ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TXN8228Z0216112519

Truss Fabricator: Anderson Truss Company

Job Identification: 9-243--Fill in later DOUG EDGLEY -- , **

Truss Count: 26

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A1101505-GBLLETIN-PB120-A1103005-VAL130-

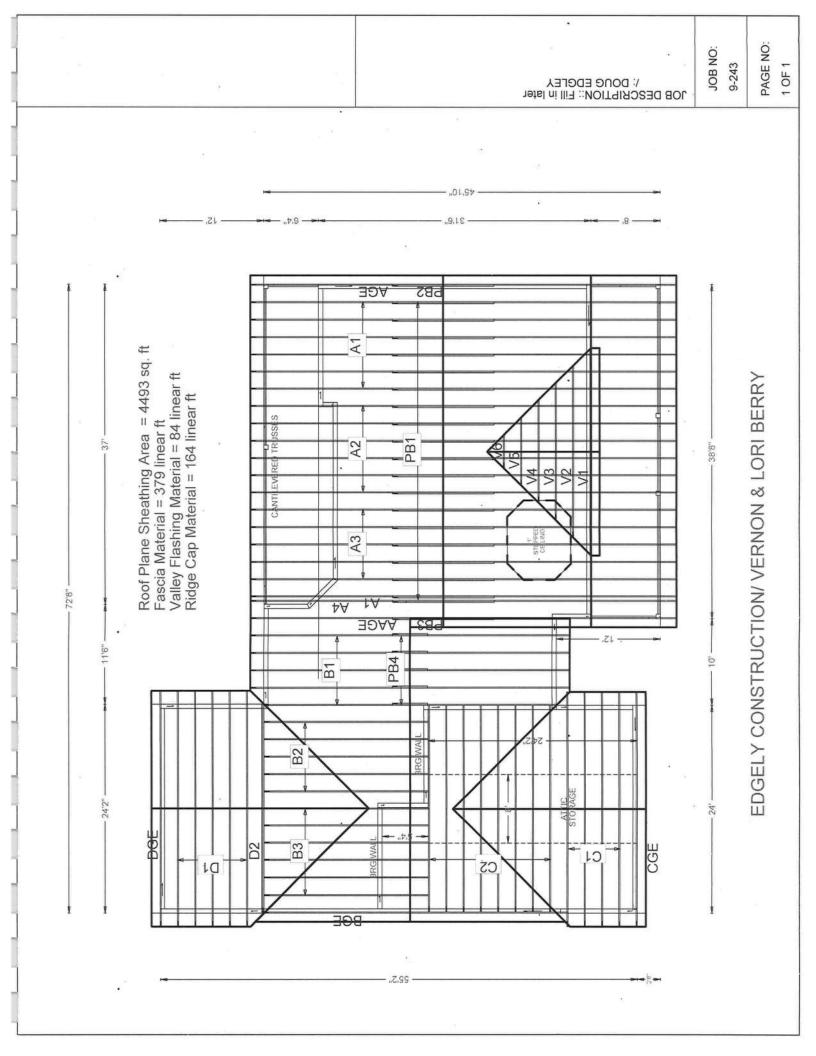
#	Ref Description	Drawing#	Date
1	48875 D1	09350002	12/16/09
2	48876A1	09350008	12/16/09
3	48877 A2	09350009	12/16/09
4	48878 A4	09350010	12/16/09
5	48879AGE	09350020	12/16/09
6	48880 AAGE	09350021	12/16/09
7	48881 A3	09350011	12/16/09
8	48882B1	09350003	12/16/09
9	48883 BGE	09350022	12/16/09
10	48884B3	09350012	12/16/09
11	48885 B2	09350013	12/16/09
12	48886 C1	09350014	12/16/09
13	48887C2	09350015	12/16/09
14	48888 CGE	09350023	12/16/09
15	48889DGE	09350024	12/16/09
16	48890 D2	09350025	12/16/09
17	48891PB1	09350016	12/16/09
18	48892 PB4	09350017	12/16/09
19	48893 PB3	09350026	12/16/09
20	48894 PB2	09350027	12/16/09
21	48895 V2	09350004	12/16/09
22	48896V3	09350005	12/16/09
23	48897 V4	09350006	12/16/09
24	48898V5	09350007	12/16/09
25	48899 V6	09350018	12/16/09
26	48900 V1	09350019	12/16/09

1)+

Seal Date: 12/16/2009

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844





Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

 $\widehat{\geq}$ Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

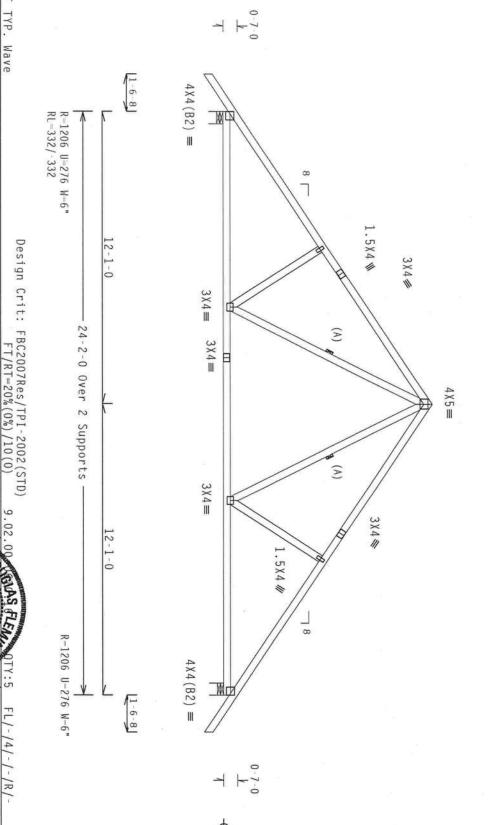
Wind reactions based on MWFRS pressures

100

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

Bottom chord checked for 10.00 psf non-concurrent live load



REFER TO BOST (BULLDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 2DB NORTH LEE STREET, SUITE 312, ATEXANDRA, VA. 22314) AND NICA (AROOD TRUSS COUNCIL OF AMERICA, 65000 ENTERDENSES LANE, MADISON, NI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED ATTACHED ATTACHED ATTACHED ATTACHED. OUTRE EXTREME CARE IN FABRICATION, HUNG COMPONENT SAFETY INFORMATION), PUR 312, ALEXANDRIA, VA. 22314) AND WICA , HANDLING, SHIPPING, INSTALLING AND BRACING, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 PLOCA (HOOD TRUSS COUNCIL OF AMERICA, 6300

IMPORTANISIBMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. THE, SHALL NOT BE RESPONSIBLE FOR ANY EXPLAINANCE HIS DESIGN. ANY FALLURE TO BUILD THE TRUSS IN COMPORMACE WITH PRISOR TABRICATING, MANDLING, SHIPPING, HESTALLING & BRACING OF TRUSSES.

RESIGN COMPORES WITH APPLICABLE PROVISIONS OF MOS (MATERIAL DESIGN ESPEC, BY AFAPA) AND FPI. CONTROL PROVISIONS OF MOS (MATERIAL DESIGN ESPEC, BY AFAPA) AND FPI. CONTROL PROVISIONS OF MOS (MATERIAL DESIGN ESPEC, BY AFAPA) AND FPI. PLATES TO EACH FACE OF TRUSS AND, ANY INSPECTION OF PLATES FOLLOWED ANING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPI OF IPI1-2002 SEC.3. A SEAL ON THIS SHSTBILLITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE SPEE, BY AFAPA) AND TP1.

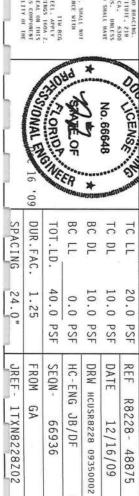
1TW BCG
40/50 (W. K/H.SS) GALY. STEEL, APPLY
DESIGN, POSITION PER DRAWINGS 160A 2.

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL Co. 19 278

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



1TXN8228Z02

66936

Scale = .25"/Ft.

12/16/09

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

(A) Continuous lateral bracing equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

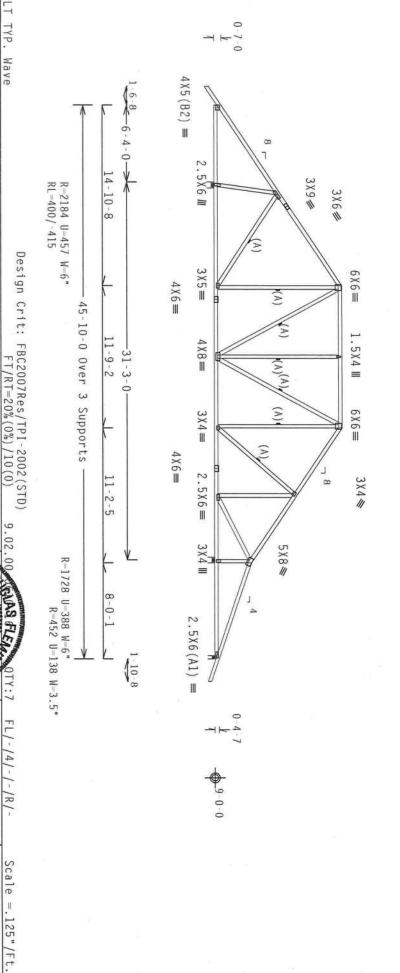
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping



WARNING IRUSSIS REDUIRE EXTREME CARE IN FARMICATION, MANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PHRELISION BY PT (FRUSS PLATE INSTITUTE, 213 MODIN LEE SIREE, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODID TRUSS COUNCIL OF AMERICA, 6300 ETHIERPELSE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OPHERMYS INDICATED OF CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE

IMPORTANTCURNISH A COPY OF THIS RESIGN TO THE INSTALLATION CONTRACTOR. THE MCG. HE. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS RESIGN. ANY FAILURE TO BUILD THE FRUSS IN COMPORMACE WITH PICE OR EARLIAND THE APPLICABLE PROVISIONS OF THIS REACHED OF TRUSSES.

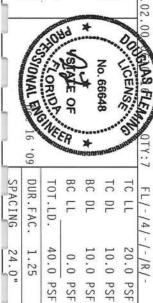
DESIGN COMPORES WITH APPLICABLE PROVISIONS OF THIS CALIDARY DESIGN SPEC. BY AFRAYA AND TPI. THE BCC CONNECTOR PLAITS ARE MODE TO 20/18/16/04 (M.1M/SS), ASTA MCS SIGNAE 40/60 (M. X/M/SS) GAVE. STREEL, APPLY PLAITS TO EACH FACE OF TRUSS AND, DURESS OTHER/MS COCATED ON THIS DESIGN, POSITION FEE BRANCHES 160A-Z.

ANY INSPECTION OF PLAITS FORLOWED BY (1) SHALL BE FEE AMERY AS OF TPI1-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN, THE SULVABILITY AND USE OF THIS COMPONENT DESIGN SHOWN. THE SULVABILITY AND USE OF THIS COMPONENT DESIGN SHOWN THE SULVABILITY AND USE OF THIS COMPONENT DESIGN SHOWN. THE SULVABILITY AND USE OF THIS COMPONENT DESIGN SHOWN THE SULVABILITY AND USE OF THIS COMPONENT DESIGN SHOWN.

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL C 278 278



PSF PSF

SEQN-

67120

HC-ENG JB/DF

DRW HCUSR8228 09350008

JREF -FROM

1TXN8228Z02

DATE REF

12/16/09 48876

R8228-

Top chord chord 2x4 SP chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42° -high x 24° -wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

edge. MWFRS loads based on trusses located at least 15.00 ft. from roof

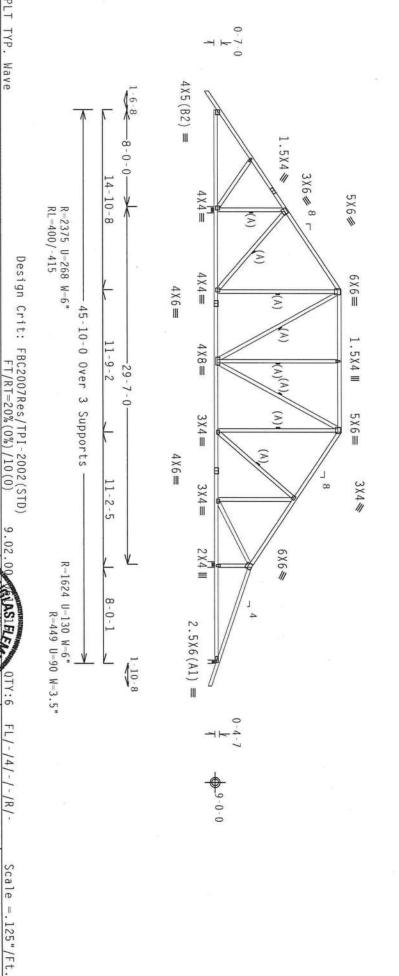
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18

Wind reactions based on MWFRS pressures

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load

contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. WARNING: Furnish a copy of this DWG to the installation shipping



WARNING IRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST. QUILIDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CHROSS PLATE INSTITUTE, ZUB MORIH LEE SIREE, SUITE 317, ALEXANDRIA, VA, Z2314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 EMPREPASE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP COMOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE

IMPORTANT*UBBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG. HEC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARBLISHING, MANUFACH, INSTALLING & BRACHEG OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY AFAPA) AND TPI. THE RCG CONFORMS HITH APPLICABLE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY AFAPA) AND TPI. THE RCG CONFORMS HITH APPLICABLE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY AFAPA) AND TPI. THE RCG CONFORMS HITH APPLICABLE OF EACH SPEC. A CONFORM HIS SECONDAL OF THE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY AFAPA) AND TRUSS CONFORMS HITH APPLICABLE OF FORCE OF THE SECONDAL DESIGN SPECAL OF THE TRUSS OF THE PROVISION OF THE PROVI y/166A (H-1/25/F) ASIH A653 GRADE 60/60 (H, KJH.S5) GALY, STELL, APPLY DIMESS OHNERALSE COCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. BOY (1) SHALL BE FER AMPLY A3 OF 1P11-2002 SEC.3.

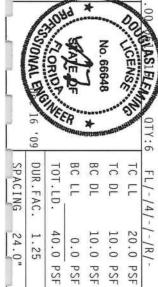
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DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI I SEC. 2. OF 1911-2002 SEC.3. A SEAL ON THIS DWS181LITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 7 7 278



FROM SEQN-

JREF -

1TXN8228Z02

DATE REF

12/16/09

R8228- 48877

HC-ENG

JB/DF 67105

DRW HCUSR8228 09350009

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load

(2) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live in areas with 42"-high x 24"-wide clearance. load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

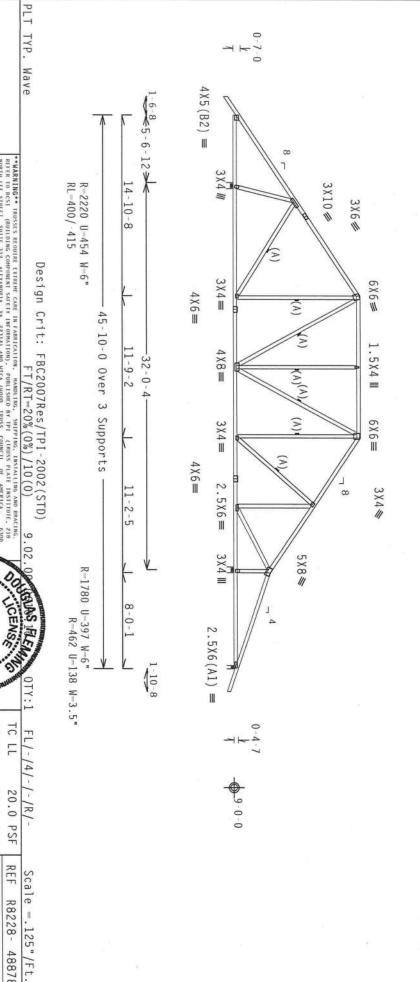
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



REFER TO BOST (GUILDING COMPONENT SAFETY INFORMATION), HANDLING, SWIPPING, INSTALLING AND BRACING.

WORTH LEE STREET, SUITE 312. ALEXANDRAL, VA, 22314) AND WICE (AGOOD TRUSS COUNCIL OF AMERICA, 6300
CHTERPETS LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMENT OF USE FUNCTIONS. UNLESS
OTHERPIST LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMENT OFFICE FUNCTIONS. UNLESS
OTHERPIST LOUGH TO THE CORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGIOD CELLING.

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALIDRE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARBICATING, INSHALLING, INSTALLING A BRACING OF BUSSESS.

BESTOR COMPORES WITH A APPLICABLE PROVISIONS OF MOS (MALTONAL DESIGN SPEC, BY ATAPA) AND TRI. ITW BCG CONNECTION FLAIRS, ARE MADE OF 20/18/15/08 OF MOS (MALTONAL DESIGN SPEC, BY ATAPA) AND TRI. APPLY PRAIRES (OR MALTES, ARE MADE OF 20/18/15/08 OF MOS (MALTONAL DESIGN SPEC, BY ATAPA) AND TRI. APPLY PRAIRES (OR MADE OF 20/18/15/08 OF MOS (MALTONAL DESIGN SPEC, BY ATAPA) AND TRI. THE MOST CONNECTION FLAIRS ARE MADE OF 20/18/15/08 OF MOST CONNECTION FLOW FROM THE MADE OF THE MADE OF

BUILDING DESIGNER PER ANSI/IPI I SEC. DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY AFAPA) AND IP.

CONRECTOR PLATES ARE MADE OF 20/18/1660 (M. M/SSZ) ASTH MASS GRADE 40/60 (M. K/MLSS) GALV. STEEL. APPLY OF PLATES OF REMAINS STORAGE MATERIAL FOR THE MATERIAL FOR THE MASS AND. MISTER LOCATED ON THIS DESIGN. POSITION FOR DOMAINGS 1606.2 AND HISSES OF REMAINS 106.4 OF THIS DESIGN OF THE TROPE SEC.3.

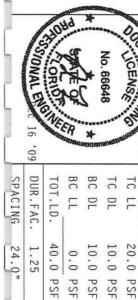
AND INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF THIS 2002 SEC.3.

A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFIESTIONAL CHORUCENIG RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 7 278



PSF PSF

HC-ENG JB/DF

67502

DRW HCUSR8228 09350010

DATE

12/16/09

REF

R8228 - 48878

JREF -

1TXN8228Z02

FROM SEQN-

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 #2 Dense #2 Dense

2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load

must not be cut or notched. Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top ch Top chord

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR DESIGNED BY THE BUILDING DESIGNER. SUPPORTING SHEAR WALLS. LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE SHEAR WALLS MUST PROVIDE CONTINUOUS DESIGN WALLS, AND

> (**) 6 plate(s) require special positioning. Refer plot details for special positioning requirements. to scaled plate

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

See DWGS Al1015050109 & GBLLETIN0109 for more requirements.

chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice chord in notchable area using 3x6. Dropped Stacked top chord must NOT be notched or cut in area (NNL). top chord braced at 24" o.c. intervals. Attach stacked dol top

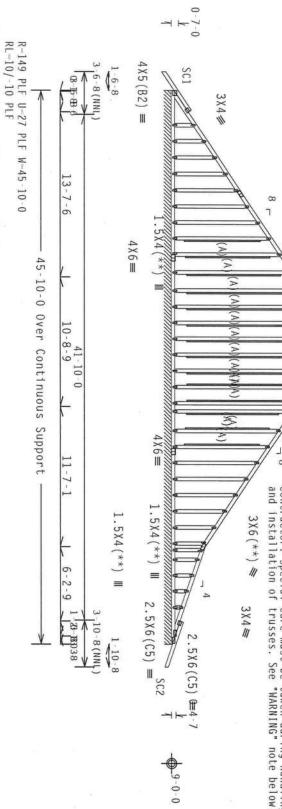
(A) 1×4 #3SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

Bottom chord checked for 10.00 psf non-concurrent live

1.5X4(**) 4X6(**) ≥

contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping



Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

PLT TYP.

Wave

PROPERLY ATTACHED RIGID CEILING. FT/RT=20%(0%)/10(0) 9

FBC2007Res/TPI-2002(STD)

ITW Building Components Group Haines City, FL 33844 FL Co. "9 278

ALPINE

DRANTHS INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE FRUSS COMPONENT FOR SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER FER ANSIJED I SEC. 2.



24.0" JREF -FROM 1TXN8228Z02

40.0

PSF

SEQN-

HC-ENG

JB/DF 67180

20.0 PSF

Scale =.125"/Ft.

R8228-

10.0 PSF 10.0 PSF

DRW HCUSR8228 09350020

DATE

12/16/09 48879

Bot chord op chord

:Stack chord 2x4 SP chord 2x4 SP Webs 2x4 SP ck Chord SCI #2 Dense #2 Dense #3 :W10 2x4 SP #2 Dense: 2x4 SP #2 Dense::Lt Slider 2x4 SP #3: BLOCK LENGTH = 2.000

Roof overhang supports 2.00 psf soffit load

See DWGS All015050109 & GBLLETIN0109 for more requirements

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" 0C.

In lieu of structural panels or rigid ceiling use purlins to brace 0C, BC @ 24" 0C. 0 01

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

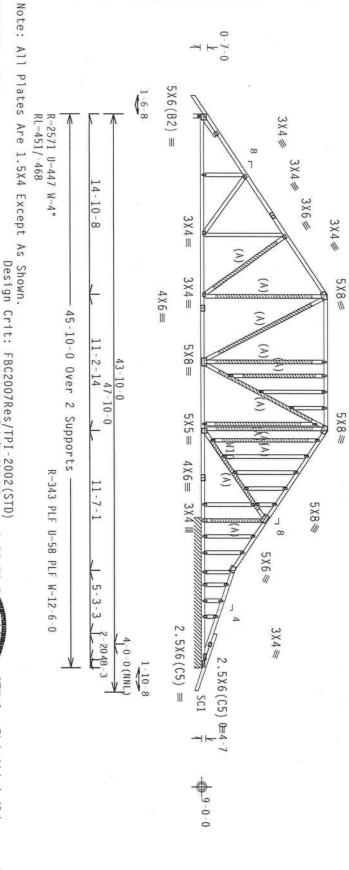
Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

chord in notchable area using 3x6. Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top

areas Truss passed check for 20 psf additional bottom chord live load areas with $42\mbox{\ensuremath{^{\prime\prime}}}\mbox{-high}$ x $24\mbox{\ensuremath{^{\prime\prime}}}\mbox{-wide clearance.}$ in

Bottom chord checked for 10.00 psf non-concurrent live

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. Wave /RT=20%(0%)/10(0)

0

Scal

|e| = .125"/Ft.

*****MARNING*** IRUSSIS BEQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
RELEE TO RESI (BUILDING COMPONENT SERTY) HEFORMATION), PRINCIPED BY TPI (TRUSS PLAIR INSTITUTE, 218
NORTH LEE SIREET, SUITE 312: ALEXANDRIA, VA. 22314) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 60.00
RINERPRISE LOLGCLIED (OF CORROS SMALL MANE PROPERTY ATTACHED SHOCKLONAL PARIES AND BOTTON CHORD SMALL HAVE
STREAMS INDICALED (OF CORROS SMALL MANE PROPERTY ATTACHED SHOCKLONAL PARIES AND BOTTON CHORD SMALL HAVE PROPERLY ATTACHED RIGID CEILING

IMPORTANT*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE FOR BHILD THE TRUSS IN COMPORMANCE WITH PI; OR FARREACTING, MANDLING, SHEPPING, HISTALLING A BRACHING OF TRUSSES, MANDA AND TEL.

THE SCI DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DUS (MATIONAL DESIGN SPEC, S. MANDA AND TEL.

THE SCI COMMERCION PLATES ARE HADE OF 20/18/1606, (PAUSSEX) ASTH ASAS DRADE 40/60 (M. K/M.SS) GALV. STEEL., APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAHINGS 160A-Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PETI-2000 SEC. 3.

A SCAN ON THIS DESIGNATION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PETI-2000 SEC. 3.

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ITW Building Components Group

Inc.

ALPINE

Haines City, FL 33844 FL F 278

02 OO ICENS STONAL BUSINES CENSE No. 66648 BC LL BC DL SPACING DUR.FAC. TC DL C TOT.LD. FL/-/4/-20.0

40.0 24.0" 1.25 10.0 10.0 PSF 0.0 PSF PSF PSF PSF JRFF-FROM SEQN-DATE DRW REF HC-ENG HCUSR8228 09350021 R8228-1TXN8228Z02 JB/DF 67256 12/16/09 48880

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

(A) Continuous lateral bracing equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

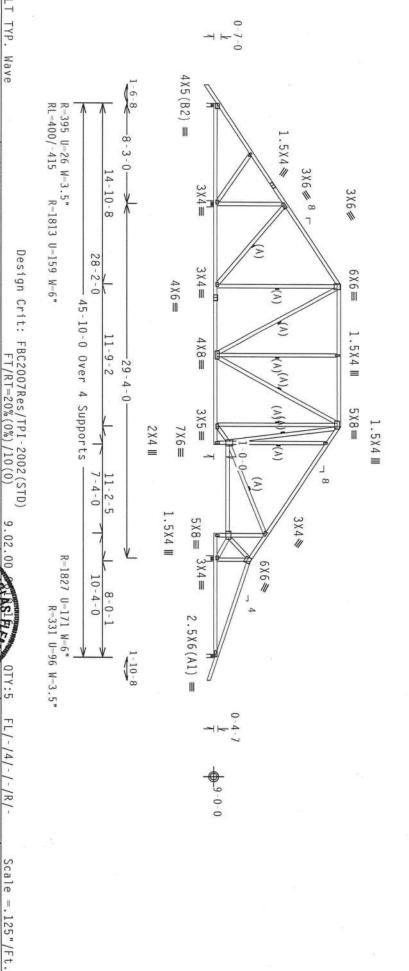
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C. to

Bottom chord checked for 10.00 psf non-concurrent live load

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



REFER TO BCS1 (BUILDING COMPONEN MORNIN LEE SIREIT, SUITE 332, ALEXAS ENTERPRISE LANE, MADISON, WI 537 OTHERWISE INDICATED TOP CHORD SHALL A PROPERLY ATTACHED HIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BGG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP: OR FARELATING. HANDLIG, SHEPPING., INSTALLIG A BRACING OF TRUSSES.

DESIGN CONFOCRAS WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY MERCH) AND TPI.

DESIGN SPECIAL ARE MADE OF 20/18/16GA (M.,U/SS/)). ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. SITEL, APPLY PLATES TO LACH FACE OF TRUSS AND. UNLESS OF UNDERSIES LOCATED ON THIS DESIGN. POSITION FER BRACHMOS 16GA-Z.

ANY INSPECTION OF FLATES FOLURED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.

ANY INSPECTION OF PLATES FOLURED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.

ANY INSPECTION OF PLATES FOLURED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.

PROPRIED THE RESPONSIBILITY OF THE

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 272 '0 278

BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

COUNTY STELL COSTONAL BUSINES CENS .09 BC LL BC DL TC DL TC LL SPACING DUR.FAC TOT.LD. 40.0 24.0" 1.25 20.0 PSF 10.0 PSF 10.0 PSF 0.0

PSF PSF

HC-ENG

JB/DF

67148

DRW HCUSR8228 09350011

DATE REF

12/16/09

R8228-

48881

JREF -FROM SEON-

1TXN8228Z02

Note: Top chord Bot chord ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL TC @ Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Bottom chord checked for 10.00 psf non-concurrent live load CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER (A) 1x4 #3SRB SPF-S or Attach with 8d Box or THE BUILDING DESIGNER IS RESPONSIBLE FOR THE :Stack Chord SC1 (9-243--Fill in later TW Building Components Group Inc. lieu of structural @ 24" OC, BC @ 24" TYP. Wave Haines City, FL 33844 FL 278 '0 278 0-7-0 chord 2x4 SP chord 2x4 SP Webs 2x4 SP All Plates Are 1.5X4 Except As Shown. R-142 PLF U-23 RL-13/-13 PLF ALPINE 4X5 (B2) ≡ - JNN) 0 - d 210-0 #2 Dense #2 Dense #3 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense: 3X4 / PLF panels or rigid ceiling use purlins to brace OC. better "L" brace. 80% length of web member Gun (0.113"x2.5", min.) nails @ 6" 0C. DOUG EDGLEY **IMPORTANT***TURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEPLATION FROM THIS DESIGN ANY FAILURE TO BUILD THE BRUSS IN COMPORMACE WITH IP): OR FAREACATHG, HANGING, SITEPING, INSTALLING A BRACING OF RUSSES.

BESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY ATAPA) AND IP). SEEL, APPLY CONFECTION PAIRS ARE ADDITIONAL OF THE DESIGN FOR SHALL SHALL APPLY AND THE APPLY CONFECTION OF PLAIRS AND JUNESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS BOAN Z. ANY INSPECTION OF PLAIRS FOLLOWED BY (I) SHALL BE FER ANNEX AS OF IP11-2002 SEC.3. A SEAL ON THIS APPLY ANY INSPECTION OF PLAIRS FOLLOWED BY (I) SHALL BE FER ANNEX AS OF IP11-2002 SEC.3. A SEAL ON THIS ANY INSPECTION OF PLAIRS FOLLOWED BY (I) SHALL BE FER ANNEX AS OF IP11-2002 SEC.3. **WARNING** TRUSSES REQUIRE LITERE CARE IN FARRICATION, HANDLING, SHIPPING, HISTALLING AND REACHIG. RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), BULLSHED BY PIT (TRUSS PLAIT INSTITUTE, 21) HORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND HICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ERIFERDASS LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERCHHUG THEST FUNCTIONS. HALESS OTHERMISS INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/IPI W=33-10-0 œ 4-11-0 Design Crit: DESIGN OF THE BGE) WALLS, 33-10-0 Over Continuous Support 3X5 = AND A FBC2007Res/TPI-2002(STD) A FT/RT=20%(0%)/10(0) 4 X 5 (R) 29-10-0 7 3X5= Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top 110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase chord in notchable area using 3x6. Wind reactions based on MWFRS pressures. factor See DWGS All015050109 & GBLLETIN0109 for more requirements 5 X 4 // for OU ICENS dead load is 1.50. STOWAL ENGINEE CENSE No. 66648 8 3X4// -0-0 (NN) 4X5(B2) =21000 09 BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/-/R/-40.0 20.0 24.0" 10.0 PSF 10.0 PSF PSF PSF PSF JREF-SEQN-DATE REF FROM DRW HCUSR8228 09350022 HC-ENG Scale =.1875"/Ft. R8228-1TXN8228Z02 JB/DF 12/16/09 67070 48883

Top chord 2x4 SP # Bot chord 2x4 SP # Webs 2x4 SP # R-429 U-0 RL-420/-227 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Truss passed check for 20 psf additional bottom chord live in areas with 42"-high x 24"-wide clearance. (9-243--Fill in later DOUG EDGLEY --TW Building Components Group Continuous lateral bracing equally spaced on member TYP. Haines City, FL 33844 FL 79 278 ALPINE Wave #2 Dense #2 Dense #3 DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BRILD THE TRUSS IN COMFORMANCE WITH THE THE OR FABRICATING, INADILING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMFORMS WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY AFAPA) AND IFF. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/166A UL/1/55/P), ASTH A653 GRADE 40/60 (W. K/H-SS) GALV. STEEL, APPLY PLATES TO FACH FACE OF TRUSS AND, UNIESS OFHEREMES CONCERN ON HIS DESIGN, POSITIOS REDBAMINGS HOM. A SEAL ON THIS ARE TO THE APPLY PLATES TO FACH FACE OF TRUSS AND. **WARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, UNAD REFER TO BCSI (BUILDING COMPONENT SAFETY MEMONATOR). PUBLIT PROBLEMENT OF THE OWNERS AND A CHARLES, ALEXANDER, VA. 22314) AND HEA CHEF SHEE 315. ALEXANDER, VA. 22314) AND HEA CHEF PRACTICES PRIORICATED CHEMICS SHALL HAVE PROPERTY ATTACHED STRUMA PROPERTY ATTACHED PRO **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. 4X4(B2) =312, ALEXANDRIA, VA. 22314) AND NECK (MODO TRUSS COUNCIL OF AMERICA. 6400 . NI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HESE FUNCTIONS. MULESS CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE Design Crit: 19-0-8 Over 2 Supports 1.5X4 Ⅲ 3X4 / FBC2007Res/TPI-2002(STD) FT/RT=20%(0%)/10(0) 3X4 / 3 X 4 ≡ HANDLING, SHIPPING, INSTALLING AND BRACING,
PUBLISHED BY TPI (TRUSS PLANE INSTITUTE, 218
DICK, (MOOD TRUSS COUNCIL OF AMERICA, 6300)
TRUS COUNCIL OF AMERICA, 6300 R-1291 U-388 W-6" ON CONTRACTOR. ITH BCG. INC. SHALL NOT BUILD THE TRUSS IN CONFORMANCE WITH 3 3 \ 4 ≡ 3X4 / 92 SEC.3. A SEAL ON THIS SOLELY FOR THE IRUSS COMPONENT 3 \ 4 ≡ (A) (A) 3 X 4 (R) Ⅲ In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Right end vertical not exposed to wind pressure 110 mph wind, 15.22 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Bottom chord checked for 10.00 psf non-concurrent live load Wind reactions based on MWFRS pressures. 2X4 III SO JCENS ORIOT IN No. 66648 10 QTY:6 BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. 40.0 1.25 10.0 20.0 PSF 10.0 PSF 0.0 PSF PSF PSF DATE SEQN-JREF -FROM HC-ENG JB/DF DRW HCUSR8228 09350012 Scale = .1875"/Ft. R8228- 48884 1TXN8228Z02 67491 12/16/09

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

8 Continuous lateral bracing equally spaced on member

Truss passed check for 20 psf additional bottom chord live in areas with 42"-high x 24"-wide clearance. load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

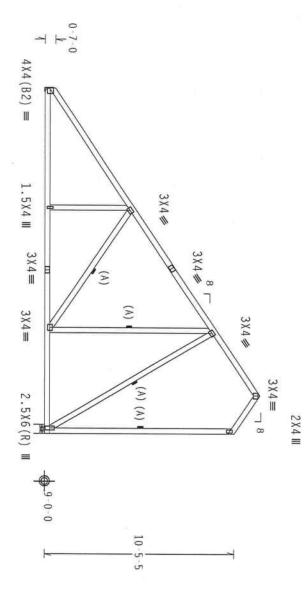
110 mph wind, 15.22 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

Bottom chord checked for 10.00 psf non-concurrent live load





WARNING TRUSSES REQUIRE EXIBENE CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO REST. (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 21B MORTH LEE SIREE, SHITE 312, ALEXANDRA, VA. 22314) AND MICA (POOD TRUSS COUNCIL OF AMERICA, 6300 ERITERPALSE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERMISE INDICATED TOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED TOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE Design Crit: FBC2007Res/TPI-2002(STD FT/RT=20%(0%)/10(0)

OUNCENS,

QTY:5

FL/-/4/-/-/R/-

BC DL TC DL TC LL

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR8228 09350013

DATE

12/16/09

REF

Scale = .1875"/Ft. R8228- 48885

PSF PSF

HC-ENG

JB/DF

67046

40.0

24.0" 1.25

JREF -

1TXN8228Z02

FROM SEQN- PLT

TYP.

Wave

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG. INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY TAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FARELANTHE, INNOLLIG. SHEPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPORMS WITH APPLICABLE PROVISIONS OF MOS (MALIONAL DESIGN SPEC, BY AFAPA) AND TPI.

THE RCG.
CONNECTOR PLATES ARE MADE OF 20/10/166A (M.19/SS/K) ASHM A653 GRADE 40/60 (M. K/M.SS) GRAVE PROVISIONS OF MOS (MALIONAL DESIGN SPEC, BY AFAPA) AND TPI.

PLATES TO EACH TAKE OF TRUSS AND. MULESS OTHERMS (LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z
ANY INSPECTION OF PLATES FOLLOWED BY (1) SMALL BE PER MANUEX AS OF THE 2002 SEC. 3. A SEAL ON THIS

ANY INSPECTION OF PLATES FOLLOWED BY (1) SMALL BE PER MANUEX AS OF TPI1-2002 SEC. 3.

TW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL "2278

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DRAWING INDICATES



Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense: Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense: Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load

Calculated horizontal deflection is 0.10" due to live load and 0.21" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8.

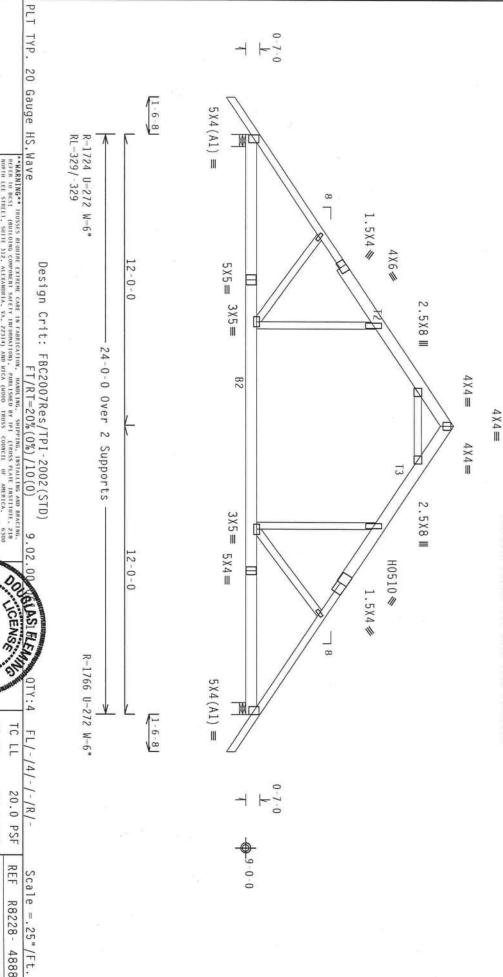
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



IMPORTANT*CORMISM A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DEG. INC. SUER RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMMORMAGE IP: OR FARELANTIAG, MANDIAG, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MATIGNAL DESIGN SPEC, BY ALFAY), AND FILE CONNECTED PLAIRS, ARE MOSE OF 70/18/15/06, (M.1/15/54), ASTH AGS GRADE 40/56 (M. 2/1/55) GAVE, STEEL, CONNECTED PLAIRS, ARE MOSE OF 70/18/15/06, (M.1/15/54), ASTH AGS GRADE 40/56 (M. 2/1/55) GAVE, STEEL, POLITION FACE OF TRUSS AND, DHILESS OFFICENS OF THIS DESIGN, POSITION FER DEATHINGS OF THE POSITION FER DEATHINGS OF ***MARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION, UNADDING, SHEPPING, INSTALLING AND BRACING, BETTER ID BOSS (MULTUNG COMPONENT SKETT) HUMBARNION), PRINCE BY ID (TRUSS PLAIT INSTITLE, ZIR HORTH LEE STREET, SHITE 132, ALEXANDRIA, V.A. 22343) AND WICA (MODO TRUSS CONTROL OF ARRENCA, GOOD CHIEDRISE LAME, MAISSM, AL MAYE PROPERTY ATTACHED STRUCTURAL PARTY AND BOTTON CHORD SHALL HAVE DIRECTLY INDICATED FOR CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARTY AND BOTTON CHORD SHALL HAVE DIRECTLY INDICATED FOR CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARTY. AND BOTTON CHORD SHALL HAVE DIRECTLY OF TRUSCHED SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARTY. A PROPERLY ATTACHED RIGID CEILING SS IN COMFORMANCE WITH

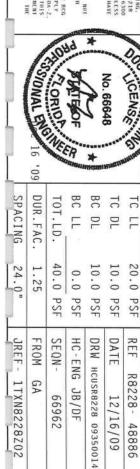
SPEC, BY ARRAY AND PE. THE REG 40/50 (M. K/M.SS) GALY, STELL, APPLY 5 DESIGN, POSITION FER DRAYHRIS 160A-Z-7 FEIL-2002 SEC. 3. A SLAI ON THIS TELL SOURCE FOR THE TRUSS COMPORED.

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844

BUILDING DESIGNER PER ANSI/TPI



1TXN8228Z02

66962

R8228- 48886

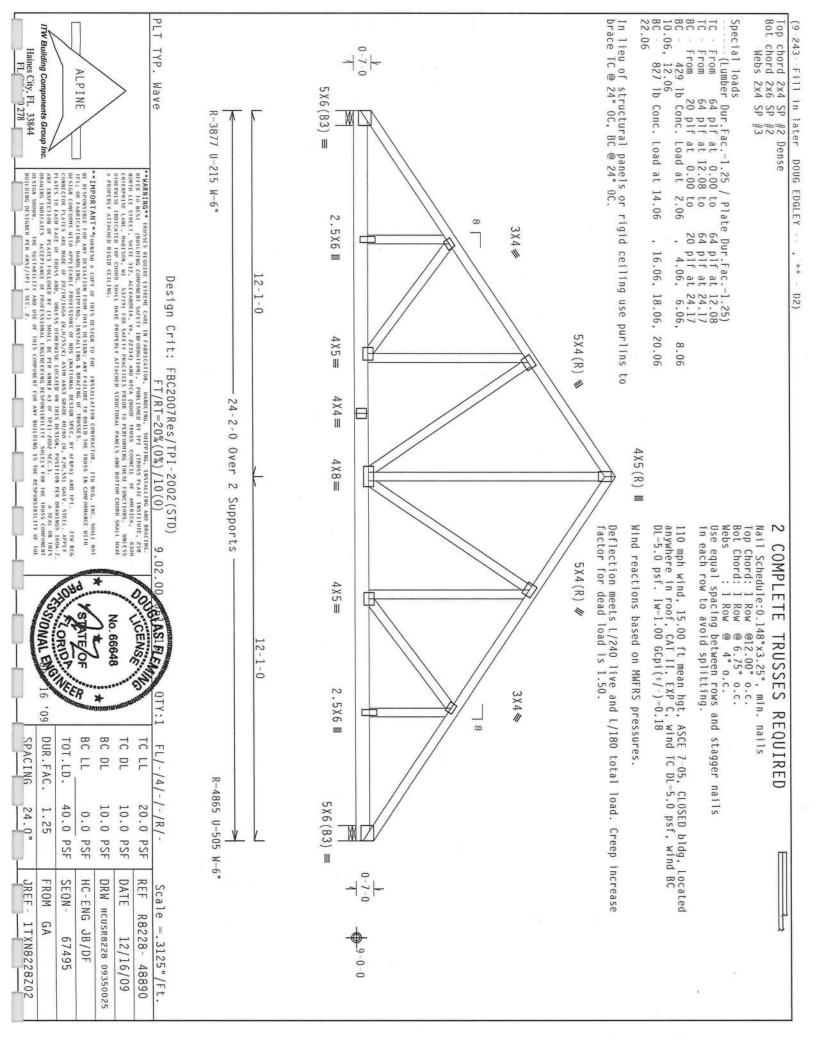
12/16/09

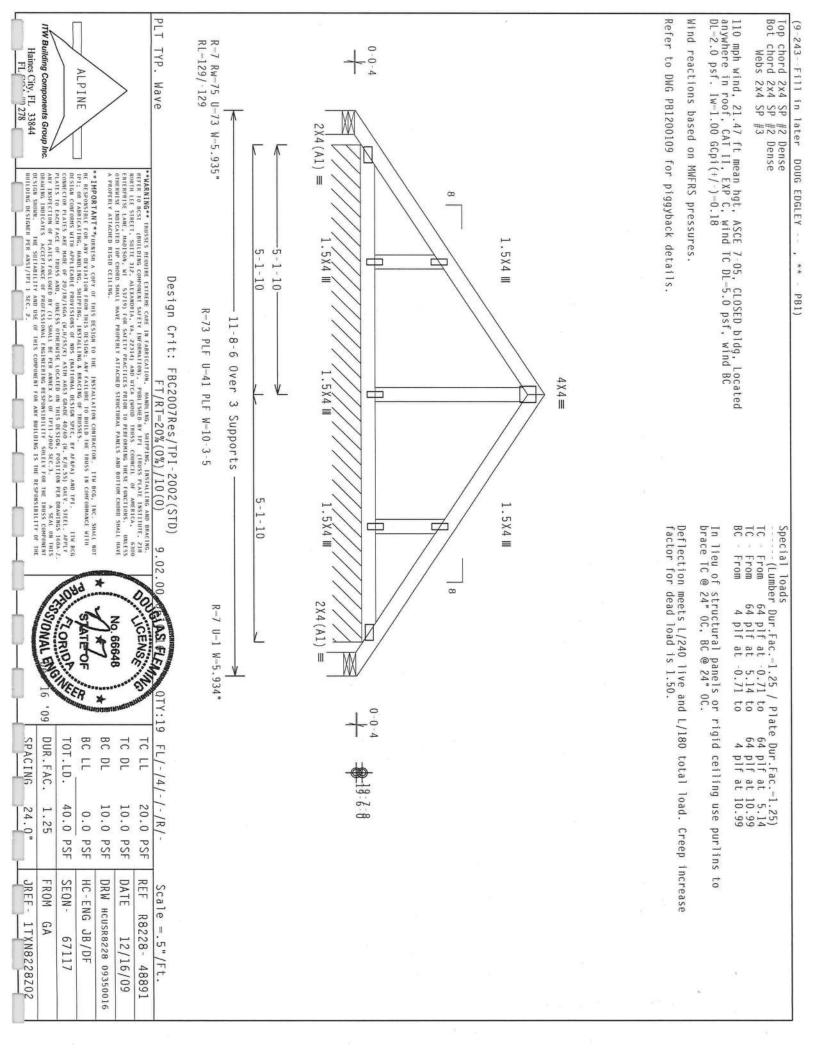
Top chord 2x4 SP #
Bot chord 2x6 SP #
Webs 2x4 SP # Calculated horizontal deflection is 0.09" due to live load and 0.21" due to dead load. PLT TYP. Wave Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. Collar-tie braced with continuous lateral bracing at 24 $^{\circ}$ OC. or rigid ceiling. Bottom chord checked for 10.00 psf non-concurrent live load (9-243--Fill in later DOUG EDGLEY --ITW Building Components Group Inc. 7-8-0 Haines City, FL 33844 FI '9 278 ALPINE 5X4(B1) = RL-262/-263 R-1613 U-236 W-6" (4.5" Effective Contact) #2 Dense :T2, T3 2x6 SP #2 :B2 2x6 SP #1 Dense: #3 **IMPORTANT*** TURBUISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, TANY FAILURE TO BUILD THE 18USS IN COMPORMANCE WITH 1PT; OR FARRICATION, JANDILLOS, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONDICTED PLATES ARE HADE OF 20/18/16GA (M. 1/85X/R) ASTH AGS. GRADE AG/80 (M. X/M.SS) GAN. STEEL. APPLY CONNECTOR PLATES ARE HADE OF 20/18/16GA (M. 1/85X/R) ASTH AGS. GRADE AG/80 (M. X/M.SS) GAN. STEEL. APPLY PLATES TO EACH FACT OF TRUSS AND. MILESS OFFIRM STEEL ORACITE ON THIS DESIGN, POSITION OF BLATES FOLLOWED OF (1) SHALL BE FOR ANNEX AS OF THIS 2003 SEC.3. A SEAL ON THIS DESIGN OF THIS COMPONENT OF THE PROPOREIT OR ANY INSPECTION OF PLATES FOLLOWED OF (1) SHALL BE FOR ANNEX AS OF THIS 2003 SEC.3. A SEAL ON THIS DESIGN. SHALLS ACCEPTANCE OF PROPOSED SALVANGE OF THE PROPOREIT OR ANY BUILDING IS THE RESPONSIBILLITY OF THE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, LHAQUIAG, SHIPPING, INSTALLING AND BRACING.
RETER TO BEST (BUTTOING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (CRUSS PLATE INSTITUTE, ZIB
MORIH LEE STREET, SUHT 312, ALEXANDRIA, VA. 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPISE LANE, MOISSON, WI 53719) FOR SAFETY FRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS
OTHERWISE INDICATED TOP COMED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE
A PROPERLY ATTACHED REGIO CELLING. BUILDING DESIGNER PER ANSI/IPI 1.5X4 4X6# #1 Dense: 5 X 5 = 1-10-8 Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=20%(0%)/10(0) 3 X 5 ≡ 8 2.5X8 III 24-0-0 Over 4 X 4 == 82 4 X 4 ≡ 2 Supports 4 X 4 == J BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8. 110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Wind reactions based on MWFRS pressures 2.5X8 III 9 .02.00 GOOD ASI FLEA 3×5= No. 66648 5 X 4 ≡ 12-0-0 4×6/ 0TY:8 1.5X4 / BC DL TC DL BC LL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-R=1654 U=236 W=6" 4X5 (B1) = 40.0 24.0" 1.25 10.0 10.0 PSF 20.0 PSF 0.0 PSF PSF PSF DATE REF FROM SEQN-JREF- 1TXN8228Z02 HC-ENG DRW HCUSR8228 09350015 Scale =.3125"/Ft. R8228- 48887 JB/DF 66969 12/16/09 9-0-0

Top chord 2x4 SP # Bot chord 2x6 SP # Webs 2x4 SP # :Stack Chord SC1 2 Stacked top chord must NOT be notched or cut in area (NNL). All stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6. Note: All Plates Are 1.5X4 Except As Shown. OTHERS Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Roof overhang supports 2.00 psf soffit load See DWGS All015050109 & GBLLETIN0109 for more requirements 9-243--Fill in later DOUG EDGLEY --TW Building Components Group Inc. MEMBER TO BE LATERALLY BRACED BRACING SYSTEM TO BE DESIGNED TYP. 0 - 7 - 0Haines City, FL 33844 FL 79 278 RL-62/-62 R-177 PLF U-33 ALPINE Wave 3X6(C6) = 4-0-0 (NNL) 2X4(C6) =-6-8 2x4 SP #2 Dense::Stack Chord #2 Dense :T2, T3 2x6 SP #1 Dense: #2 :B2 2x6 SP #1 Dense: must NOT be notched or cut in area (NNL). Attach PLF W=6 41 15 3X4 m **IMPORTANT**CHRNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. HC. SHALL NOT HE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALLURE TO BUILD THE TRUSS IN CONFORMANCE WITH PILL OF LOWER FOR THE PROPERTY OF THE RETER TO BOSS (QUILIDING COMPONENT SAFETY INFORMATION), IMANDEING, SMIPPING, INSTALLING AND BRACING, MORDH LEE STREET, SUITE 312, ALEXANDRIA, VA, 223143 AND WIGA (MODO) ERUSS COUNCIL OF AMERICA, 6300 CHIERPEISC LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OFHERMISE INDICATED TOP CHORD SHALL MAY PROPERLY ATTACHED STRUCTURAL PRACTICES AND STRUCTURAL PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. BUILDING DESIGNER PER ANSI/TPI I SEC. 2. 8 0 FOR 4 X 5 (R) 4 X 4 ≡ 12-0-0 OUT OF PLANE WIND LOADS FURNISHED BY 4X4≡ R=159 PLF U=20 PLF W=12-0-0 10-2-9 Design Crit: 24-0-0 SC2 2x4 SP 0ver 28-0-0 4 X 4 ≡ W FBC2007Res/TPI-2002(STD) #2 Dense: Supports FT/RT=20%(0%)/10(0) R=176 10-2 4X5(R) / 4 X 4 ≡ - 9 PLF U = 343X4// PLF W-6-0-0 BC attic 8-0-0 to THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. Truss spaced at 24.0" OC designed to support 1–0–0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Wind 110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Bottom chord checked for 10.00 psf non-concurrent live load In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C. BC @ 24" 0C. 3X6(C6) =4-0-0 (NNL) 9 -50 64,-1 reactions based on MWFRS pressures .02 2X4(C6) = room floor loading: LL = 40.00 psf; DL = 10.00 psf; 16.3-8. COURT PLE CENSE SC2 No. 66648 CORIOR 500 BUNEER DUR.FAC. BC 10 TC LL SPACING TOT.LD. FL/-/4/-DL DL /-/R/-40.0 20.0 24.0" 10.0 10.0 0.0 PSF. Top chord PSF PSF PSF PSF PSF REF JREF -SEQN-DRW DATE FROM HC-ENG Scale = .1875"/Ft. HCUSR8228 09350023 R8228-1TXN8228Z02 JB/DF 12/16/09 67207 48888

Note: All Plates Are 1.5X4 Except As Shown. Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. ROOF Bottom chord checked for 10.00 psf non-concurrent live load Roof overhang supports 2.00 psf soffit load SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS chord in notchable area using 3x6. See DWGS A11015050109 & GBLLETIN0109 for more requirements THE BUILDING DESIGNER IS RESPONSIBLE FOR THE (9-243--Fill in later ITW Building Components Group Inc. TYP. 0-7-0 AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, Haines City, FL 33844 FL 0278 ALPINE Wave RL=15/-15 PLF R=136 PLF U=26 PLF 1-6-8 3-6-8 (NNL) > 4X5 (B2) = 03 19 190-15 DOUG EDGLEY --**IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BOS, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; ANY TAILURE TO BUILD THE BUSS. IN COMPORMACE WITH IT: OR FLAREFACTING, HANDLING, SHEPTING, THISALLING A BRACHE OF TRUSSES, OF AREA) AND IPI. DESIGN CONFIDENCE WITH APPLICABLE PROVISIONS OF 1005 (MALIONAL DESIGN SPEC, BY AFREA) AND IPI. SITUATION OF THE BOS OF THE BUSS OF THE BOS **WARNING** TRUSSES BEQUIFE EXTREME CARE IN FARRICATION, IMBULING, SUPPPING, INSTALLING AND BRACING.
REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREET, SUFIE 312, ALEXANDRIA, VA. 22314) AND WICA (4000 TRUSS COUNCIL OF AMERICA, 6300
CHIERRIS ENDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI I 3X4 m PROPERLY ATTACHED RIGID CEILING W = 24 - 2 - 0** Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=20%(0%)/10(0) DESIGN OF THE DGE) 10-1-0 2x4 SP AND 24-2-0 Over Continuous Support #2 Dense: 20 $4 \times 4 =$ 2-0 IS THE RESPONSIBILITY OF 8 3 X 4 ≡ Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord 110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. must not be cut or notched. Wind reactions based on MWFRS pressures. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. (A) $1\times4~\#3$ SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. 9.02.00OUICENS, THE ORIOS IS CENSE p. 66648 3X4 // -10 3@ \$1 81 OTY:1 4X5 (B2) = -6-8 (NNL) 1-6-8 BC DL DUR.FAC. C TC LL SPACING TOT.LD. FL/-/4/-PL 40.0 10.0 1.25 10.0 20.0 -/R/-24.0" 0.0 PSF PSF PSF PSF PSF SEQN-DATE REF FROM DRW HCUSR8228 09350024 HC-ENG JREF - 1TXN8228Z02 Scale = .25"/Ft. R8228-JB/DF 12/16/09 67017 48889





MWFRS edge. 110 mph wind, 20.19 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL-2.0 psf. Iw=1.00 GCpi (+/-)=0.18 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Refer to DWG PB1200109 for piggyback details. Wind reactions based on MWFRS pressures [9-243--Fill in later DOUG EDGLEY --ITW Building Components Group TYP. Haines City, FL 33844 FL Co. "9 278 R=16 Rw=22 U=21 W=5.935" RL=41/-41 loads based on trusses located at least 20.19 ft. from roof ALPINE Wave 0-0-4 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. NIC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORANCE WITH IP: OR FARBICATION, DANNIELDES, SHIPPHOE, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND IPI. THE BCG CONNECTOR PLATES ARE MODE OF 20/18/166A (M.1458A), ASTA MCS GRADE 40/60 (M. X/M.55) GAV. STEEL, APPLY PARTES TO EACH FACE OF TRUSS AND, UNITES OTHERWISE (COLIED ON THIS DESIGN, POSITION PER DRAWINGS 160A. Z. AND INSPECTION OF PLATES TO LOUGHD BY (1) SHALL BE FER ANNIX A3 OF IPI-2002 SEC.3.

ANY INSPECTION OF PLATES ACCEPTANCE OF PROTESSIONAL ENGINEERING RESPONSEBILITY SOLETY FOR THE TRUSS COMPONENT OR ANY INSPECTION OF PLATES FOR THE TRUSS COMPONENT OF THE TRUSS COMPONE PROPERLY ATTACHED RIGID CEILING DESIGNER PER ANSI/TPI 1 SEC. $2X4(A1) \equiv$ 8 4-0-4 Over 3 Supports -> **1**-3-9→ R-81 PLF U-18 PLF W-2-7-2 1-3-9 11-3-9 ** Design Crit: FBC2007Res/TPI-2002(STD) 4 X 4 ≡ 1.5 X4 III PB4) 中 $2X4(A1) \equiv$ 8 IS THE RESPONSIBILITY OF THE R=16 U=6 W=5.935' In lieu of brace TC @ TC -BC -Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Special 9 From 6
From 6
From 6 .02.00 A TORION IS loads COUSTAST FLE structural panels or rigid ceiling use purlins to 24" OC. BC @ 24" OC. er Dur.Fac.=1.25 / Pla 64 plf at -0.71 to 64 plf at 1.30 to 4 plf at -0.71 to CENS No. 66648 Plate te Dur.Fac.=1.25) 64 plf at 1.30 64 plf at 3.31 4 plf at 3.31 BC DL TC DL DUR.FAC. BC LL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-24.0" 1.25 40.0 10.0 10.0 PSF 20.0 PSF 0.0 PSF PSF PSF JREF- 1TXN8228Z02 SEQN-DATE REF FROM HC-ENG DRW HCUSR8228 09350017 Scale =.5"/Ft. R8228- 48892 GA JB/DF 67049 12/16/09

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ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.
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Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
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                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
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plate plot details for special positioning requirements.
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                                                                                          ITW Building Components Group Inc.
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Haines City, FL 33844
FL Co. 40 278
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                                                                                                               **IMPORTANT**QUENTS A CORY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGNS, ANY FALLER TO BUILD THE FUNS ST. CHORDON CONTRACT OF THE PROPERTY OF THE 
                                                                                                                                                                                                                                                                                                                                               *** PARKILING*** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SUIPPING, INSTALLING AND REACING, REFER TO BECST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY 1PI (TRUSS PLAIE INSTITUTE, 218 MORTH LET STREET, SUITE 312, ALEXANDRIA, "A. 22314) AND WICA (MODO TRUSS COMPCTL OF AMERICA. 6300 ENTERPRIST LANE, MADISMY, M. 5319) FOR SAFETY PRACTICES PRIOR TO TERRORHING HASE UNCLIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL BARRIES AND SAFETY OF THE PROPERTY ATTACHED STRUCTURAL BARRIES AND SAFETY OF THE SAFETY OF T
                                  DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/TPI 1 SEC.
                                                                                       DRAWING INDICATES
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TES ACCEPTANCE OF F
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R=-16 Rw=76 U=80 W=3.5"
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                                                                                              SOLELY FOR THE TRUSS COMPONENT
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                           SPACING
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                               1TXN8228Z02
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Bot chord 2x4 SP Webs 2x4 SP 110 mph wind, 21.30 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL-2.0 psf. lw=1.00 GCpi(+/-)=0.18 Note: Refer to DWG PB1200109 for piggyback details In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures. See DWGS All030050109 & GBLLETIN0109 for more requirements. (9-243--Fill in later DOUG EDGLEY ITW Building Components Group Haines City, FL 33844 FL 77 79 278 TYP. All Plates Are 1.5X4 Except As Shown. R-7 RW-68 U-67 W-5.313" RL-117/-117 ALPINE Wave #2 Dense #2 Dense #3 0-0-4 **IMPORTANT** TUBBLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE RUSSS IN COMPORMANCE WITH THE DESIGN CONTROLLED, SHIPPING, INSTALLING A BRACING OF RUSSES.

DESIGN CONTROLLED, AND THE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY ALRAY) AND FPI. THE RCG CONTROLLED FOR THE ARE THAT OF THE PROVISIONS OF MOS (ANTIONAL DESIGN SPEC, BY ALRAY) AND FPI. THE RCG CONTROLLED FOR THATES ARE PANCE OF TRUSS ON, UNIVESS OF AND ASTA ARESTOR ARE THAT OF THE PROVISION OF THE THAT OF THE PROVIDENCE OF THE PROVIDENCE OF THE PROVIDENCE OF THE PROPERTY OF THE PROVIDENCE OF THE PROVIDENCE OF THE PROPERTY OF THE PROPERTY OF THE PROSPECTION OF PLATES OF THE PROPERTY OF TH **WARNING.** TRUSSES REQUIRE EXTREME CARE IN FARRICATION, DANDLING, SHIPPING, INSTALLING AND BRACING. TREER TO BEST. (BUILDING COMPONENT SAFETY IMPORATION), PHRILISING DAY DET (TRUSS PLATE INSTITUTE, 218 MORTH LEE SIREET, SHIE 312, ALEXANDRAA, VA, 22134) AND HICA (1000) TRUSS COUNCIL OF AMERICA, 6300 (ENTERPORTS LANE, MONISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OBJECTIVES AND BOTTON CHORD SHALL HAVE PROPERLY ALTACHED SPRICTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERLY ALTACHED REGION SHALL HAVE PROPERLY ALTACHED SPRICTURAL PARELS AND BOTTON CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI I SEC. 2X4(A1) R=103 PLF U=41 PLF W=9-3-0 * 111 Design Crit: FBC2007Res/TPI-2002(STD) 8 PB2) -4-7-8 FT/RT=20%(0%)/10(0) 10-7-13 Over 句 $4 \times 4 =$ Ф w Supports THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. 中 BC 1C Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Special loads From From 6 02.00 AND TORIOT IS SOUBLAS MEN \Box 64 plf at -0.68 to 64 plf at 4.64 to 4 plf at -0.68 to CENSE No. 66648 Dur.Fac.=1.25 / 8 2X4(A1) = R--34 RW-5 U-4 W-5.313" 09 Plate Dur.Fac.-1.25) 64 plf at 64 plf at 4 plf at BC DL TC DL DUR.FAC. TC LL SPACING FOT.LD. FL/-/4/-/-/R/-4.64 9.97 9.97 24.0" 1.25 40.0 10.0 20.0 PSF 10.0 0.0 PSF PSF PSF PSF JREF -DATE REF FROM SEQN-HC-ENG DRW HCUSR8228 09350027 Scale =.5"/Ft. R8228- 48894 1TXN8228Z02 GA JB/DF 12/16/09 67442

edge. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Wave MWFRS loads based on trusses located at least 8.45 ft. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. (9-243--Fill in later DOUG EDGLEY --TW Building Components Group Inc. Haines City, FL 33844 FL 7278 R=84 PLF U=18 PLF W=19-9-1 RL=11/-11 PLF ALPINE **IMPORTANT***URBHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY TAILURE TO BUILD THE RUSS IN COMPORMANCE WITH TP: OR FARBLISH THIS. MAINTILLOS. SHEPPING. INSTALLING & BRACTHO OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROPISIONS OF HOS (MAITONAL DESIGN SPEC, BY ATAPA) AND TPI. THE RCG CONNECTOR PLATES ARE MADE OF ZOJEGISCA, URJUSSEY, ASTH MASS JERME 40/260 (M. X.M.SS) AGAY. SHEEL, APPLY LAKES TO EACH FACE OF THUSS AND. BULLSS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF DRAWING INDICATES PROPERLY ATTACHED RIGID CETCING 8 9-10-8 Design Crit: V2) 1.5X4 Ⅲ .5X4 III from roof FT/RT=20%(0%)/10(0) 19-9-1 Over Continuous Support THIS DESIGN, POSITION PER DRAWINGS 160A-Z
OF TPI1-2002 SEC.3. A SEAL ON THIS
WSJBILLIY SOLELY FOR THE TRUSS COMPONENT 4 X 4 == 5 X 4 ≡ RESPONSIBILITY OF Bottom chord checked for 10.00 psf non-concurrent live load 110 mph wind, 16.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18 Wind reactions based on MWFRS pressures. See DWG VAL1300109 for valley details. 9.02 1.5X4 Ⅲ .5X4 III No. 66648 CENS 9-10-9 0TY:1 8 BC DL TC DL DUR.FAC. BC LL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-3X4(D1) =1.25 40.0 10.0 24.0" 10.0 20.0 PSF 0.0 PSF PSF PSF PSF REF DATE JREF - 1TXN8228Z02 FROM SEQN-HC-ENG DRW HCUSR8228 09350004 Scale = .375"/Ft. R8228 - 48895 GA JB/DF 12/16/09 66980

Top chord 2x4 SP | Bot chord 2x4 SP | Webs 2x4 SP | edge. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. MWFRS loads based on trusses located at least 8.78 ft. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (9-243--Fill in later DOUG EDGLEY --ITW Building Components Group TYP. Wave Haines City, FL 33844 FL 278 R-84 PLF U=18 PLF W=15-9-1 RL=10/-10 PLF ALPINE #2 Dense #2 Dense #3 3 X 4 (D1) **IMPORTANT**TORNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH IP: OR FARELSTING, MANUFLIG, SHEPPLY, HISTALLIGA BRACTHO OF TRUSSES.

ORSIGN COMPORES WITH APPLICABLE PROVISIONS OF HOS (MATIGNAL DESIGN SPEC, BY ALAPA) AND TPI. THE BCG CONNECTION PLATES ARE MORE OF 20/189/1604 (M.M.SYS), ASTH ASSES DEADE 40/50 (M.M.SYS), ASSES DEADE 40/50 (M. MARKAING. IRUSSES REQUIRE EXTREME CARE IN FARRICATION, IMADILING, SHIPPING, INSTALLING AND RRACING, RELER TO REST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLAIE INSTITUTE, 218 MORTH LEE STREET, SUITE 117, ALEKAMDELA, WA, 22314) AND MICA (MODD TRUSS CONDICTL OF AMERICA, 63500 EHICERPRIST LANE, AMADISMA, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMIG HEEST UNCITONS. UNLESS CONDICTS IN 18 MORTH AND AND MICA STREET, AND M ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING PROPERLY ATTACHED RIGID CETLING. DESIGNER PER ANSI/IPI 1 SEC. 2. 1.5X4 Ⅲ Design Crit: FBC2007Res/TPI-2002(STD) .5 X 4 iII -10-8 V3) 中 from roof 8 FT/RT=20%(0%)/10(0) 15-9-1 Over Continuous Support IS THE RESPONSIBILITY OF .5X4 4 X 4 = \Box See DWG VAL1300109 for valley details. Bottom chord checked for 10.00 psf non-concurrent live load. Wind reactions based on MWFRS pressures. 110 mph wind, 17.56 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18 9 .02.00 COURTE PLEAT CENSE 7-10-9 1.5X4 III QTY:1 09 BC DL TC DL DUR.FAC. TC LL SPACING FOT.LD. FL/-/4/-/-/R/-1.25 40.0 3X4(D1) =24.0" 10.0 10.0 PSF 20.0 PSF 0.0 PSF PSF PSF DATE REF JREF- 1TXN8228Z02 FROM SEQN-HC-ENG DRW HCUSR8228 09350005 Scale =.5"/Ft. R8228- 48896 JB/DF 66983 12/16/09

edge. MWFRS Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (9-243--Fill in later DOUG EDGLEY --TW Building Components Group Inc. 0-0-1 Haines City, FL 33844 FL 278 TYP. Wave loads based on trusses located at least 9.11 ft. from roof R=84 PLF U=17 PLF W=11-9-1 RL=10/-10 PLF ALPINE 3X4 (D1) BE RESONSTANT *TRUMENSES A COMP OF THIS DESIGN TO THE INSTANTANT CONTRACTOR. THE BCG. HAC. SHALL NOT BE RESONSTANT FOR ANY PACLATION FROM THIS DESIGN. ANY FALLING TO BRILLD HIS RUSS IN CONTORNANCE WITH THIS LOW FARE CALLED ANY FALLING TO BRILLD HE RUSS IN CONTORNANCE WITH APPLICABLE PROPERTIES. HESTANT HAS GRADE OF THE SET OF THE FACE OF TRUSS AND HELES AND HALLES AND HALLES AND HALLES AND HALLES OF TRUSS AND, HALLES OF HERMING SHOULD BE LOCALED ON HIS BESIGN. POSSITION FOR BRANINGS HOLAZ AND HASPECTION OF PLATES INCLUDED BY ICL SHALL BE PER AWARY AND IPTI-ZOOP SEC. 3. HALLES COMPONENT HE SHIFT AND THE BLOCK COMPONENT HE SHIFT AND THE BLOCK COMPONENT HE SHIFT AND THE SHIFT FOR THE BUSS COMPONENT HE SHIFT AND THE SHIFT OF THE BUSS COMPONENT HE SHIFT AND THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI **HARNING** HRUSSES REQUIRE EXTREME CARE IN FABRICATION, IMADILIAG, SHIPPING, INSTALLING AND BRACING.
BETER TO BOSS (BUILDING COMPONENT SAFETY INFORMATION), PRINCISHED BY THE (TRUSS PLATE HISTITUTE, 218
WORTH LEE STREET, SHITE 312, ALEXANDRIA, VA. 2231A) AND NICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPLIS LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HRESE FUNCTIONS. UNLESS
OTHERWISE HUNCLARED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGID CEILING. 00 5-10-8 ** Design Crit: 11-9-1 Over Continuous Support V4) FBC2007Res/TPI-2002(STD) 4 X 4 = 中 /RT=20% (0%) /10 (0) 5-10-9 110 mph wind, 18.23 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 See DWG VAL1300109 for valley details. Bottom chord checked for 10.00 psf non-concurrent live load. Wind reactions based on MWFRS pressures. 9.02.00 Salla Svalle CORIONAL BUSINESS 3X4(D1) =No. 66648 .09 BC LL TC DL DUR.FAC. BC TC LL SPACING TOT.LD. FL/-/4/-DL €16-1-7 40.0 24.0" 1.25 10.0 10.0 PSF 20.0 PSF 0.0 PSF PSF PSF JREF -DATE REF SEQN-FROM HC-ENG DRW HCUSR8228 09350006 Scale =.5"/Ft. R8228- 48897 GA 1TXN8228Z02 JB/DF 66987 12/16/09

edge. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. MWFRS loads based on trusses located at least 9.45 ft. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (9-243--Fill in later DOUG EDGLEY ITW Building Components Group Inc. TYP. Haines City, FL 33844 FL "7 278 ALPINE Wave RL-10/-10 PLF R-84 PLF U-15 PLF **IMPORTANT**CUBMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. THE. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TATURE TO BRILD THE FRUSS IN COMPORANCE MITH IP: OR FABRICATION. ANNOTHER, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI. THE BCG CONNECTOR PLATES ARE ANDE TO ZO/BEJGEA (M. 14/SEY). ASTH AGS GRADE 40/SOR (M. K/M.SS) GAAV. SITEL. APPLY PLATES TO FACH FACE OF TRUSS AND. UNITS OTHER DEATH OF STEAL AND TRANSPORT OF PLATES OF THE STEAL AND THE STEAL AND THE BOARDINGS FOR A SEA, OUT HIS BRACING OF PROFESSIONAL ENGINEER BY AFARA AS OF FPI-ZOOZ SEC.). A SEA, OUT HIS BRACING OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE BUSS COMPONENT. DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. PROPERLY ATTACHED RIGID CEILING. 3 X 4 (D1 ** W=7-9-1 8 Design Crit: 3-10-8 7-9-1 Over Continuous Support V5) from roof FBC2007Res/TPI-2002(STD) .5X4 4 X 4 == 中 /RT=20%(0%)/10(0) IS THE RESPONSIBILITY OF THE 8 3-10-9 3X4(D1) = Bottom chord checked for 10.00 psf non-concurrent live load 110 mph wind, 18.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18 Wind reactions based on MWFRS pressures See DWG VAL1300109 for valley details. 02.00 OU GENSE ORIDA IN No. 66648 DUR.FAC. BC LL BC DL TC DL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-40.0 24.0" 1.25 20.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF PSF SEQN-DATE REF JRFF- 1TXN8228Z02 FROM HC-ENG DRW HCUSR8228 09350007 Scale =.5"/Ft. R8228- 48898 GA JB/DF 66990 12/16/09

(9-243--Fill in later DOUG EDGLEY --** V6)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

Bottom chord checked for 10.00 psf non-concurrent live load

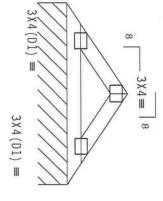
See DWG VAL1300109 for valley details.

110 mph wind, 19.56 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 19.56 ft. from roof



18-9-8

3-9-1 -10-8 Over Continuous Support 1-10-8

R=83 PLF U=2 PLF W=3-9-1 RL=8/-8 PLF

Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=20%(0%)/10(0)

TYP.

Wave

NORTH LEE STREET, SUITE 31
ENTERPRISE LANE, MADISON,
OTHERWISE INDICATED TOP CH PROPERLY ATTACHED RIGID CEILING BUSSIS ROUBER TYPERE CARE IN FARRICATION, HARDLING, SHIPPING, I (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (FIRST LINE) TO THE COUNCIL. SHITE 312, ALEXANDRIA, VA. 22310) AND WICK (MODE TRUSS COUNCIL. SHITE 312, ALEXANDRIA, PERFORENT FRACTICES PRIOR TO PERFORMING IN CARED TO PERFORMING IN CARED TO PERFORMING THE MADIES FRACTICES OF PROPERTY ATTACHED STRUCTURAL PARILES AND HANDLING. SHIPPING, INSTALLING AND BRACING.
PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
ATCA (MOOD TRUSS COUNCIL OF AMERICA. 6300 THESE FUNCTIONS. HMLESS OD BOTTOM CHORD SHALL HAVE

** IMPORTANT** "BURNISM, COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE MCG. WINCL SHALL NOT

RE RESPONSIBLE TOR ANY REPLYANTON FROM THIS DESIGNS, ANY FAILURE TO BULLD THE RUNSES. ALCOHOROMACE WITH

THE OR FARRICATING, MANDLING, SHIPPING, INSTALLING, A BRACHEG OF REMISES.

RESIGN CONTROLS HITM APPLICABLE PROPESSIONS OF MCS. (MATIONAL USSIGN SPIC. BY ATAPA) AND THE

CONNICTOR PLACES ARE MODE OF 20/18/166A (M.1/SS/R) ASTH A653 GRACE A0/60 (M. KULSS) GALV, STEEL, APPLY

TALTES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN, PERSITION FER DRAWINGS 160A-2.

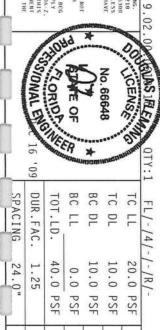
ANY HERECTION OF PLACES TOLLOWED BY (1) SHALL HE BER ANNEX AS OF 1PT1-2002 SEC.3.

A SEAL ON THIS DRAWING INDICATES DESIGNER PER ANSI/TPI I SEC. 2. SOLELY FOR THE TRUSS COMPONENT

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 7278



SEQN-

HC-ENG

JB/DF 66994

DRW HCUSR8228 09350018

FROM

GA

JREF-

1TXN8228Z02

REF

R8228- 48899

Scale =.5"/Ft.

DATE

12/16/09

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. See Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. Wave (9-243--Fill in later DOUG EDGLEY --TW Building Components Group Inc. DWG VALI300109 for valley details. DWGS All030050109 & GBLLETIN0109 for more requirements. R=77 PLF U=20 PLF RL=10/-10 PLF Haines City, FL 33844 FL 0278 ALPINE $2X4(A1) \equiv$ #2 Dense #2 Dense #3 W-23-9-1 **IMPORTANT***UBBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. TO THE TRUSS IN COMPORANCE WITH PI; OR FARELAND THIS, MANUFLLOS, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF HOS (MATHRMAL DESIGN SPEC, BY AFAPA) AND FPI. THE BCG CONNECTION FLATES ARE MADE OF 20/181/66A (M. HASSAY), ASIN 663 GRADE 40/50 (M. X/H.SS) GRAV. SHEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, HULESS OTHERWISE LOCALED ON HIS DESIGN. POSITION FEE DEALINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND, HULESS OTHERWISE LOCALED ON HIS DESIGN. POSITION FEE DEALINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND. BULLES OTHERWISE LOCALED ON HIS DESIGN. POSITION FEE DEALINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND. BULLES OTHERWISE LOCALED ON HIS DESIGN. POSITION FEE DEALINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND ALERGAMENT OF THE TRUSS COMPONENT OF THE TRUSS COMPONE **WARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND RRACING, REFER TO REST. (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY PT (TRUSS PLATE INSTITUTE, 210 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, Z2314) AND NICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ERREPRISE LANE, MANISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS ORNEWLISE HANDSON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS ORNEWLISE HANDSON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DESIGNER PER ANSI/IPI 10-9-15 Design Crit: 22-8-9 FBC2007Res/TPI-2002(STD) FT/RT=20%(0%)/10(0) Over Continuous Support 3 \ 4 = 4 X 4 == RESPONSIBILITY OF 110 mph wind, 15.91 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Bottom chord checked for 10.00 psf non-concurrent live Wind reactions based on MWFRS pressures. 9 .02.00 MONAS FLES 10-9-15 CENS No. 66648 ROINEER 8 DUR.FAC. BC BC. C SPACING TOT.LD. TC $2X4(A1) \equiv$ FL/-/4/-/-/R/-DL DL 40.0 24.0" 1.25 10.0 10.0 20.0 PSF 0.0 PSF PSF PSF PSF DATE REF JREF- 1TXN8228Z02 FROM SEQN-HC-ENG DRW HCUSR8228 09350019 Scale =.3125"/Ft. R8228- 48900 JB/DF 67009 12/16/09

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES

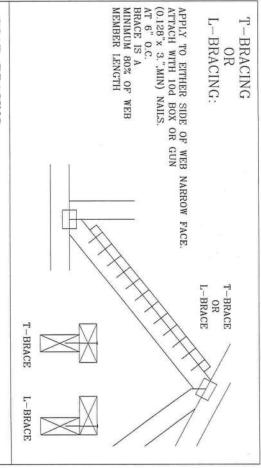
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

2-2X6(*)	2X6	2 ROWS	2X8	
1-2X8	2X6	1 ROW	2X8	
2-2X4	2X6	2 ROWS	2X6	
1-2X6	2X4	1 ROW	2X6	
2-2X4	2X6	2 ROWS	3 OR 2X4	2X3
1-2X4	2X4	1 ROW	OR	2X3
ALTERNATIVE BRACING ,—BRACE SCAB BRACE	T OR L-BRACE	SPECIFIED CLB BRACING	WEB MEMBER SIZE	WEB

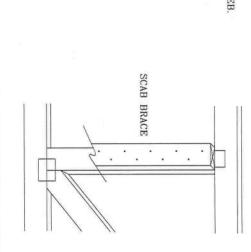
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.126"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
BOZ OF WEB MEMBER LENGTH

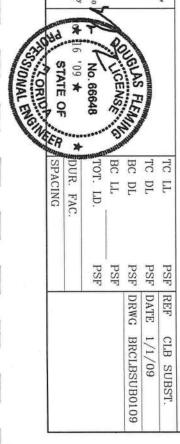


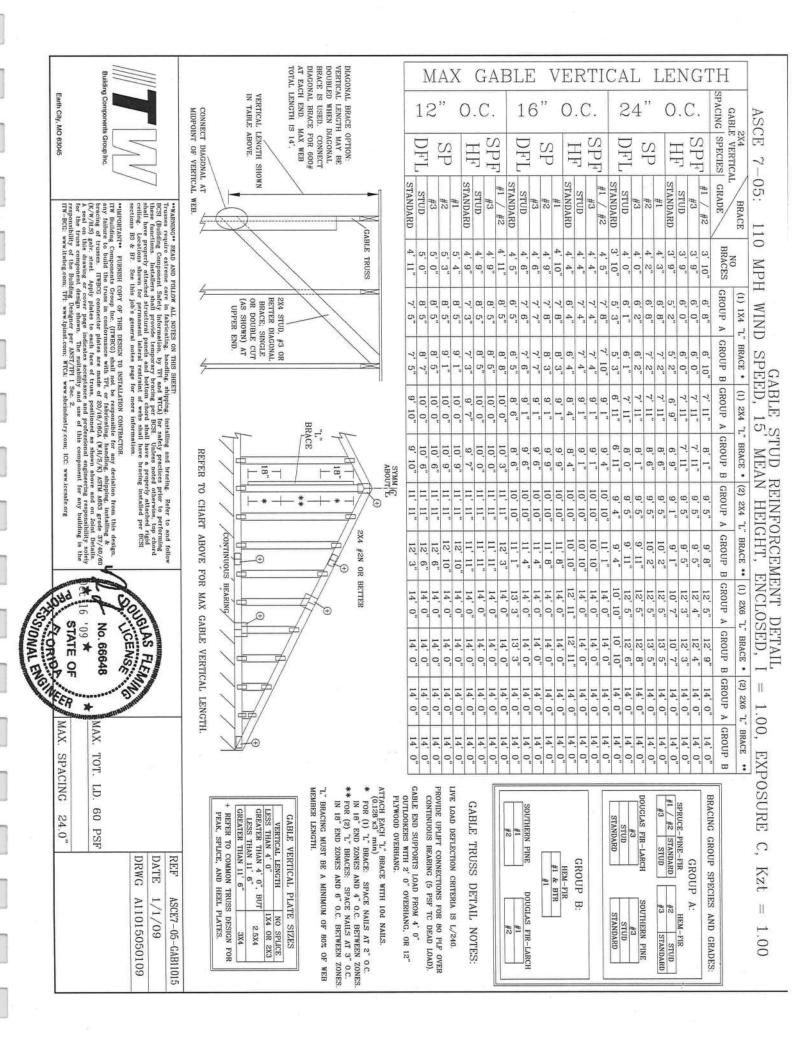


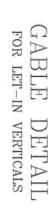
Earth City, MO 63045

RABING READ AND FOLLOW ALL NOTES ON THIS SHEET;
Trueses require extreme care in febricating, handling, shipping, installing and bracing. Refer to and follow
BCSI (Building Component Safety Information, by TPI and WTCs) for safety practices prior to performing
these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, lop chote
shall have properly attached structural panels and bottom chord shall have a properly attached rigid
ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI
ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI
ceiling as & Br. See this job's general notes page for more information.

■*MPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
I'W Building Components Group Inc. (I'WECG) shall not be responsible for any deviation from this design.
I'W Building Components Group Inc. (I'WECG) shall not be responsible for any deviation.
Installing a function of the trues in conformance with TP, or fabricating, handling, shipping, installing & Practing of trueses. I'WECG connector plates are made of 20/26/56A, (Wil/S/K) ASTM ASS3 grade 37/40/60.
(K/WHIS) galls, steel. Apply plates to each face of trues, positioned as shown above and on Joint bealts.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the trues component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/FPI 1 Sec. 2.
I'W-BCC: www.itscbag.com; TPI: www.iphnat.com; WCC: www.icscbaldustry.com; ICC: www.iccsafe.org







ABOUT C

 IF GABLE VERTICAL PLATES OVERLAP, USE A (+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK SPLICE, WEB AND HEEL PLATES. MINIMUM PLATE SIZES FOR VERTICAL STUDS. REFER TO APPROPRIATE ITW GABLE DETAIL FOR SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

TOENAIL

OR

ENDNAIL

GABLE TRUSS PLATE SIZES

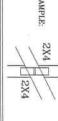
"T" REINFORCEMENT ATTACHMENT DETAIL

"T" REINFORCING

"T" REINFORCING

EXAMPLE: 2X4





2X8

TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

WEB LENGTH INCREASE

W/ "T"

BRACE

WIND SPEED "T" REINF.

AND MRH

MBR. SIZE

INCREASE

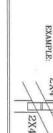
30 FT 130 MPH

15 FT

2x6 2x4 140 MPH 140 MPH

15 FT

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.





END DRIVEN NAILS: ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

10d COMMON (0.148"X 3.",MIN) NAILS AT 4" O.C. PLUS (4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:

10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

ASCE 7-02 GABLE DETAIL DRAWINGS A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109 A10030980109

A13015020109, A12015020109, A11015020109,

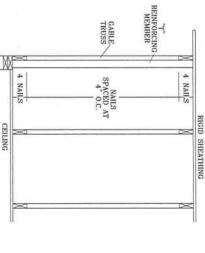
ASCE 7-05 GABLE DETAIL DRAWINGS A13030020109, A12030020109, A11030020109, A10015020109, A14015020109, A10030020109, A14030020109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM A13030050109, A12030050109, A11030050109, A13015050109, A12015050109, A11015050109, A10015050109, A14015050109, A10030050109, A14030050109

UNREINFORCED GABLE VERTICAL LENGTH.



TRUSS 4 NAILS NAILS SPACED AT 4 O.C. 4 NAILS CEILING



""*WARNIGA" READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme core in fabricating, handling, shipping, installing and bracing. Refer to and folio
RESI (Building Component Safety information, by TPI and WTCA) for safety practices prior to performing
these functions. Installarts shill provide temporary bracing per RESI, Unless noted otherwise, top chord
shall have properly attached structural panels and bottom chord shall have a properly attached rigid
ceiling. Locations aboven for permanent lateral restraint of webs shall have bracing installed per BCSI
sections 83 & 87. See this job's general notes page for more information. Refer to and follow

HIPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

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I'M Building Components Group ho. (ITHECC) shall not be responsible for any deviation from this design, any failure to blild the truss in conformance with TH, or fabricating, handling, shipping, installing & L. Parker of the Contract of the

* - ₀ 66648 STATE OF ACENS . IAX TOT. LD.

GABLE VERTICAL = 24" O.C. SP #3 MEAN ROOF HEIGHT = 30 FT, Kzt = MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 2X4 "L" BRACE LENGTH = 6' 7" "T" REINFORCING MEMBER SIZE = 2X4 ASCE WIND SPEED = 100 MPH T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 15 FT 90 MPH 30 FT 30 FT 100 MPH 110 MPH 15 FT 120 MPH 30 FT 90 MPH 120 MPH 100 MPH 110 MPH 130 MPH 30 FT 15 FT 15 30 1.10 x 6' 7" = 7' 2x6 2x4 2x4 2x6 2x4 2x6 2x4 2x6 2x4 2x4 2x6 2x4 1.00

REF LET-IN VERT

DRWG GBLLETIN0109

DATE 1/1/09

ANY 60 PSF 24.0"

Earth City, MO 63045

AND TORIOR OF AN

MAX SPACING

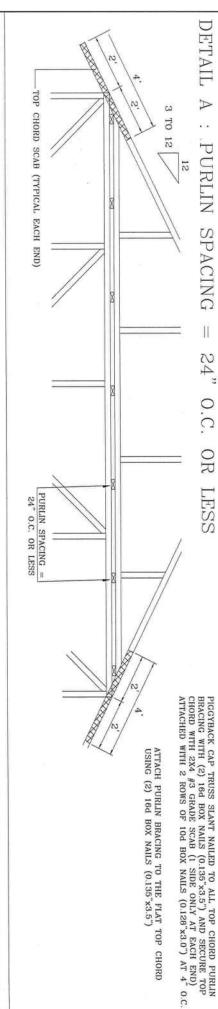
DUR. FAC.

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZT=1.0. 120 PIGGYBACK

MAXIMUM TRUSS SPACING IS 24" O.C. DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

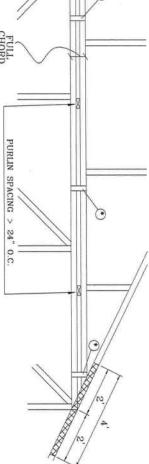
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.





PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C. USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND SAIO TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE ® 8 O.C. WITH (4) 0.120°x1.375° NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUST TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.



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12

12

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

2x4 VERTICAL SCABS

B"AB"X, /2" RATED SHEATHING GUSSETS (EACH FACE), ATTACH @ R O.C. WITH (B) GA COMMON (0.113" x2"), NAILS PER GUSSET (44 IN) CAP BUTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4 O.C. FRONT TO BACK FACES.

PLYWOOD GUSSET

IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

2x4 SPF#2, FULL CHORD DEPTH SCABS ® 8' FACE. STAGGERED 4' O.C. ATTACH WITH (3) NAILS (0.128°x3") INTO BOTH CHORDS (TOTAL NAILS PER SCAB). 0.C. 10d BOX 6

WAVE PIGGYBACK PLATE

NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

DEPTH

TOP CHORD SCAB (TYPICAL EACH END)

"IMPORTANT" FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

"IT'S Building Components Group Inc. (ITWECG) shall not be responsible for any deviation from this design, any failure to build the trues in conformance with TPI, or fabricating, bandling, shipping, installing, k any failure to build the trues in conformance with TPI, or fabricating, bandling, shipping, installing & bracing of trueses. ITWECG connector plates are made of 20,161/66A (W.J/S/K), ASTM A653 greet 20/140/60 bracing of trueses. ITWECG connector plates are made of 20,161/66A (W.J/S/K), ASTM A653 greet 20/140/60 (K.JW.H.S) gair steel. Apply plates to each face of trues, positioned as shown above and on Joint Details (K.JW.H.S) are steel. Apply plates to each face of trues, positioned as shown above and on Joint Details (K.JW.H.S) gair steel. Apply plates to each face of trues.

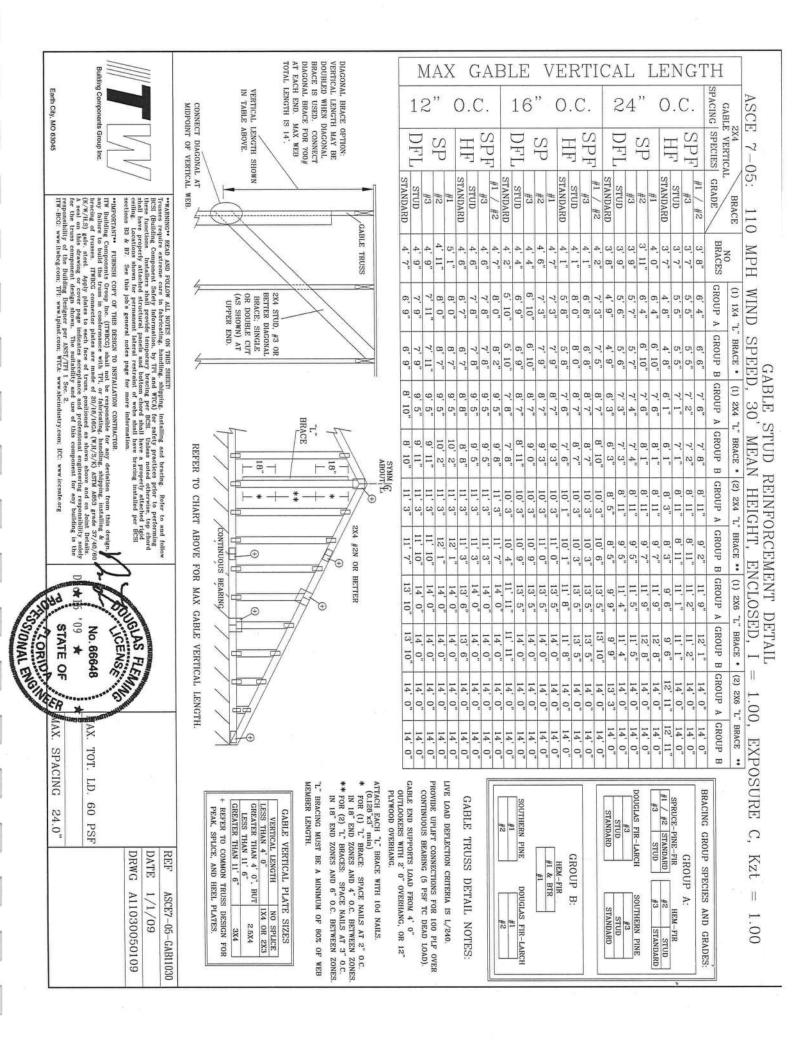
ICC: www.iccsafe.org

Earth City, MO 63045



DATE REF PB1201009 10/01/09 PIGGYBACK

24.0"



VALLEY TRUSS DETAIL

TOP CHORD BOT CHORD 2X4 2X4 2X4 SP SP #2N, #2N. OR SPF #1/#2 OR BETTER. SPF #1/#2, DF-L #2 OR BETTER SPF #1/#2, DF-L #2 OR BETTER

** SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, WIND TC DL=5 PSF, Kzt = 1.00ASCE 7-02 OR ASCE 7-05 130 MPH. 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'-9". LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" O.C., OR CONTINUOUS LATERAL BRACING

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%

FOR VERTICALS OVER 10'-0" TALL, APPLY (2) 1x4 "T" BRACE, TO NARROW FACE, SAME GRADE AS WEB MEMBER, ATTACH WITH 8d OR 0.128"x3" GUN NAILS @6" 0.C., STAGGERED

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" O.C. OR AS OTHERWISE SPECIFIED ON ENGINEER'S SEALED DESIGN

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ENGINEER'S SEALED DESIGN

CUT FROM 2X6 OR LARGER AS REQ'D

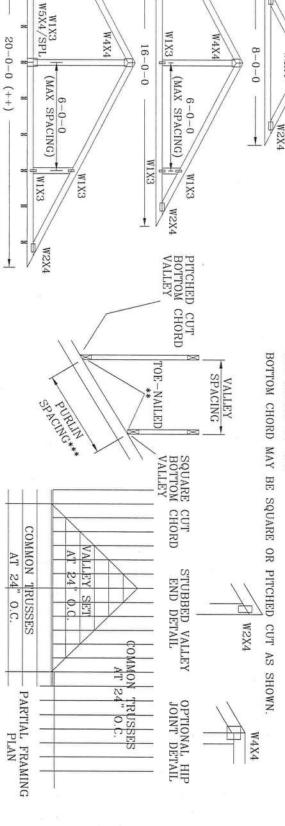
12 MAX.

W2X4

-0-0

W2X4

LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 14'-0".



W1X3

WIX3

12 MAX.

12

12 MAX.

W1X3 W1X3



Earth City, MO 63045

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FUSS! (Building Component Safety information, by TPI and WTCA) for safety practices prior to performing
these functions. Installers shall provide temporary bracing per BCS. Unless noted otherwise, top chord
shall have properly attached structural panets and bottom chord shall have a properly attached rigid
ceiling. Locations shown for permanent lateral restraint of seets shall have bracing installed per BCSI
sections B3 & B7. See this job's general notes page for more information.

SUPPORTING TRUSSES AT 24" O.C. MAXIMUM SPACING

