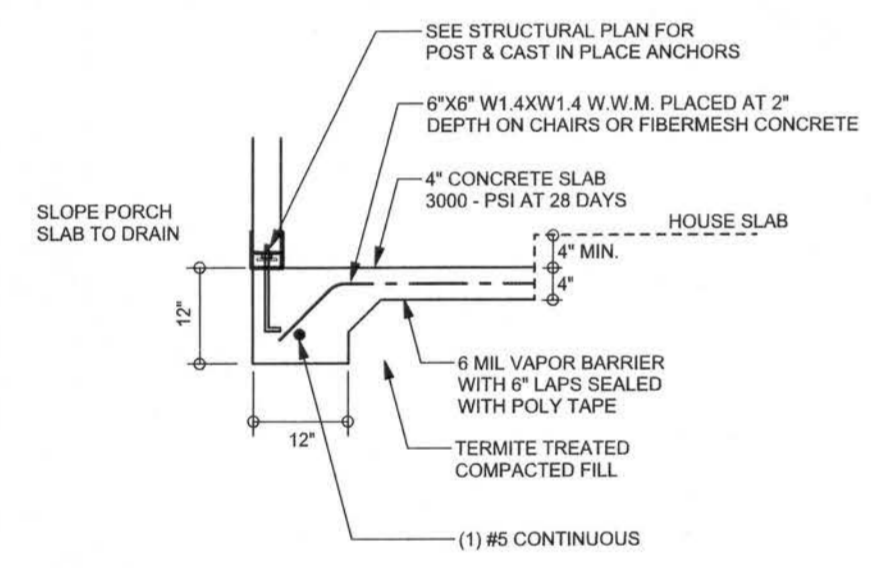
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



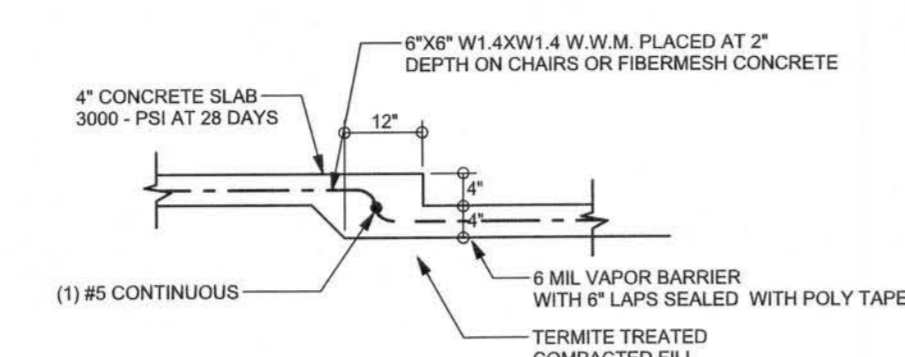
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



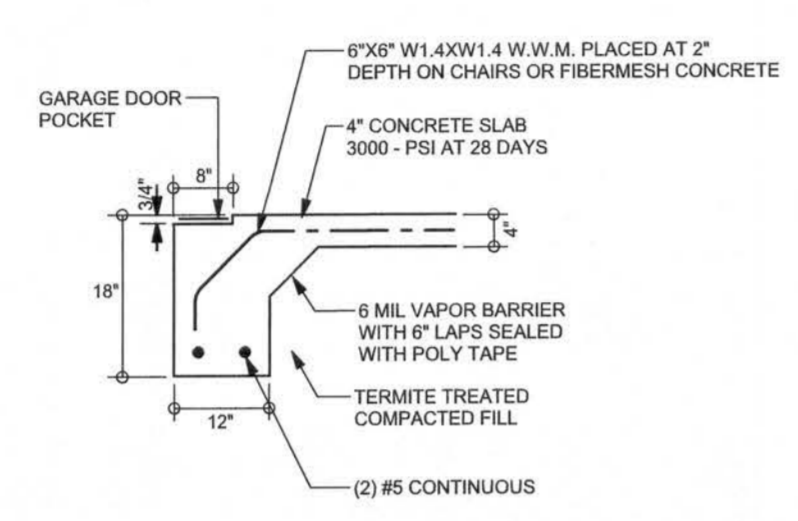
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



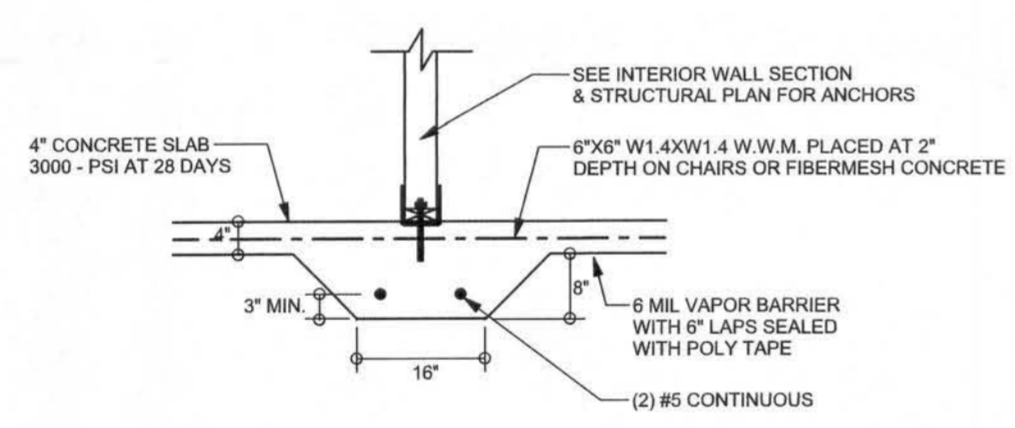
F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



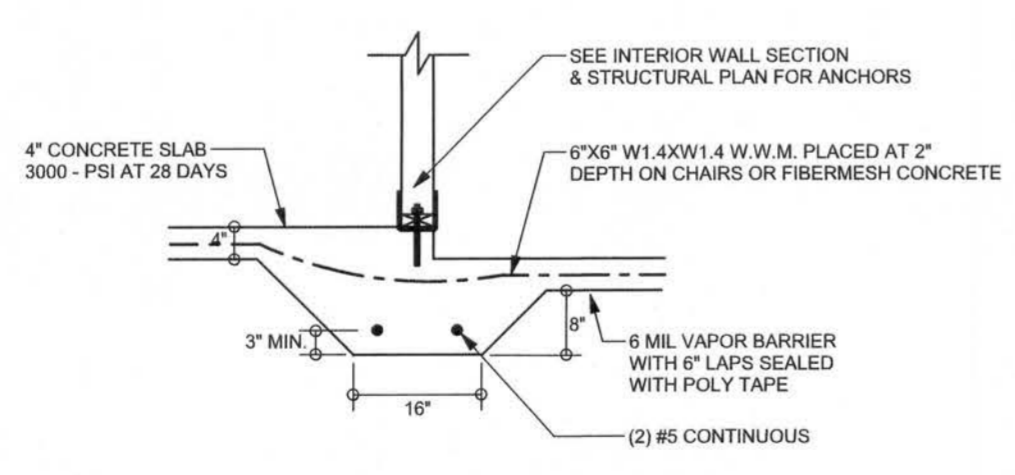
F6 S-2 TYPICAL NON-BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

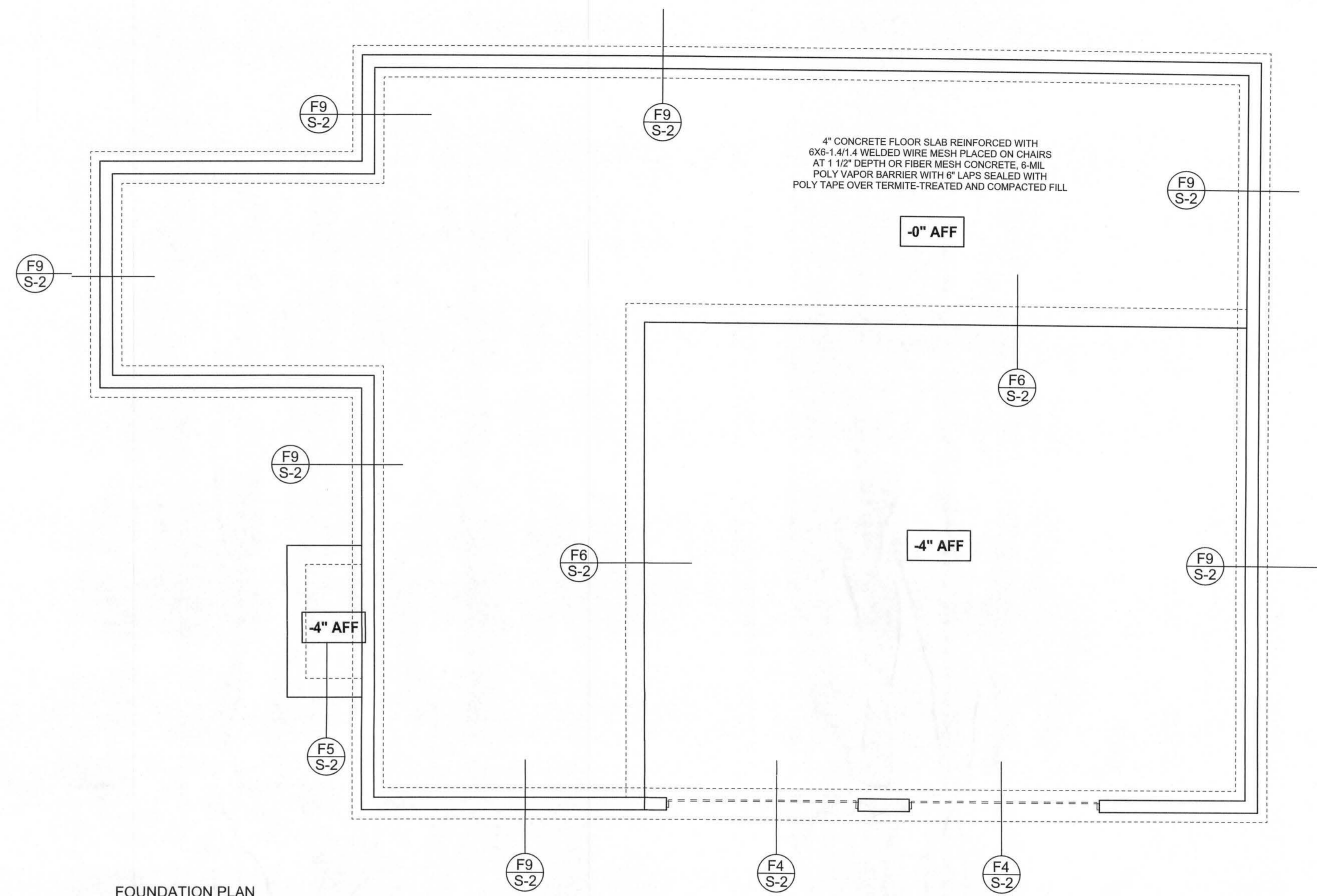


F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	96	96	96	96	96	96
5.3	5.0	96	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, (386) 754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS-01.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
Mark Disoway
18 OCT 06
SEAL

Dunham - Black Residence

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Columbia County, Florida

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P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
October 17, 2006

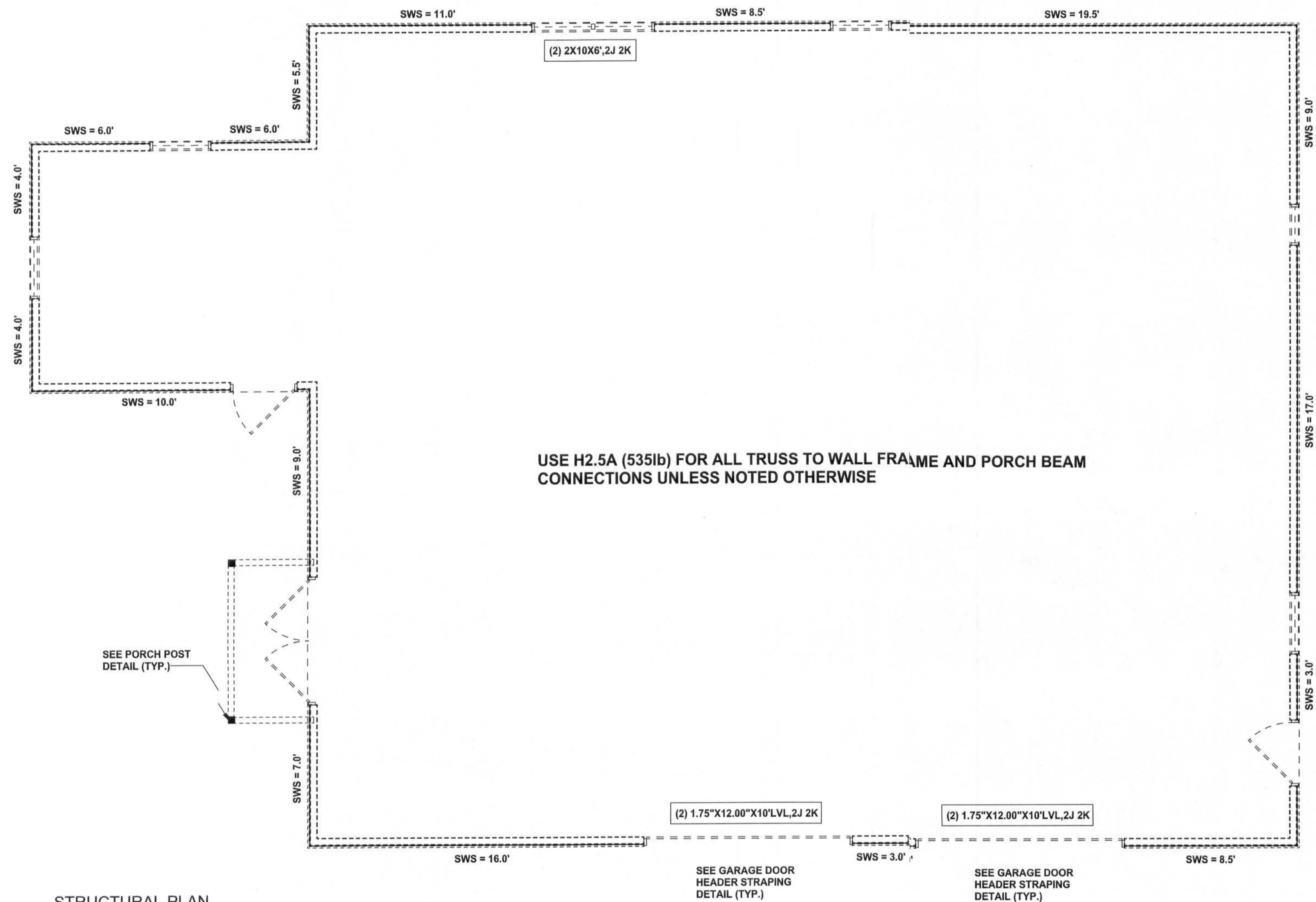
DRAWN BY: David Disoway
CHECKED BY:

FINALS DATE:
17 / Oct / 06

JOB NUMBER:
610163

DRAWING NUMBER
S-2

OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

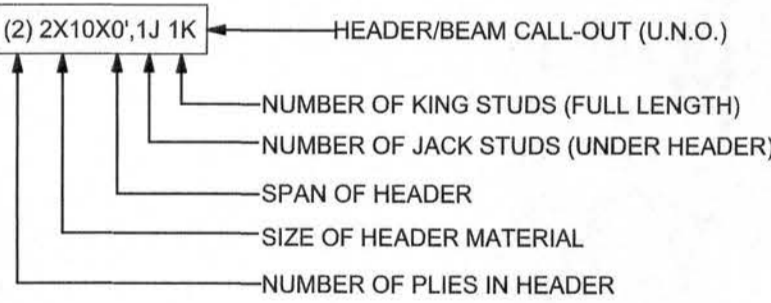
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	35.2'	58.5'
LONGITUDINAL	36.5'	88.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS JOB #DUNHAM-BLACK

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway,
PE No. 53915, PCB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
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form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915

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SEAL

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PRINTED DATE:
October 17, 2006

DRAWN BY:
David Disosway

CHECKED BY:

FINALS DATE:
17 / Oct / 06

JOB NUMBER:
610163

DRAWING NUMBER

S-3

OF 3 SHEETS