

FLOOR PLAN SCALE: 1/4" = 1'-0"

MODEL 1404 FOR:

MALE ROBERTS

PROJECT ADDRESS:

©VILLIAM MYERS

DESIGN

P.O. BOX 1513

LAKE CITY, FL 32056

(386) 758-8406

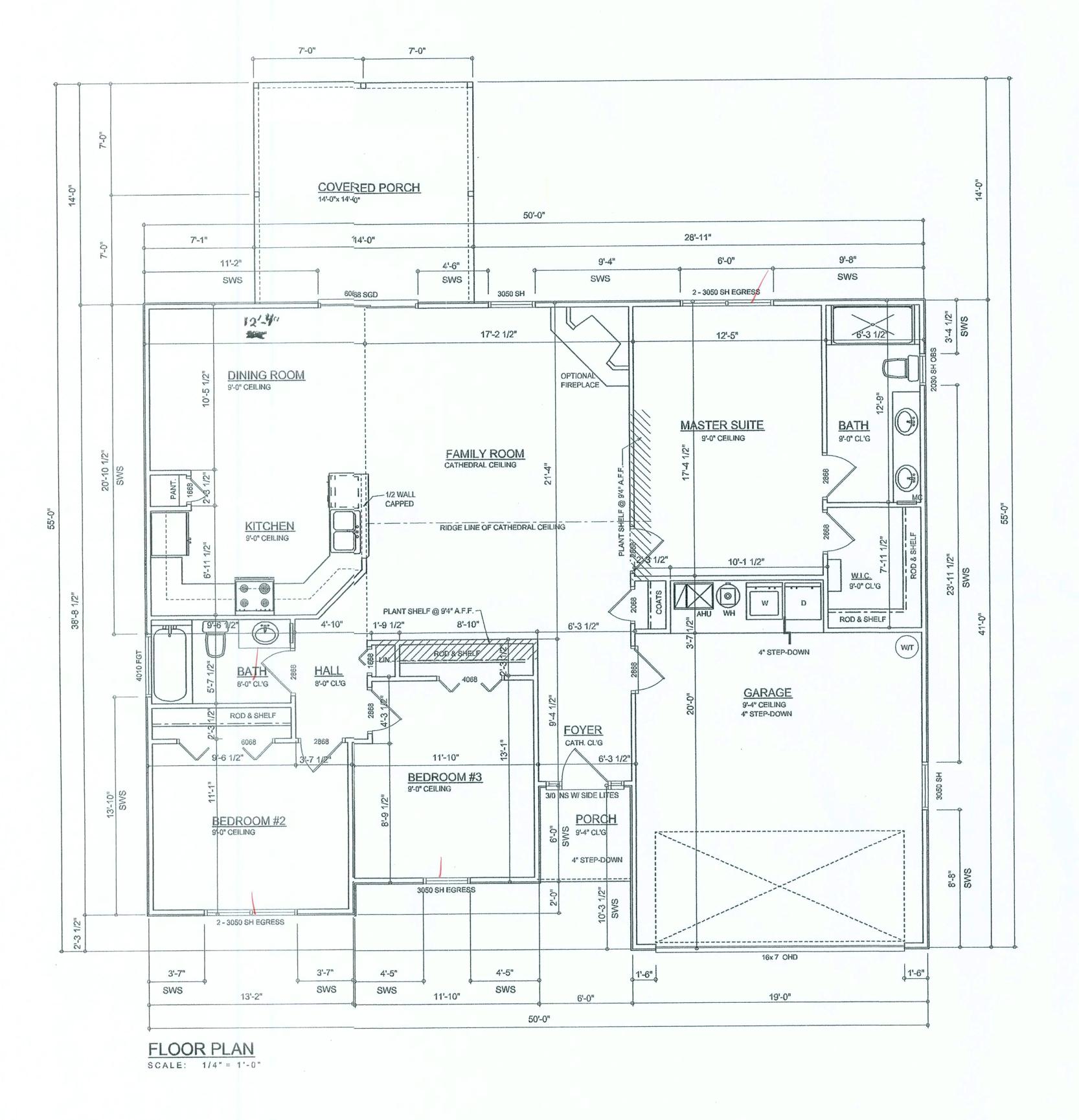
will@willmyers.net



JOB NUMBER 050113

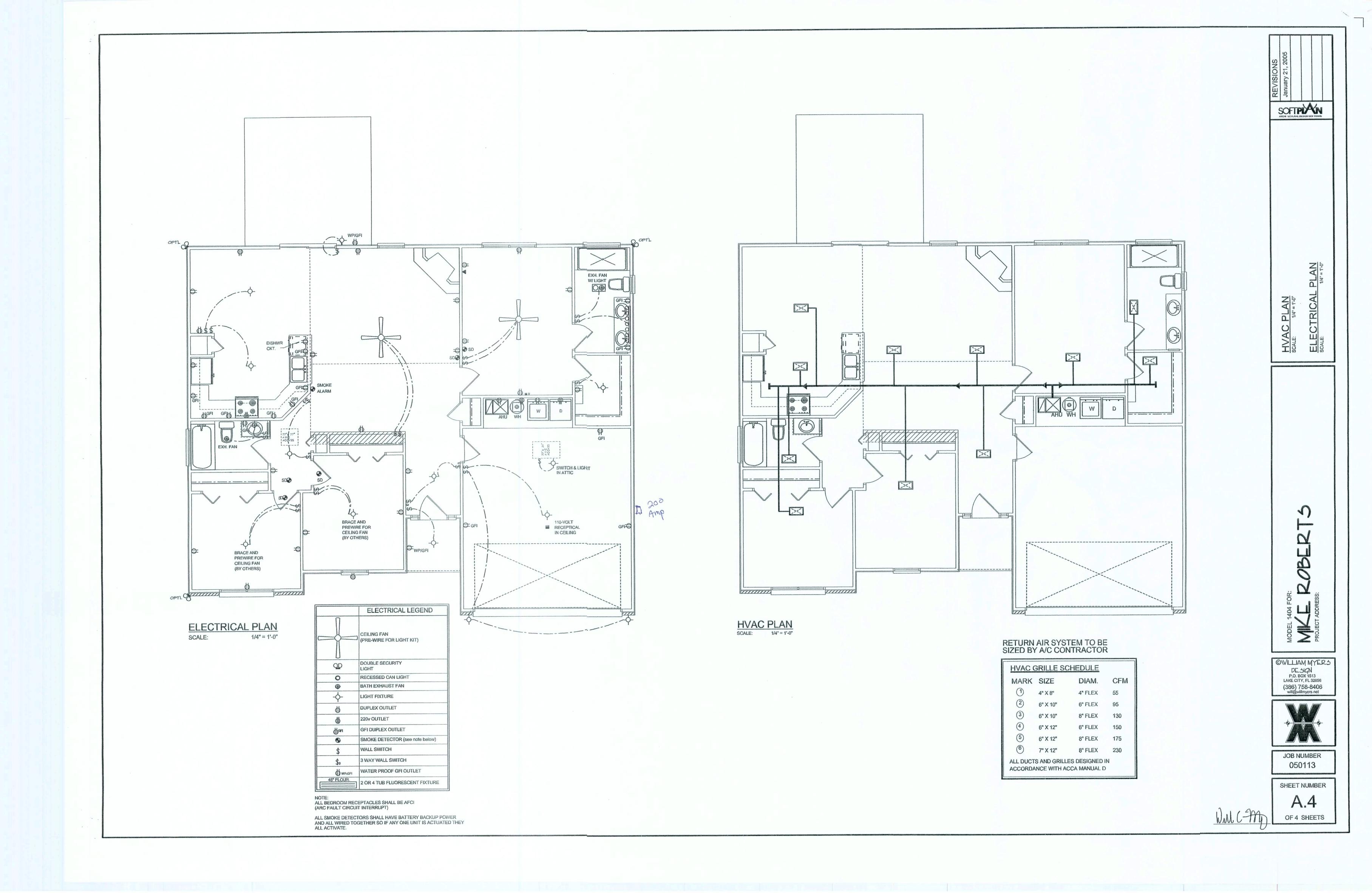
A.3

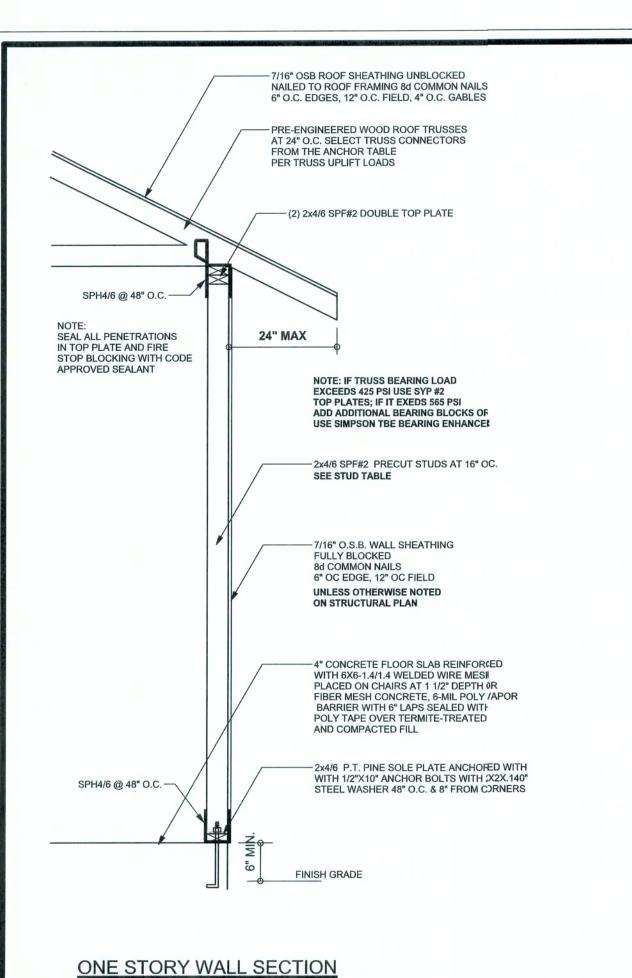
OF 4 SHEETS



AREA SUMMARY

) ( ] ( Law ) ( G G G III III		
LIVING AREA	1495	S.
GARAGE AREA	410	S.
PORCH AREA	196	S.
<b>ENTRY PORCH AREA</b>	36	S.
TOTAL AREA	2137	S.





SCALE: 3/4" = 1'-0"

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS** 

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

TO 18'-10' STUD HEIGHT

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

(2) 2X12 SYP #2 MIN. ---SEE STRUCTURAL PLAN

SIMPSON HUS412 MIN. -

SEE STRUCTURAL PLAN

-SIMPSON LSTA24

BEAM W/4-16d

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

- SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

LSTA18 -

- (4)-2x4 SPF #2 NAILED

TOGETHER W/2-16d NAILS AT 16" O.C.

**BEAM MID-WALL CONNECTION DETAIL** 

MIN. (SEE STRUCTURAL PLAN)

-(2) 2X12 SYP #2 MIN. ----

SÉE STRUCTURAL PLAN

(1) 2x6 @ 16" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O. — SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

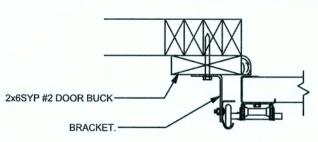
AND (8) -16d TO POST

#### — (2) 2X4 SPF #2 TOP PLATE (2) SIMPSON SPH4 w/ (6) - 10d--SIMPSON SPH4 @ 48" O.C. (2) SIMPSON LSTA21w/ (8) -16d TO HEADER AND (8) -16d TO STUD PACK — (2) 2X12 SYP #2 HEADER U.N.O SEE STRUCTURAL PLAN (2) KINGS STUDS — -(2) JACKS STUDS w/ (2) ROWS 10d @ w/ (2) ROWS 10d @ 12" O.C. EACH SIDE 12" O.C. EACH SIDE SIMPSON LTTI31 w/ (18) - 10d & 5/8" x 10" ANCHOR BOLT -FOUNDATION SEE SEE FOOTING DETAILS

TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

#### 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK, HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

OOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.

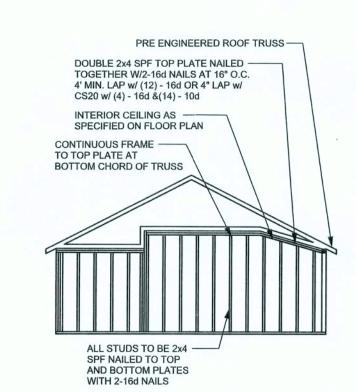


GN PER TABLE BELOW:

**GARAGE DOOR BUCK INSTALLATION DETAIL** SCALE: N.T.S.

#### **GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



#### CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCALE: N.T.S.

### ARE NOT REQUIRED -(6) .131 x 3 1/4" GUN NAILS -SPH4/6 ALL OPENINGS (U.N.O.)-INTO KING STUD INTO KING STUD ✓ SPH4/6 @ 48" O.C. (U.N.O.) CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS - TOE NAILED THRU SILL -INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) --- SPH4/6 ALL OPENINGS (U.N.O.)----(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### **BUILDER'S RESPONSIBILITY**

CONFIRM SITE CONDIT	IONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIN	D SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS A	ND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
	HE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUO	JS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
THE WIND LOAD ENGIN	ITS A CONTINUOUS LOAD PATH CONNECTION, CALL EER IMMEDIATELY.
	NUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
TRUSS-TO-TRUSS CON	LANS, TEMPORARY AND PERMANENT BRACING DETAILS, NECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.	NEOTIONS, AND OF EIL I AND NEACTION EDADS FOR ALL

#### **ROOF SYSTEM DESIGN**

П	THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
Ш	R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
Ш	TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
Ш	THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
Ш	COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
Ш	MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
ı	PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED
ı	LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
l	REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
ı	SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
l	BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
l	DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
١	RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
l	TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
١	RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
۱	TRUSS SHEETS.

**MASONRY NOTES:** 

Grout

3.3.E.7 Movement joints

CMU standard

ACI530.1-02 Section

Clay brick standard

Reinforcing bars, #3 - #11

2.4F Coating for corrosion protection

Coating for corrosion protection

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

ASTM C 270, Type N, UNO

5.5"x2.75"x11.5"

or 304SS

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

#### **ANCHOR TABLE** OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d	-	
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d	-	
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
	< 2300	ABU66	12-16d		1/2" AB
< 2300			10U		114-7363

#### **DESIGN DATA**

																	DENTL	_				
(EN	CI	LOSE	D S	IMF	LE	DIA	\PI	HF	RA	GM	/ BL	JILE	DIN	GS W	/ITH	FLA	T, HIP	PED,	OR	G/	ABLE	ROO
ON	LII	PPF	SHA	I F	OF	HII	וכ	OF	ν ν	-50		NG I	LEA	1 1 CF	TOKI,	ZUI	P. B, 3	DIME	1210	N	OR 6	UFI;
SLC	P	EAN	D U	NO	BST	RU	IC.	TE	D	UP	>WI	ND	FO	R 50	K HEI	GH	T OR 1	MIL	E WH	110	CHEV	ER IS
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5.)	F	ROOF	AN	IGLI	E = '	10-	45	D	EG	3RI	EES	3										
6.)	٨	/IEAN	IRC	OF	HEI	GH	IT	= •	<30	0 F	Т											
7.)	11	NTE	RNA	L PI	RES	SU	RE	Ξ (	CO	EF	FIC	IEN	IT =	= N/A	(EN	CLC	SED E	BUILD	ING)	)		
8.)	C	COMI	PON	EN	TS A	NE	0 0	CL	AD	DII	NG	DE	SIG	SN W	IND F	PRE	SSUR	ES (1	TABL	E	R301	.2(2)
																Т	Zone	Effe	ctive \	Wi	ind Ar	ea (ft:
		X		A														-	10			100
			$\times$	汝		//										T	1	19.9	-21.	8	18.1	-18.
	1	4	2	>	2			1									2	19.9	-25.	5	18.1	-21.
	5		\	1		1	_	2	1	X	3						2 O'hg		-40.	6		-40.
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						5 5	/										5	218	-29.	1	18.5	-22.

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

Zone	Ellec	no Area (112)				
	1	0		100		
1	19.9	-21.8	18.1	-18.1		
2	19.9	-25.5	18.1	-21.8		
2 O'hg		-40.6		-40.6		
3	19.9	-25.5	18.1	-21.8		
3 O'hg		-68.3		-42.4		
4	21.8	-23.6	18.5	-20.4		
5	21.8	-29.1	18.5	-22.6		
Doors & Wors (Zone	st Cas	е	21.8	-29.1		
8x7 Gara	age D	oor	19.5	-22.9		
16x7 Ga	rage [	Door	18.5	-21.0		

Lake City, Florida 32056

December 11, 2007 STRUCTURAL BY

FINALS DATE: 04 / Dec / 07 JOB NUMBER:

> 712041 **S-1**

Mike Roberts

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL

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ERTIFICATION: I hereby certify that I have mined this plan, and that the applicable portions of the plan, relating to wind engineer comply with section R301.2.1, florida building

code residential 2004, to the best of my

IMITATION: This design is valid for one

MARK DISOSWAY P.E. 53915

building, at specified location.

ission and consent of Mark Disosway.

fark Disosway, P.E. hereby expressly reserve ts common law copyrights and property right ese instruments of service. This document is

tated dimensions supercede scaled

imensions. Refer all questions to

lark Disosway, P.E. for resolution

Do not proceed without clarification

32056, 386-754-5419

REVISIONS

SOFTPIAN

Spec House Lot 12

> Phase 1 ADDRESS:

Crosswinds S/D

833 SW Chesterfield Lake City, Florida 32024 Mark Disosway P.E. P.O. Box 868

Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE:

DRAWING NUMBER

OF 3 SHEETS

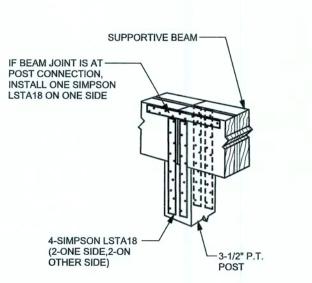
#### IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. (6) .131 x 3 1/4" GUN NAILS ---TOE NAILED THRU HEADER 3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON ----OPPOSITE SIDE) EA. NAILED WITH 14-10d SUPPORTIVE POST TO BEAM

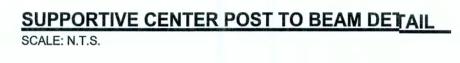
-NON-SUPPORTIVE

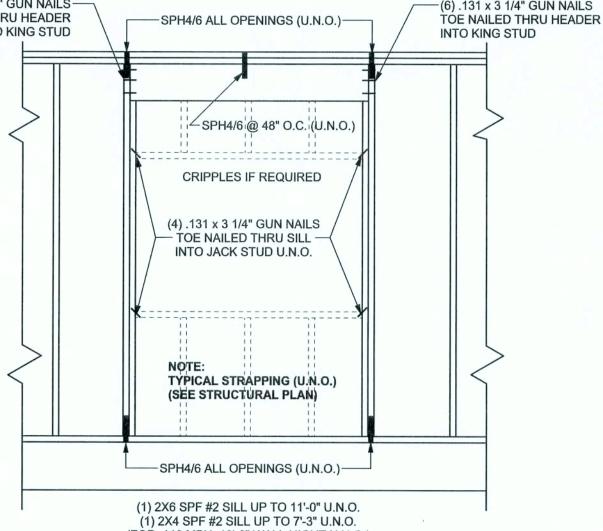
2X4 LADDER BEAM



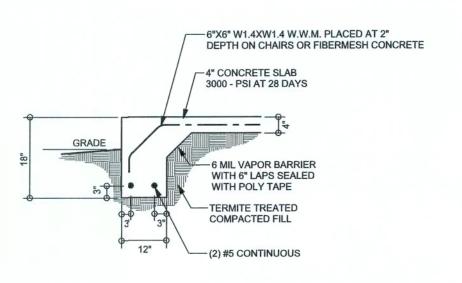
SUPPORTIVE -**BEAMS** 



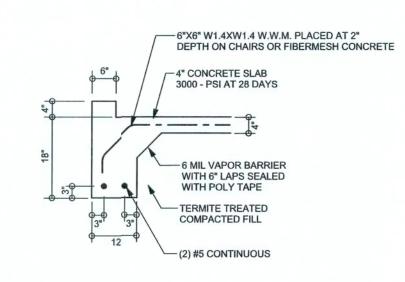




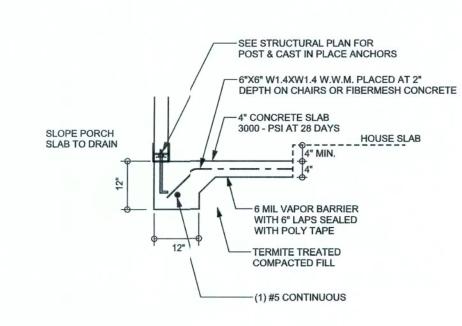
TYPICAL HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"



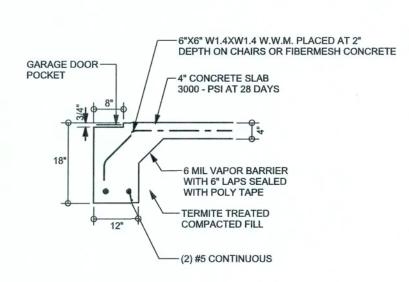
## F1 MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



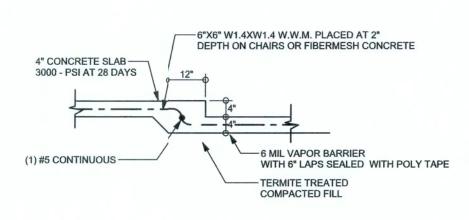
### F8 GARAGE CURB FOOTING S-2 SCALE: 1/2" = 1'-0"



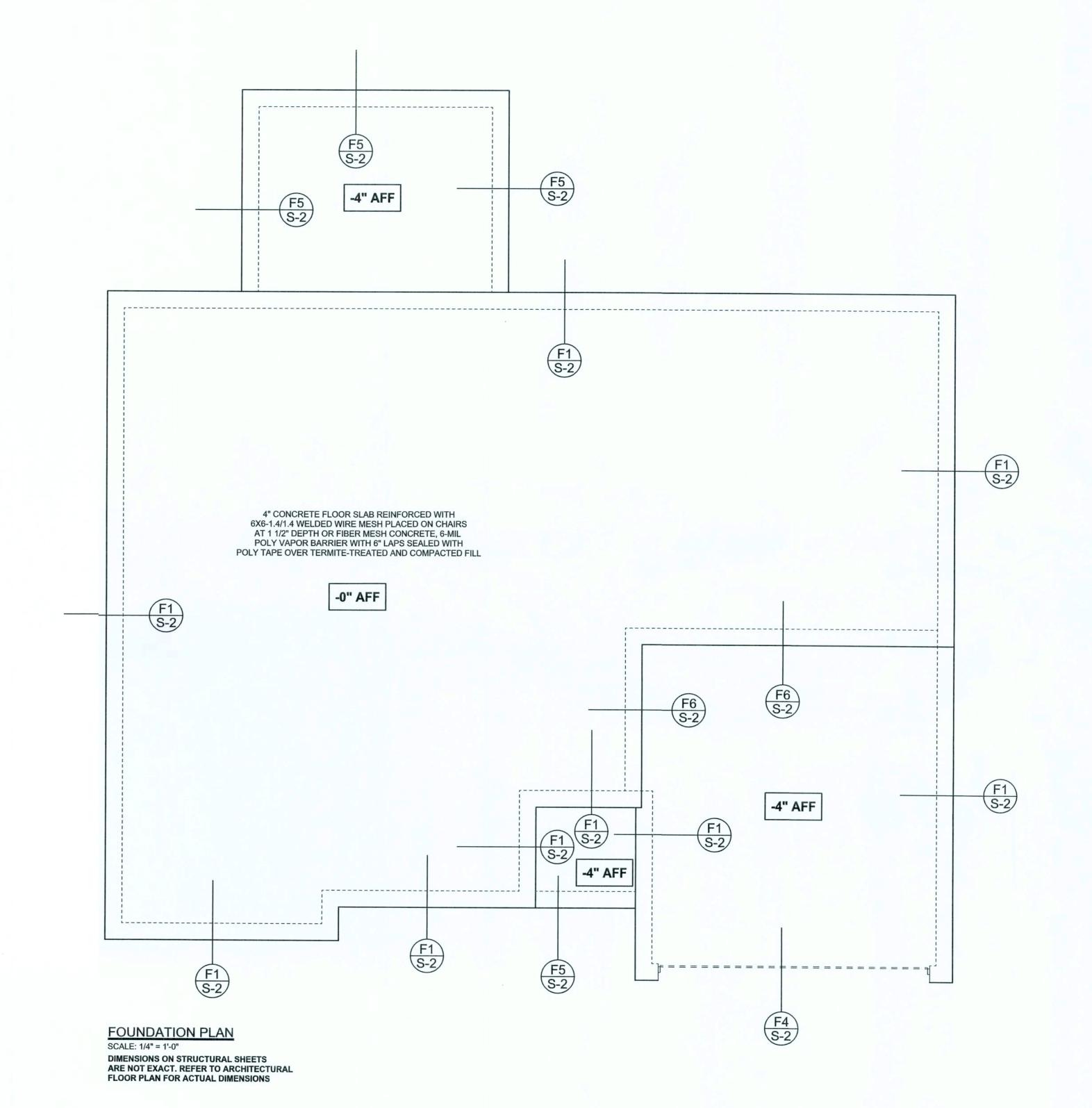
#### F5 PORCH FOOTING S-2 \$CALE: 1/2" = 1'-0"



# F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"







REVISIONS

SOFTPINA ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Mike Roberts

Spec House
Lot 12
Crosswinds S/D
Phase 1

ADDRESS:
833 SW Chesterfield
Lake City, Florida 32024

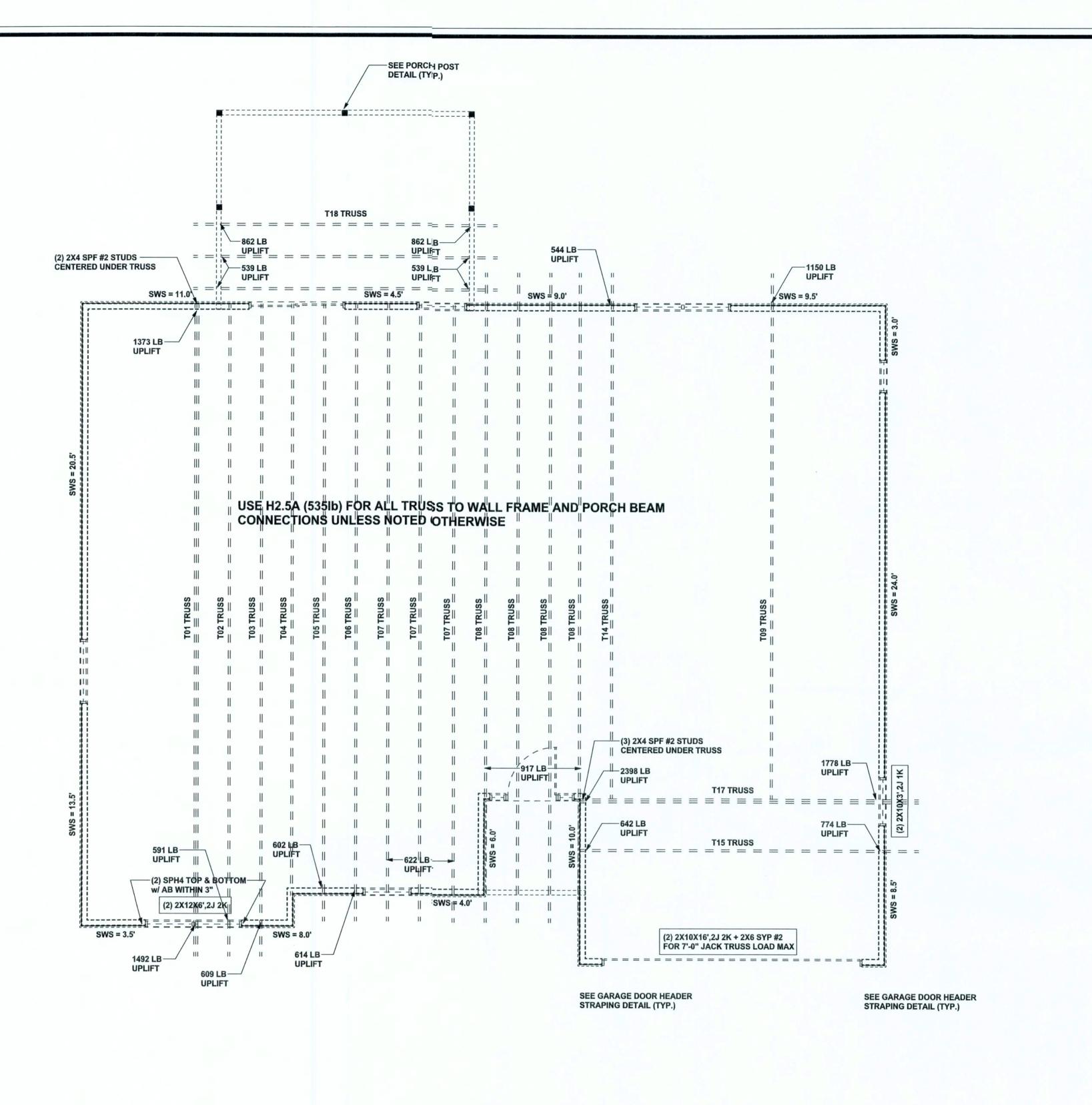
Mark Disosway P.E.
P.O. Box 868

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

December 11, 2007
STRUCTURAL BY:

FINALS DATE: 04 / Dec / 07 JOB NUMBER 712041

S-2
OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

#### STRUCTURAL PLAN NOTES

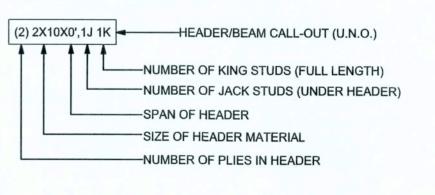
- ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### WALL LEGEND

sws = 0,0'	1ST FLOOR EXTERIOR WALL
SWS = 0 <sub>1,0</sub> ,	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

#### **HEADER LEGEND**



#### TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	85.5'
LONGITUDINAL	28.6'	49.5'

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

**REVISIONS** 

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#### Mike Roberts

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> PRINTED DATE: December 11, 2007 STRUCTURAL BY:

FINALS DATE:

04 / Dec / 07 JOB NUMBER: 712041

DRAWING NUMBER CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE

JOB #L229281

**S-3** OF 3 SHEETS