MiTek®
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RE: 11783 - Amelia Jewel; L9 Meadowlands

MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746

Site Information:

Project Customer: Amelia Jewel Inc Project Name: L9 Medowlands
Lot/Block: 9 Subdivision: Meadowlands PI
Address:
City: Columbia County State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 6.5
Wind Code: ASCE 7-02 Wind Speed: 120 mph Floor Load: N/A psf
Roof Load: 50.0 psf

This package includes 22 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I14424959	A01	8/26/08	18	I14424976	J01	8/26/08
2	I14424960	A02	8/26/08	19	I14424977	J03	8/26/08
3	I14424961	A02A	8/26/08	20	I14424978	J05	8/26/08
4	I14424962	A03	8/26/08	21	I14424979	J07	8/26/08
5	I14424963	A03A	8/26/08	22	I14424980	JC09	8/26/08
6	I14424964	A04	8/26/08				
7	I14424965	A04A	8/26/08				
8	I14424966	A05	8/26/08				
9	I14424967	A05A	8/26/08				
10	I14424968	A06	8/26/08				
11	I14424969	A06A	8/26/08				
12	I14424970	A07	8/26/08				
13	I14424971	A07A	8/26/08				
14	I14424972	B01	8/26/08				
15	I14424973	B01A	8/26/08				
16	I14424974	C01	8/26/08				
17	I14424975	C01A	8/26/08				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders Truss Manufacturing.

Truss Design Engineer's Name: Fox, Steve
My license renewal date for the state of Florida is FEBRUARY 28, 2009

Steven E. Fox, FL Lic #044975
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NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

August 26,2008

Fox, Steve

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424959
11783	A01	ROOF TRUSS	7	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:47 2008 Page 1

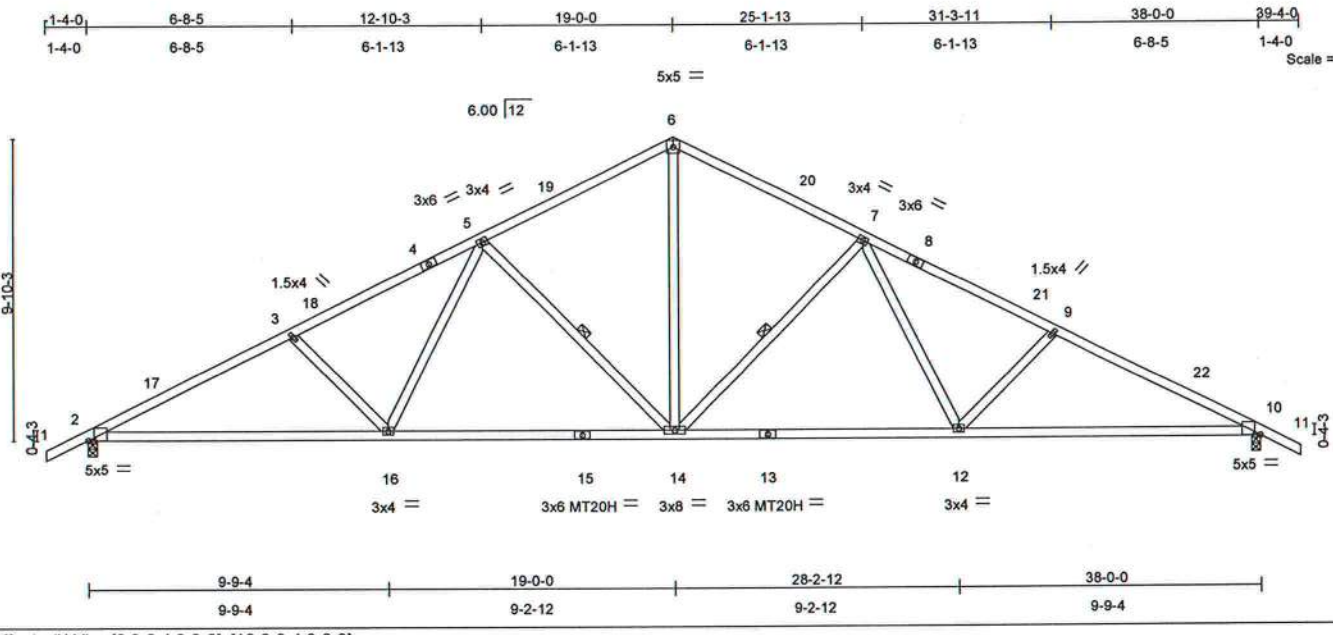


Plate Offsets (X,Y): [2:0-2-4,0-0-2], [10:0-2-4,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.30	2-16	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.68	2-16	>670	240	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.18	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.17	14	>999	240	Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

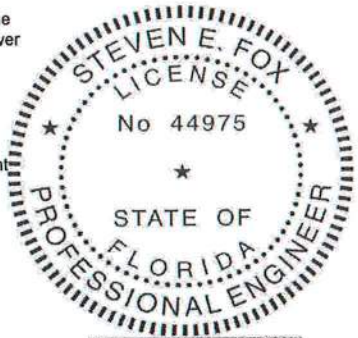
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-14

REACTIONS (lb/size) 2=1974/0-3-8, 10=1974/0-3-8
Max Horz 2=-157(LC 4)
Max Uplift 2=-474(LC 6), 10=-474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-17=-3462/817, 3-17=-3411/848, 3-18=-3198/784, 4-18=-3108/797, 4-5=-3047/814, 5-19=-2209/692, 6-19=-2061/713, 6-20=-2061/713, 7-20=-2209/692, 7-8=-3047/814, 8-21=-3108/797, 9-21=-3198/784, 9-22=-3411/848, 10-22=-3462/816, 10-11=0/35
BOT CHORD 2-16=-584/3030, 15-16=-396/2461, 14-15=-396/2461, 13-14=-396/2461, 12-13=-396/2461, 10-12=-584/3030
WEBS 3-16=-365/289, 5-16=-108/744, 5-14=-823/299, 6-14=-325/1520, 7-14=-823/299, 7-12=-108/744, 9-12=-365/289

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 15-2-6, Exterior(2) 15-2-6 to 19-0-0, Interior(1) 22-9-10 to 35-6-6 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 10.

LOAD CASE(S) Standard



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August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424960
11783	A02	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:48 2008 Page 1

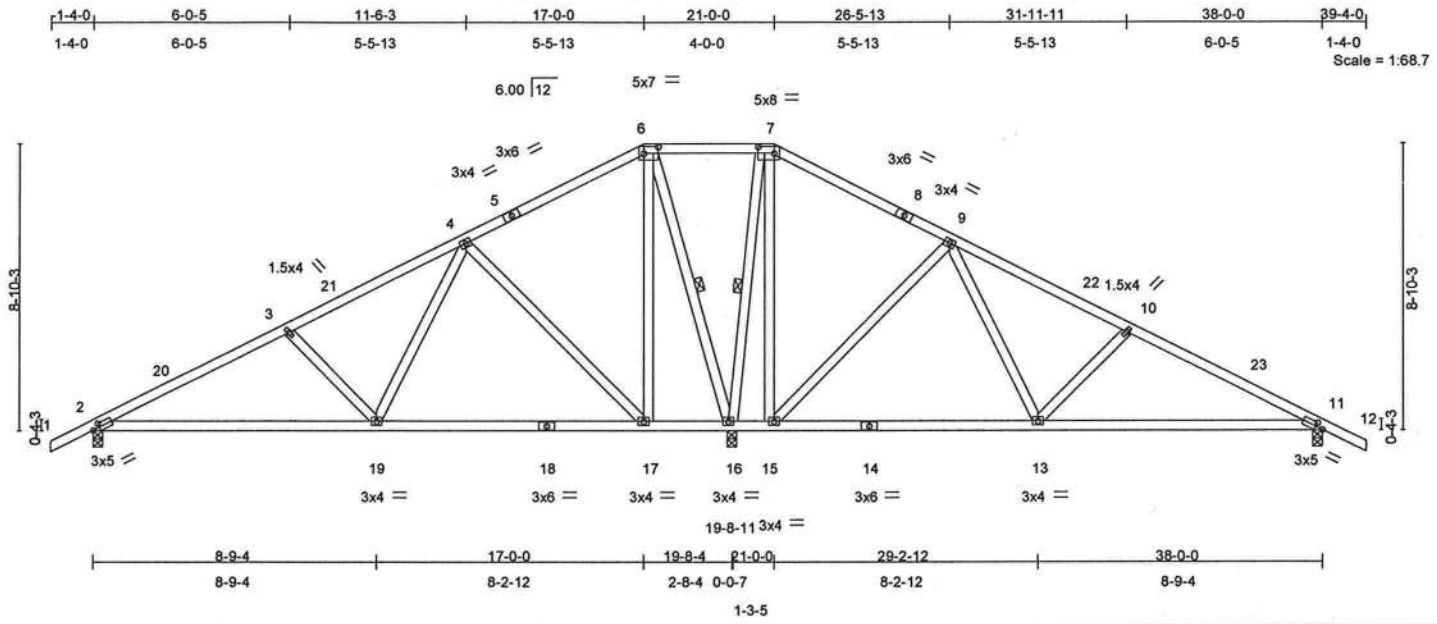

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-5-4,0-2-8], [7:0-6-0,0-2-8], [11:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-19	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.31	2-19	>764	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.02	11	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.05	2-19	>999	240	Weight: 227 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-16, 7-16

REACTIONS (lb/size) 2=734/0-3-8, 16=2576/0-3-8, 11=637/0-3-8
Max Horz 2=140(LC 5)
Max Uplift 2=-268(LC 6), 16=-427(LC 6), 11=-271(LC 7)
Max Grav 2=802(LC 10), 16=2576(LC 1), 11=714(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-20=-980/206, 3-20=-932/235, 3-21=-736/178, 4-21=-656/205, 4-5=0/362, 5-6=0/495, 6-7=0/754, 7-8=-10/709, 8-9=-26/576, 9-22=-411/183, 10-22=-546/156, 10-23=-743/231, 11-23=-791/212, 11-12=0/35
BOT CHORD 2-19=-171/829, 18-19=-59/286, 17-18=-59/286, 16-17=-365/332, 15-16=-556/375, 14-15=-172/176, 13-14=-172/176, 11-13=-69/661
WEBS 3-19=-353/273, 4-19=-105/707, 4-17=-764/294, 6-17=-104/770, 6-16=-1286/293, 7-16=-1324/256, 7-15=-54/757, 9-15=-764/295, 9-13=-106/711, 10-13=-357/273

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 13-2-6, Exterior(2) 13-2-6 to 24-9-10, Interior(1) 24-9-10 to 35-6-6 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2, 427 lb uplift at joint 16 and 271 lb uplift at joint 11.

LOAD CASE(S) Standard

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August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424961
11783	A02A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:50 2008 Page 1

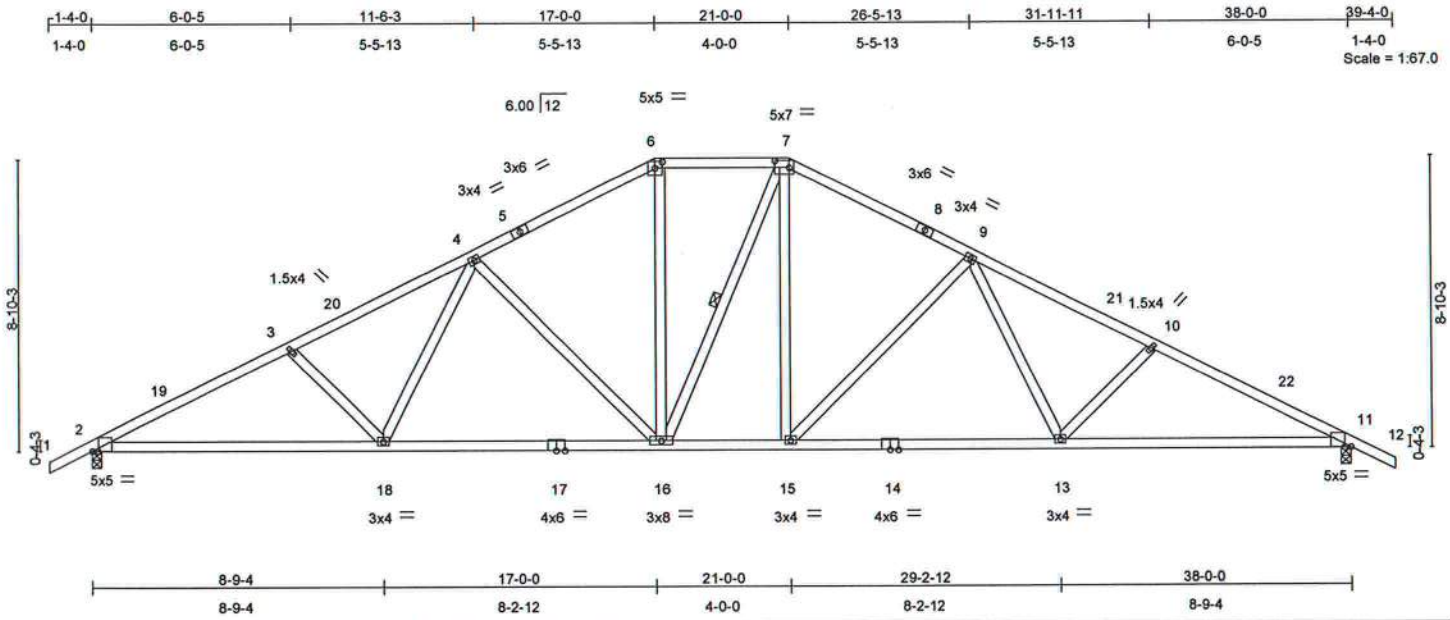

Plate Offsets (X,Y): [2:0-2-4,0-0-2], [6:0-2-8,0-2-4], [7:0-5-4,0-2-8], [11:0-2-4,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.27	13-15	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.58	13-15	>774	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.18	11	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.17	15	>999	240	Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-1 oc bracing.
WEBS 1 Row at midpt 7-16

REACTIONS (lb/size) 2=1974/0-3-8, 11=1974/0-3-8
Max Horz 2=-140(LC 4)
Max Uplift 2=-474(LC 6), 11=-474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-19=-3512/885, 3-19=-3463/914, 3-20=-3282/860, 4-20=-3204/887, 4-5=-2389/774, 5-6=-2315/801, 6-7=-2081/785, 7-8=-2312/800, 8-9=-2388/774, 9-21=-3205/887, 10-21=-3283/860, 10-22=-3464/914, 11-22=-3512/885, 11-12=0/35
BOT CHORD 2-18=-649/3074, 17-18=-496/2584, 16-17=-496/2584, 15-16=-299/2079, 14-15=-496/2584, 13-14=-496/2584, 11-13=-649/3074
WEBS 3-18=-308/265, 4-18=-94/666, 4-16=-740/286, 6-16=-137/754, 7-16=-177/187, 7-15=-138/755, 9-15=-743/286, 9-13=-95/667, 10-13=-308/265

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 13-2-6, Exterior(2) 13-2-6 to 24-9-10, Interior(1) 24-9-10 to 35-6-6 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 11.

LOAD CASE(S) Standard

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August 26,2008

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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424962
11783	A03	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:51 2008 Page 1

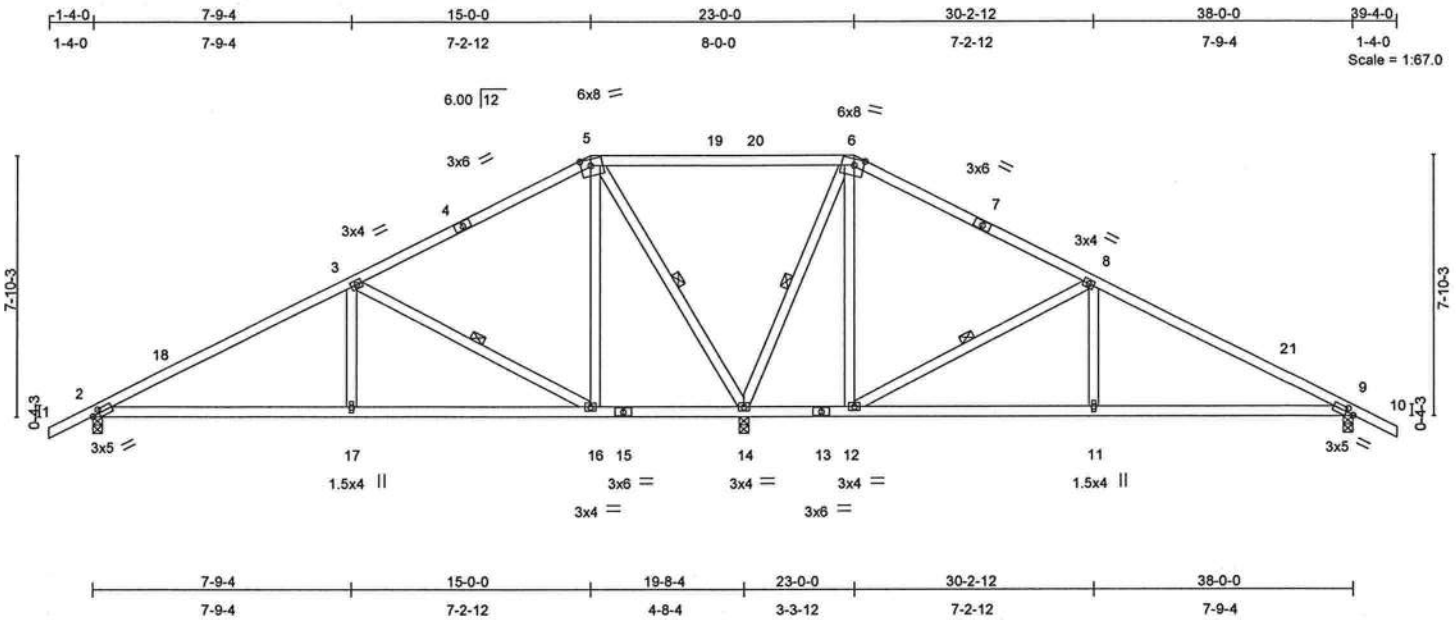


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [5:0-3-8,0-2-4], [6:0-3-8,0-2-4], [9:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-17	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.28	2-17	>837	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.09	2-17	>999	240	Weight: 204 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-8-14 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 3-16, 5-14, 6-14, 8-12

REACTIONS (lb/size) 2=754/0-3-8, 14=2533/0-3-8, 9=662/0-3-8
Max Horz 2=-123(LC 4)
Max Uplift 2=-282(LC 6), 14=-404(LC 6), 9=-285(LC 7)
Max Grav 2=814(LC 10), 14=2533(LC 1), 9=728(LC 11)

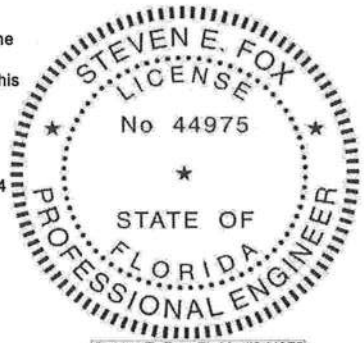
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/35, 2-18=-986/209, 3-18=-893/238, 3-4=-100/140, 4-5=0/229, 5-19=-2/831, 19-20=-2/831, 6-20=-2/831, 6-7=0/434, 7-8=0/302, 8-21=-707/209, 9-21=-799/179, 9-10=0/35
BOT CHORD	2-17=-144/794, 16-17=-144/794, 15-16=-133/251, 14-15=-133/251, 13-14=-290/265, 12-13=-290/265, 11-12=-45/628, 9-11=-45/628
WEBS	3-17=0/357, 3-16=-903/314, 5-16=-61/654, 5-14=-1375/385, 6-14=-1347/387, 6-12=-65/627, 8-12=-909/314, 8-11=0/361

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 11-2-6, Exterior(2) 11-2-6 to 15-0-0, Interior(1) 18-9-10 to 19-2-6, Exterior(2) 23-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2, 404 lb uplift at joint 14 and 285 lb uplift at joint 9.

LOAD CASE(S) Standard



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August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424963
11783	A03A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:53 2008 Page 1

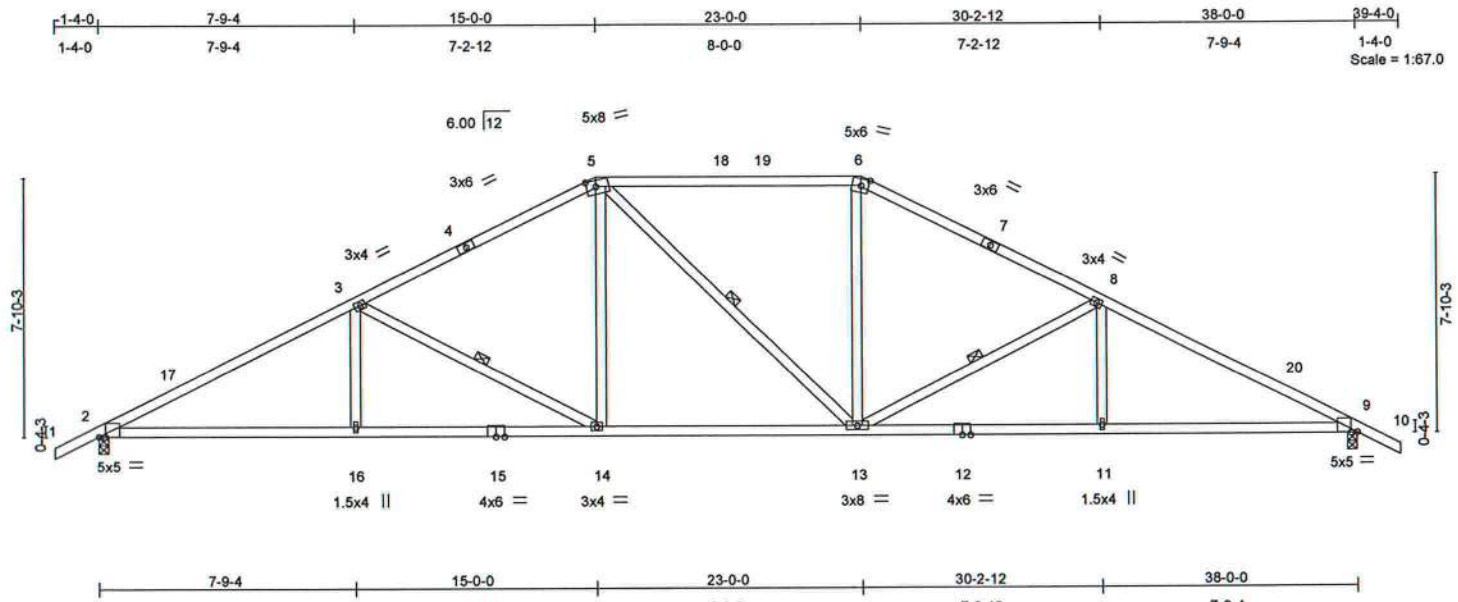


Plate Offsets (X,Y): [2:0-2-4,0-0-2], [5:0-3-4,0-2-4], [6:0-2-12,0-2-8], [9:0-2-4,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	-0.26	13-14	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.54	13-14	>844	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.19	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.18	13-14	>999	240	Weight: 195 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-5-10 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 3-14, 5-13, 8-13

REACTIONS (lb/size) 2=1974/0-3-8, 9=1974/0-3-8
Max Horz 2=-123(LC 4)
Max Uplift 2=-476(LC 6), 9=-476(LC 7)

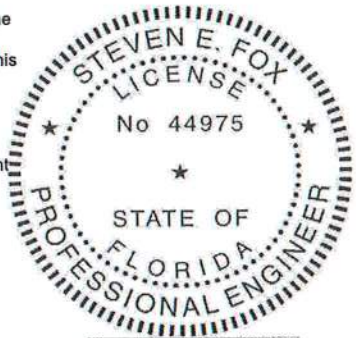
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/35, 2-17=-3507/907, 3-17=-3414/936, 3-4=-2661/836, 4-5=-2486/857, 5-18=-2306/853, 18-19=-2306/853, 6-19=-2306/853, 6-7=-2487/856, 7-8=-2662/835, 8-20=-3414/936, 9-20=-3506/907, 9-10=0/35
BOT CHORD	2-16=-651/3036, 15-16=-651/3036, 14-15=-651/3036, 13-14=-412/2305, 12-13=-652/3035, 11-12=-652/3035, 9-11=-652/3035
WEBS	3-16=0/338, 3-14=-843/309, 5-14=-51/693, 5-13=-183/185, 6-13=-39/693, 8-13=-842/309, 8-11=0/337

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 11-2-6, Exterior(2) 11-2-6 to 15-0-0, Interior(1) 18-9-10 to 19-2-6, Exterior(2) 23-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 9.

LOAD CASE(S) Standard



Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert #6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424964
11783	A04	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Mar 8 2007 MiTek Industries, Inc. Tue Aug 26 15:53:49 2008 Page 1

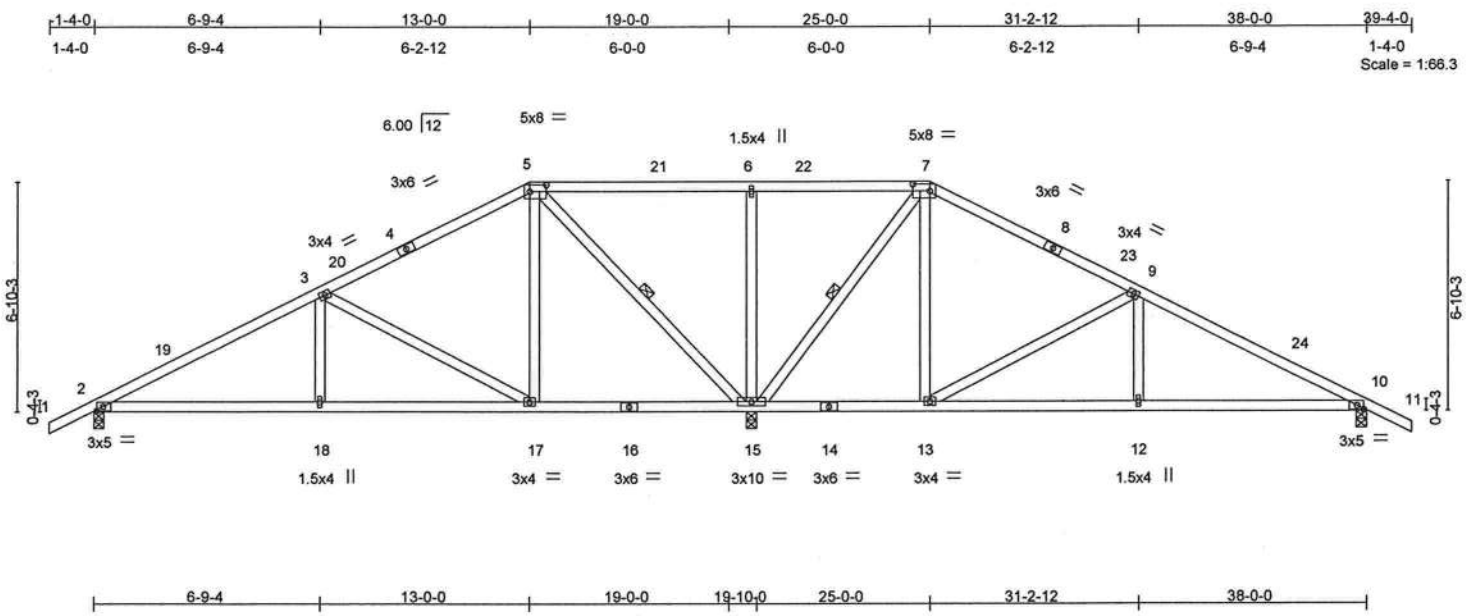


Plate Offsets (X,Y): [2:0-2-4,0-1-8], [5:0-6-0,0-2-8], [7:0-6-0,0-2-8], [10:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.08	2-18	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.18	2-18	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.03	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.05	2-18	>999	240	Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 13-15.
WEBS 1 Row at midpt 5-15, 7-15

REACTIONS (lb/size) 2=778/0-3-8, 15=2480/0-3-8, 10=690/0-3-8
Max Horz 2=-105(LC 4)
Max Uplift 2=-282(LC 6), 15=-398(LC 6), 10=-284(LC 7)
Max Grav 2=825(LC 10), 15=2480(LC 1), 10=741(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-19=-1074/239, 3-19=-993/263, 3-20=-324/159, 4-20=-302/170, 4-5=-174/190, 5-21=-21/812, 6-21=-21/813, 6-22=-20/811, 7-22=-20/811, 7-8=-36/161, 8-23=-105/117, 9-23=-127/113, 9-24=-818/226, 10-24=-898/202, 10-11=0/35
BOT CHORD 2-18=-146/882, 17-18=-146/882, 16-17=-32/227, 15-16=-32/227, 14-15=-110/211, 13-14=-110/211, 12-13=-49/727, 10-12=-49/727
WEBS 3-18=0/296, 3-17=-761/285, 5-17=-52/609, 5-15=-1326/363, 6-15=-392/280, 7-15=-1198/331, 7-13=-66/580, 9-13=-790/289, 9-12=0/305

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 9-2-6, Exterior(2) 9-2-6 to 13-0-0, Interior(1) 16-9-10 to 21-2-6, Exterior(2) 25-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2, 398 lb uplift at joint 15 and 284 lb uplift at joint 10.

LOAD CASE(S) Standard

Steven E. Fox, FL Lic #044975
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14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424965
11783	A04A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:56 2008 Page 1

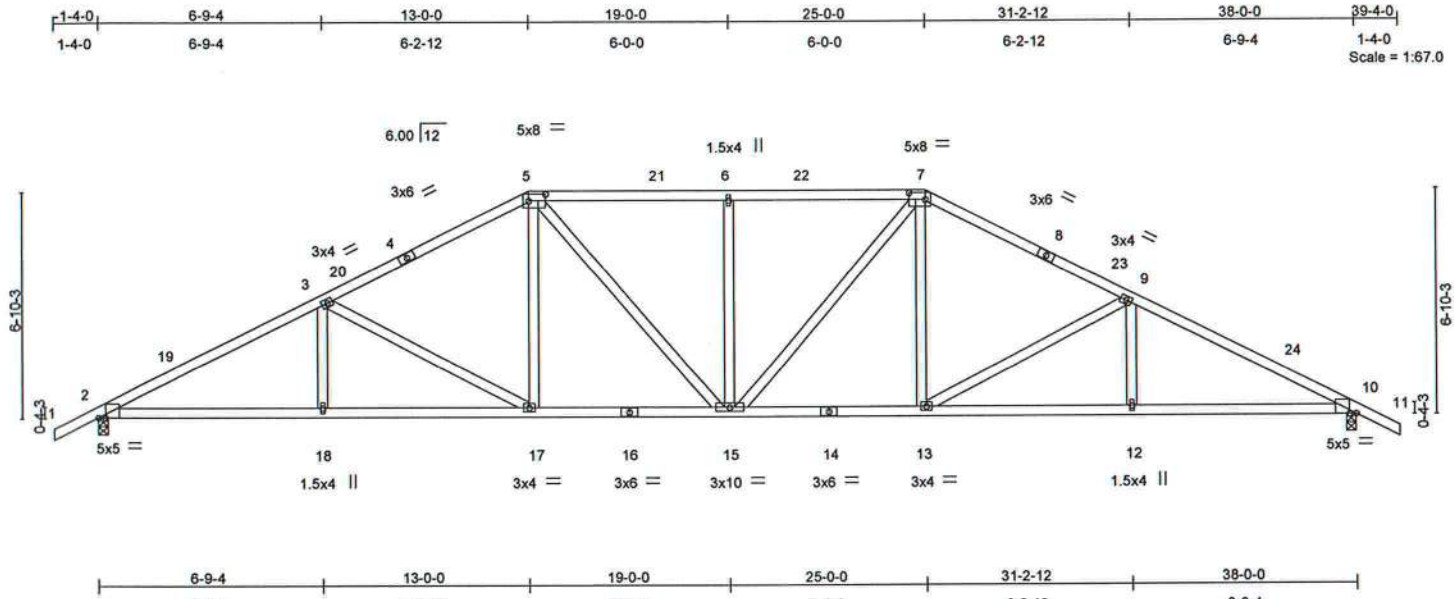


Plate Offsets (X,Y): [2:0-2-8,0-0-2], [5:0-6-0,0-2-8], [7:0-6-0,0-2-8], [10:0-2-8,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.23	13-15	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.46	13-15	>974	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.19	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.19	15	>999	240	Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

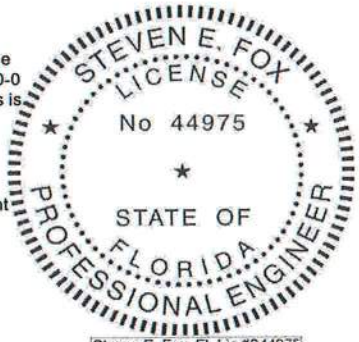
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.

REACTIONS (lb/size) 2=1974/0-3-8, 10=1974/0-3-8
Max Horz 2=-105(LC 4)
Max Uplift 2=-474(LC 6), 10=-474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-19=-3565/928, 3-19=-3484/952, 3-20=-2844/858, 4-20=-2776/868, 4-5=-2760/888, 5-21=-2742/933, 6-21=-2742/933, 6-22=-2742/933, 7-22=-2742/933, 7-8=-2760/888, 8-23=-2776/868, 9-23=-2844/858, 9-24=-3484/952, 10-24=-3565/928, 10-11=0/35
BOT CHORD 2-18=-678/3095, 17-18=-678/3095, 16-17=-474/2479, 15-16=-474/2479, 14-15=-474/2479, 13-14=-474/2479, 12-13=-678/3095, 10-12=-678/3095
WEBS 3-18=0/290, 3-17=-719/277, 5-17=-56/582, 5-15=-206/525, 6-15=-363/273, 7-15=-206/525, 7-13=-56/582, 9-13=-719/277, 9-12=0/290

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 9-2-6, Exterior(2) 9-2-6 to 13-0-0, Interior(1) 16-9-10 to 21-2-6, Exterior(2) 25-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 10.

LOAD CASE(S) Standard



Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424966
11783	A05	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Mar 8 2007 MiTek Industries, Inc. Tue Aug 26 15:54:20 2008 Page 1

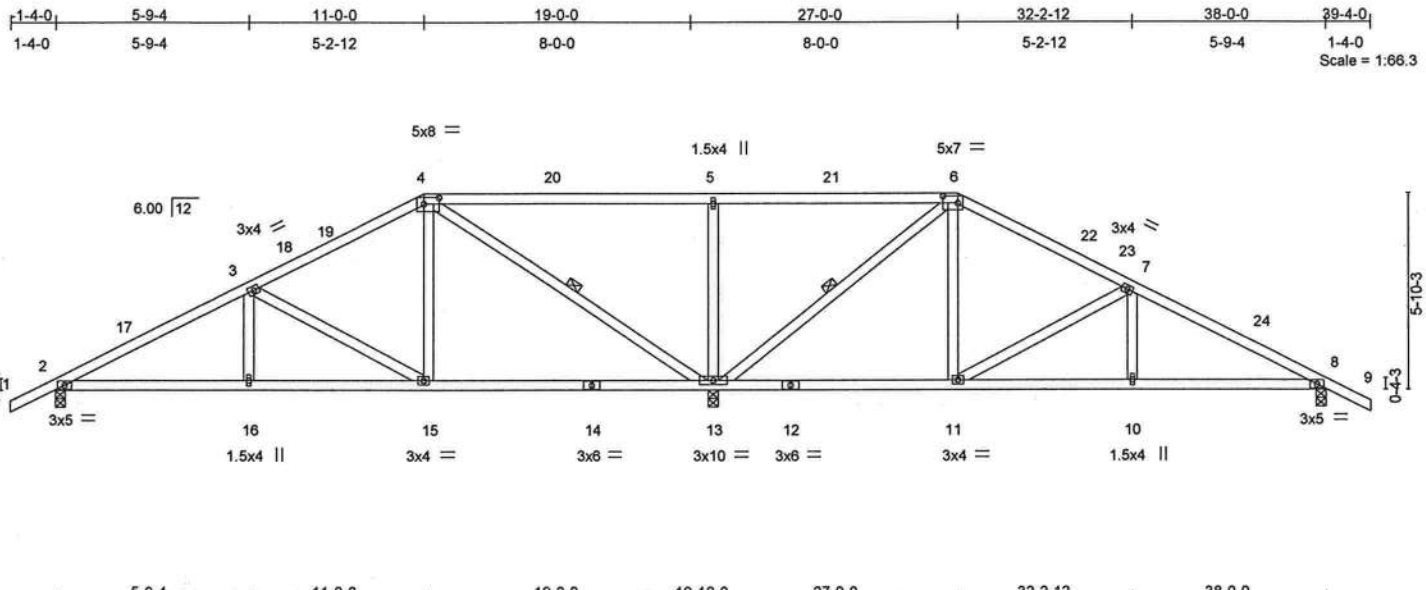


Plate Offsets (X,Y): [4:0-5-8,0-2-4], [6:0-5-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.10	13-15	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.24	13-15	>975	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.04	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.03	2-16	>999	240	Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-13

REACTIONS (lb/size) 2=843/0-3-8, 13=2344/0-3-8, 8=761/0-3-8
Max Horz 2=-93(LC 7)
Max Uplift 2=-292(LC 6), 13=-449(LC 5), 8=-291(LC 7)
Max Grav 2=874(LC 10), 13=2344(LC 1), 8=793(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-17=-1224/300, 3-17=-1151/318, 3-18=-654/238, 18-19=-605/245, 4-19=-527/263, 4-20=-23/675, 5-20=-23/675, 5-21=-22/672, 6-21=-22/672, 6-22=-326/213, 22-23=-404/194, 7-23=-453/187, 7-24=-992/273, 8-24=-1063/255, 8-9=0/35
BOT CHORD 2-16=-178/1021, 15-16=-178/1021, 14-15=-91/548, 13-14=-91/548, 12-13=0/357, 11-12=0/357, 10-11=-86/881, 8-10=-86/881
WEBS 3-16=0/223, 3-15=-557/242, 4-15=-26/549, 4-13=-1380/392, 5-13=-531/378, 6-13=-1213/348, 6-11=-45/523, 7-11=-613/250, 7-10=0/241

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 7-2-6, Exterior(2) 7-2-6 to 11-0-0, Interior(1) 14-9-10 to 23-2-6, Exterior(2) 27-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 449 lb uplift at joint 13 and 291 lb uplift at joint 8.

LOAD CASE(S) Standard

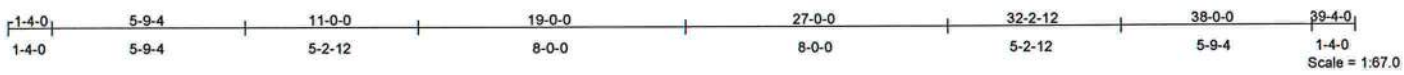
Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert #6634

August 26,2008

MiTek
POWER TO PERFORM.
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424967
11783	A05A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:57:58 2008 Page 1



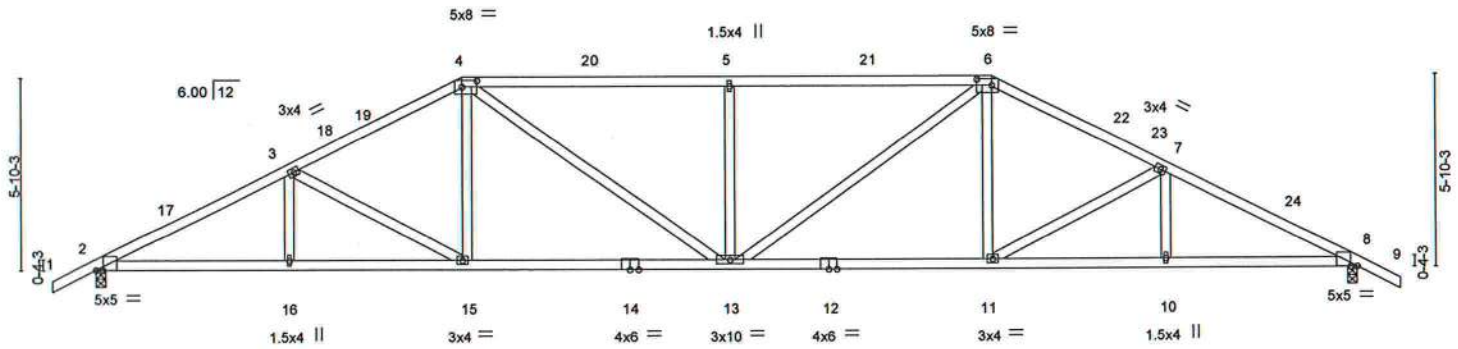


Plate Offsets (X,Y): [2:0-2-8,0-0-2], [4:0-5-8,0-2-4], [6:0-5-8,0-2-4], [8:0-2-8,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.29	13-15	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.61	13-15	>743	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.19	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.22	13	>999	240	Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

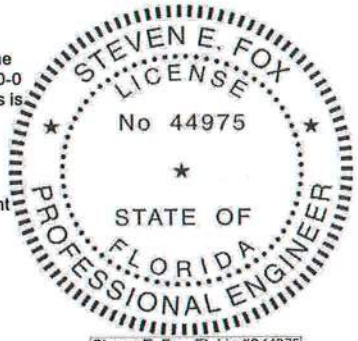
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-7 oc bracing.

REACTIONS (lb/size) 2=1974/0-3-8, 8=1974/0-3-8
Max Horz 2=-93(LC 7)
Max Uplift 2=-474(LC 6), 8=-474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-17=-3597/958, 3-17=-3523/976, 3-18=-3054/906, 18-19=-3005/913, 4-19=-2928/931, 4-20=-3288/1049, 5-20=-3288/1049, 5-21=-3288/1049, 6-21=-3288/1049, 6-22=-2928/931, 22-23=-3005/913, 7-23=-3054/906, 7-24=-3523/976, 8-24=-3597/958, 8-9=0/35
BOT CHORD 2-16=-709/3126, 15-16=-709/3126, 14-15=-550/2690, 13-14=-550/2690, 12-13=-550/2690, 11-12=-550/2690, 10-11=-709/3126, 8-10=-709/3126
WEBS 3-16=0/218, 3-15=-516/234, 4-15=-32/519, 4-13=-299/845, 5-13=-507/371, 6-13=-299/845, 6-11=-32/519, 7-11=-516/235, 7-10=0/218

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 7-2-6, Exterior(2) 7-2-6 to 11-0-0, Interior(1) 14-9-10 to 23-2-6, Exterior(2) 27-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 8.

LOAD CASE(S) Standard

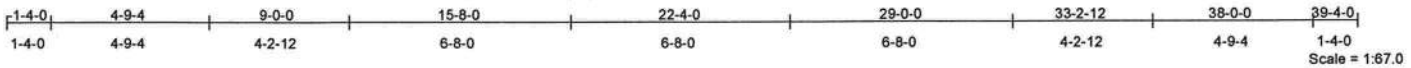


Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424968
11783	A06	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:00 2008 Page 1



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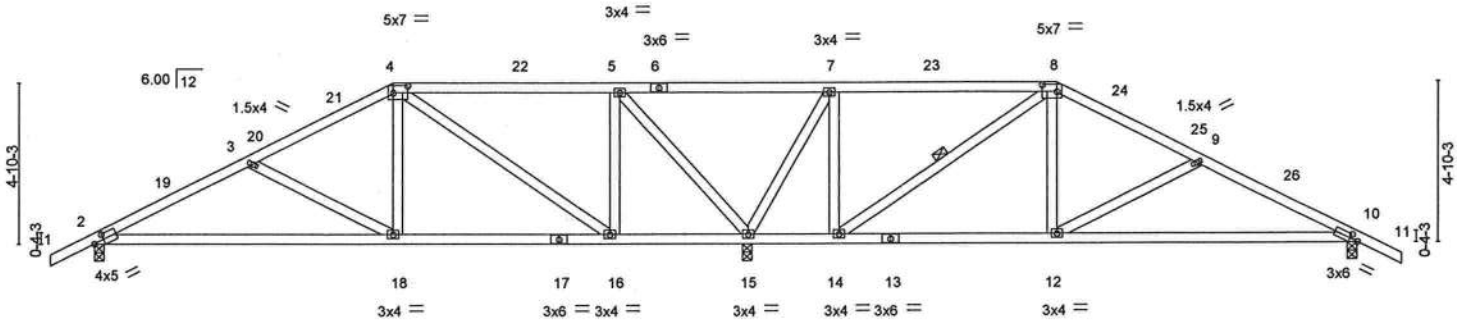


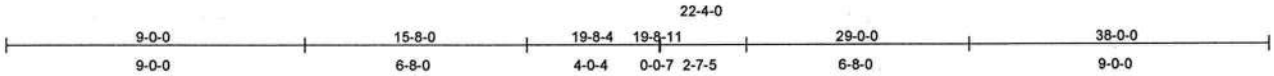


Plate Offsets (X,Y): [2:0-3-10,0-2-0], [4:0-5-4,0-2-8], [8:0-5-4,0-2-8], [10:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.15	2-18	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.37	2-18	>642	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.04	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.03	2-18	>999	240	Weight: 199 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 14-15.
WEBS 1 Row at midpt 8-14

REACTIONS (lb/size) 2=873/0-3-8, 15=2290/0-3-8, 10=785/0-3-8
Max Horz 2=-93(LC 7)
Max Uplift 2=-293(LC 6), 15=-538(LC 5), 10=-291(LC 7)
Max Grav 2=893(LC 10), 15=2290(LC 1), 10=807(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-19=-1207/370, 3-19=-1154/392, 3-20=-914/275, 20-21=-898/283, 4-21=-808/296, 4-22=-148/143, 5-22=-148/143, 5-6=-132/853, 6-7=-132/853, 7-23=0/219, 8-23=0/219, 8-24=-622/244, 24-25=-712/231, 9-25=-728/223, 9-26=-972/341, 10-26=-1024/319, 10-11=0/35
BOT CHORD 2-18=-241/1044, 17-18=-117/784, 16-17=-117/784, 15-16=-28/183, 14-15=-219/249, 13-14=-22/617, 12-13=-22/617, 10-12=-155/883
WEBS 3-18=-313/242, 4-18=-9/491, 4-16=-803/224, 5-16=-64/638, 5-15=-1406/414, 7-15=-1276/392, 7-14=-95/692, 8-14=-960/264, 8-12=-8/497, 9-12=-319/243

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 5-2-6, Exterior(2) 5-2-6 to 9-0-0, Interior(1) 12-9-10 to 25-2-6, Exterior(2) 29-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 293 lb uplift at joint 2, 538 lb uplift at joint 15 and 291 lb uplift at joint 10.

LOAD CASE(S) Standard

Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424969
11783	A06A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:01 2008 Page 1

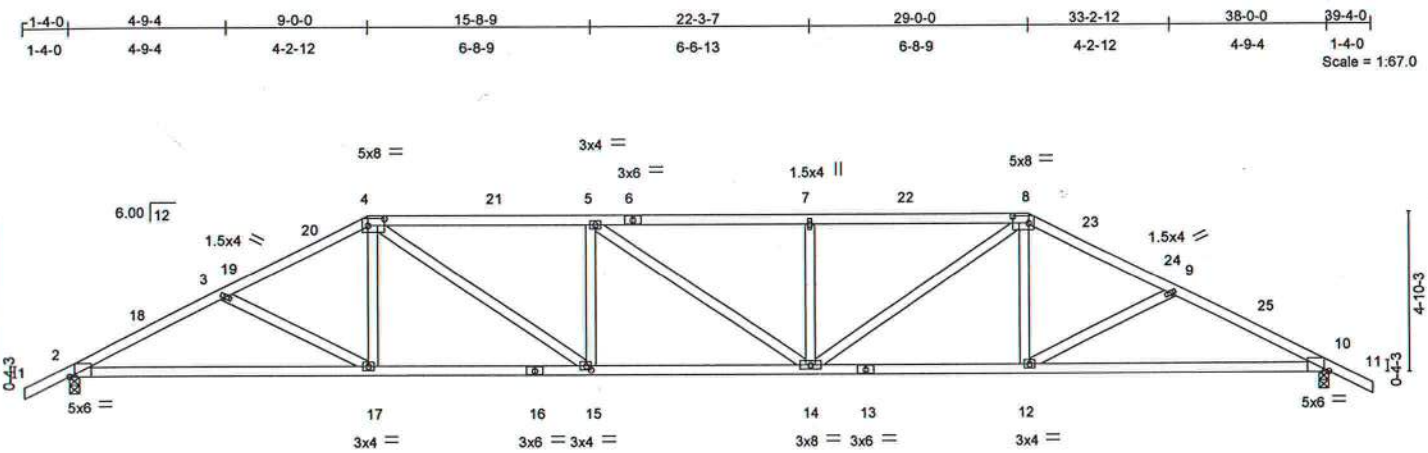


Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-6-0,0-2-8], [8:0-6-0,0-2-8], [10:0-1-11,Edge], [15:0-1-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.35	14-15	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.71	14-15	>642	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.20	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.27	14-15	>999	240	Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

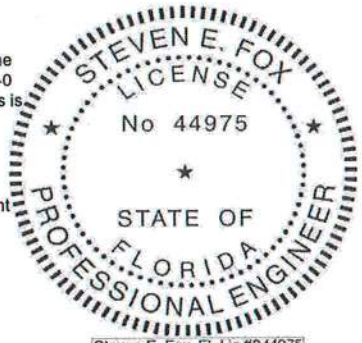
BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1974/0-3-8, 10=1974/0-3-8
Max Horz 2=-93(LC 7)
Max Uplift 2=-474(LC 6), 10=-474(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-18=-3515/1012, 3-18=-3463/1034, 3-19=-3267/930, 19-20=-3251/937, 4-20=-3161/951, 4-21=-3840/1162, 5-21=-3839/1162, 5-6=-3839/1162, 6-7=-3839/1162, 7-22=-3839/1161, 8-22=-3839/1161, 8-23=-3161/951, 23-24=-3251/937, 9-24=-3267/930, 9-25=-3463/1034, 10-25=-3515/1012, 10-11=0/35
BOT CHORD 2-17=-767/3084, 16-17=-623/2890, 15-16=-623/2890, 14-15=-905/3839, 13-14=-603/2890, 12-13=-603/2890, 10-12=-767/3084
WEBS 3-17=-238/228, 4-17=-5/449, 4-15=-431/1229, 5-15=-445/315, 5-14=-69/68, 7-14=-406/292, 8-14=-430/1228, 8-12=-6/449, 9-12=-238/229

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 2-5-10, Interior(1) 2-5-10 to 5-2-6, Exterior(2) 5-2-6 to 9-0-0, Interior(1) 12-9-10 to 25-2-6, Exterior(2) 29-0-0 to 39-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 474 lb uplift at joint 10.

LOAD CASE(S) Standard



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August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424970
11783	A07	ROOF TRUSS	1	2	Job Reference (optional)	

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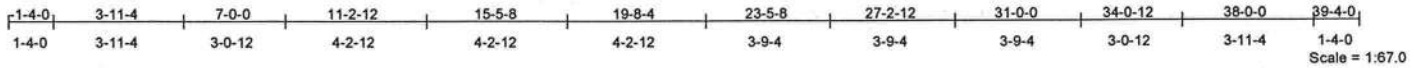

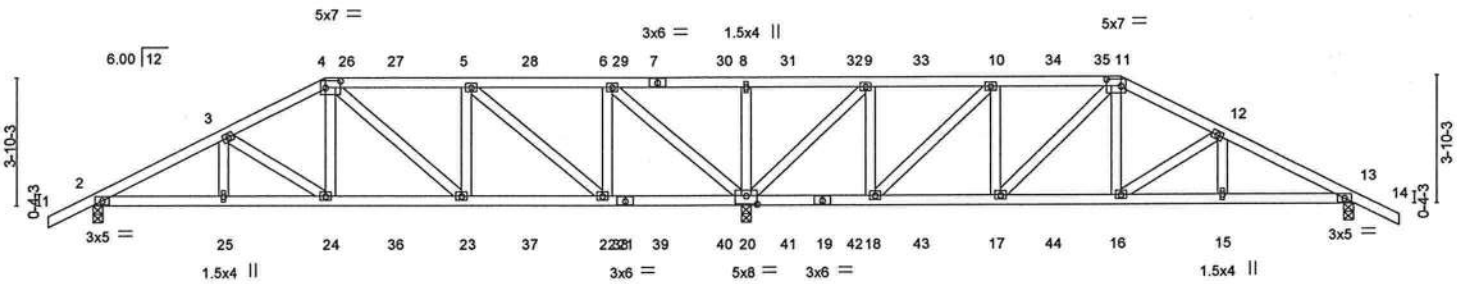

TRUSS NOT SYMMETRIC
DO NOT ERECT THE TRUSS BACKWARDS

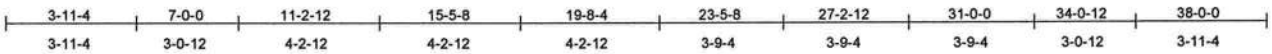

Plate Offsets (X,Y): [4:0-5-4,0-2-8], [11:0-5-4,0-2-8], [20:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.05	23-24	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.11	23-24	>999	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.06	13	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.05	24	>999	240	Weight: 427 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 18-20.

REACTIONS (lb/size) 2=1707/0-3-8, 20=5626/0-3-8, 13=1507/0-3-8
Max Horz 2=-88(LC 6)
Max Uplift 2=-836(LC 5), 20=-2512(LC 4), 13=-759(LC 6)
Max Grav 2=1719(LC 9), 20=5626(LC 1), 13=1520(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3097/1418, 3-4=-2822/1421, 4-26=-1911/932, 26-27=-1910/932, 5-27=-1910/932, 5-28=-150/98, 28-29=-150/98, 6-29=-150/98, 6-7=-1188/2715, 7-30=-1188/2715, 8-30=-1188/2715, 8-31=-1188/2715, 31-32=-1188/2715, 9-32=-1188/2715, 9-33=-109/291, 10-33=-109/291, 10-34=-1410/749, 34-35=-1411/749, 11-35=-1411/749, 11-12=-2394/1255, 12-13=-2682/1258, 13-14=0/35
BOT CHORD 2-25=-1221/2685, 24-25=-1221/2685, 24-36=-1209/2560, 23-36=-1209/2560, 23-37=-850/1910, 37-38=-850/1910, 22-38=-850/1910, 21-22=-11/150, 21-39=-11/150, 39-40=-11/150, 20-40=-11/150, 20-41=-291/281, 19-41=-291/281, 19-42=-291/281, 18-42=-291/281, 18-43=-595/1411, 17-43=-595/1411, 17-44=-990/2167, 16-44=-990/2167, 15-16=-992/2320, 13-15=-992/2320
WEBS 3-25=0/121, 3-24=-215/75, 4-24=-653/1363, 4-23=-867/539, 5-23=-300/944, 5-22=-2319/1104, 6-22=-646/1838, 6-20=-3677/1675, 8-20=-555/439, 9-20=-3334/1518, 9-18=-683/1849, 10-18=-2285/1082, 10-17=-374/1075, 11-17=-1057/604, 11-16=-660/1360, 12-16=-247/87, 12-15=0/129

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All plates are 3x4 MT20 unless otherwise indicated.
8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 836 lb uplift at joint 2, 2512 lb uplift at joint 20 and 759 lb uplift at joint 13.

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August 26,2008

Continued on page 2

NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 134 lb down and 152 lb up at 7-0-0, 134 lb down and 152 lb up at 9-0-12, 134 lb down and 152 lb up at 11-0-12, 134 lb down and 152 lb up at 13-0-12, 134 lb down and 152 lb up at 15-0-12, 134 lb down and 152 lb up at 17-0-12, 134 lb down and 152 lb up at 19-0-0, 134 lb down and 152 lb up at 20-11-4, 134 lb down and 152 lb up at 22-11-4, 134 lb down and 152 lb up at 24-11-4, 134 lb down and 152 lb up at 26-11-4, and 134 lb down and 152 lb up at 28-11-4, and 134 lb down and 152 lb up at 31-0-0 on top chord, and 1048 lb down and 653 lb up at 7-0-0, 96 lb down at 9-0-12, 96 lb down at 11-0-12, 96 lb down at 13-0-12, 96 lb down at 15-0-12, 96 lb down at 17-0-12, 96 lb down at 19-0-0, 96 lb down at 20-11-4, 96 lb down at 22-11-4, 96 lb down at 24-11-4, 96 lb down at 26-11-4, and 96 lb down at 28-11-4, and 1048 lb down and 653 lb up at 30-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-11=-60, 11-14=-60, 2-13=-40

Concentrated Loads (lb)

Vert: 4=-134(F) 7=-134(F) 11=-134(F) 24=-1048(F) 23=-96(F) 5=-134(F) 10=-134(F) 17=-96(F) 16=-1048(F) 27=-134(F) 28=-134(F) 29=-134(F) 30=-134(F) 31=-134(F) 32=-134(F) 33=-134(F) 34=-134(F) 36=-96(F) 37=-96(F) 38=-96(F) 39=-96(F) 40=-96(F) 41=-96(F) 42=-96(F) 43=-96(F) 44=-96(F)

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	
11783	A07A	ROOF TRUSS	1	2	Job Reference (optional)	I14424971

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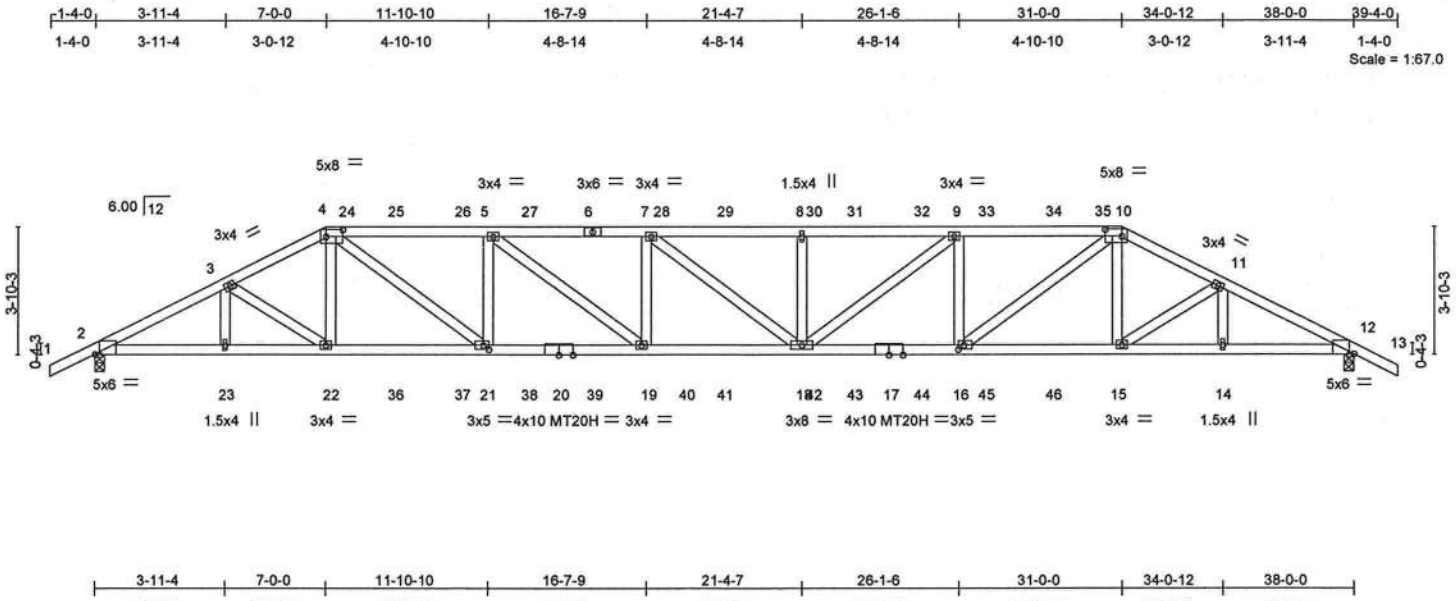

Plate Offsets (X,Y): [2:0-1-10,Edge], [4:0-6-0,0-2-8], [10:0-6-0,0-2-8], [12:0-1-10,Edge], [16:0-2-0,0-1-8], [21:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.51	18-19	>884	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-1.02	18-19	>445	240	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.23	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.53	18-19	>859	240	Weight: 412 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-12 oc bracing.

REACTIONS (lb/size) 2=4421/0-3-8, 12=4420/0-3-8
Max Horz 2=-88(LC 6)
Max Uplift 2=-2011(LC 5), 12=-2010(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-8761/3879, 3-4=-8695/4012, 4-24=-10618/4828, 24-25=-10618/4828, 25-26=-10618/4827, 5-26=-10617/4827, 5-27=-11981/5388, 6-27=-11981/5388, 6-7=-11981/5388, 7-28=-11972/5383, 28-29=-11972/5383, 29-30=-11972/5383, 8-30=-11972/5383, 8-31=-11972/5383, 31-32=-11972/5383, 9-32=-11972/5383, 9-33=-10618/4827, 33-34=-10618/4827, 34-35=-10618/4827, 10-35=-10619/4827, 10-11=-8694/4011, 11-12=-8760/3879, 12-13=0/35
BOT CHORD 2-23=-3414/7680, 22-23=-3414/7680, 22-36=-3568/7822, 36-37=-3568/7822, 21-37=-3568/7822, 21-38=-4745/10618, 20-38=-4745/10618, 20-39=-4745/10618, 19-39=-4745/10618, 19-40=-5306/11981, 40-41=-5306/11981, 41-42=-5306/11981, 18-42=-5306/11981, 18-43=-4692/10618, 17-43=-4692/10618, 17-44=-4692/10618, 16-44=-4692/10618, 16-45=-3513/7821, 45-46=-3513/7821, 15-46=-3513/7821, 14-15=-3360/7680, 12-14=-3360/7680
WEBS 3-23=0/87, 3-22=-250/242, 4-22=-603/1304, 4-21=-1506/3502, 5-21=-1643/951, 5-19=-747/1742, 7-19=-623/502, 7-18=-50/29, 8-18=-599/488, 9-18=-741/1731, 9-16=-1638/948, 10-16=-1506/3504, 10-15=-603/1302, 11-15=-250/241, 11-14=0/87

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2011 lb uplift at joint 2 and 2010 lb uplift at joint 12.

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August 26,2008

Continued on page 2

NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 134 lb down and 152 lb up at 7-0-0, 134 lb down and 152 lb up at 9-0-12, 134 lb down and 152 lb up at 11-0-12, 134 lb down and 152 lb up at 13-0-12, 134 lb down and 152 lb up at 15-0-12, 134 lb down and 152 lb up at 17-0-12, 134 lb down and 152 lb up at 19-0-0, 134 lb down and 152 lb up at 20-11-4, 134 lb down and 152 lb up at 22-11-4, 134 lb down and 152 lb up at 24-11-4, 134 lb down and 152 lb up at 26-11-4, and 134 lb down and 152 lb up at 28-11-4, and 134 lb down and 152 lb up at 31-0-0 on top chord, and 1048 lb down and 653 lb up at 7-0-0, 96 lb down at 9-0-12, 96 lb down at 11-0-12, 96 lb down at 13-0-12, 96 lb down at 15-0-12, 96 lb down at 17-0-12, 96 lb down at 19-0-0, 96 lb down at 20-11-4, 96 lb down at 22-11-4, 96 lb down at 24-11-4, 96 lb down at 26-11-4, and 96 lb down at 28-11-4, and 1048 lb down and 653 lb up at 30-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-10=-60, 10-13=-60, 2-12=-40

Concentrated Loads (lb)

Vert: 4=-134(F) 6=-134(F) 10=-134(F) 22=-1048(F) 15=-1048(F) 25=-134(F) 26=-134(F) 27=-134(F) 28=-134(F) 29=-134(F) 30=-134(F) 31=-134(F) 32=-134(F) 33=-134(F) 34=-134(F) 36=-96(F) 37=-96(F) 38=-96(F) 39=-96(F) 40=-96(F) 41=-96(F) 42=-96(F) 43=-96(F) 44=-96(F) 45=-96(F) 46=-96(F)

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424972
11783	B01	ROOF TRUSS	3	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:05 2008 Page 1

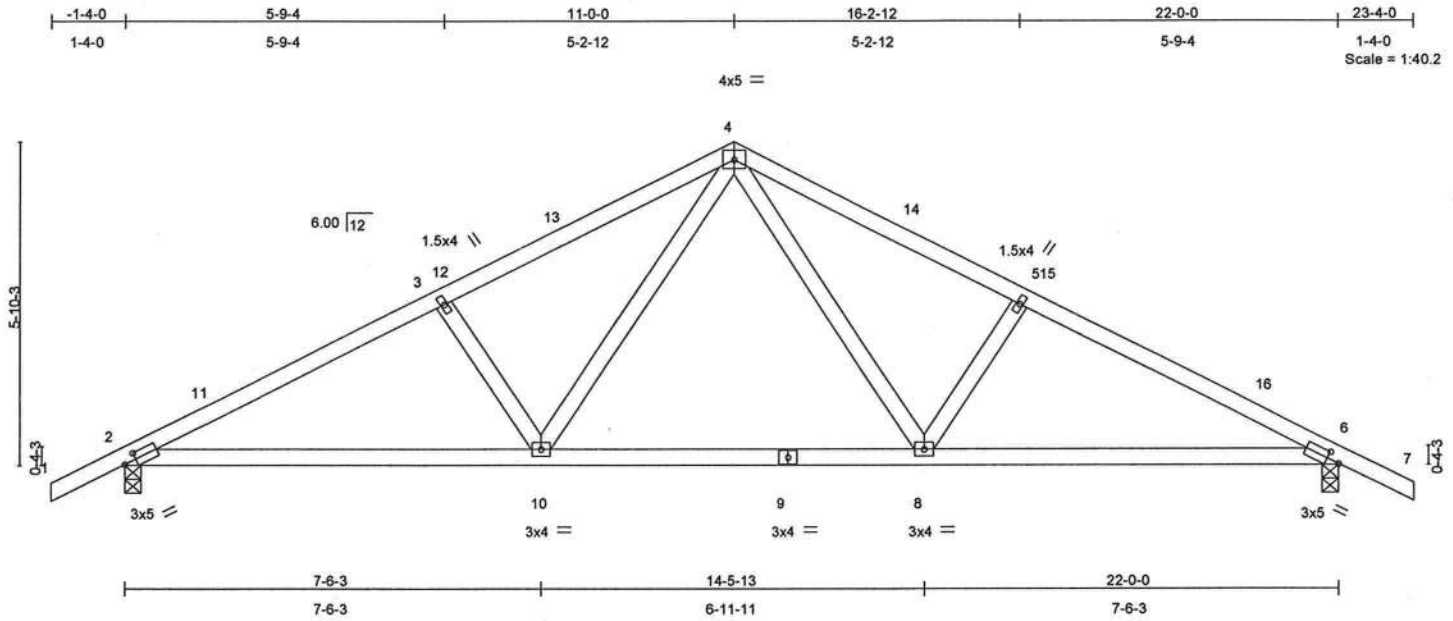


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.11	2-10	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.25	2-10	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(TL)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.05	8-10	>999	240	Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1174/0-3-8, 6=1174/0-3-8
Max Horz 2=-88(LC 4)
Max Uplift 2=-334(LC 6), 6=-334(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-11=-1820/447, 3-11=-1772/475, 3-12=-1654/456, 12-13=-1646/468, 4-13=-1580/484, 4-14=-1580/484, 14-15=-1646/468, 5-15=-1654/456, 5-16=-1772/475, 6-16=-1820/447, 6-7=0/35
BOT CHORD 2-10=-269/1572, 9-10=-108/1064, 8-9=-108/1064, 6-8=-269/1572
WEBS 3-10=-313/228, 4-10=-128/672, 4-8=-129/672, 5-8=-313/228

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 14-0-0 to 20-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 334 lb uplift at joint 2 and 334 lb uplift at joint 6.

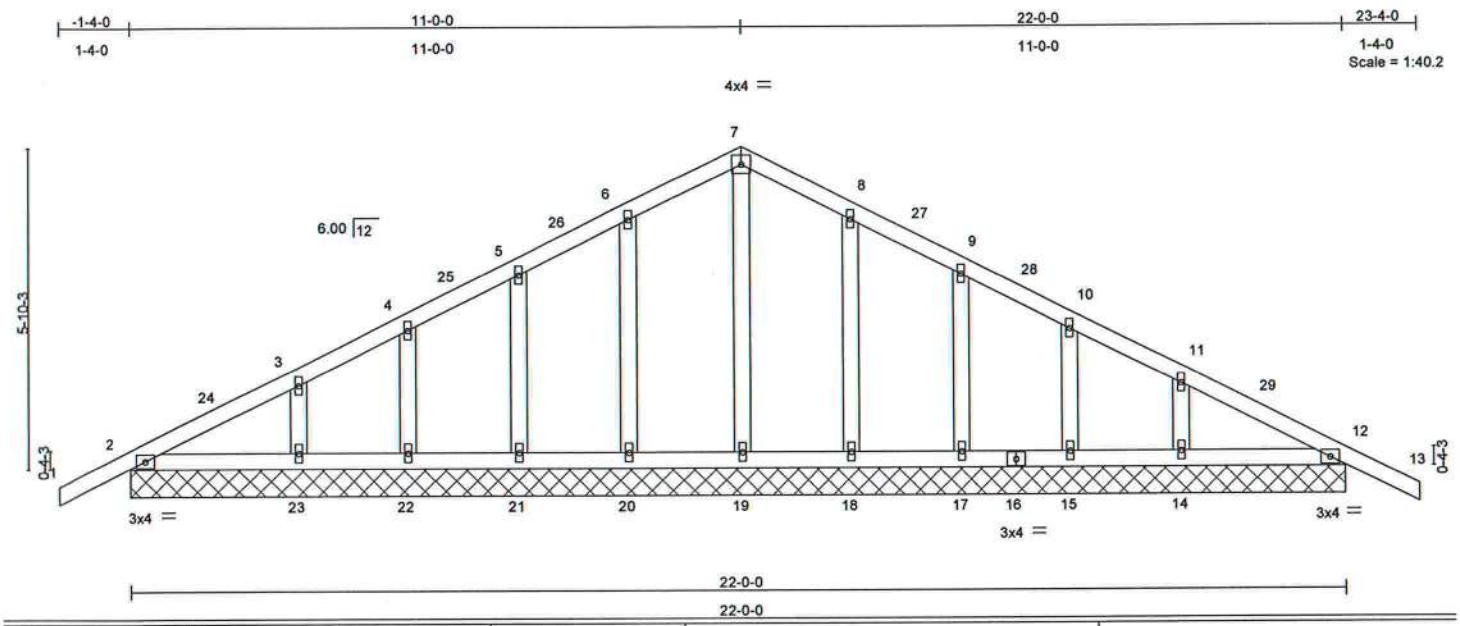
LOAD CASE(S) Standard

Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424973
11783	B01A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:07 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	13	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	13	n/r	120		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.01	12	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

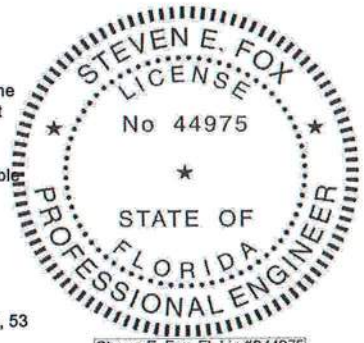
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=232/22-0-0, 19=188/22-0-0, 20=198/22-0-0, 21=207/22-0-0, 22=174/22-0-0, 23=275/22-0-0, 18=198/22-0-0, 17=207/22-0-0, 15=174/22-0-0, 14=275/22-0-0, 12=232/22-0-0
Max Horz 2=-88(LC 4)
Max Uplift 2=-118(LC 6), 20=-40(LC 6), 21=-53(LC 6), 22=-86(LC 6), 23=-94(LC 6), 18=-40(LC 6), 17=-54(LC 7), 15=-86(LC 7), 14=-94(LC 7), 12=-133(LC 7)
Max Grav 2=232(LC 1), 19=188(LC 1), 20=203(LC 10), 21=207(LC 1), 22=174(LC 10), 23=275(LC 10), 18=203(LC 11), 17=207(LC 1), 15=174(LC 11), 14=275(LC 11), 12=232(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-24=-94/40, 3-24=-84/53, 3-4=-46/70, 4-25=-40/125, 5-25=-13/130, 5-26=-41/200, 6-26=-14/206, 6-7=-41/302, 7-8=-41/302, 8-27=-14/206, 9-27=-41/200, 9-28=-13/130, 10-28=-40/125, 10-11=-46/70, 11-29=-47/16, 12-29=-57/3, 12-13=0/34
BOT CHORD 2-23=0/169, 22-23=0/169, 21-22=0/169, 20-21=0/169, 19-20=0/169, 18-19=0/169, 17-18=0/169, 16-17=0/169, 15-16=0/169, 14-15=0/169, 12-14=0/169
WEBS 7-19=-108/0, 6-20=-124/191, 5-21=-123/144, 4-22=-106/111, 3-23=-162/171, 8-18=-124/191, 9-17=-123/144, 10-15=-106/111, 11-14=-162/171

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Corner(3) -1-4-0 to 1-8-0, Exterior(2) 1-8-0 to 8-0-0, Corner(3) 8-0-0 to 11-0-0, Exterior(2) 14-0-0 to 20-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 2, 40 lb uplift at joint 20, 53 lb uplift at joint 21, 86 lb uplift at joint 22, 94 lb uplift at joint 23, 40 lb uplift at joint 18, 54 lb uplift at joint 17, 86 lb uplift at joint 15, 94 lb uplift at joint 14 and 133 lb uplift at joint 12.

LOAD CASE(S) Standard



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August 26,2008

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Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424974
11783	C01	ROOF TRUSS	4	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:08 2008 Page 1

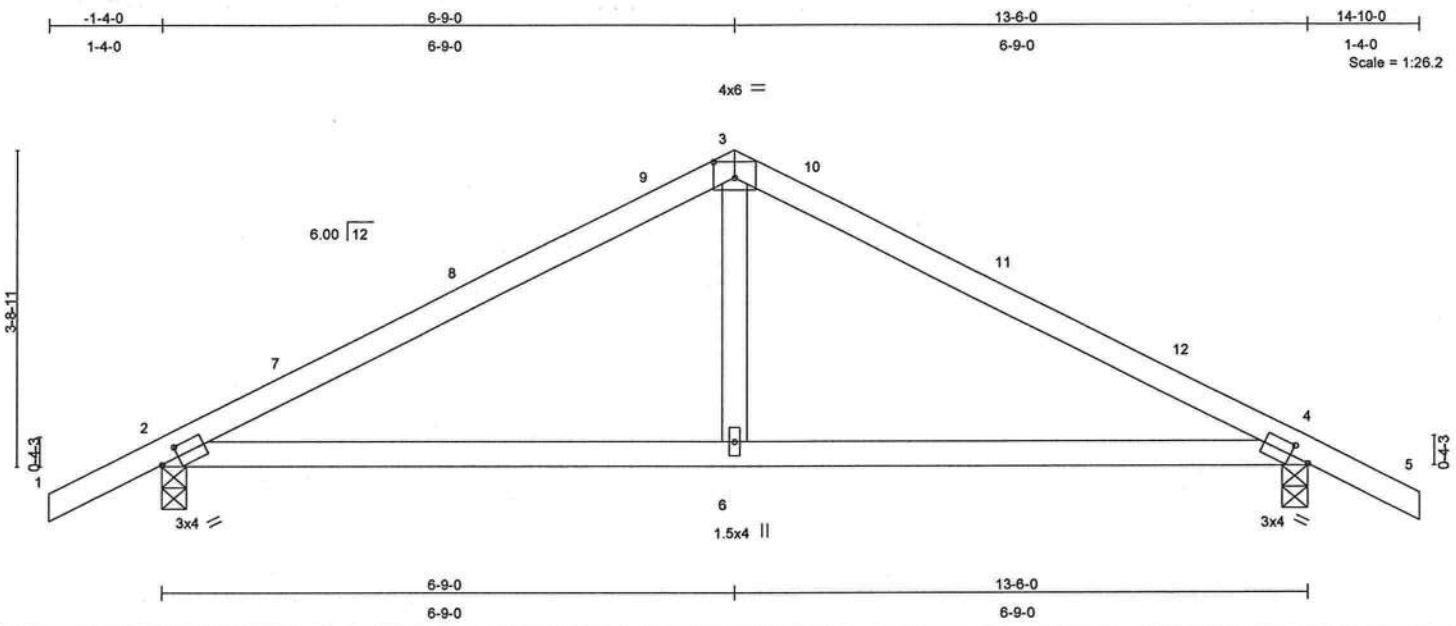

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-3-0,0-2-4], [4:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.07	2-6	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.15	2-6	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.02	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.04	2-6	>999	240	Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=749/0-3-8, 4=749/0-3-8
Max Horz 2=-80(LC 7)
Max Uplift 2=-264(LC 6), 4=-264(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-7=-936/271, 7-8=-860/286, 8-9=-837/287, 3-9=-773/304, 3-10=-773/304, 10-11=-837/287, 11-12=-860/286, 4-12=-936/271, 4-5=0/35
BOT CHORD 2-6=-99/764, 4-6=-99/764
WEBS 3-6=0/331

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 3-9-0, Exterior(2) 3-9-0 to 6-9-0, Interior(1) 9-9-0 to 11-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 2 and 264 lb uplift at joint 4.

LOAD CASE(S) Standard

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August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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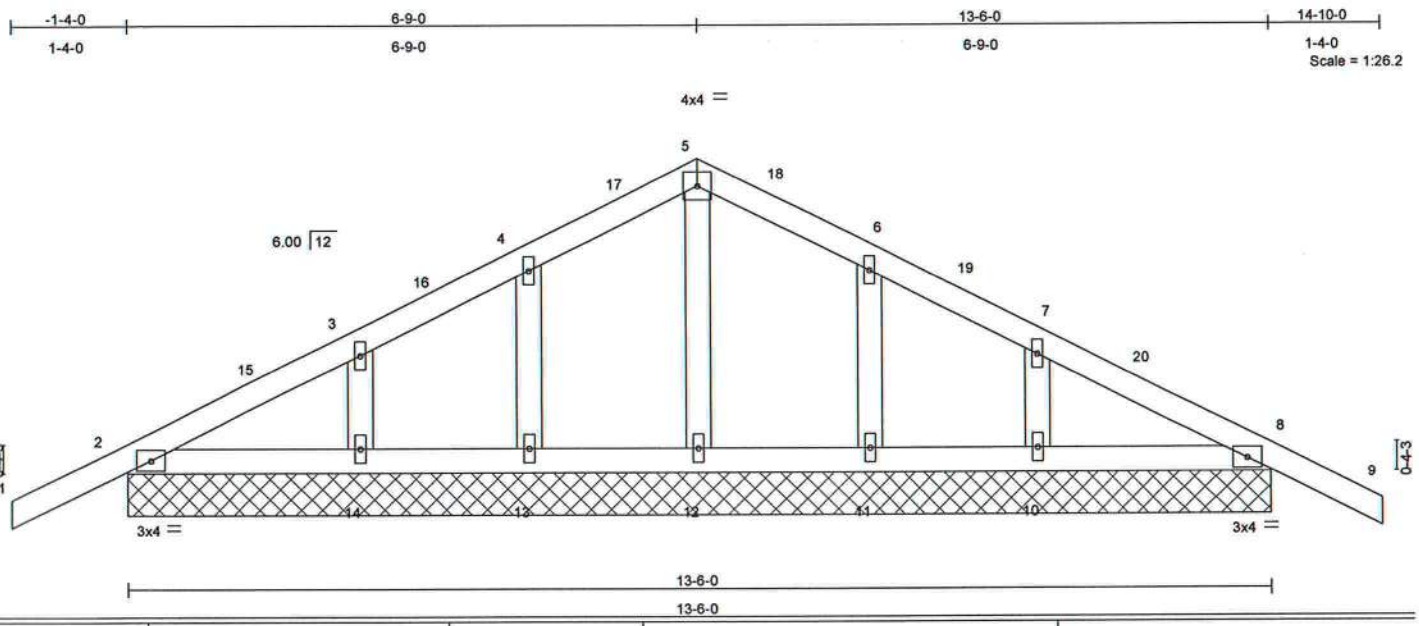
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424975
11783	C01A	ROOF TRUSS	1	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:09 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	9	n/r	120		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 62 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

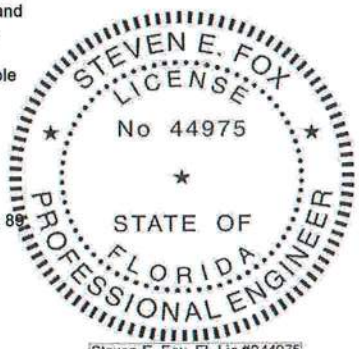
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=224/13-6-0, 8=224/13-6-0, 12=195/13-6-0, 13=183/13-6-0, 14=251/13-6-0, 11=183/13-6-0, 10=251/13-6-0
Max Horz 2=-80(LC 7)
Max Uplift 2=-132(LC 6), 8=-147(LC 7), 13=-89(LC 6), 14=-82(LC 6), 11=-88(LC 7), 10=-82(LC 7)
Max Grav 2=224(LC 1), 8=224(LC 1), 12=195(LC 1), 13=188(LC 10), 14=251(LC 1), 11=188(LC 11), 10=251(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-15=-53/32, 3-15=-36/44, 3-16=-45/111, 4-16=-10/121, 4-17=-41/218, 5-17=-4/224, 5-18=-4/224, 6-18=-41/218, 6-19=-10/121, 7-19=-45/111, 7-20=-15/28, 8-20=-53/15, 8-9=0/34
BOT CHORD 2-14=0/135, 13-14=0/135, 12-13=0/135, 11-12=0/135, 10-11=0/135, 8-10=0/135
WEBS 5-12=-111/0, 4-13=-116/206, 3-14=-149/181, 6-11=-116/206, 7-10=-149/181

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Corner(3) -1-4-0 to 1-8-0, Exterior(2) 1-8-0 to 3-9-0, Corner(3) 3-9-0 to 6-9-0, Exterior(2) 9-9-0 to 11-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 2, 147 lb uplift at joint 8, 89 lb uplift at joint 13, 82 lb uplift at joint 14, 88 lb uplift at joint 11 and 82 lb uplift at joint 10.

LOAD CASE(S) Standard



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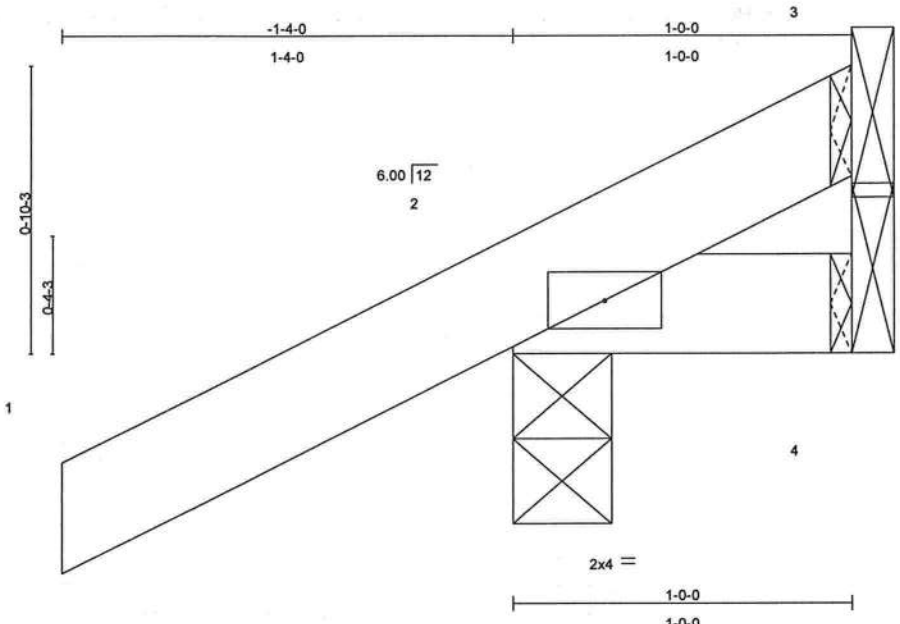
August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424976
11783	J01	ROOF TRUSS	8	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:10 2008 Page 1



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LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	2	****	240	Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=188/0-3-8, 4=19/Mechanical, 3=-29/Mechanical
Max Horz 2=77(LC 6)
Max Uplift 2=-174(LC 6), 3=-29(LC 1)
Max Grav 2=188(LC 1), 4=19(LC 1), 3=50(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-42/27
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 2 and 29 lb uplift at joint 3.

LOAD CASE(S) Standard

STEVEN E. FOX LICENSE No 44975 STATE OF FLORIDA PROFESSIONAL ENGINEER

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MiTek Industries, Inc.
14515 North Outer Forty Drive
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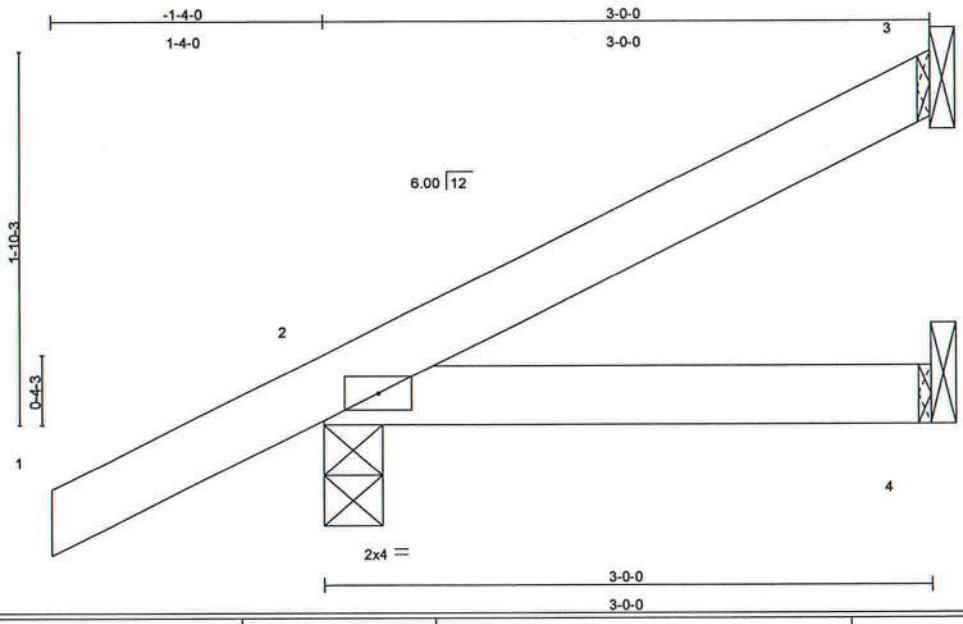
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424977
11783	J03	ROOF TRUSS	8	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:10 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	2	****	240	Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=60/Mechanical, 2=252/0-3-8, 4=56/Mechanical
Max Horz 2=131(LC 6)
Max Uplift 3=-51(LC 6), 2=-148(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-60/21
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 3 and 148 lb uplift at joint 2.

LOAD CASE(S) Standard

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August 26,2008

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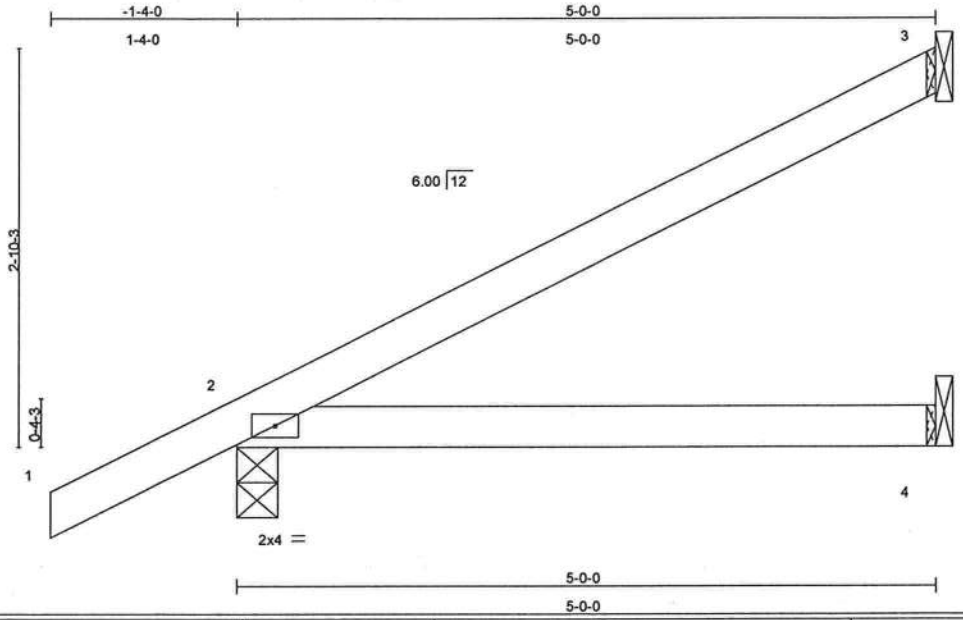
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	
11783	J05	ROOF TRUSS	8	1	Job Reference (optional)	I14424978

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:11 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.07	2-4	>831	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	2	****	240	Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=130/Mechanical, 2=342/0-3-8, 4=96/Mechanical
Max Horz 2=186(LC 6)
Max Uplift 3=-121(LC 6), 2=-152(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-119/47
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 3 and 152 lb uplift at joint 2.

LOAD CASE(S) Standard

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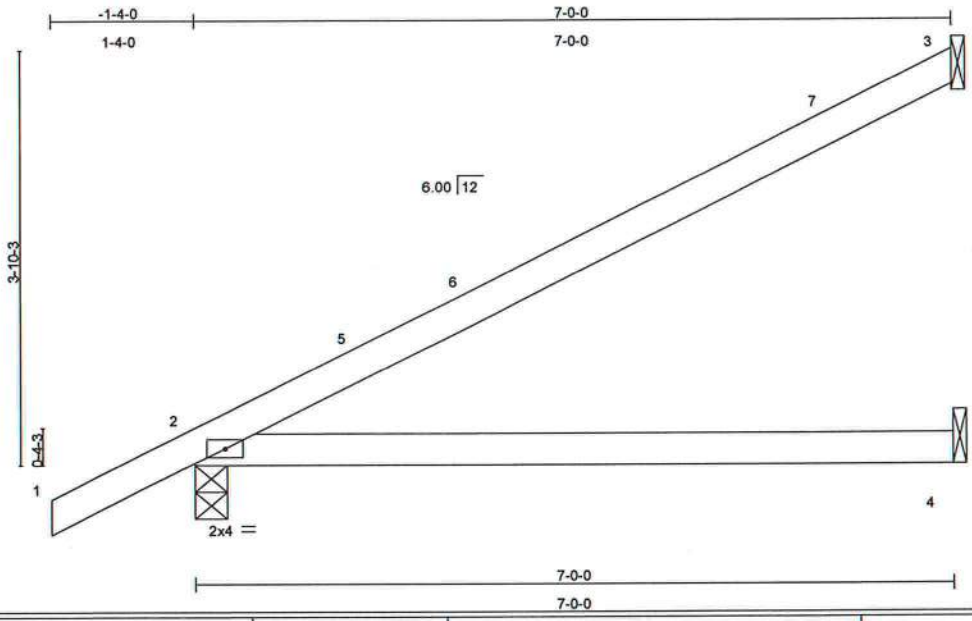
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	I14424979
11783	J07	ROOF TRUSS	26	1	Job Reference (optional)	

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:11 2008 Page 1



Scale = 1:20.5

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.11	2-4	>730	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.28	2-4	>292	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.00	2	****	240	Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=194/Mechanical, 2=438/0-3-8, 4=136/Mechanical
Max Horz 2=233(LC 6)
Max Uplift 3=-164(LC 6), 2=-167(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-5=-159/0, 5-6=-145/0, 6-7=-139/0, 3-7=-127/70
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 2-8-5, Exterior(2) 2-8-5 to 6-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 3 and 167 lb uplift at joint 2.

LOAD CASE(S) Standard

STEVEN E. FOX LICENSE No 44975 STATE OF FLORIDA PROFESSIONAL ENGINEER

Steven E. Fox, FL Lic #044975
MiTek Industries, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017
FL Cert.#6634

August 26,2008

Job	Truss	Truss Type	Qty	Ply	Amelia Jewel; L9 Meadowlands	
11783	JC09	ROOF TRUSS	4	1	Job Reference (optional)	I14424980

Builders Truss Mfg., Woodbine, GA — 6.500 s Aug 27 2007 MiTek Industries, Inc. Tue Aug 26 14:58:13 2008 Page 1

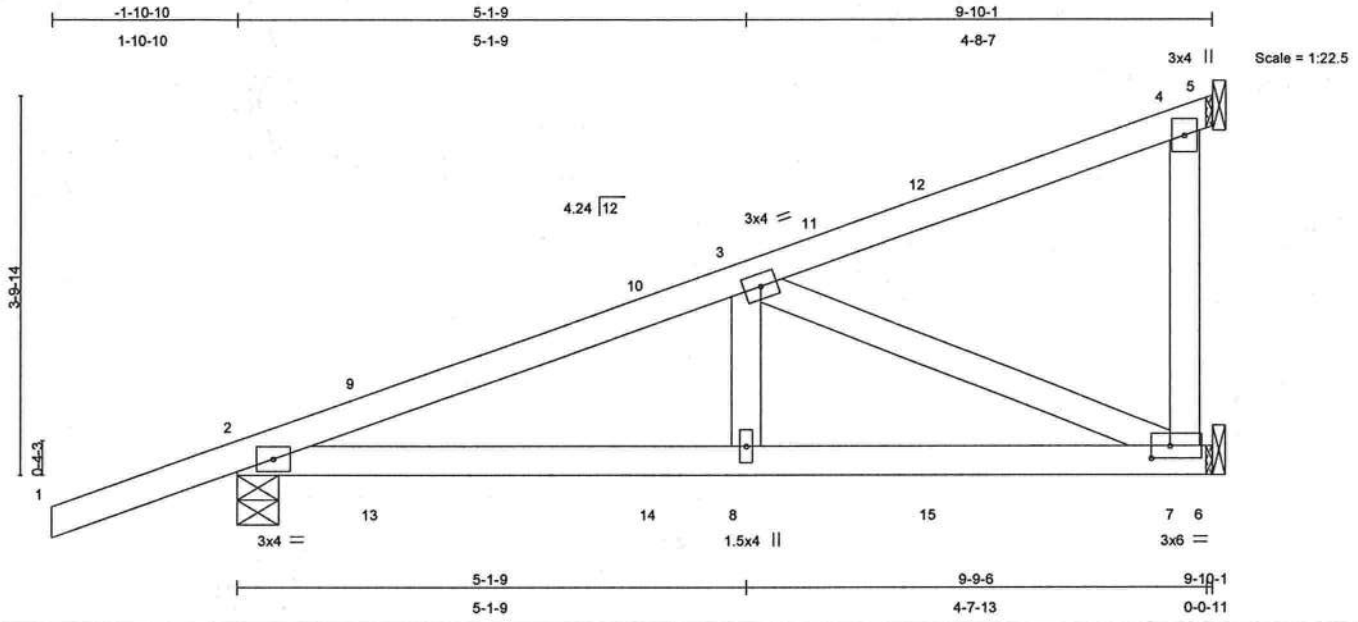


Plate Offsets (X,Y): [7:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.06	7-8	>999	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.01	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.02	7-8	>999	240	Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=-332/Mechanical, 7=992/Mechanical, 2=644/0-4-15
Max Horz 2=228(LC 4)
Max Uplift 5=-332(LC 1), 7=-641(LC 4), 2=-249(LC 5)
Max Grav 5=381(LC 4), 7=992(LC 1), 2=644(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-9=-954/241, 9-10=-905/263, 3-10=-863/245, 3-11=-125/80, 11-12=-86/86, 4-12=-85/26, 4-5=-115/127, 4-7=-521/542
BOT CHORD 2-13=-315/847, 13-14=-315/847, 8-14=-315/847, 8-15=-315/847, 7-15=-315/847, 6-7=0/0
WEBS 3-8=0/293, 3-7=-860/285

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 5, 641 lb uplift at joint 7 and 249 lb uplift at joint 2.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 35 lb up at 4-2-15, 35 lb up at 4-2-15, 67 lb down and 105 lb up at 7-0-14, 67 lb down and 105 lb up at 7-0-14, and 41 lb down at 1-4-15, and 41 lb down at 1-4-15 on top chord, and 21 lb up at 1-4-15, 21 lb up at 1-4-15, 16 lb down at 4-2-15, 16 lb down at 4-2-15, and 56 lb down at 7-0-14, and 56 lb down at 7-0-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-5=-60, 2-7=-40
Concentrated Loads (lb)
Vert: 10=8(F=4, B=4) 12=-133(F=-67, B=-67) 13=41(F=21, B=21) 14=-33(F=-16, B=-16) 15=-113(F=-56, B=-56)

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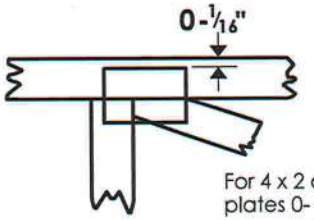
FL Cert.#6634

August 26,2008

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

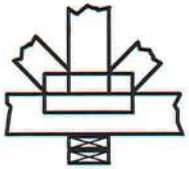
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



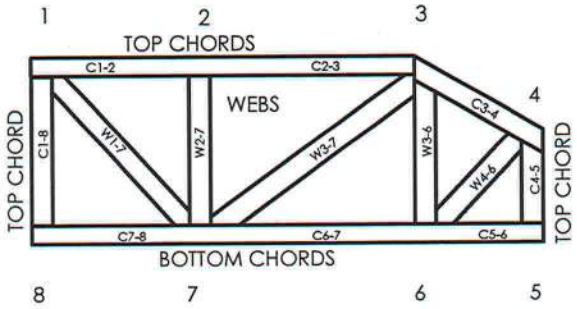
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

ADDITIONAL LAYOUT NOTES

READ AND COMPREHEND ALL DOCUMENTS FOUND IN JOB PACKET (PLASTIC BAG) BEFORE ERECTING ANY TRUSSES.

Verify that **dimensional information** shown on layout is correct before attempting to erect trusses. If not; contact manufacturer before proceeding.

Truss design is based on a **placement** of trusses which is consistent with the accompanying layout. Failure to erect trusses in compliance with this layout and the notes hereon may result in unsafe and libelous conditions. Call truss manufacturer before making any modifications.

Engineered wood trusses always require various forms of **temporary bracing** during installation to prevent toppling and an ensuing domino like effect. It is entirely and finally, the responsibility of the erection contractor. To determine the propriety of bracing to be used. The contractor should refer to "hib-91 summary sheet" (provided) for assistance. Specifying the erection bracing is beyond the scope responsibility of truss manufacturer.

To reduce the number of unique pieces, some short **jacks** may have been built the same as longer jacks. Field trim jacks to fit!!

When reading **dimensions** on papers relative to his job, always consider the length to have three (3) parts…feet, inches & sixteenths. Examples: 6'7"12 would mean, 6 ft, 7 and 12/16 inches, or 6 ft, 7 and ¾ inches. In case where there are no fractions of inches, only feet and whole inches are shown, rather than display zero sixteenths.

Bottom cord **fillers** (if required) have been designed to give builder maximum field flexibility. If filler is factory installed, square end target is the center of the wall. Builder must field install vertical scab to assure perfect alignment with partition wall. If filler is fabricated as a separate "frame," trusses should first be erected, then the high end of the filler should be aligned with the partition wall then "lap" nailed to the face of the trusses.

Unless specifically engineered, none of these trusses have been designed to carry additional hvac loads. It is the responsibility of the building contractor to provide any additional support required.

If girder(s) consisting of **more than one ply** are required, they have been delivered as individual trusses. It is the responsibility of the building contractor to fasten girder plies together as shown on the engineered drawing(s) which have been provided in the same packet as the layout.

If trusses have been delivered which require the attachment of **bearing block(s) and/or scab(s)**, it is the responsibility of the building contractor to attach them as shown on the engineered drawings which have been provided in the same packet as the layout.

If **valley trusses** have been provided, bottom chords **do not required beveling**! Refer to the various detail drawings and engineered drawings (provided) for proper installation.

If **conventional overframing** (valley or otherwise) is required, it is the responsibility contractor to distribute the overframing loads uniformly to the supporting trusses below. Refer to the detail drawing(s) provided for recommendations. Note that pre-cut conventional valleys (valley rafter sets [vrs]) Are conventional overframing.

Refer to engineered drawing (provided with layout) to determine if trusses have been delivered which are designed to **bear and/or support additional loads** at specific locations. Particular care is advised during installation to ensure that these trusses are erected properly. Shim all supports to solid bearing. Trusses may not be symmetrical.

How to read the... MiTek 20/20 Engineering Drawing

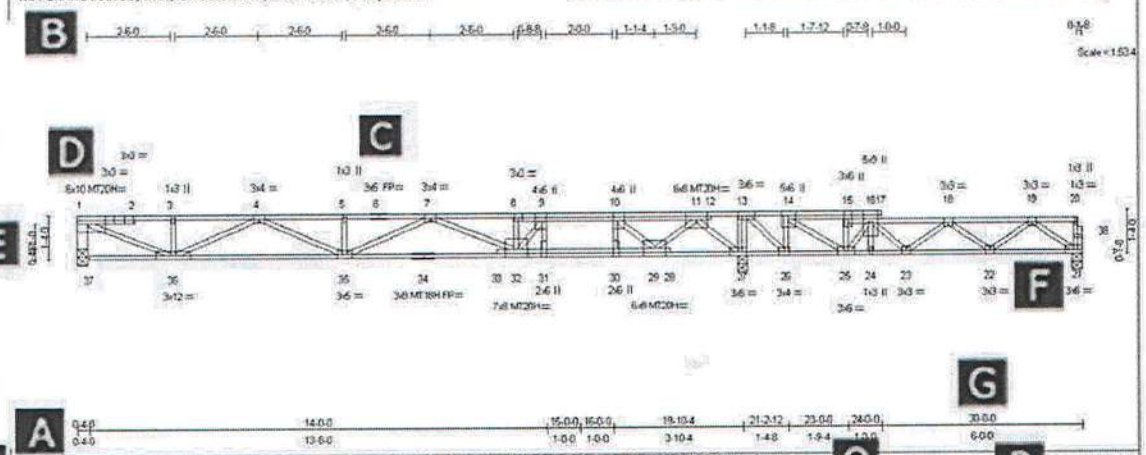
Floor Truss

- **A** Cumulative Dimensions
- **B** Panel Length (feet-inches-sixteenths)
- **C** Pre-splice face plate
- **D** Plate Size and Orientations
- **E** Truss Depth
- **F** Bearing Location
- **G** Truss Span (feet-inches-sixteenths)
- **H** Plate Offsets
- **I** Design Loading (PSF)
- **J** Spacing O.C. (feet-inches-sixteenths)
- **K** Duration of Load for Plate and Lumber Design
- **L** Code
- **M** Top Chord, Bottom Chord and Web Maximum Combined Stress Indicies
- **N** Deflections (inches) and Span to Deflection Ratio
- **O** Input Span to Deflection Ratio
- **P** MiTek Plate Allowables (PSI)
- **Q** Lumber Requirements
- **R** Reaction (pounds)
- **S** Maximum Bearing Required (inches)
- **T** Maximum Uplift and/or Horizontal Reaction if Applicable
- **U** Required Member Bracing
- **V** Member Axial Forces for Load Case 1
- **W** Notes
- **X** Additional Loads/Load Cases
- **Y** Truss Weight

MiTek Industries, Inc.
14515 North Outer forty Drive
Suite 300
Chesterfield, MO 63017-5746
(314)434-1200 Phone
www.mii.com

Job	Truss	Truss Type	Qty	Ply	MiTek Web Site
WEB SITE	FT-01	FLOOR	1	1	3 Bearing Floor Truss Job Reference (optional)

MiTek Industries, Inc., Chesterfield, MO 63017, Chip Dean — 6.000 s Feb 16 2004 MiTek Industries, Inc. Wed Feb 18 16:30:30 2004 Page 1



H Plate Offsets (X,Y): [2:0-1-0,Edge], [9:0-3-0,Edge], [10:0-3-0,Edge], [30:0-3-0,Edge], [31:0-3-0,Edge]

I LOADING (psf)		J SPACING	2-0-0	M CSI		N DEFL	in	(loc)	l/defl	L/d	O PLATES	P GRIP
TCLL	40.0	Plates Increase	1.00	TC	0.97	Vert(LL)	-0.35	31-32	>678	360	MT20	244/190
TCDL	10.0	K Lumber Increase	1.00	BC	0.98	Vert(TL)	-0.54	31-32	>439	240	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.06	27	n/a	n/a	MT18H	244/190
BCDL	5.0	L Code	BOCA/TPI2002	(Matrix)							Weight: 178 lb	Y

Q LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.1 *Except*
34-37 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3
OTHERS 4 X 4 SYP 1200F 1.2E

U BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

R S REACTIONS (lb/size) 39=937/0-3-8, 27=2066/0-3-8, 21=259/0-3-8
T Max Uplift21=-100(load case 2)
Max Grav39=956(load case 2), 27=2066(load case 1), 21=431(load case 3)

V FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 21-38=-36/0, 20-38=-36/0, 1-2=-1879/0, 2-3=-1875/0, 3-4=-1875/0, 4-5=-3448/0, 5-6=-3448/0, 6-7=-3448/0, 7-8=-2710/0, 8-9=-2684/0, 9-10=-1844/0, 10-11=-352/250, 11-12=0/2271, 12-13=0/2276, 13-14=0/1742, 14-15=-370/1284, 15-16=-373/1285, 16-17=-750/864, 17-18=-759/864, 18-19=-770/374, 19-20=-2/0, 1-39=-956/0
BOT CHORD 36-37=0/0, 35-36=0/2936, 34-35=0/3355, 33-34=0/3355, 32-33=0/3340, 31-32=0/1844, 30-31=0/1844, 29-30=0/1844, 28-29=-1178/0, 27-28=-1184/0, 26-27=-2276/0, 25-26=-1724/0, 24-25=-1065/582, 23-24=-1065/582, 22-23=-620/950, 21-22=-166/553
WEBS 9-31=-741/0, 10-30=0/749, 13-27=-1023/0, 16-24=0/3, 1-36=0/2036, 3-36=-237/0, 4-36=-1173/0, 4-35=0/566, 5-35=-258/0, 7-35=0/179, 7-32=-784/0, 8-32=-524/0, 9-32=0/1586, 11-27=-1488/0, 11-29=0/1604, 10-29=-2068/0, 19-21=-671/203, 19-22=-261/273, 18-22=-227/310, 18-23=-450/0, 16-23=0/410, 15-25=-116/34, 14-26=-635/0, 13-26=0/907, 14-25=0/1023, 16-25=-591/0

W NOTES
1) Unbalanced floor live loads have been considered for this design.
2) All plates are MT20 plates unless otherwise indicated.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Bearing at joint(s) 39 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
5) One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) 21.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
8) CAUTION, Do not erect truss backwards.

X LOAD CASE(S) Standard

ALL 50 STATES REGISTERED PROFESSIONAL ENGINEER — MiTek

SAMPLE:
NOT FOR PRODUCTION

How to read the... MiTek 20/20 Engineering Drawing

Roof Truss

- **A** Cumulative Dimensions
- **B** Panel Length (feet-inches-sixteenths)
- **C** Slope
- **D** Plate Size and Orientations
- **E** Overall Truss Height
- **F** Bearing Location
- **G** Truss Span (feet-inches-sixteenths)
- **H** Plate Offsets
- **I** Design Loading (PSF)
- **J** Spacing O.C. (feet-inches-sixteenths)
- **K** Duration of Load for Plate and Lumber Design
- **L** Code
- **M** Top Chord, Bottom Chord and Web maximum combined stress indicies
- **N** Deflections (inches) and Span to Deflection ratio
- **O** Input Span to Deflection Ratio
- **P** MiTek Plate Allowables (PSI)
- **Q** Lumber Requirements
- **R** Reaction (pounds)
- **S** Maximum Bearing Required (inches)
- **T** Maximum Uplift and/or Horizontal Reaction if Applicable
- **U** Required Member Bracing
- **V** Member Axial Forces for Load Case 1
- **W** Notes
- **X** Additional Loads/Load Cases
- **Y** Truss Weight

MiTek Industries, Inc.
14515 North Outer forty Drive
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Chesterfield, MO 63017-5746
(314)434-1200 Phone
www.mii.com

Job	Truss	Truss Type	Qty	Ply	MiTek Web Site
WEB SITE	ROOF1	ROOF TRUSS	13	1	MiTek Engineering Drawing Explained Job Reference (optional)

MiTek Industries, Inc., Chesterfield, MO 63017, Chip Dean 6.000 s Feb 16 2004 MiTek Industries, Inc. Wed Feb 18 15:06:38 2004 Page 1

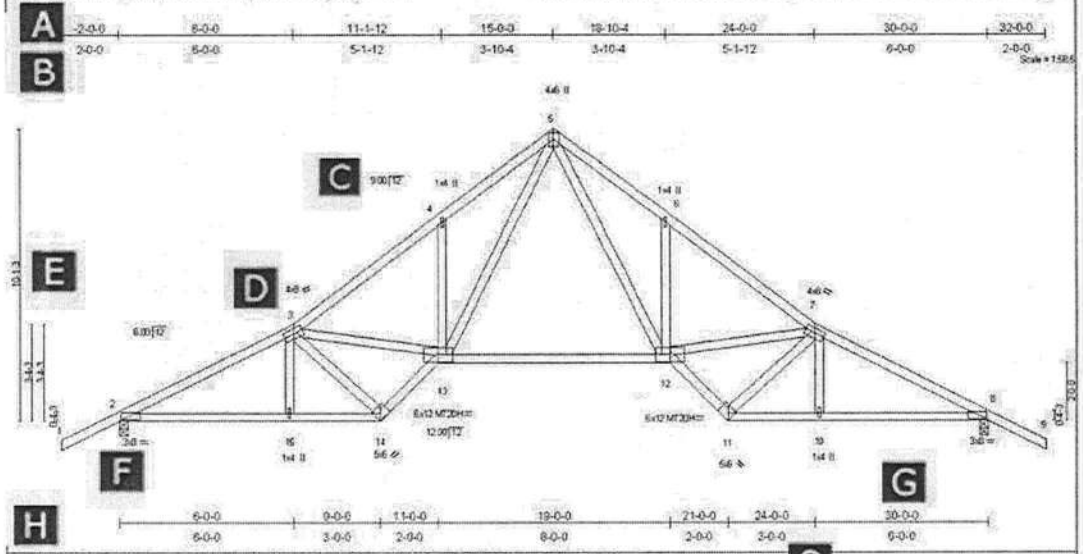


Plate Offsets (X,Y): [2:0-8-8,0-0-10], [8:0-8-8,0-0-10], [11:0-2-8,Edge], [14:0-2-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.38	Vert(LL)	-0.21	12-13	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.15	BC	0.57	Vert(TL)	-0.52	12-13	>688	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.21	8	n/a	n/a		
BCDL	10.0	Code IBC2000/TPI2002		(Matrix)								

Weight: 182 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1657/0-3-8, 8=1657/0-3-8
Max Horz 2=183(load case 4)
Max Uplift2=-101(load case 5), 8=-101(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/70, 2-3=-2712/26, 3-4=-2759/53, 4-5=-2715/184, 5-6=-2715/104, 6-7=-2759/29, 7-8=-2712/27, 8-9=0/70
BOT CHORD 2-15=-78/2314, 14-15=-82/2311, 13-14=-56/1703, 12-13=0/1401, 11-12=0/1703, 10-11=0/2311, 8-10=0/2314
WEBS 3-15=0/197, 3-14=-1521/45, 3-13=0/884, 4-13=-354/172, 5-13=-156/1578, 5-12=-103/1578, 6-12=-354/172, 7-12=0/884, 7-11=-1521/0, 7-10=0/197

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 90mph; h=25ft; TCDL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) 2 and 8.

LOAD CASE(S) Standard

ALL 50 STATES REGISTERED PROFESSIONAL ENGINEER — MiTek

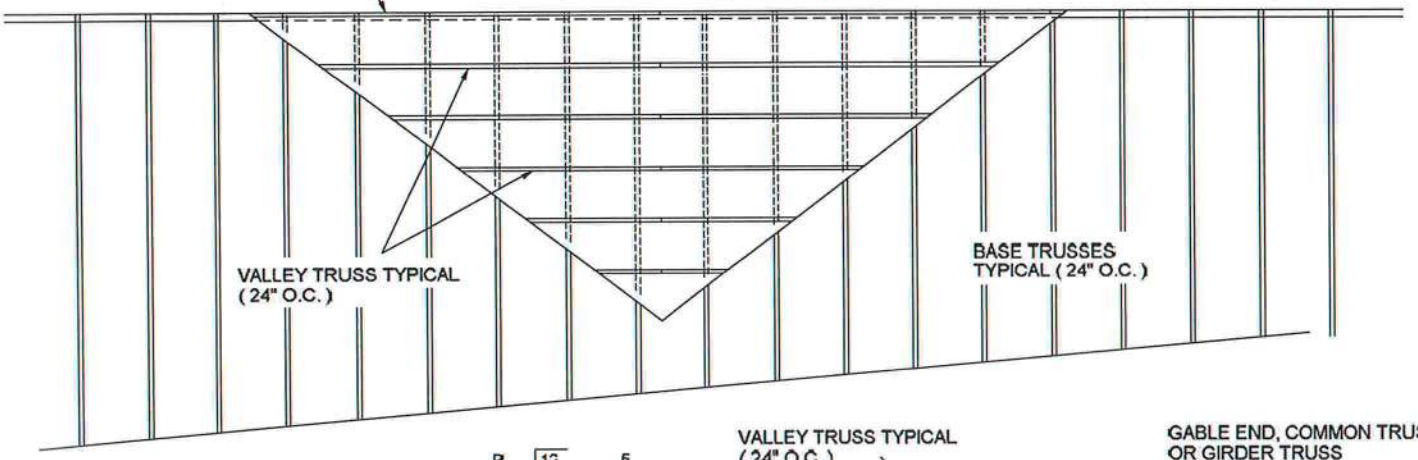
SAMPLE:
NOT FOR PRODUCTION

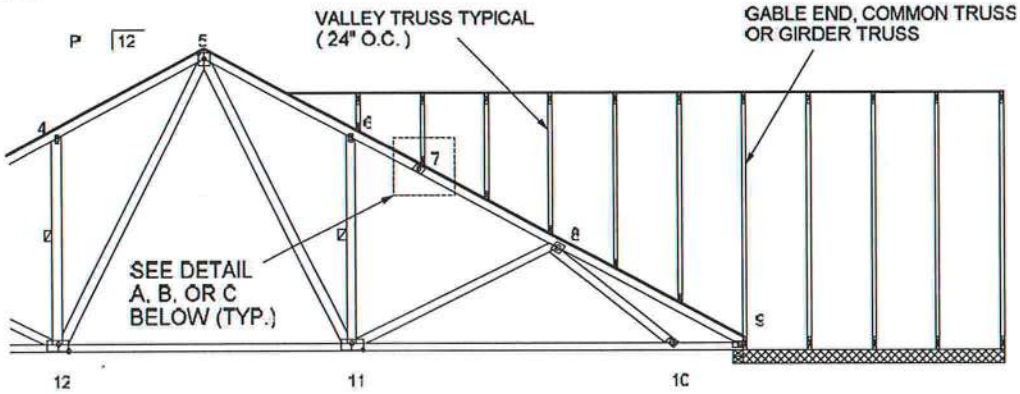
APRIL 21, 2004	TRUSSED VALLEY SET DETAIL	ST-VALLEY3

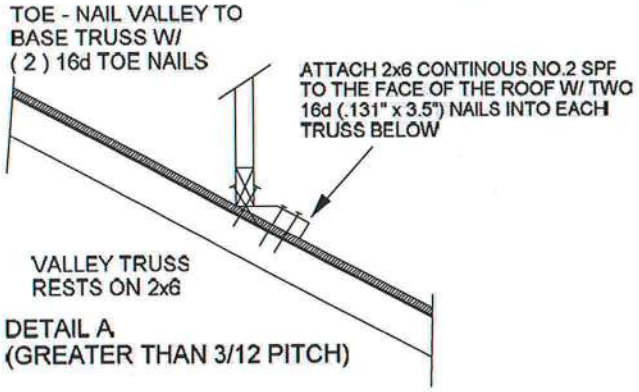
GABLE END, COMMON TRUSS OR GIRDER TRUSS

LIVE LOAD = 30 PSF (MAX)
DEAD LOAD = 15 PSF (MAX)
D.O.L. INC = 1.15
ASCE 7-98 (MWFRS) 100 MPH

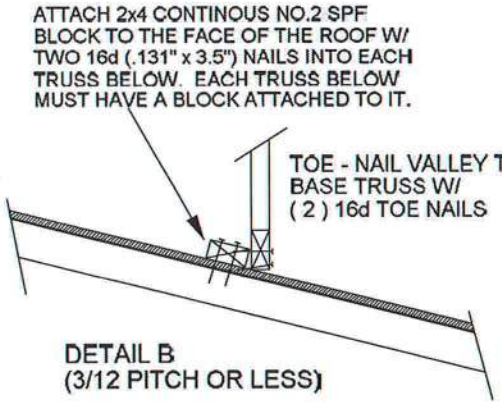
NOTE: VALLEY STUD SPACING NOT TO EXCEED 48" O.C. SPACING



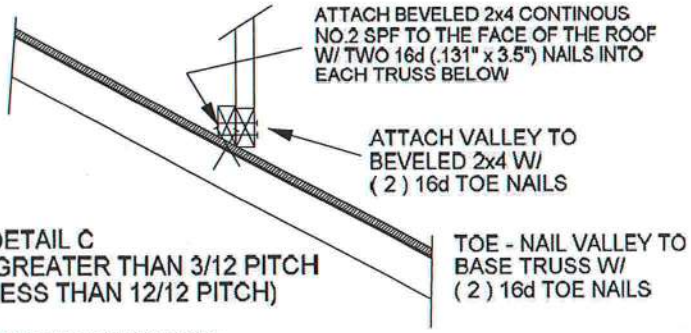




DETAIL A
(GREATER THAN 3/12 PITCH)



DETAIL B
(3/12 PITCH OR LESS)

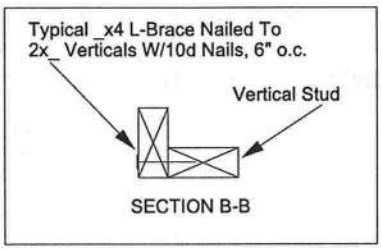


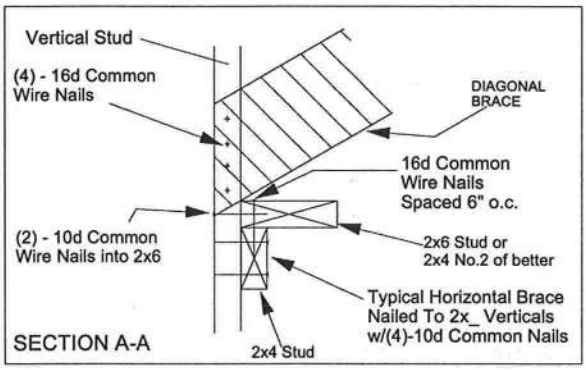
DETAIL C
(GREATER THAN 3/12 PITCH
LESS THAN 12/12 PITCH)

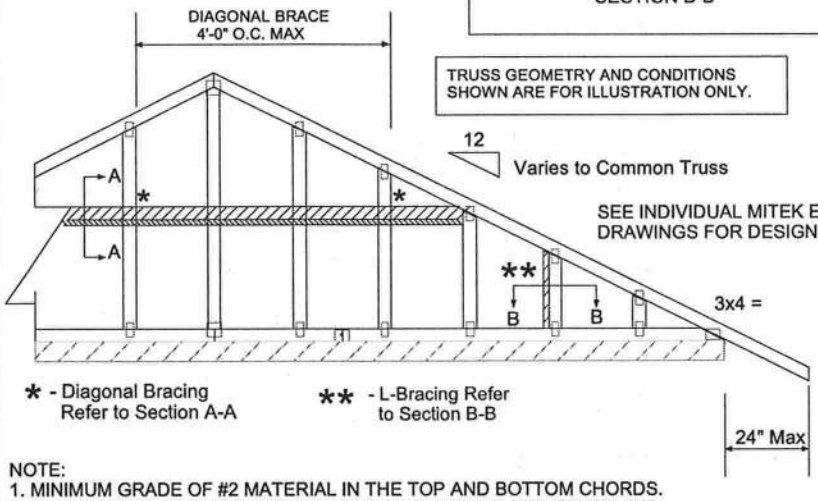
1. INSTALL BASE TRUSSES.
2. APPLY SHEATHING TO TOP CHORD OF SUPPORTING TRUSSES. VALLEY TRUSSES MAY PROVIDE BRACING IF SHEATHING IS NOT APPLIED. BASE TRUSSES MUST BE DESIGNED FOR PURLIN SPACING EQUIVALENT TO VALLEY TRUSS SPACING (NOT TO EXCEED 24" O.C.).
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A, B, OR C ABOVE.
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.

February 14, 2003	Standard Gable End Detail	ST-GE120-001

MiTek Industries, Inc.







TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

12
Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

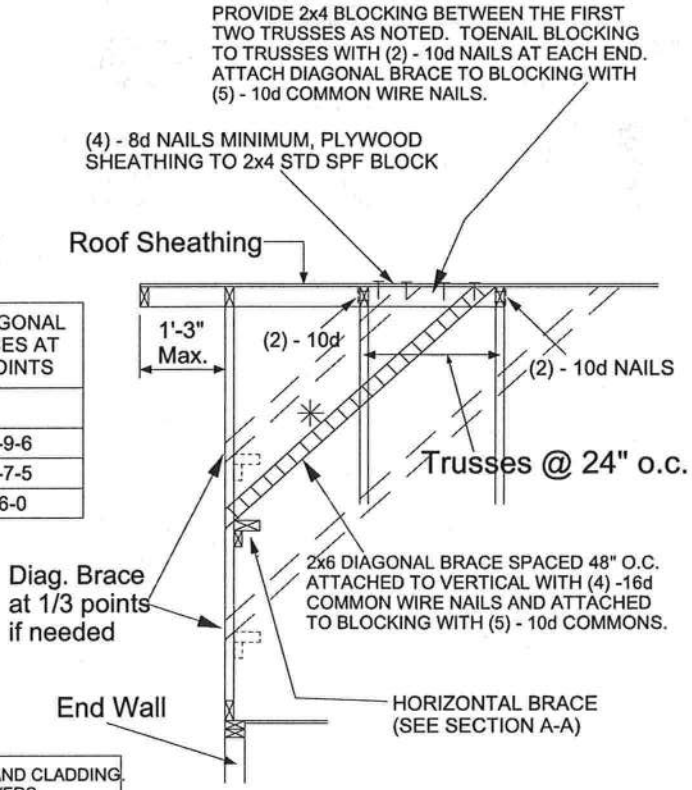
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-3-2	4-7-6	6-6-5	8-6-3	12-9-6
2x4 SPF Std/Stud	16" O.C.	3-10-7	4-0-0	5-7-13	7-8-14	11-7-5
2x4 SPF Std/Stud	24" O.C.	3-2-0	3-3-2	4-7-6	6-4-0	9-6-0

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

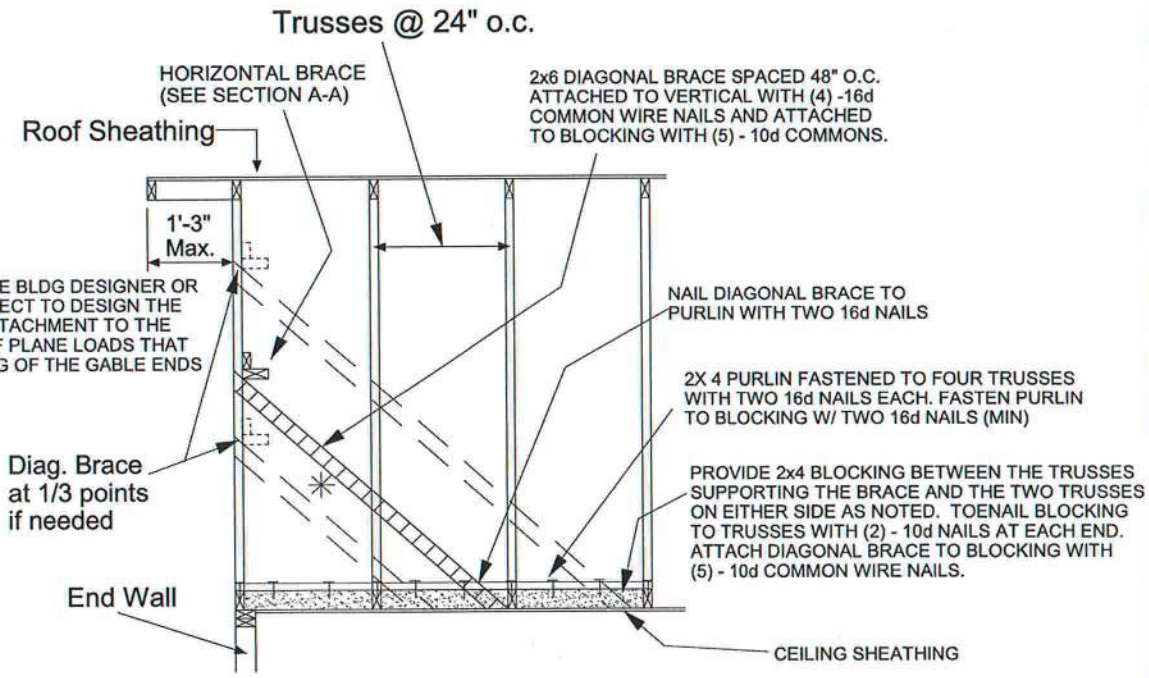
MAXIMUM WIND SPEED = 120 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



MiTek Industries, Inc.

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

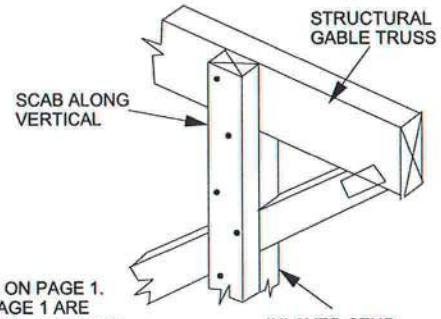
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

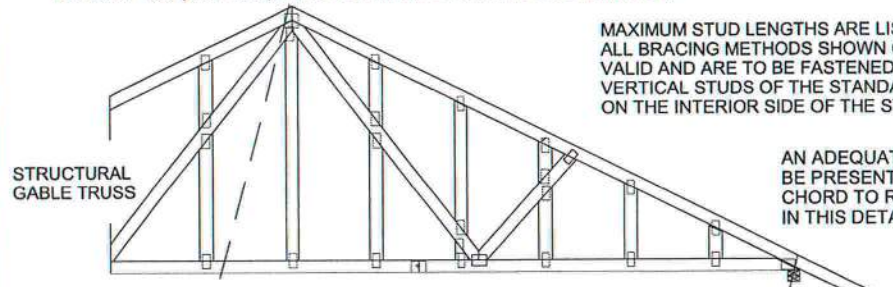
METHOD 2 : ATTACH 2X_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

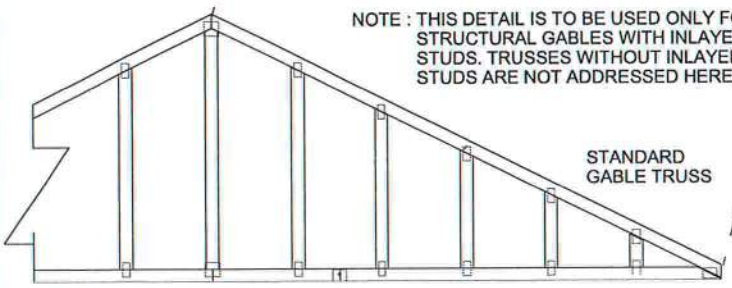


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



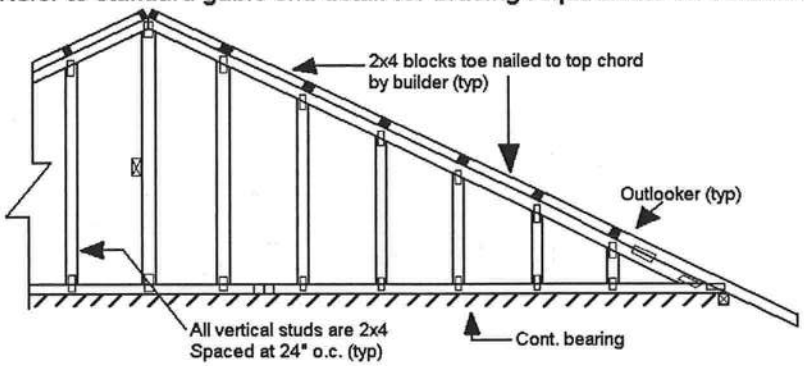
AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

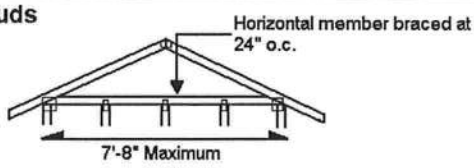
NOTE : THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS. TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE.



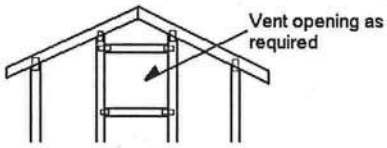
STANDARD DROPPED GABLE END DETAIL

*Refer to standard gable end detail for bracing requirments on vertical studs

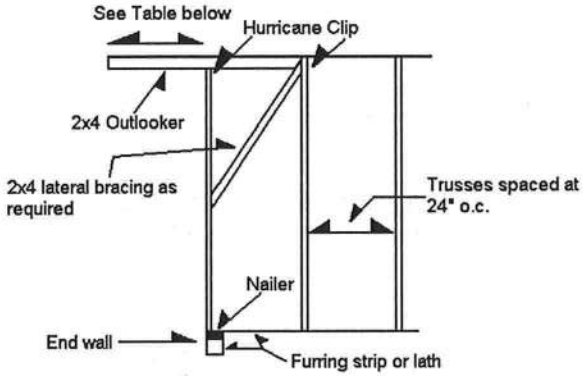


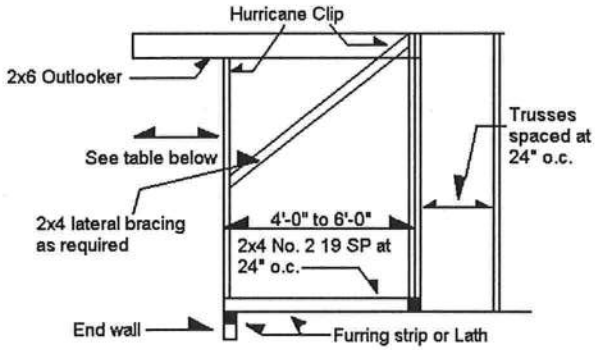


LOUVER OPENING DETAIL



VENT OPENING DETAIL





MAXIMUM OUTLOOKERS EXTENSION				
Lumber	With Plaster		Without Plaster	
	2x4	2x6	2x4	2x6
No.2ND 19 SP	2-8-3	3-10-11	2-10-13	4-2-9
No.2 19 SP	2-9-15	4-8-11	3-0-12	4-4-11
No.2D 19 SP	3-0-2	4-4-7	3-3-2	4-8-12
No.1ND 19 SP	3-0-2	4-5-6	3-3-2	4-8-11
No.1 19 SP	3-1-11	4-7-15	3-4-13	5-0-8
No.1D 19 SP	3-3-3	4-9-10	3-6-7	5-2-5
ND SS 19 SP	3-7-3	5-6-12	3-7-3	5-7-14
SS 19 SP	3-8-1	5-9-3	3-8-1	3-8-1
DSS 19 SP	3-8-13	5-10-7	3-8-13	5-10-7

Notes:

1. Connection between bottom chord of gable end truss and wall to be provided by project engineer or architect
2. Furnish copy pf this drawing to contractor to for installation of outlookers
3. Refer to standard gable end detail for bracing requirements of vertical studs

NOV 13, 2001	WEB BRACING RECOMMENDATIONS	ST-WEBBRACE

Mitek Industries, Inc.

X-BRACE BAY SIZE	MAXIMUM WEB FORCE (lbs.)									
	24"O.C.				48"O.C.				72" O.C.	
	BRACING MATERIAL TYPE				BRACING MATERIAL TYPE				BRACING MATERIAL TYPE	
	A	B	C	D	A	B	C	D	C	D
10'-0'	3680 *	4600 *	4600 *	6900 *	1344	4600 *	4600 *	6900 *	4034	6382
12'-0'	3154 *	3942 *	3942 *	5914 *	1344	3942 *	3942 *	5914 *	3942 *	5914 *
14'-0'	2760 *	3450 *	3450 *	5175 *	1344	3450 *	3450 *	5175 *	3450 *	5175 *
16'-0'	2453 *	3066 *	3066 *	4600 *	1344	3066 *	3066 *	4600 *	3066 *	4600 *
18'-0'	2208 *	2760 *	2760 *	4140 *	1344	2760 *	2760 *	4140 *	2760 *	4140 *
20'-0"	2007 *	2509 *	2509 *	3763 *	1344	2509 *	2509 *	3763 *	2509 *	3763 *

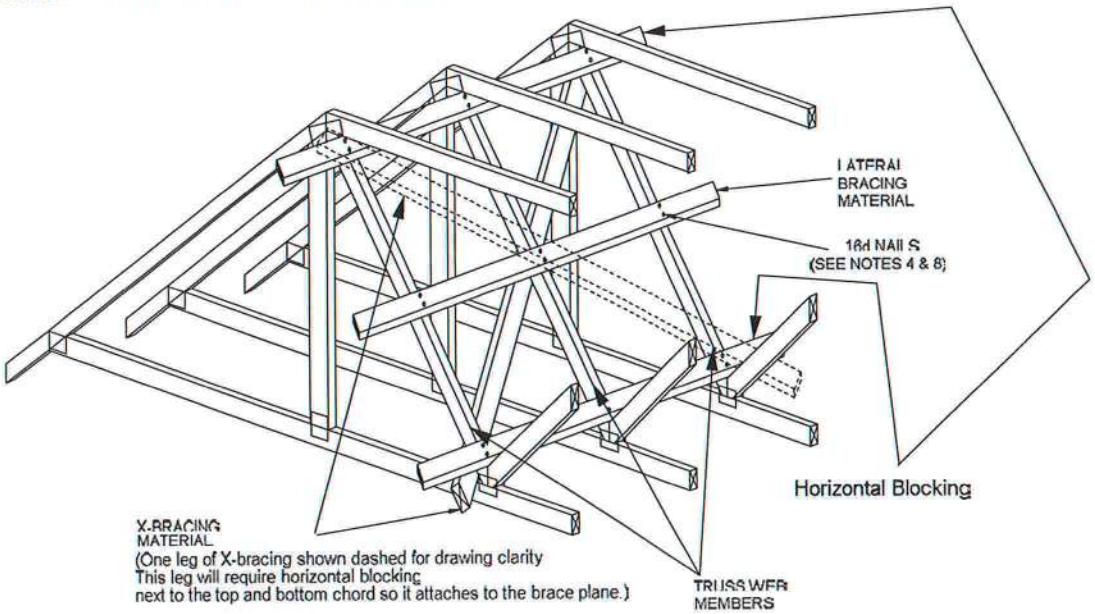
* -CONTROLLED BY CONNECTION

TYPE	BRACING MATERIALS
A	1 X 4 IND. 45 SYP -OR- 1 X 4 #2 SRB (DF, HF, SPF)
B	2 X 3 #3, STD, CONST (SPF, DF, HF, OR SYP)
C	2 X 4 #3, STD, CONST (SPF, DF, HF, OR SYP)
D	2 X 6 #3 OR BETTER (SPF, DF, HF, OR SYP)

GENERAL NOTES

1. X-BRACING IS REQUIRED TO TRANSFER THE CUMULATIVE LATERAL BRACE FORCE INTO THE ROOF AND/OR CEILING DIAPHRAGM. THE DIAPHRAGM IS TO BE DESIGNED BY A QUALIFIED PROFESSIONAL
2. THESE CALCULATIONS BASED ON LATERAL BRACE CARRYING 2% OF THE WEB FORCE.
3. X-BRACING MATERIAL MUST BE SAME SIZE AND GRADE OR BETTER, AS THE LATERAL BRACE MATERIAL, AND SHALL BE INSTALLED IN SUCH A MANNER THAT IT INTERSECTS WEB MEMBERS AT APPROX. 45 DEGREES AND SHALL BE NAILED AT EACH END AND EACH INTERMEDIATE TRUSS WITH 2-16d COMMON WIRE NAILS. (3-16d NAILS FOR 2X6 MATERIAL).
4. CONNECT LATERAL BRACE TO EACH TRUSS WITH TWO 16d COMMON WIRE NAILS. (THREE 16d NAILS FOR 2X6 LATERAL BRACES)
5. LATERAL BRACE SHOULD BE CONTINUOUS AND SHOULD OVERLAP AT LEAST ONE TRUSS SPACE FOR CONTINUITY.
6. FOR ADDITIONAL GUIDANCE REGARDING DESIGN AND INSTALLATION OF BRACING, CONSULT DSB-89 TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES AND HIB-91 HANDLING INSTALLING AND BRACING FOR RECOMMENDATIONS FROM TRUSS PLATE INSTITUTE 583 D'ONOFRIO DRIVE, MADISON, WI. 53719
7. SEE SEPERATE TRUSS ENGINEERING FOR DESIGN OF WEB MEMBER
8. THE 16d NAILS SPECIFIED SHOULD BE 3.5" LONG AND 0.162" IN DIAMETER, IN ACCORDANCE WITH NDS 1991.

NOTE: FOR A SPACING OF 24" O.C. ONLY, MITEK STABILIZER TRUSS BRACING SYSTEMS CAN BE SUBSTITUTED FOR TYPE A, B, C AND D BRACING MATERIAL. CROSS BRACING FOR STABILIZERS ARE TO BE PROVIDED AT BAY SIZE INDICATED ABOVE. WHERE DIAPHRAGM BRACING IS REQUIRED AT PITCH BREAKS, STABILIZERS MAY BE REPLACED WITH WOOD BLOCKING. SEE STABILIZER TRUSS BRACING INSTALLATION GUIDE AND PRODUCT SPECIFICATION.

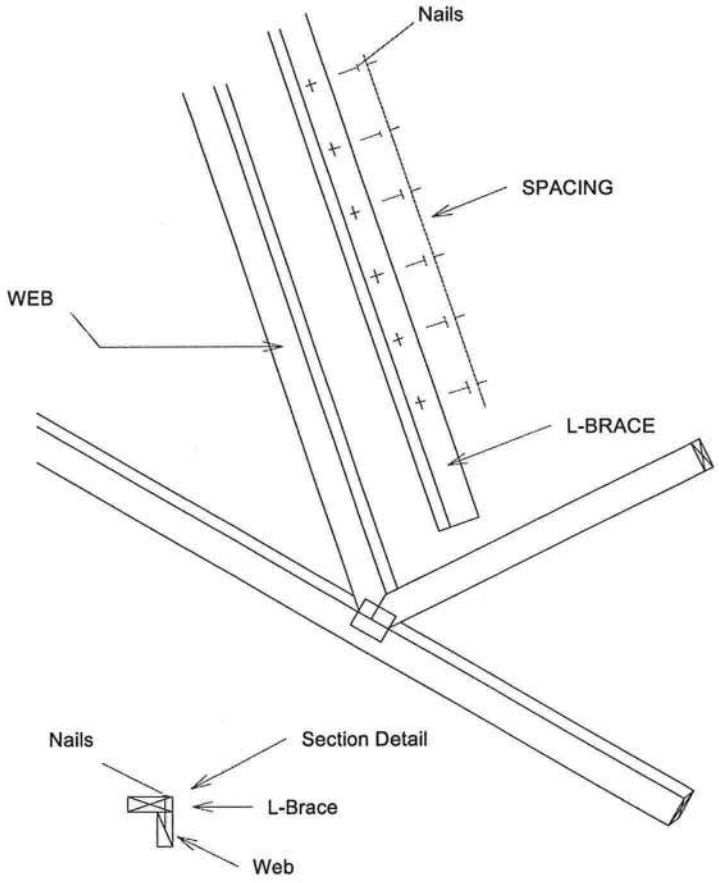


JANUARY 21, 2000	L-BRACE DETAIL	ST - L-BRACE

MiTek Industries, Inc. Chesterfield, MO.

Nailing Pattern		
L-Brace size	Nail Size	Nail Spacing
1x4 or 6	10d	8" o.c.
2x4, 6, or 8	16d	8" o.c.
Note: Nail along entire length of L-Brace (On Two-Ply's Nail to Both Plies)		

Note: L-Bracing to be used when continuous lateral bracing is impractical. L-brace must cover 90% of web length.



	L-Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	1x4	***
2x6	1x6	***
2x8	2x8	***

*** DIRECT SUBSTITUTION NOT APLICABLE.

	L-Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4	***
2x6	2x6	***
2x8	2x8	***

*** DIRECT SUBSTITUTION NOT APLICABLE.

L-Brace must be same species grade (or better) as web member.

OCTOBER 11, 2001	T-BRACE / I-BRACE DETAIL	ST - T-BRACE

MiTek Industries, Inc. Chesterfield, MO.

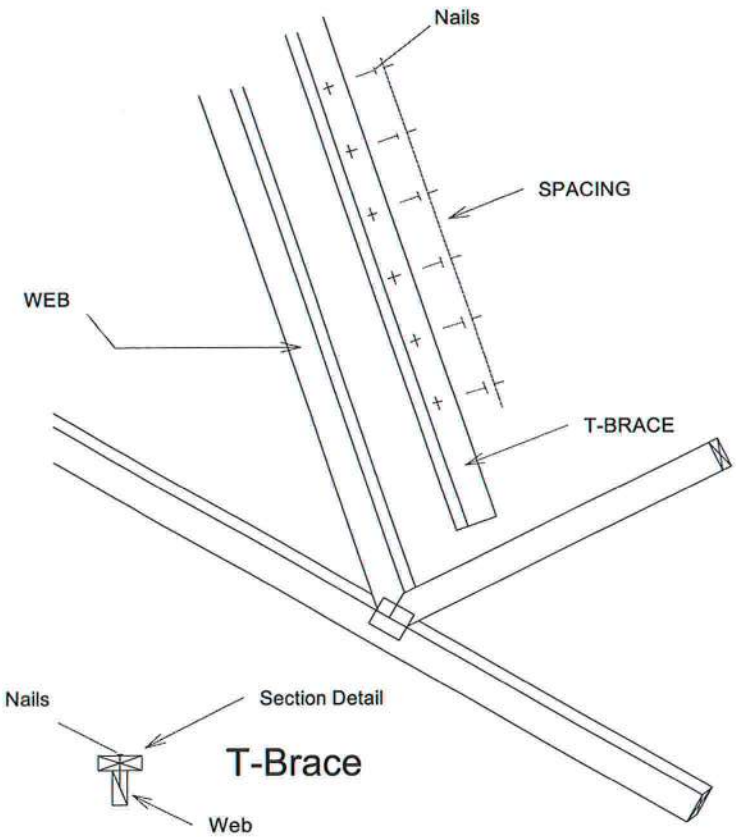
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
1x4 or 6	10d	8" o.c.
2x4, 6, or 8	16d	8" o.c.

Note: Nail along entire length of T-Brace (On Two-Ply's Nail to Both Plies)

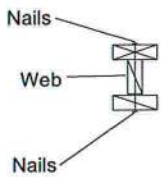
Note: T-Bracing to be used when continuous lateral bracing is impractical. T-brace must cover 90% of web length.

	T-Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	1x4	1x4 I-Brace
2x6	1x6	2x6 I-Brace
2x8	2x8	2x8 I-Brace

*** DIRECT SUBSTITUTION NOT APLICABLE.



T-Brace



I-Brace

	T-Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4	2x4 I-Brace
2x6	2x6	2x6 I-Brace
2x8	2x8	2x8 I-Brace

*** DIRECT SUBSTITUTION NOT APLICABLE.

T-Brace / I-Brace must be same species grade (or better) as web member.

FEB. 1, 2000	BEARING BLOCK DETAIL	ST-BLCK1

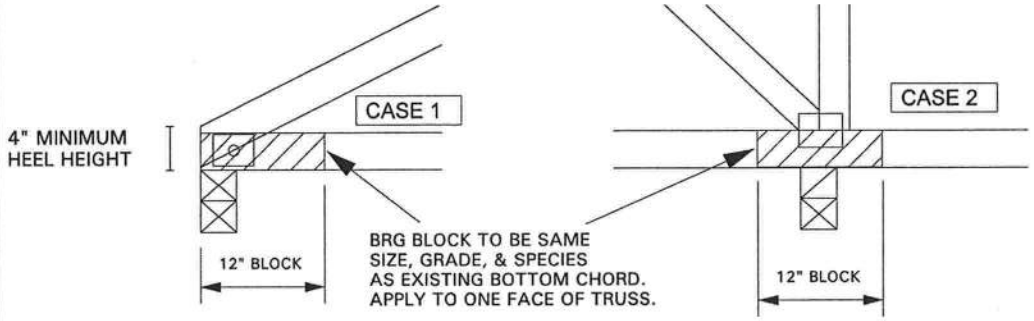
MiTek Industries, Inc.

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES

IMPORTANT
This detail to be used only with one ply trusses
with a D.O.L. lumber increase of 1.15 or higher.
Trusses not fitting these criteria should be examined individually.

0-3-8 ACTUAL BEARING SIZE

BOTTOM CHORD SIZE AND NAILING PATTERN	LUMBER GRADE	ALLOWABLE REACTION (lb) *	BEARING BLOCK ALLOWABLE LOADS	BEARING BLOCK & WOOD BEARING ALLOWABLE LOADS	
				ALLOWABLE LOAD (lb)	TOTAL EQUIVALENT BEARING LENGTH
2x4 BOTTOM CHORD 2 ROWS @ 3" O.C. (8 TOTAL NAILS)	SYP	2966	929	3895	0-4-9
	DF	3281	855	4136	0-4-6
	HF	2126	736	2862	0-4-11
	SPF	2231	726	2957	0-4-10
2x6 BOTTOM CHORD 3 ROWS @ 3" O.C. (12 TOTAL NAILS)	SYP	2966	1393	4359	0-5-2
	DF	3281	1282	4563	0-4-13
	HF	2126	1104	3230	0-5-5
	SPF	2231	1089	3320	0-5-3
2x8 BOTTOM CHORD 4 ROWS @ 3" O.C. (16 TOTAL NAILS)	SYP	2966	1858	4824	0-5-11
	DF	3281	1710	4991	0-5-5
	HF	2126	1472	3598	0-5-14
	SPF	2231	1452	3683	0-5-12



NOTES:
1. USE LOWER OF TOP PLATE OR TRUSS WOOD SPECIES.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. NAILS DESIGNATED ARE 10d (.131" DIAM. x 3")

* FOR BEARINGS NOT NEARER THAN 3" TO THE END OF A MEMBER (CASE 2), THESE VALUES MAY BE MULTIPLIED BY A BEARING FACTOR OF 1.10

LOADS BASED ON FOLLOWING Fc PERPENDICULAR VALUES:
SYP = 565 psi
DF = 625 psi
HF = 405 psi
SPF = 425 psi

NOTE: VALUES DO NOT INCLUDE MSR LUMBER WITH "E" VALUES GREATER THAN 1,900,000 PSI OR NON-DENSE GRADE LUMBER.

FEB. 7, 2000	BEARING BLOCK DETAIL	ST-BLCK2

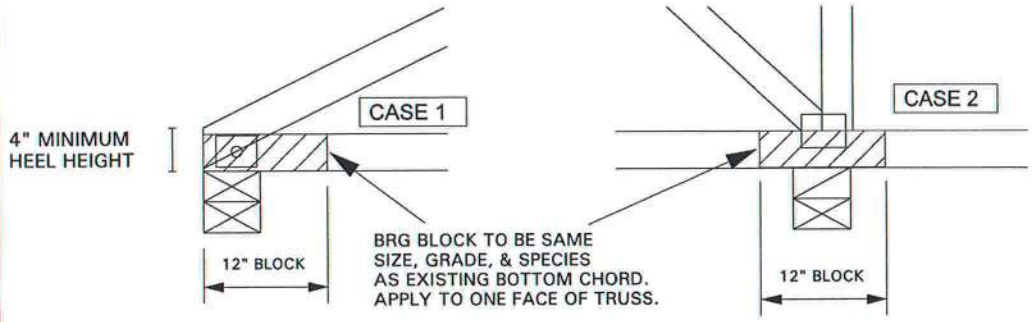
MiTek Industries, Inc.

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES

IMPORTANT
This detail to be used only with one ply trusses
with a D.O.L. lumber increase of 1.15 or higher.
Trusses not fitting these criteria should be examined individually.

0-5-8 ACTUAL BEARING SIZE

BOTTOM CHORD SIZE AND NAILING PATTERN	LUMBER GRADE	ALLOWABLE REACTION (lb) *	BEARING BLOCK ALLOWABLE LOADS	BEARING BLOCK & WOOD BEARING ALLOWABLE LOADS	
				ALLOWABLE LOAD (lb)	TOTAL EQUIVALENT BEARING LENGTH
2x4 BOTTOM CHORD 2 ROWS @ 3" O.C. (8 TOTAL NAILS)	SYP	4661	929	5590	0-6-9
	DF	5156	855	6011	0-6-6
	HF	3341	736	4077	0-6-11
	SPF	3506	726	4232	0-6-10
2x6 BOTTOM CHORD 3 ROWS @ 3" O.C. (12 TOTAL NAILS)	SYP	4661	1393	6054	0-7-2
	DF	5156	1282	6438	0-6-13
	HF	3341	1104	4445	0-7-5
	SPF	3506	1089	4595	0-7-3
2x8 BOTTOM CHORD 4 ROWS @ 3" O.C. (16 TOTAL NAILS)	SYP	4661	1858	6519	0-7-11
	DF	5156	1710	6866	0-7-5
	HF	3341	1472	4813	0-7-14
	SPF	3506	1452	4958	0-7-12



NOTES:
1. USE LOWER OF TOP PLATE OR TRUSS WOOD SPECIES.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. NAILS DESIGNATED ARE 10d (.131" DIAM. x 3")

* FOR BEARINGS NOT NEARER THAN 3" TO THE END OF A MEMBER (CASE 2), THESE VALUES MAY BE MULTIPLIED BY A BEARING FACTOR OF 1.03

LOADS BASED ON FOLLOWING Fc PERPENDICULAR VALUES:
SYP = 565 psi
DF = 625 psi
HF = 405 psi
SPF = 425 psi

NOTE: VALUES DO NOT INCLUDE MSR LUMBER WITH "E" VALUES GREATER THAN 1,900,000 PSI OR NON-DENSE GRADE LUMBER.

15%	STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICES	ST-REP01A1

JLG 6/19/03

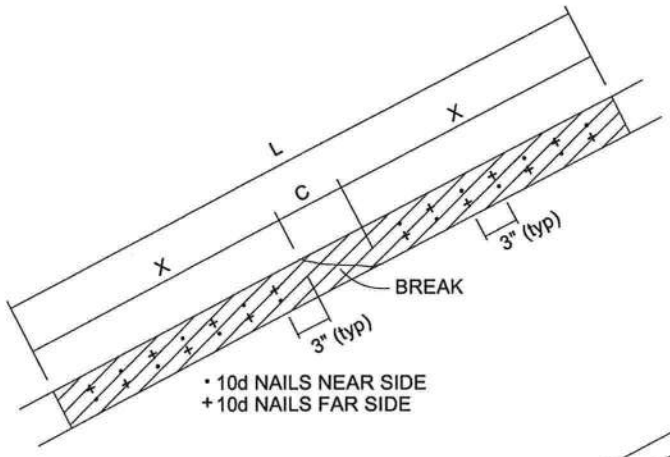
MiTek®

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION CONFORMS TO NDS 2001							
			SYP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
14	21	24"	1706	2559	1561	2342	1320	1980	1352	2028
18	27	30"	2194	3291	2007	3011	1697	2546	1738	2608
22	33	36"	2681	4022	2454	3681	2074	3111	2125	3187
26	39	42"	3169	4754	2900	4350	2451	3677	2511	3767
30	45	48"	3657	5485	3346	5019	2829	4243	2898	4347

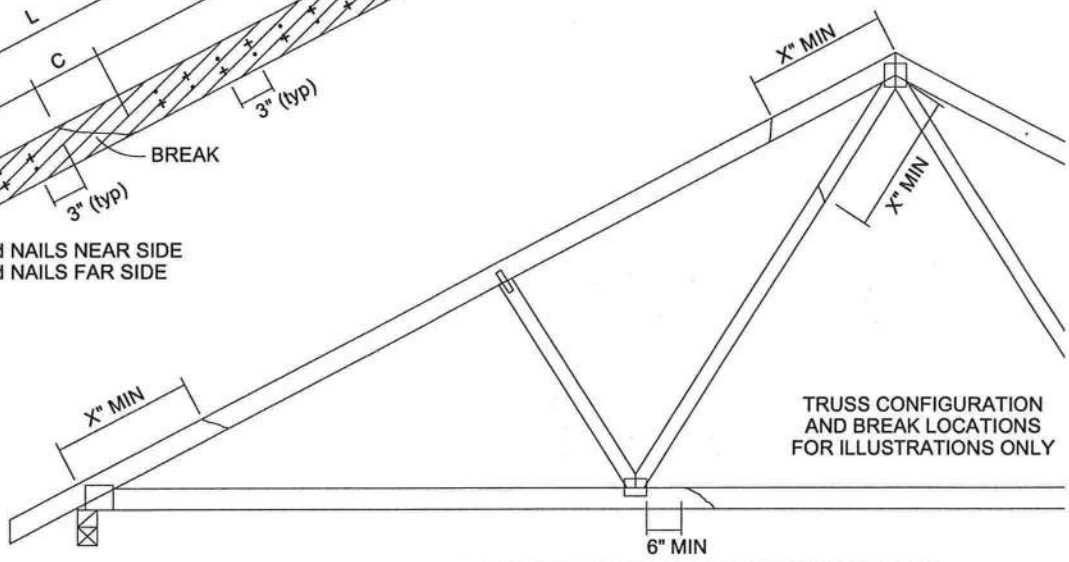
* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH CONSTRUCTION QUALITY ADHESIVE * AND 10d NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 3" O.C. STAGGERED AS SHOWN. (0.131" dia. x 3")

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C = PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS: L = (2) X + C



* THE USE OF CONSTRUCTION QUALITY ADHESIVE IS STRONGLY RECOMMENDED TO PROVIDE A SUPERIOR CONNECTION. HOWEVER, THE TABULATED LOAD IS ACHIEVED THROUGH THE USE OF A NAIL ONLY CONNECTION.



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

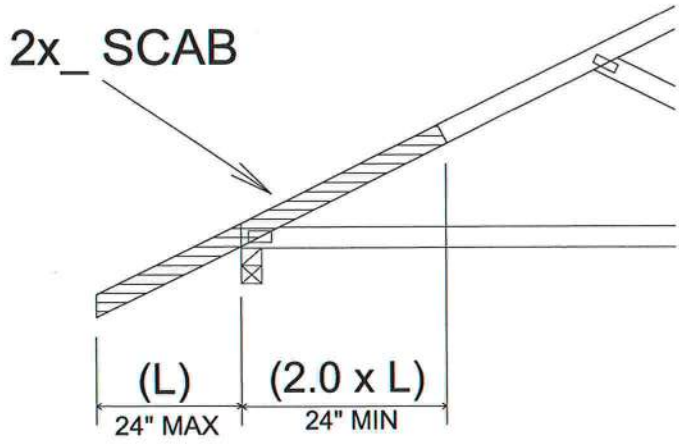
NOTES:
1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

MAY 20, 1997	SCAB APPLIED OVERHANGS	ST-REP13A

MiTek Industries, Inc.

TRUSS CRITERIA:
LOADING: 40-10-0-10
DURATION FACTOR: 1.15
SPACING: 24" O.C.
TOP CHORD: 2x4 OR 2x6
PITCH: 4/12 - 12/12
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL
END BEARING CONDITION

NOTES:
1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SYP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d COMMON WIRE NAILS (.148"DIA. x 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



NOTE: TRUSS BUILT WITHOUT AN OVERHANG.
THIS DETAIL IS NOT TO BE USED WHEN
OVERHANG HAS BEEN BROKEN OFF.

IMPORTANT
This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.
Trusses not fitting these criteria should be examined individually.

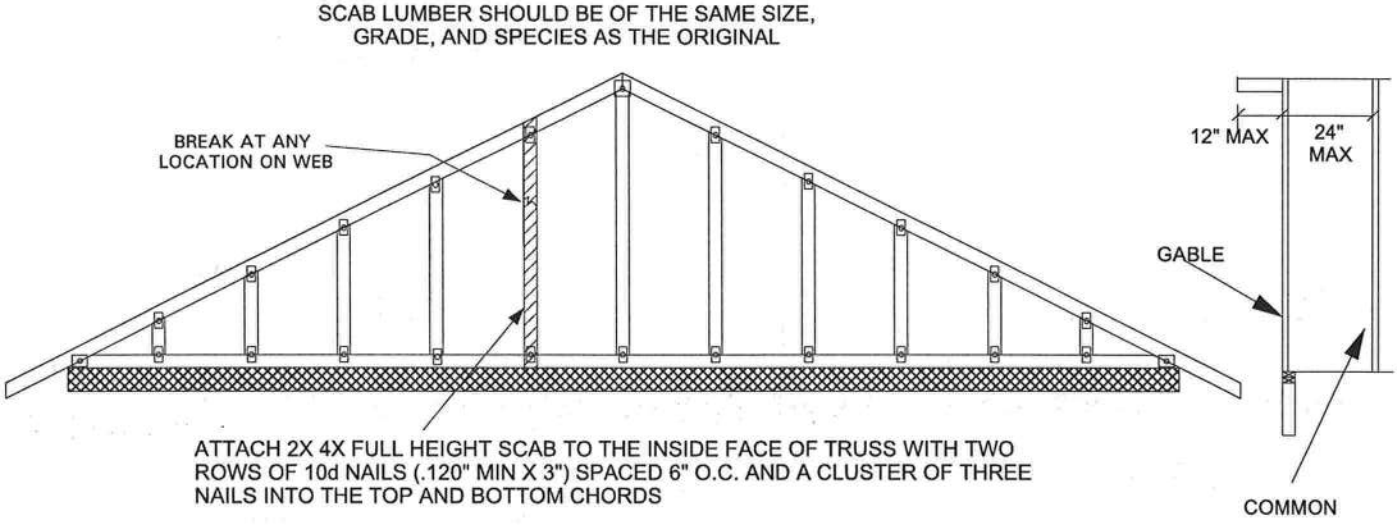
REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES

NOVEMBER 8, 1999	REPAIR FOR A BROKEN STUD ON A GABLE TRUSS	ST-REP16

MiTek Industries, Inc.

MiTek Industries, Inc. Chesterfield, MO

1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. WHEN NAILING SCABS OR GUSSETS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2X_ ORIENTATION ONLY.



THE OUTSIDE FACE OF THE GABLE MUST BE SHEATHED W/ (MIN) 7/16" O.S.B OR PLYWOOD.
SEE MITEK STANDARD GABLE END DETAILS FOR WIND BRACING REQUIREMENTS.

TRUSS CRITERIA

LOADING : 40-10-0-10 (MAX)
DURATION FACTOR : 1.15
SPACING : 24"
TOP CHORD : 2X 4 OR 2X 6 (NO 2 MIN)
PITCH : 3/12 - 12/12
BEARING : CONTINUOUS
STUD SPACING :24" O.C. (MAX)

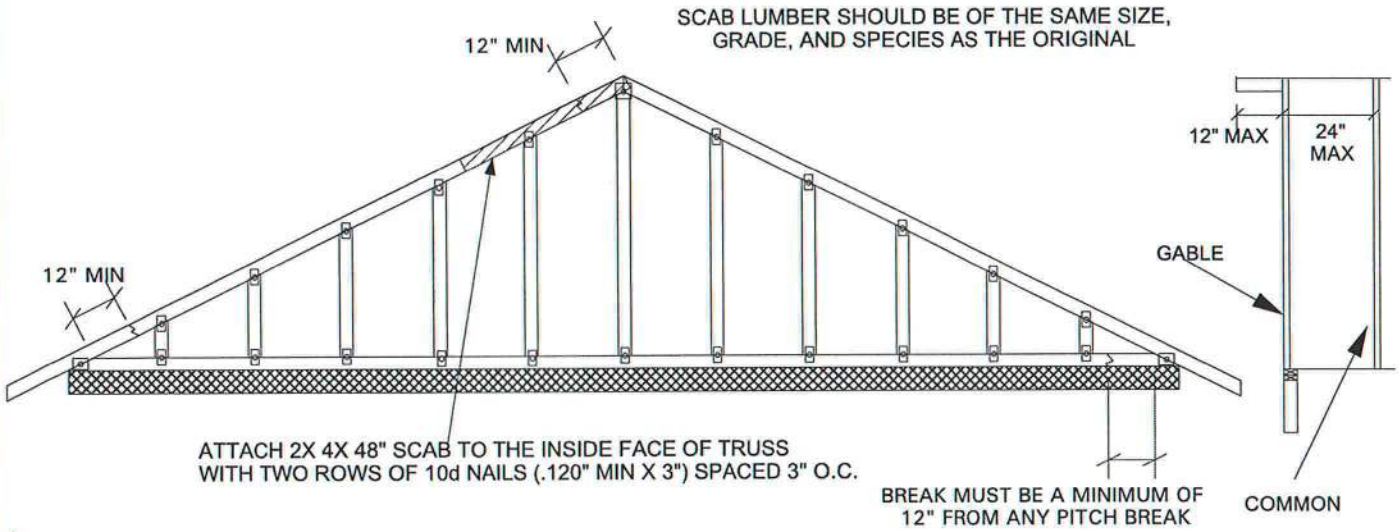
REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES

NOVEMBER 8, 1999	REPAIR FOR A BROKEN CHORD ON A GABLE TRUSS	ST-REP17

MiTek Industries, Inc.

MiTek Industries, Inc. Chesterfield, MO

1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. WHEN NAILING SCABS OR GUSSETS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2X_ ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



THE OUTSIDE FACE OF THE GABLE MUST BE SHEATHED W/ (MIN) 7/16" O.S.B OR PLYWOOD.
SEE MITEK STANDARD GABLE END DETAILS FOR WIND BRACING REQUIREMENTS.

TRUSS CRITERIA

LOADING : 40-10-0-10 (MAX)
DURATION FACTOR : 1.15
SPACING : 24"
TOP CHORD : 2X 4 OR 2X 6 (NO 2 MIN)
PITCH : 3/12 - 12/12
BEARING : CONTINUOUS
STUD SPACING :24" O.C. (MAX)

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES

FEBRUARY 8, 2000	REPAIR TO REMOVE CENTER STUD ON A GABLE TRUSS	ST-REP18

MiTek Industries, Inc.

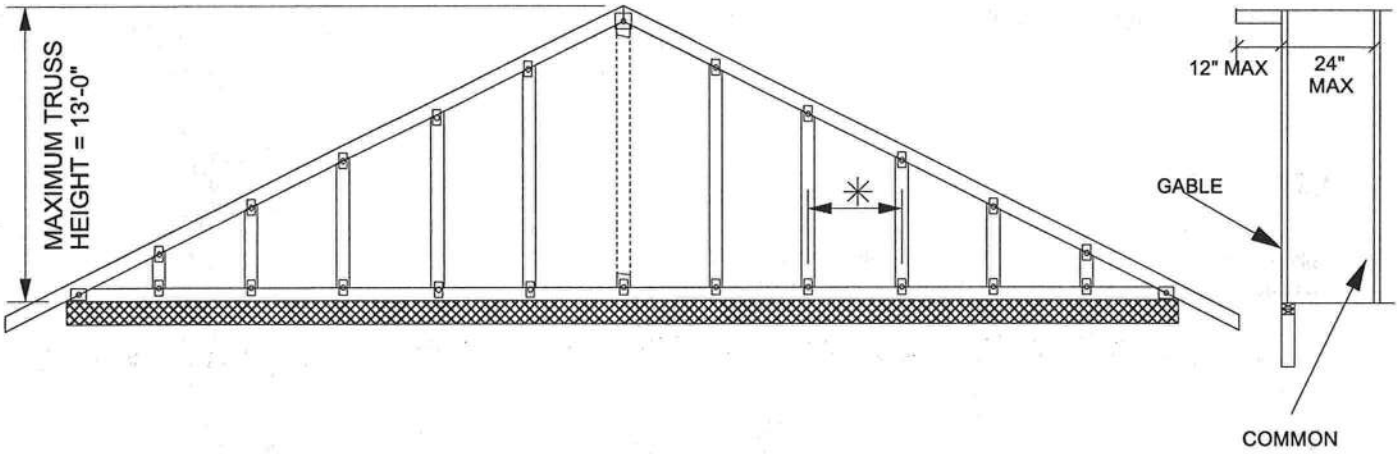
MiTek Industries, Inc. Chesterfield, MO

1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2X_ ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.

✳ MAXIMUM STUD SPACING = 24" O.C.

LUMBER TO BE CUT CLEANLY AND ACCURATELY,
NO PLATES ARE TO BE DISTURBED.
NO REPAIR NEEDED.



THE OUTSIDE FACE OF THE GABLE MUST BE SHEATHED W/ (MIN) 7/16" O.S.B OR PLYWOOD.
SEE MITEK STANDARD GABLE END DETAILS FOR WIND BRACING REQUIREMENTS.

IMPORTANT
This repair to be used only with trusses (spans less than 50') spaced 24" o.c. maximum, having pitches between 3/12 and 12/12, total top chord loads less than 50 psf and maximum wind speeds of 100 mph. Trusses not fitting these criteria should be examined individually.

TRUSS CRITERIA

LOADING : 40-10-0-10 (MAX)
DURATION FACTOR : 1.15
SPACING : 24" MAXIMUM
TOP CHORD : 2X 4 OR 2X 6 (NO 2 MIN)
PITCH : 3/12 - 12/12
BEARING : CONTINUOUS
STUD SPACING :24" O.C. (MAX)

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES

INSTALLATION GUIDE

PLATED TRUSS CONNECTORS

SIMPSON Strong-Tie®

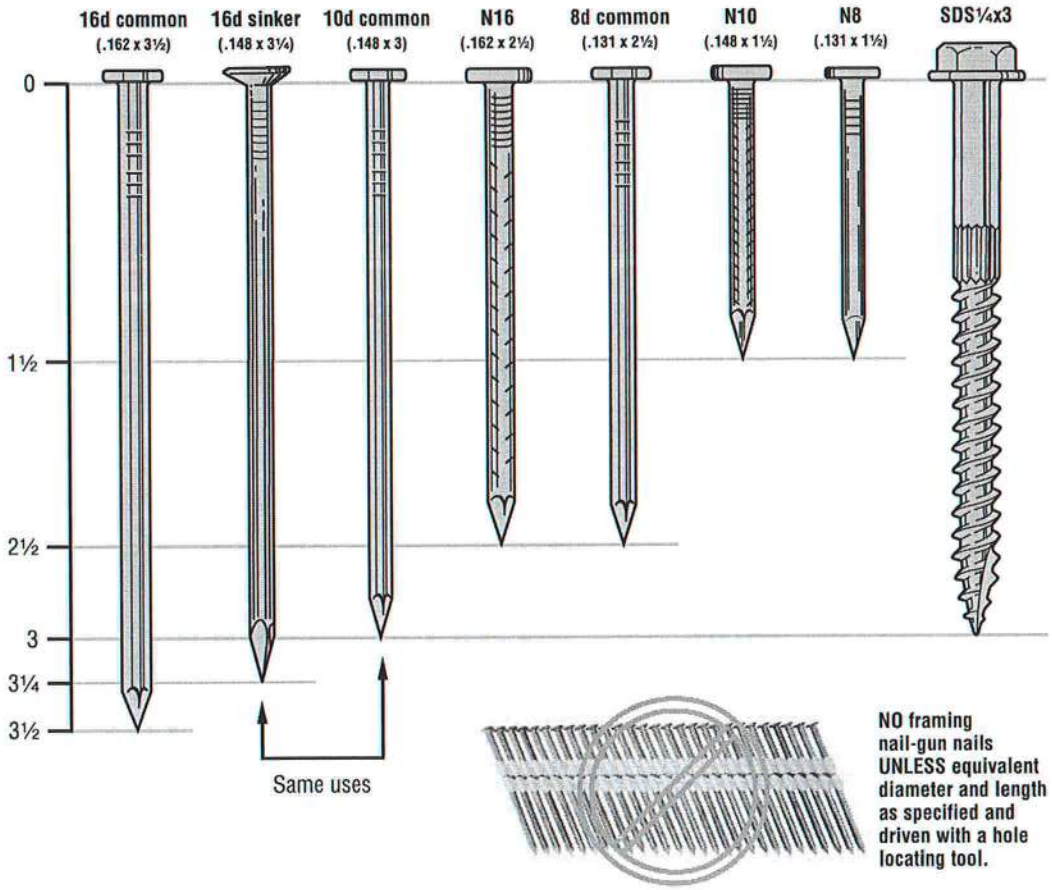
GENERAL NOTES

THIS POSTER IS A GUIDE FOR SIMPSON STRONG-TIE® STRUCTURAL CONNECTORS ONLY!

1. See the current Wood Construction Connectors catalog for hanger design information.
2. Simpson Strong-Tie does not express and will not accept any responsibility for any manufactured wood component including, but not limited to, bearing blocks and backing blocks.
3. The structural component manufacturer is the primary source of information concerning the use of their products.
4. All specified fasteners must be installed according to the instructions in this installation guide and current *Wood Construction Connectors* catalog. Incorrect fastener quantity, size, type, material or finish may cause connection failure.
5. Install all fasteners before loading the connection.
6. The hanger must be sized for the wood truss being used.
7. Multiple members must be fastened together to act as a single unit to resist the applied load.

FASTENERS

Use only the specified size, type, and quantity of fasteners!
16d sinkers (.148 x 3¼) may be substituted for 10d common nails (.148 x 3).
No other substitutions are permitted unless approved and specified by the designer.



NO framing nail-gun nails UNLESS equivalent diameter and length as specified and driven with a hole locating tool.

Home Office
4120 Dublin Blvd., Ste 400
Dublin, CA 94568
FAX: 925/833-1496

Northwest USA
5151 S. Airport Way
Stockton, CA 95206
FAX: 209/234-3868

Southwest USA
260 N. Palm St
Brea, CA 92821
FAX: 714/871-9167

Southeast USA
2221 Country Lane
McKinney, TX 75069
FAX: 972/542-5379

Northeast USA
2600 International St
Columbus, OH 43228
FAX: 614/876-0636

Quik Drive Factory
436 Calvert Drive
Gallatin, TN 37066
FAX: 615/451-9806

Eastern Canada
5 Kenview Blvd.
Brampton, ON L6T 5G5
Canada
FAX: 905/458-7274

Western Canada
11476 Kingston St.
Maple Ridge, BC V2X 0Y5
Canada
FAX: 604/465-0297

Warehouses:
Enfield, CT; Jacksonville, FL; Kent, WA; Langley, BC

800-999-5099
www.strongtie.com

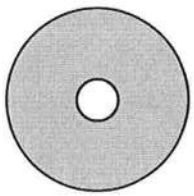
Printed in the USA

WC-PTBI06 3/06

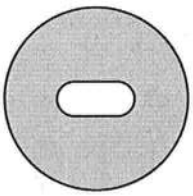
GENERAL CONNECTOR INSTALLATION

SIMPSON Strong-Tie®

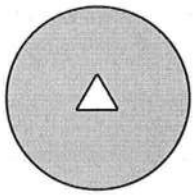
FASTENER HOLE SHAPES



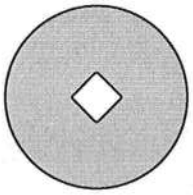
Round Holes
All holes must be filled except for the THA adjustable height hanger.



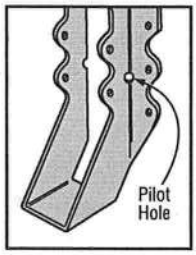
Obround Holes
Used to provide easier nailing access in tight locations. Fasteners may be installed at an angle. Holes must be filled.



Triangle Holes
Provided on some products in addition to round holes for additional load. Round and triangle holes must be filled to achieve the maximum load value.

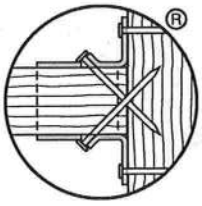


Diamond Holes
Optional holes to temporarily secure connectors to the member during installation.



Pilot Holes
Tooling holes for manufacturing purpose. No fasteners required.

DOUBLE SHEAR (SLANT) NAILING



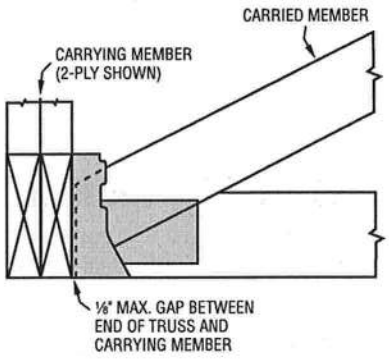
Double Shear Nailing Top View
(The nail is installed into the carried truss and header.)

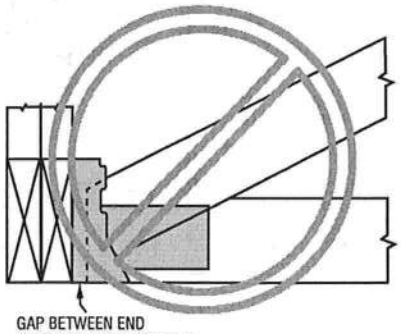
Dome Double Shear Nailing

Tab Double Shear Nailing
(Do not bend tabs except where noted for straight nailing.)

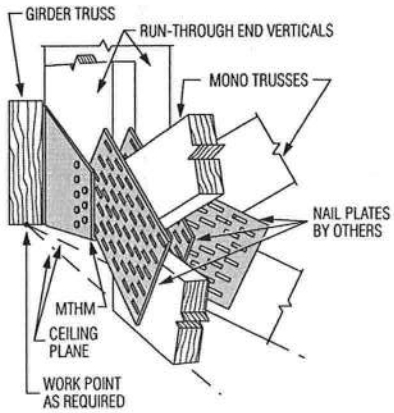
PROPER INSTALLATION

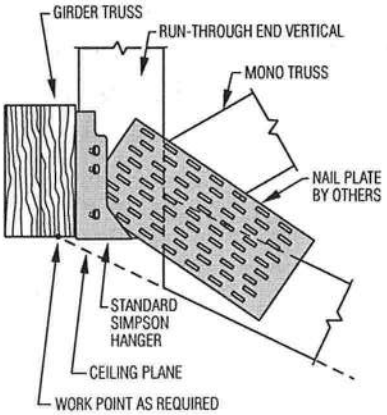
Truss shall bear completely on the connector seat, and the gap between the truss end and the header (carrying member) shall not exceed ⅛".

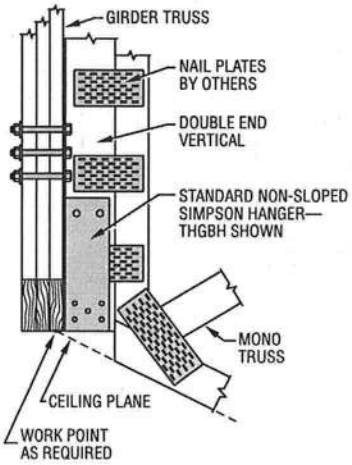




USING A STANDARD CONNECTOR AND SLOPED TRUSS







ADJUSTABLE STRAP HANGERS

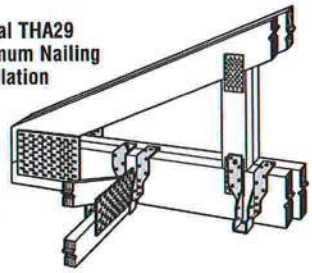
SIMPSON Strong-Tie®

THA SERIES

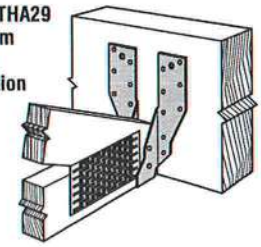
Minimum and maximum nailing options. See design information for correct installation option. For minimum nailing, straps must be bent over the top of the carrying member a minimum of 2½" for the THA29 and 1½" for the THA213.

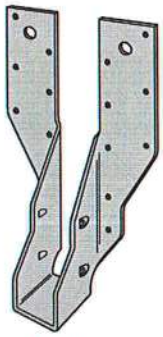
Model No.	Fasteners (Quantity & Size)			
	Minimum Nailing Installation (Top Flange)		Maximum Nailing Installation	
	Carrying Member	Carried Member	Carrying Member	Carried Member
THA29	Top: (4)-.148x3 Face: (4)-.148x3	(4)-.148x3 (slant)	Face: (16)-.148x3	(4)-.148x3 (slant)
THA213	Top: (4)-.148x3 Face: (2)-.148x3	(4)-.148x1½ (straight)	Face: (14)-.148x3	(4)-.148x3 (slant)

Typical THA29 Minimum Nailing Installation

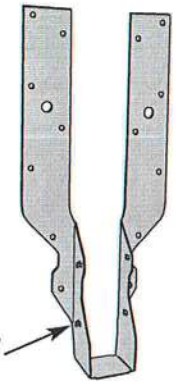


Typical THA29 Maximum Nailing Installation





THA29



For minimum nailing only, tabs must be straightened so that nails can be driven straight.

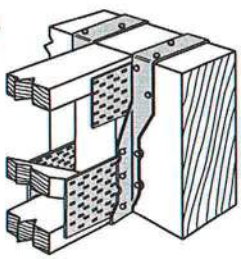
THA213

ADJUSTABLE FLOOR TRUSS HANGERS (THA AND THAR/L)

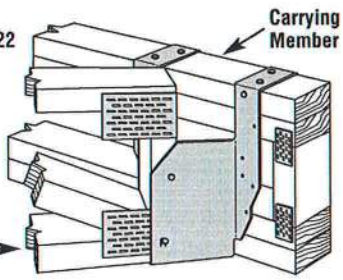
THAR/L has a standard skew of 45°. THAR/L straps must be bent over the carrying member a minimum of 2½". THA straps must be bent a minimum of 2" for the minimum nailing installation.

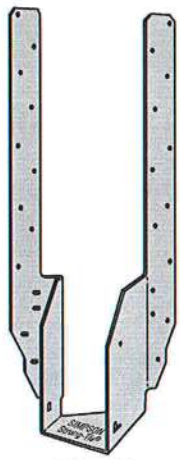
Model No.	Fasteners (Quantity & Size)			
	Minimum Nailing Installation (Top Flange)		Maximum Nailing Installation	
	Carrying Member	Carried Member	Carrying Member	Carried Member
THA418	Top: (4)-.162x3½ Face: (2)-.162x3½	(6)-.162x3½ (straight)	Face: (16)-.162x3½	(6)-.162x3½ (slant)
THA422/ THAC422	Top: (4)-.162x3½ Face: (2)-.162x3½	(6)-.162x3½ (straight)	Face: (22)-.162x3½	(6)-.162x3½ (slant)
THA422-2/ THAC422-2	Top: (4)-.162x3½ Face: (4)-.162x3½	(6)-.162x3½ (straight)	Face: (30)-.162x3½	(6)-.162x3½ (slant)
THAR/L422	Top: (4)-.148x3 Face: (2)-.148x3	(1)-.148x3 (straight) (2)-.148x1½ (slant)	Top: (4)-.148x3 Face: (12)-.148x3	(1)-.148x3 (straight) (2)-.148x1½ (slant)

Typical THA Installation with a 4x2 floor truss



Typical THAL422 Installation on 4x2 wood truss.





THAL422

INSTALLATION GUIDE FOR PLATED TRUSS CONNECTORS

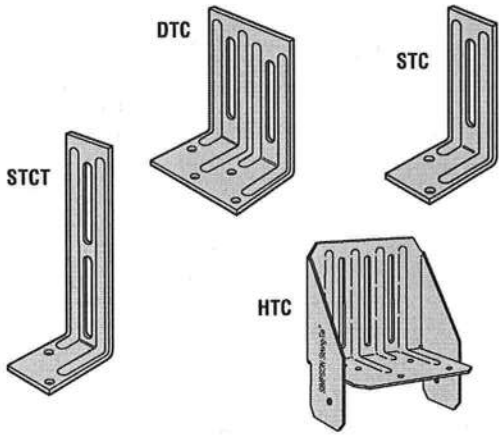
TRUSS-TO-WALL AND VALLEY TRUSS CONNECTORS

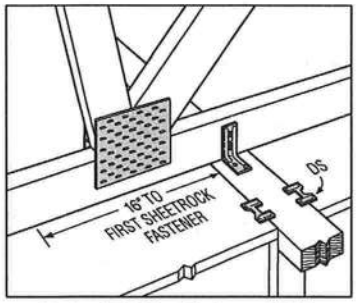
SIMPSON Strong-Tie®

STC/STCT/DTC/HTC SLOTTED TRUSS CLIPS

Install slot nails in the middle of the slot. Allow 1/16" under slot nail heads for movement of truss.

Model No.	Fasteners	
	To Wall	To Truss (Slot)
STC	(2)-.131x2½	(1)-.131x2½
STCT	(2)-.131x2½	(1)-.131x2½
DTC	(4)-.131x2½	(2)-.131x2½
HTC	(6)-.148x3	(3)-.148x3





Typical STC Installation with DS

Nails should not be driven completely flush against the connector, to allow vertical truss movement.

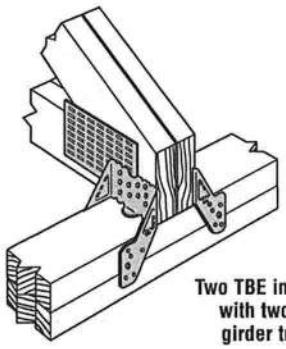
TBE TRUSS BEARING ENHANCER

Must be installed in pairs.

FASTENERS:

1-ply truss:
Plate: (10)-.148x1½
Truss: (10)-.148x1½

2 or more plies:
Plate: (10)-.148x3
Truss: (10)-.148x3



Two TBE installed with two ply girder truss

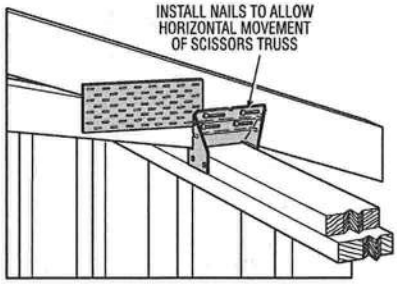
TC SCISSOR TRUSS CONNECTOR

Install nails at the inside of slotted holes and do not set.

FASTENERS:

TC24:
Plate: (4)-.148x3
Truss: (4)-.148x3

TC26 and TC28:
Plate: (6)-.148x3
Truss: (5)-.148x3

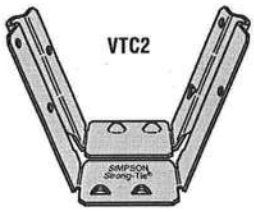


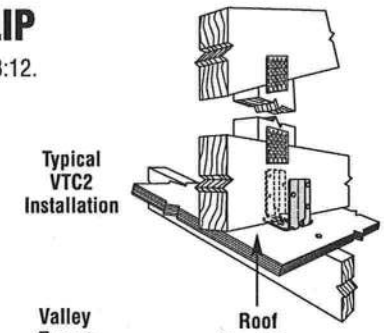
Typical TC24 Installation

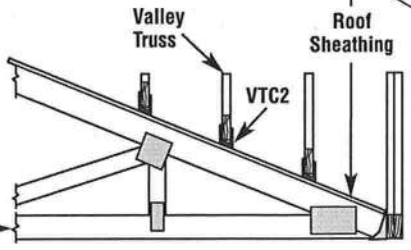
VTC VALLEY TRUSS CLIP

Field adjustable to slopes 2:12 through 8:12.

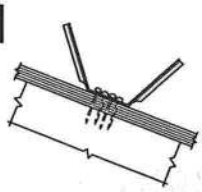
FASTENERS:
Common Truss: (4)-.148x3
Valley Truss: (5)-.148x1½







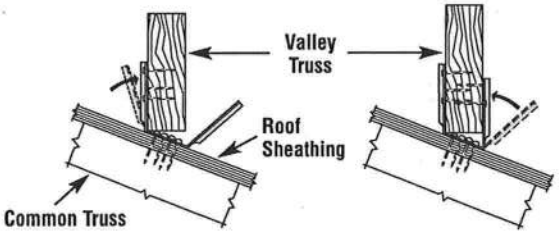
VTC2 INSTALLATION SEQUENCE



Step 1: Align the centerline of VTC2 with the common truss top chord centerline. Attach through the roof sheathing to the common truss top chord.

Step 2: Adjust the upslope stirrup vertically and attach to valley truss bottom chord.

Step 3: Adjust the downslope stirrup vertically and attach to bottom chord.
Bend stirrups one time only.

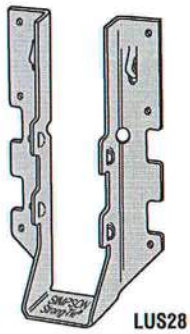


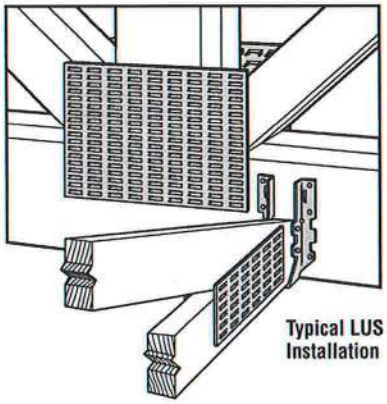
FACE MOUNT HANGERS

SIMPSON Strong-Tie

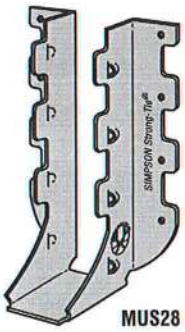
LUS/MUS/HUS/HHUS/HGUS SERIES

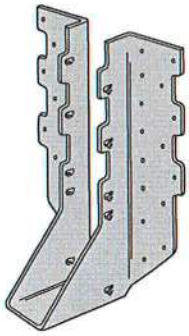
Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
1-PLY	LUS24	(4)-.148x3	(2)-.148x3
	LUS26	(4)-.148x3	(4)-.148x3
	LUS28	(6)-.148x3	(4)-.148x3
	LUS210	(8)-.148x3	(4)-.148x3
	MUS26	(6)-.148x3	(6)-.148x3
	MUS28	(8)-.148x3	(8)-.148x3
	HUS26	(14)-.162x3½	(6)-.162x3½
	HUS28	(22)-.162x3½	(8)-.162x3½
	HUS210	(30)-.162x3½	(10)-.162x3½
	HGUS26	(20)-.162x3½	(8)-.162x3½
	HGUS28	(36)-.162x3½	(12)-.162x3½
2-PLY	HHUS26-2	(14)-.162x3½	(6)-.162x3½
	HHUS28-2	(22)-.162x3½	(8)-.162x3½
	HHUS210-2	(30)-.162x3½	(10)-.162x3½
	HGUS26-2	(20)-.162x3½	(8)-.162x3½
	HGUS28-2	(36)-.162x3½	(12)-.162x3½
	HGUS210-2	(46)-.162x3½	(16)-.162x3½
3-PLY	HGUS26-3	(20)-.162x3½	(8)-.162x3½
	HGUS28-3	(36)-.162x3½	(12)-.162x3½
	HGUS210-3	(46)-.162x3½	(16)-.162x3½
4-PLY	HHUS210-4	(30)-.162x3½	(10)-.162x3½
	HGUS26-4	(20)-.162x3½	(8)-.162x3½
	HGUS28-4	(36)-.162x3½	(12)-.162x3½
	HGUS210-4	(46)-.162x3½	(16)-.162x3½
	HGUS212-4	(56)-.162x3½	(20)-.162x3½
	HGUS214-4	(66)-.162x3½	(22)-.162x3½
4X	HUS410	(8)-.162x3½	(8)-.162x3½
	HHUS410	(30)-.162x3½	(10)-.162x3½

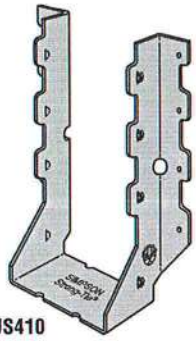

LUS28

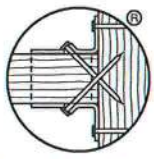

Typical LUS Installation


HGUS28-2


MUS28


HUS210 (HHUS similar)

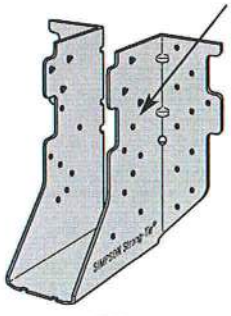

HUS410


Double Shear Nailing Top View

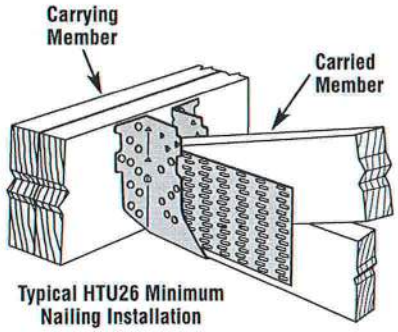
HTU SERIES

Minimum and maximum nailing options. See design information for correct installation option

Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
1-PLY	HTU26	(20)-.162x3½	(Min: 14)/(Max: 20)-.148x1½
	HTU28	(26)-.162x3½	(Min: 14)/(Max: 26)-.148x1½
	HTU210	(32)-.162x3½	(Min: 14)/(Max: 32)-.148x1½
2-PLY	HTU26-2	(20)-.162x3½	(Min: 14)/(Max: 20)-.148x3
	HTU28-2	(26)-.162x3½	(Min: 14)/(Max: 26)-.148x3
	HTU210-2	(32)-.162x3½	(Min: 14)/(Max: 32)-.148x3



HTU26



Typical HTU26 Minimum Nailing Installation

HEAVY GIRDER TRUSS HANGERS

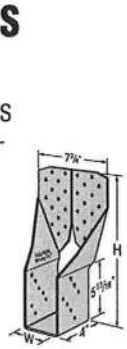
SIMPSON Strong-Tie®

THGB/THGBH/THGW & THGQ/THGQH SERIES

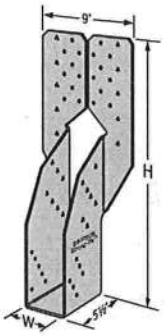
THGB uses Strong-Drive Screws (SDS1/4x3) or bolts into the carrying member. Refer to design information for correct fastener type. Pre-drill for bolts a maximum of 1/16" larger than bolt size. THGQ/THGQH uses all SDS screws (supplied with hanger). Minimum and maximum fastening options – see design information for correct installation option.

Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
2-PLY	THGB2	(19)-SDS¼x3 or (4)-¾" MB	(10)-.148x3
	THGBH2	(8)-¾" MB	(10)-.148x3
	THGQ2	2x6 vert: (22)-SDS¼x3 2x8 vert: (28)-SDS¼x3	(Min: 10)/(Max: 14) SDS¼x3
	THGQH2	2x6 vert: (18)-SDS¼x3 2x8 vert: (28)-SDS¼x3	(Min: 12)/(Max: 26) SDS¼x3
3-PLY	THGB3	(19)-SDS¼x3 or (4)-¾" MB	(10)-.148x3
	THGBH3	(8)-¾" MB	(10)-.148x3
	THGW3	(8)-¾" MB	(10)-.148x3
	THGQ3	2x6 vert: (22)-SDS¼x3 2x8 vert: (28)-SDS¼x3	(Min: 10)/(Max: 14) SDS¼x3
	THGQH3	2x8 vert: (32)-SDS¼x3 2x10 vert: (38)-SDS¼x3	(Min: 12)/(Max: 26) SDS¼x3
4-PLY	THGBH4	(8)-¾" MB	(10)-.148x3
	THGW4	(8)-¾" MB	(10)-.148x3
	THGQH4	2x8 vert: (34)-SDS¼x3 2x10 vert: (40)-SDS¼x3	(Min: 12)/(Max: 26) SDS¼x3

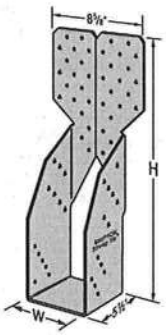
1. MB (machine bolts) refer to A307 Grade A through bolts (not lag screws).



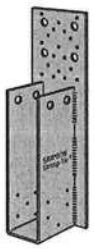
THGQ2-SDS3
(THGQ3-SDS3 Similar)



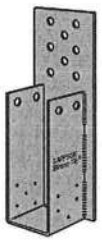
THGQH2-SDS3



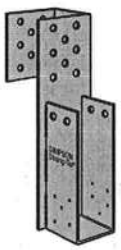
THGQH3-SDS3
(THGQH4-SDS3 Similar)



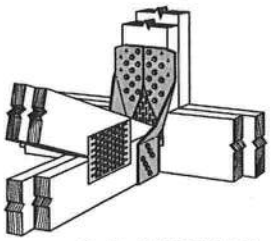
THGB2 (THGB3 Similar)



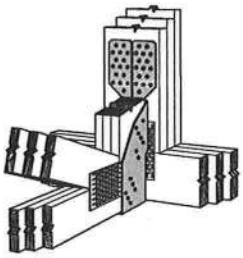
THGBH3 THGBH2 & THGBH4 Similar)



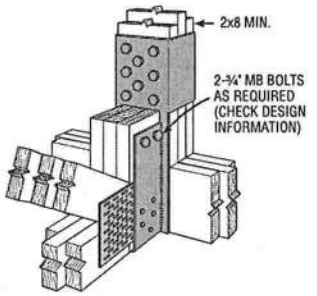
THGW3



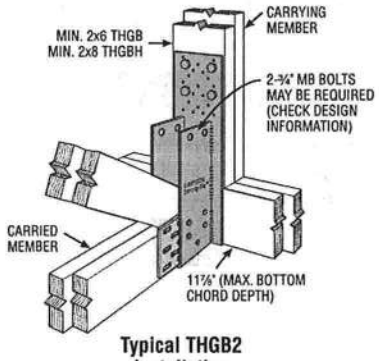
Typical THGQ2-SDS3 Installation



Typical THGQH3-SDS3 Installation



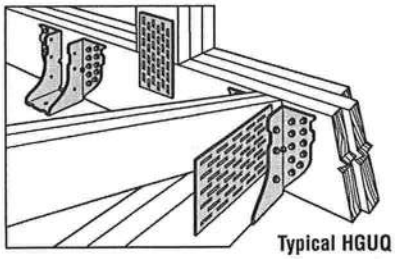
Typical THGW3-3 Installation



Typical THGB2 Installation

HGUQ SERIES

SDS screws supplied with hanger.
No pre-drilling required.



Typical HGUQ Installation

Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
2-PLY	HGUQ26-2	(12)-SDS¼x3	(4)-SDS¼x3
	HGUQ28-2	(20)-SDS¼x3	(6)-SDS¼x3
	HGUQ210-2	(28)-SDS¼x3	(8)-SDS¼x3
3-PLY	HGUQ26-3	(12)-SDS¼x4.5	(4)-SDS¼x4.5
	HGUQ28-3	(20)-SDS¼x4.5	(6)-SDS¼x4.5
	HGUQ210-3	(28)-SDS¼x4.5	(8)-SDS¼x4.5
4-PLY	HGUQ26-4	(12)-SDS¼x6	(4)-SDS¼x6
	HGUQ28-4	(20)-SDS¼x6	(6)-SDS¼x6
	HGUQ210-4	(28)-SDS¼x6	(8)-SDS¼x6

INSTALLATION GUIDE FOR PLATED TRUSS CONNECTORS

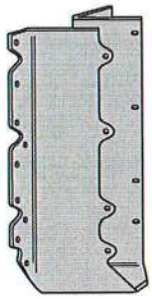
SKEWED AND FIELD SKEWABLE HANGERS

SIMPSON Strong-Tie®

SUR/L & HSUR/L

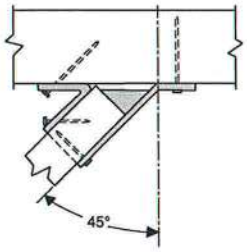
Standard 45-degree skew.

Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
1-PLY	SUR/L24	(4)-.162x3½	(4)-.148x1½
	SUR/L26	(6)-.162x3½	(6)-.148x1½
	SUR/L210	(10)-.162x3½	(10)-.148x1½
2-PLY	HSUR/L26-2	(12)-.162x3½	(4)-.162x2½
	HSUR/L210-2	(20)-.162x3½	(6)-.162x2½



HSUR

SUL

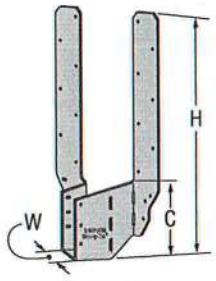


Typical SUL Installation with Square Cut Joist *(HSUL similar)*

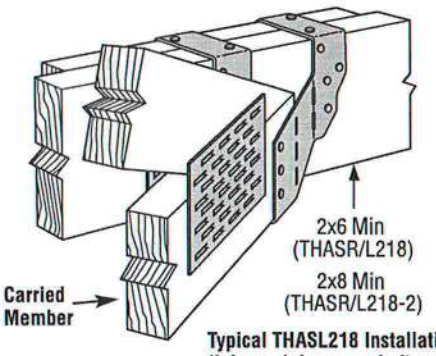
THASR/L

45-degree skewed, field skewable from 46° to 84°.
For 45° floor truss hanger, see THAR/L Adjustable Strap Hanger.

Carried Member	Model No.	Fasteners (Quantity & Size)	
		Carrying Member	Carried Member
1-PLY	THASR/L218	Top: (4)-.148x3 Face: (6)-.148x3	(6)-.148x1½
2-PLY	THASR/L218-2	Top: (4)-.148x3 Face: (8)-.148x3	(6)-.148x1½

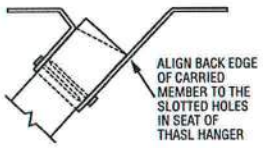


THASL218

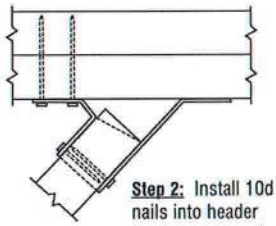


Typical THASL218 Installation (L in model name = Left, R in model name = Right)

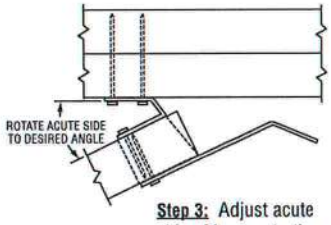
INSTALLATION SEQUENCE



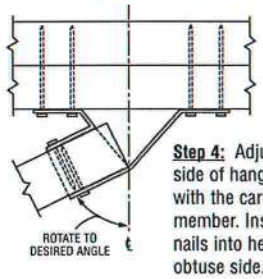
Step 1: Install carried member into the seat of the hanger. Secure with 6 10dx1½" nails.



Step 2: Install 10d nails into header on the acute angle side first.



Step 3: Adjust acute side of hanger to the desired angle.



Step 4: Adjust obtuse side of hanger flush with the carrying member. Install 10d nails into header on obtuse side.

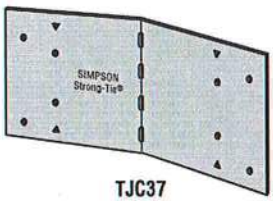
TJC37

Field skewable from 0° to 67.5°. Bend one time only. Minimum and maximum nailing options (check design information).

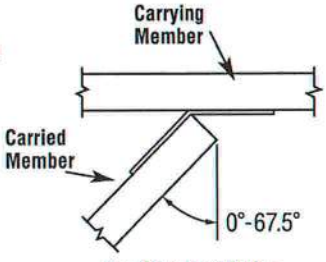
FASTENERS:

Carried Member:
Min: (4)-.131x1½
Max: (6)-.131x1½

Carrying Member:
Min: (4)-.131x1½
Max: (6)-.131x1½



TJC37



Top View Installation