

Max Uplift 12=-223(load case 6), 8=-223(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/55, 2-3=-230/279, 3-4=-764/548, 4-5=-764/548, 5-6=-230/279, 6-7=0/55,

2-12=-335/362, 6-8=-335/362

BOT CHORD 11-12=-189/590, 10-11=-67/445, 9-10=-67/445, 8-9=-83/590 WEBS 3-11=-205/278, 4-11=-229/291, 4-9=-229/291, 5-9=-205/278

3-11=-205/278, 4-11=-229/291, 4-9=-229/291, 5-9=-205/278, 3-12=-682/155, 5-8=-682/155

150000 (SOUND) - 40 (TO (SOUND AND THE SOUND AND THE SOUN

JOINT STRESS INDEX

2 = 0.28, 3 = 0.30, 4 = 0.69, 5 = 0.30, 6 = 0.28, 8 = 0.66, 9 = 0.48, 10 = 0.16, 11 = 0.48 and 12 = 0.66

Office Cesion Engineer 100ride PE No. 3-1869 1100 Costal Bay Blyd Boynton Beach, FL 33435

Continued on page 2

January 28,2008





Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	
L265361	T10	COMMON	2	1		J1926985
				70.	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jan 15 14:30:08 2008 Page 2

NOTES

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All plates are 3x6 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 12 and 223 lb uplift at joint 8.

LOAD CASE(S) Standard

January 28,2008



🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job Truss Truss Type Qty JAMES & ERICA COOK / ROOF Ply J1926986 L265361 T11G **GABLE** Job Reference (optional) Builders FirstSource, Lake City, FI 32055

-1-6-0 6-1-0

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jan 15 14:30:09 2008 Page 1

Scale = 1:40.9

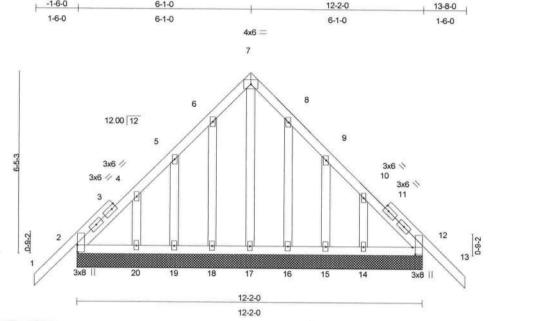


Plate Offsets (X,Y): [2:0-3-0,0-0-2], [12:0-3-0,0-1-2] LOADING (psf) SPACING 2-0-0 CSI DEFL L/d in (loc) I/defl **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.19 Vert(LL) -0.0113 n/r 120 MT20 244/190 TCDL 7.0 BC Lumber Increase 1.25 0.05 Vert(TL) -0.01 13 n/r 90 BCLL 10.0 Rep Stress Incr Horz(TL) NO WB 0.08 0.00 12 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 93 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **OTHERS** 2 X 4 SYP No.3 BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=220/12-2-0, 12=220/12-2-0, 17=69/12-2-0, 18=95/12-2-0, 19=103/12-2-0,

20=94/12-2-0, 16=95/12-2-0, 15=103/12-2-0, 14=94/12-2-0

Max Horz 2=217(load case 5)

Max Uplift 2=-100(load case 4), 12=-82(load case 7), 18=-86(load case 6),

19=-128(load case 6), 20=-83(load case 6), 16=-80(load case 7),

15=-130(load case 7), 14=-82(load case 7)

Max Grav 2=220(load case 1), 12=220(load case 1), 17=136(load case 7), 18=98(load case 10), 19=103(load case 1), 20=94(load case 1), 16=98(load case 11), 15=103(load case 1), 14=94(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/56, 2-3=-184/120, 3-4=-179/127, 4-5=-117/106, 5-6=-58/107, 6-7=-53/150,

7-8=-53/150, 8-9=-55/86, 9-10=-60/49, 10-11=-122/71, 11-12=-128/63, 12-13=0/56

2-20=-26/216, 19-20=-26/216, 18-19=-26/216, 17-18=-26/216, 16-17=-26/216,

15-16=-26/216, 14-15=-26/216, 12-14=-26/216

WEBS 7-17=-131/0, 6-18=-85/95, 5-19=-87/132, 4-20=-84/107, 8-16=-85/90, 9-15=-87/132,

10-14=-84/107

JOINT STRESS INDEX

BOT CHORD

NT STRESS INDEX
2 = 0.63, 3 = 0.00, 3 = 0.15, 3 = 0.16, 4 = 0.05, 5 = 0.06, 6 = 0.05, 7 = 0.08, 8 = 0.05, 9 = 0.06, 10 = 0.05, 11 = 0.00, 11 = 0.16

January 28,2008 Continued $0n^{1}$ and 2 = 0.03, 14 = 0.06, 15 = 0.07, 16 = 0.05, 17 = 0.04, 18 = 0.05, 19 = 0.07 and 20 = 0.06

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	
L265361	T11G	GABLE	1	1		J1926986
	ourse Lake City El				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jan 15 14:30:09 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 1-4-0 oc.

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 2, 82 lb uplift at joint 12, 86 lb uplift at joint 18, 128 lb uplift at joint 19, 83 lb uplift at joint 20, 80 lb uplift at joint 16, 130 lb uplift at joint 15 and 82 lb uplift at joint 14.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-7=-64(F=-10), 7-13=-64(F=-10), 2-12=-10

January 28,2008





Job Truss Truss Type Qty Ply JAMES & ERICA COOK / ROOF J1926987 L265361 T12G **GABLE** Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jan 15 15:11:44 2008 Page 1 -1-6-0 4-11-12 14-4-4 19-4-0 20-10-0

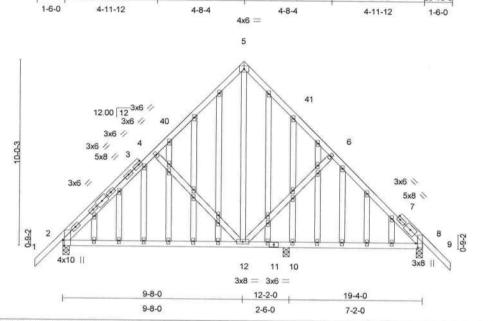


Plate Offse	ets (X,Y):	[2:0-3-8,Edge], [8:0	-2-12,0-1-2	2]		7.11						
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TR	2-0-0 1.25 1.25 NO PI2002	CSI TC BC WB (Mat	0.30 0.41 0.47 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.14 -0.27 0.02	(loc) 8-10 2-12 8	l/defl >581 >551 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 205 lb	GRIP 244/190

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 2=806/0-4-0, 8=835/0-4-0, 10=71/0-4-0 REACTIONS

Max Horz 2=-341(load case 4)

Max Uplift 2=-376(load case 6), 8=-477(load case 7), 10=-139(load case 4) Max Grav 2=806(load case 1), 8=835(load case 1), 10=122(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-860/364, 3-4=-711/379, 4-40=-694/383, 5-40=-575/371, 5-41=-456/372,

6-41=-689/382, 6-7=-804/379, 7-8=-908/370, 8-9=0/56

BOT CHORD 2-12=-243/570, 11-12=-167/568, 10-11=-167/568, 8-10=-167/568 **WEBS** 4-12=-244/305, 5-12=-285/378, 6-12=-241/326

JOINT STRESS INDEX

 $2 = 0.88, \ 3 = 0.07, \ 3 = 0.34, \ 3 = 0.24, \ 3 = 0.24, \ 3 = 0.24, \ 3 = 0.24, \ 3 = 0.24, \ 4 = 0.34, \ 5 = 0.46, \ 6 = 0.34, \ 7 = 0.18, \ 7 = 0.34, \ 7 = 0.23, \ 7 = 0.23, \ 7 = 0.24, \ 7 =$ = 0.00, 8 = 0.83, 10 = 0.34, 11 = 0.37, 12 = 0.57, 13 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 16 = 0.34, 17 = 0.34, 1819 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.00, 24 = 0.34, 25 = 0.34, 26 = 0.00, 26 = 0.34, 27 = 0.34, 28 = $0.34,\ 29 = 0.34,\ 29 = 0.34,\ 30 = 0.34,\ 31 = 0.34,\ 31 = 0.34,\ 32 = 0.34,\ 33 = 0.34,\ 34 = 0.34,\ 34 = 0.34,\ 35 = 0.34,\ 36 = 0.34,\ 37 = 0.34,\ 38 = 0.34$ 38 = 0.34 and 39 = 0.34

1) Unbalanced roof live loads have been considered for this design.

January 28,2008

Scale = 1:62.4

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	
L265361	T12G	GABLE	1	1		J1926987
		1 TO STATE OF THE			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jan 15 15:11:44 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 376 lb uplift at joint 2, 477 lb uplift at joint 8 and 139 lb uplift at joint 10.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-40=-54, 5-40=-91(F=-37), 5-41=-91(F=-37), 9-41=-64(F=-10), 2-8=-10

January 28,2008



🔔 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters and READ NOTES ON THIS AND INCLODED MITER REPERENCE PAGE MIL-7473 BEFORE USE.

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-97 Handling Installing and Bracing Recommendation suitable from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF
L265361	T13	COMMON	1	1	J1926988
				5.7	Job Reference (optional)
Builders FirstSo	ource, Lake City, FI	32055 6.3	300 s Feb 15 2006	MiTek I	ndustries, Inc. Tue Jan 15 14:30:11 2008 Page 1

-1-6-0 4-11-12 9-8-0 19-4-0 20-10-0 1-6-0 4-11-12 4-8-4 4-11-12 4x6 ||

Scale = 1:66.6

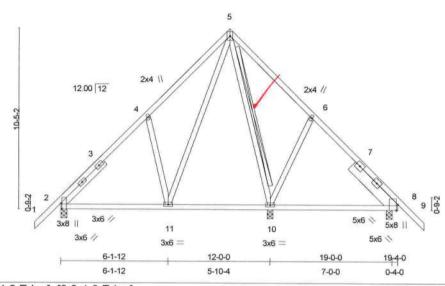


Plate Of	fsets (X,Y	'): [2:0-4-3,Edge], [8:	0-4-3,Edg	e]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.16	8-10	>545	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.08	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)		C. 100 100 100 100 100 100 100 100 100 10	70	1.00		Weight: 139 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

SLIDER Left 2 X 4 SYP No.3 3-5-11,

Right 2 X 8 SYP No.1D 3-5-13

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing. **WEBS** T-Brace:

2 X 4 SYP No.3 -

5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=517/0-4-0, 8=401/0-4-0, 10=481/0-4-0

Max Horz 2=-278(load case 4)

Max Uplift 2=-154(load case 6), 8=-238(load case 7), 10=-155(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD WEBS

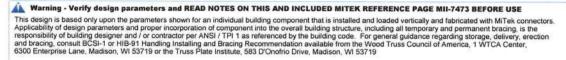
5-10=-235/28, 6-10=-210/338, 5-11=-256/288, 4-11=-218/320

JOINT STRESS INDEX

2 = 0.68, 2 = 0.15, 2 = 0.15, 3 = 0.00, 4 = 0.33, 5 = 0.46, 6 = 0.33, 7 = 0.00, 8 = 0.53, 8 = 0.09, 8 = 0.09, 10 = 0.51 and 11 = 0.51

Continued on page 2

January 28,2008





Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	Para a social
L265361	T13	COMMON	1	1		J1926988
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jan 15 14:30:11 2008 Page 2

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2, 238 lb uplift at joint 8 and 155 lb uplift at joint 10.

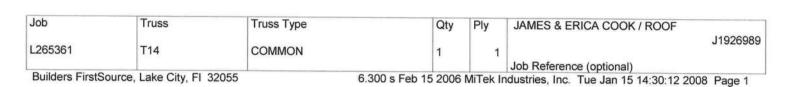
LOAD CASE(S) Standard

January 28,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719





Scale = 1:66.6

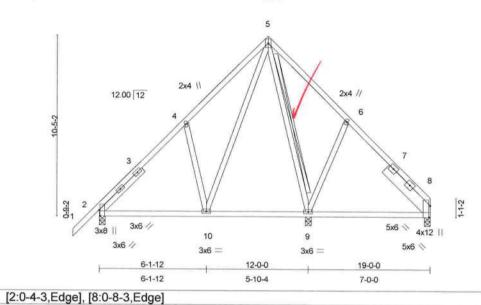


Plate Offsets (X,Y): LOADING (psf) **PLATES** SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **GRIP** TCLL 20.0 Plates Increase 1.25 TC 0.32 Vert(LL) 0.14 8-9 >598 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.20 Vert(TL) -0.068-9 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.52 Horz(TL) 0.01 8 n/a n/a BCDL Code FBC2004/TPI2002 (Matrix) Weight: 135 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

SLIDER Le

Left 2 X 4 SYP No.3 3-5-11, Right 2 X 8 SYP No.1D 3-7-3 BRACING TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

j. e: 2 X 4 SYP No.3 - 5-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=326/0-4-0, 2=530/0-4-0, 9=441/0-4-0

Max Horz 2=300(load case 5)

Max Uplift 8=-195(load case 7), 2=-168(load case 6), 9=-111(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3

1-2=0/48, 2-3=-496/259, 3-4=-386/275, 4-5=-411/473, 5-6=-217/436, 6-7=-233/279,

7-8=-315/249

BOT CHORD 2-10=-192/273, 9-10=-92/172, 8-9=-83/165

WEBS 5-9=-271/25, 6-9=-205/337, 5-10=-257/288, 4-10=-218/318

Julius Lee Truss Design Chaineer Florida PE No. 24865 1409 Chastal Ray Blvd Boynton Besch, FL 23435

JOINT STRESS INDEX

2 = 0.68, 2 = 0.15, 2 = 0.15, 3 = 0.00, 4 = 0.33, 5 = 0.44, 6 = 0.33, 7 = 0.00, 8 = 0.53, 8 = 0.09, 8 = 0.09, 9 = 0.51 and 10 = 0.51

Continued on page 2

January 28,2008

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Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	
L265361	T14	COMMON	1	1		J1926989
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jan 15 14:30:12 2008 Page 2

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 8, 168 lb uplift at joint 2 and 111 lb uplift at joint 9.

LOAD CASE(S) Standard

January 28,2008



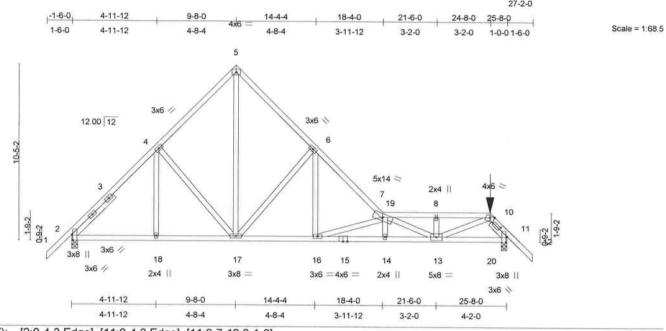
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Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	William A Donald All Model
L265361	T15	SPECIAL	1	1		J1926990
		100	1.27		Job Reference (optional)	

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					//							
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.19	14-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.36	14-16	>849	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.06	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)			1750			Weight: 172 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 SLIDER Left 2 X 4 SYP N

LIDER Left 2 X 4 SYP No.3 3-5-8, Right 2 X 4 SYP No.3 1-3-9 BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-5

oc purlins

BOT CHORD Rigid ceiling directly applied or 6-8-5 oc bracing.

REACTIONS (lb/size) 2=936/0-4-0, 11=1110/0-4-0

Max Horz 2=-278(load case 3)

Max Uplift 2=-243(load case 5), 11=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-1047/241, 3-4=-870/260, 4-5=-832/309, 5-6=-837/296, 6-7=-1543/407,

7-19=-2337/781, 8-19=-2336/781, 8-9=-2337/781, 9-10=-1173/453, 10-11=-1205/451,

11-12=0/48

BOT CHORD 2-18=-204/645, 17-18=-204/645, 16-17=-198/1065, 15-16=-894/3326, 14-15=-894/3326,

13-14=-891/3326, 13-20=-216/709, 11-20=-216/709

WEBS 4-18=0/141, 4-17=-199/175, 5-17=-310/793, 6-17=-814/369, 6-16=-258/885, 7-16=-2404/819, 7-14=0/81, 7-13=-1088/233, 8-13=-234/217, 9-13=-563/1787

Julius Law Trues Design Engineer Florida PE No. 24860 100 Coasial Ray Blod Doynton Desch. FL 33435

JOINT STRESS INDEX

2 = 0.61, 2 = 0.20, 2 = 0.20, 3 = 0.00, 4 = 0.47, 5 = 0.53, 6 = 0.72, 7 = 0.68, 8 = 0.34, 9 = 0.68, 10 = 0.00, 11 = 0.74, 11 = 0.46, 13 = 0.82, 14 = 0.34, 15 = 0.99, 16 = 0.67, 17 = 0.57 and 18 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; Continued on Mage 2; Lumber DOL=1.60 plate grip DOL=1.60.

January 28,2008

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Job	Truss	Truss Type	Qty	Ply	JAMES & ERICA COOK / ROOF	01/01/01/01/01/01/01/01/01
L265361	T15	SPECIAL	1	1		J1926990
D 11 E 10		THE SHIP STREET, STREET,			Job Reference (optional)	

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NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2 and 444 lb uplift at joint

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 7-19=-54, 9-19=-83(F=-29), 9-12=-54, 2-14=-10, 11-14=-15(F=-5)

Concentrated Loads (lb) Vert: 9=-27(F)

January 28,2008



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