ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UJ0487-Z0224113549

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

Truss Fabricator:	Anderson Truss Company
Job Identification:	12-012--Fill in later BRYAN ZECHER/COLEMAN -- , **
Truss Count:	40
Model Code:	Florida Building Code 2007 and 2009 Supplement
Truss Criteria:	FBC2007Res/TPI-2002(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

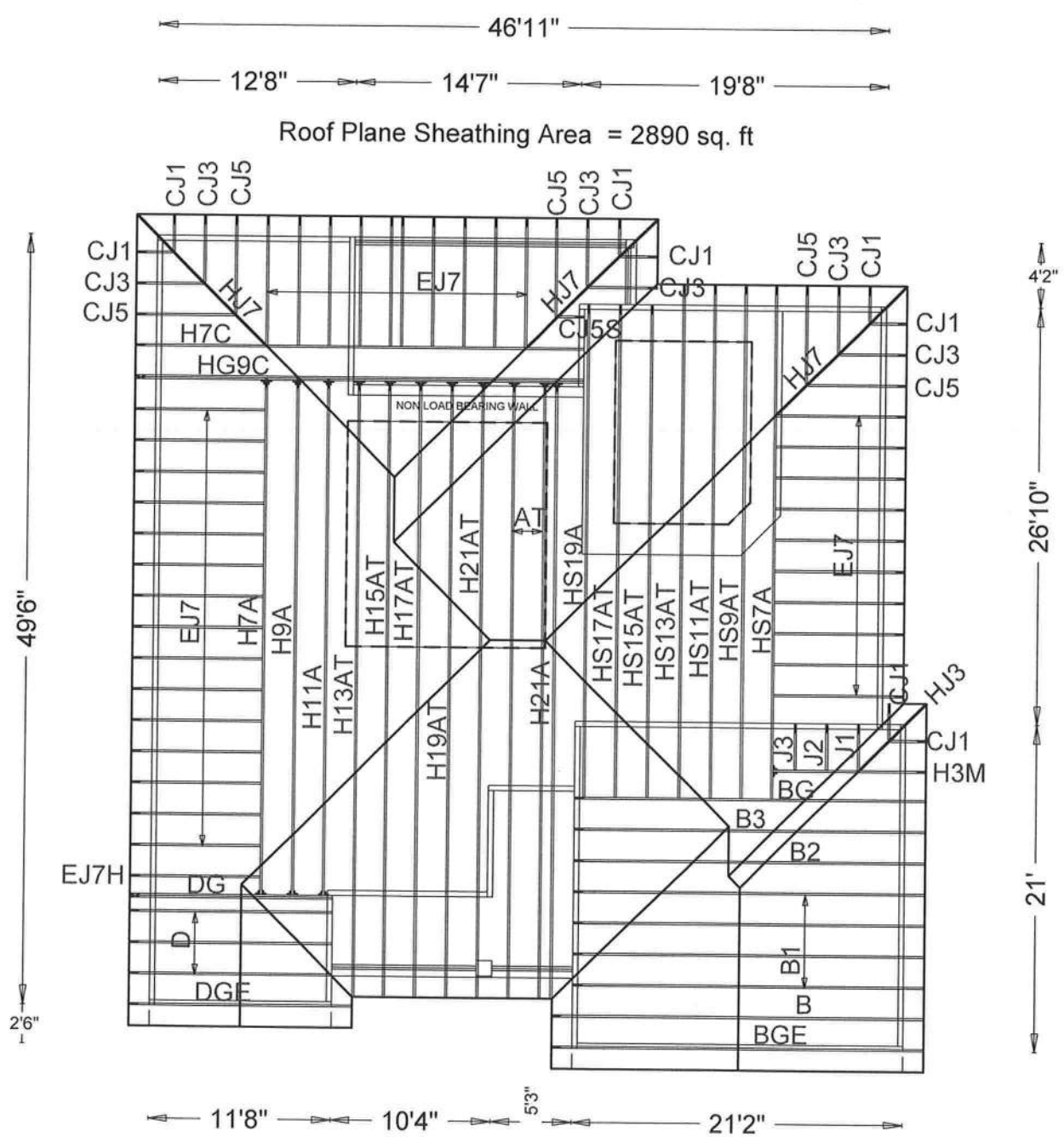
1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-A1101505-GBLLETIN-

#	Ref Description	Drawing#	Date
1	58403--H7A	12024029	01/24/12
2	58404--H9A	12024015	01/24/12
3	58405--H11A	12024027	01/24/12
4	58406--H21A	12024005	01/24/12
5	58407--HS7A	12024040	01/24/12
6	58408--HS19A	12024006	01/24/12
7	58409--HS9AT	12024002	01/24/12
8	58410--HS17AT	12024008	01/24/12
9	58411--HS11AT	12024018	01/24/12
10	58412--HS13AT	12024009	01/24/12
11	58413--HS15AT	12024039	01/24/12
12	58414--H13AT	12024019	01/24/12
13	58415--H15AT	12024014	01/24/12
14	58416--H17AT	12024003	01/24/12
15	58417--AT	12024024	01/24/12
16	58418--H21AT	12024023	01/24/12
17	58419--H19AT	12024021	01/24/12
18	58420--BGE	12024028	01/24/12
19	58421--B	12024033	01/24/12
20	58422--B1	12024036	01/24/12
21	58423--B2	12024034	01/24/12
22	58424--B3	12024031	01/24/12
23	58425--BG	12024038	01/24/12
24	58426--HG9C	12024025	01/24/12
25	58427--H7C	12024037	01/24/12
26	58428--D	12024011	01/24/12
27	58429--DGE	12024001	01/24/12
28	58430--DG	12024012	01/24/12
29	58431--EJ7	12024026	01/24/12
30	58432--CJ5	12024013	01/24/12
31	58433--CJ3	12024017	01/24/12
32	58434--CJ1	12024016	01/24/12
33	58435--HJ7	12024020	01/24/12
34	58436--HJ3	12024032	01/24/12
35	58437--J1	12024010	01/24/12
36	58438--J2	12024035	01/24/12

#	Ref Description	Drawing#	Date
37	58439--J3	12024030	01/24/12
38	58440--EJ7H	12024007	01/24/12
39	58441--CJ5S	12024004	01/24/12
40	58442--H3M	12024022	01/24/12

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER



BRYAN ZECHER/COLEMAN

JOB DESCRIPTION:: Fill in later
/: BRYAN ZECHER/COLEMAN

JOB NO:
12-012

PAGE NO:
1 OF 1

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H7A)

Top chord 2x6 SP #2 :T3 2x4 SP #1:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W2, W10 2x4 SP #2:

End verticals not exposed to wind pressure.

Max JT VERT DEFL: LL: 0.32" DL: 0.33" recommended camber 5/8"

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

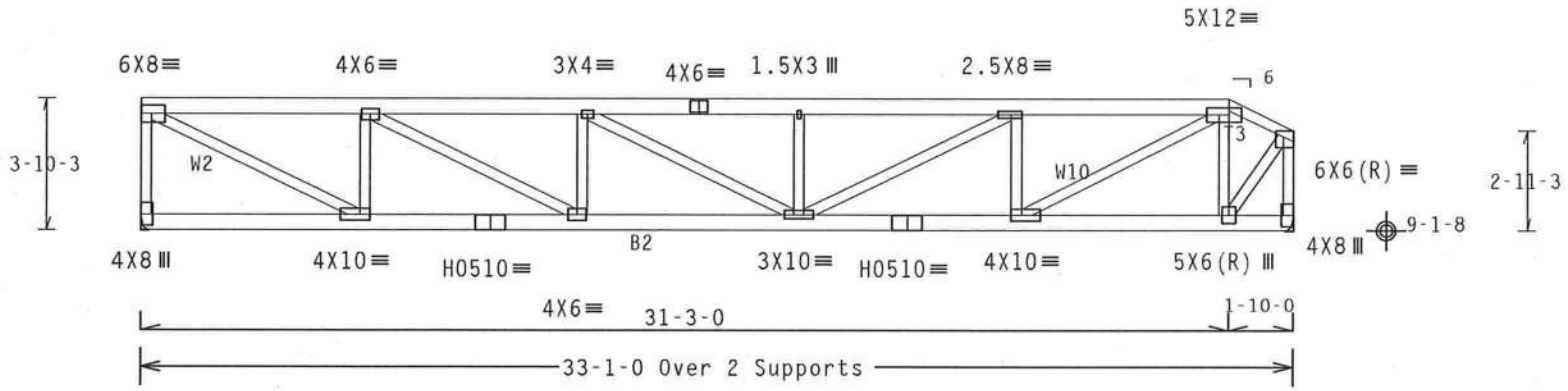
Girder supports 7-0-0 span to TC/BC split one face and 2-0-0 span to TC/BC split opposite face.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/240 live and L/180 total load.



R=3065 U=276 H=Simpson HUS26
Supported Member Face: (6) 0.162"x3.5" nails
Supporting Member Face: (14) 0.162"x3.5" nails
Supporting Member : (2) 2x6 SP #2

R=3065 U=276 H=Simpson HUS26
Supported Member Face: (6) 0.162"x3.5" nails
Supporting Member Face: (14) 0.162"x3.5" nails
Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58403
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024029
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265124
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H9A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

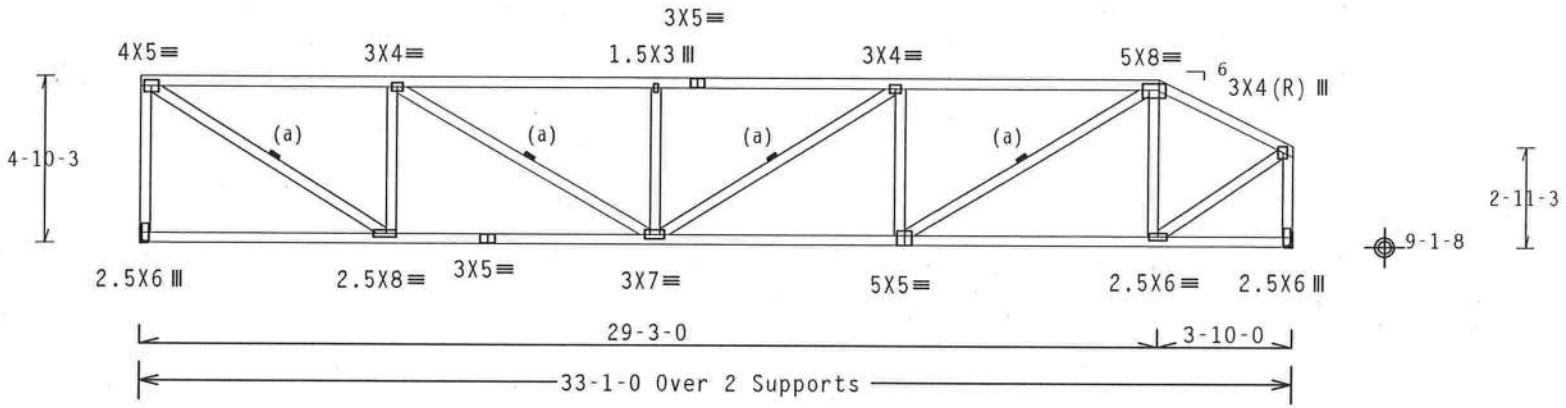
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1362 U=147
RL=-44 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1362 U=132 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.04.0601.16 | QTY 1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58404
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024015
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265137
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H11A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

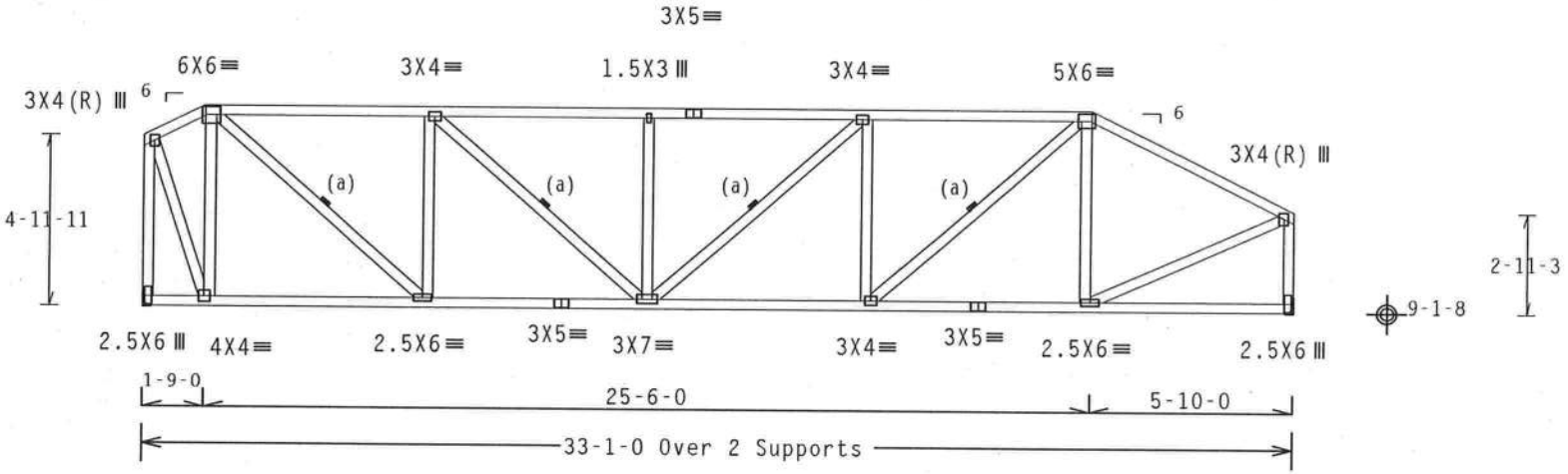
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1362 U=143
RL=29/-49 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1362 U=129 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF R487-- 58405
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024027
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265156
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H21A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

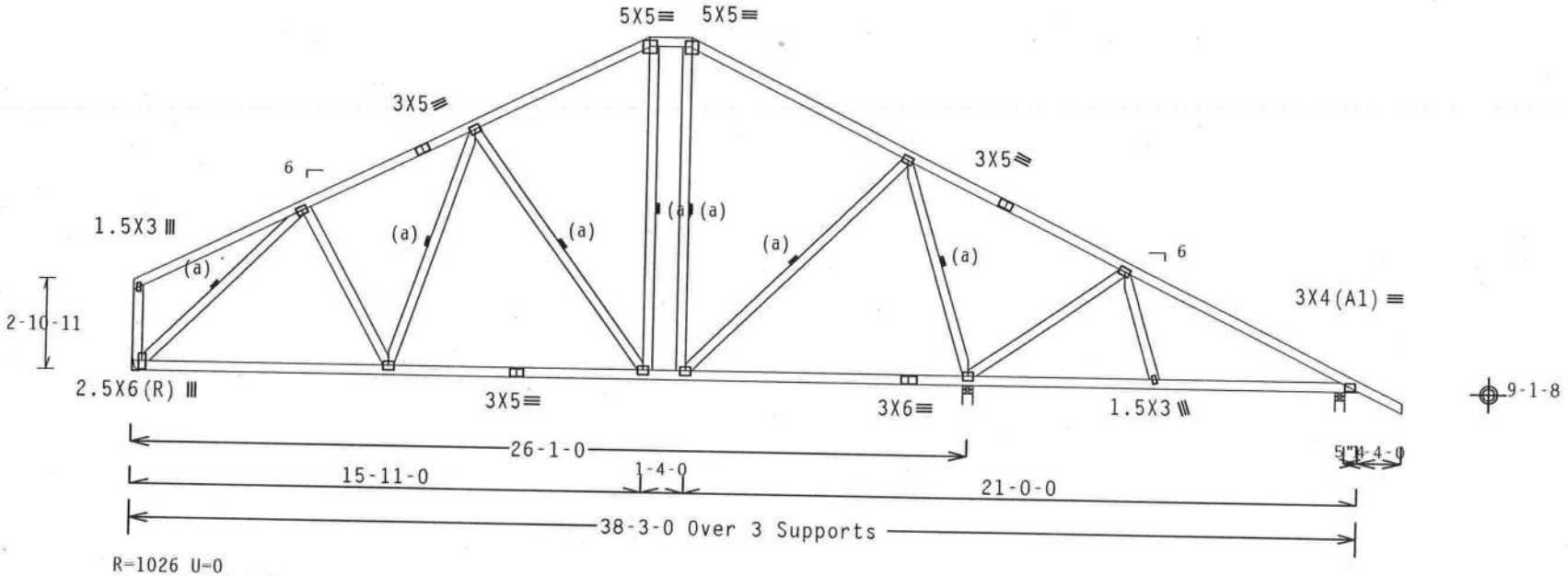
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1026 U=0
RL=205/-221 H=Simpson LU26
Supported Member Face: (4) 0.148"x1.5" nails
Supporting Member Face: (6) 0.162"x3.5" nails
Supporting Member : (2) 2x6 SP #2

R=516 U=0 W=3.5"

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 6

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
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FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

TC LL	20.0 PSF	REF R487-- 58406
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024005
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265304
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS7A)

Top chord 2x6 SP #2 :T1 2x4 SP #1:
Bot chord 2x6 SP #1 Dense :B2 2x6 SP #2:
Webs 2x4 SP #3 :W8 2x4 SP #2:

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	62 plf at	-1.33 to	62 plf at	0.00
TC- From	62 plf at	0.00 to	62 plf at	7.00
TC- From	132 plf at	7.00 to	132 plf at	31.71
BC- From	4 plf at	-1.33 to	4 plf at	0.00
BC- From	20 plf at	0.00 to	20 plf at	7.00
BC- From	44 plf at	7.00 to	44 plf at	31.71

BC- 498.54 lb Conc. Load at 7.00
BC- 495.00 lb Conc. Load at 29.83

Wind loads and reactions based on MWFRS with additional C&C member design.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

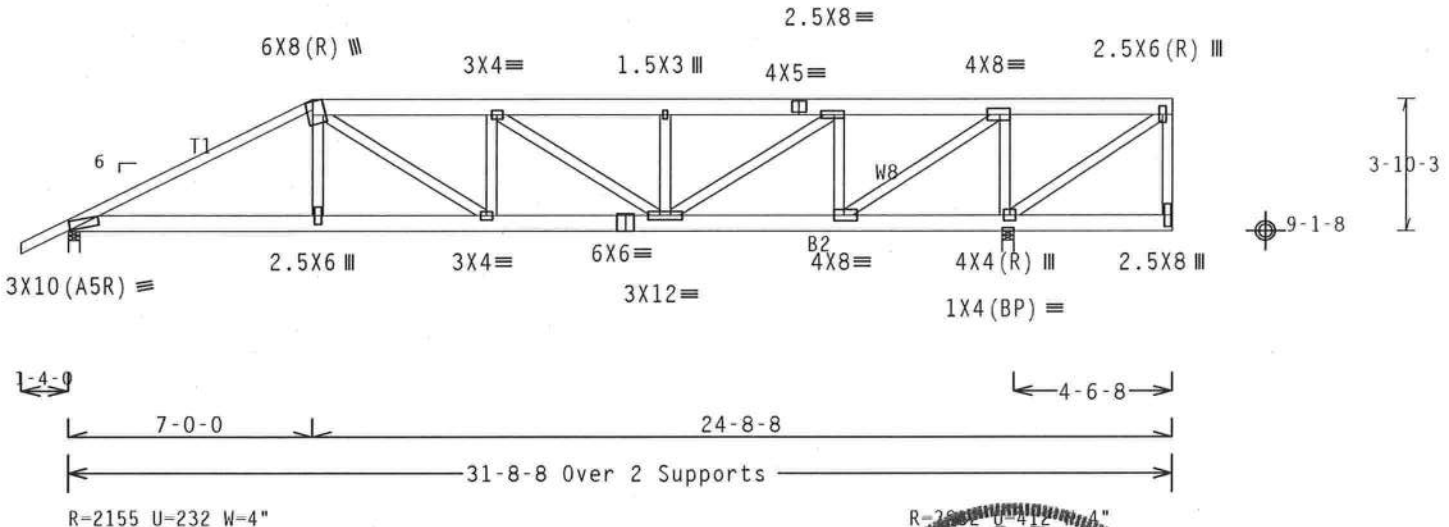
Right end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58407
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024040
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265470
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS19A)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Right cantilever is exposed to wind

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

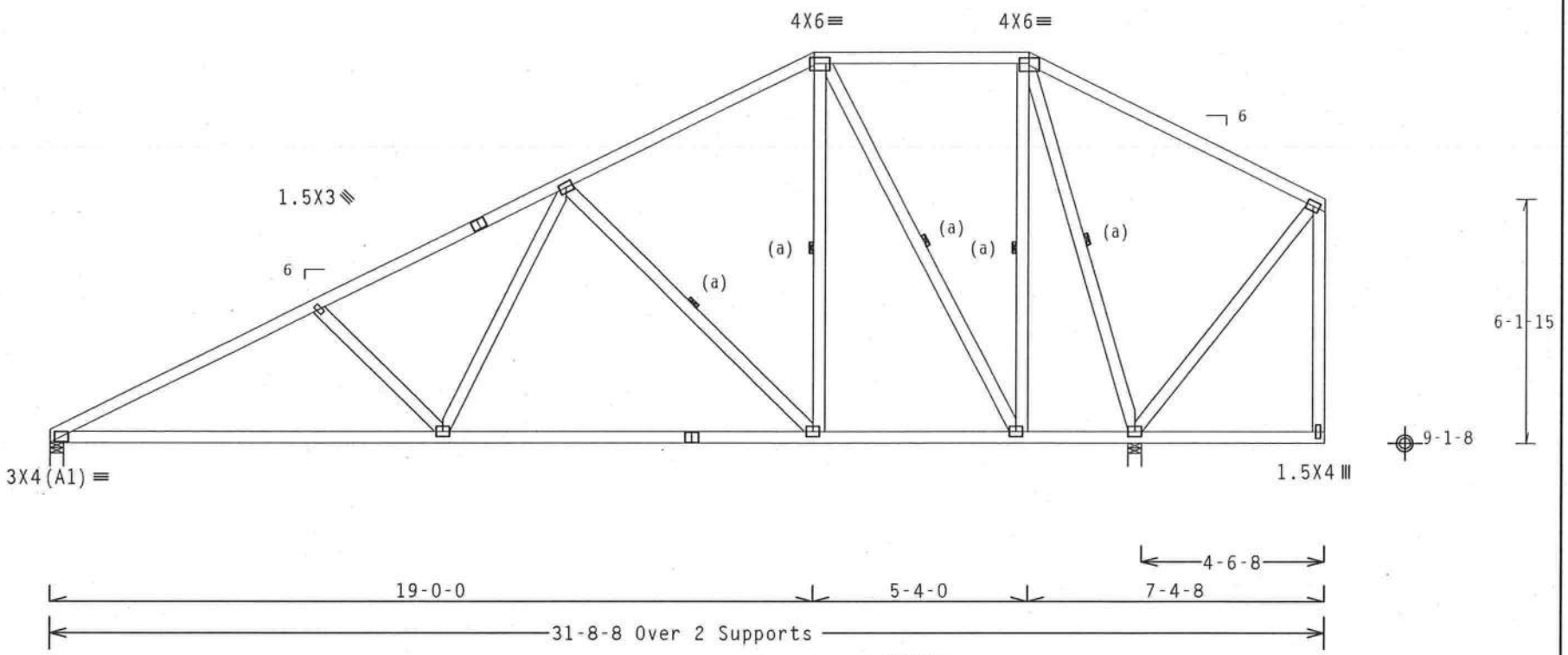
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1084 U=0 W=4"
RL=166/-108

R=1528 U=59 W=4"

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58408
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024006
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 265287
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS9AT)

Top chord 2x4 SP #1 :T1 2x6 SP M-26:
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.21" due to live load and 0.22" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

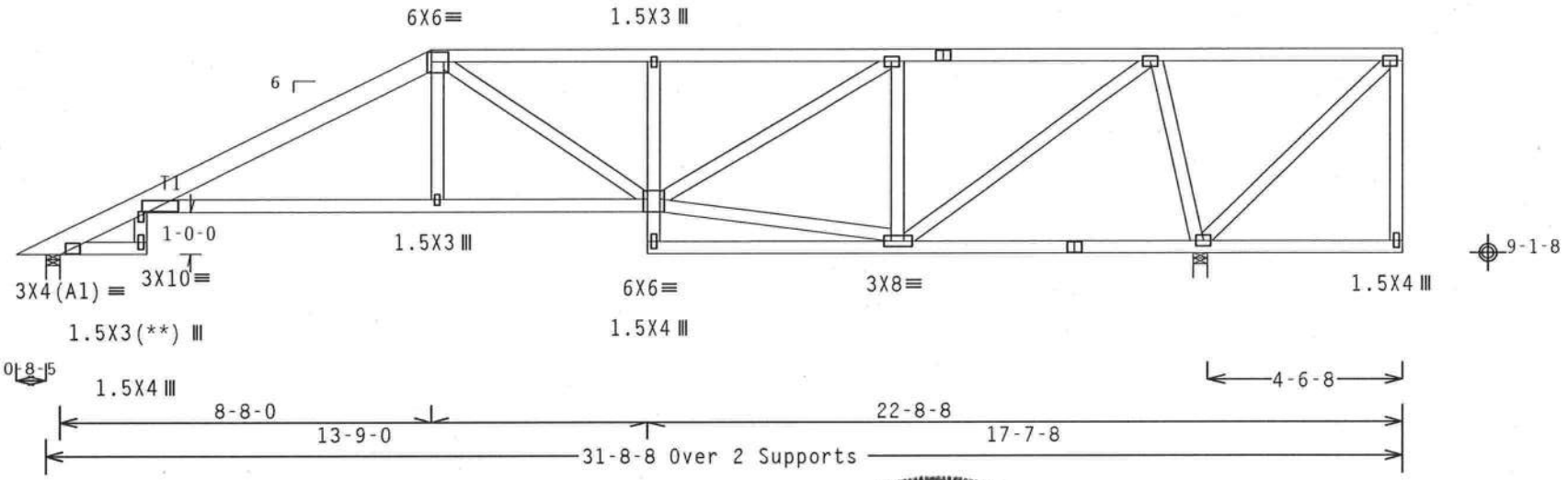
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.00.04/0601.16 QTY: FL/-/4/-/-/R/- Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58409
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024002
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265109
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS17AT)

Top chord 2x4 SP #1 :T1 2x6 SP M-26:
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.23" due to live load and 0.24" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

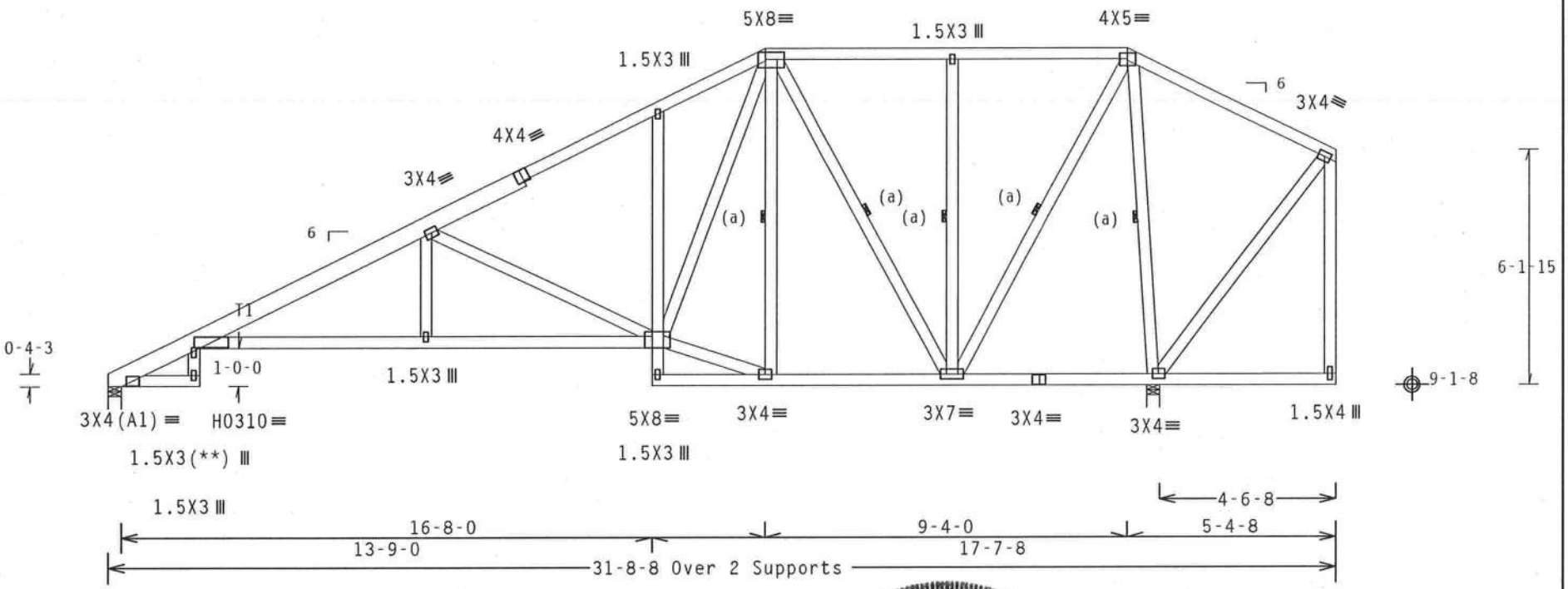
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.05.04 0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

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TC LL	20.0 PSF	REF R487-- 58410
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024008
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265106
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS11AT)

Top chord 2x4 SP #1 :T1 2x6 SP SS:
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.18" due to live load and 0.19" due to dead load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

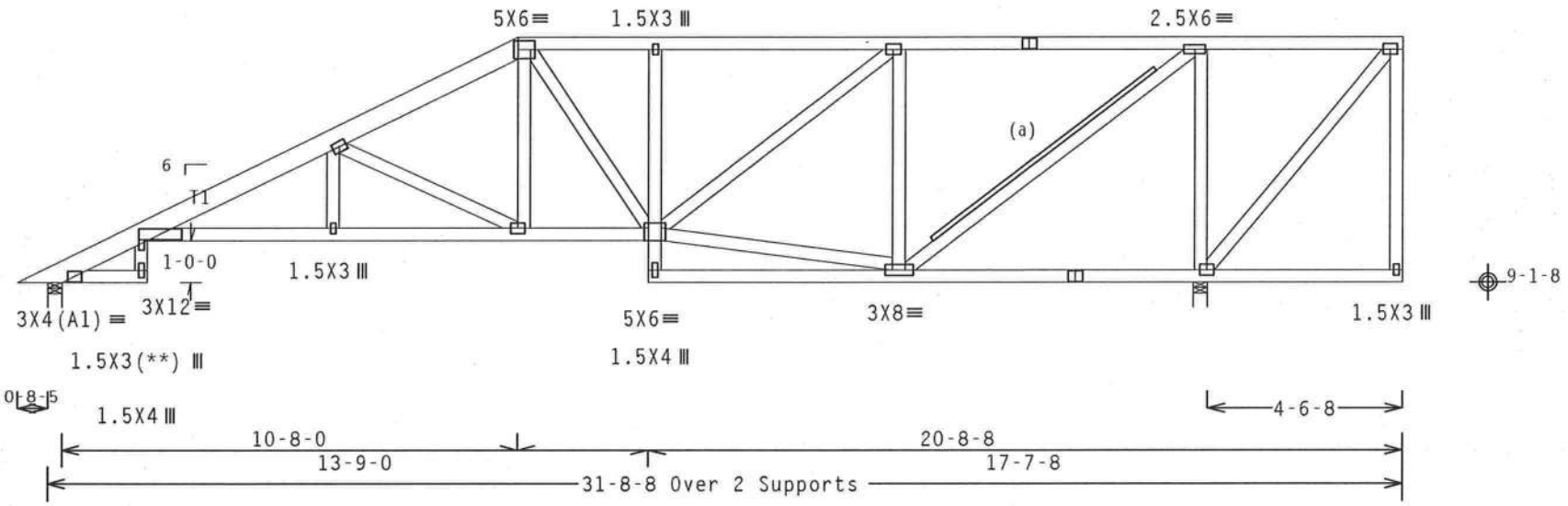
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58411
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024018
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265238
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS13AT)

Top chord 2x4 SP #1 :T1 2x6 SP M-26:
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.18" due to live load and 0.19" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

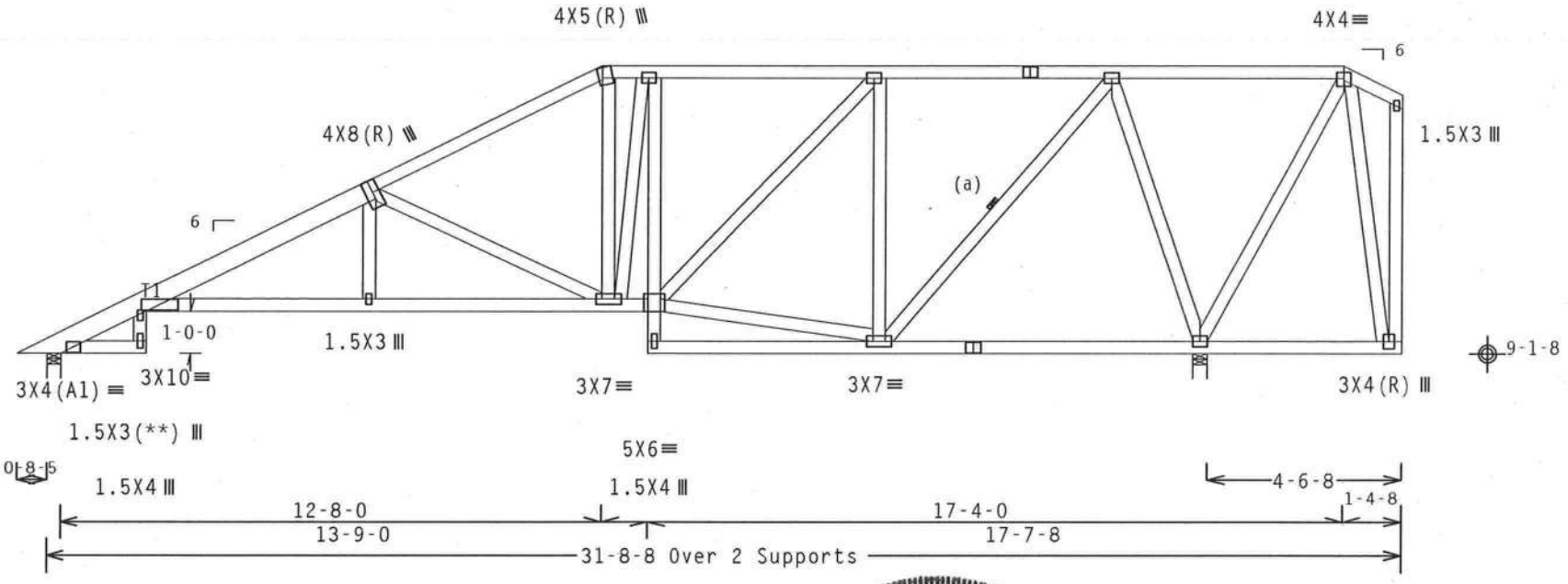
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04/0601.16 QTY: FL/-/4/-/-/R/- Scale =.25"/Ft.

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ALPINE
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TC LL	20.0 PSF	REF R487-- 58412
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024009
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265235
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HS15AT)

Top chord 2x4 SP #1 :T1 2x6 SP M-26:
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

Calculated horizontal deflection is 0.23" due to live load and 0.25" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

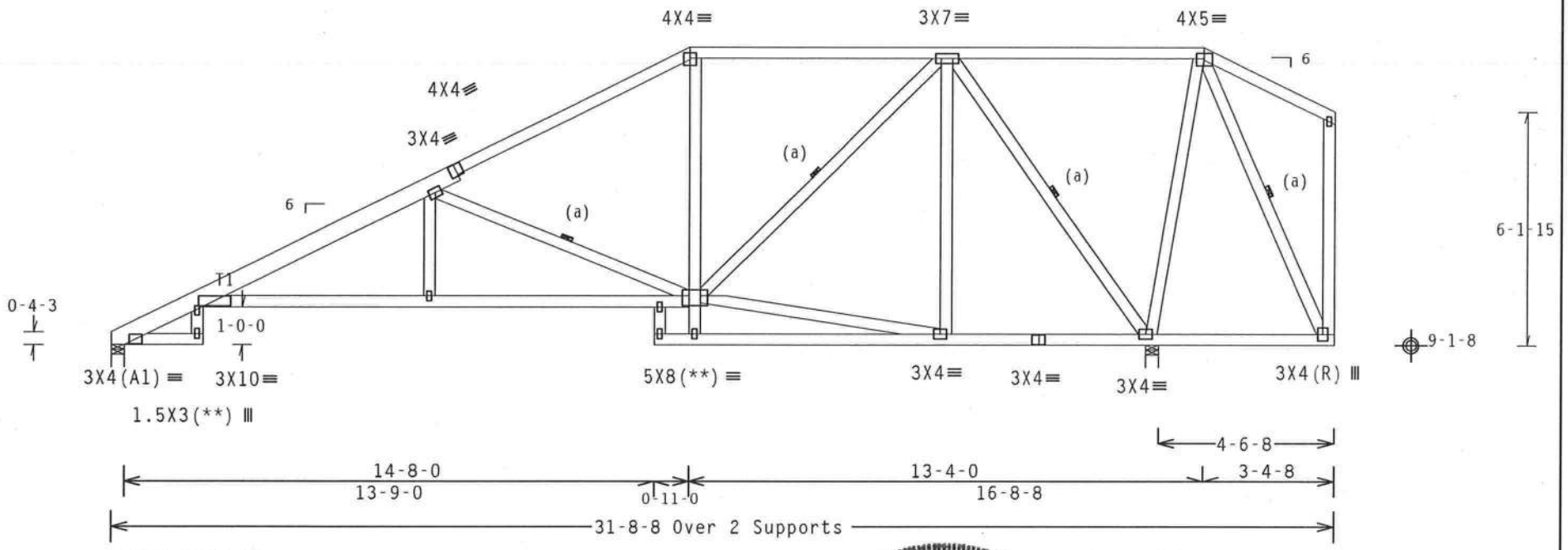
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.00.04 0601.16 QTY: FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58413
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024039
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265270
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H13AT)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W3 2x4 SP #1:

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

Calculated horizontal deflection is 0.13" due to live load and 0.14" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

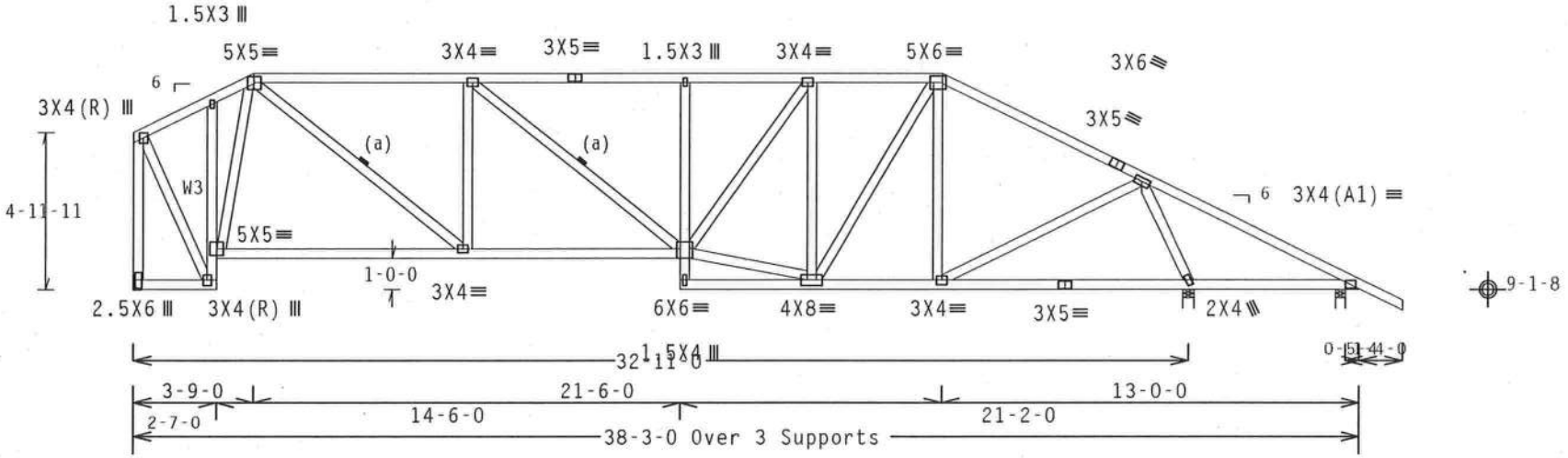
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=1304 U=139
RL=86/-122 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

1930 U=152 W=4"

R=5 Rw=76 U=66 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY: 6 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58414
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024019
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265312
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H15AT)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W3 2x4 SP #2:

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

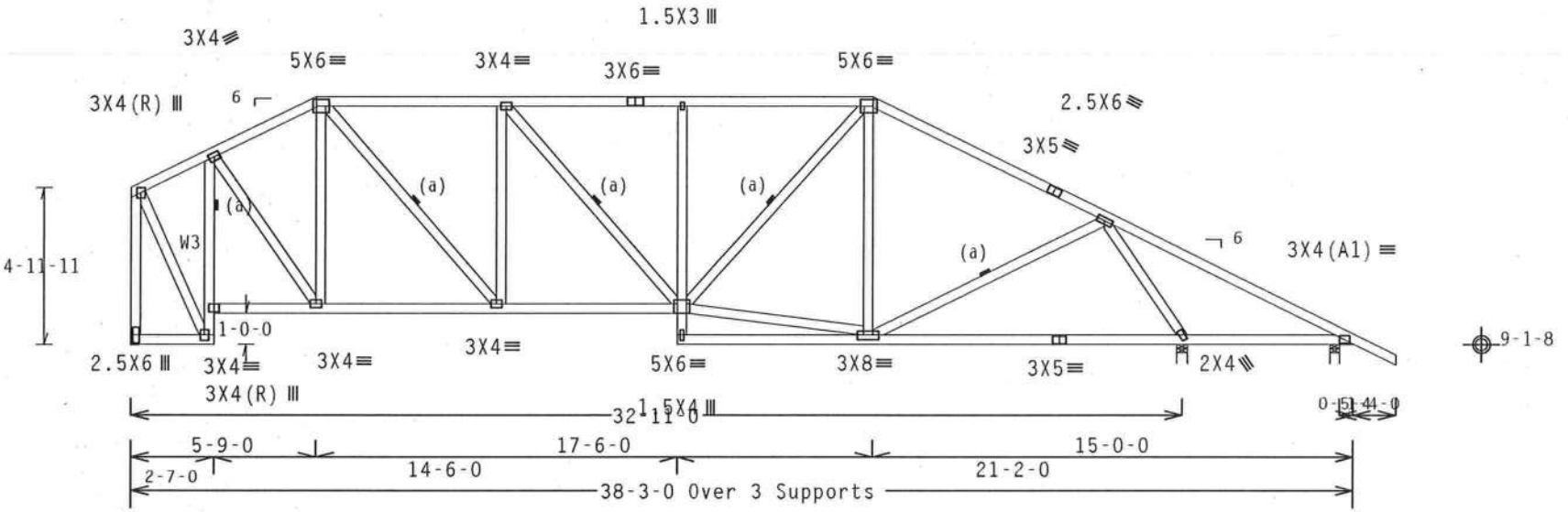
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1315 U=29
RL=108/-143 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1843 U=0 W=4"
R=81 RW=88 U=72 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04/0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58415
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024014
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265323
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H17AT)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

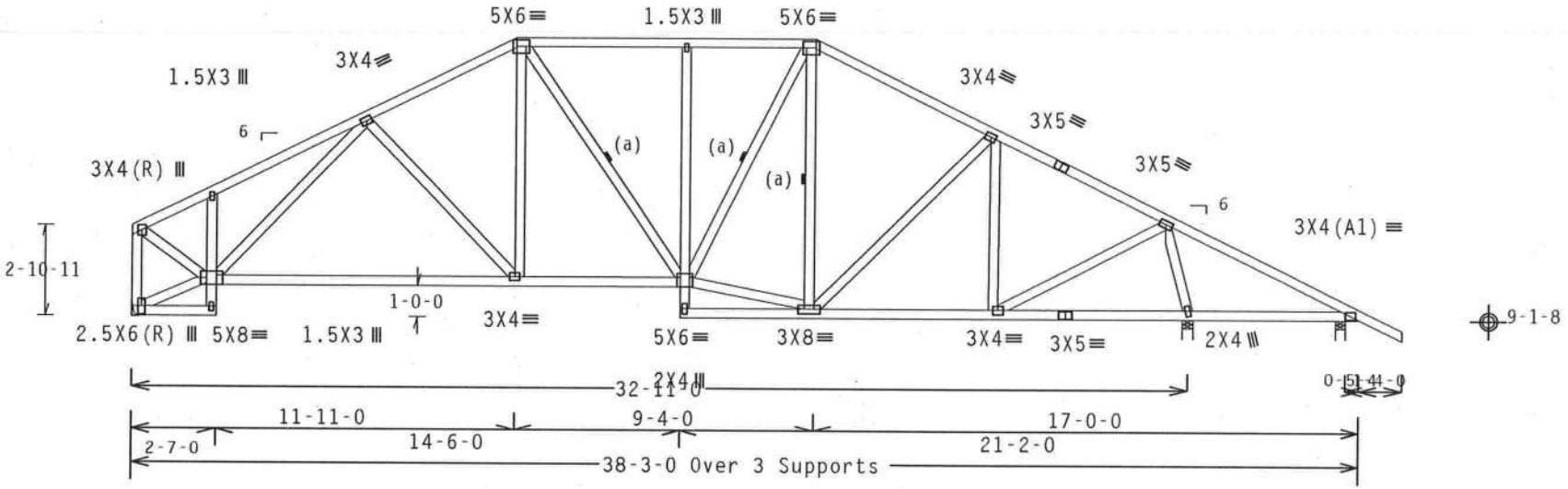
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1323 U=0
RL=162/-178 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1780 U=0 W=4"

R=136 U=20 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58416
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024003
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265348
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - AT)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

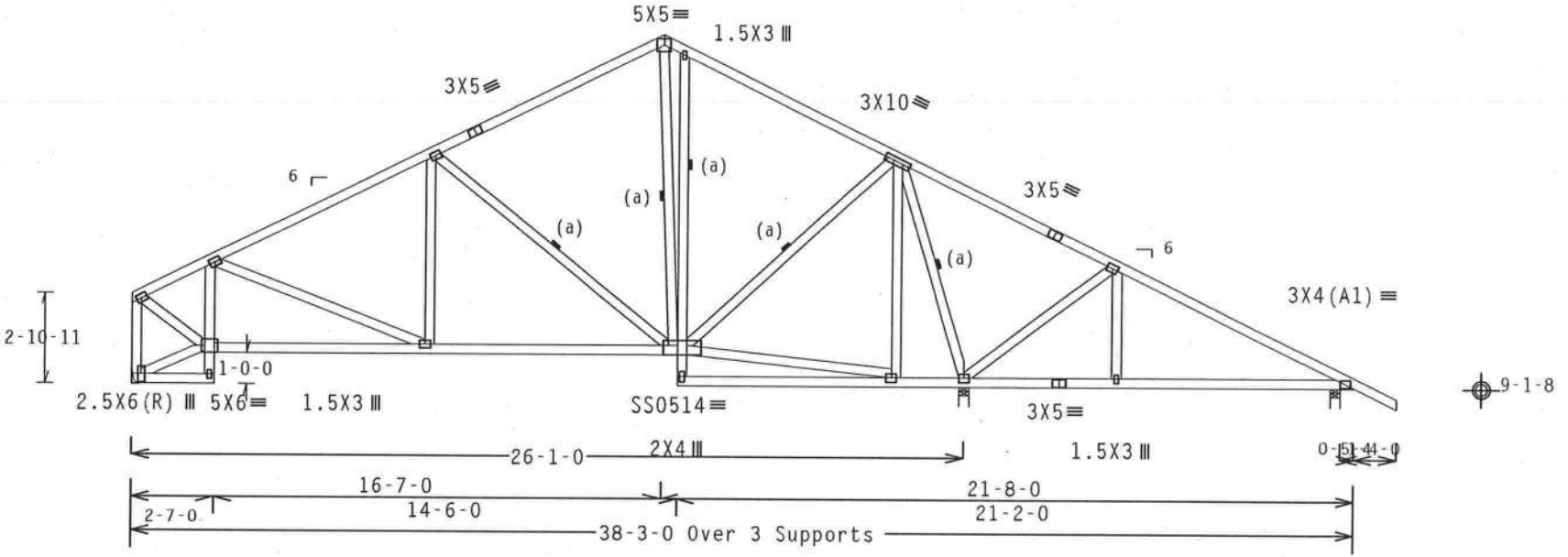
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.



R=1043 U=0
RL=212/-228 H=Simpson LUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (2) 2x6 SP #2

R=412 U=9 W=3.5"

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 18 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.04/0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

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ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 58417
DATE	01/24/12
DRW	HCUSR487 12024024
HC-ENG	TCE/DF
SEQN-	265372
JREF-	1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H21AT)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

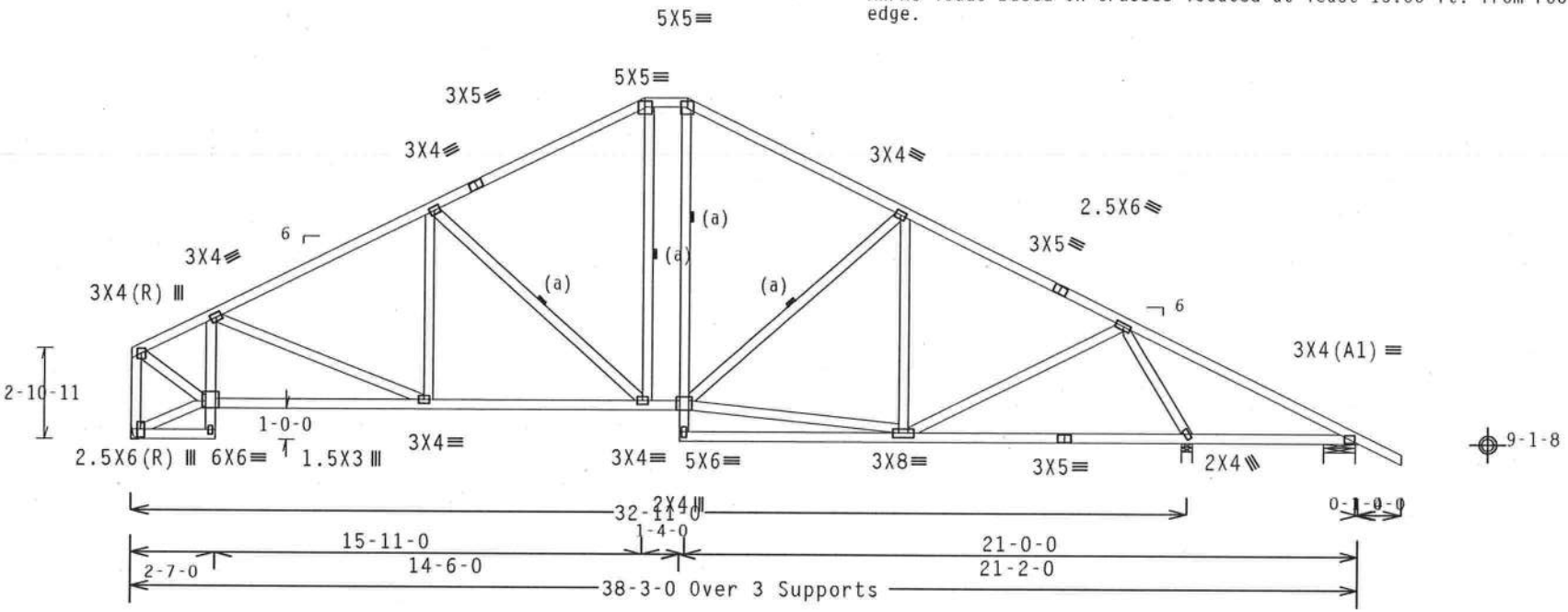
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1322 U=0
RL=206/-222 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1804 U=0 W=4"

R=113 U=41 W=12"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58418
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024023
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265397
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H19AT)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Right cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

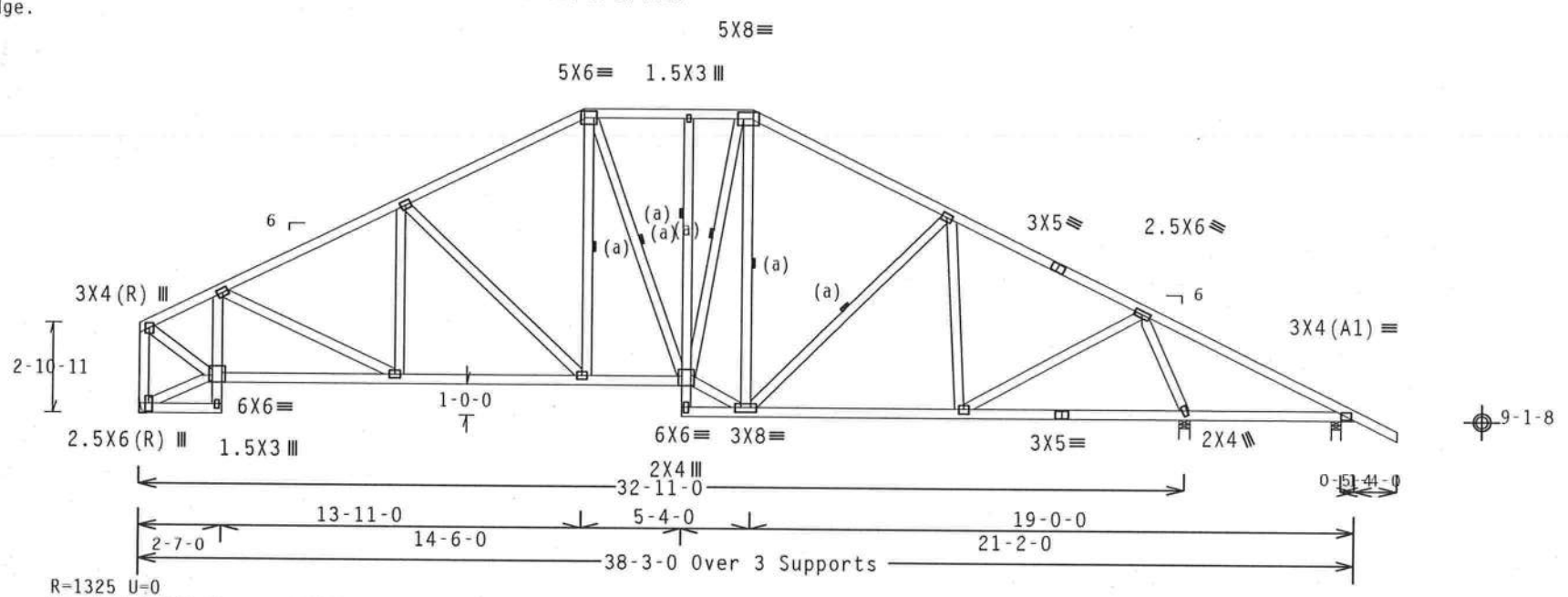
Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1325 U=0
RL=184/-199 H=Simpson LUS26
Supported Member Face: (4)
Supporting Member Face: (4)
Supporting Member : (2) 2x6 SP #2

R=1768 U=0 W=4"

R=147 U=24 W=3.5"

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

ALPINE
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FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58419
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024021
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265425
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - BGE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

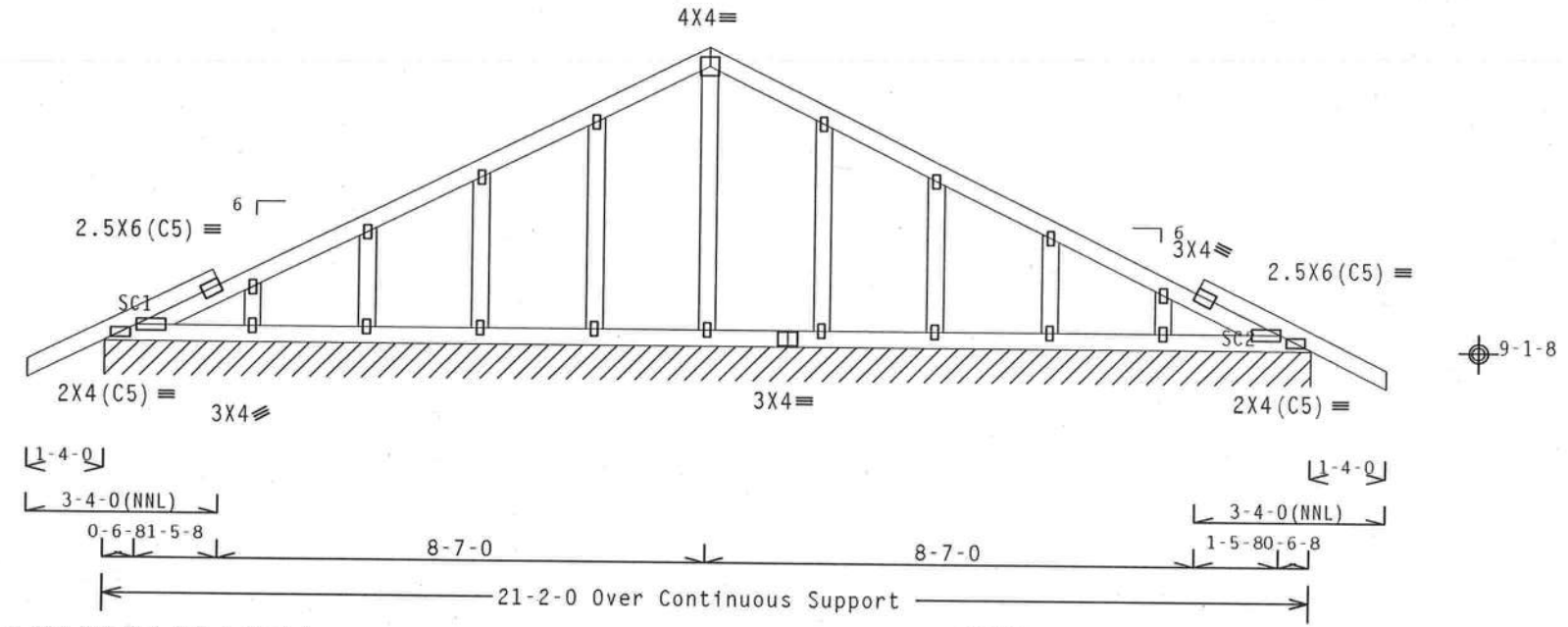
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/180 live and L/120 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R=121 PLF U=8 PLF W=21-2-0
RL=6/-6 PLF

Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58420
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024028
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 264993
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - B)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

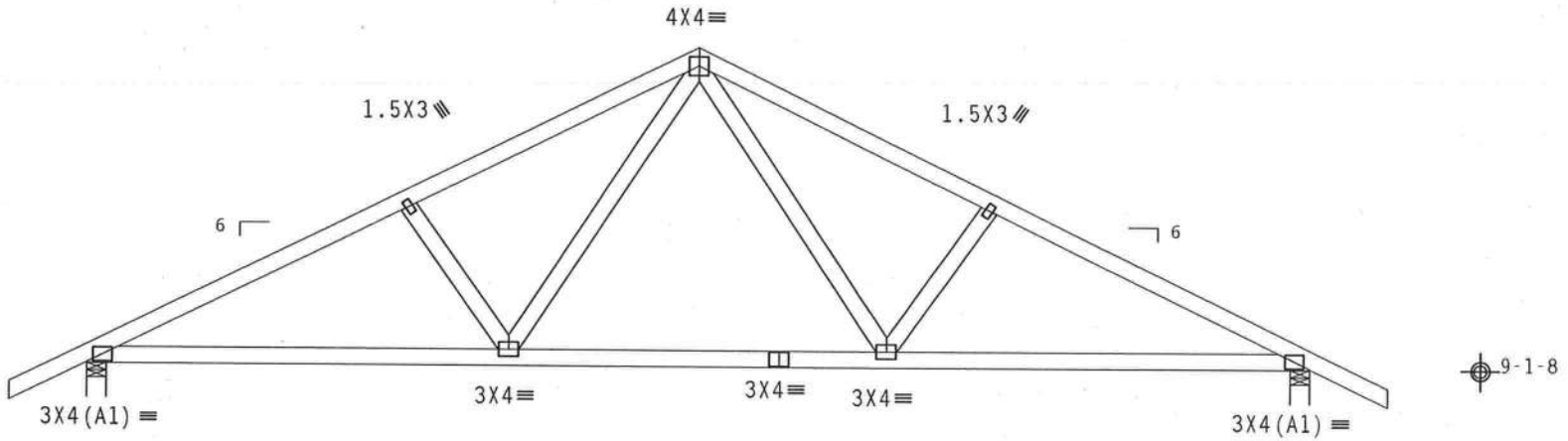
Roof overhang supports 2.00 psf soffit load.

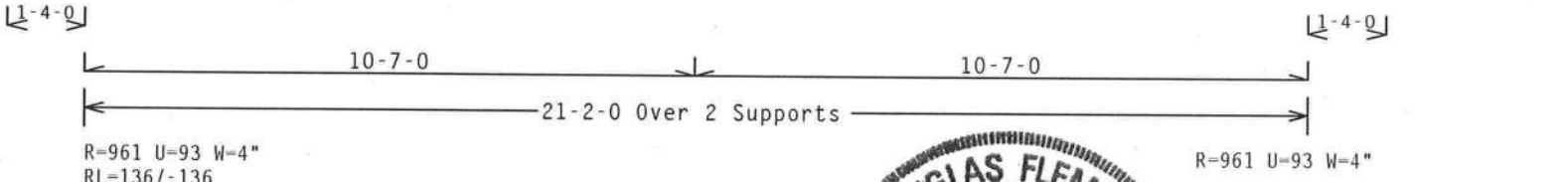
Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=961 U=93 W=4"
RL=136/-136

R=961 U=93 W=4"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.05.04 0603 16

QTY: 1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58421
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024033
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 265002
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - B1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

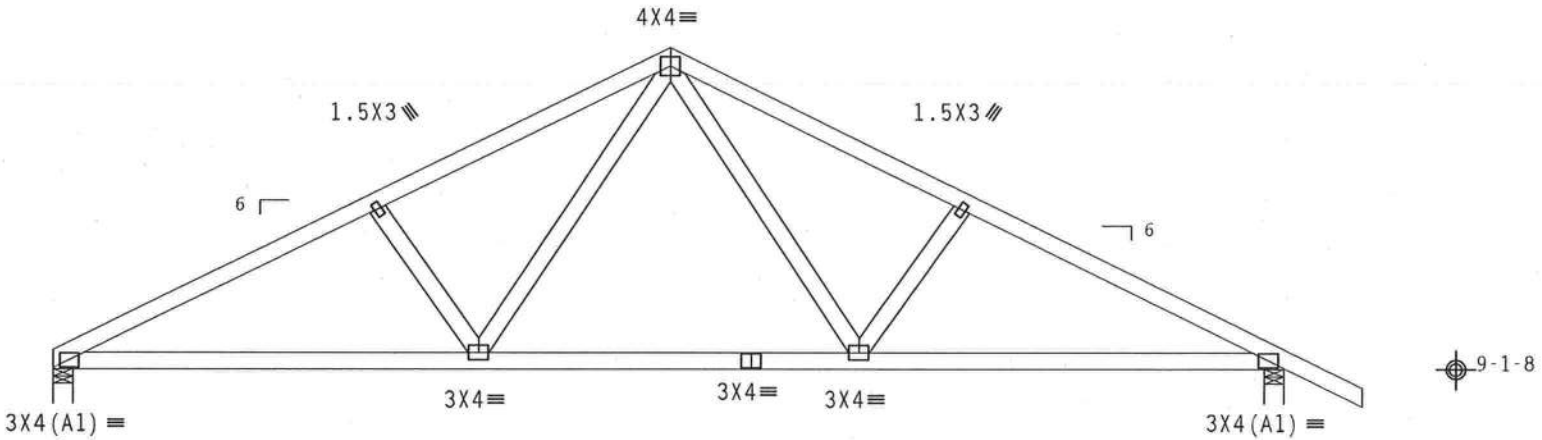
Roof overhang supports 2.00 psf soffit load.

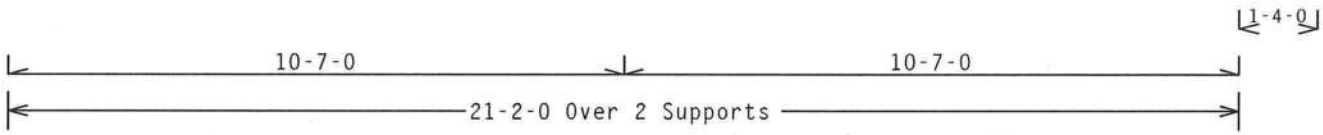
Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=868 U=75 W=4"
RL=126/-119

R=964 U=93 W=4"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.060/1.16 QTY:4 FL/-/4/-/-/R/- Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF R487-- 58422
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024036
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 265009
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - B2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

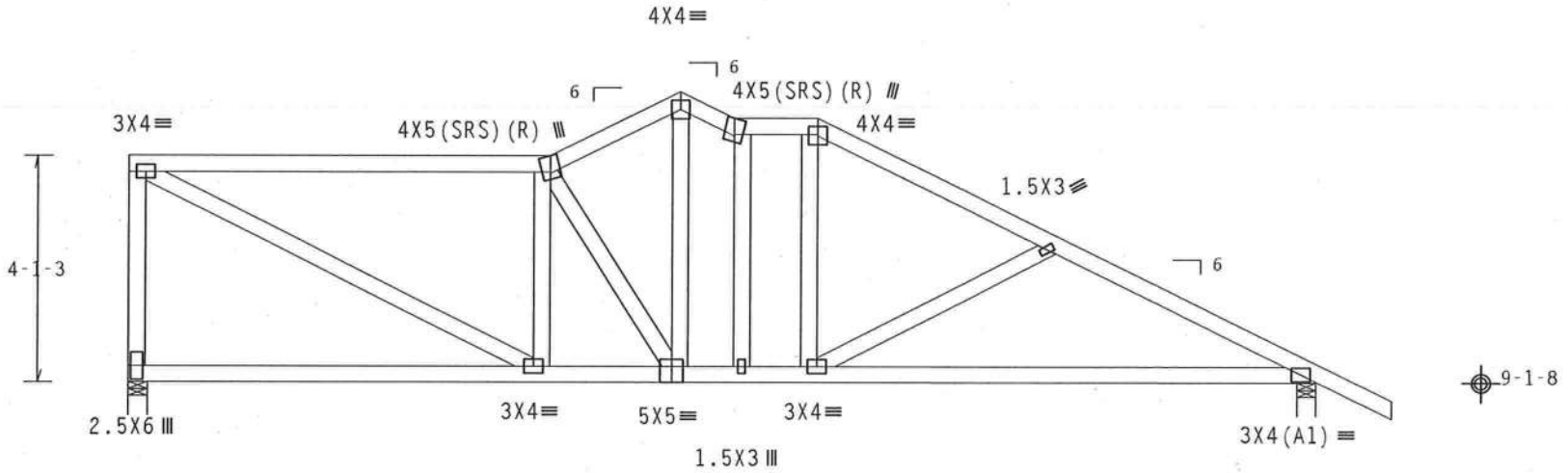
Deflection meets L/240 live and L/180 total load.

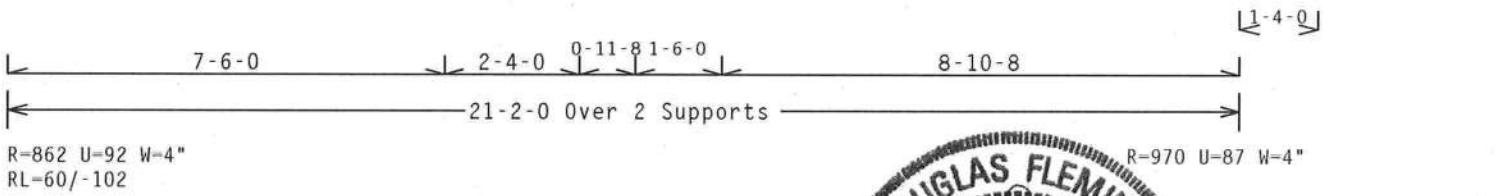
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.





Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.04.04 060J.16

QTY:

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58423
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024034
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265019
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - B3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

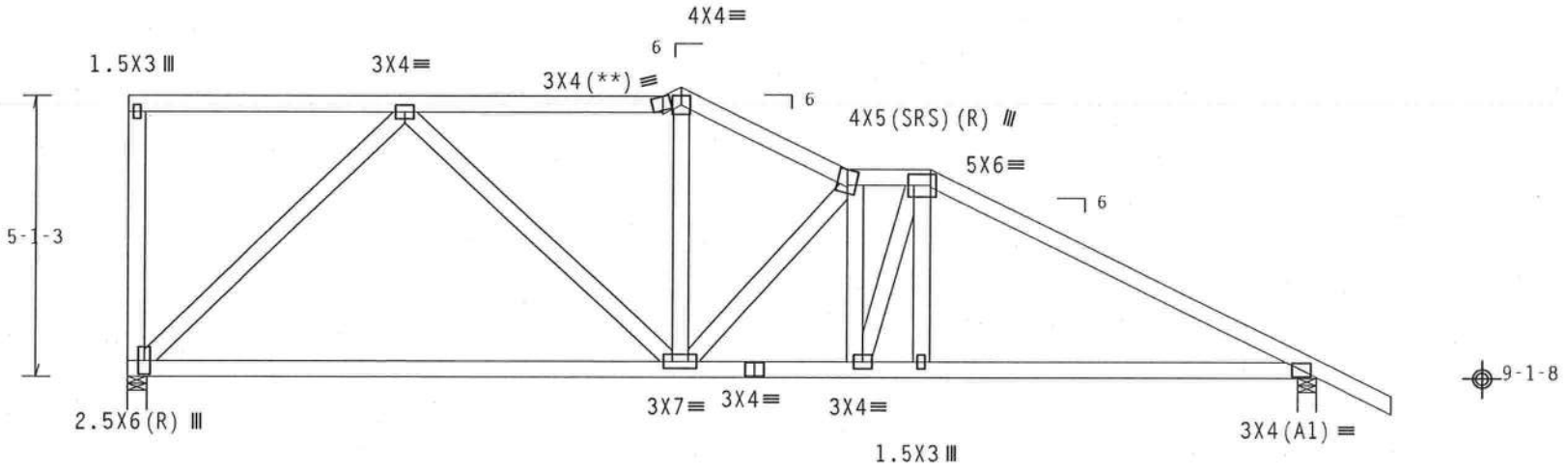
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

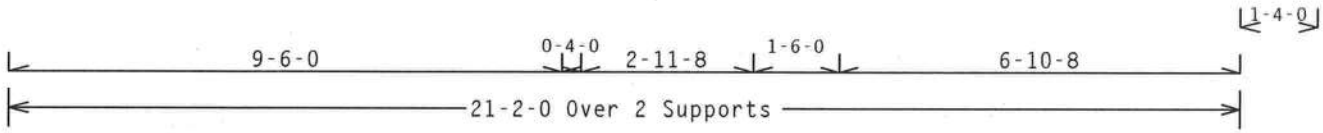
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





R=862 U=73 W=4"
RL=44/-86

R=970 U=18 W=4"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58424
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024031
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265025
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - BG)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

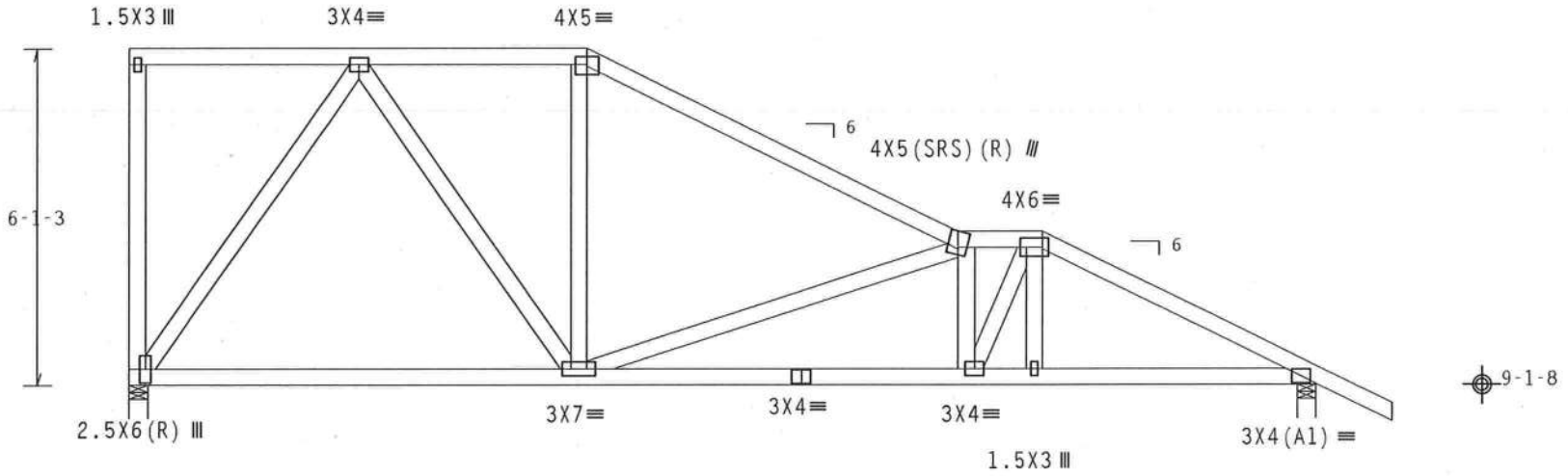
Deflection meets L/240 live and L/180 total load.

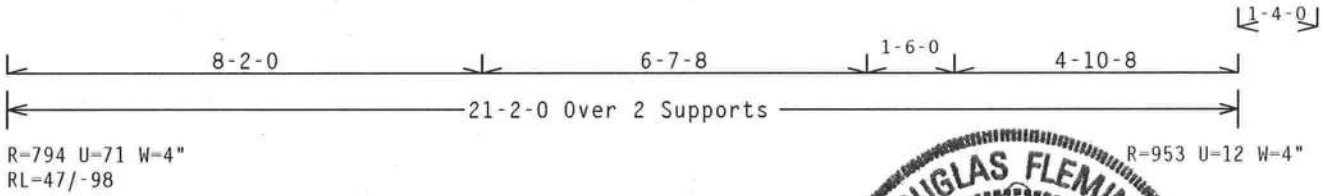
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.04.04.0601.16 QTY: FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 58425
DATE	01/24/12
DRW	HCUSR487 12024038
HC-ENG	TCE/DF *
SEQN-	265169
JREF-	1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HG9C)

Top chord 2x4 SP #1
Bot chord 2x6 SP M-26
Webs 2x4 SP #3 :W7 2x4 SP #2:

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at -1.33 to 62 plf at 9.00
TC- From 62 plf at 9.00 to 62 plf at 21.83
TC- From 62 plf at 21.83 to 62 plf at 27.58
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.06
BC- From 10 plf at 7.06 to 10 plf at 27.58
BC- 3065.36 lb Conc. Load at 7.06
BC- 1362.38 lb Conc. Load at 9.06,11.06
BC- 1304.35 lb Conc. Load at 13.06
BC- 1315.36 lb Conc. Load at 15.06
BC- 1323.39 lb Conc. Load at 17.06
BC- 1324.86 lb Conc. Load at 19.06
BC- 1321.74 lb Conc. Load at 21.06
BC- 1043.44 lb Conc. Load at 23.06,25.06
BC- 1025.92 lb Conc. Load at 25.85

Deflection meets L/240 live and L/180 total load.

2 COMPLETE TRUSSES REQUIRED

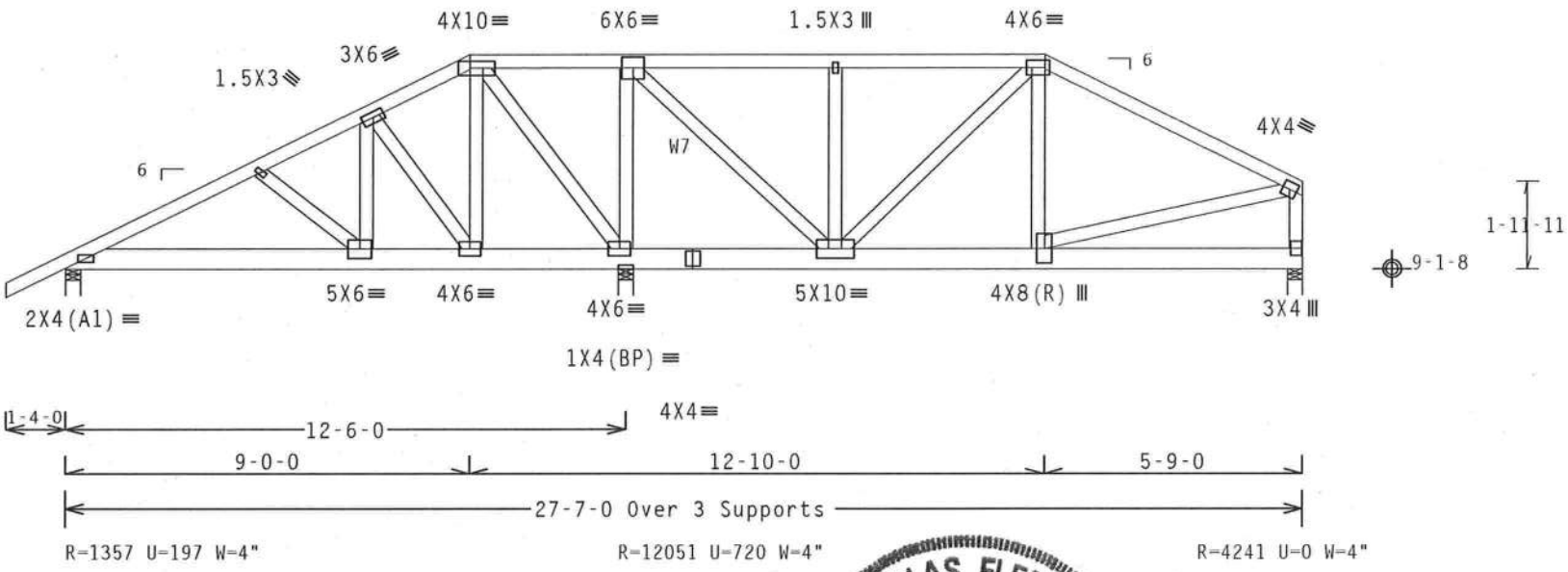
Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58426
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024025
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265446
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H7C)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	62 plf at	-1.33 to	62 plf at	0.00		
TC- From	62 plf at	0.00 to	62 plf at	7.00		
TC- From	128 plf at	7.00 to	128 plf at	23.83		
TC- From	62 plf at	23.83 to	62 plf at	27.58		
BC- From	4 plf at	-1.33 to	4 plf at	0.00		
BC- From	20 plf at	0.00 to	20 plf at	7.00		
BC- From	44 plf at	7.00 to	44 plf at	23.83		
BC- From	20 plf at	23.83 to	20 plf at	27.58		
BC- 498.54 lb Conc. Load at	7.00,23.83					

The Building Designer shall evaluate and approve load magnitudes and locations as shown under "SPECIAL LOADS". Truss Engineer & Fabricator are not responsible for load magnitudes and locations.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

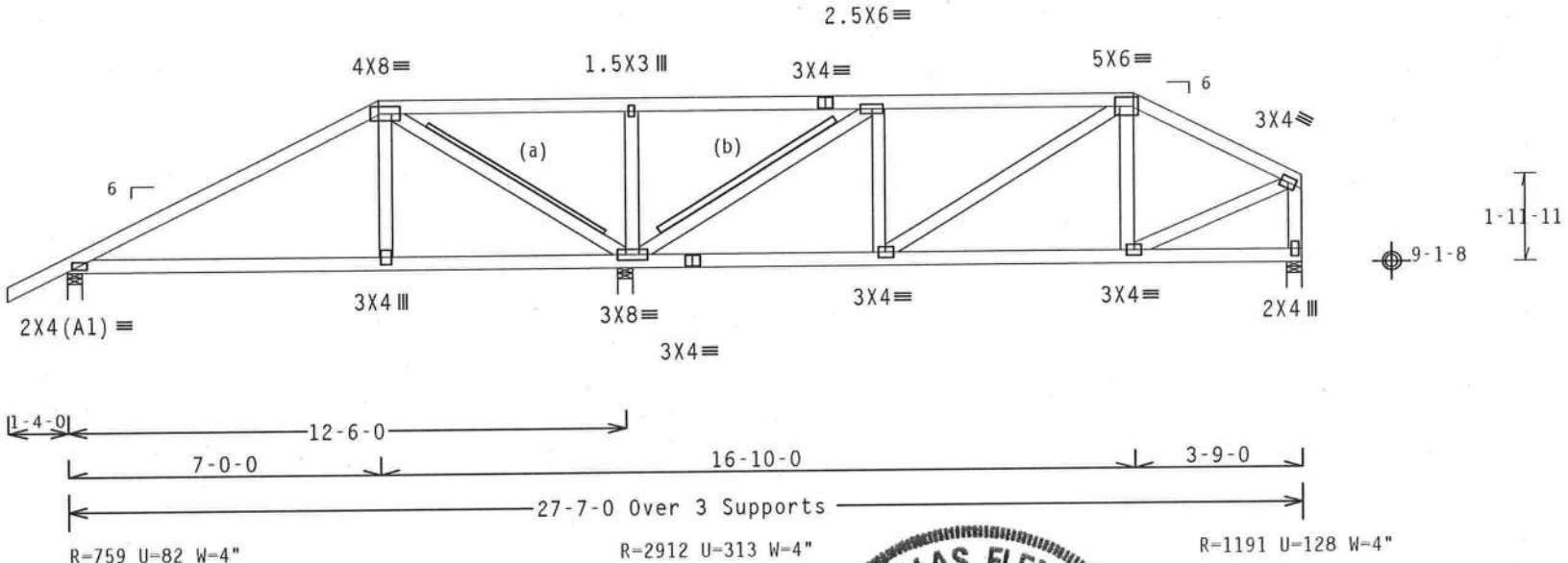
Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R487-- 58427
DATE 01/24/12
DRW HCUSR487 12024037
HC-ENG TCE/DF
SEQN- 265463
JREF- 1UJ0487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - D)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

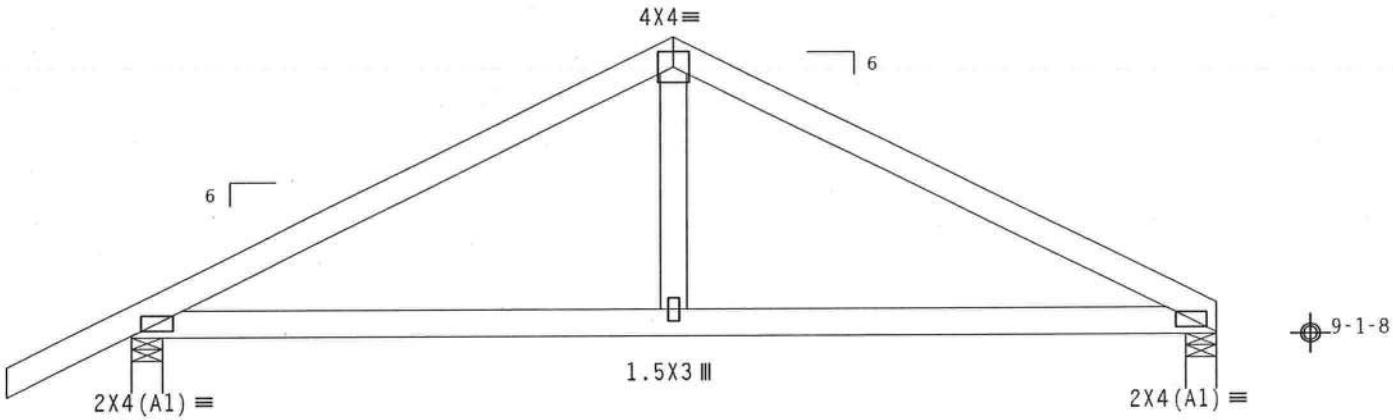
Roof overhang supports 2.00 psf soffit load.

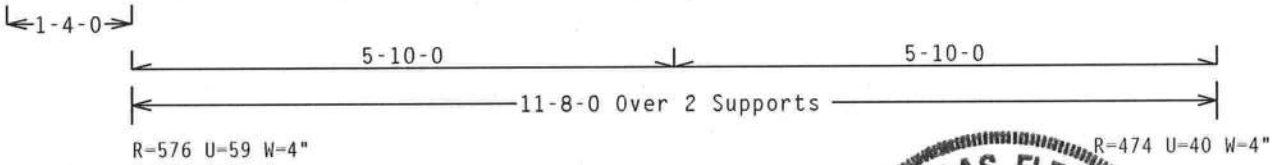
Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 3 FL/-/4/-/-/R/- Scale =.5"/Ft.

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FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF	R487-- 58428
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024011
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	264983
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - DGE)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

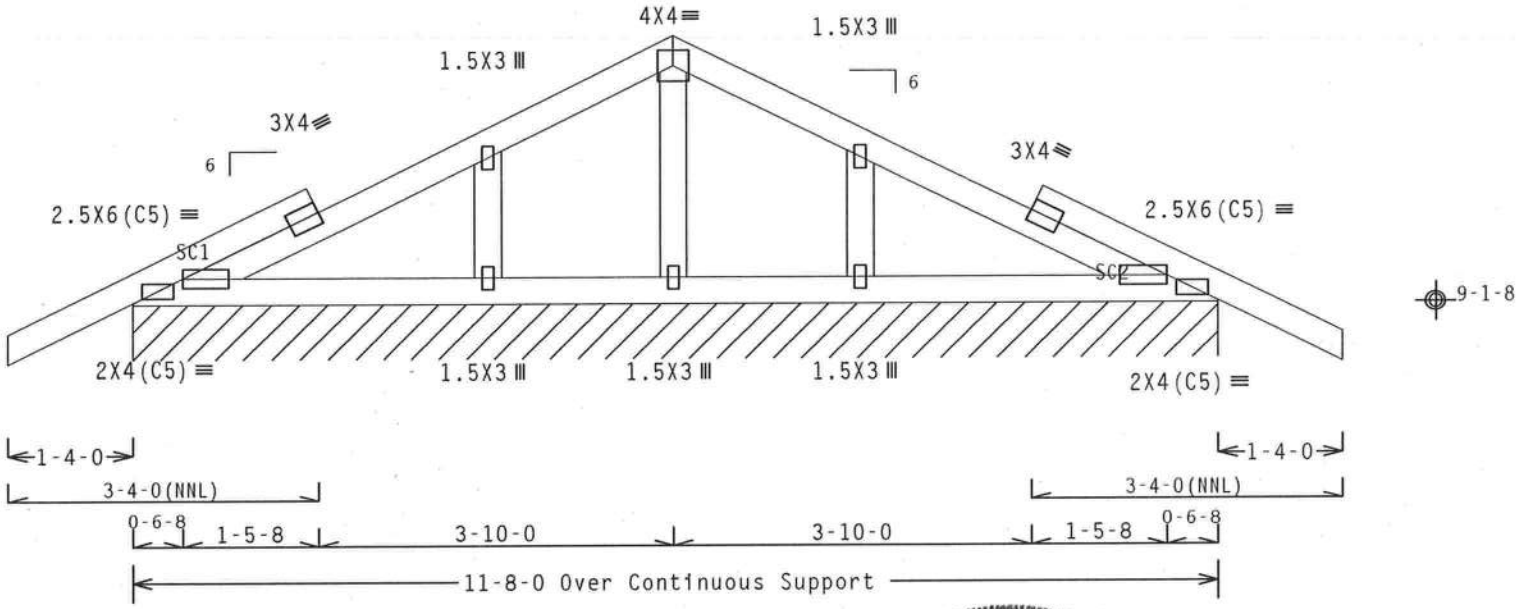
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/180 live and L/120 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R=117 PLT U=9 PLT W=11-8-0
RL=7/-7 PLT

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 58429
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024001
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	264987
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - DG)

Top chord 2x4 SP #1
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at -1.33 to 62 plf at 5.83
TC- From 62 plf at 5.83 to 62 plf at 11.67
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.06
BC- From 10 plf at 7.06 to 10 plf at 11.67
BC- 3065.36 lb Conc. Load at 7.06
BC- 1362.38 lb Conc. Load at 9.06,11.06

Roof overhang supports 2.00 psf soffit load.

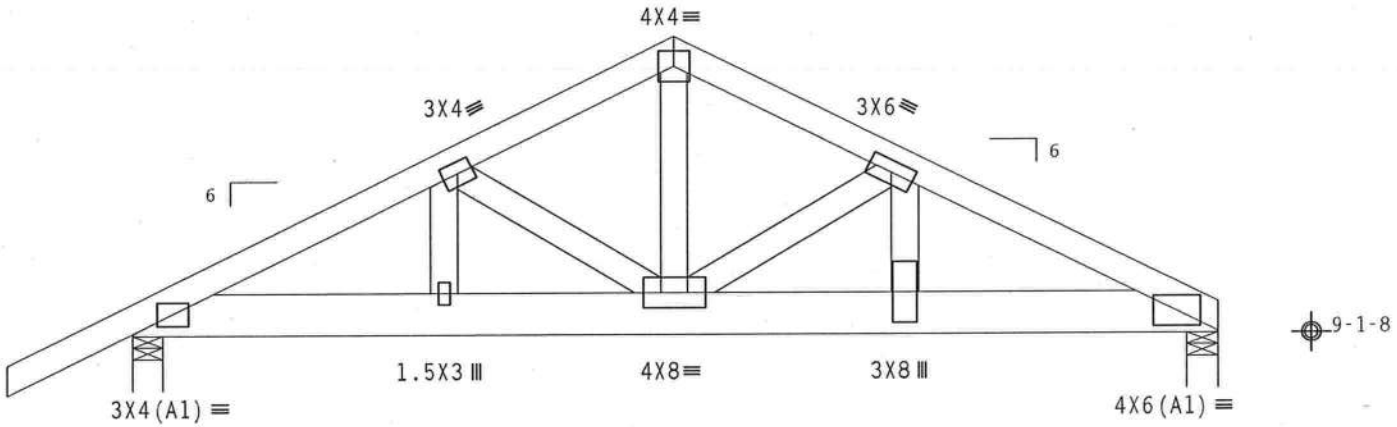
Deflection meets L/240 live and L/180 total load.

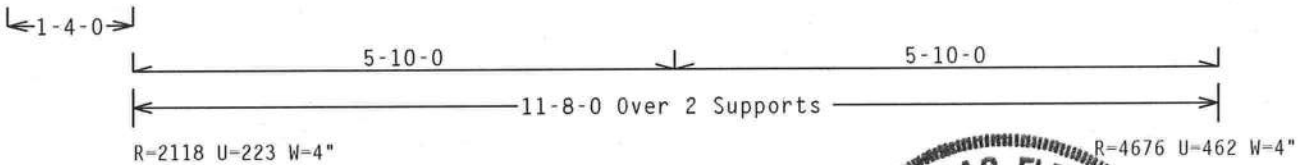
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.





Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.04.0601.16 | QTY 1 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF	R487-- 58430
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024012
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	265160
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - EJ7)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

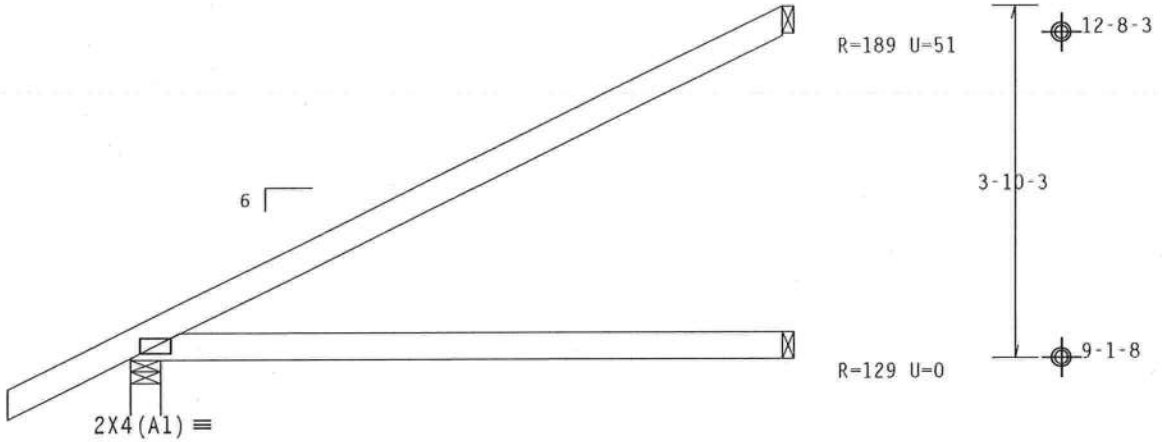
Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



1-4-0

7-0-0 Over 3 Supports

R=394 U=22 W=4"
RL=96/-35

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY:35 FL/-/4/-/-/R/-

Scale =.5"/Ft.

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01/24/2012

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TC LL	20.0 PSF	REF	R487-- 58431
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024026
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	264940
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - CJ5)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

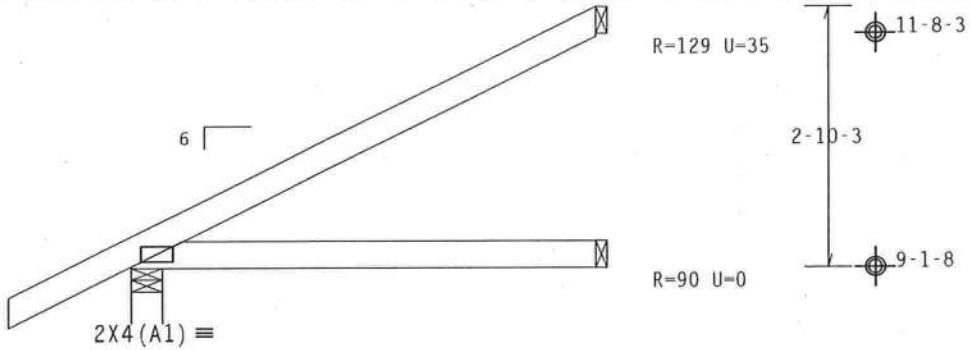
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



R=316 U=21 W=4"
RL=73/-30

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.00.04.0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 58432
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024013
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 264946
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - CJ3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

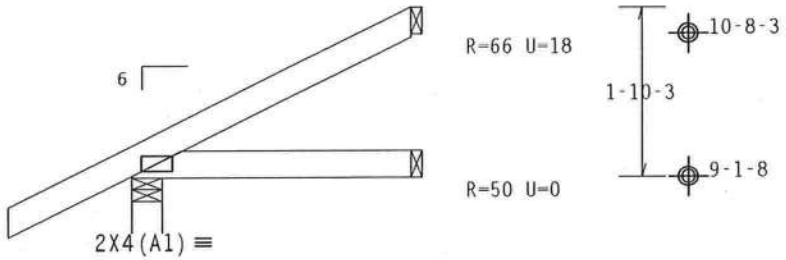
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



R=244 U=22 W=4"
RL=50/-25

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58433
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024017
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 264949
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - CJ1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

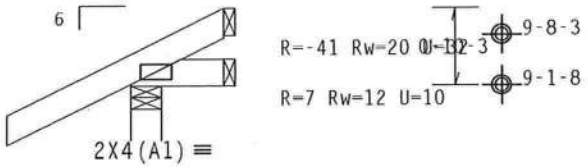
Bottom chord checked for 10.00 psf non-concurrent live load.

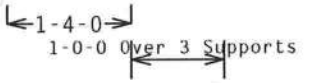
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.





R=222 U=40 W=4"
RL=26

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04/0601.16 QTY:

FL/-/4/-/-/R/-		Scale =.5"/Ft.
TC LL	20.0 PSF	REF R487-- 58434
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024016
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 264952
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HJ7)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

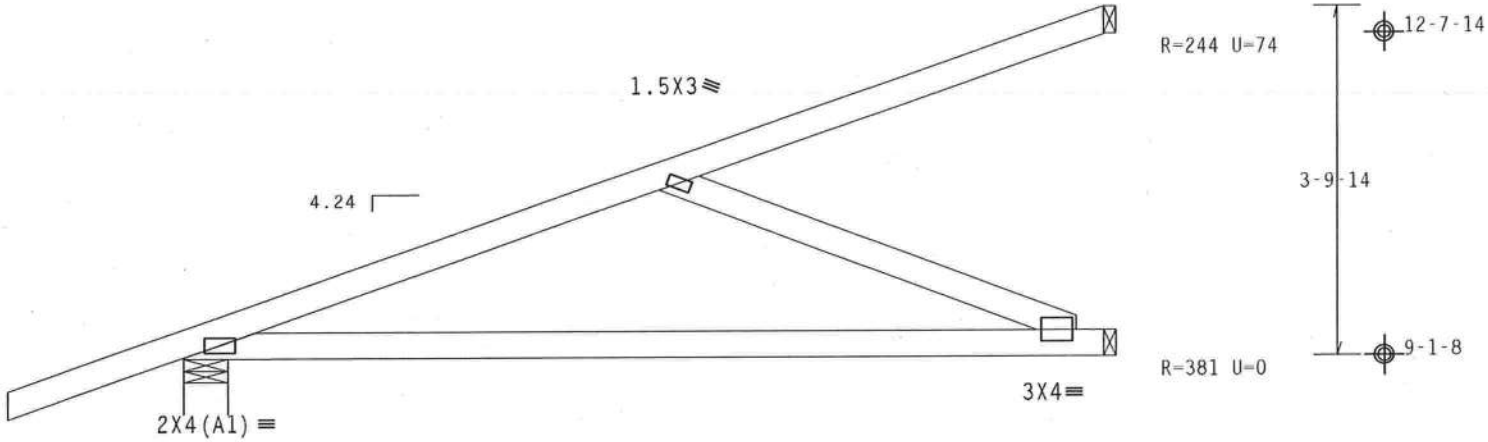
Hipjack supports 7-0-0 setback jacks with no webs.

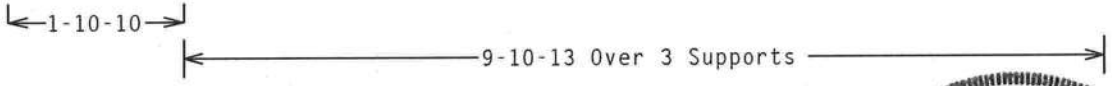
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.





R=439 U=51 W=5.657"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.00.04.0601.16

QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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01/24/2012

TC LL	20.0 PSF	REF R487-- 58435
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024020
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265460
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - HJ3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

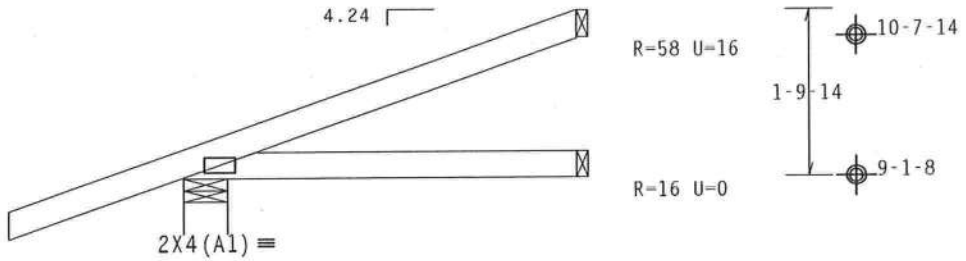
Hipjack supports 3-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0607.16 QTY: 8

FL/-/4/-/-/R/- Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 58436
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024032
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	264974
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - J1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

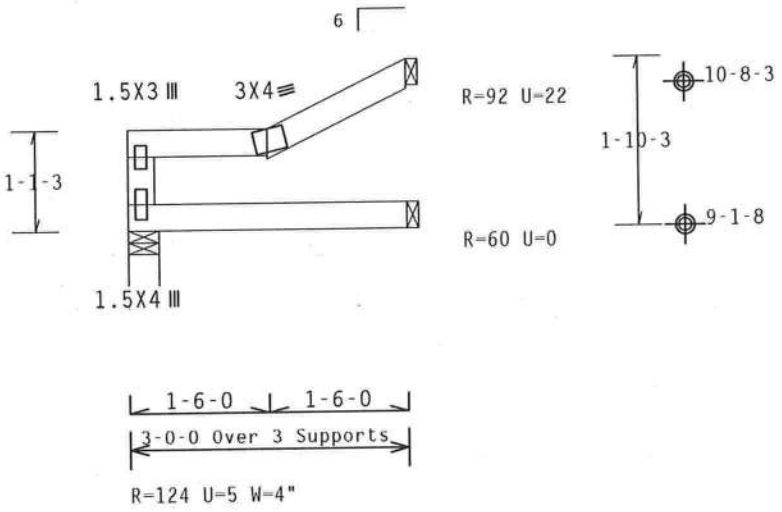
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 58437
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024010
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	264962
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - J2)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

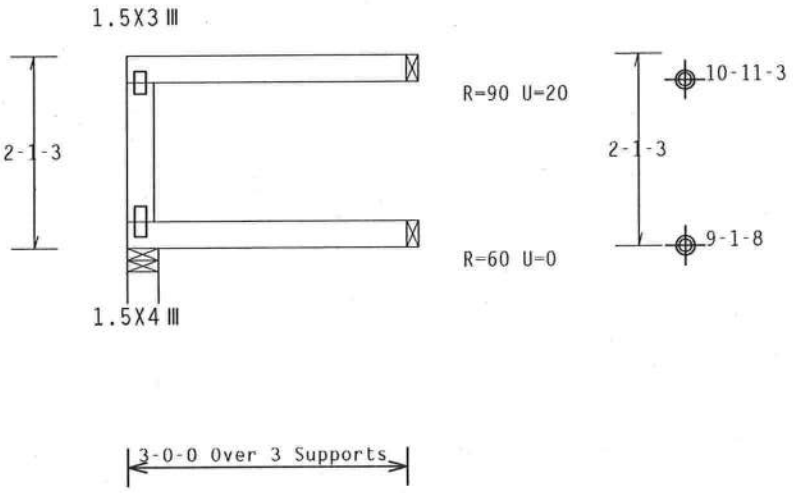
Truss must be installed as shown with top chord up.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.060/1.16 / QTY: 1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
01/24/2012

TC LL	20.0 PSF	REF R487-- 58438
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024035
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 264967
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - J3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

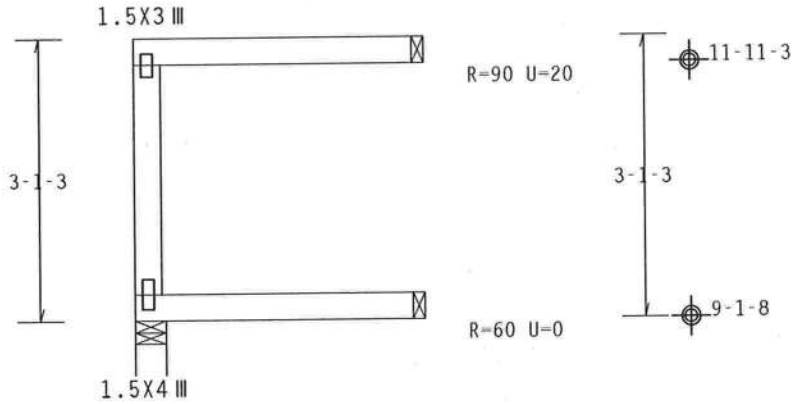
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.00.04 0603.16 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

TC LL	20.0 PSF	REF R487-- 58439
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024030
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 264971
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - EJ7H)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

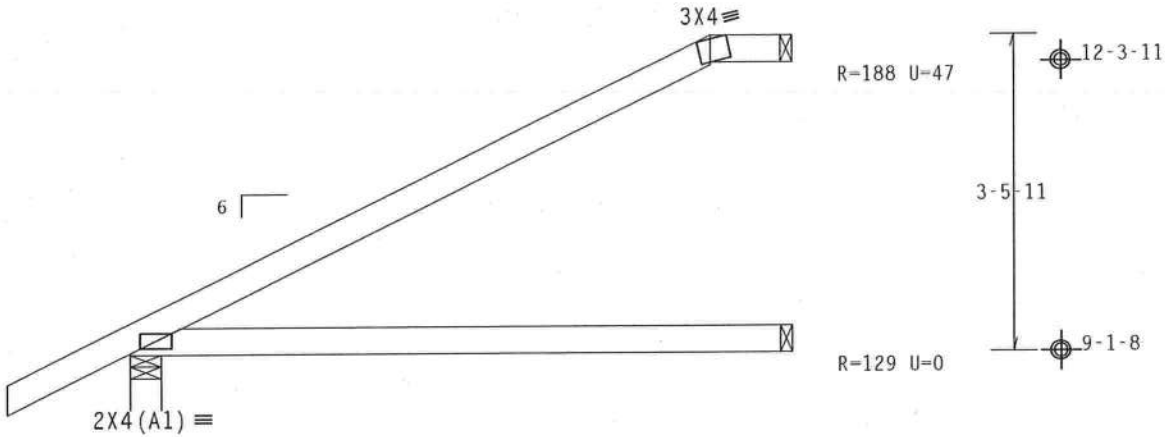
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

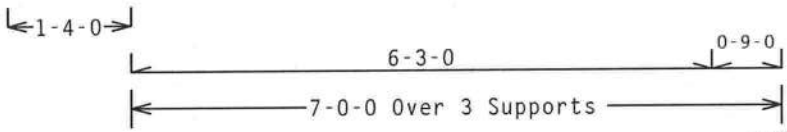
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.





R=394 U=26 W=4"
RL=87/-33

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY:

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

TC LL	20.0 PSF	REF R487-- 58440
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024007
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265058
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UJ0487_Z02

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - CJ5S)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

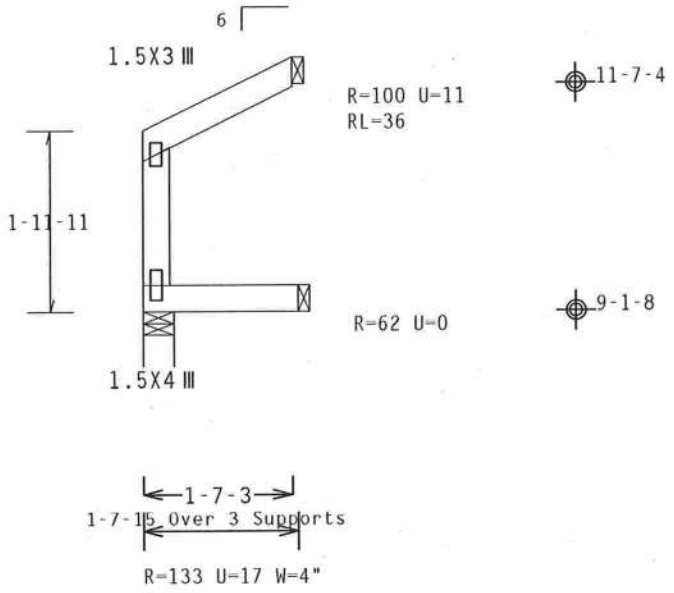
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Trusses to be spaced at 48.0" OC maximum.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

TC LL	20.0 PSF	REF R487-- 58441
TC DL	10.0 PSF	DATE 01/24/12
BC DL	10.0 PSF	DRW HCUSR487 12024004
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 265451
DUR.FAC.	1.25	
SPACING	48.0"	JREF- 1UJ0487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-012--Fill in later BRYAN ZECHER/COLEMAN -- , ** - H3M)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Special loads

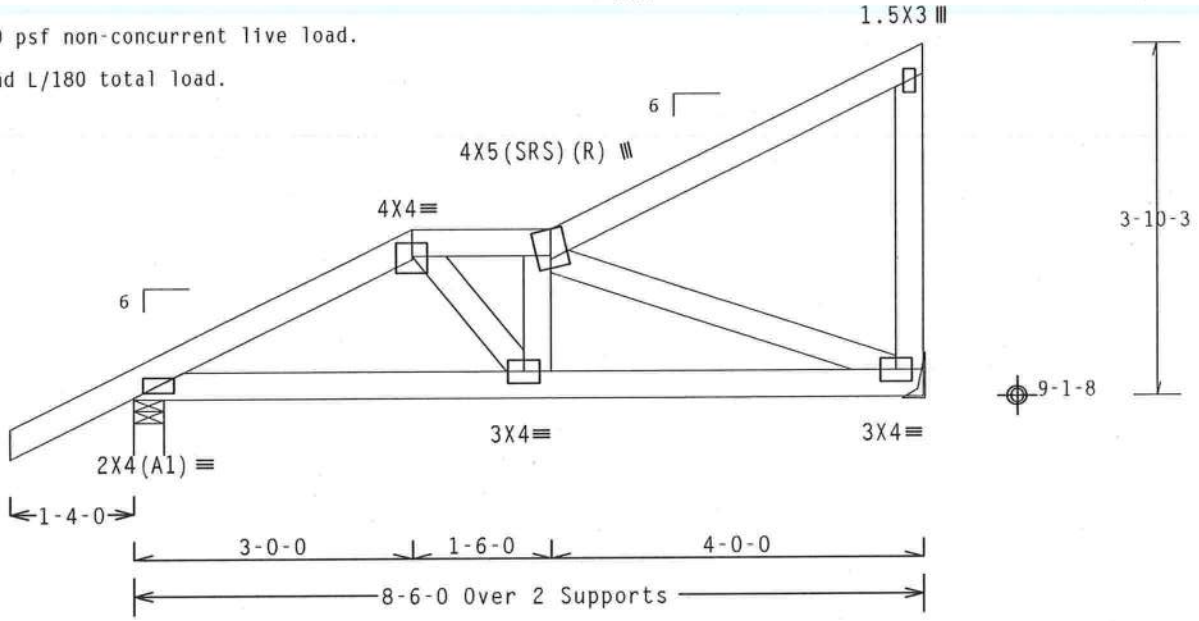
(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC-	From	62 plf	at	-1.33	to	62 plf at 3.00
TC-	From	62 plf	at	3.00	to	62 plf at 4.50
TC-	From	31 plf	at	4.50	to	31 plf at 8.50
BC-	From	4 plf	at	-1.33	to	4 plf at 0.00
BC-	From	20 plf	at	0.00	to	20 plf at 3.05
BC-	From	10 plf	at	3.05	to	10 plf at 8.50

TC- 150.96 lb Conc. Load at 3.05
TC- 90.00 lb Conc. Load at 5.06, 7.06
BC- 76.25 lb Conc. Load at 3.05
BC- 60.00 lb Conc. Load at 5.06, 7.06

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



R=642 U=78 W=4"

R=495 U=43 H=Simpson LU24
Supported Member Face: (2) 0.148"x1.5" nails
Supporting Member Face: (4) 0.162"x3.5" nails
Support Member : (1) 2x6 SP #2

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 58442
TC DL	10.0 PSF	DATE	01/24/12
BC DL	10.0 PSF	DRW	HCUSR487 12024022
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	265051
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UJ0487_Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

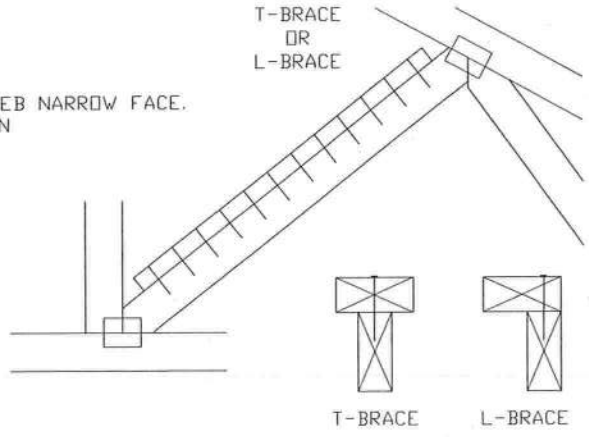
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

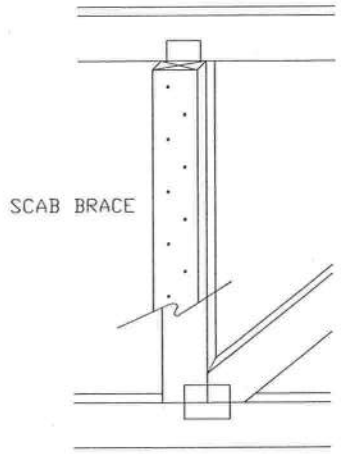
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

01/24/2012

Building Components Group Inc.

Earth City, MO 63045

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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 'L' BRACE *		(1) 2X4 'L' BRACE *		(2) 2X4 'L' BRACE **		(1) 2X6 'L' BRACE *		(2) 2X6 'L' BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

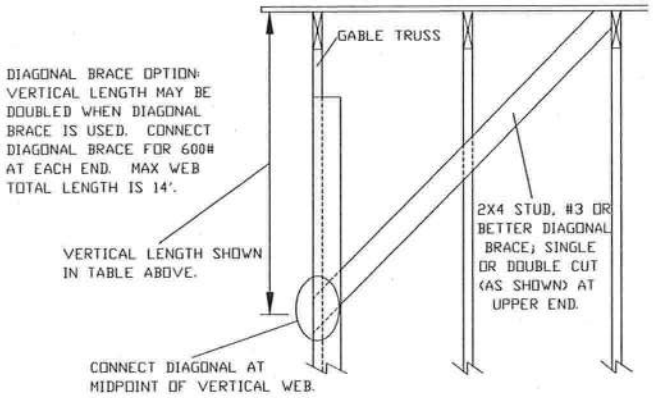
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

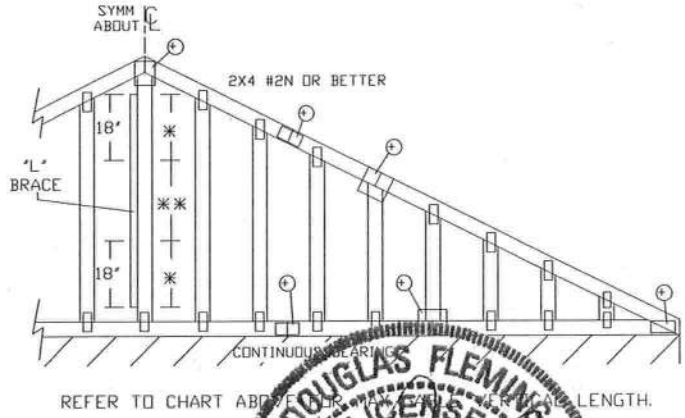
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX. GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH 'L' BRACE WITH 10d NAILS. (0.128"x3" min

* FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 01/24/2012

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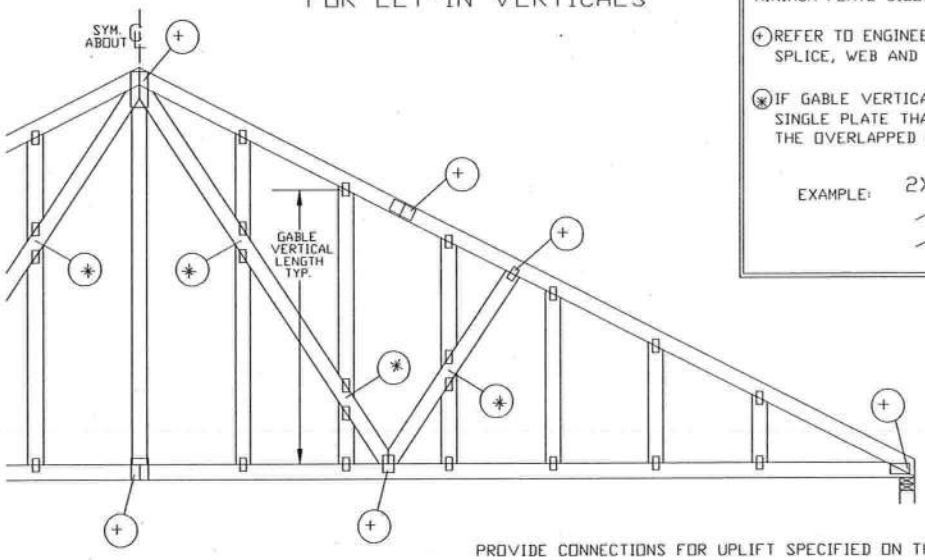
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11015
DATE	1/1/09
DRWG	A11015050109

GABLE DETAIL
FOR LET-IN VERTICALS



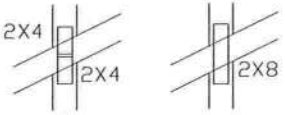
GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

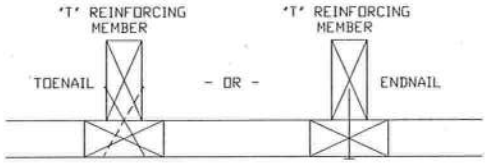
(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

(*) IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



'T' REINFORCEMENT ATTACHMENT DETAIL



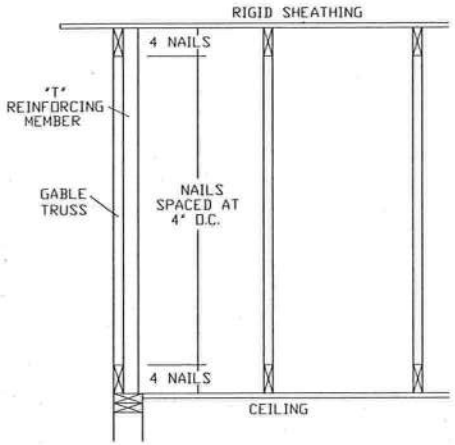
TO CONVERT FROM 'L' TO 'T' REINFORCING MEMBERS, MULTIPLY 'T' INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE 'T' REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	'T' REINF. MBR. SIZE	'T INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
'T' REINFORCING MEMBER SIZE = 2X4
'T' BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 'L' BRACE LENGTH = 6' 7"
MAXIMUM 'T' REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH 'T' REINFORCING MEMBER WITH

END DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) NAILS AT 4" O.C. PLUS
(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:
10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 GABLE DETAIL DRAWINGS
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 GABLE DETAIL DRAWINGS
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

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MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

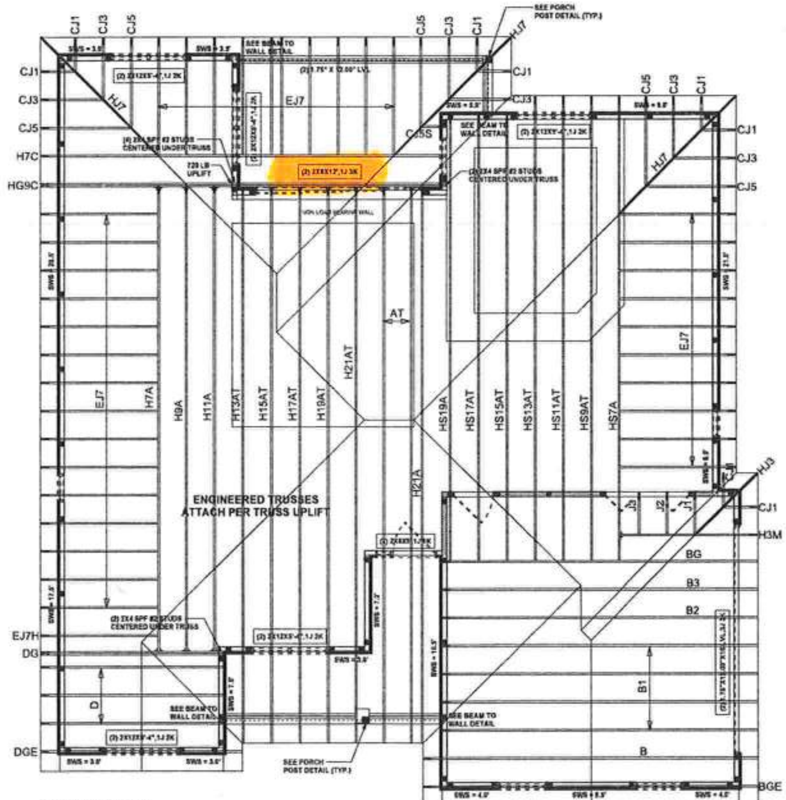
REF LET-IN VERT
DATE 1/1/09
DRWG GBLLETIN0109

Laurie Hodson

From:	Bryan Zecher [bczecher@comcast.net]
Sent:	Thursday, April 12, 2012 10:56 AM
To:	Janice Williams; Laurie Hodson
Subject:	Coleman
Attachments:	S-3.pdf

Please give a copy of the attached to Troy for permit #29935

Thank you, Bryan



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-B3. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

(2) 2X12X8',1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)

- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 5/8" A307 ALL THREADED ROD
- INDICATES LOCATION OF: 2ND FLOOR 5/8" A307 ALL THREADED ROD

TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	48.8	87.0'
LONGITUDINAL	40.0	44.5'

CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB # 12-012

REVISIONS

12Apr12

SOFTPLAN

April 12, 2012

Bryan Zecher Construction

Coleman Residence

ADDRESS: Emerald Cove S/D Phase I Columbia County, FL

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE April 12, 2012

DRAWN BY | STRUCTURAL BY David Disosway

FINALS DATE 3/Jan12

JOB NUMBER: 1201054

DRAWING NUMBER

S-3

OF 3 SHEETS

29935

Lake City • (386) 755-3633
Fax • (386) 752-5456

Jacksonville • (904) 381-8901
Fax • (904) 381-8902

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254

JOB NO.: 12-058
DATE TESTED: 2/29/12

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ (D-2922) Nuclear _____ (D-2937) Drive Cylinder _____ Other

PROJECT: COLEMAN RESIDENCE

CLIENT: BRYAN ZOCKOR CONST.

GENERAL CONTRACTOR: _____ EARTHWORK CONTRACTOR: _____

SOIL USE (SEE NOTE): 1 SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: BILL S.

MODIFIED (ASTM D-1557): ✓ STANDARD (ASTM D-698): _____

TEST NO.	TEST LOCATION	TEST: ___DEPTH ___ELEV. ___LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
1	10' N.W OF S.E CRNR of PAD	12"	1	113.1	105.6	7.1	95
2	CNTR OF PAD	↓	↓	116.0	107.1	8.4	97
3	15' S.E. OF N.W CRNR of PAD	↓	↓	115.8	107.3	7.9	97

REMARKS: ALL TEST PASSING

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
1		111	12

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Bayard Dr.
City Lake City Phone 752-1703

Site Location: Subdivision Emerald Cove Bryan Zecher
Lot # _____ Block# _____ Permit # 29935
Address 158 SW Woodleaf Ct.

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐		

Type treatment: ☑ Soil

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2320	251	193 gals

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

3-5-12	8:55	F799
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

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