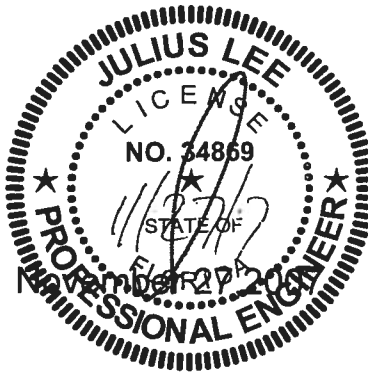
Builders FirstSource



Project Information for: L261921

Address:	414 Southwest Santucknee Terrace
...	Ft. White, Florida
County:	Columbia
Truss Count:	15
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002

Truss Design Load Information:

Gravity:	Wind:	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James H. Johnston, III Florida Registered Residential Contractor License No. RR0066976

Address: RT. #15 Box 3693 Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1912260	CJ1	11/27/07
2	J1912261	CJ3	11/27/07
3	J1912262	CJ5	11/27/07
4	J1912263	EJ5	11/27/07
5	J1912264	EJ7	11/27/07
6	J1912265	HIP1	11/27/07
7	J1912266	HJ7	11/27/07
8	J1912267	HJ9	11/27/07
9	J1912268	T01	11/27/07
10	J1912269	T02	11/27/07
11	J1912270	T03	11/27/07
12	J1912271	T04	11/27/07
13	J1912272	T05	11/27/07
14	J1912273	T06	11/27/07
15	J1912274	T06A	11/27/07

Project Information for: **L261921**

Address: 414 Southwest Santucknee Terrace
Ft. White, Florida
County: Columbia
Truss Count: 15
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

November 27,2007

Truss Design Load Information:

Gravity:
Roof (psf): 42.0
Floor (psf): N/A

Wind:
Wind Standard: ASCE 7-02
Wind Speed (mph): 110
Wind Exposure: B

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
James H. Johnston, III Florida Registered Residential Contractor License No. RR0066976
Address: RT. #15 Box 3693 Lake City, Florida 32024

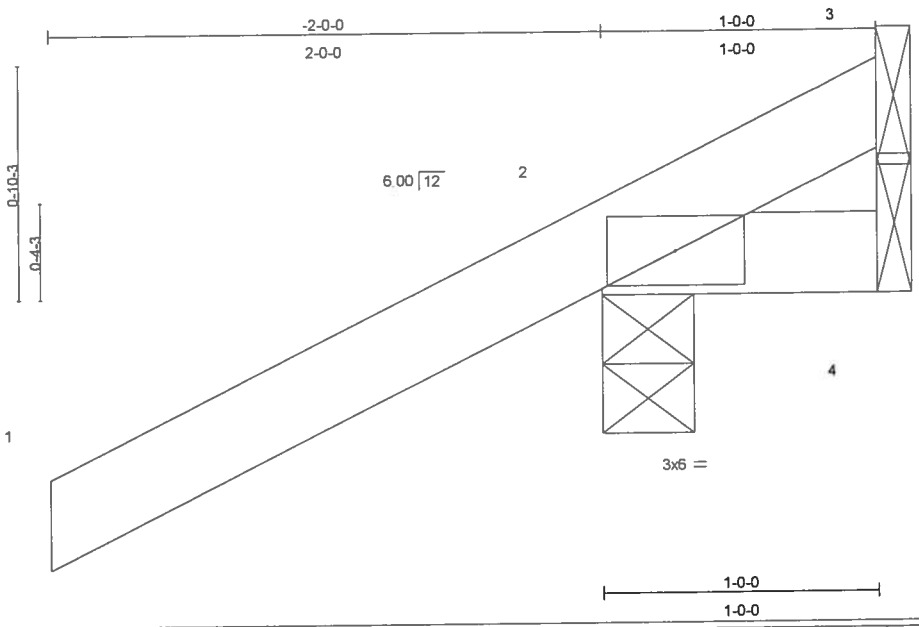
Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1912260	CJ1	11/27/07
2	J1912261	CJ3	11/27/07
3	J1912262	CJ5	11/27/07
4	J1912263	EJ5	11/27/07
5	J1912264	EJ7	11/27/07
6	J1912265	HIP1	11/27/07
7	J1912266	HJ7	11/27/07
8	J1912267	HJ9	11/27/07
9	J1912268	T01	11/27/07
10	J1912269	T02	11/27/07
11	J1912270	T03	11/27/07
12	J1912271	T04	11/27/07
13	J1912272	T05	11/27/07
14	J1912273	T06	11/27/07
15	J1912274	T06A	11/27/07

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912260
L261921	CJ1	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Nov 27 16:34:36 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-287(load case 6), 4=-9(load case 4), 3=-91(load case 1)
Max Grav 2=257(load case 1), 4=14(load case 2), 3=128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-70/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

Julius Lee
Ocean Breeze Engineering
1168 Greenleaf Bay Drive
Boynton Beach, FL 33435

LOAD CASE(S) Standard

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912261
L261921	CJ3	JACK	12	1	Job Reference (optional)

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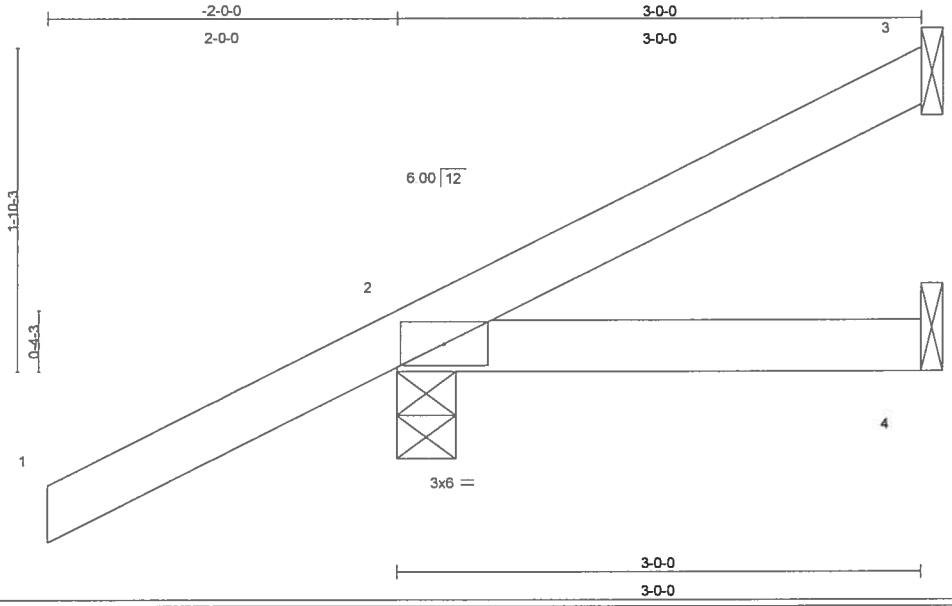

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-27(load case 7), 2=-240(load case 6), 4=-26(load case 4)
Max Grav 3=29(load case 1), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

Continued on page 2

John Lee
Truss Design Engineer
Florida PE# No. 34810
1105 Crandall Bay Main
Boynton Beach, FL 33435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912261
L261921	CJ3	JACK	12	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No 34985
1104 Coastal Bay Blvd
Ocean Beach, FL 32456

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912262
L261921	CJ5	JACK	8	1	Job Reference (optional)

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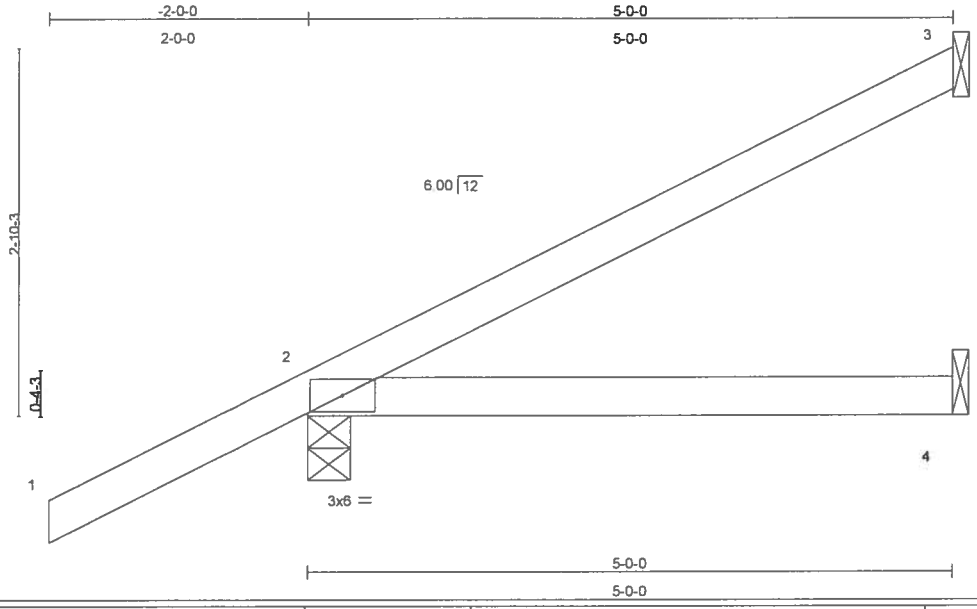


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-86(load case 6), 2=-201(load case 6)
Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 201 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34908
1100 Commercial Way Palm
Heaven Beach, FL 32435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912262
L261921	CJ5	JACK	8	1	Job Reference (optional)

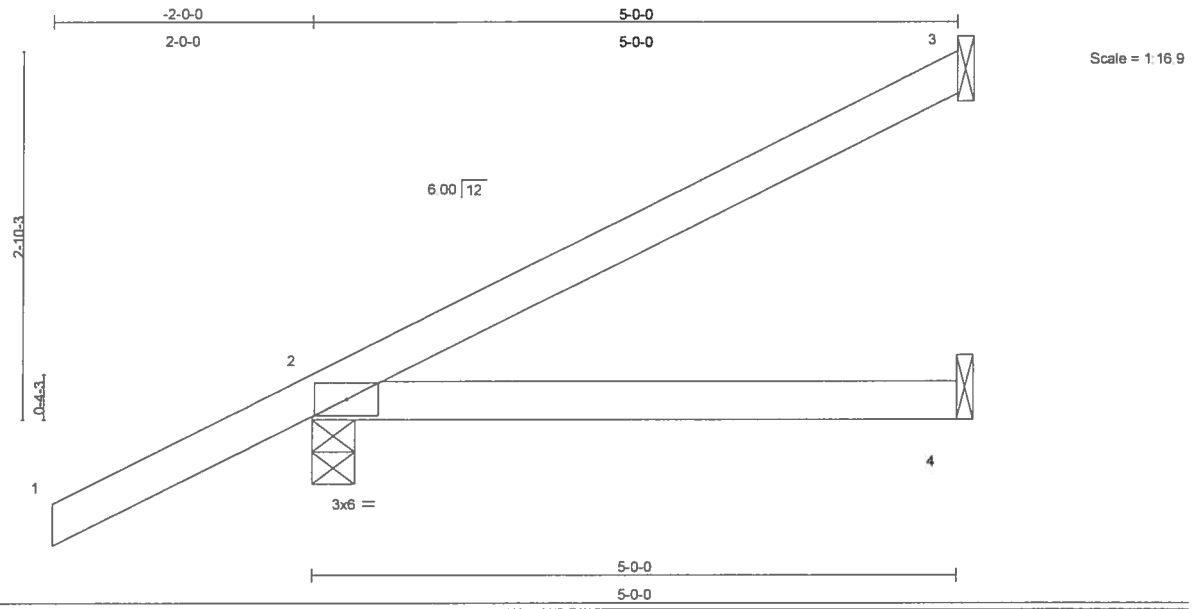
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34931
1105 Crandall Bay Blvd
Jovian Beach, FL 33435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912263
L261921	EJ5	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 27 16:28:39 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>672	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-87(load case 6), 2=-261(load case 6), 4=-46(load case 4)
Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34160
1 1/2 Crestwood Bay Blvd
Jacksonville, FL 32221

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912263
L261921	EJ5	JACK	4	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34810
1 Ross Creek Bay Blvd
Destin Beach, FL 32548

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912264
L261921	EJ7	MONO TRUSS	14	1	Job Reference (optional)

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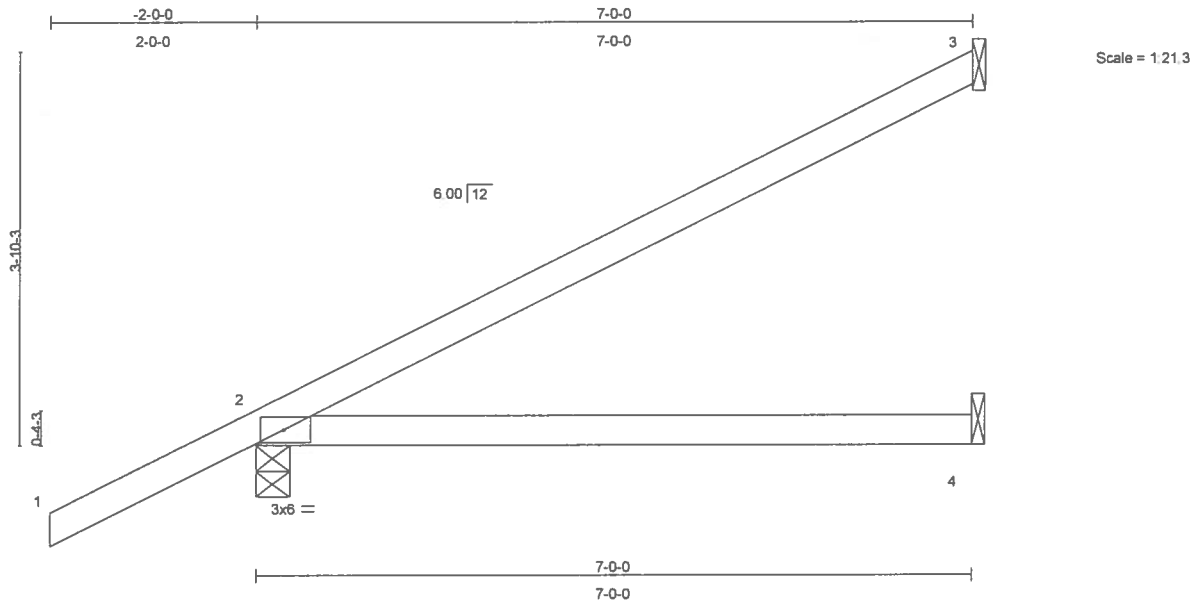

Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-84(load case 6), 2=-140(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.70

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 140 lb uplift at joint 2.

Julius Lee
Truss Design Engineer
Florida PE# No. 37183
1180 Coastal Bay Blvd
Boynton Beach, FL 33435

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912264
L261921	EJ7	MONO TRUSS	14	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No 31810
1352 Crestwell Bay Blvd
Jupiter, FL 33458

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912265
L261921	HIP1	GABLE	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 27 16:28:40 2007 Page 1

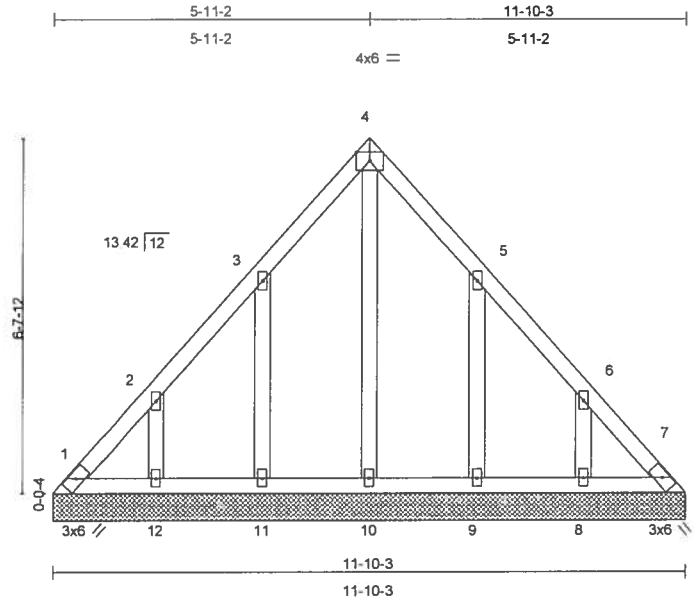

Plate Offsets (X,Y): [4:Edge,0-1-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=62/11-10-3, 7=62/11-10-3, 10=82/11-10-3, 11=127/11-10-3, 12=129/11-10-3, 9=127/11-10-3, 8=129/11-10-3
Max Horz 1=225(load case 5)
Max Uplift 1=-79(load case 4), 7=-45(load case 5), 11=-172(load case 6), 12=-183(load case 6), 9=-171(load case 7), 8=-184(load case 7)
Max Grav 1=158(load case 5), 7=129(load case 7), 10=121(load case 7), 11=132(load case 10), 12=129(load case 1), 9=132(load case 11), 8=129(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-223/126, 2-3=-130/114, 3-4=-71/148, 4-5=-71/148, 5-6=-83/67, 6-7=-194/81
BOT CHORD 1-12=-54/163, 11-12=-54/163, 10-11=-54/163, 9-10=-54/163, 8-9=-54/163, 7-8=-54/163
WEBS 4-10=-121/0, 3-11=-113/195, 2-12=-104/200, 5-9=-113/195, 6-8=-104/200

Julius Lee
Truss Design Engineer
Florida PE No. 34863
1146 Coastal Ray Run
Boynton Beach, FL 33435

JOINT STRESS INDEX
1 = 0.09, 2 = 0.09, 3 = 0.09, 4 = 0.13, 5 = 0.09, 6 = 0.09, 7 = 0.09, 8 = 0.11, 9 = 0.11, 10 = 0.03, 11 = 0.11 and 12 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912265
L261921	HIP1	GABLE	2	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 1, 45 lb uplift at joint 7, 172 lb uplift at joint 11, 183 lb uplift at joint 12, 171 lb uplift at joint 9 and 184 lb uplift at joint 8.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No 31489
1109 Coastal Ray Blvd
Daytona Beach, FL 32118

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912266
L261921	HJ7	MONO TRUSS	2	1	Job Reference (optional)

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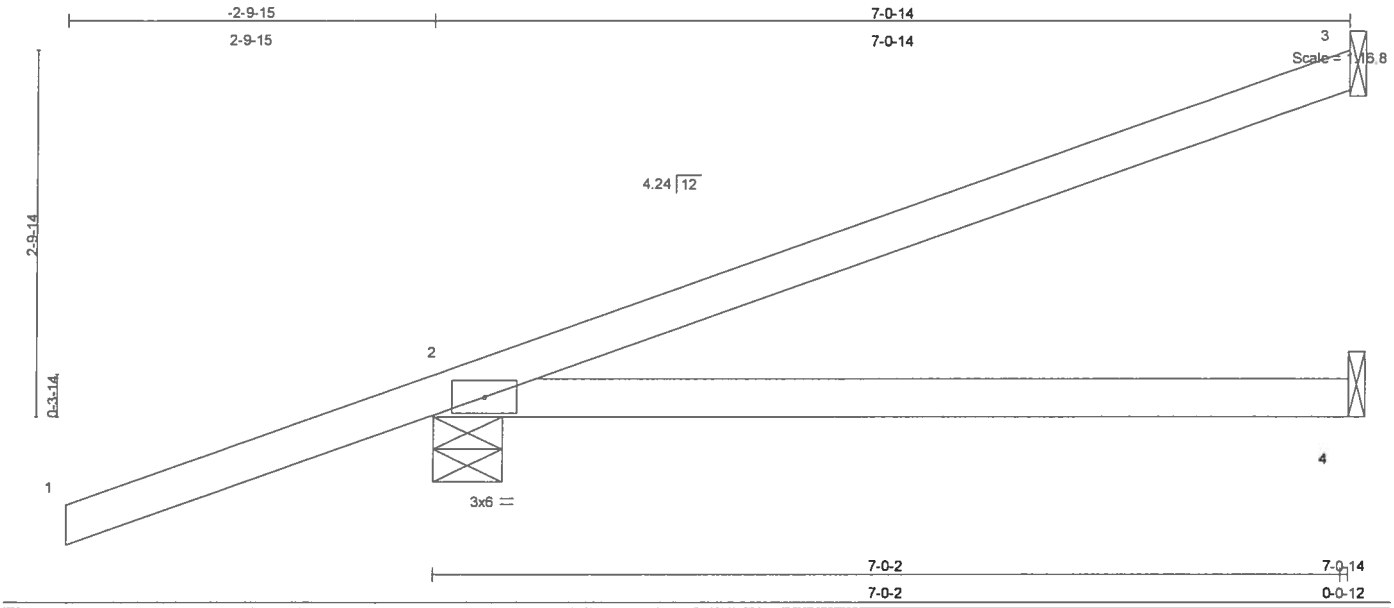

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.10	2-4	>773	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>618	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=183/Mechanical, 2=341/0-6-6, 4=37/Mechanical
Max Horz 2=168(load case 3)
Max Uplift 3=-153(load case 3), 2=-338(load case 3), 4=-55(load case 6)
Max Grav 3=183(load case 1), 2=341(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-71/42
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.51

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 3, 338 lb uplift at joint 2 and 55 lb uplift at joint 4.

Julius Lee
Truss Design Engineer
Florida PE No 34930
1 Box Ground Ray Avon
Lake City, FL 32055

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912266
L261921	HJ7	MONO TRUSS	2	1	Job Reference (optional)

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54
   Trapezoidal Loads (plf)
   Vert: 2=-4(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=0(F=5, B=5)-to-4=-18(F=-4, B=-4)

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Florida PE No. 34869
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Boynton Beach, FL 33435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912267
L261921	HJ9	MONO TRUSS	4	1	Job Reference (optional)

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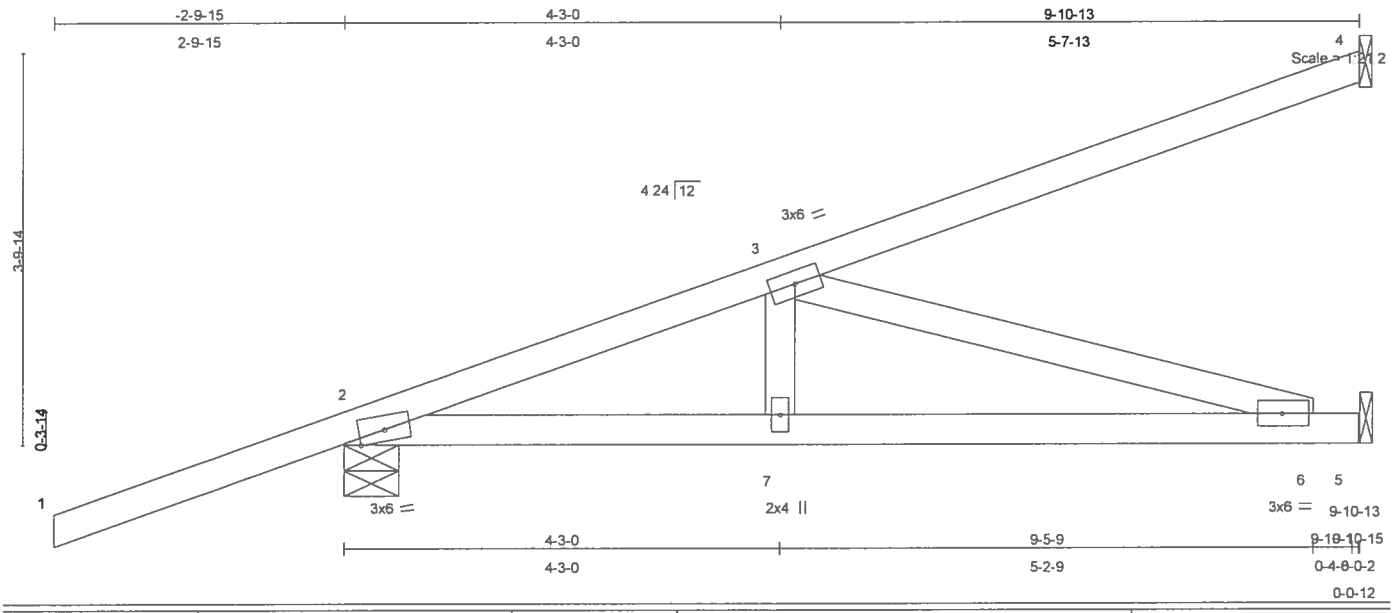

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>986	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=458/0-6-6, 5=217/Mechanical
Max Horz 2=270(load case 3)
Max Uplift 4=-232(load case 3), 2=-284(load case 3), 5=-61(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-642/116, 3-4=-105/65
BOT CHORD 2-7=-305/593, 6-7=-305/593, 5-6=0/0
WEBS 3-7=0/189, 3-6=-618/317

JOINT STRESS INDEX
2 = 0.78, 3 = 0.16, 6 = 0.17 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 284 lb uplift at joint 2 and 61 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34353
1180 Commercial Bay Drive
Vero Beach, FL 32960

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912267
L261921	HJ9	MONO TRUSS	4	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-2=-54
   Trapezoidal Loads (plf)
      Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1109 Central Ray Blvd
Lavon Lisen, FL 32025

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912268
L261921	T01	HIP	1	1	Job Reference (optional)

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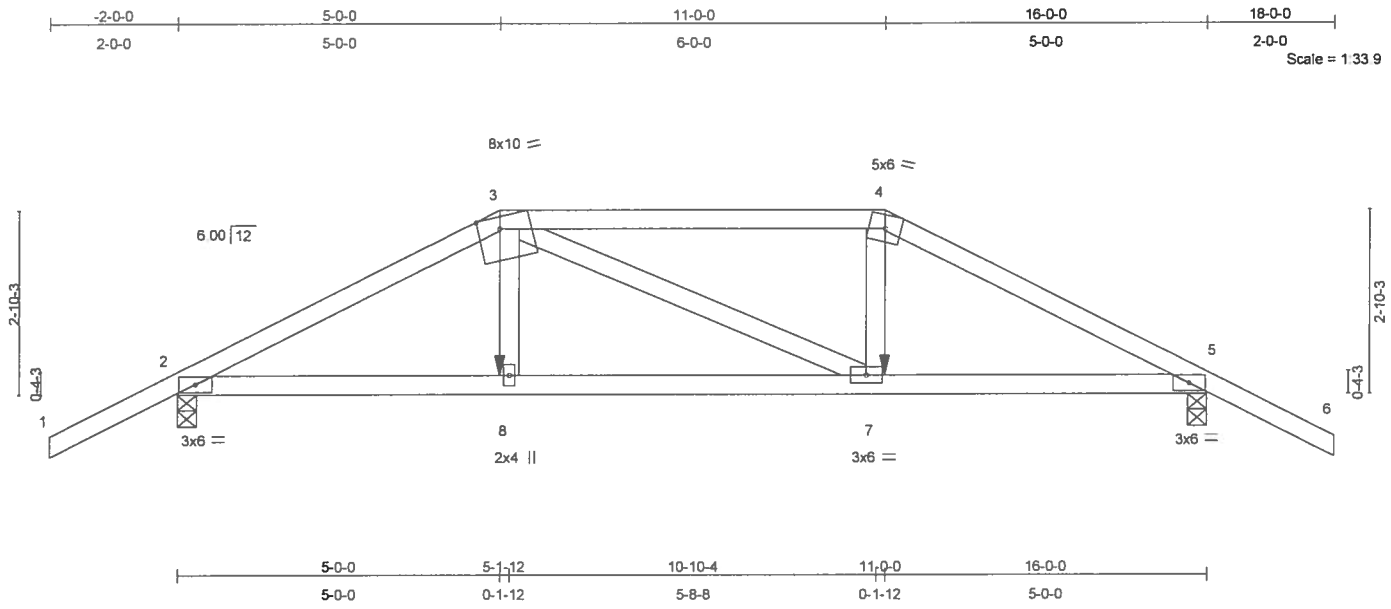

Plate Offsets (X,Y): [3:0-4-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.09	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-15 oc bracing.

REACTIONS (lb/size) 2=934/0-3-8, 5=934/0-3-8
Max Horz 2=-73(load case 6)
Max Uplift 2=-733(load case 5), 5=-733(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1516/1056, 3-4=-1314/987, 4-5=-1516/1056, 5-6=0/47
BOT CHORD 2-8=-910/1299, 7-8=-923/1313, 5-7=-884/1299
WEBS 3-8=-220/260, 4-7=-226/285, 3-7=-77/78

JOINT STRESS INDEX
2 = 0.65, 3 = 0.57, 4 = 0.61, 5 = 0.65, 7 = 0.18 and 8 = 0.18

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
[illegible]
November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912268
L261921	T01	HIP	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 733 lb uplift at joint 2 and 733 lb uplift at joint 5.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-10, 7-8=-17(F=-7), 5-7=-10
   Concentrated Loads (lb)
   Vert: 8=-187(F) 7=-187(F)

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Truss Design Engineer
Florida PE No. 34869
1179 Corporal Ray Allen
Boynton Beach, FL 33435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912269
L261921	T02	HIP	2	1	Job Reference (optional)

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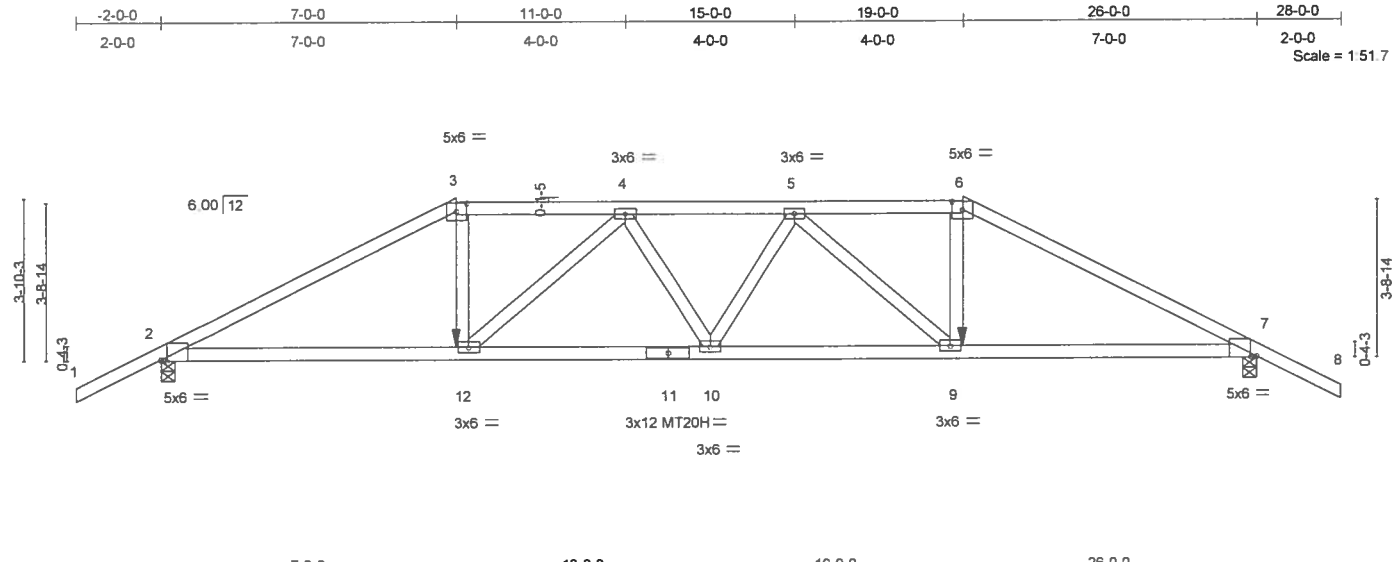

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-0,0-2-5], [6:0-3-0,0-2-5], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.20	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.39	10-12	>798	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 122 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-4 oc purlins, except 2-0-0 oc purlins (3-1-12 max.): 3-6.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.

REACTIONS (lb/size) 2=1799/0-4-0, 7=1799/0-4-0
Max Horz 2=77(load case 5)
Max Uplift 2=-586(load case 5), 7=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3331/1047, 3-4=-2941/974, 4-5=-3592/1162, 5-6=-2941/974, 6-7=-3331/1047, 7-8=0/47
BOT CHORD 2-12=-899/2893, 11-12=-1131/3555, 10-11=-1131/3555, 9-10=-1117/3555, 7-9=-866/2893
WEBS 3-12=-345/1128, 4-12=-929/374, 4-10=0/136, 5-10=0/136, 5-9=-929/374, 6-9=-345/1128

JOINT STRESS INDEX
2 = 0.81, 3 = 0.79, 4 = 0.42, 5 = 0.42, 6 = 0.79, 7 = 0.81, 9 = 0.72, 10 = 0.42, 11 = 0.71 and 12 = 0.72

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1300 Sansbury Way #104
Vero Beach, FL 32966

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912269
L261921	T02	HIP	2	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 7.
8) Girder carries hip end with 7-0-0 end setback.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-12=-10, 9-12=-22(F=-12), 7-9=-10
   Concentrated Loads (lb)
   Vert: 12=-411(F) 9=-411(F)

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Florida PE No. 34-71811
1192 Normal Bay Blvd
Vero Beach, FL 32963

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912270
L261921	T03	HIP	2	1	Job Reference (optional)

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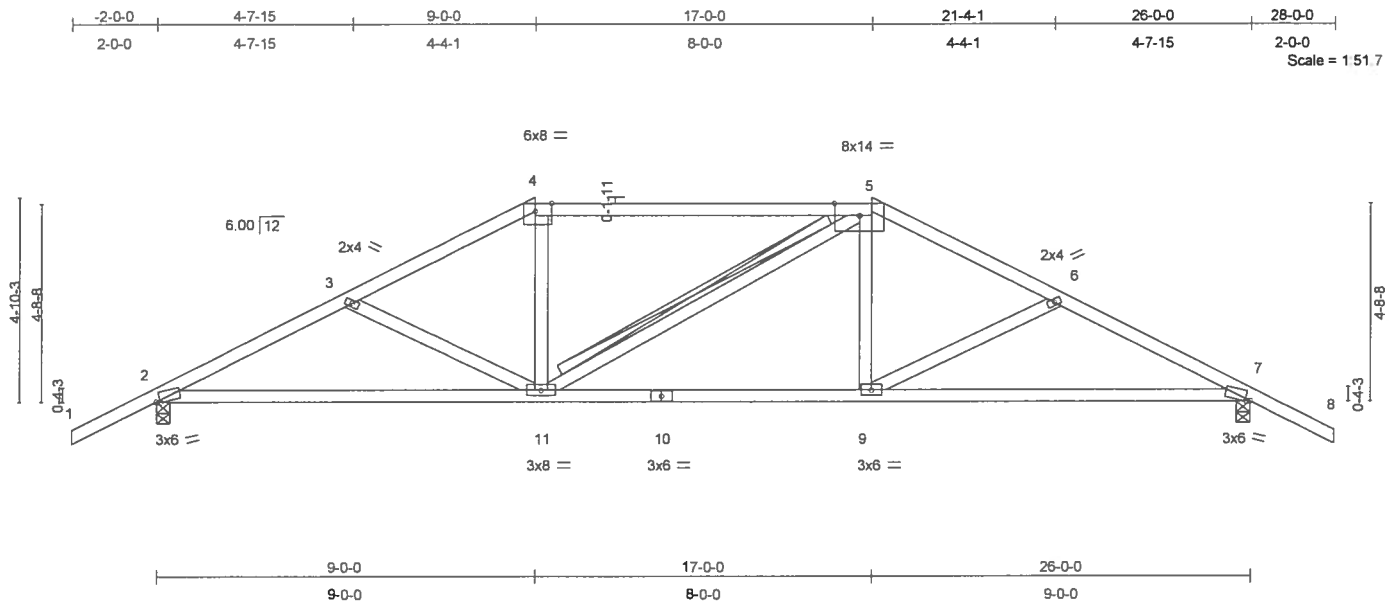

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [4:0-4-10,Edge], [5:0-7-2,Edge], [7:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.14	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.26	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 127 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-10 oc purlins, except
2-0-0 oc purlins (5-5-7 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 8-8-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=938/0-4-0, 7=938/0-4-0
Max Horz 2=88(load case 6)
Max Uplift 2=-253(load case 6), 7=-253(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1451/762, 3-4=-1252/680, 4-5=-1101/673, 5-6=-1252/680, 6-7=-1451/762, 7-8=0/47
BOT CHORD 2-11=-513/1231, 10-11=-368/1100, 9-10=-368/1100, 7-9=-513/1231
WEBS 3-11=-154/163, 4-11=-16/282, 5-11=-123/123, 5-9=-16/282, 6-9=-154/163

JOINT STRESS INDEX
2 = 0.81, 3 = 0.33, 4 = 0.95, 5 = 0.96, 6 = 0.33, 7 = 0.81, 9 = 0.34, 10 = 0.38 and 11 = 0.56

Julius Lee
Truss Design Engineer
Florida PE No. 34788
1300 Commercial Way #100
Boynton Beach, FL 33426

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912270
L261921	T03	HIP	2	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2 and 253 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida No 31683
1193 Kamal Ray Blvd
Venice Beach, FL 33445

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912271
L261921	T04	HIP	2	1	Job Reference (optional)

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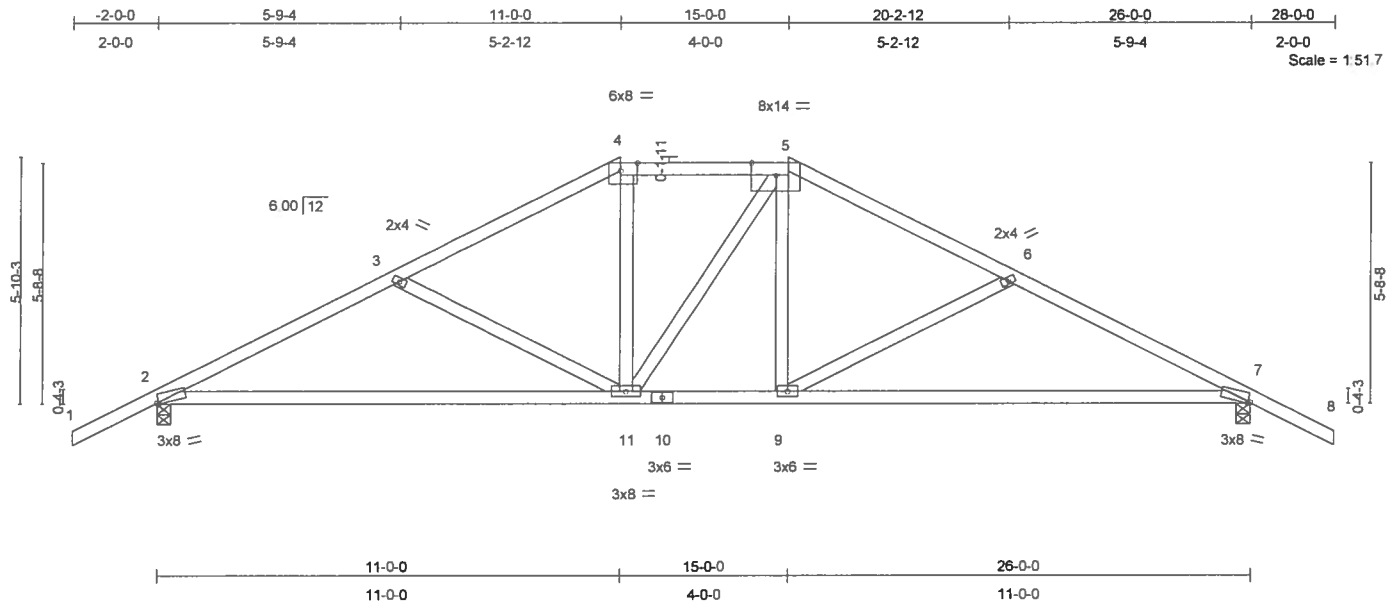

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-4-10,Edge], [5:0-7-2,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.32	7-9	>975	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.58	7-9	>536	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 130 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-1 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 8-7-3 oc bracing.

REACTIONS (lb/size) 2=938/0-4-0, 7=938/0-4-0
Max Horz 2=100(load case 6)
Max Uplift 2=-265(load case 6), 7=-265(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1434/784, 3-4=-1116/634, 4-5=-948/629, 5-6=-1116/634, 6-7=-1434/784, 7-8=0/47
BOT CHORD 2-11=-527/1217, 10-11=-272/947, 9-10=-272/947, 7-9=-527/1217
WEBS 3-11=-313/289, 4-11=-84/279, 5-11=-127/129, 5-9=-84/279, 6-9=-313/289

JOINT STRESS INDEX
2 = 0.91, 3 = 0.33, 4 = 0.52, 5 = 0.53, 6 = 0.33, 7 = 0.91, 9 = 0.34, 10 = 0.70 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Julius Lee
Truss Design Engineer
Florida PE No. 34931
1180 Crestmar Key Blvd
Boynton Beach, FL 33435

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912271
L261921	T04	HIP	2	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 2 and 265 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34930
1180 Commal Bay Blvd
Destin, FL 32541

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912272
L261921	T05	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Nov 27 16:28:46 2007 Page 1

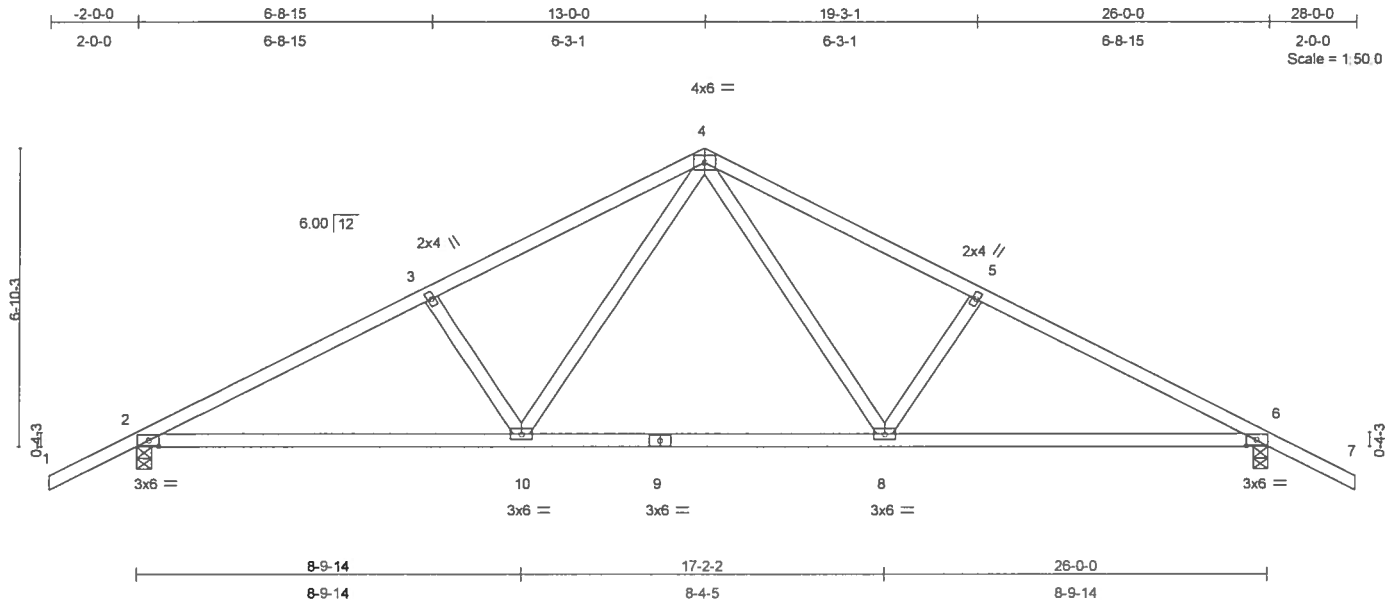

Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.24	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 122 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-10-14 oc bracing.

REACTIONS (lb/size) 2=938/0-4-0, 6=938/0-4-0
Max Horz 2=113(load case 6)
Max Uplift 2=-275(load case 6), 6=-275(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1422/767, 3-4=-1231/761, 4-5=-1231/761, 5-6=-1422/767, 6-7=0/47
BOT CHORD 2-10=-502/1195, 9-10=-214/810, 8-9=-214/810, 6-8=-502/1195
WEBS 3-10=-322/299, 4-10=-235/431, 4-8=-235/431, 5-8=-322/299

JOINT STRESS INDEX
2 = 0.69, 3 = 0.33, 4 = 0.75, 5 = 0.33, 6 = 0.69, 8 = 0.41, 9 = 0.30 and 10 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34931
1109 Coastal Bay Pkwy
Ocean Isle, NC 28469

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912272
L261921	T05	COMMON	1	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34863
1192 Coastal Bay Blvd
Boynton Beach, FL 33435

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912273
L261921	T06	SCISSOR	9	1	Job Reference (optional)

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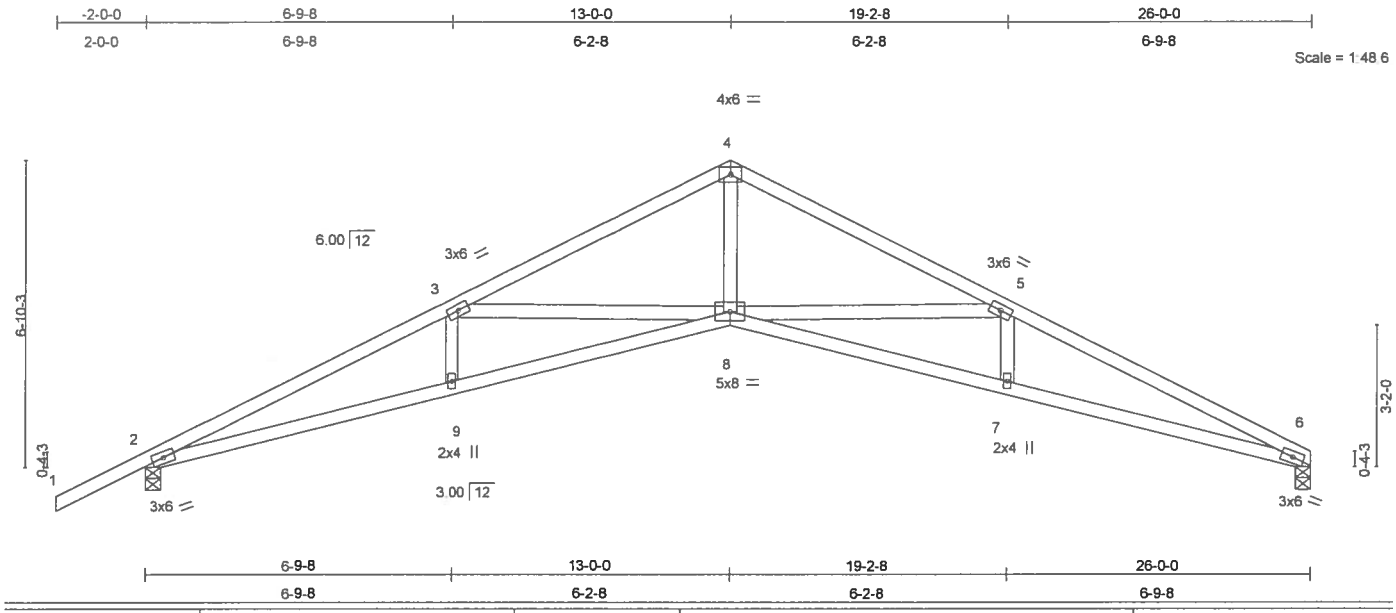

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.28	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.45	8-9	>681	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.33	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-7 oc bracing.

REACTIONS (lb/size) 2=943/0-4-0, 6=816/0-4-0
Max Horz 2=124(load case 6)
Max Uplift 2=-275(load case 6), 6=-179(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2628/1335, 3-4=-1871/949, 4-5=-1872/951, 5-6=-2680/1419
BOT CHORD 2-9=-1116/2331, 8-9=-1119/2333, 7-8=-1202/2384, 6-7=-1205/2386
WEBS 3-9=0/197, 3-8=-704/497, 4-8=-575/1267, 5-8=-757/579, 5-7=0/201

JOINT STRESS INDEX
2 = 0.80, 3 = 0.39, 4 = 0.71, 5 = 0.39, 6 = 0.80, 7 = 0.33, 8 = 0.72 and 9 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Julius Lee
Truss Design Engineer
Florida PE No. 34353
1105 Kenesaw Ave
Davison Beach, FL 32748

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912273
L261921	T06	SCISSOR	9	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 179 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 31489
1104 Emerald Bay Drive
Lake City, FL 32055

November 27,2007

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912274
L261921	T06A	SCISSORS	1	1	Job Reference (optional)

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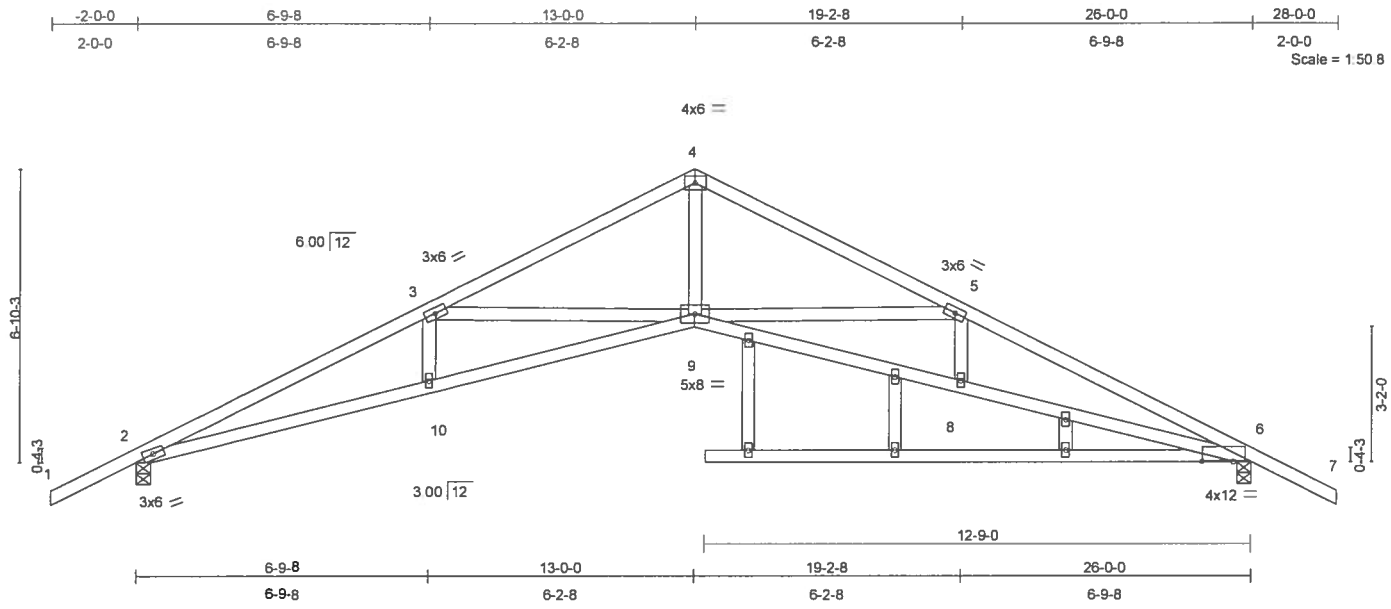


Plate Offsets (X,Y): [6:0-8-11,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.25	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.44	8-9	>696	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.32	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-12 oc bracing. Except:
1 Row at midpt 6-9

REACTIONS (lb/size) 2=938/0-4-0, 6=938/0-4-0
Max Horz 2=-112(load case 7)
Max Uplift 2=-275(load case 6), 6=-275(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2605/1226, 3-4=-1851/838, 4-5=-1851/838, 5-6=-2605/1226, 6-7=0/46
BOT CHORD 2-10=-939/2309, 9-10=-941/2311, 8-9=-941/2311, 6-8=-939/2309
WEBS 3-10=0/197, 3-9=-703/498, 4-9=-472/1246, 5-9=-703/498, 5-8=0/197

JOINT STRESS INDEX
2 = 0.80, 3 = 0.39, 4 = 0.73, 5 = 0.39, 6 = 0.72, 8 = 0.33, 9 = 0.70, 10 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33 and 17 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1, ING Emerald Bay Blvd
Ocoyion Beach, FL 32445

November 27,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - RAY RES. J1912274
L261921	T06A	SCISSORS	1	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6.

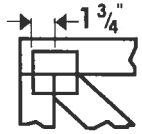
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34160
1146 Coastal Ray Blvd
Jacksonville Beach, FL 32250

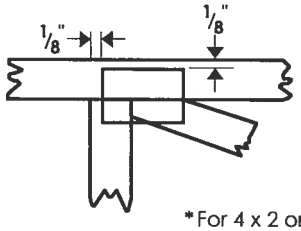
November 27,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

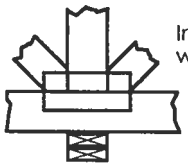
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

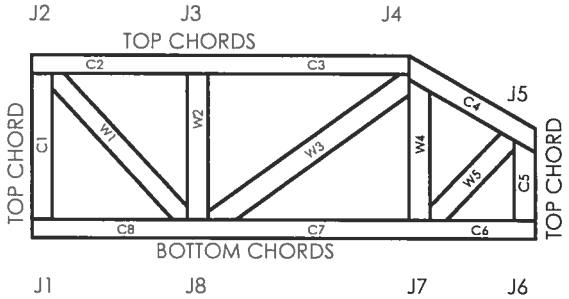
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek

TEE-LOK

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

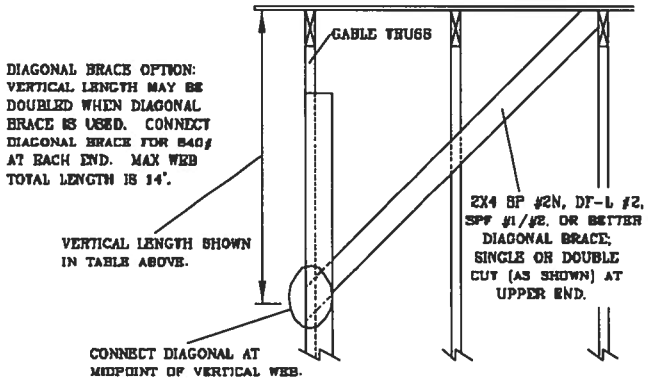
Failure to Follow Could Cause Property Damage or Personal Injury

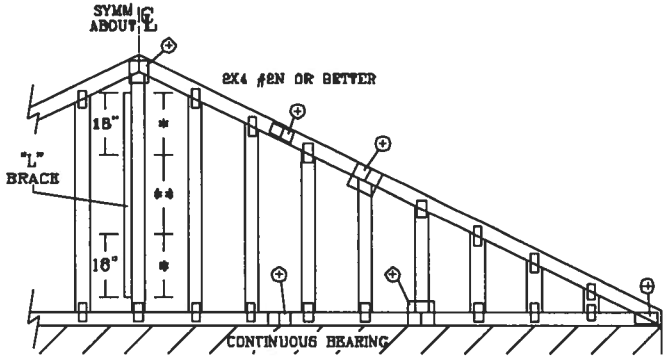
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE / GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 6"	8' 6"	11' 5"	11' 5"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	8' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 8"	12' 8"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#3
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 15 E HT
-ENG	

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	6' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 8"	5' 6"	5' 11"	8' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 8"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 5"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 5"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

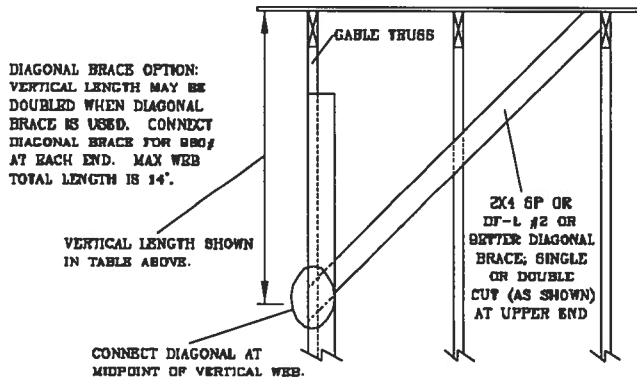
SOUTHERN PINE
#3
STUD
STANDARD

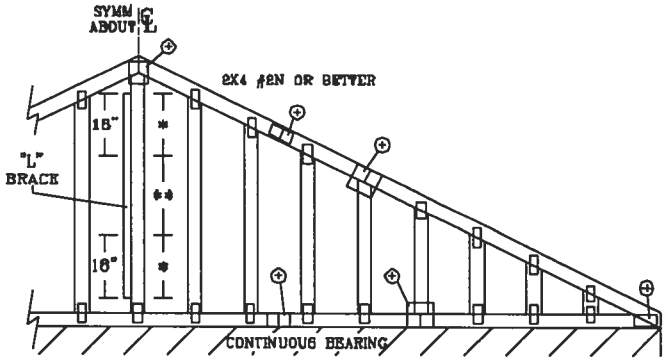
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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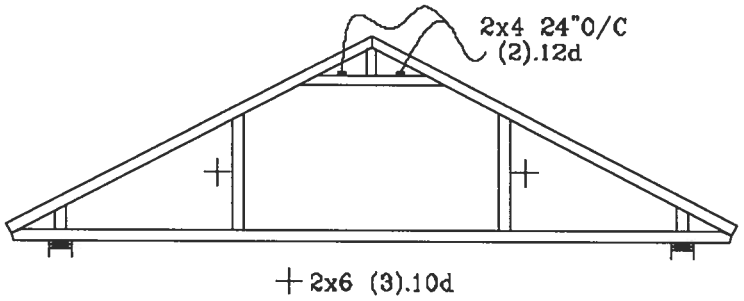
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

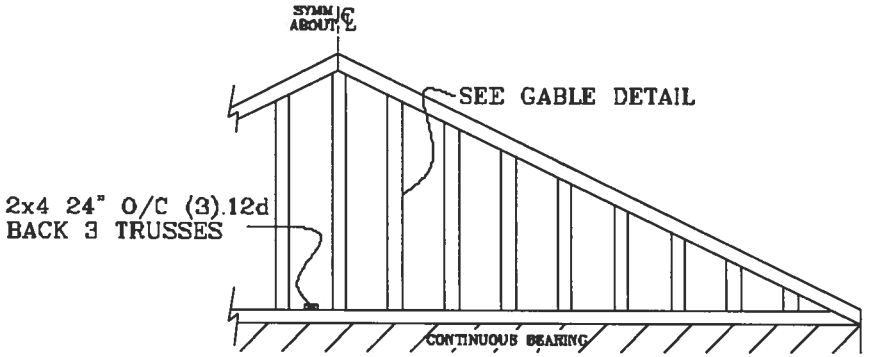
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 90' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

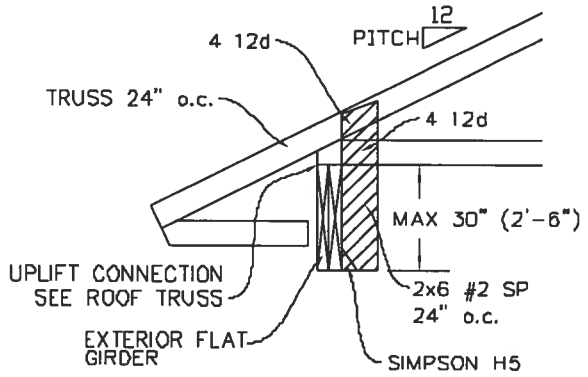


GABLE END TRUSS DETAIL

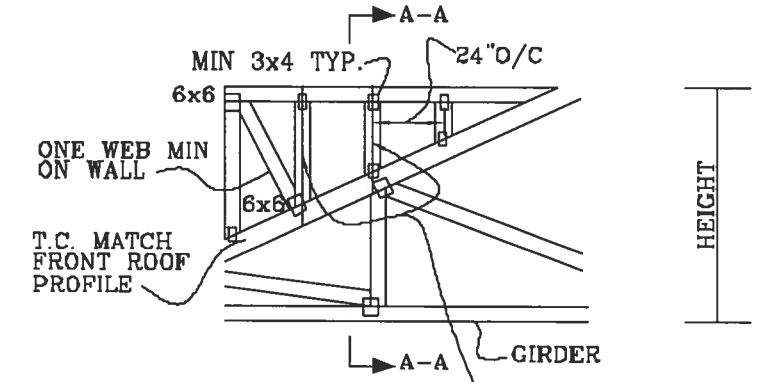


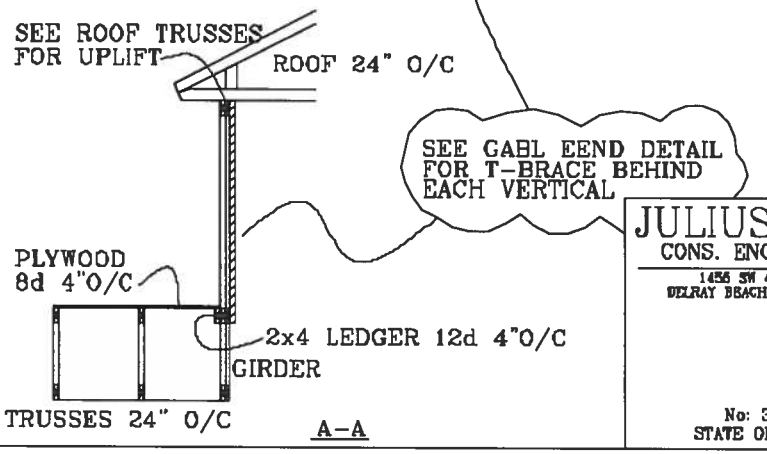
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





A-A

JULIUS LEE'S
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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

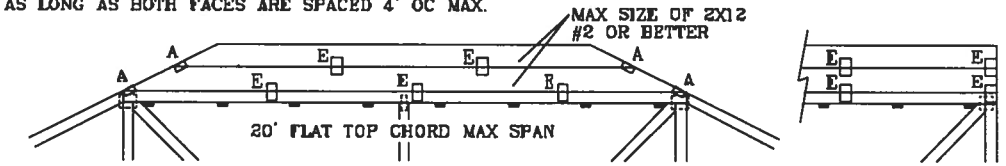
110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

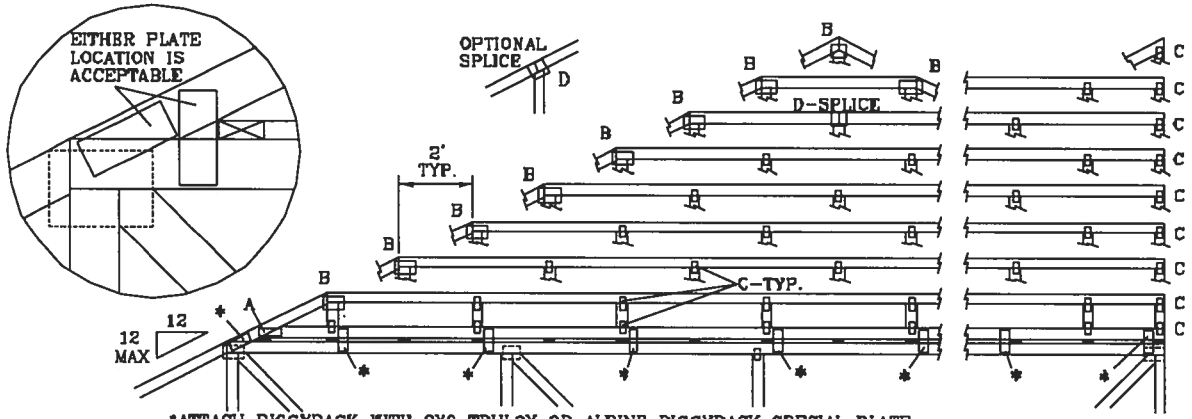
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

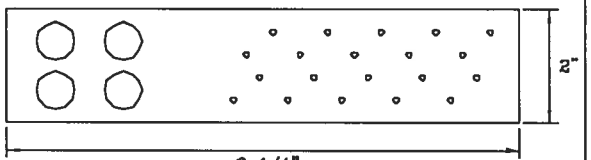




WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

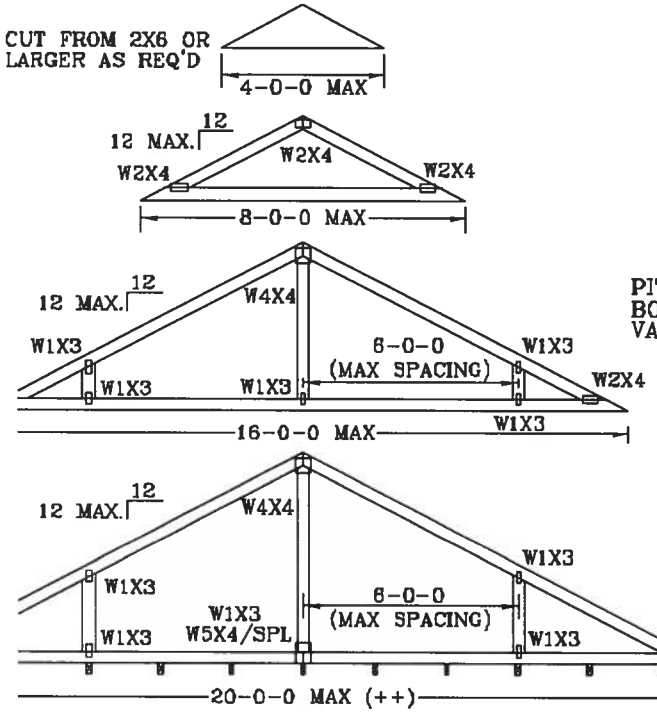
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

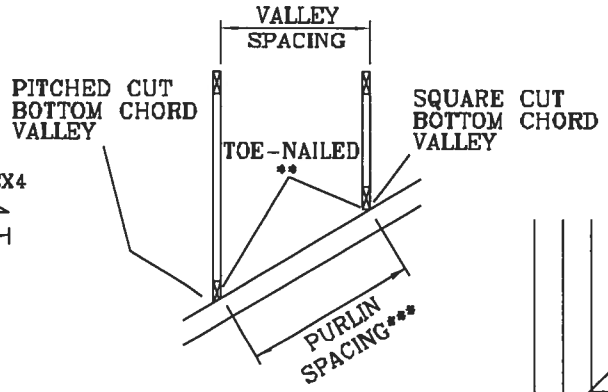
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

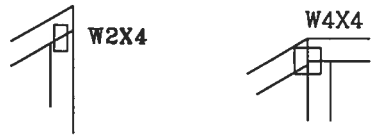
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

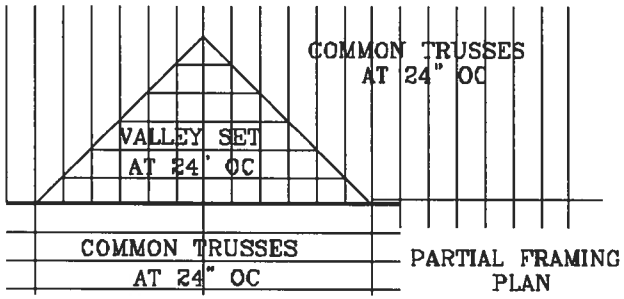
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

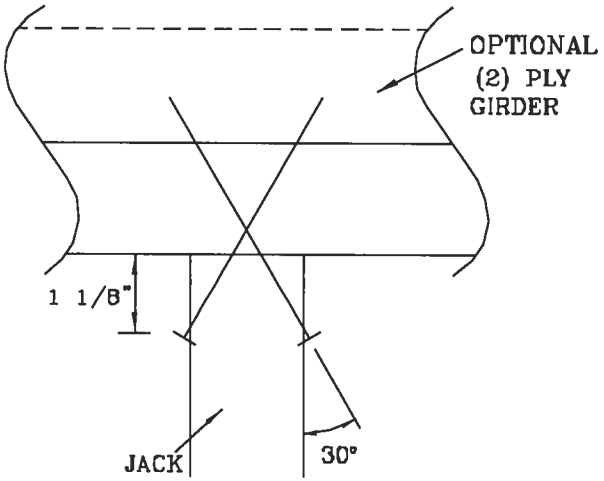
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

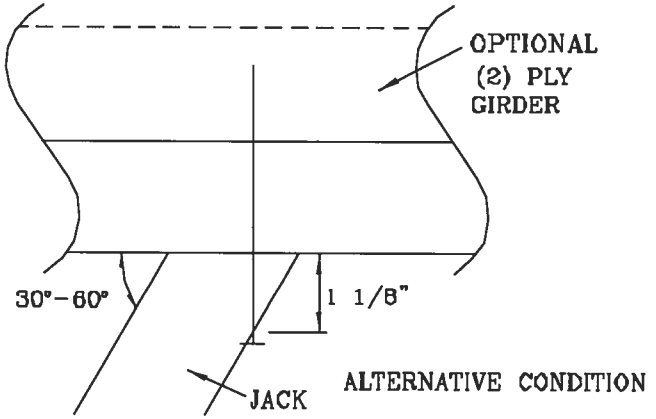
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

"WARNING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

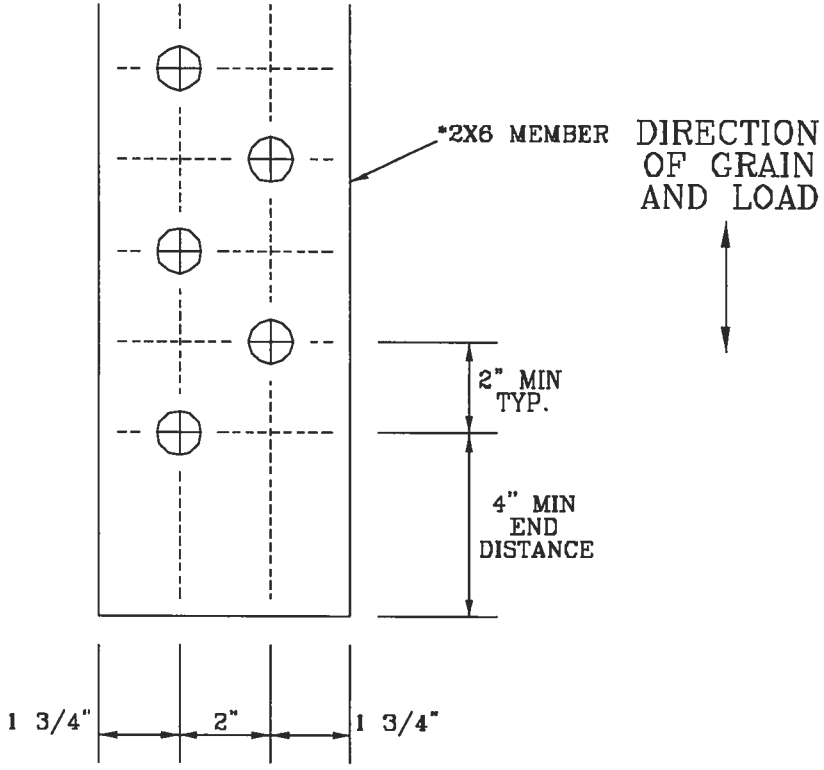
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

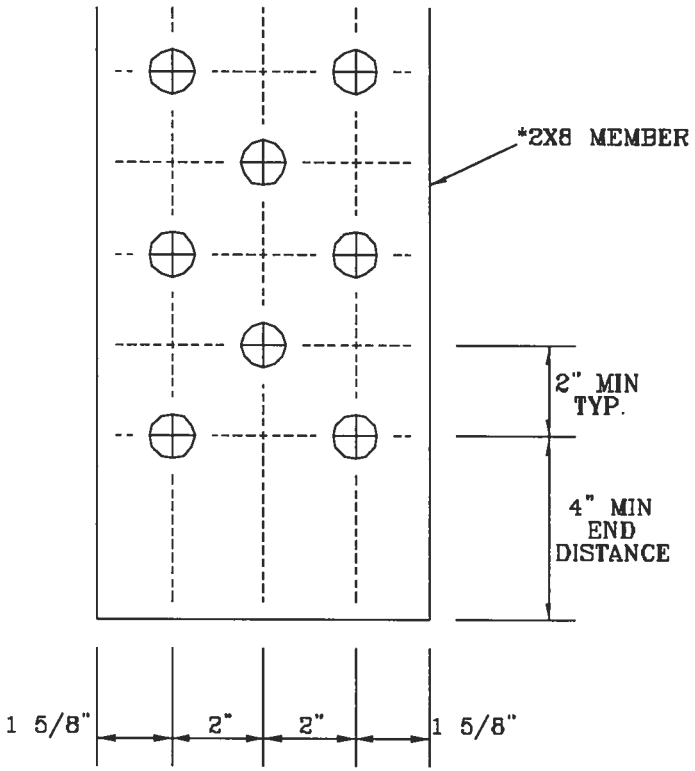
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

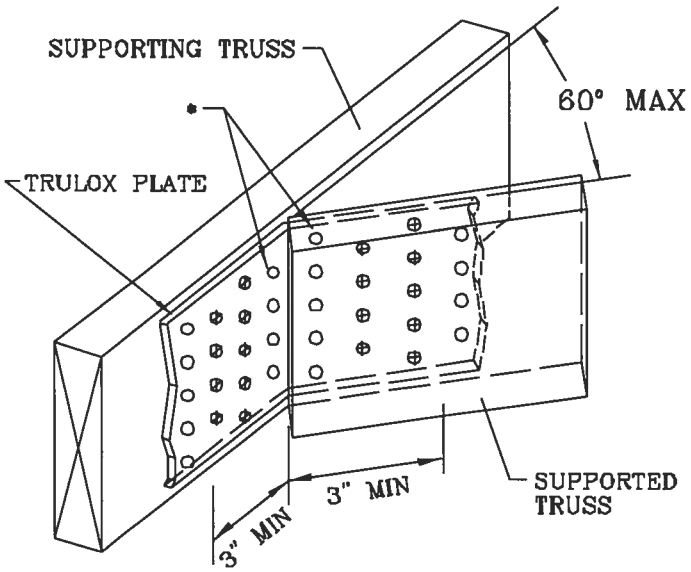
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

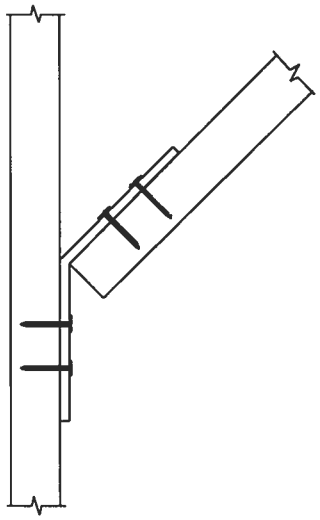
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

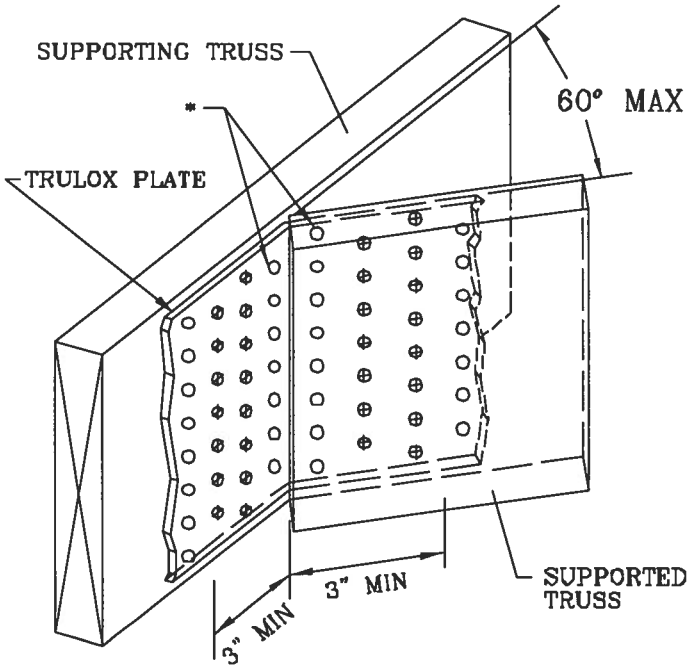
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

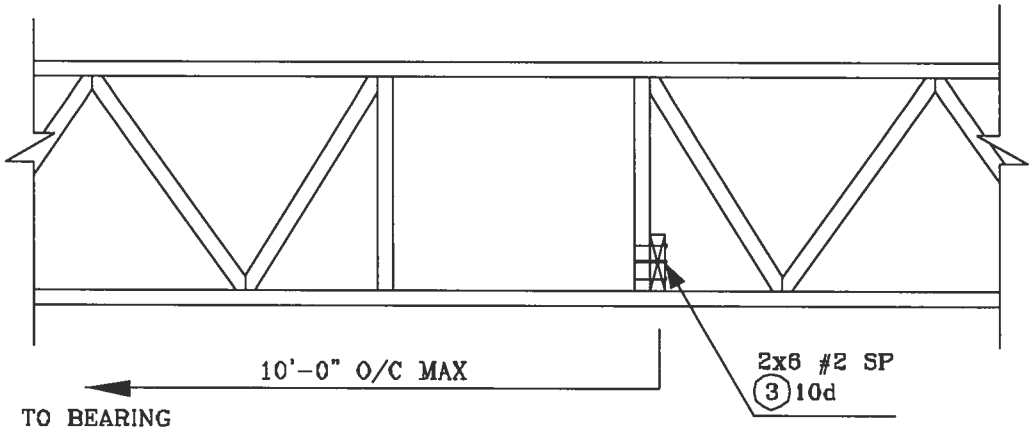
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

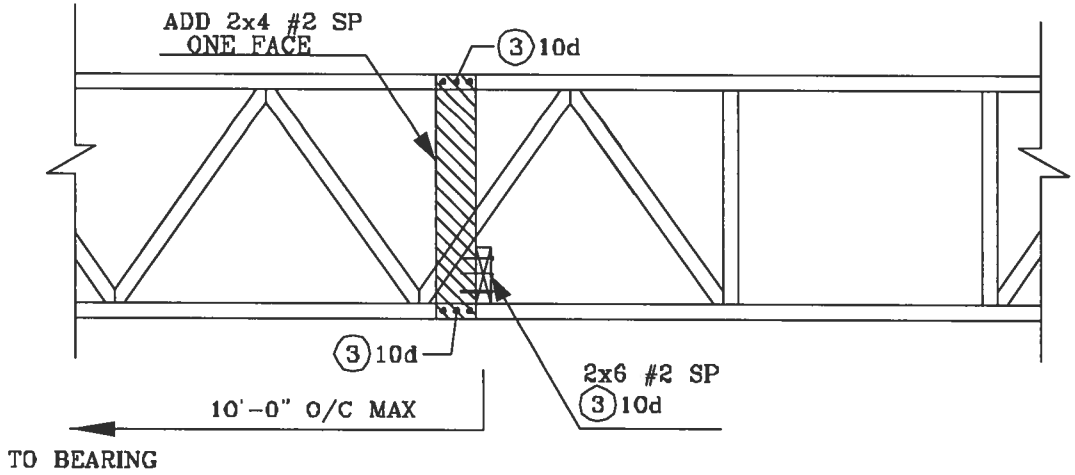
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

Residential System Sizing Calculation

Summary

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

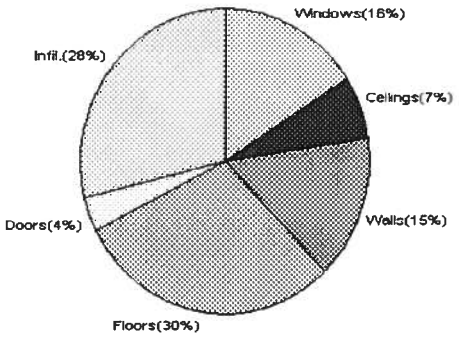
12/4/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**20873**	**Btuh**	**Total cooling load calculation**	**16381**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	115.0	24000	Sensible (SHR = 0.50)	96.9	12000
Heat Pump + Auxiliary(0.0kW)	115.0	24000	Latent	300.5	12000
			Total (Electric Heat Pump)	146.5	24000

WINTER CALCULATIONS

Winter Heating Load (for 1170 sqft)

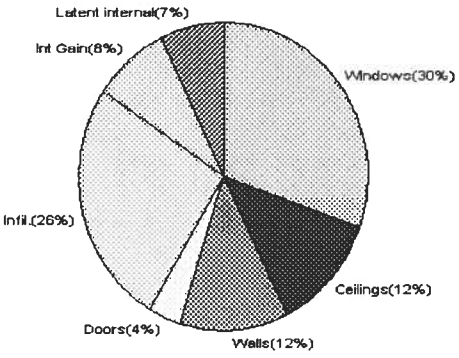
Load component			Load	
Window total	104	sqft	3348	Btuh
Wall total	972	sqft	3192	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	1202	sqft	1416	Btuh
Floor total	142	sqft	6200	Btuh
Infiltration	147	cfm	5940	Btuh
Duct loss			0	Btuh
Subtotal			**20873**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**20873**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1170 sqft)

Load component			Load	
Window total	104	sqft	4979	Btuh
Wall total	972	sqft	2027	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	1202	sqft	1991	Btuh
Floor total			0	Btuh
Infiltration	76	cfm	1423	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**12388**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			2794	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**3994**	**Btuh**
TOTAL HEAT GAIN			**16381**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 12-

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/4/2007

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	NW	9.0	32.2	290 Btuh
4	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
	Window Total		104(sqft)		3348 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	972	3.3	3192 Btuh
	Wall Total		972		3192 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Exterior		40	12.9	518 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1202	1.2	1416 Btuh
	Ceiling Total		1202		1416Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	142.0 ft(p)	43.7	6200 Btuh
	Floor Total		142		6200 Btuh
			Zone Envelope Subtotal:		14933 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	9360	146.6	5940 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**20873 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	20873 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	20873 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Ray Kevin Residence	Project Title:	Class 3 Rating
414 SW Santucknee Terr.	711302KeenRichardSpecHouse	Registration No. 0
Ft. White, FL		Climate: North

12/4/2007

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

12/4/2007

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	NW	9.0	32.2	290 Btuh
4	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
	Window Total		104(sqft)		3348 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	972	3.3	3192 Btuh
	Wall Total		972		3192 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Exterior		40	12.9	518 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1202	1.2	1416 Btuh
	Ceiling Total		1202		1416Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	142.0 ft(p)	43.7	6200 Btuh
	Floor Total		142		6200 Btuh
			Zone Envelope Subtotal:		14933 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	9360	146.6	5940 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**20873 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	20873 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	20873 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Ray Kevin Residence	Project Title:	Class 3 Rating
414 SW Santucknee Terr.	711302KeenRichardSpecHouse	Registration No. 0
Ft. White, FL		Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 12/4/2007

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type*	Ornt	Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	NW	1.5ft.	3.5ft.	9.0	0.0	9.0	29	60	540 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
5	2, Clear, 0.87, None,N,N	SE	6.25f	5.5ft.	30.0	30.0	0.0	29	63	869 Btuh
	Window Total				104 (sqft)					4979 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	972.0	2.1	2027 Btuh
	Wall Total		972 (sqft)		2027 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Exterior	40.0	9.8	392 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1202.0	1.7	1991 Btuh
	Ceiling Total		1202 (sqft)		1991 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	142 (ft(p))	0.0	0 Btuh
	Floor Total		142.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		9585 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	9360	76.4	1423 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 230 +	0	1380 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	**12388 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

12/4/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**12388 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**12388 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**12388 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	2794 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**3994 Btuh**
	TOTAL GAIN	**16381 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 12/4/2007

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	15.0	0.0	15.0	29	60	901 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	NW	1.5ft.	3.5ft.	9.0	0.0	9.0	29	60	540 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
5	2, Clear, 0.87, None,N,N	SE	6.25f	5.5ft.	30.0	30.0	0.0	29	63	869 Btuh
	Window Total				104 (sqft)					4979 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	972.0	2.1	2027 Btuh
	Wall Total		972 (sqft)		2027 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Exterior	40.0	9.8	392 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1202.0	1.7	1991 Btuh
	Ceiling Total		1202 (sqft)		1991 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	142 (ft(p))	0.0	0 Btuh
	Floor Total		142.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		9585 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	9360	76.4	1423 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	6	X 230	+	0	1380 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	**12388 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

Class 3 Rating
Registration No. 0
Climate: North

12/4/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**12388 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**12388 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**12388 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	2794 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**3994 Btuh**
	TOTAL GAIN	**16381 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Ray Kevin Residence
414 SW Santucknee Terr.
Ft. White, FL

Project Title:
711302KeenRichardSpecHouse

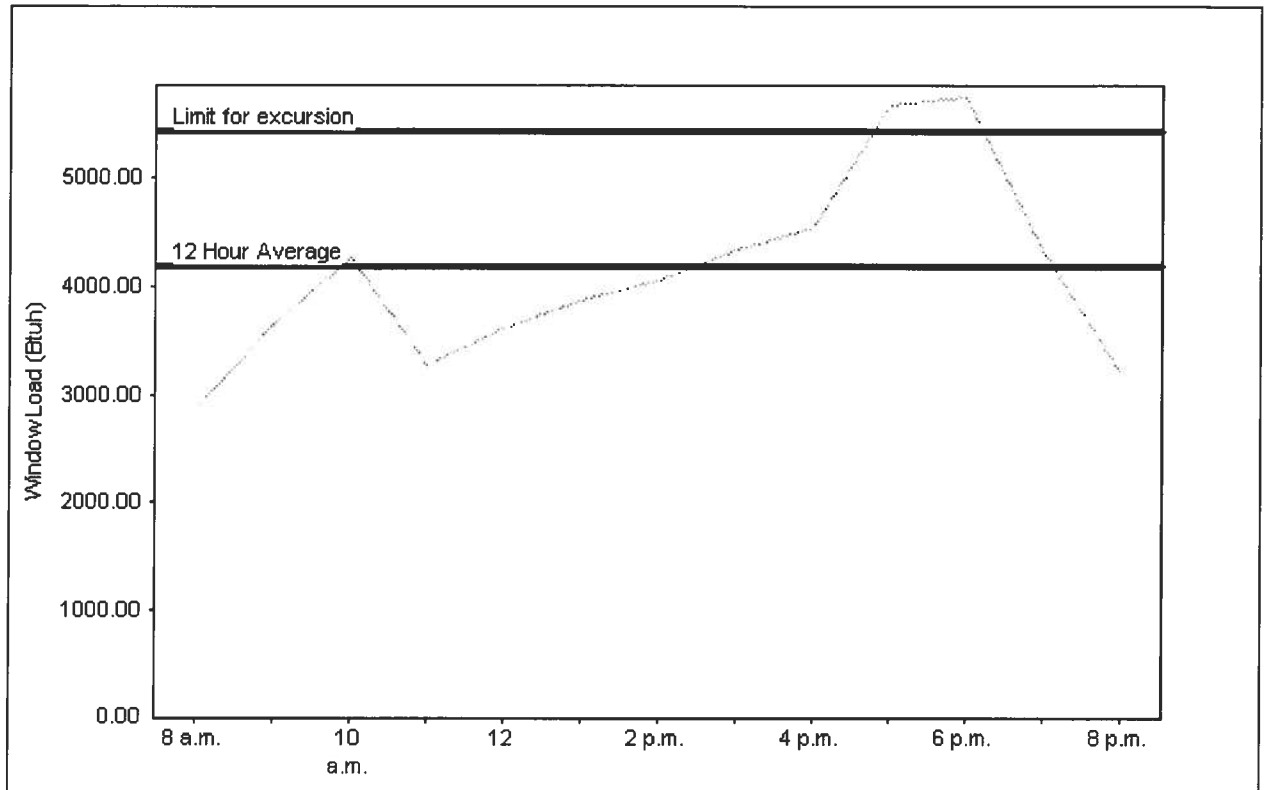
Class 3 Rating
Registration No. 0
Climate: North

12/4/2007

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	4180 Btuh
Summer setpoint	75 F	Peak window load for July	5741 Btuh
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	5434 Btuh
Latitude	29 North	Window excursion (July)	307 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:
DATE: 12-4-07

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 30-6S-16-04001-108 **Building permit No.** 000026531

Use Classification SFD/UTILITY **Fire:** 0.00

Permit Holder JAMES H. JOHNSTON **Waste:**

Owner of Building STEVEN & SHARON RAY **Total:** 0.00

Location: 414 SW SAN-TUCKNEE TERR, FT. WHITE, FL

Date: 03/06/2008

Wayne A. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)