16

NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Road
Lake City, FL 32055
386/365-4355

13 JUNE 2013

BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

RE: BELMONT ACADEMY
PERMIT Nr.: 30555

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CHANGE TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

1. THE COLUMN FOOTINGS FOR THE COVERED DRIVEWAY ENTRY SHALL BE 49" SQUARE X 18" DEEP, POURED MONOLITHICALLY WITH THE DRIVEWAY SLAB, WITH THE PAD FOOTING REINFORCING PLACED IN TWO LAYERS AS PER THE CONSTRUCTION DOCUMENTS.

2. THE ANCHOR BOLTS FOR THE ENTRY CANOPY MAY BE 3/4"ϕ X 24" LONG WITH A 3" SQ. X 1/2" STEEL PLATE WELDED TO THE EMBEDDED END OF THE ANCHOR BOLT, IN LIEU OF THE 9" HOOK CALLED FOR IN THE CONSTRUCTION DOCUMENTS.

3. THE DRIVEWAY SLAB SHALL BE A NOMINAL 6" THICK SLAB REINFORCED WITH 6X6 10/10 WELDED WIRE MESH, IN LIEU OF THAT CALLED FOR IN THE CONSTRUCTION DOCUMENTS.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005

STATE OF FLORIDA
NICHOLAS PAUL GEISLER
No. 7005
REGISTERED ARCHITECT

16

NICHOLAS
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GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Road
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07 MAY 2013

BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

RE: BELMONT ACADEMY
PERMIT Nr.: _____

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CHANGE TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

1. IN LIEU OF THE STEEL DECK FASTENING SYSTEM AS NOTED IN THE CON-DOCS, IT SHALL BE PERMISSIBLE TO ATTACH THE STEEL FLOOR DECKING TO THE SUPPORTING STEEL FRAMING AS DETAILED IN THE ENGINEERED STEEL DECK SHOP DRAWINGS.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005

STATE OF FLORIDA
NICHOLAS PAUL GEISLER
No. 7005
REGISTERED ARCHITECT

Job Title Belmont

Deck Profile BI | Depth 1.5 in | Pitch 6 in

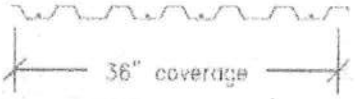
Deck Width(in) 36 | Girth 8.76 in

Deck Gage(in) 22 | Thickness 0.0295 in

Material Grade(ksi) 40 | F_y = 40 ksi | F_u = 55 ksi

Q_s (kips) 21.5

Structural Attach Pattern 36/4



Structural Attach Type Screws Size12 | Sidelap Attach Type Screws10 | ASTM Number A653

Interior Corrugation Ult. Structural Uplift Resistance 974 #/fast | Interior Quantity 2 PER SPAN.

Edge Corrugation Ult.Structural Uplift Resistance 974 #/fast | Edge Fasteners Quantity 2(1/2 are common)

Allowable Diaphragm Shear(w/o uplift)
Controlled by Fasteners = 270 #/ft

Allowable Diaphragm Shear (w/o uplift) 270 #/ft | 2 sidelap fast/span

Deck Span 4 ft | Number of Spans 3 | Wind Uplift Pressure (psf) 0

Output Data

Beta(w/o uplift) 4.44 | Number of Structural Attachments per ft = 1.000

Q_f (w/o uplift)Ultimate Shear Capacity at Frame = 1,180 #/fast | Q_s (w/o uplift) Ultimate Shear Capacity at Sidelap = 634 #/fast

Number of Sidelap Attach 6

UPLIFT RESISTANCE EQUATION

Ultimate Uplift Resistance Without Shear 974 #/ft | Continue | Fastener Tensile Safety Factor = 3

Ratio of Useable to Full Diaphragm Shear/Fast 1.000 | Service Uplift Force 0.00 #/ft

Useable Ultimate Shear Capacity Q_f (w/uplift) 1,180 #/fast | Q_s / Useable Q_f = 0.538

Beta(w/uplift) | Actual Lambda 0.854 | Use Lambda 0.854

CHECK PANEL BUCKLING

Inertia X-axis 0.17 in^4/ft | Panel Buckling Safety Factor = 2

Ultimate Diaphragm Shear 3.482 kips/ft

Allowable Diaphragm Shear 1.741 kips/ft > 0.270 kips/ft | Buckling doesn't control

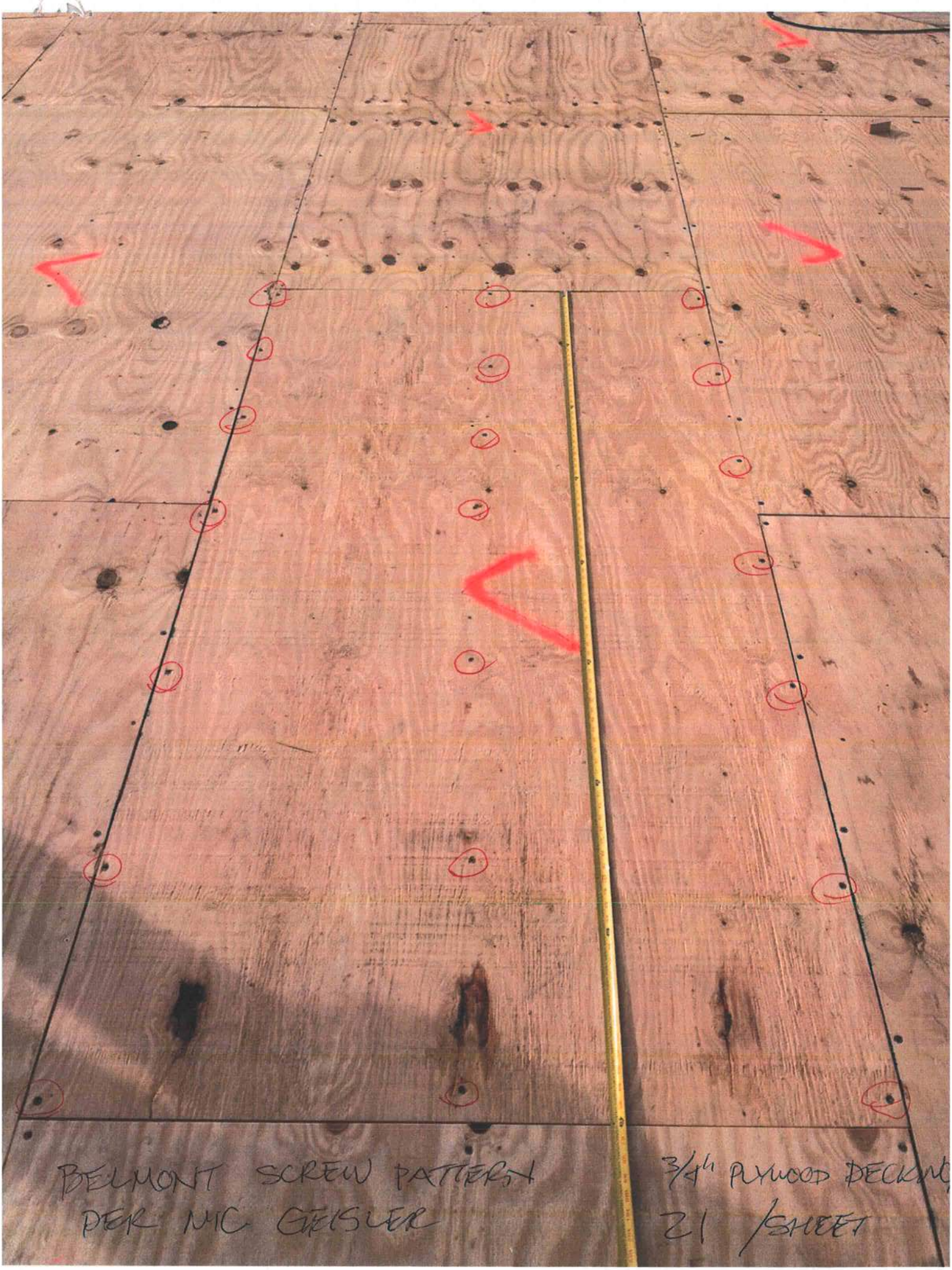
CALCULATE DIAPHRAGM SHEAR RESISTANCE

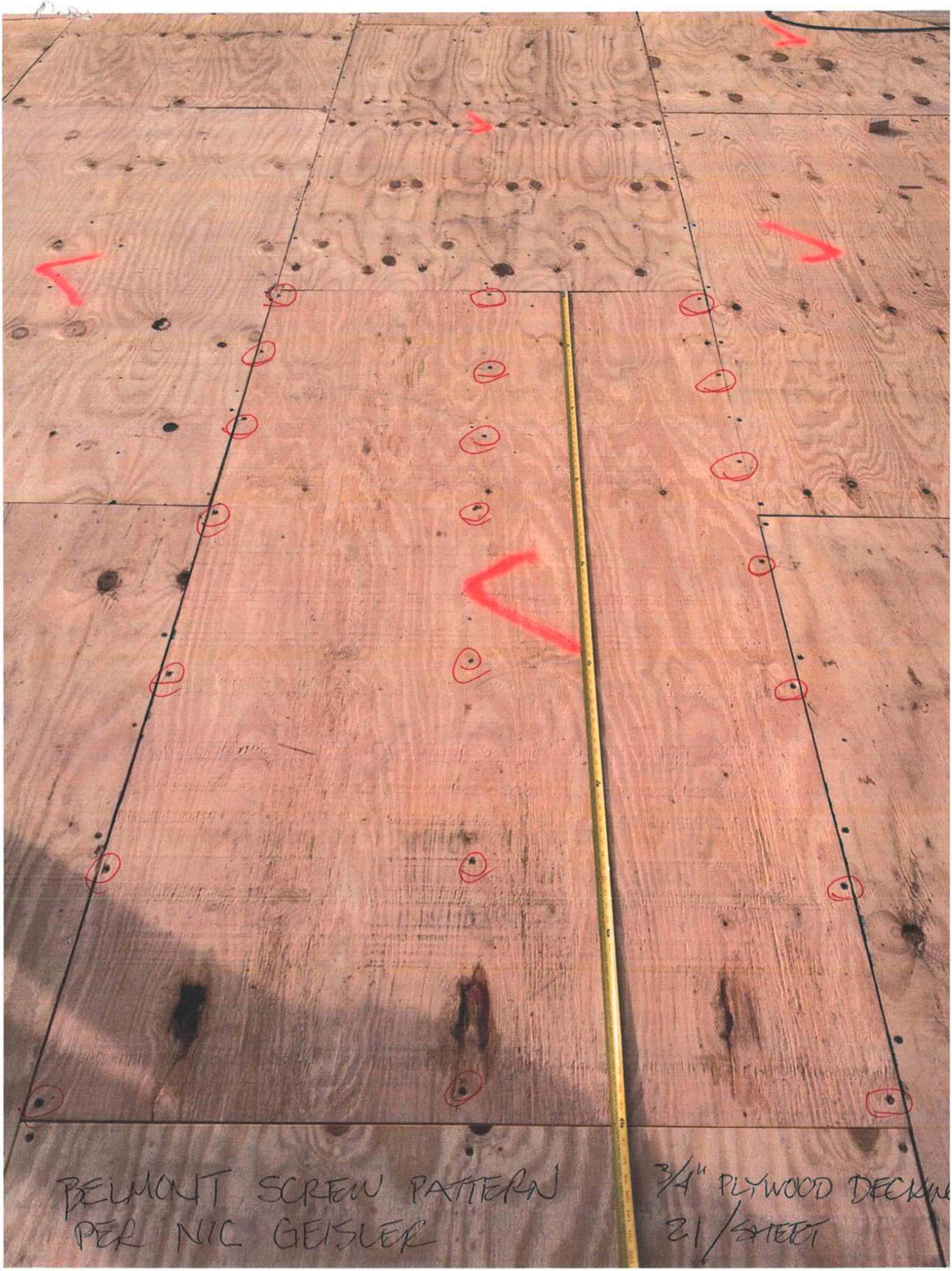
Ultimate Diaphragm Shear = 726 #/ft or 635 #/ft | Diaphragm Shear Safety Factor = 2.35

Use Ultimate Diaphragm Shear = 635 #/ft ← NOMINAL

Allowable Diaphragm Shear (w/uplift) Controlled by Fasteners = 270 #/ft

Result! Allowable Diaphragm Shear(w/uplift) = 270 #/ft | 0.0%Reduction in Allowable Diaphragm Shear Controlled by Fasteners


BELMONT SCREW PATTERN
PER MIC GEISLER
3/4" PLYWOOD DECKING
21 /SHEET


BELMONT SCREW PATTERN
PER NIC GEISLER
3/4" PLYWOOD DECKIN
21/SHEET

Jim Zuber

From: Z. Cliff Singleton [zcliff501@aol.com]
Sent: Tuesday, January 15, 2013 3:32 PM
To: jim@allseasonsplanning.com
Subject: FW: metal studs

From: Z. Cliff Singleton [mailto:zcliff501@aol.com]
Sent: Tuesday, January 15, 2013 3:16 PM
To: jim@allseasonplanning.com; russ.rocco@canam.ws
Subject: FW: metal studs

E-mail 2 0f 3

From: Mike King [mailto:m457king@gmail.com]
Sent: Tuesday, January 15, 2013 11:00 AM
To: Christian Troll; Cliff Singleton
Subject: Re: metal studs

Cliff

It is my opinion that one nuframe 3.5" 18ga wall and one 3.5" 20ga wall (total thickness 7") will work in place of walls requiring the 2-600S137-68 studs @ 48" o.c. under the joist and 1-600S137-33 20ga at 16' o.c. between columns The total strength of the wall nuframe wall with the 14 ga top plate is sufficiently more than the 6" 20ga wall specified.

And

Two nuframe 3.5 20ga walls in place of the one 1-600S137-33-20ga will also work

Mike King P.E.

On Tue, Jan 15, 2013 at 9:27 AM, Mike King <m457king@gmail.com> wrote:
Cliff here is the information on the studs

I have printed out the information on each of the stud and columns /studs under the joist

I used a program that has metal stud sections which you can select and see the max capacity of the studs
I used the 6" studs specified and found the maximum vertical load on that stud then found the equivalent 18 and 20ga 3.5" stud

I then printed out the results for 16 ft tall studs

1. 2-600S-137-68-14ga 10.9 kips equivalent column under joist 2-35S137-43-18ga 5.7 (2 rows)= 11.4 kips
2. 2-600S127-33-20 ga 5.2 kips equivalent column under joist 2-35S137-33-20ga 5.3 kips (2 rows)= 10.6 kips
3. wall studs @ 16" on Center single studs
   1-600S-137-33-20ga-wall 1850 lbs equivalent wall 1-35S137-33-20ga-wall 1900 lbs
4. single stud column
   1-600S-137-33-20ga 2.6 kips equivalent column under joist 1-35S137-33-20ga 2.6 kips

Note: the nuframe 3NF150 18 and 20 ga section properties is slightly less but the increased strength by using two rows of nuframe studs to make a 7" wall will be more than a single 6" stud

Mike King, P.E.

ATEG

--
Mike King, P.E.

J.M. King Distributing
ICF Direct Service Center
ATEG

Advanced Technology Engineering Group, LLC
6323 South Western Street
Amarillo TX 79110

PROJECT :
CLIENT :
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PAGE :
DESIGN BY :
REVIEW BY :

Jamb/Column Design Based on AISI 2001 & ICBO ER-4943P

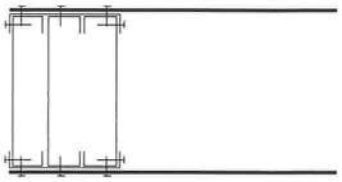
INPUT DATA & DESIGN SUMMARY

VERT. MEMBERS 2 x 600S137-68 (50 ksi)
(TOTAL SECTION: 6 x 3 , INSIDE 6 in THK. WALL)

HEIGHT h = 16 ft

SERVICE GRAVITY LOAD P = 10.9 kips
SERVICE LATERAL LOAD w = 0 kips / ft

DEFLECTION LIMITATION ? 0 No limit.
(0=No., 1= h /240, 2= h /360, 3= h /180, 4= h /120)



JAMB / WALL COLUMN SECTION

THE DESIGN IS ADEQUATE.

ANALYSIS

SECTION PROPERTIES OF EACH VERTICAL STUD (SSMA page 7 & 8)

thk =	0.0713	in	F_y =	50	ksi	I_{xx} =	3.094	in^4	M_n/Ω_b =	35.6	in-kips
t =	6	in	Wt =	2.18	lb/ft	S_{xx} =	1.029	in^3	V_n/Ω_v =	5468	lbs
A =	0.64	in^2	r_x =	2.2	in	r_y =	0.443	in	x_o =	-0.793	in
J =	0.001084	in^4	C_w =	0.911	in^6						

CHECK MAX WEB DEPTH-TO-THICKNESS RATIO (AISI B1.2)
t / (thk) = 84.15 < 200 [Satisfactory]

CHECK FLEXURAL CAPACITY (AISI C3.1)
M_n/Ω_b = 5.93 ft-kips **>** **M** **[Satisfactory]**
Where M = 0.75 w h^2 / 8 = 0.00 ft-kips, (0.75 for wind/seismic, from AISI App. A4.1.2, typical)

CHECK SHEAR CAPACITY (AISI C3.2)
V_n/Ω_v = 10.94 kips **>** **V** **[Satisfactory]**
Where V = 0.75 w L / 2 = 0.00 kips

CHECK COMPRESSION CAPACITY WITH, AT LEAST, ONE FLANGE THROUGH-FASTENED TO SHEATHING (AISI C4.6)
P_n/Ω_c = 11.04 kips **>** **P** **[Satisfactory]**
Where Ω_c = 1.8
$P_n = C_1C_2C_3$ AE / 29500 = 19.88 kips
C_1 = (0.79 x + 0.54) = 0.949
C_2 = (1.17 α t + 0.93) = 1.013
C_3 = α (2.5b - 1.63d) + 22.8 = 16.145
E = 29500 ksi (AISI pg 18)
P = 10.97 kips (included studs weight.)

CHECK CAPACITY COMBINED BENDING & SHEAR AT ANY SAME SECTION (AISI C3.3.1)

$$\left(\frac{\Omega_b M}{M_n}\right)^2+\left(\frac{\Omega_v V}{V_n}\right)^2 = 0 < 1.0$$ **[Satisfactory]**

Where M = 0.00 ft-kips, (0.75 included)
V = 0.00 kips, (0.75 included)
V_n/Ω_v = 10.94 kips

(cont'd)

CHECK CAPACITY COMBINED AXIAL LOAD & BENDING (AISI C5.2.1)

$$\frac{\Omega_c P}{P_n}+\frac{\Omega_b C_m M}{M_n \alpha}= \quad 0.99 \quad < \quad 1.0 \quad \textbf{[Satisfactory]}$$

Where M = 0.00 ft-kips, (0.75 included)
P = 10.97 kips
P_n/Ω_c = 11.04 kips
M_n/Ω_b = 5.93 in-kips
C_m = 1.0

$$P_{Ex} = \frac{\pi^2 E I_x}{(K_x L_x)^2} = \quad 48.87 \text{ kips}$$

$$\alpha = 1 - \frac{\Omega_c P}{P_{Ex}} = \quad 0.596$$

CHECK DEFLECTION

$$\Delta = \frac{5wL^4}{384EI} = \quad 0.00 \text{ in} \quad < \quad \text{No limit.} \quad \textbf{[Satisfactory]}$$

NOTE : 1. STUD FLANGES SHALL BE FASTENED TO SHEATHING AT EACH SIDE OF WALL BEFORE VERTICAL LOAD ADDED.

Technical References:

1. AISI STANDARD, 2001 Edition. American Iron and Steel Institute.
2. SSMA, Product Technical Information, ICBO ER-4943P, Steel Stud Manufactures Association, 2001.

Advanced Technology Engineering Group, LLC
6323 South Western Street
Amarillo TX 79110

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PAGE :
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REVIEW BY :

Jamb/Column Design Based on AISI 2001 & ICBO ER-4943P

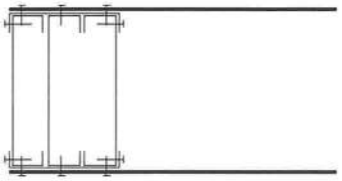
INPUT DATA & DESIGN SUMMARY

VERT. MEMBERS 1 x 600S137-33
(TOTAL SECTION: 6 x 2 , INSIDE 6 in THK. WALL)

HEIGHT h = 16 ft

SERVICE GRAVITY LOAD P = 2.6 kips
SERVICE LATERAL LOAD w = 0 kips / ft

DEFLECTION LIMITATION ? 0 No limit.
(0=No., 1= h /240, 2= h /360, 3= h /180, 4= h /120)



JAMB / WALL COLUMN SECTION

THE DESIGN IS ADEQUATE.

ANALYSIS

SECTION PROPERTIES OF EACH VERTICAL STUD (SSMA page 7 & 8)

thk =	0.0346	in	F_y =	33	ksi	I_{xx} =	1.582	in^4	M_n/Ω_b =	10.07	in-kips
t =	6	in	Wt =	1.08	lb/ft	S_{xx} =	0.51	in^3	V_n/Ω_v =	612	lbs
A =	0.318	in^2	r_x =	2.229	in	r_y =	0.464	in	x_o =	-0.823	in
J =	0.000127	in^4	C_w =	0.493	in^6						

CHECK MAX WEB DEPTH-TO-THICKNESS RATIO (AISI B1.2)
t / (thk) = 173.41 < 200 [Satisfactory]

CHECK FLEXURAL CAPACITY (AISI C3.1)
M_n/Ω_b = 0.84 ft-kips **>** **M** **[Satisfactory]**
Where M = 0.75 w h^2 / 8 = 0.00 ft-kips, (0.75 for wind/seismic, from AISI App. A4.1.2, typical)

CHECK SHEAR CAPACITY (AISI C3.2)
V_n/Ω_V = 0.61 kips **>** **V** **[Satisfactory]**
Where V = 0.75 w L / 2 = 0.00 kips

CHECK COMPRESSION CAPACITY WITH, AT LEAST, ONE FLANGE THROUGH-FASTENED TO SHEATHING (AISI C4.6)
P_n/Ω_c = 2.63 kips **>** **P** **[Satisfactory]**
Where Ω_c = 1.8
$P_n = C_1C_2C_3$ AE / 29500 = 4.73 kips
C_1 = (0.79 x + 0.54) = 0.949
C_2 = (1.17 α t + 0.93) = 0.970
C_3 = α (2.5b - 1.63d) + 22.8 = 16.145
E = 29500 ksi (AISI pg 18)
P = 2.62 kips (included studs weight.)

CHECK CAPACITY COMBINED BENDING & SHEAR AT ANY SAME SECTION (AISI C3.3.1)

$$\left(\frac{\Omega_b M}{M_n}\right)^2+\left(\frac{\Omega_v V}{V_n}\right)^2 = 0 < 1.0$$ **[Satisfactory]**

Where M = 0.00 ft-kips, (0.75 included)
V = 0.00 kips, (0.75 included)
V_n/Ω_v = 0.61 kips

(cont'd)

CHECK CAPACITY COMBINED AXIAL LOAD & BENDING (AISI C5.2.1)

$$\frac{\Omega_c P}{P_n}+\frac{\Omega_b C_m M}{M_n \alpha}= \quad 1.00 \quad < \quad 1.0 \quad \textbf{[Satisfactory]}$$

Where M = 0.00 ft-kips, (0.75 included)
P = 2.62 kips
P_n/Ω_c = 2.63 kips
M_n/Ω_b = 0.84 in-kips
C_m = 1.0

$$P_{Ex} = \frac{\pi^2 E I_x}{(K_x L_x)^2} = \quad 12.49 \text{ kips}$$

$$\alpha = 1 - \frac{\Omega_c P}{P_{Ex}} = \quad 0.623$$

CHECK DEFLECTION

$$\Delta = \frac{5wL^4}{384EI} = \quad 0.00 \text{ in} \quad < \quad \text{No limit.} \quad \textbf{[Satisfactory]}$$

NOTE : 1. STUD FLANGES SHALL BE FASTENED TO SHEATHING AT EACH SIDE OF WALL BEFORE VERTICAL LOAD ADDED.

Technical References:

1. AISI STANDARD, 2001 Edition. American Iron and Steel Institute.
2. SSMA, Product Technical Information, ICBO ER-4943P, Steel Stud Manufactures Association, 2001.

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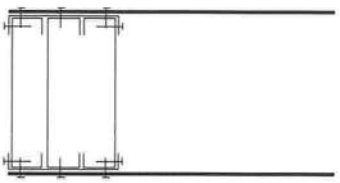

JAMB / WALL COLUMN SECTION

VERT. MEMBERS 2 x 350S125-43
(TOTAL SECTION: 3.5 x 3 , INSIDE 3.5 in THK. WALL)

HEIGHT h = 16 ft

SERVICE GRAVITY LOAD P = 5.7 kips
SERVICE LATERAL LOAD w = 0 kips / ft

DEFLECTION LIMITATION ? 0 No limit.
(0=No., 1= h /240, 2= h /360, 3= h /180, 4= h /120)

THE DESIGN IS ADEQUATE.

ANALYSIS

SECTION PROPERTIES OF EACH VERTICAL STUD (SSMA page 7 & 8)

thk =	0.0451	in	F_y =	33	ksi	I_{xx} =	0.493	in^4	M_n/Ω_b =	5.37	in-kips
t =	3.5	in	Wt =	0.93	lb/ft	S_{xx} =	0.272	in^3	V_n/Ω_v =	1777	lbs
A =	0.272	in^2	r_x =	1.352	in	r_y =	0.41	in	x_o =	-0.79	in
J =	0.000184	in^4	C_w =	0.106	in^6						

CHECK MAX WEB DEPTH-TO-THICKNESS RATIO (AISI B1.2)
t / (thk) = 77.61 < 200 [Satisfactory]

CHECK FLEXURAL CAPACITY (AISI C3.1)
M_n/Ω_b = 0.90 ft-kips **>** **M** **[Satisfactory]**
Where M = 0.75 w h^2 / 8 = 0.00 ft-kips, (0.75 for wind/seismic, from AISI App. A4.1.2, typical)

CHECK SHEAR CAPACITY (AISI C3.2)
V_n/Ω_V = 3.55 kips **>** **V** **[Satisfactory]**
Where V = 0.75 w L / 2 = 0.00 kips

CHECK COMPRESSION CAPACITY WITH, AT LEAST, ONE FLANGE THROUGH-FASTENED TO SHEATHING (AISI C4.6)
P_n/Ω_c = 5.76 kips **>** **P** **[Satisfactory]**
Where Ω_c = 1.8
$P_n = C_1C_2C_3$ AE / 29500 = 10.37 kips
C_1 = (0.79 x + 0.54) = 0.959
C_2 = (1.17 α t + 0.93) = 0.983
C_3 = α (2.5b - 1.63d) + 22.8 = 20.220
E = 29500 ksi (AISI pg 18)
P = 5.73 kips (included studs weight.)

CHECK CAPACITY COMBINED BENDING & SHEAR AT ANY SAME SECTION (AISI C3.3.1)

$$\left(\frac{\Omega_b M}{M_n}\right)^2+\left(\frac{\Omega_v V}{V_n}\right)^2 = 0 < 1.0$$ **[Satisfactory]**

Where M = 0.00 ft-kips, (0.75 included)
V = 0.00 kips, (0.75 included)
V_n/Ω_v = 3.55 kips

(cont'd)

CHECK CAPACITY COMBINED AXIAL LOAD & BENDING (AISI C5.2.1)

$$\frac{\Omega_c P}{P_n}+\frac{\Omega_b C_m M}{M_n \alpha}= \quad 0.99 \quad < \quad 1.0 \quad \textbf{[Satisfactory]}$$

Where M = 0.00 ft-kips, (0.75 included)
P = 5.73 kips
P_n/Ω_c = 5.76 kips
M_n/Ω_b = 0.90 in-kips
C_m = 1.0

$$P_{Ex} = \frac{\pi^2 E I_x}{(K_x L_x)^2} = \quad 7.79 \text{ kips}$$

$$\alpha = 1 - \frac{\Omega_c P}{P_{Ex}} = \quad -0.324$$

CHECK DEFLECTION

$$\Delta = \frac{5 w L^4}{384 EI} = \quad 0.00 \text{ in} \quad < \quad \text{No limit.} \quad \textbf{[Satisfactory]}$$

NOTE : 1. STUD FLANGES SHALL BE FASTENED TO SHEATHING AT EACH SIDE OF WALL BEFORE VERTICAL LOAD ADDED.

Technical References:

1. AISI STANDARD, 2001 Edition. American Iron and Steel Institute.
2. SSMA, Product Technical Information, ICBO ER-4943P, Steel Stud Manufactures Association, 2001.

Owner's Information Certificate

Name / Address of property to be protected with sprinkler protection:

MOUNTAINTOP MINISTRIES WORLDWIDE – MULTIPURPOSE BLDG.
1476 SW WALTER AVE.
LAKE CITY, FL. 32024

Name of Owner: MOUNTAINTOP MINISTRIES WORLDWIDE

Existing or planned construction is:

- [x] Fire resistive or noncombustible
- [ ] Wood frame or ordinary (masonry walls with wood beams)
- [ ] Unknown

Is the system installation intended for one of the following special occupancies:

Aircraft hangar	[ ] yes	[x] no
Fixed guide way transit system	[ ] yes	[x] no
Racetrack stable	[ ] yes	[x] no
Marine terminal, pier, or wharf	[ ] yes	[x] no
Airport terminal	[ ] yes	[x] no
Aircraft engine test facility	[ ] yes	[x] no
Power plant	[ ] yes	[x] no
Water-cooling tower	[ ] yes	[x] no

If the answer to any of the above is "yes," the appropriate NFPA standard shall be referenced for sprinkler density/area criteria.

Indicate whether any of the following special materials are intended to be present:

Flammable or combustible liquids	[ ] yes	[x] no
Aerosol products	[ ] yes	[x] no
Nitrate film	[ ] yes	[x] no
Pyroxylin plastic	[ ] yes	[x] no
Compressed or liquefied gas cylinders	[ ] yes	[x] no
Liquid or solid oxidizers	[ ] yes	[x] no
Organic peroxide formulations	[ ] yes	[x] no
Idle pallets	[ ] yes	[x] no

If the answer to any of the above is "yes," describe type, location, arrangement, and intended maximum quantities.

Indicate whether the protection is intended for one of the following specialized occupancies or areas:

Spray area or mixing room	[ ] yes	[x] no
Solvent extraction	[ ] yes	[x] no
Laboratory using chemicals	[ ] yes	[x] no
Oxygen-fuel gas system for welding or cutting	[ ] yes	[x] no
Acetylene cylinder charging	[ ] yes	[x] no

16

Production or use of compressed or liquefied gas	☐ yes ☑ no
Commercial cooking operation	☐ yes ☑ no
Class A hyperbaric chamber	☐ yes ☑ no
Cleanroom	☐ yes ☑ no
Incinerator or waste handling system	☐ yes ☑ no
Linen handling system	☐ yes ☑ no
Industrial furnace	☐ yes ☑ no
Water-cooling tower	☐ yes ☑ no

If the answer to any of the above is "yes," describe type, location, arrangement, and intended maximum quantities.

Does the project contain organized storage: ☐ yes ☑ no

If the answer to the above is "yes," complete the following for a description of the storage arrangement – material handling operations as follows:

1. Provide the name of each type of commodity.
2. Provide the method of material handling operation; i.e. forklift (elect., propane) or pallet jack, etc.
3. Provide pallet type: ☐ wood ☐ metal ☐ expanded plastic ☐ non-expanded plastic

Will there be any storage of products over 12 ft. in height? ☐ yes ☐ no

If the answer is "yes," describe product, intended storage arrangement, and height.

Will there be any storage of plastic, rubber, or similar products over 5 ft. high except as described above? ☐ yes ☐ no

If the answer is "yes," describe product, intended storage arrangement, and height.

I certify that I have knowledge of the intended use of the property and that the above information is correct:

Signature of owner's representative or agent: _____ Date: 9/19/12

Name of owner's representative or agent completing certificate (print):
Relationship and firm of agent (print):
GENERAL CONTRACTOR AND AUTHORIZED AGENT

Owner's Information Certificate

Name / Address of property to be protected with sprinkler protection:

MOUNTAINTOP MINISTRIES WORLDWIDE – MULTIPURPOSE BLDG.
1476 SW WALTER AVE.
LAKE CITY, FL. 32024

Name of Owner: MOUNTAINTOP MINISTRIES WORLDWIDE

Existing or planned construction is:

- [x] Fire resistive or noncombustible
- [ ] Wood frame or ordinary (masonry walls with wood beams)
- [ ] Unknown

Is the system installation intended for one of the following special occupancies:

Aircraft hangar	[ ] yes	[x] no
Fixed guide way transit system	[ ] yes	[x] no
Racetrack stable	[ ] yes	[x] no
Marine terminal, pier, or wharf	[ ] yes	[x] no
Airport terminal	[ ] yes	[x] no
Aircraft engine test facility	[ ] yes	[x] no
Power plant	[ ] yes	[x] no
Water-cooling tower	[ ] yes	[x] no

If the answer to any of the above is "yes," the appropriate NFPA standard shall be referenced for sprinkler density/area criteria.

Indicate whether any of the following special materials are intended to be present:

Flammable or combustible liquids	[ ] yes	[x] no
Aerosol products	[ ] yes	[x] no
Nitrate film	[ ] yes	[x] no
Pyroxylin plastic	[ ] yes	[x] no
Compressed or liquefied gas cylinders	[ ] yes	[x] no
Liquid or solid oxidizers	[ ] yes	[x] no
Organic peroxide formulations	[ ] yes	[x] no
Idle pallets	[ ] yes	[x] no

If the answer to any of the above is "yes," describe type, location, arrangement, and intended maximum quantities.

Indicate whether the protection is intended for one of the following specialized occupancies or areas:

Spray area or mixing room	[ ] yes	[x] no
Solvent extraction	[ ] yes	[x] no
Laboratory using chemicals	[ ] yes	[x] no
Oxygen-fuel gas system for welding or cutting	[ ] yes	[x] no
Acetylene cylinder charging	[ ] yes	[x] no

Production or use of compressed or liquefied gas	☐ yes ☑ no
Commercial cooking operation	☐ yes ☑ no
Class A hyperbaric chamber	☐ yes ☑ no
Cleanroom	☐ yes ☑ no
Incinerator or waste handling system	☐ yes ☑ no
Linen handling system	☐ yes ☑ no
Industrial furnace	☐ yes ☑ no
Water-cooling tower	☐ yes ☑ no

If the answer to any of the above is "yes," describe type, location, arrangement, and intended maximum quantities.

Does the project contain organized storage: ☐ yes ☑ no

If the answer to the above is "yes," complete the following for a description of the storage arrangement – material handling operations as follows:

1. Provide the name of each type of commodity.
2. Provide the method of material handling operation; i.e. forklift (elect., propane) or pallet jack, etc.
3. Provide pallet type: ☐ wood ☐ metal ☐ expanded plastic ☐ non-expanded plastic

Will there be any storage of products over 12 ft. in height? ☐ yes ☐ no

If the answer is "yes," describe product, intended storage arrangement, and height.

Will there be any storage of plastic, rubber, or similar products over 5 ft. high except as described above? ☐ yes ☐ no

If the answer is "yes," describe product, intended storage arrangement, and height.

I certify that I have knowledge of the intended use of the property and that the above information is correct:

Signature of owner's representative or agent: _____ Date: 9/19/12

Name of owner's representative or agent completing certificate (print):
Relationship and firm of agent (print):
GENERAL CONTRACTOR AND AUTHORIZED AGENT