

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0.278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: ITXN8228Z0216112519

Truss Fabricator: Anderson Truss Company
Job Identification: 9-243--Fill in later DOUG EDGLEY -- , **
Truss Count: 26
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A1101505-GBLLETIN-PB120-A1103005-VAL130-

Seal Date: 12/16/2009

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	48875--D1		09350002	12/16/09
2	48876--A1		09350008	12/16/09
3	48877--A2		09350009	12/16/09
4	48878--A4		09350010	12/16/09
5	48879--AGE		09350020	12/16/09
6	48880--AAGE		09350021	12/16/09
7	48881--A3		09350011	12/16/09
8	48882--B1		09350003	12/16/09
9	48883--BGE		09350022	12/16/09
10	48884--B3		09350012	12/16/09
11	48885--B2		09350013	12/16/09
12	48886--C1		09350014	12/16/09
13	48887--C2		09350015	12/16/09
14	48888--CGE		09350023	12/16/09
15	48889--DGE		09350024	12/16/09
16	48890--D2		09350025	12/16/09
17	48891--PB1		09350016	12/16/09
18	48892--PB4		09350017	12/16/09
19	48893--PB3		09350026	12/16/09
20	48894--PB2		09350027	12/16/09
21	48895--V2		09350004	12/16/09
22	48896--V3		09350005	12/16/09
23	48897--V4		09350006	12/16/09
24	48898--V5		09350007	12/16/09
25	48899--V6		09350018	12/16/09
26	48900--V1		09350019	12/16/09





Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G C p (+/-) = -0.18$

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

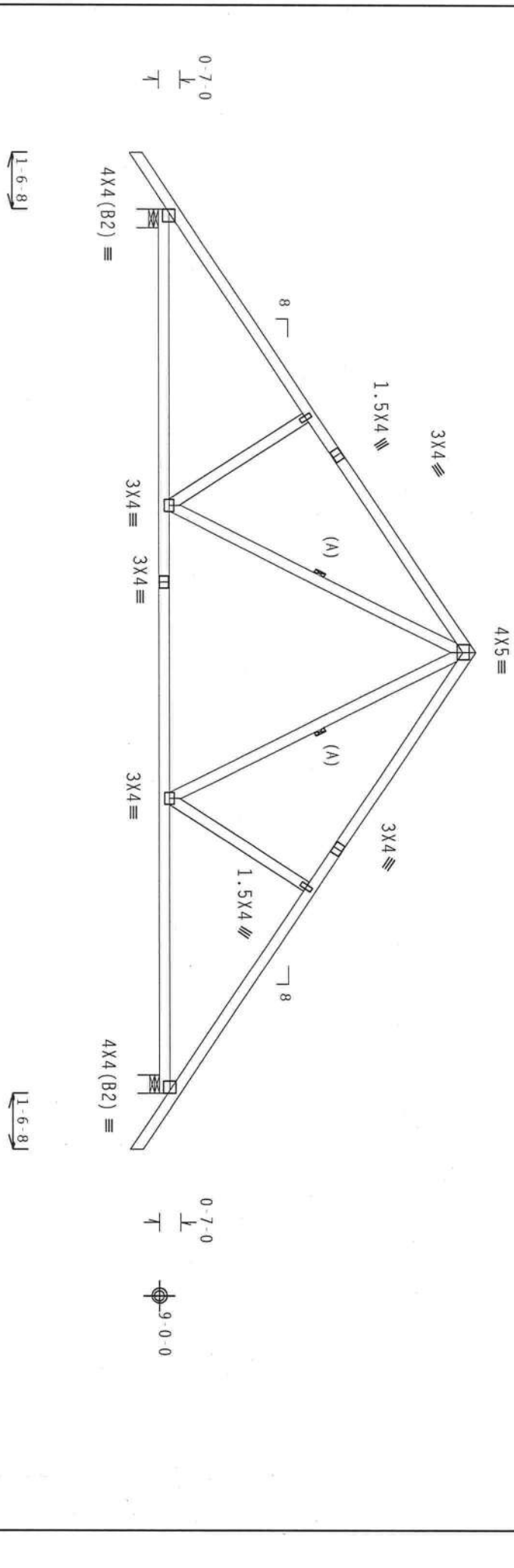
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



12'-1-0
24'-2-0 over 2 Supports
12'-1-0

R=1206 U=276 W=6"
RL=332/-332

R=1206 U=276 W=6"

PLT TYP. Wave

Design Crit: FBC2007Res/IPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY: 5 FL/-/4/-/R/-

Scale = .25"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RES1 (INCLUDING COMPONENT SAFETY INFORMATION), PROVIDED BY THE TRUSS MANUFACTURER, FOR ALL TRUSS SPECIFICATIONS, INCLUDING BUT NOT LIMITED TO: TRUSS TYPE, TRUSS SIZE, TRUSS WEIGHT, TRUSS LENGTH, TRUSS WIDTH, TRUSS HEIGHT, TRUSS SPACING, TRUSS BRACING, TRUSS CONNECTIONS, TRUSS MATERIALS, TRUSS FINISHES, TRUSS TOLERANCES, TRUSS DEFLECTIONS, TRUSS VIBRATIONS, TRUSS NOISE, TRUSS CORROSION, TRUSS PESTS, TRUSS FIRE, TRUSS COLLAPSE, TRUSS FAILURE, TRUSS DAMAGE, TRUSS REPAIR, TRUSS REPLACEMENT, TRUSS DEMOLITION, TRUSS DISPOSAL, TRUSS STORAGE, TRUSS TRANSPORT, TRUSS INSTALLATION, TRUSS MAINTENANCE, TRUSS INSPECTION, TRUSS TESTING, TRUSS CERTIFICATION, TRUSS REGISTRATION, TRUSS LICENSING, TRUSS INSURANCE, TRUSS WARRANTY, TRUSS LIABILITY, TRUSS COMPLIANCE, TRUSS PERMITS, TRUSS ZONING, TRUSS ORDINANCES, TRUSS STANDARDS, TRUSS CODES, TRUSS REGULATIONS, TRUSS ENFORCEMENT, TRUSS VIOLATIONS, TRUSS PENALTIES, TRUSS FINE, TRUSS JAIL, TRUSS DEATH, TRUSS HELL.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 000000278

DOUGLAS FLEMING

PROFESSIONAL ENGINEER

No. 666648

FLORIDA

16 '09

TC LL	20.0 PSF	REF R8228- 48875
TC DL	10.0 PSF	DATE 12/16/09
BC DL	10.0 PSF	DRW HCURSR8228 09350002
BC LL	0.0 PSF	HC-ENG JB/DF
TOT. LD.	40.0 PSF	SEON- 66936
DUR. FAC.	1.25	FROM GA
SPACING	24.0"	JREF- ITXN8228202

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MIFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



9.02.00

QTY: 7

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

2.00
DOUGLAS FLEMING
LICENSE
No. 66648
OTY

ITW Building Components Group Inc

Haines City, FL 33844

FL 278

16.09

TC LL	20.0 PSF	REF	R8228 - 48876
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67120
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF -	1TXN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MFERS loads based on trusses located at least 15.00 ft. from roof edge.

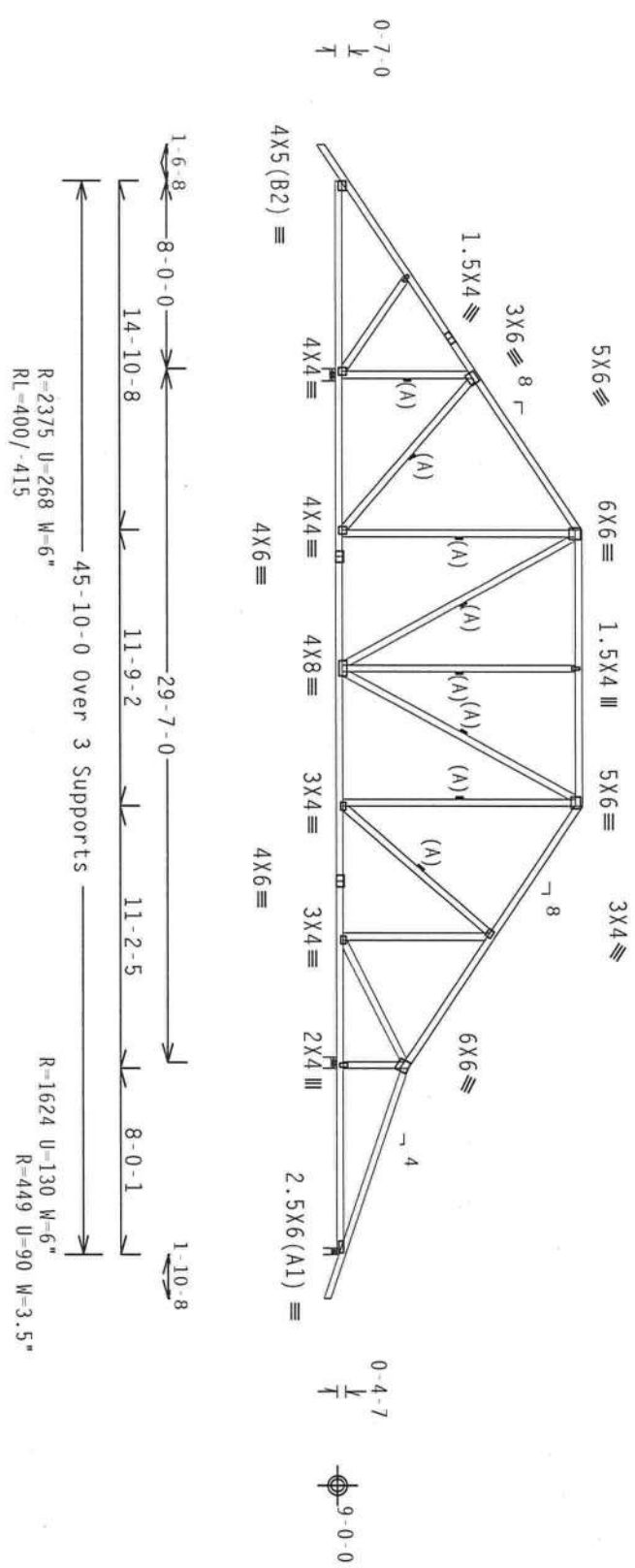
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 gcpl(+/-)=0.18

Wind reactions based on MFERS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:6

FL/-/4/-/-/R/-

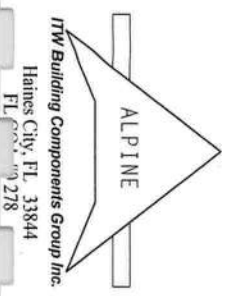
Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BOULDER COUNTY SAFETY INFORMATION), PUBLISHED BY THE (CROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICK BRIDGE TRUSS COMPANY, 21800 WICK BRIDGE ROAD, SUITE 100, WICK BRIDGE, NJ, 07095) FOR PROPER HANDLING AND BRACING. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS ARE IN FEET AND INCHES. DIMENSIONS SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AREA) AND TPI. THE BCG, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE CROSS COMPONENT DESIGN SHOWN. THE SUFFICIENCY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 48877
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEON-	67105
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	UREF-	1TXN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

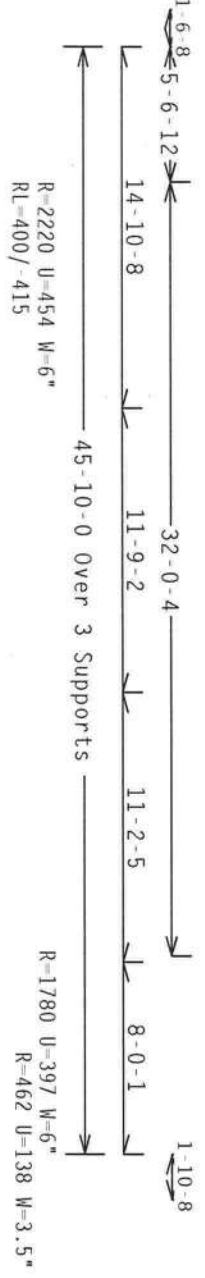
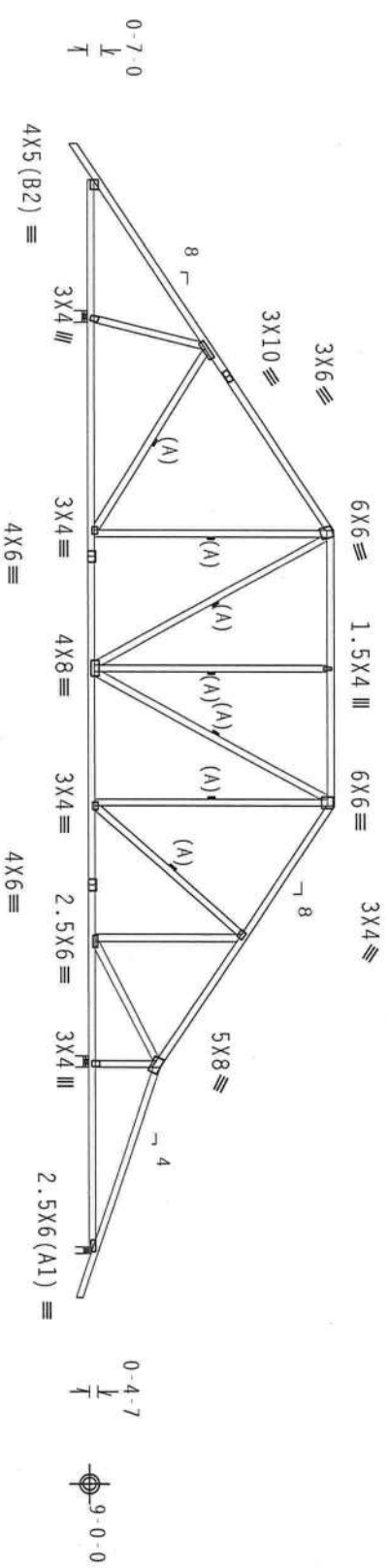
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

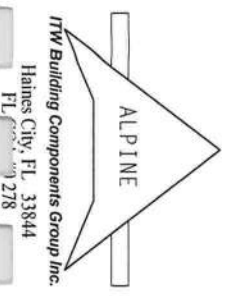
QTY:1

FL/-/4/-/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (QUALITY) COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS CONSTRUCTION, 6500 ENTERPRISE LANE, HOUSTON, TX, 77061) FOR SAFETY PRACTICES PRIOR TO PREVENTING THESE INSTRUCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHALL BE PROPERLY ATTACHED STRUCTURAL PLATES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE RCG DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. THE RCG CORRELATION PLATES ARE MADE OF 20/18/16GA (GALV/SS/GA) ASH AND GRADE 40/60 (W, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48878
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350010
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEON-	67502
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

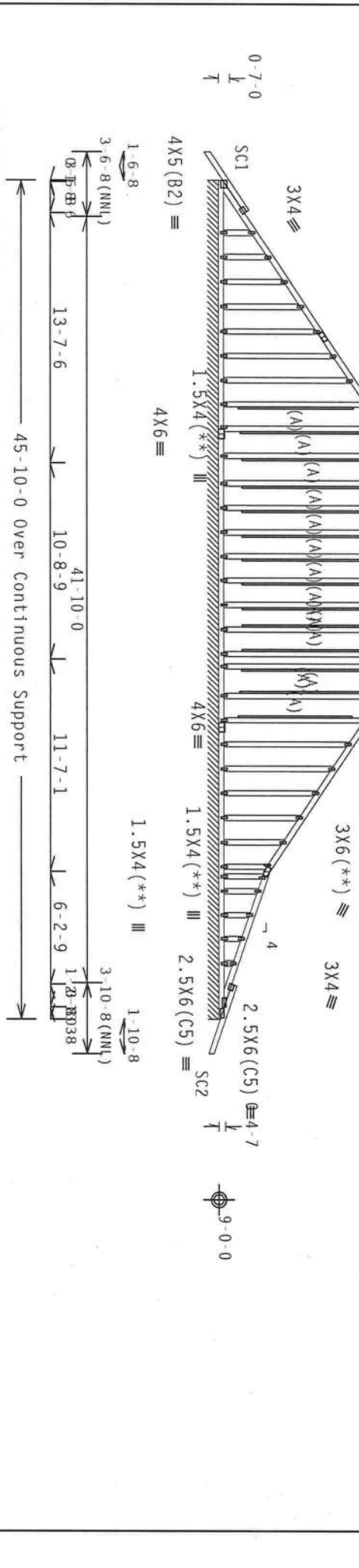
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE
ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND
SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS
LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE
DESIGNED BY THE BUILDING DESIGNER.

(**) 6 plate(s) require special positioning. Refer to scaled plate
plot details for special positioning requirements.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MMFRS pressures.
See DWGS A11015050109 & 6BLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML).
Dropped top chord braced at 24" o.c. intervals. Attach stacked top
chord (SC) to dropped top chord in notchable area using 3x4
tie plates 24" o.c. Center plate on stacked/dropped chord
interface, plate length perpendicular to chord length. Splice top
chord in notchable area using 3x6.

(A) 1x4 #3SRB SP-F-S or better "L" brace. 80% length of web member.
Attach with 8d Box or Gun (0-.113"x2.5", min.) nails @ 6" OC.
In lieu of structural panels or rigid ceiling use purlins to brace
TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.
WARNING: Furnish a copy of this DWG to the installation
contractor. Special care must be taken during handling, shipping
and installation of trusses. See "WARNING" note below.



R-149 PLF U-27 PLF W=45-10-0
RL=10/-10 PLF

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

QTY:1
FL/-/4/-/R/-
Scale = .125"/ft.

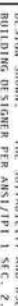
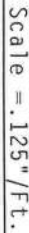
ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL 000000278

PROFESSIONAL ENGINEER
FLORIDA
No. 06648
DOUGLAS E. EDGLEY
LICENSE

TC LL	20.0 PSF	REF	R8228- 48879
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350020
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEON-	67180
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



REF	R8228-48880
DATE	12/16/09
DRW	HCUSR8228_09350021
HC-ENG	JB/DF
SEQN-	67256
TOT.LD.	40.0 PSF
BC LL	0.0 PSF
BC DL	10.0 PSF
TC DL	10.0 PSF
TC LL	20.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
JREF-	1TYN8228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

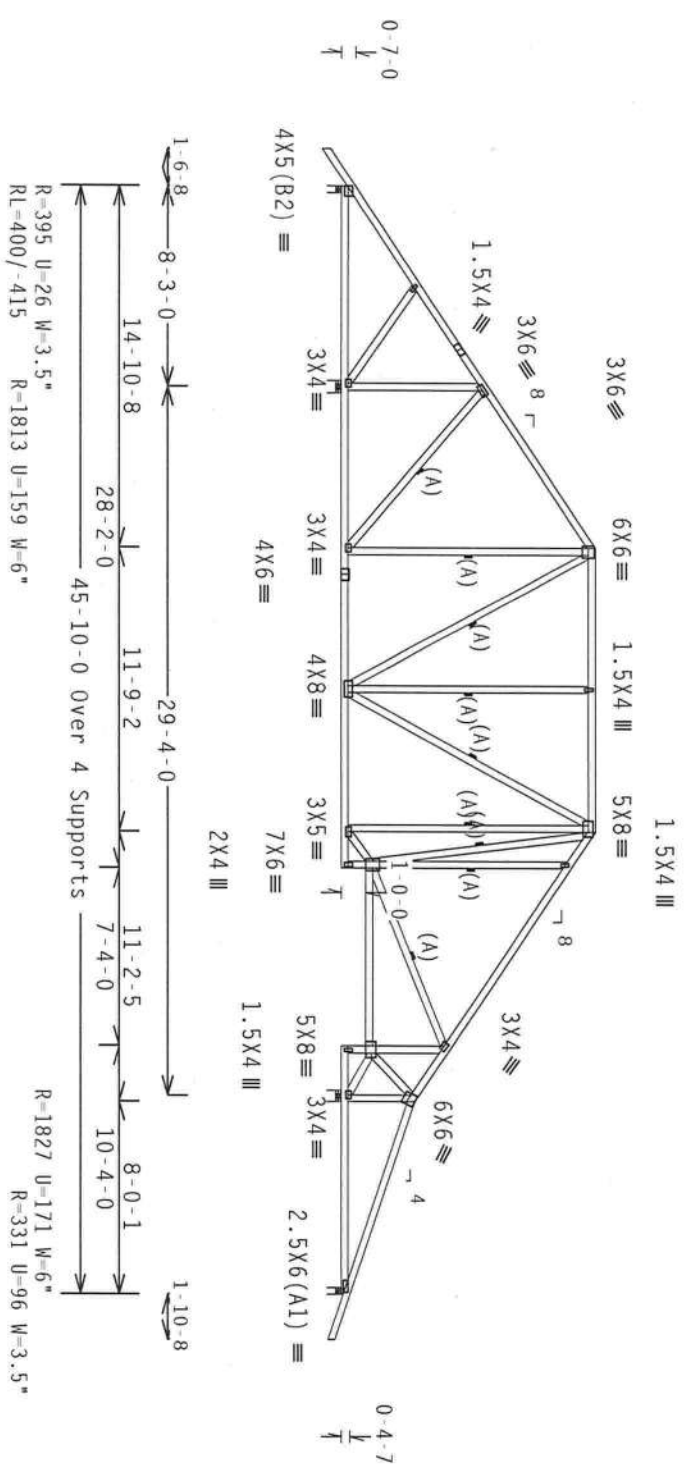
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 Gcp1(+/-)=0.18

Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY: 5

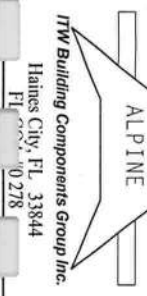
FL/-/4/-/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. BEFORE TRUSS CONSTRUCTION, CONSULT THE TRUSS MANUFACTURER'S INSTRUCTIONS. 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICKIWOOD TRUSS COMPANY OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

ITW BCG CORP. 11000 W. 150TH AVENUE, SUITE 100, MINNETONKA, MN 55345. PHONE: 952.891.1100. FAX: 952.891.1101. E-MAIL: SALES@ITWB.COM. WWW.ITWB.COM. DESIGN, ENGINEERING, AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48881
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350011
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEON-	67148
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	ITXN8228202

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $Gcpi(+/-)=0.18$

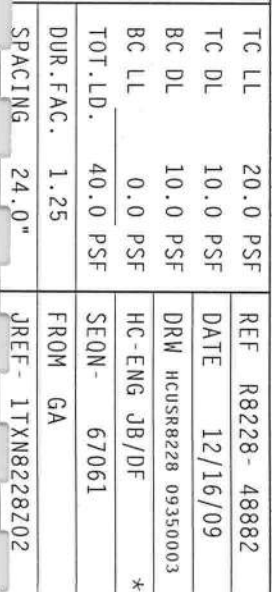
Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

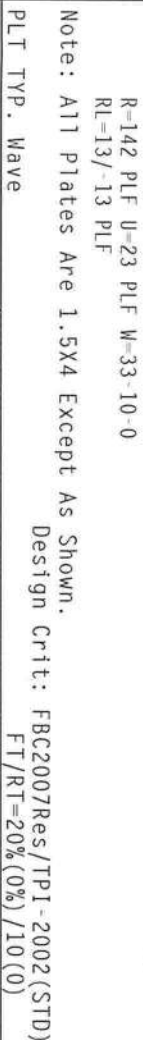
Wind reactions based on MWFRS pressures.

See DWGS A11015050109 & GBULLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL).
Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4

interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



FT/RT=20%(0%)/10(0)	9.02.00
---------------------	---------


FL/-/4/-/-/R/-

Scale = .1875"/Ft.

Design Crit: FBC2007Res/TP1-2002(STD)

R=142 PLF U=23 PLF W=33-10-0
RL=13/-13 PLF

"WARNING - FRICKS, BRIGOFF, ERTEN, CASE IN PUBLICATION, MANAGING, SHIPPING, INSTALLING, AND BRACING REFER TO DC-51 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE FRICKS PLANT INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK GOOD TRESS COUNCIL OF AMERICA, 62000 MIDWAY, ENTERPRISE LAKE, MISSOURI, MO, 63129 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. INTERESTED PROPERTIES INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BILD CELLING.



ALPINE

Haines City, FL 33844
FL 33844-0278



TC LL	20.0 PSF	REF	R8228- 48883
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350022
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67070
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

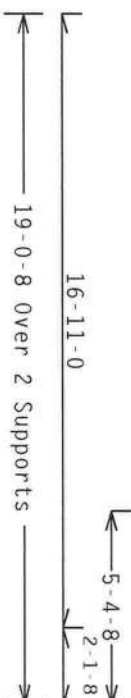
110 mph wind, 15.22 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

100

Bottom chord checked for 10.00 psf non-concurrent live load.



R=1291 U=388 W=6"

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$FT/RT=20\%(0\%)/10(0)$$


3.02.00

QTY: 6

FL/-/4/-/-/R/-

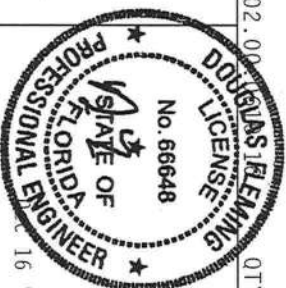
Scale = .1875"/Ft.

WARNING—FIRMS REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO GC61 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE CRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA GOOD TRUSS COMPANY OF AMERICA, 6500 UNIVERSITY ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUTTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CEILING.



Haines City, FL 33844

FL 2014-117278



60.

SPACING 24.0"

JREF - 1TXN8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

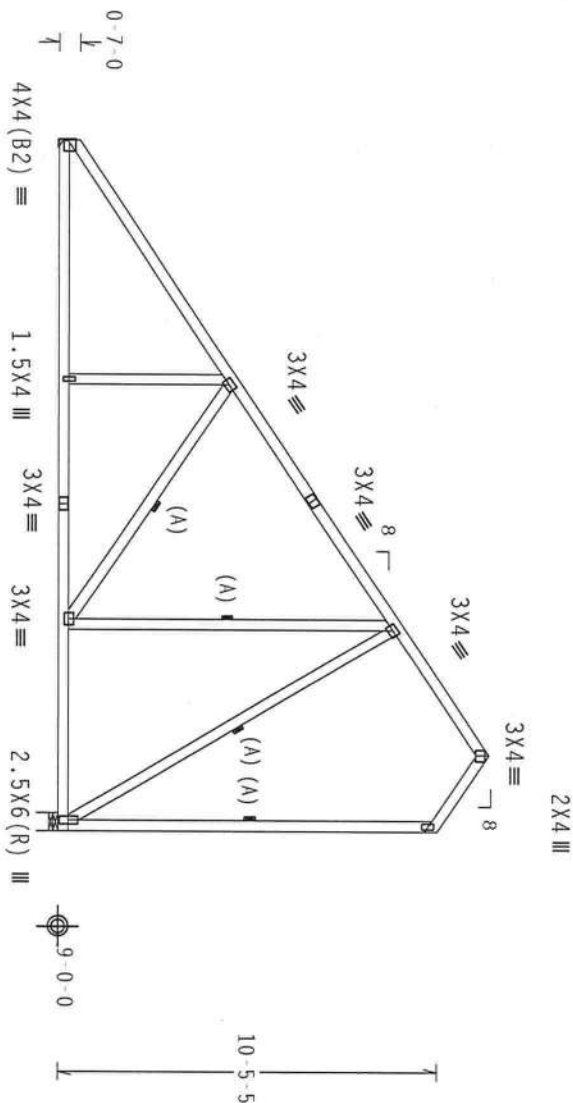
110 mph wind, 15.22 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.



16-11-0
2-1-8
19-0-8 Over 2 Supports
R-827 U=79
RL=420/-227
R-893 U=306 W=6"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:5

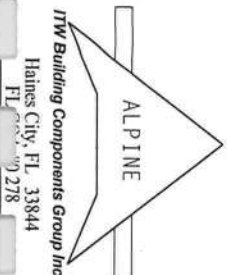
FL/-/4/-/-/R/-

Scale = .1875"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE NATIONAL BUREAU OF STANDARDS, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK BOND TRUSS COMPANY OF AMERICA, UNLESS OTHERWISE INDICATED. MODISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 2018/1604 (C/HS/PS) ASH 6051 GRADE 40/50 (W, K/L, S) GALV. STEEL. APPLY TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1604-2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE DESIGN. THE SEAL ON THIS BUILDING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 48885
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350013
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEON-	67046
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	ITXN8228202

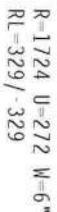
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. on

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

QTY: 4

Scale = .25" / Ft.

DUPLICATE LICENSE
No. 66648

STATE OF



TC LL	20.0 PSF	REF	R8228 - 48886
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350014
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	66962
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF -	1TXN8228202

Top chord 2x4 SP #2 Dense: T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 : B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Calculated horizontal deflection is 0.09" due to live load and 0.21" due to dead load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

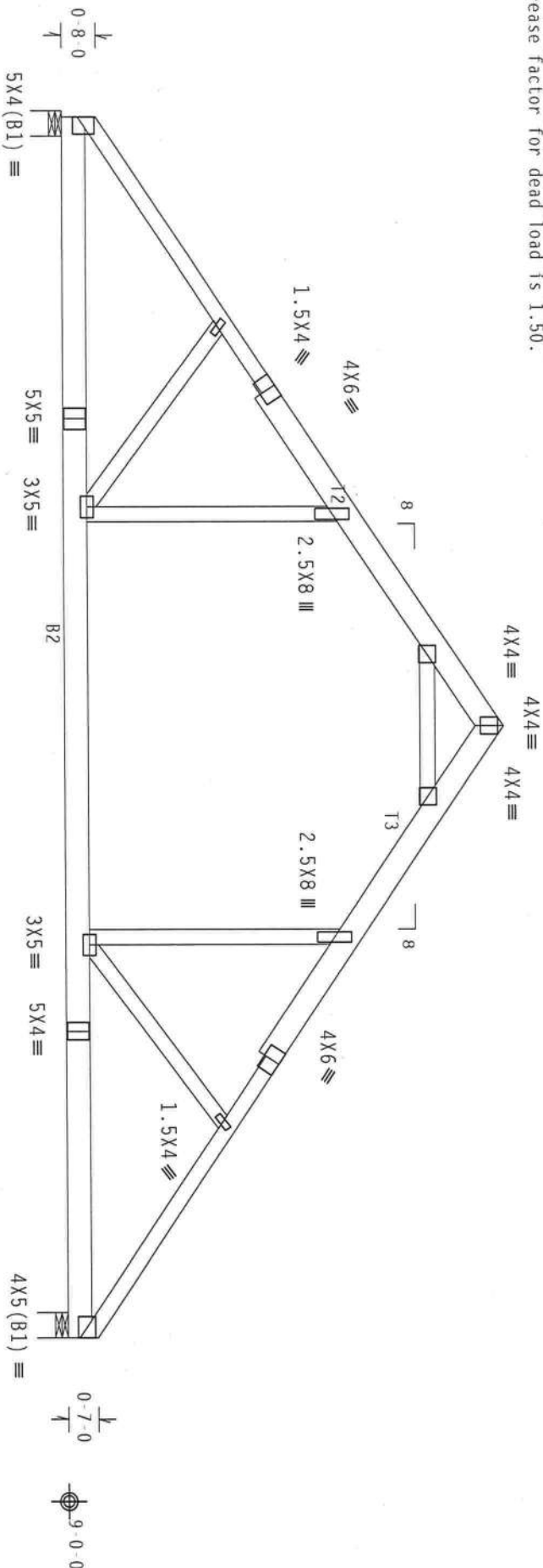


Diagram illustrating the distribution of 24-0-0 fertilizer over 2 supports. The total length is 11-10-8, and the segment length is 12-0-0.

R=1654 U=236 W=6"

R=1613 U=236 W=6"
(4.5" Effective Contact)
RL=262/-263

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$\text{FT/RT} = 20\% (0\%) / 10 (0)$$

9.02.00

QTY: 8

$$\text{FL} / - / 4 / - / - / \text{R} / -$$

Scale = .3125"/ft.

WARNING: THESE PRODUCTS REQUIRE CARE IN FABRICATION, HANDLING, CUTTING, INSTALLING AND DRACING. REFER TO DECSI (DUST/DIOXIDE COMPONENT SAFETY INFORMATION), PUBLISHED BY THE GIBBS PAPER INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WICA (WOOD TOXIC COMPONENT) OF AMERICA, 6300 INTERSTATE LANE, MIDLAND, TX 79706 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SECTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 9278



16.09

SPACING 24.0"

JREF - 1TXN8228Z02

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

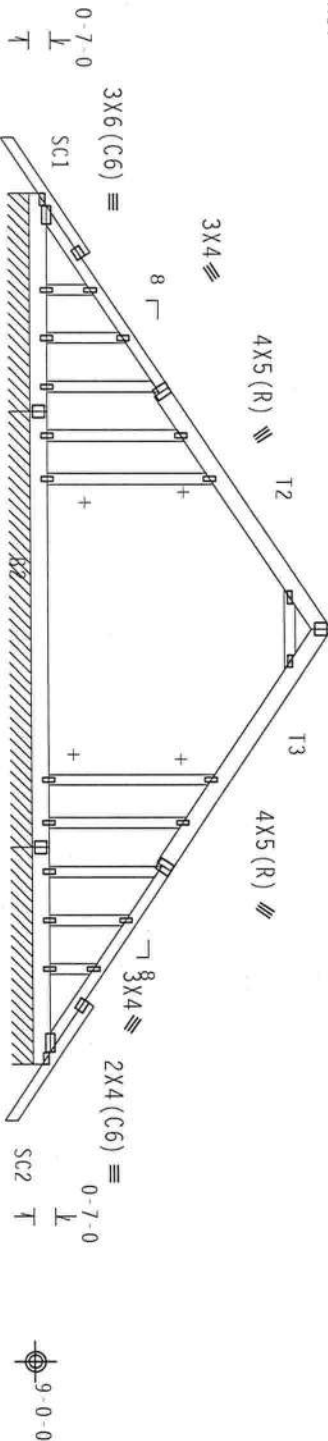
See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.



110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 gcpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-3-8.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

R=177 PLF U=33 PLF W=6-0-0
RL=62/ 62 PLF
R=159 PLF U=20 PLF W=12-0-0
R=176 PLF U=34 PLF W=6-0-0

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale =.1875"/ft.

WARNING TRUSSES BROUGHT EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA 6000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



ITW Building Components Group Inc.

Haines City, FL 33844

FL 33844



TC LL	20.0 PSF	REF	R8228- 48888
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCSR8228 09350023
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67207
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	ITXN8228202

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC

DL=5.0 psf. IW=1.00 GCPI (+/-)=0.18

MIENCO ASSOCIATES

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outlookers. Cladding load shall not exceed 10.00 PSF. Top chord

MEMBER OF THE BOARD OF DIRECTORS

Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

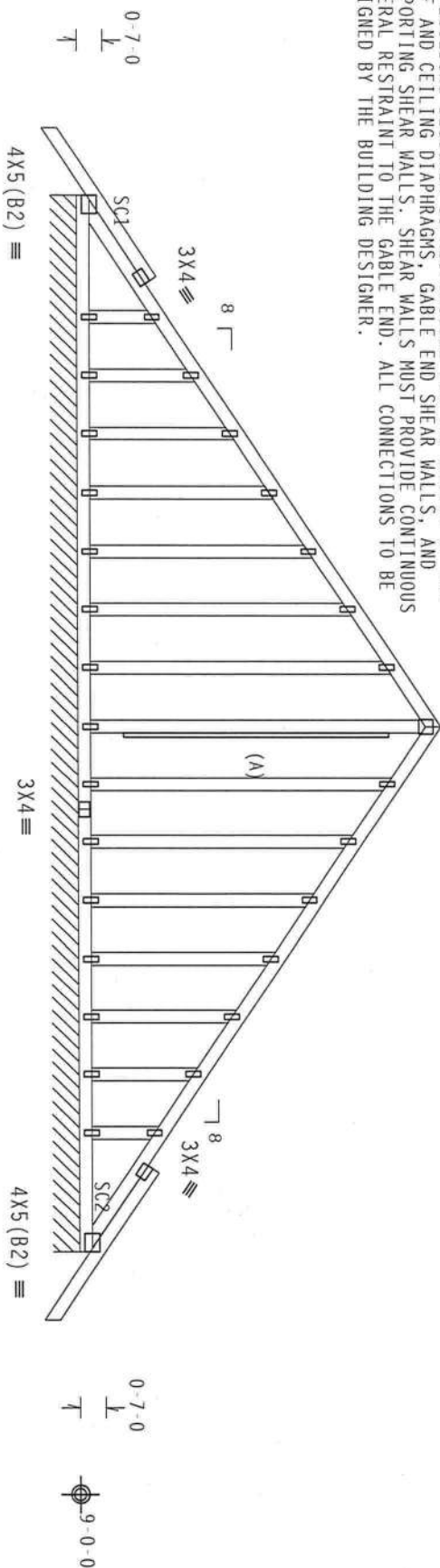
In lieu of structural panels or rigid ceiling use purlins to brace

TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase

factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



—4VC

$$4 \times 5 (B2) \equiv$$

8-0-1

20-2-0

1

3-6-8(NN)
1-10 305181

24-2-0 Over Continuous Support

R=136 PLF U=26 PLF W=24-2-0

RL=15/-15 PLF

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TP1-2002(STD)

PLT TYP. Wave

$$FT/RT=20\%(0\%)/10(0)$$

9.02.00

QTY:1

FL-4-R-

Scale = .25" / ft.

WARNING: THESE RIGID EXTERIOR CASES IN FABRICATION, HANDLING, OR SHIPPING, INSTALLING, AND REMOVING REFER TO ONLY ONE (OR MORE) COMPONENT OF THE SAFETY INFORMATION. PUBLISHED BY THE FIBROSS PLASTIC INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (6000) TRUSS COMPANY OF AMERICA, 6300 ENTERPRISE LANE, MORTON, WI 53159 FOR SALE PRACTICES AND VICES TO PERFORMING THESE FUNCTIONS. IF THE INFORMATION INDICATED FOR CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING.

****IMPORTANT***FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

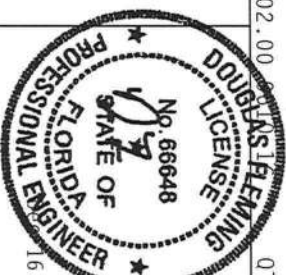
TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TP1.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W./H./SS./K.) ASTM A653 GRADE 40/60 (W./K./H./SS.) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2

ANY INSPECTION OF LATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IPI/2002 SEC.3. A SEAL ON THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROSS COMPONENT OF THE SUBSTANTIAL AND USE OF THIS ENGINEERING FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

OF STAIN STAINING. THE SOLIDITY AND USE OF THIS INFORMATION FOR THE DETERMINATION OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group Inc.
Haines City, FL 33844
FL 888.70.0278



TC LL	20.0 PSF	REF	R8228- 48889
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350024
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67017
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JRFF-	1TXN8228Z02

Wind reactions based on MMFRS pressures.

MFERS loads based on trusses located at least 20.19 ft. from roof edge.

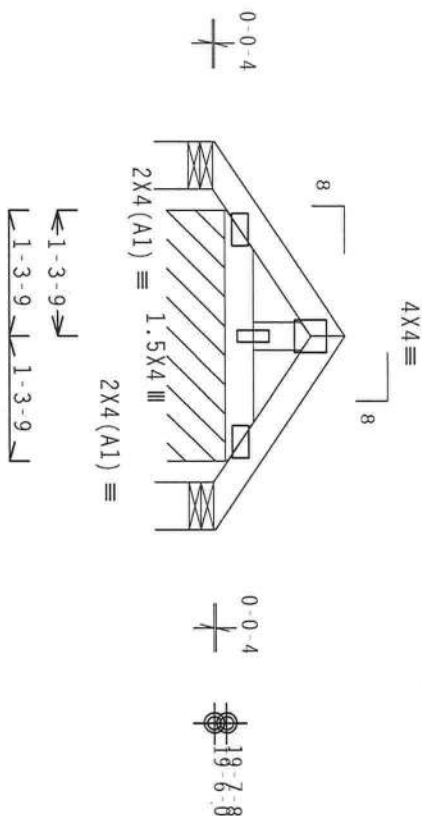
Refer to DWG PB1200109 for piggyback details.

Special loads

-----	(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From	64 pif at -0.71 to 64 pif at 1.30
TC - From	64 pif at 1.30 to 64 pif at 3.31
BC - From	4 pif at -0.71 to 4 pif at 3.31

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



R=16 RW=22 U=21 W=5.935"
RL=41/-41

R=81 PLF U=18 PLF W=2-7-2

R=16 U=6 W=5.935"

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

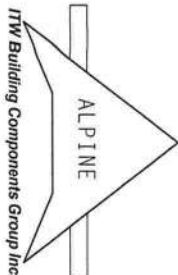
$$\overline{FT/RT} = 20\% (0\%) / 10 (0)$$

9.02.00

QTY: 5

$$FL/ - / 4 / - / - / R / -$$

Scale = .5" / ft.

[illegible]

16.09

TC LL	20.0 PSF	REF	R8228- 48892
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350017
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEON-	67049
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TYMR228202

Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Stack Chord SC1 2x4 SP #2 Dense:

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

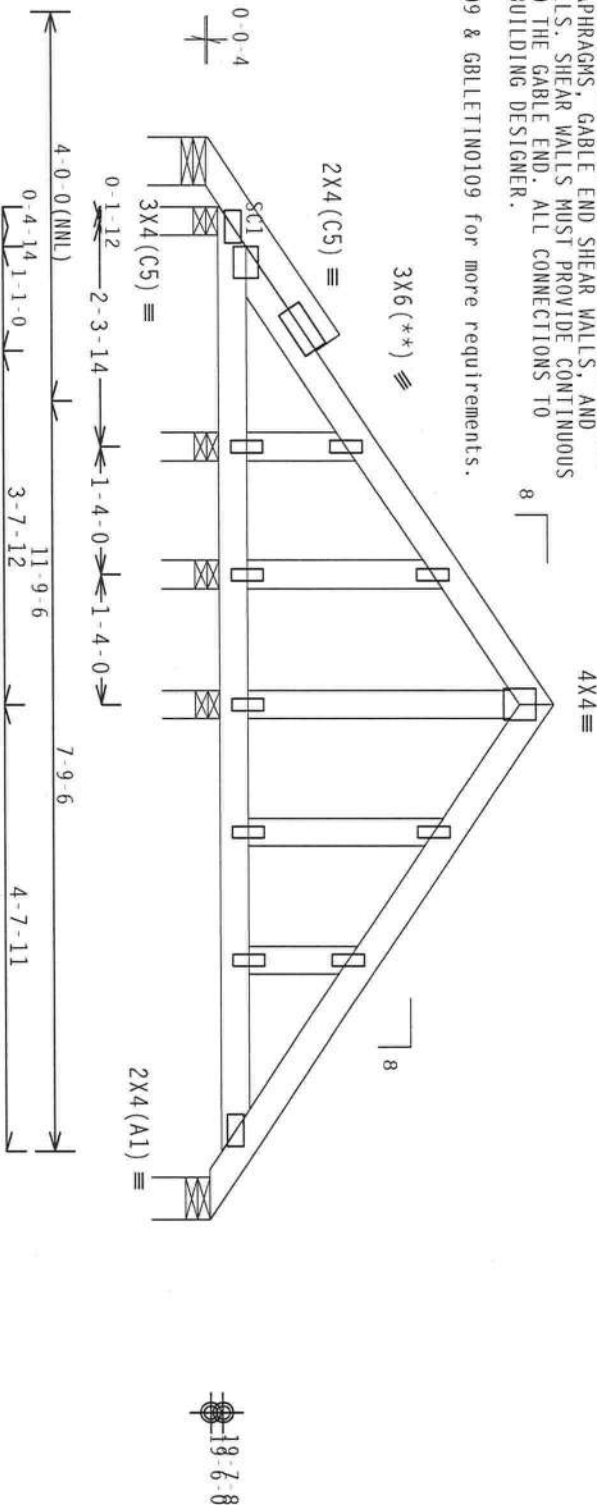
Wind reactions based on MMFRS pressures.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

See DWGS A11030050109 & GBLTET10109 for more requirements.



R=6 Rw=67 U=64 W=5.935"
Rl=129/-124

R=32 Rw=98 U=85 W=3.5"
R=166 U=107 W=3.5" R=779 U=299 W=3.5"
R=16 Rw=76 U=80 W=3.5"

R=53 U=19 W=5.313"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0)

9.02.00

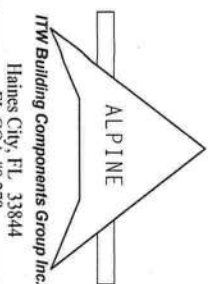
QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES BY ALPINE AND TPI. DESIGN CONFORMS WITH ANGLE CONNECTIONS (W/ST/2) AND W/ST/2 (ASTM A573) GRADE 40/60 (40, K70, S5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK AS OF TPII-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Haines City, FL 33844

FL 000000278



TC LL	20.0 PSF	REF	R8228-48893
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350026
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	67410
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 21.30 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Refer to DWG PB1200109 for piggyback details.

See DWGS A11030050109 & GBLETTIN0109 for more requirements.

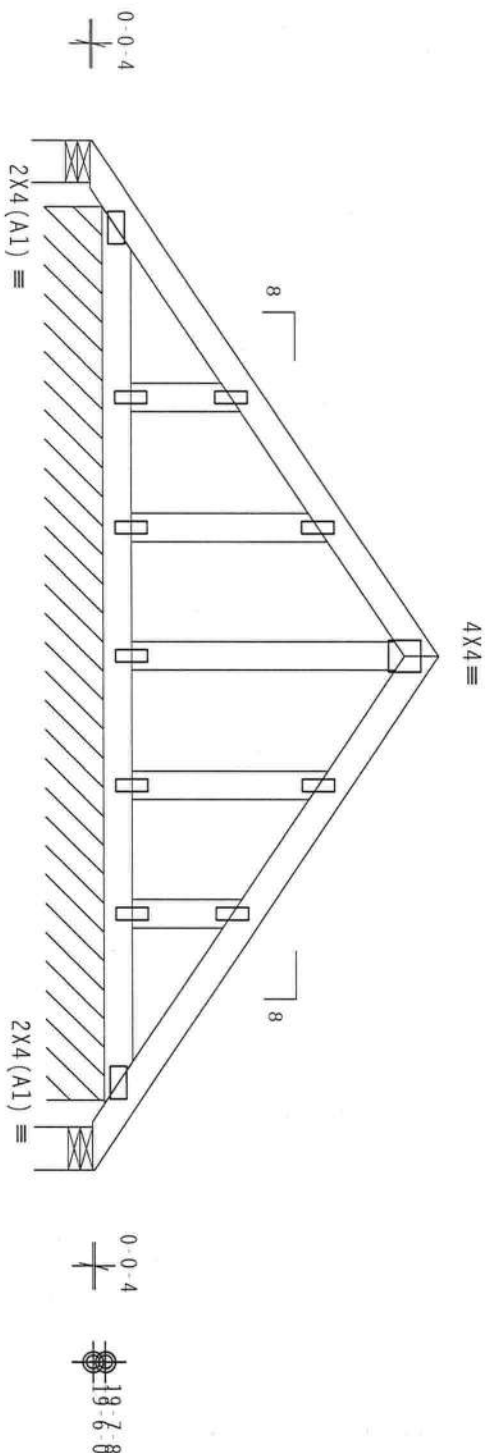
Special loads

Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25
TC - From 64 p1f at -0.68 to 64 p1f at 4.64
TC - From 64 p1f at 4.64 to 64 p1f at 9.97
BC - From 4 p1f at -0.68 to 4 p1f at 9.97

Truss spaced at 24.0" OC designed to support 1-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE
ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND
SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS
LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE
DESIGNED BY THE BUILDING DESIGNER.



$R=7$ $Rw=68$ $U=67$ $W=5.313''$
 $RL=117/-117$

10-7-13 Over 3 Supports

$R=34$ $Rw=5$ $U=4$ $W=5.313''$

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

$$\overline{FT/RT} = 20\% (0\%) / 10 (0)$$

9.02.00

QTY:1

FL/-/4/-/-/R/-/

Scale = .5"/Ft.

[illegible]

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IIR BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY OVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE BUSES IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF PROSSES. IIR BCGS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NIOS (NATIONAL DESIGN SPEC., BY AIAA) AND TPI.

CONTRACTOR FOR PLATES. THE DATE FOR THE FIRST SET OF PLATES TO EACH PAGE OF THE DRAWING SHALL BE THE DATE OF THE FIRST SET OF PLATES TO EACH PAGE OF THE DRAWING. THE DATE FOR THE FIRST SET OF PLATES TO EACH PAGE OF THE DRAWING SHALL BE THE DATE OF THE FIRST SET OF PLATES TO EACH PAGE OF THE DRAWING.



TC LL	20.0 PSF	REF	R8228- 48894
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350027
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67442
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228Z02

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Web	2x4	SP	#3	

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

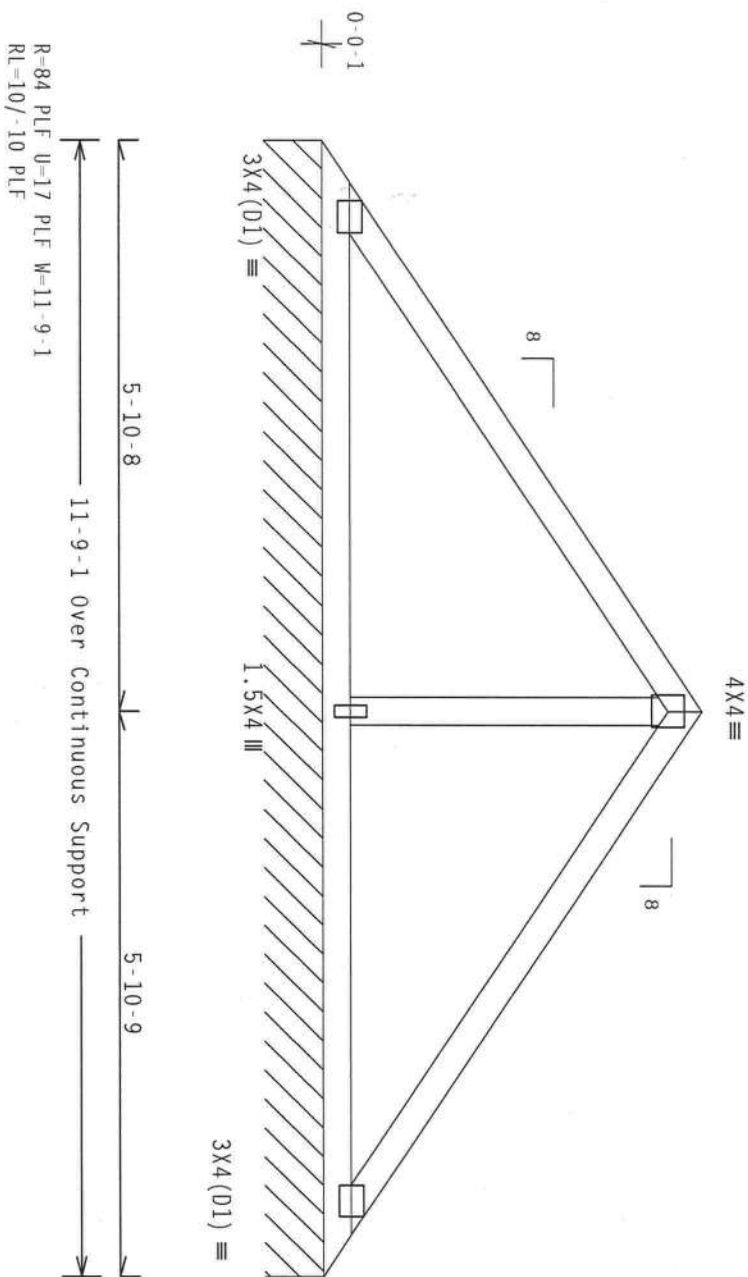
MUFRS loads based on trusses located at least 9.11 ft. from roof edge.

110 mph wind, 18.23 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1w=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

See DWG VAL1300109 for valley details.



16-1-7

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$FT/RT=20\%(0\%)/10(0)$$

9.02.00.00 AS FILE QTY:1

QTY:1

FL/-/4/-/-/R/-

Scale = .5" / Ft.

WARNING:—FIBERS, RIGID EXHIBIT CASE IN FABRICATION, HANDLING, DRIPPING, INSTALLING AND BRACING REFER TO DCST (ROUTED THROUGH EXISTING CEILING FOR REMOVAL), PUBLISHED BY THE FIBER GLASS PANEL INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 UNIVERSITY LANE, SUITE 500, WILSON, NC 27157) FOR SAFETY PRACTICES PERTAINING TO THE USE OF FIBERS. UNLESS OTHERWISE INDICATED, THE ABOVE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 278



TC LL	20.0 PSF	REF	R8228- 48897
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350006
BC LL	0.0 PSF	HC-ENG	JB/DF *
TOT.LD.	40.0 PSF	SEON-	66987
DUR.FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXR8228202

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	Wabs	2x4	SP	#3	

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

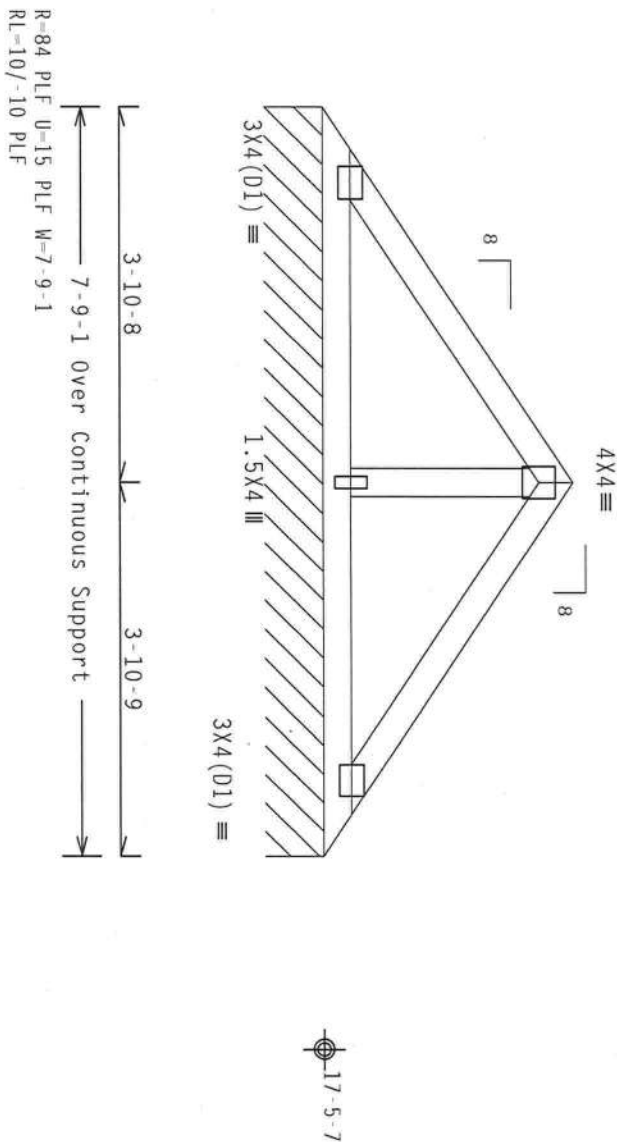
MMFRS loads based on trusses located at least 9.45 ft. from roof edge.

110 mph wind, 18.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DE=5.0 psf, wind BC DE=5.0 psf lw=1.00 gcpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non concurrent live load.

See DWG VAL1300109 for valley details.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$\overline{FT/RT} = 20\% (0\%) / 10 (0)$$

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .5" / ft.

WARNING: THESE BUILDING COMPONENTS ARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND DRIPPING. REFER TO BEST (800) DUE TO COMPONENT SAFETY (E-180-001-001), PUBLISHED BY THE FIBERS PASTE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 OR (800) TRUSS CONSULT OR (703) 6500 6200. ENTERPRISE LAM, 5075 312, 523719 FOR SAFETY PRACTICES AND PLEASE TO PERFORM THE USE OF FIREWORKS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 278



16.09

SPACING 24.0"

JRFF - 1TXN8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to
brace TC @ 24" OC, BC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

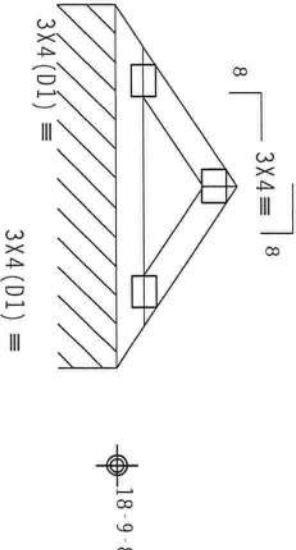
See DWG VAL1300109 for valley details.

110 mph wind, 19.56 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=5.0 psf, lw=1.00 GCFI(+/-)-0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

MMFRS loads based on trusses located at least 19.56 ft. from roof
edge.



R=83 PLF U=2 PLF W=3-9-1
RL=8/-8 PLF

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

9.02.00

QTY:1 FL/-/4/-/-/R/-

Scale =.5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE
NTW Building Components Group Inc.
Haines City, FL 33844
FL 000000278



TC LL	20.0 PSF	REF	R8228- 48899
TC DL	10.0 PSF	DATE	12/16/09
BC DL	10.0 PSF	DRW	HCUSR8228 09350018
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT. LD.	40.0 PSF	SEQN-	66994
DUR. FAC.	1.25	FROM	GA
SPACING	24.0"	JREF-	1TXN8228202

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

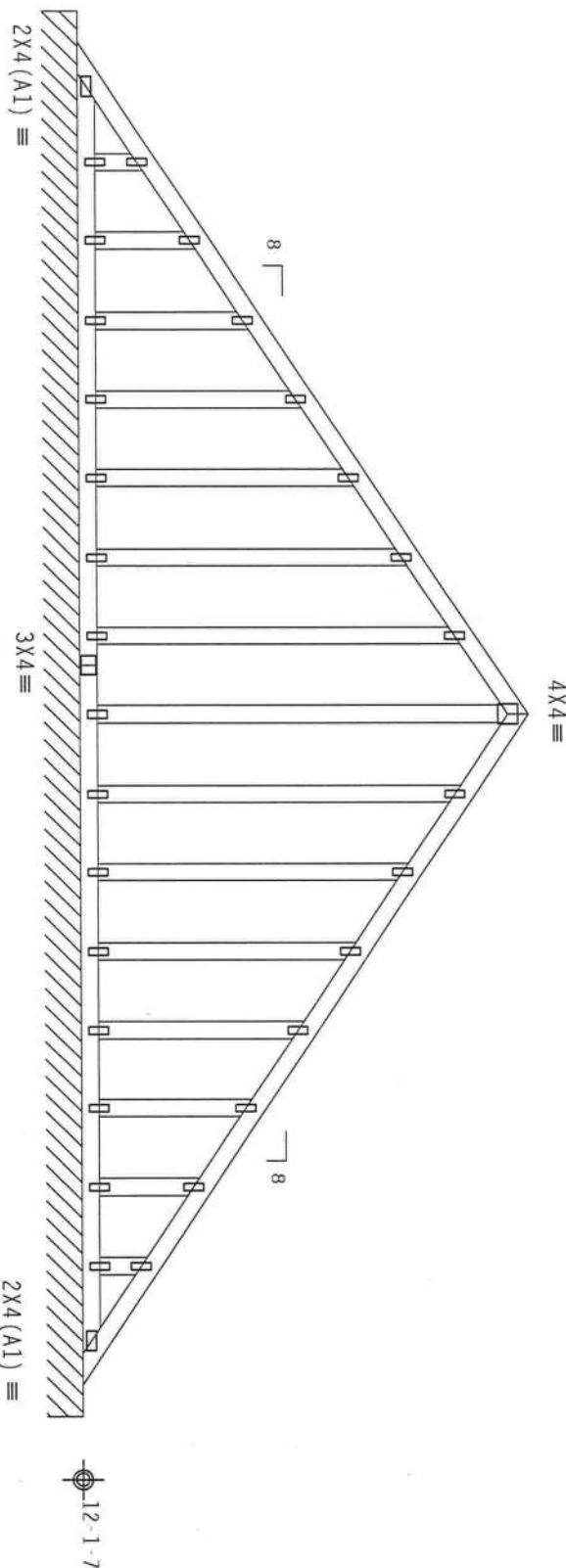
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.91 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=20%(0%)/10(0)

$$FT/RT=20\%(0\%)/10(0)$$

9.02.00

QTY: 1

$$\text{FL} / - / 4 / - / - / \text{R} / -$$

Scale = .3125" / ft.

"WARNING"—THEY'RE BLOWING THE HOUSE CAVE IN. FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO SC-1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PTI (TERRACE PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WPCA (WOOD TRUSS COUNCIL OF AMERICA), 63000 CREEPSTEEL LANE, MADISON, WI 53713) FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, FOR CHORD SHAFTS HAVE PROPERLY ATTACHED STRUCTURAL PARTS, AND BOTTOM CHORD SHAFTS HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL 0000000278



60

SPACING 24.0"

JREF- 1TXN8228Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

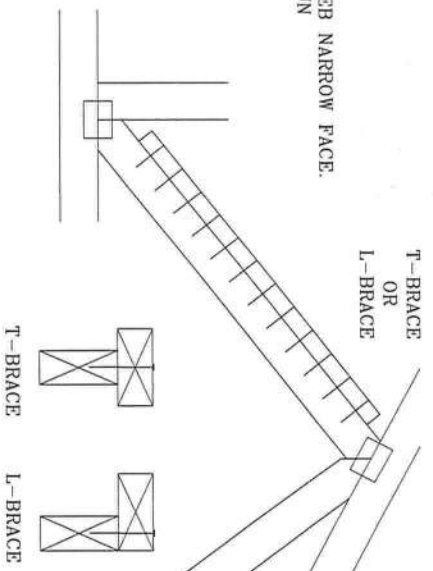
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

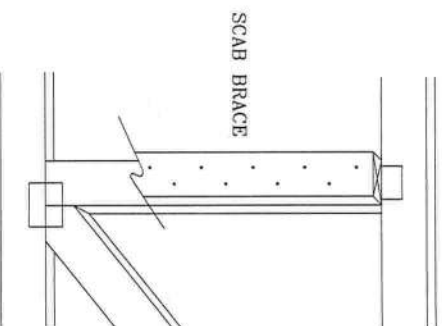
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3", MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3", MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow these instructions. Compliance with these instructions is required by the Building Code of America (BCA) for safety practices prior to performing any truss work. Truss installers shall provide temporary bracing per BCA. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCA sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

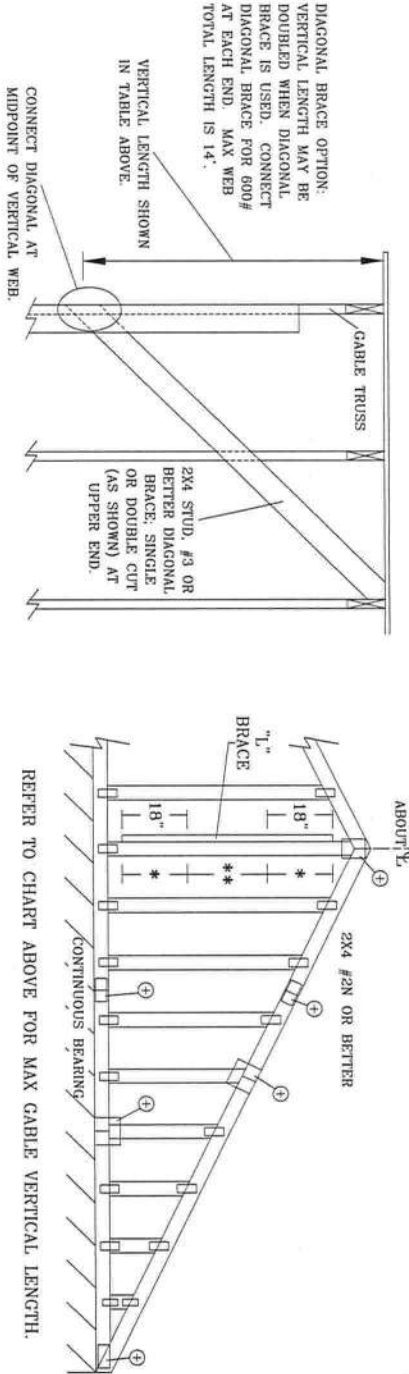
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPL or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W/H/S/N) ASTM A653 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the building designer per ANSI Z99.1. See www.bchindustry.com. IFC: www.local6.org ITW-BCG: www.itwbcg.com; TPL: www.tpl.com; BTCA: www.bchindustry.com; IFC: www.local6.org



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCBLSUB0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

MAX GABLE VERTICAL LENGTH

GABLE VERTICAL SPECIES	BRACE	NO	(1) 1X4 "L" BRACE •		(1) 2X4 "L" BRACE •		(2) 2X4 "L" BRACE •		(1) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE •	
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
2X4	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"
	STUD	#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"
	HF	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	10' 7"	12' 3"	12' 3"	14' 0"
	SP	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"
2X4	SP	#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"
	STUD	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"
	DFL	STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"
	SPF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"
2X4	SPF	#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"
	STUD	STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"
	HF	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"
	SP	#2	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"
2X4	DFL	STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"
	SPF	#1 / #2	4' 11"	8' 5"	8' 5"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"
	STUD	#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"
2X4	SP	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"
	STUD	#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"
	DFL	STANDARD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"



GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

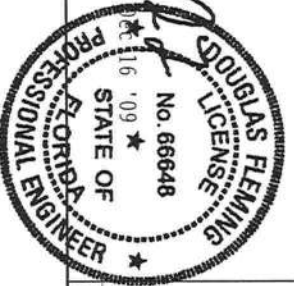


Building Components Group Inc.

Earth City, MO 63045

WARNING: READ AND FOLLOW ALL NOTES ON THIS SHEET. TRUSSES require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow these guidelines. Connectors shall be installed in accordance with the manufacturer's instructions. All bracing shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W/A/S/N) ASTM A656 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. This seal is not to be used for any other building or truss component. ITWBCG: www.itwbcg.com; TPI: www.tpi.com; WTCI: www.abendustry.com; ICC: www.iccsafe.org



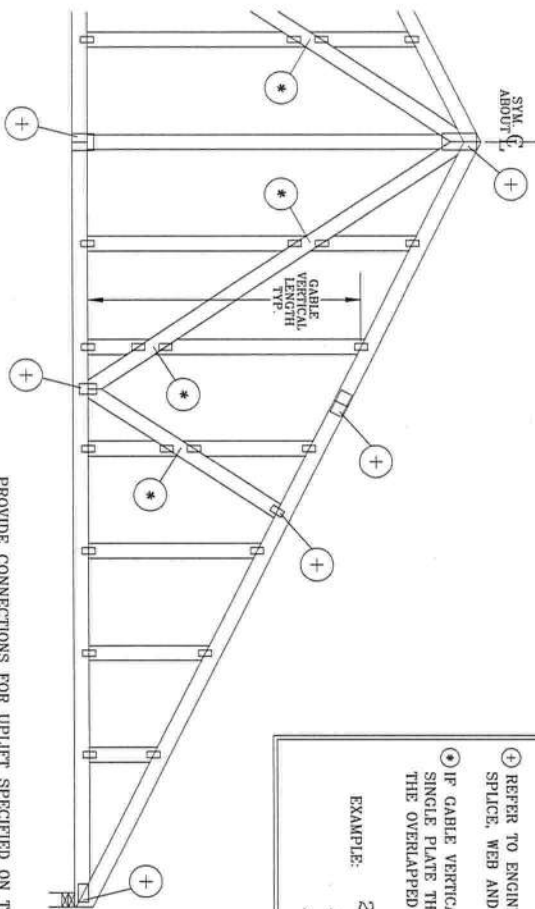
REF	ASC7-05-CAB11015
DATE	1/1/09
DRWG	A11015050109
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.
PROVIDE UPLIFT CONNECTIONS FOR 60 PSF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" IN 18' END ZONES AND 4' 0" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" IN 18' END ZONES AND 6' 0" O.C. BETWEEN ZONES.
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-FIR	HEM-FIR	SPRUCE-PINE-FIR	HEM-FIR
#1 / #2 STUD	#1 / #2 STUD	#1 / #2 STUD	#1 / #2 STUD
#3 STUD	#3 STUD	#3 STUD	#3 STUD
STANDARD	STANDARD	STANDARD	STANDARD

GABLE DETAIL FOR LET-IN VERTICALS



GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

⊙ IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

END DRIVEN NAILS:

- 10d COMMON (0.148" X 3.125") NAILS AT 4" O.C. PLUS
- (4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:

- 10d COMMON (0.148" X 3.125") TOENAILS AT 4" O.C. PLUS
- (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE

WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015980109, A12015980109, A10015980109, A13030980109, A12030980109, A11030980109

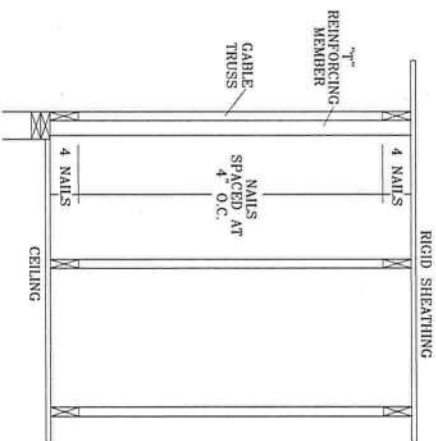
ASCE 7-02 GABLE DETAIL DRAWINGS

A13015020109, A12015020109, A11015020109, A10015020109, A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015050109, A12015050109, A11015050109, A10015050109, A13030050109, A12030050109, A11030050109, A10030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.



****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**

Trusses require extreme care in design, fabrication, bracing, and installation. Refer to and follow ITW Building Components Group Inc. (ITWBCG) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCIS. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCIS sections B3 & B7. See this job's general notes page for more information.

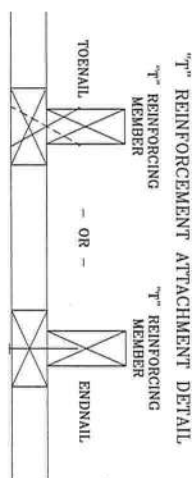
****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W/H/S/N) ASTM A563 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. This design is for use of this component for any building in the United States. ITWBCG is not responsible for the use of this component for any building in the United States. ITW-BCG: www.itwbcg.com, TPI: www.tpi.com, WTC: www.wtcindustry.com, ICC: www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045



TO CONVERT FROM "T" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MHR	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT, K_z = 1.00

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "T" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"

REF LET-IN VERT

DATE 1/1/09

DRWG GILLETINO109

MAX TOT. LD. 60 PSF

MAX SPACING 24.0"

MAX SPACING 24.0"



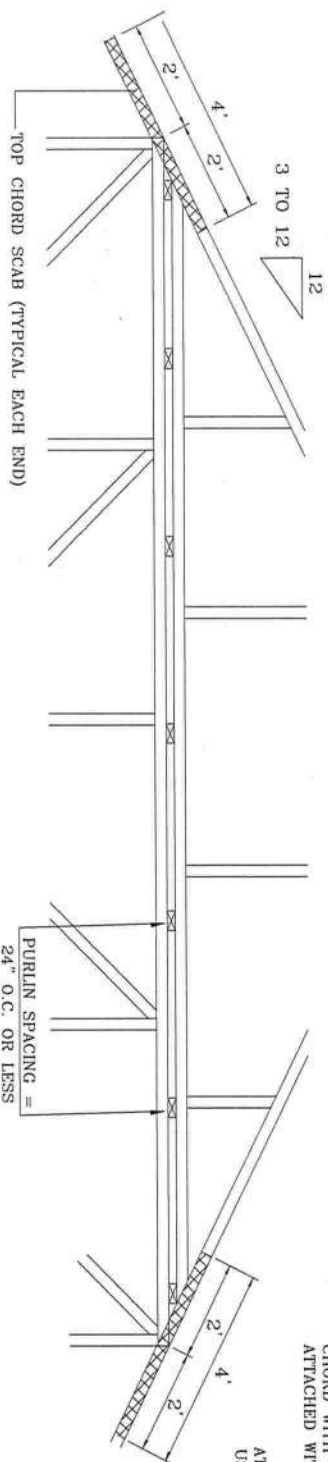
120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZT=1.0.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

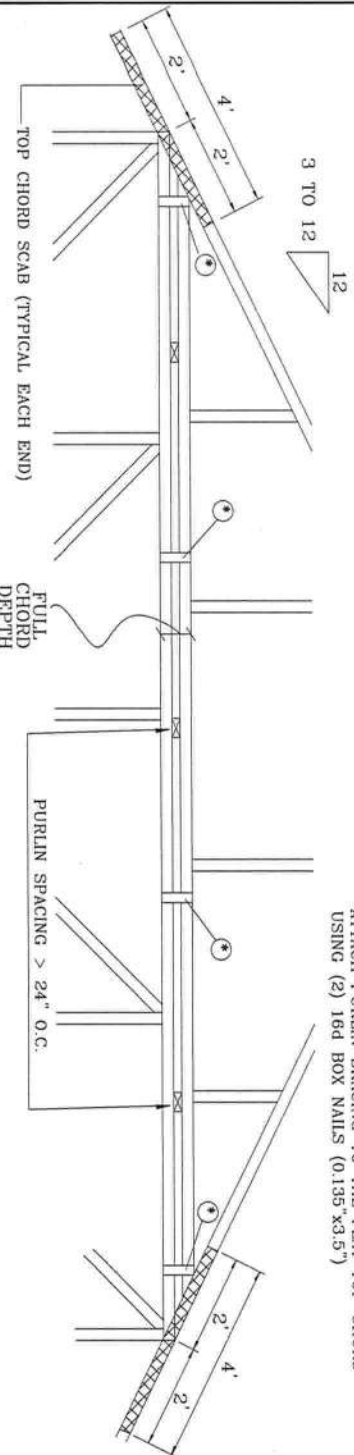
DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4' O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

DETAIL B : PURLIN SPACING > 24" O.C.



PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4' O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

- TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8' O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.
- PLYWOOD GUSSET
8"x8"x1/2" RATED SHEATHING GUSSETS (EACH FACE) ATTACHED @ 8' O.C. WITH (4) 0.113"x2" NAILS PER GUSSET. IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.
- 2x4 VERTICAL SCABS
2x4 SP#2, FULL CHORD DEPTH SCABS @ 8' O.C. EACH FACE, STAGGERED 4' O.C. ATTACH WITH (3) 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB).
- 28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8' O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION, ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLATE. PIGGYBACK PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.



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NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

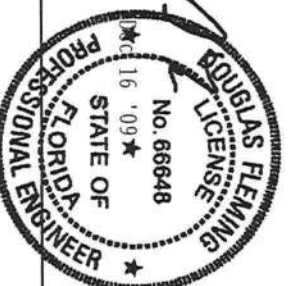
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTA) safety procedures regarding the use of trusses. Trusses shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

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ITW-BCG: www.itwbcg.com; TPI: www.tpi.net; WTA: www.wtaindustry.com; ICC: www.iccsafe.org



REF	PIGGYBACK
DATE	10/01/09
DRWG	PB1201009
SPACING	24.0"

ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL

MAX GABLE VERTICAL LENGTH																
GABLE VERTICAL SPACING	2X4 BRACE SPECIES	GRADE	NO BRACES	(1) 1X4 "L" BRACE •		(1) 2X4 "L" BRACE •		(2) 2X4 "L" BRACE •		(1) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE •		(2) 2X6 "L" BRACE •		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
12" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"	14' 0"	14' 0"	
	SP	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	13' 3"	14' 0"	14' 0"	14' 0"	
	16" O.C.	SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
HF		STUD	4' 1"	5' 8"	5' 8"	7' 6"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
SP		#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
24" O.C.		SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	4' 1"	5' 8"	5' 8"	7' 6"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	12" O.C.	SPF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 5"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
HF		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
SP		#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
STANDARD		STUD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	

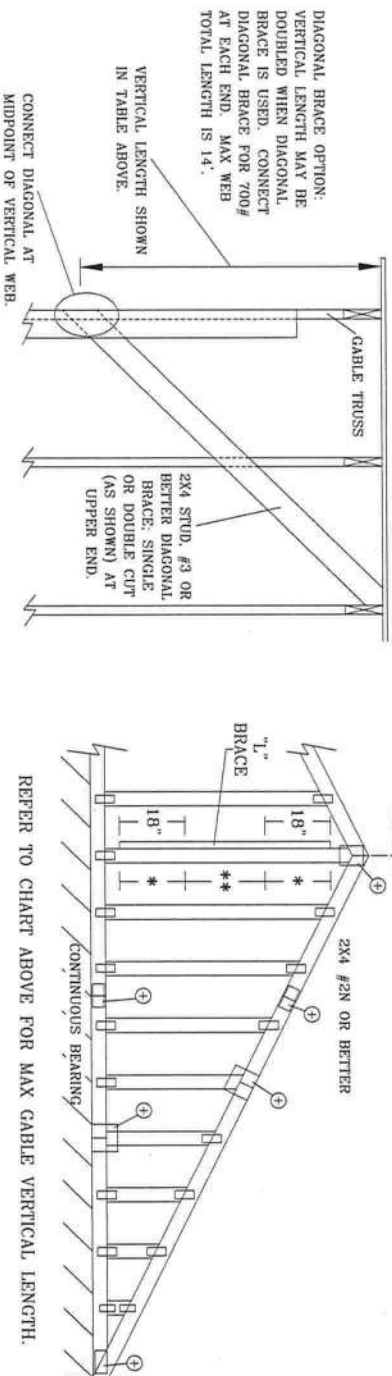
GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 100 PSF OVER
 CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
 GABLE END SUPPORTS LOAD FROM 4' 0"
 OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"
 PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
 * FOR (1) "L" BRACE: SPACE NAILS AT 2' 0".
 IN 18" END ZONES AND 4' 0" O.C. BETWEEN ZONES.
 ** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0".
 IN 18" END ZONES AND 6' 0" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 80% OF WEB
 MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR
 PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

DIAGONAL BRACE OPTION:
 VERTICAL LENGTH MAY BE
 DOUBLED WHEN DIAGONAL
 BRACE IS USED. CONNECT
 DIAGONAL BRACE FOR 700#
 AT EACH END. MAX WEB
 TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
 IN TABLE ABOVE.

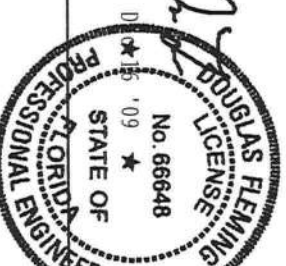
CONNECT DIAGONAL AT
 MIDPOINT OF VERTICAL WEB.



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 shall have properly attached structural panels and bottom chord shall have a properly attached rigid
 ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI
 sections B3 & B7. See this job's general notes page for more information.
 IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
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 any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing &
 (K/M/H/S) giv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
 A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely
 for the truss component design shown. This seal and use of this component for any building is the
 responsibility of the truss designer. Do not alter or modify the design without the written consent of
 BCCG. www.bccg.com, TPI: www.tpi.net, WTC: www.abendustry.com, ICC: www.iccsafe.org



MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

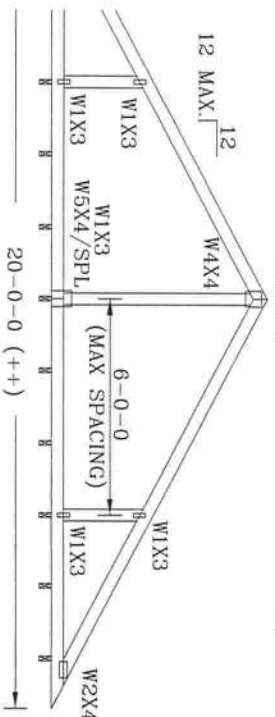
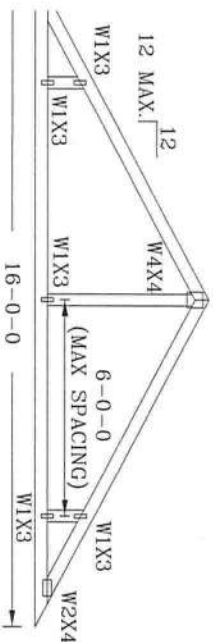
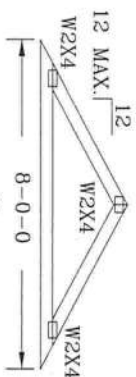
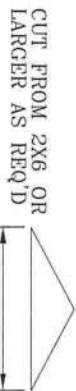
REF ASCE7-05 GAB1030
 DATE 1/1/09
 DRWG A11030050109

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2N, SPF #1/#2, DF-L #2 OR BETTER.
BOT CHORD 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #2N, SPF #1/#2, DF-L #2 OR BETTER.

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

- (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH, 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF, Kzt = 1.00



UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" O.C., OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'-9".

FOR VERTICALS OVER 10'-0" TALL, APPLY (2) 1x4 "T" BRACE, TO NARROW FACE, SAME GRADE AS WEB MEMBER, ATTACH WITH 8d OR 0.128"x3" GUN NAILS @6" O.C., STAGGERED

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

OR

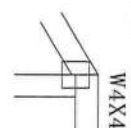
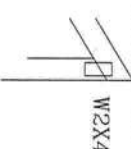
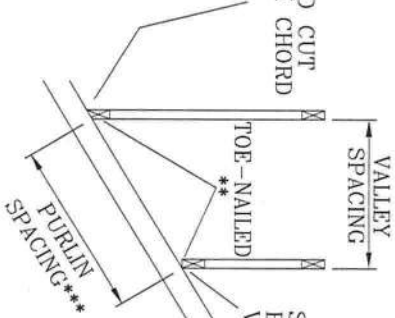
PURLINS AT 24" O.C. OR AS OTHERWISE SPECIFIED ON ENGINEER'S SEALED DESIGN OR

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEER'S SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 14'-0".

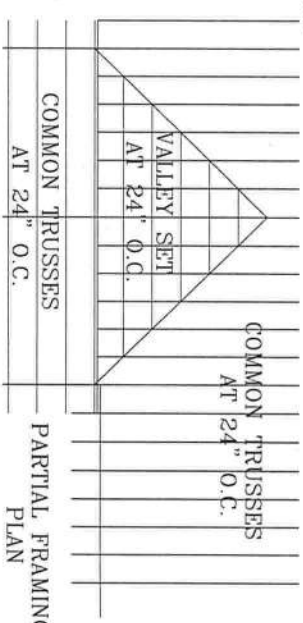
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SQUARE CUT BOTTOM CHORD VALLEY

STUBBED VALLEY END DETAIL

OPTIONAL HIP JOINT DETAIL



COMMON TRUSSES AT 24" O.C.

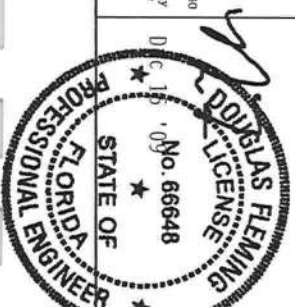
PARTIAL FRAMING PLAN



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. THIS TRUSS SYSTEM IS DESIGNED FOR USE IN CONFORMANCE WITH THE FOLLOWING: 1. IBC 2006 INTERNATIONAL BUILDING CODES (IBC) 2. ASCE 7-98 MINIMUM DESIGN LOADS AND CRITICAL WIND SPEEDS 3. AISC 136-03 STEEL ECTIONAL MEMBERS 4. AISC 360-03 STEEL STRUCTURAL DESIGN AND CONSTRUCTION 5. AISC 358-03 STEEL ECTIONAL CONNECTIONS 6. AISC 341-03 STEEL ECTIONAL MOMENT RESISTING JOINTS 7. AISC 308-03 STEEL ECTIONAL COMPRESSION MEMBERS 8. AISC 360-03 STEEL STRUCTURAL DESIGN AND CONSTRUCTION 9. AISC 358-03 STEEL ECTIONAL CONNECTIONS 10. AISC 341-03 STEEL ECTIONAL MOMENT RESISTING JOINTS 11. AISC 308-03 STEEL ECTIONAL COMPRESSION MEMBERS 12. AISC 360-03 STEEL STRUCTURAL DESIGN AND CONSTRUCTION 13. AISC 358-03 STEEL ECTIONAL CONNECTIONS 14. AISC 341-03 STEEL ECTIONAL MOMENT RESISTING JOINTS 15. AISC 308-03 STEEL ECTIONAL COMPRESSION MEMBERS 16. AISC 360-03 STEEL STRUCTURAL DESIGN AND CONSTRUCTION 17. AISC 358-03 STEEL ECTIONAL CONNECTIONS 18. 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TC LL	30	30	40 PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	1/1/09
BC DL	10	10	10 PSF	DRWG	VAL1300109
BC LL	0	0	0 PSF		
ROT. LD.	60	55	57 PSF		
BRPAC	1.25/1.33	1.15/1.15			
SPACING	24"				