

Approval does not relieve Contractor of responsibility that this drawing, or drawings, shall meet all applicable codes and ordinances in cases of omissions or errors.



FRONT ELEVATION
 SCALE: 1/4" = 1'-0"

ROOF VENTILATION:
 R800.2 Minimum vent area.
 The minimum net free ventilating area shall be 1/150 of the area of the vented space.
 Exception: The minimum net free ventilation area shall be 1/300 of the vented space provided one or more of the following conditions are met:
 1. In Climate Zones 6, 7 and 8, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.
 2. At least 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located no more than 3 feet below the ridge or highest point of the space, measured vertically, with the balance of the required ventilation provided by eave or cornice vents. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet below the ridge or highest point of the space shall be permitted.

- 10'-0"
- 10'-0"
- 2'-0" (TYP.)
- 38'-10" ELEVATION @ TOP OF MAIN FLOOR
- 1'-6"
- 36' ELEVATION @ BOTTOM OF LOWEST HORIZONTAL STRUCTURAL MEMBER
- 34'-2" ELEVATION @ BOTTOM OF A/C CONDENSER & METER
- 9'-4"
- 28' ELEVATION @ TOP OF BOTTOM FLOOR
- 27'-6" ELEVATION @ BOTTOM OF EXTERIOR STEPS WHERE STAIRS MEET GROUND



REAR ELEVATION
 SCALE: 1/4" = 1'-0"

Corey Amira Custom Homes
 Caruthers Res.
 PROJECT ADDRESS:
 SW
 Fort White, FL 32038

FL PE 53915
 This item has been digitally signed and sealed by Mark Disosway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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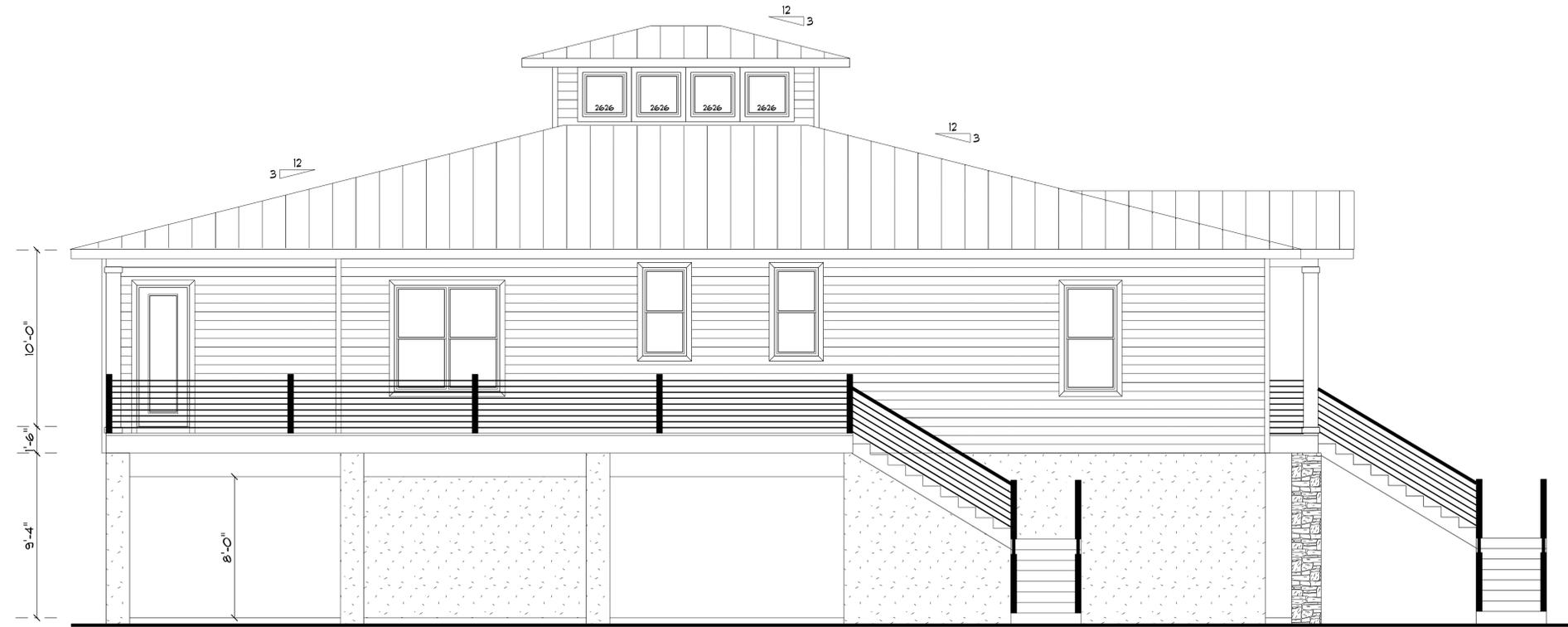
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

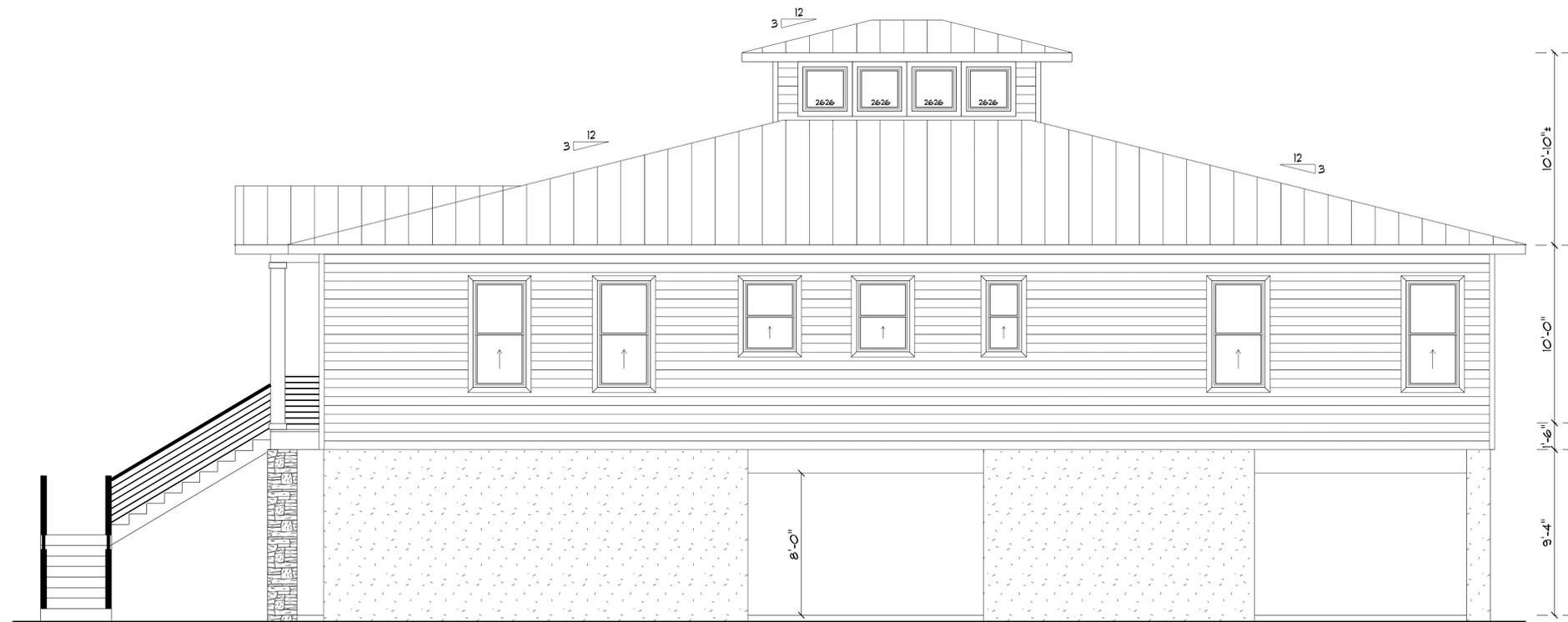
Mark Disosway P.E.
 163 SW Midtown Place
 Suite 103
 Lake City, Florida 32025
 386.754.5419
 disoswaydesign@gmail.com

JOB NUMBER:
 250555

1
 OF 9 SHEETS



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

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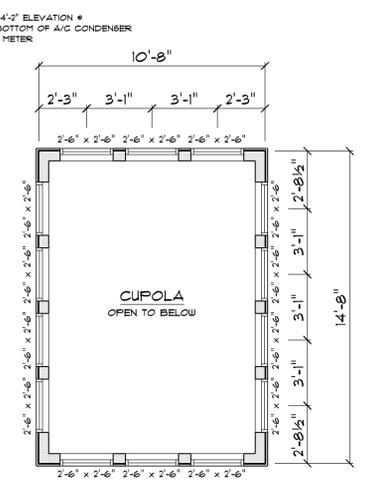
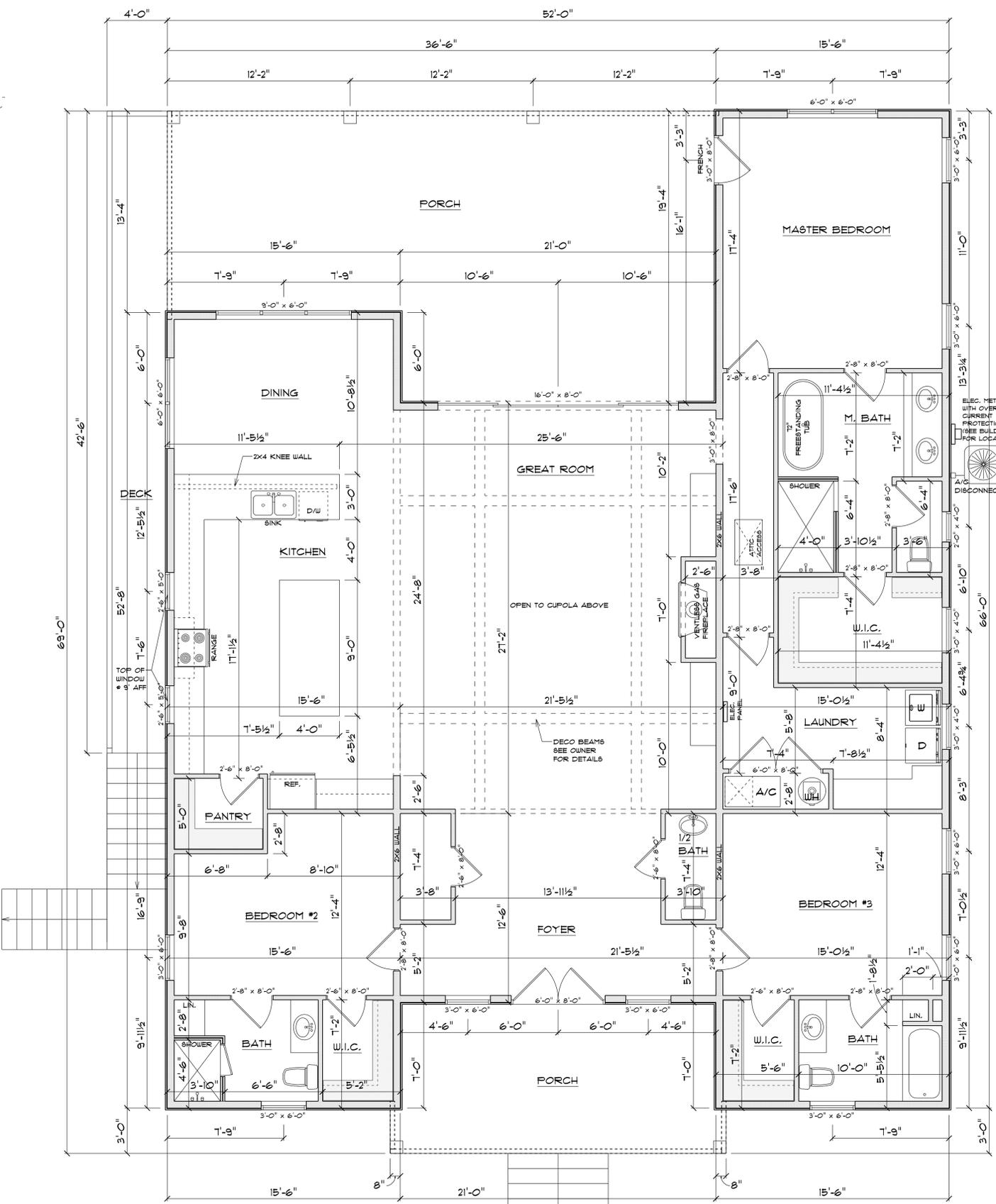
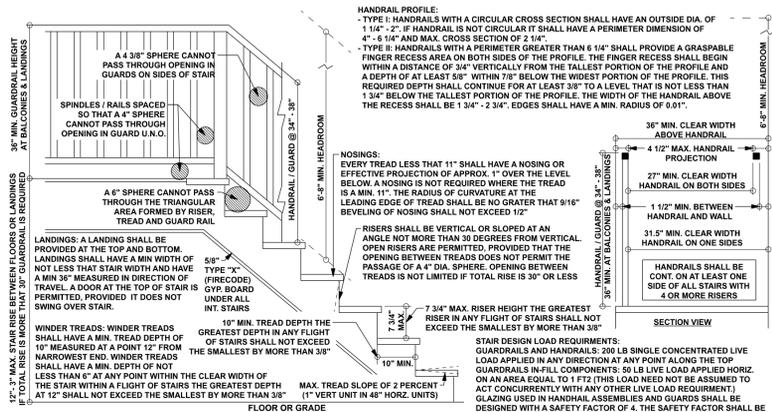
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Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
250555

1.1
OF 9 SHEETS



AREA SCHEDULE	
NAME	AREA
Main Floor Living	2673 sq ft.
Main Floor Rear Porch	613 sq ft.
Main Floor Front Porch	214 sq ft.
Cantilevered Deck	168 sq ft.
Ground Floor Storage / Garage	1248 sq ft.
Ground Floor Open Area	2184 sq ft.
Total	7100 sq ft.

Corey Amira Custom Homes

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Fort White, FL 32038

FL PE 53915
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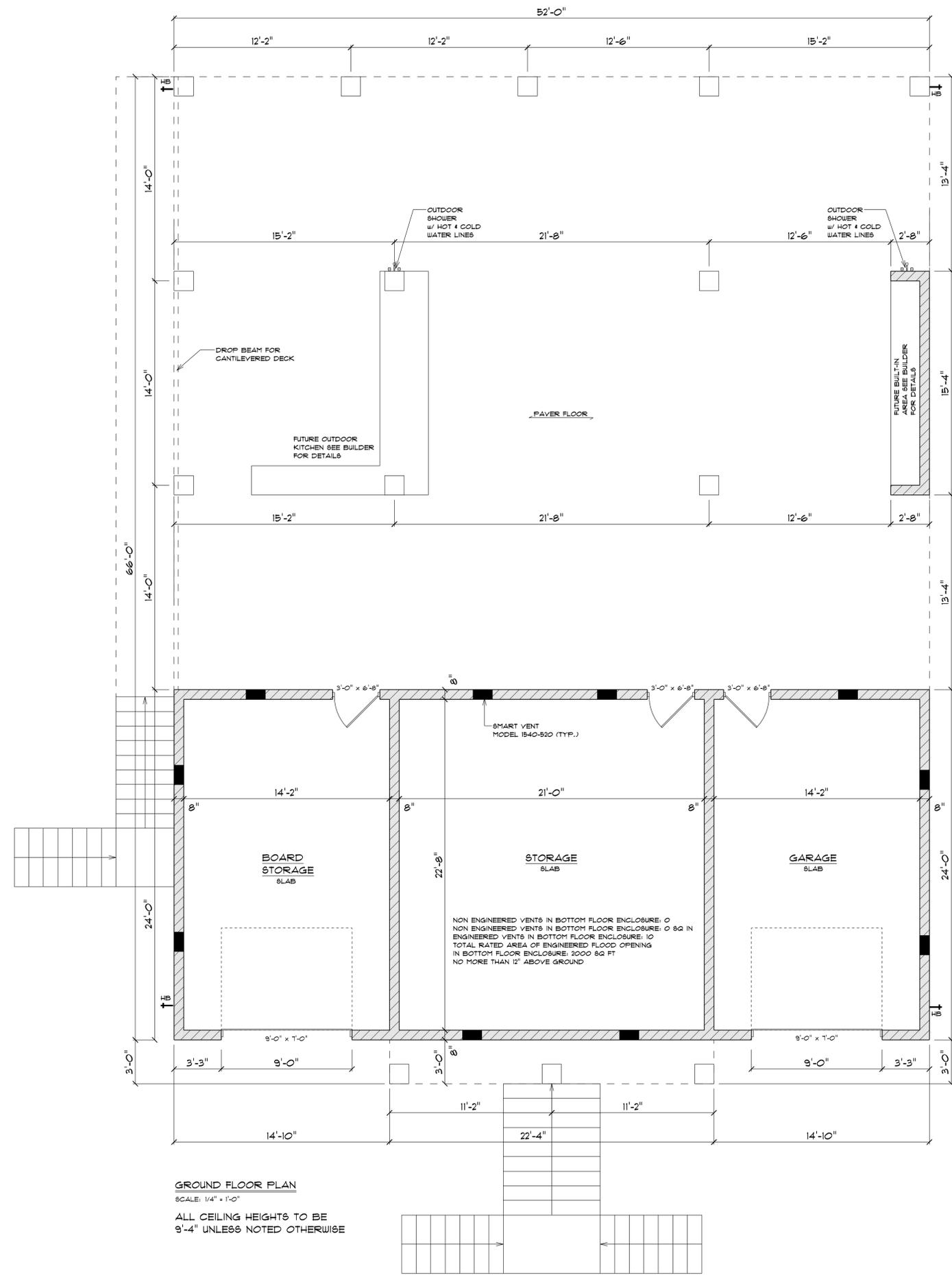
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Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

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2
OF 9 SHEETS



GROUND FLOOR PLAN

SCALE: 1/4" = 1'-0"

ALL CEILING HEIGHTS TO BE 9'-4" UNLESS NOTED OTHERWISE

Corey Amira Custom Homes

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Fort White, FL 32038

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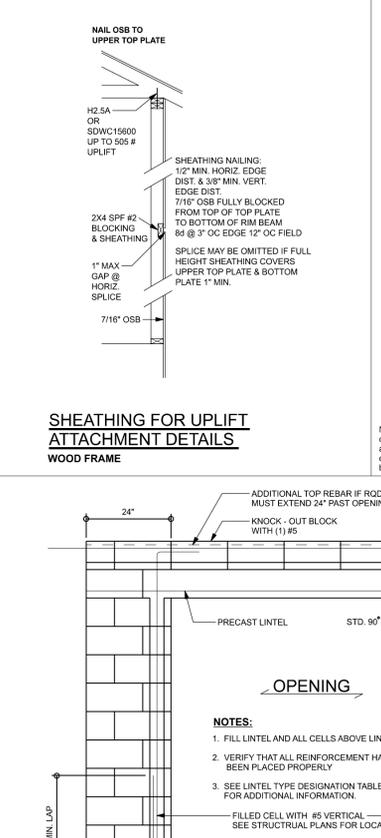
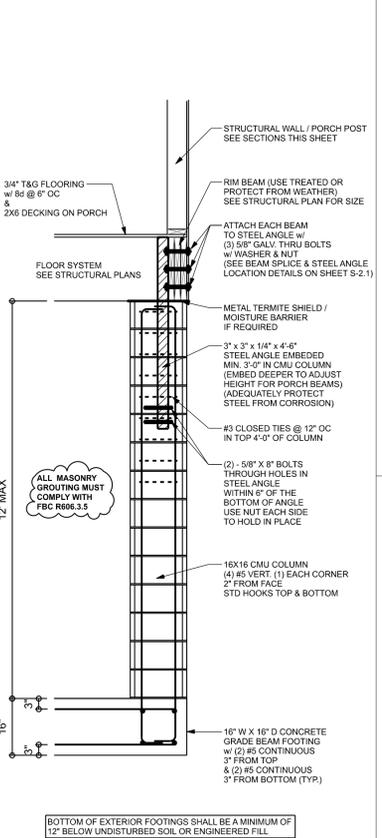
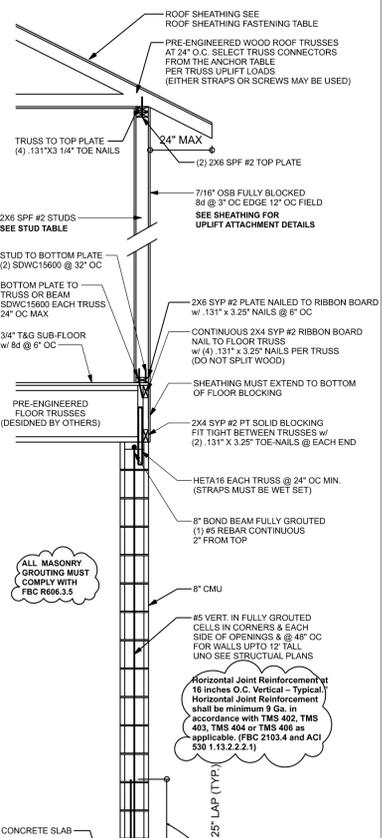
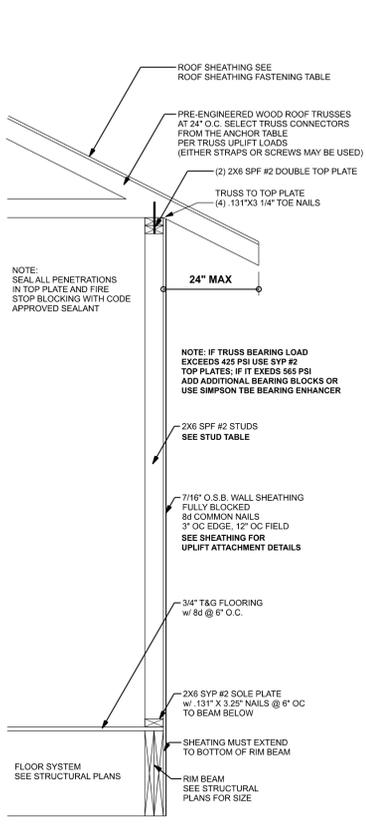
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386.754.5419
disoswaydesign@gmail.com

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2.1
OF 9 SHEETS



ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2" x 3" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2" x 3" x 0.113")	6" oc	6" oc
130 mph Exp. B	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2" x 3" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. B	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2" x 1 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based on the type of roofing material being used. See manufacturer Florida product approval.

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	4-131"x1 1/2"	4-131"x1 1/2"
1000	290	H3	5-131"x1 1/2"	5-131"x1 1/2"
1040	1015	H10A	9-148"x1 1/2"	9-148"x1 1/2"
1415	1215	LTS12-20	6-148"x1 1/2"	6-148"x1 1/2"
990	850	MFS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415	1215	HTS20-30	8-148"x1 1/2"	8-148"x1 1/2"
1335	1235	LSTA21	9-148"x1 1/2"	9-148"x1 1/2"
1640	1460	MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030	1030	CS20	7-148"x1 1/2"	7-148"x1 1/2"
555	535	SP1	4-148"x3"	4-148"x3"
1010	605	SP2	6-148"x3"	6-148"x3"
1220	1150	SPH46	12-148"x1 1/2"	wrap under or over plate
770	771	LSTA24	10-148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148"x1 1/2"	wrap under or over plate
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HIT4	18-162"x3 1/2"	12"x12" Titen HD
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	12"x6" Titen HD
4235	3640	HIT4	18-162"x3 1/2"	12"x12" Titen HD
1900	ABU42	ABU42	5/8"x12" Drill & Epoxy	
2475	ABU62	ABU62	5/8"x12" Drill & Epoxy	
1900	ABU42	ABU42	5/8"x7" Drill & Epoxy	
2475	ABU62	ABU62	5/8"x7" Drill & Epoxy	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND REACTION FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTIONS EACH END; 2X6 RAFTERS 70 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 100 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'-0".

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 EVIDENCE OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SWAP CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND ALL CONTROL JOINTS SHALL BE SUBMITTED TO THE WIND LOAD ENGINEER FOR REVIEW. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPACES 40" DB (25" FOR 5BARS); UNO, ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT LISTED IN THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, TAPCON AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMBITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM MANUFACTURER AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL SECTION @ LVL BEAM
SCALE: 3/4" = 1'-0"

EXTERIOR WALL SECTION @ CMU WALL
SCALE: 3/4" = 1'-0"

TYPICAL 16 X 16 CMU COLUMN SECTION HOUSE & PORCH
SCALE: 1/2" = 1'-0"

TYPICAL FILLED LINTEL ASSEMBLY
SCALE: 1/2" = 1'-0"

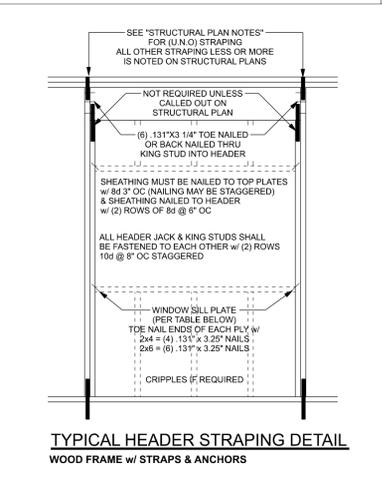
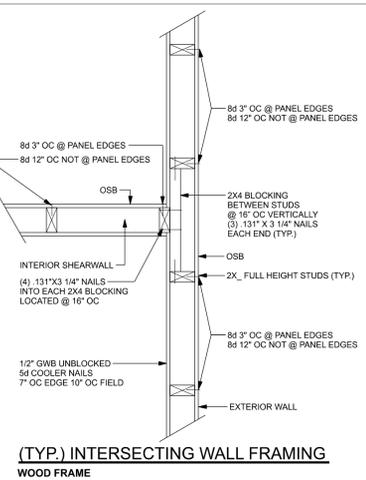
DOOR & WINDOW BUCK ATTACHMENT

DOOR & WINDOW BUCK ATTACHMENT

OPENING SIZE / TYPE	3/16" TAPCON MAX SPACING	1/4" TAPCON MAX SPACING	1/2" ANCHOR BOLTS INTO FILLED CELL
WINDOWS & DOORS UP TO 4' W	14" OC	22" OC	N/A
WINDOWS & DOORS UP TO 6' W	10" OC	18" OC	N/A
WINDOWS & DOORS UP TO 10' W	9" OC	14" OC	N/A
SLIDING DOORS UP TO 8' TALL	8" OC	18" OC	N/A
GARAGE DOOR UP TO 10' WIDE	9" OC	14" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED
GARAGE DOOR UP TO 18' WIDE	4" OC	7" OC	(4) 1/2" x 8" ANCHOR BOLTS PER BUCK EVENLY SPACED

1-TAPCON IN FACE OF CMU; 2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT, 3" MIN. SPACING
WINDOWS AND DOORS MAY BE ATTACHED DIRECTLY TO CMU PER MANUFACTURER AND FLORIDA PRODUCT APPROVAL.
A 1x 1" PT "SPACER" BUCK MAY BE USED IF WINDOW / DOOR IS ATTACHED TO CMU PER FLORIDA PRODUCT APPROVAL.

DOOR & WINDOW BUCK ATTACHMENT



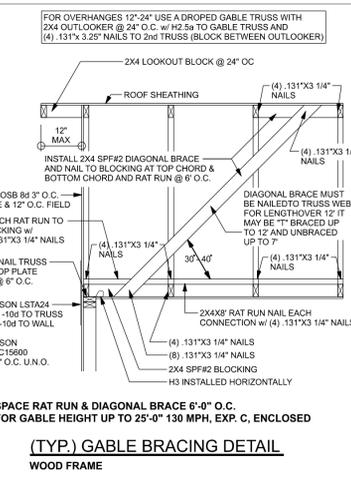
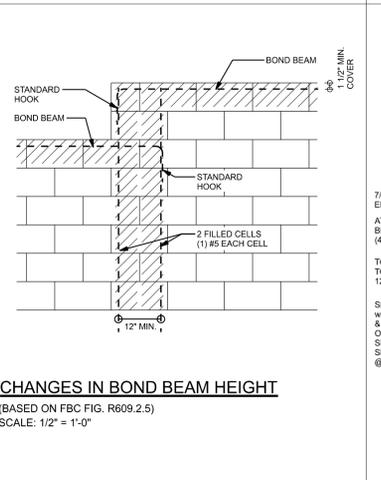
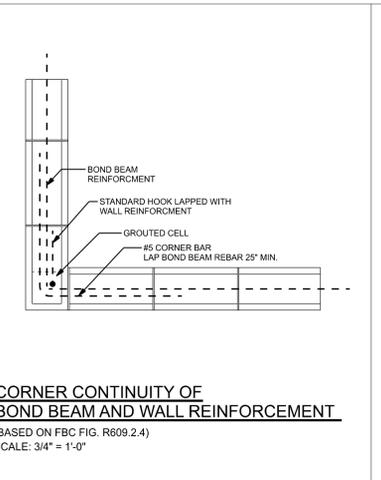
HEADER STRAP TABLE

Top Connection	Bottom Connection	SPH416	MSTA24	18-10d stud to rim beam
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SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

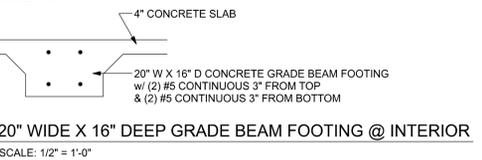
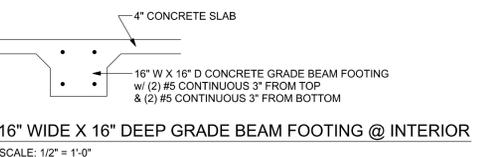
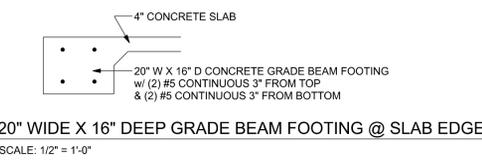
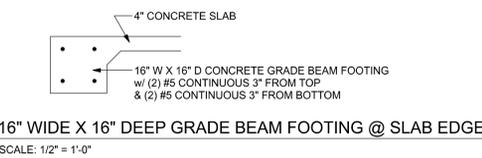
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.23B
130 MPH EXP. C	5'-2"	7'-9"
	(1) 2x4	(2) 2x4
	(1) 2x6	(1) 2x6
	(2) 2x8	(2) 2x8

FOR OTHER WALL HEIGHTS (SILL SPAN SHALL BE DETERMINED BY (R)10)

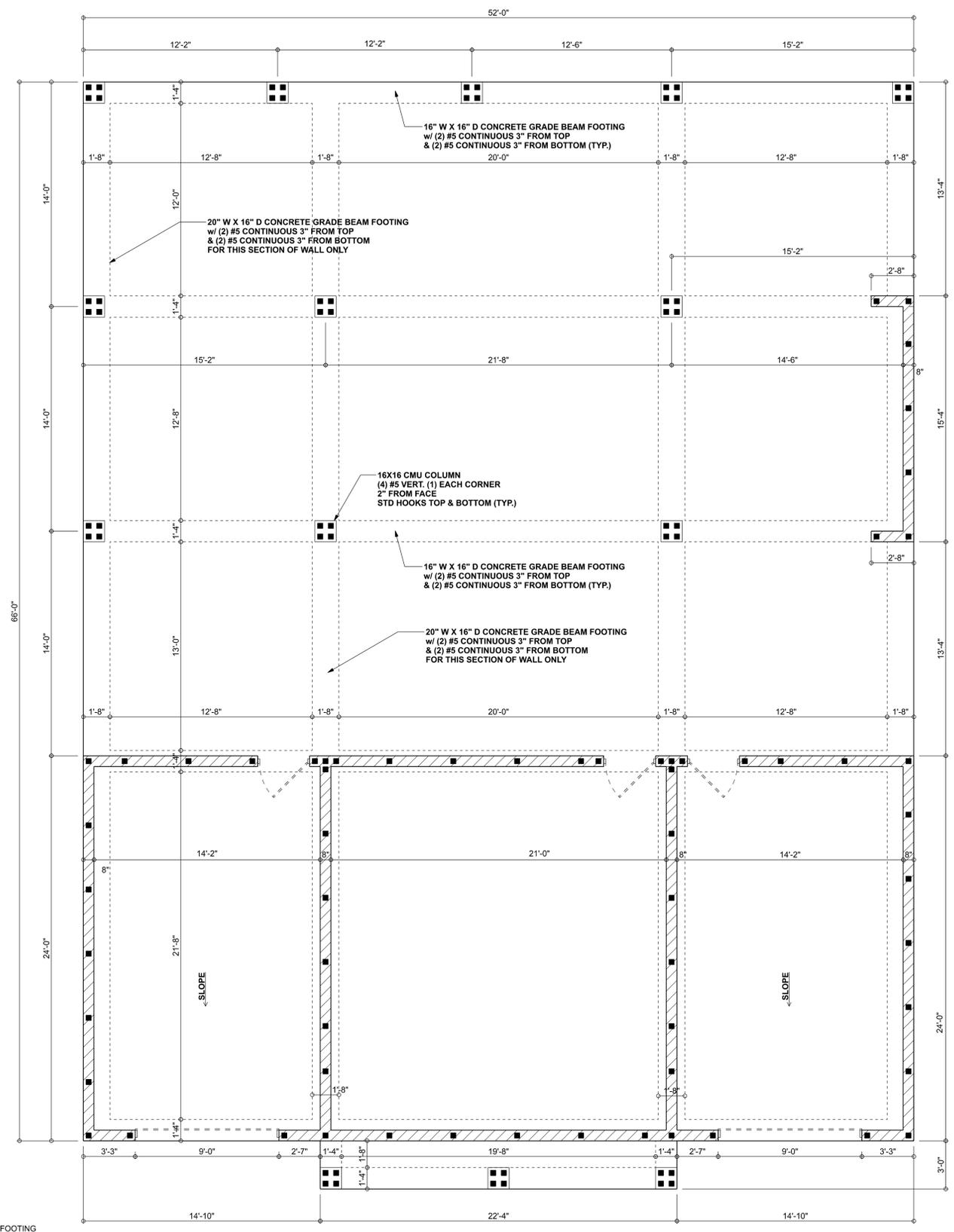
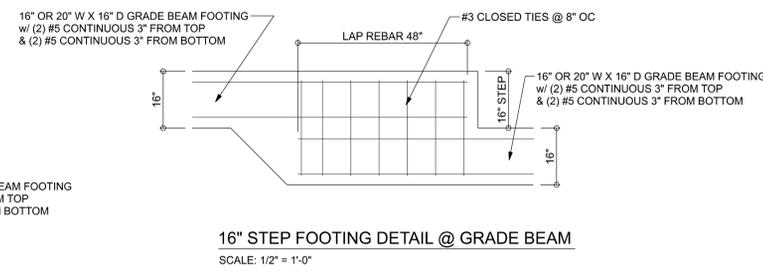
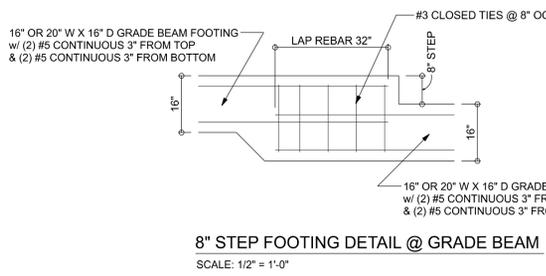


MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls F'm = 1500 psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout. ASTM A525, Class 550, 0.60 oz/lb or 30MSS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout. ASTM A153, Class B2, 1.50 oz/lb or 30MSS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



FOUNDATION PLAN
 SCALE: 1/4" = 1'-0"

- FOUNDATION NOTES**
- FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP-DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
 - FN-2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
 - FN-3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ #6X1-4/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ #1 LAPS SEALED w/ POLY TAPE OVER TERMITICIDE TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITICIDE TREATMENT METHOD CAN BE USED INSTEAD).

Corey Amira Custom Homes

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PROJECT ADDRESS:
 SW Rowland Ave
 Fort White, FL 32038

FL PE 53915
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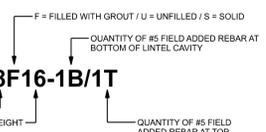
LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
 163 SW Midtown Place
 Suite 103
 Lake City, Florida 32025
 386.754.5419
 disoswaydesign@gmail.com

JOB NUMBER:
 250555

S-2
 OF 9 SHEETS

TYPE DESIGNATION



MATERIALS

1. Fc 8" precast lintel = 3500 psi
2. Fc prestressed lintel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/ minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U Lintels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606-05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:
Applied vertical load + Applied horizontal load / Safe vertical load + Safe horizontal load ≤ 1.0
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right.

SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
- Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d/4 way from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example 7'-6" lintel type 8F32-1B safe gravity load = 6472(40)0.469(15)H0.0781; w/ 15% reduction 6472 = (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT									
		8U8	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F36-0B	8F40-0B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809		
3'-6" (42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809		
4'-0" (48")	PRECAST	1966	2561	3751	5020	6290	7561	8832	10103		
4'-6" (54")	PRECAST	1599	1969	2110	2931	3753	4576	5400	6224		
5'-4" (64")	PRECAST	1217	1349	1438	1999	2560	3123	3686	4249		
5'-10" (70")	PRECAST	1062	1653	3090	5385	7547	9709	11871	13973		
6'-6" (78")	PRECAST	908	699	1160	1625	2054	2486	2919	3352		
7'-6" (90")	PRECAST	743	1011	1729	2661	3698	4735	5772	6809		
9'-4" (112")	PRECAST	554	752	1245	1843	2564	3286	4007	4728		
10'-6" (126")	PRECAST	475	535	890	1247	2093	2777	3461	4145		
11'-4" (136")	PRECAST	362	582	945	1366	1846	2423	3000	3577		
12'-0" (144")	PRECAST	337	540	873	1254	1684	2193	2703	3212		
13'-4" (160")	PRECAST	276	471	755	1075	1428	1838	2316	2883		
14'-0" (168")	PRECAST	219	424	706	1002	1326	1697	2127	2630		
14'-8" (176")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.		
15'-4" (184")	PRESTRESSED	N.R.	412	710	1250	1733	2058	2320	2513		
17'-4" (208")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.		
19'-4" (232")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.		
21'-4" (256")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.		
22'-0" (264")	PRESTRESSED	N.R.	165	315	550	784	1047	1285	1399		
24'-0" (288")	PRESTRESSED	N.R.	129	250	450	654	884	1092	1222		

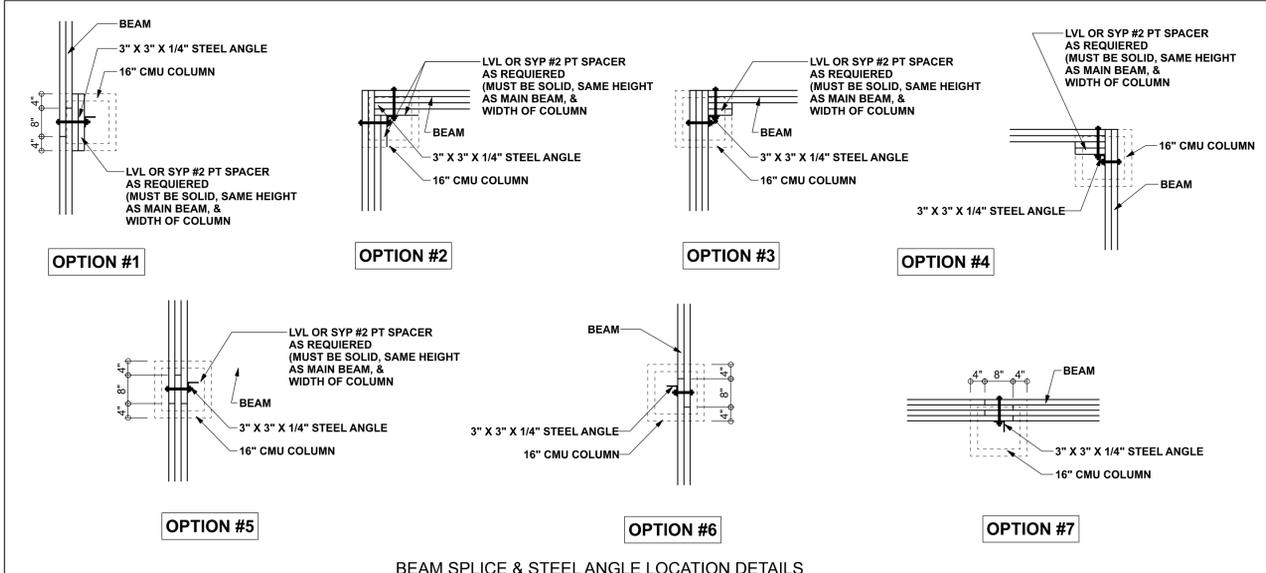
SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT									
		8RU5	8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B	8RF34-0B	8RF38-0B
4'-4" (52")	PRECAST	1635	1749	3355	5020	6349	7421	8403	9267		
4'-6" (54")	PRECAST	1494	1598	3063	3922	3968	4945	5924	6904		
5'-0" (60")	PRECAST	866	920	1770	1716	2277	2839	3402	3966		
5'-10" (70")	PRECAST	810	1167	2481	4567	6389	8060	9717	11361		
6'-8" (80")	PRECAST	797	859	1653	1900	2124	2649	3174	3700		
7'-6" (90")	PRECAST	669	1113	2342	4242	6039	8060	10206	12476		
9'-8" (116")	PRECAST	411	901	1825	3120	5048	7747	10448	13260		
			755	1490	2459	3776	5743	7329	8623		
			755	1490	2459	3776	5743	7329	8623		
			466	999	1568	2253	3129	4091	5146		
			526	999	1568	2253	3129	4150	5891		

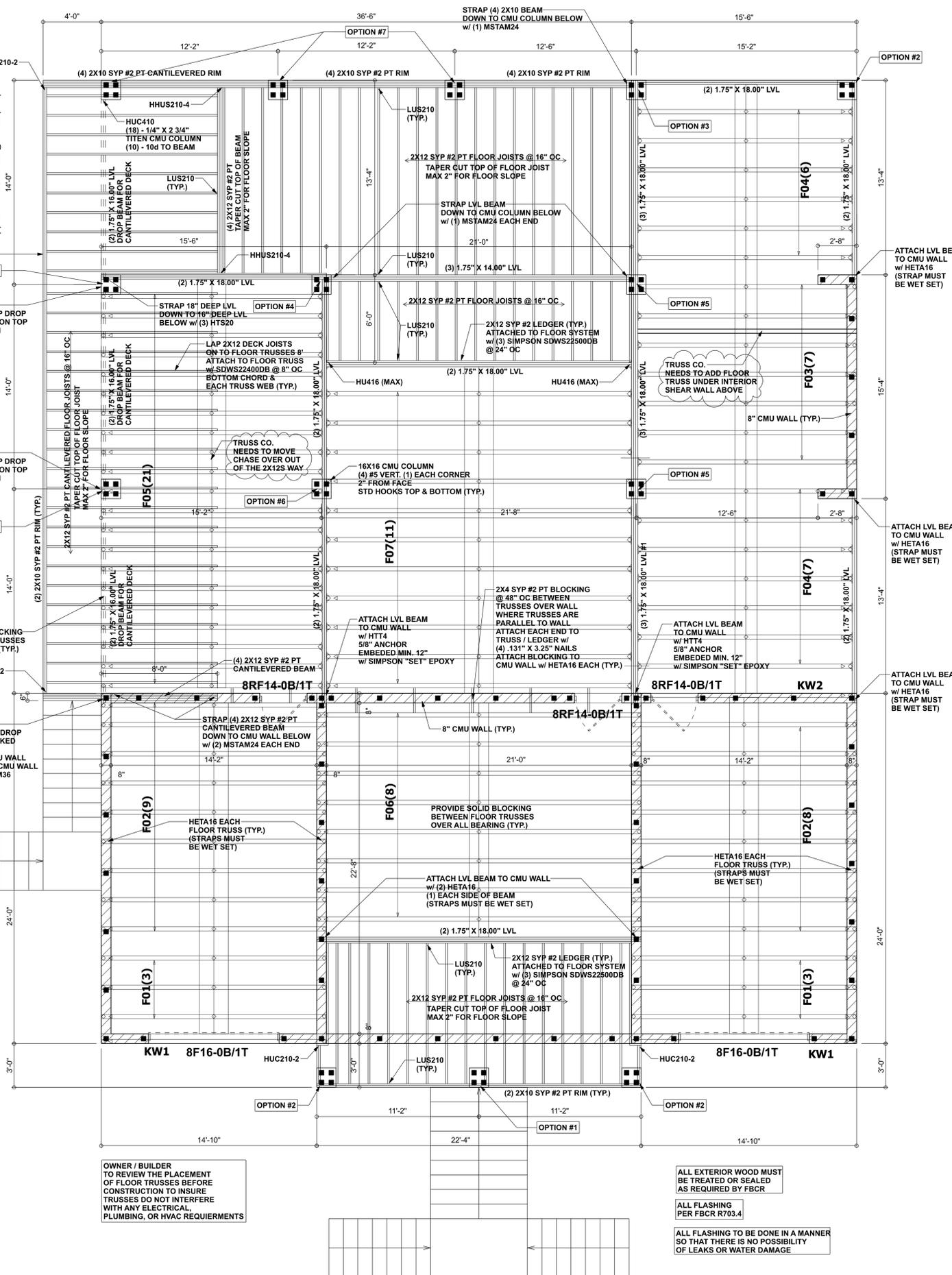
ALL EXTERIOR WOOD MUST BE TREATED OR SEALED AS REQUIRED BY FBCR

ALL FLASHING PER FBCR R703.4

ALL FLASHING TO BE DONE IN A MANNER SO THAT THERE IS NO POSSIBILITY OF LEAKS OR WATER DAMAGE



BEAM SPLICE & STEEL ANGLE LOCATION DETAILS



GROUND FLOOR STRUCTURAL PLAN & 1ST FLOOR FRAMING

SCALE: 1/4" = 1'-0"

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PROJECT ADDRESS:
519 Riverside Ave.
Fort White, FL 32038

FL PE 53915
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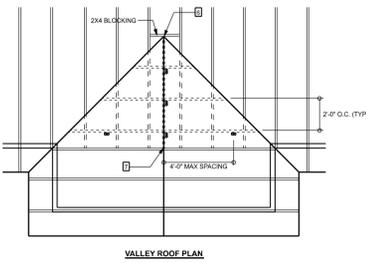
LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
250555

S-2.1
OF 9 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
ROOF BOARD	2X6 SYP #2
RAFTER SPANS 20'0" OR LESS	2X4 SYP #2
PURLINS (LATERAL BRACING)	2X4 SYP #2
BLEEBERS	2X4 SYP #2 OR 2X4 SYP #2
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL

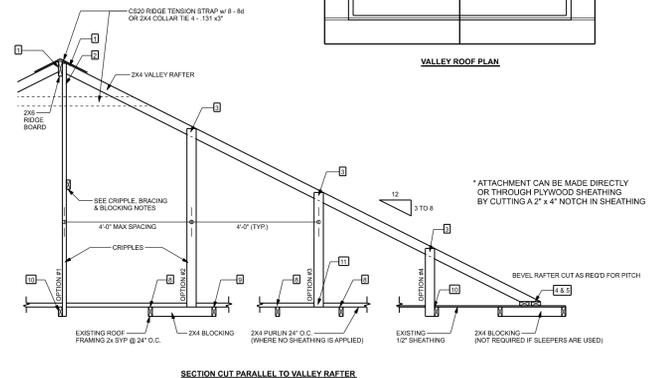


VALLEY ROOF PLAN MEMBER LEGEND	
TRUSS	1
TRUSS UNDER VALLEY FRAMING	2
VALLEY RAFTER OR RIDGE	3
CRIPPLE	4

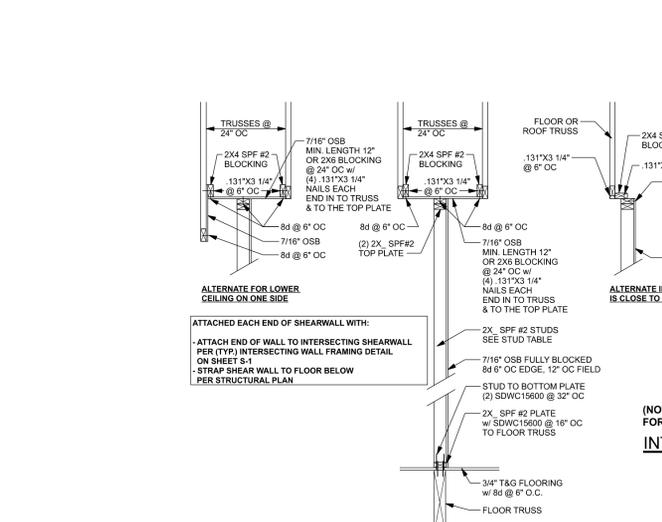
CONNECTION REQUIREMENT NOTES	
1	2X4 RAFTERS TO RIDGE 4-131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE 4-131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS 4-131 x 3" FACE NAILS
4	RAFTER TO BLEEBER OR BLOCKING 4-131 x 3" TOE NAILS
5	BLEEBER TO TRUSS 4-131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK 4-131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS 4-131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP) 4-131 x 3" NAILS
9	PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN 4-131 x 3" NAILS
10	CRIPPLE TO TRUSS 4-131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN 4-131 x 3" FACE NAILS

GENERAL NOTES
 MAXIMUM RAFTER SPAN: 6'0" FOR 2X4 SYP #2 OR SYP #2
 MAXIMUM ROOF AREA PER SUPPORT: 150 SQ. FT. (EXAMPLE: 4'0" O.C. X 4'0" SPN *150' OR 2'0" X 6'0" SPAN *150)
 PURLIN REQUIRED: 2" O.C. IF EXISTING SHEATHING IS REMOVED, PURLIN SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING MINIMUM 2" IF AND IN ALL UPWARD THROUGH SHEATHING INTO PURLIN WITH MINIMUM 2" X 4" COMMON WIRE NAILS
 THE FOLLOWING CONDITIONS APPLY TO ALL TRUSSES:
 - SPAN (DISTANCE BETWEEN HEADS) 4'0" OR LESS
 - MAXIMUM WIND SPEED: 130 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS RES. ASSET WIND REQUIREMENTS - EXPOSURE CATEGORY "C", I = 1.0, K1 = 1.0 - ENCLOSED BUILDING

CRIPPLE BRACING & BLOCKING NOTES
 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 6'0" TO 10'0" LONG NAILED 2-180 NAILS OR 2X4 1" OR SCAB NAIL TO EACH EDGE OF CRIPPLE WITH MIN. 1" O.C. 1" OR SCAB MUST BE 80% OF CRIPPLE LENGTH CRIPPLES OVER 10'0" LONG REQUIRE TWO CLBS OR BOTH FRAMES 1" O.C. OR USE STRESS CRIPPLES
 GRADED LUMBER IS 30# OR COMMON NAILS
 NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS CRIPPLE IS FULLY BLOCKED WITH MIN. 1" O.C. NAILS
 INSTALL BLOCKING UNDER RAFTERS IF BLEEBERS ARE NOT USED
 INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
 APPL. ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



ROOF OVER FRAMING & BRACING DETAIL
 SCALE: N.T.S.

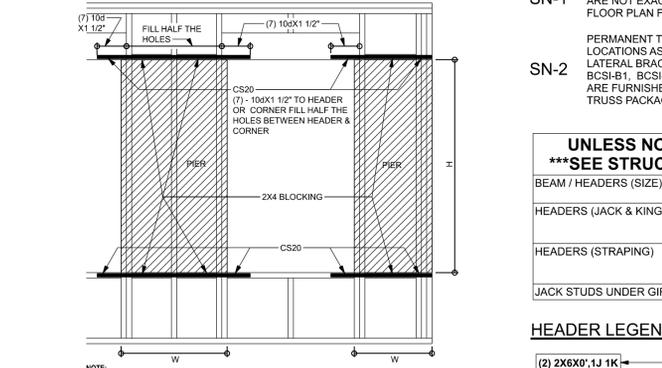


STRUCTURAL PLAN NOTES
 SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
 SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

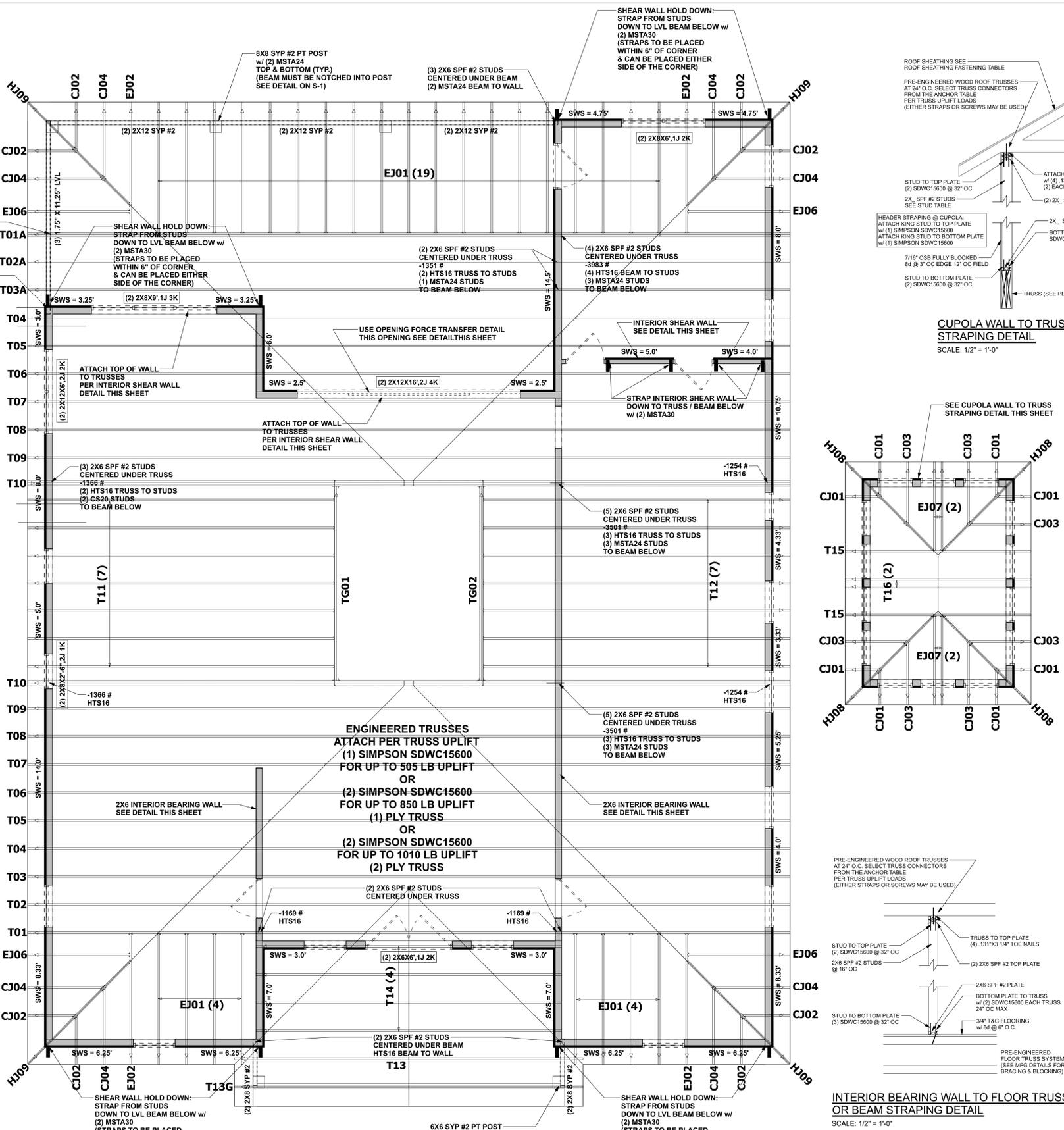
UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS) *SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS*****

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SPF #2 (U.N.O.)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) SP#6 ON KING STUD @ TOP OF WALL & (1) MSTA24 STRAP FROM KING STUD TO RIM BEAM BOTTOM
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND	
(2) 2X6X6' .1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
(1) 100X1 1/2"	NUMBER OF KING STUDS EACH SIDE OF OPENING (FULL LENGTH)
(1) 100X1 1/2"	NUMBER OF JACK STUDS EACH SIDE OF OPENING (UNDER HEADER)
(1) 100X1 1/2"	SPAN OF HEADER
(1) 100X1 1/2"	SIZE OF HEADER MATERIAL
(1) 100X1 1/2"	NUMBER OF PLIES IN HEADER

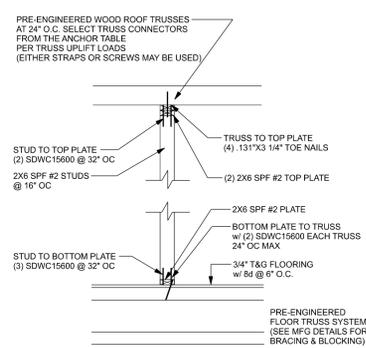
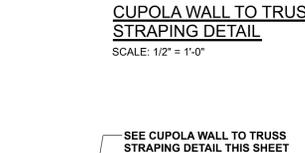
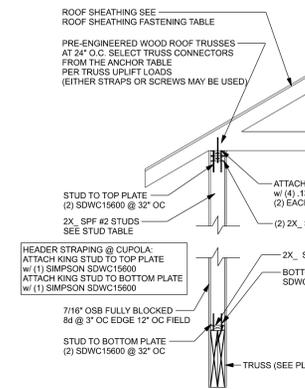


NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BEHIND THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT HW > 1.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGH OR 9' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.



STRUCTURAL PLAN
 SCALE: 1/4" = 1'-0"

	ACTUAL vs REQUIRED SHEARWALL	
	TRANSVERSE	LONGITUDINAL
ACTUAL	14750 LBF	28036 LBF
REQUIRED	13642 LBF	9828 LBF



Corey Amira Custom Homes
 Caruthers Res.
 PROJECT ADDRESS:
 Fort White, FL 32838

FL PE 53915
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 Mark Disoway P.E.
 163 SW Midtown Place
 Suite 103
 Lake City, Florida 32025
 386.754.5419
 disowaydesign@gmail.com

JOB NUMBER:
 250555
 S-3
 OF 9 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2809539