

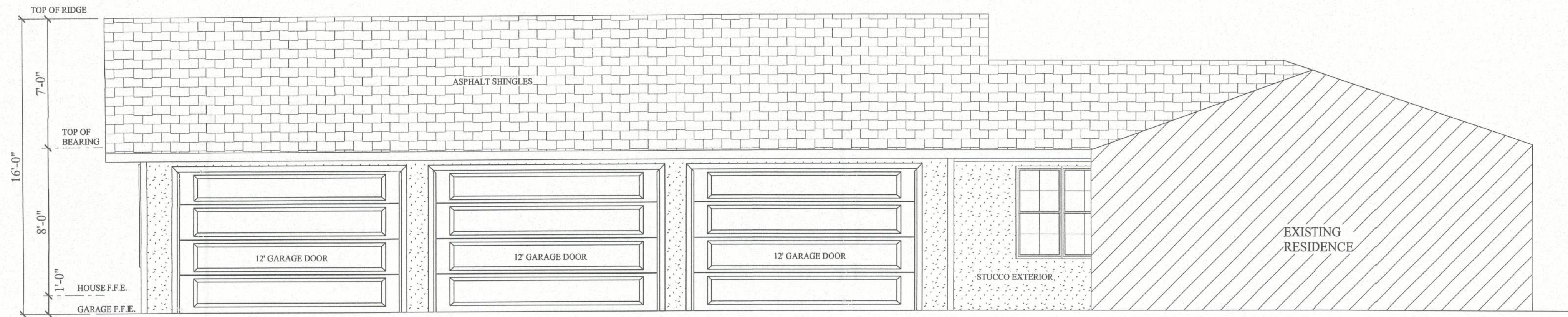
CONDITIONED AREA: 167 sq.ft.  
GARAGE AREA: 1,760 sq.ft.  
ENTRY: 134 sq.ft.

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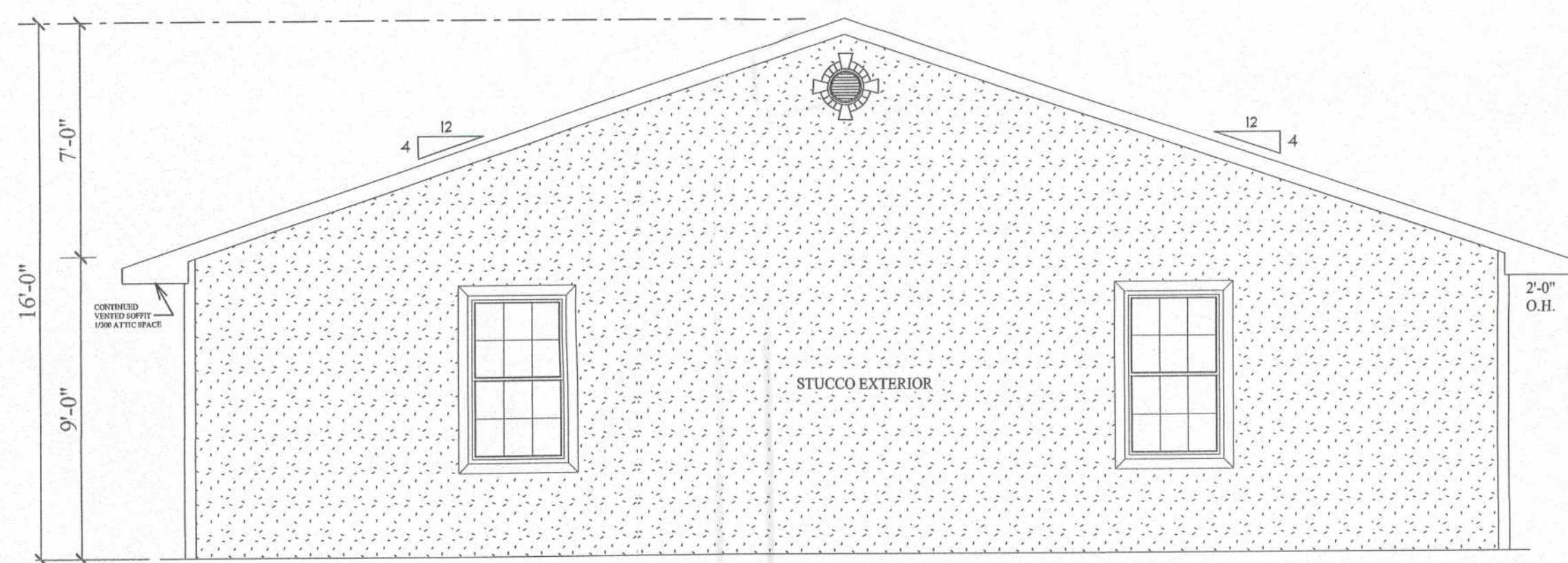
TOTAL AREA: 2,061 s.f.

A-1





**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"



**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"



**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"

REVISIONS:  
BUILD SET

212 SE HICKORY DR  
LAKE CITY, FLORIDA 32025  
386-752-7578  
WWW.ONEALCOMPANIES.COM

**O'NEAL COMPANIES**  
O'NEAL CONTRACTING INC. CBC057550

**Mr. Rick Parker**  
COLUMBIA COUNTY, FLORIDA  
**ELEVATIONS**

DATE: JULY 19, 2013

DRAWN BY: MD

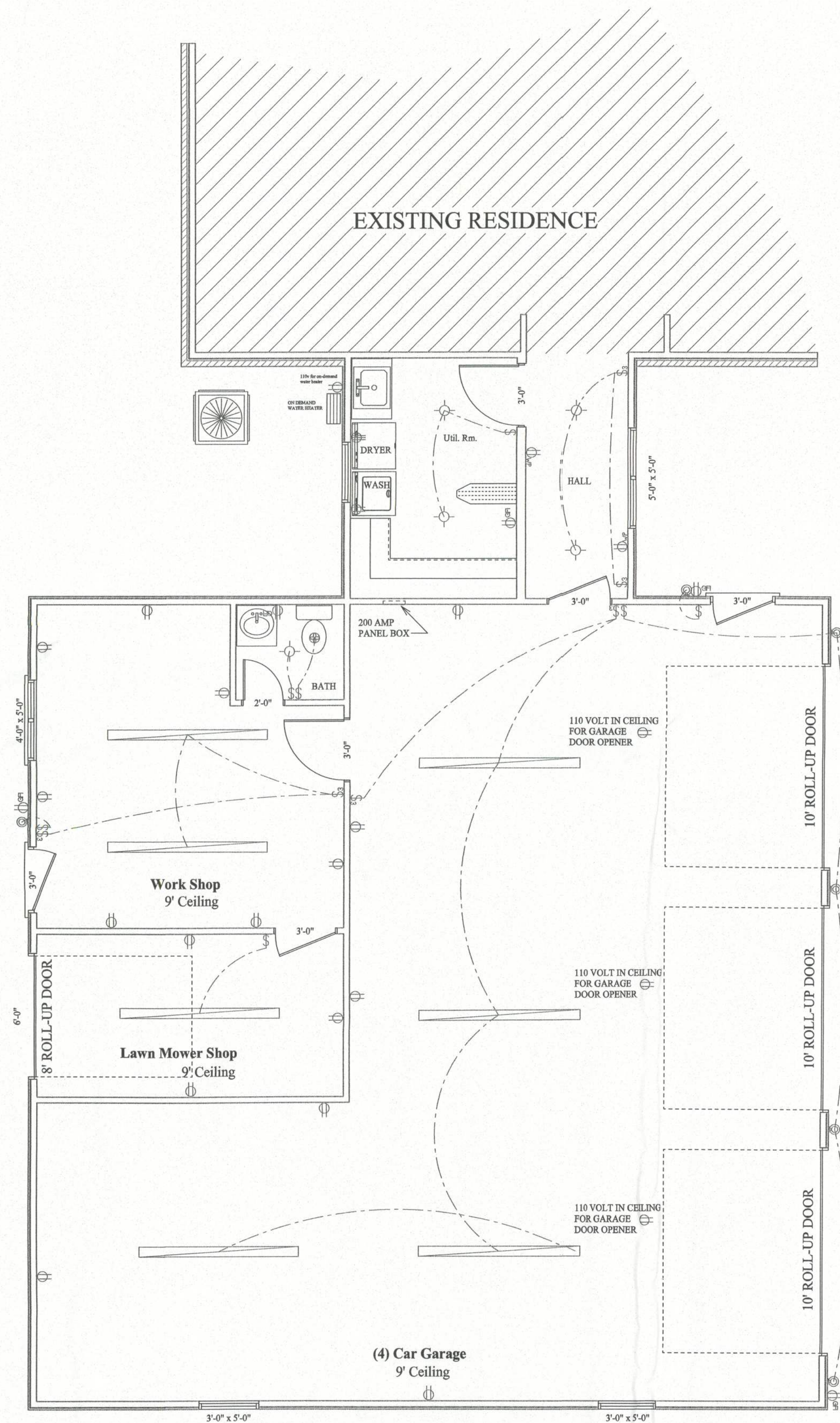
APPROVED BY: JGC

PROJECT NO:

SHEET:

**A-2**





Garage / Utility Rm. Electrical Plan

SCALE: 1/4" = 1'-0"

ELECTRICAL NOTES:

ELECTRICAL CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELECTRICAL PLAN, ADDITIONS TO THE ELECTRICAL PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CIRCUITS IDENTIFIED W/ CIRCUIT NUMBER, DESCRIPTION & BREAKER. SERVICE ENTRANCE & ALL UNDERGROUND WIRE LOCATIONS RISER DIAGRAM SHALL INCLUDE WIRE SIZES/TYPE AND EQUIPMENT TYPE W/RATINGS & LOADS. ALL WET LOCATIONS SHALL BE PROTECTED BY GFCI PROTECTION AND ALL BEDROOM RECEPITS SHALL BE PROTECTED BY AFCI PROTECTION.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DRAWINGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY. TELEPHONE, TELEVISION, AUDIO, SECURITY SYSTEM OR OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF THE NEC-LATEST EDITION.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER.

ALL BEDROOMS RECEPITS SHALL BE AFCI.

ELECTRICAL	COUNT	SYMBOL
fluorescent light 2 bulb 8'	7	
wall mount exterior light	6	
200 amp electrical panel	1	
exhaust fan w/light kit	1	
light	5	
outlet 110v	21	
outlet 220v	1	
outlet gfi 110v	5	
arc fault outlet 110v	2	
switch	7	
switch 3 way	6	

REVISIONS:  
BUILD SET

212 SE HICKORY DR  
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O'NEAL COMPANIES  
O'NEAL CONTRACTING INC. CBC057550

Mr. Rick Parker  
COLUMBIA COUNTY, FLORIDA  
ELECTRICAL PLAN

DATE: JULY 19, 2013

DRAWN BY: MD

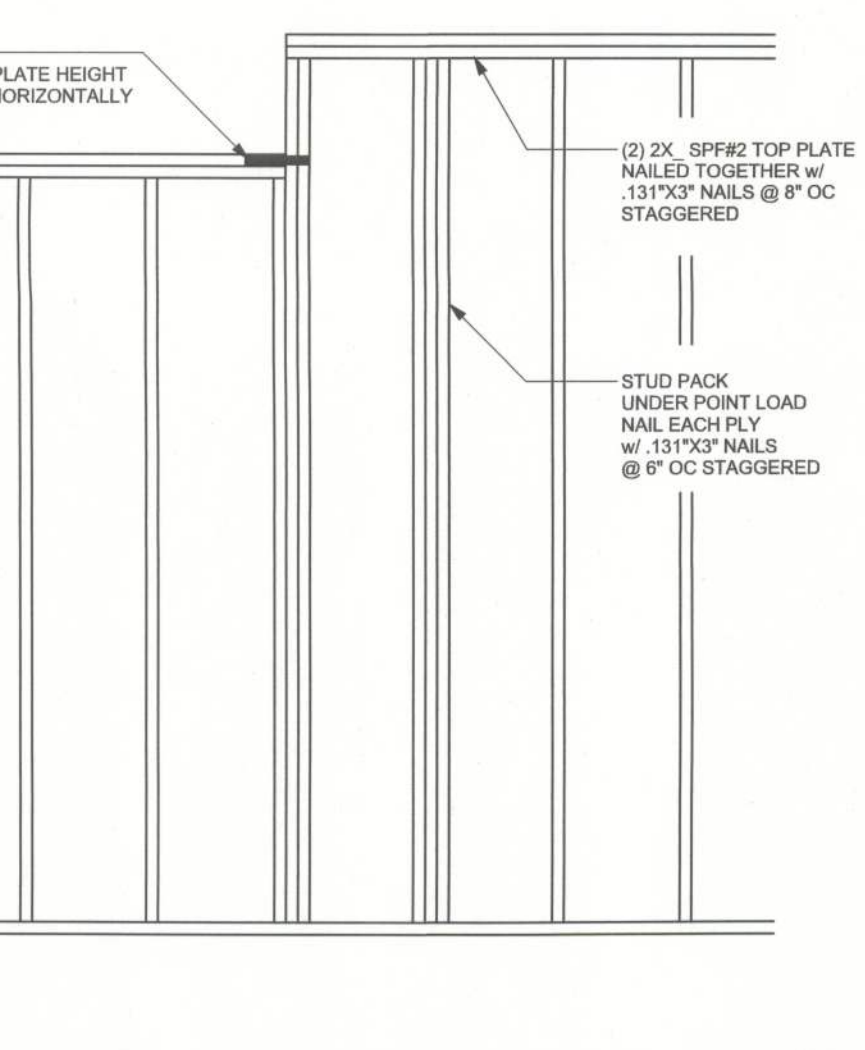
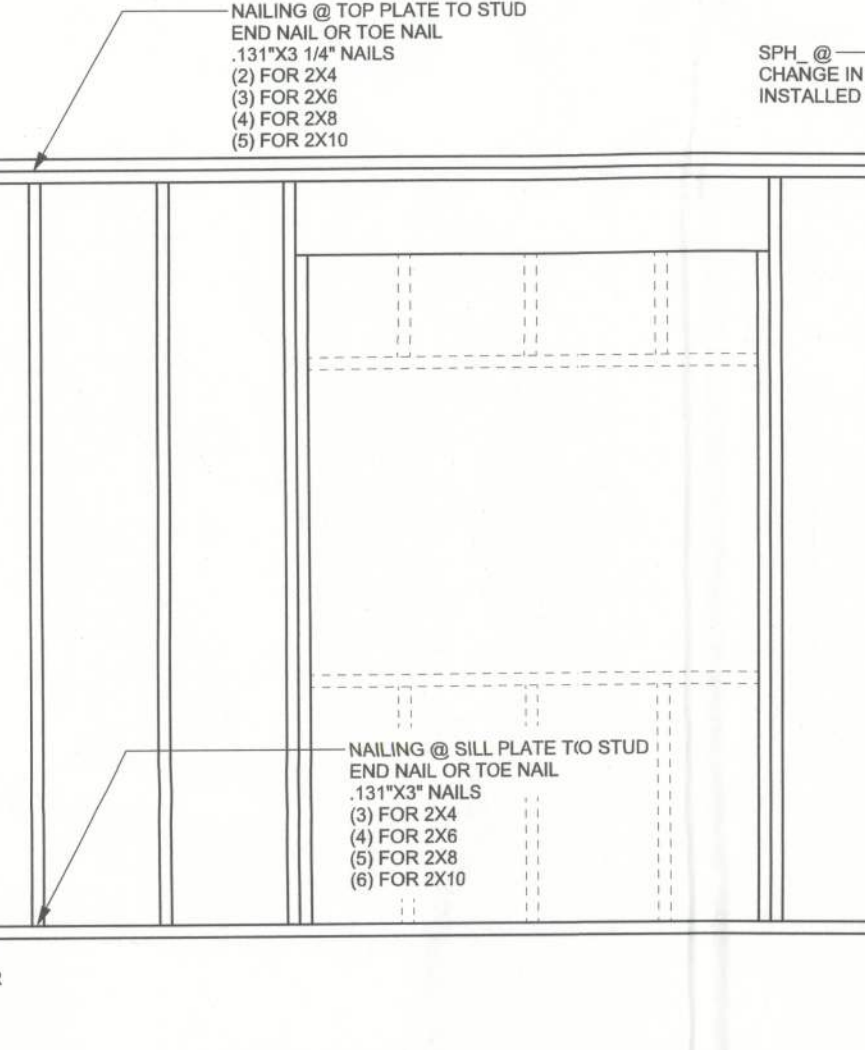
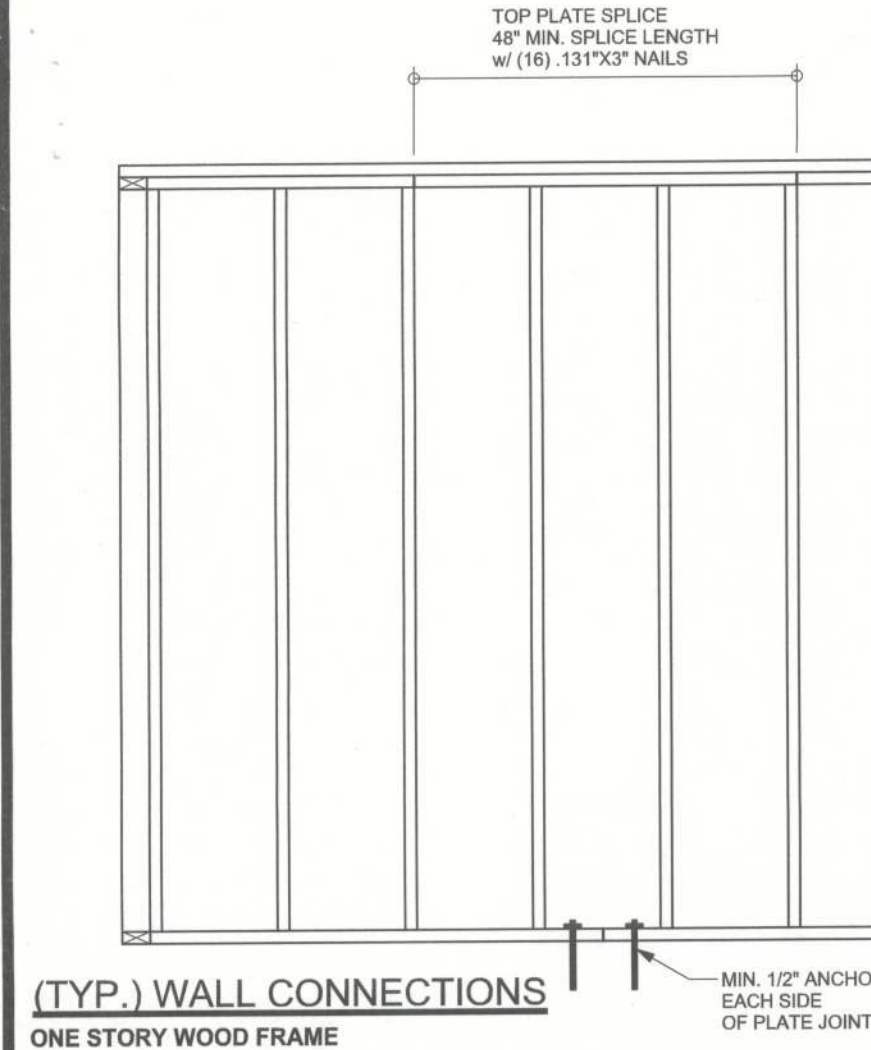
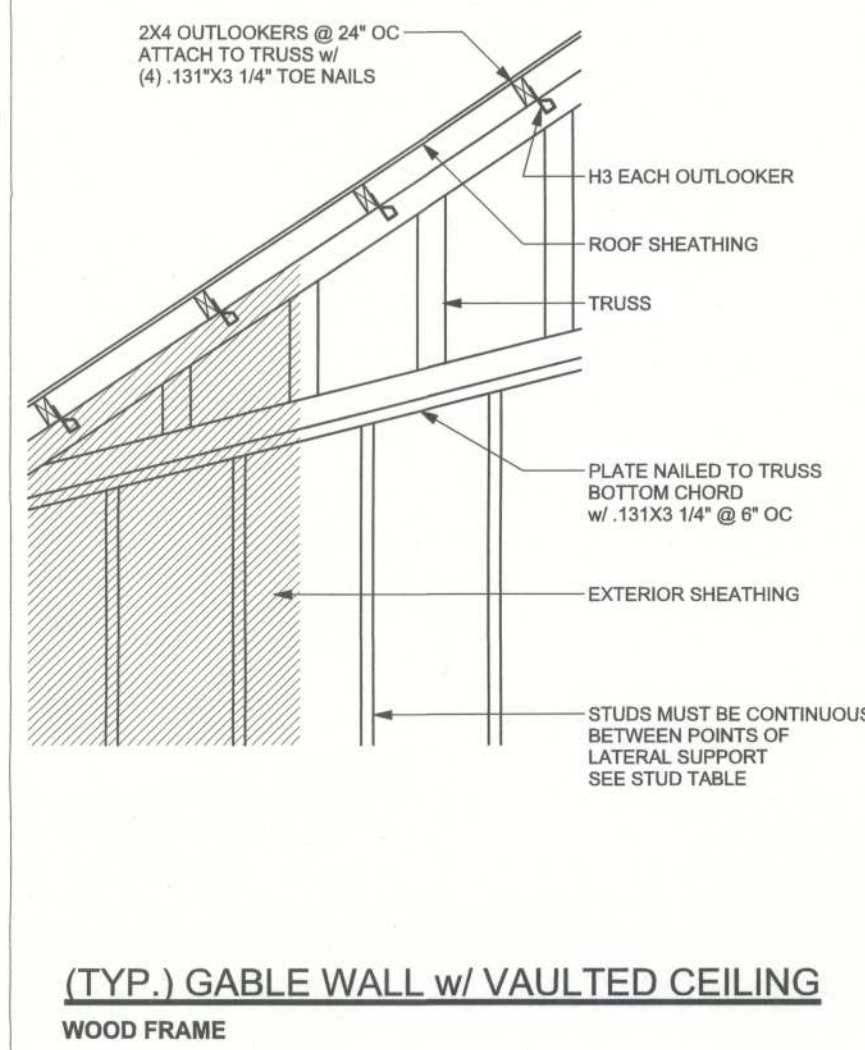
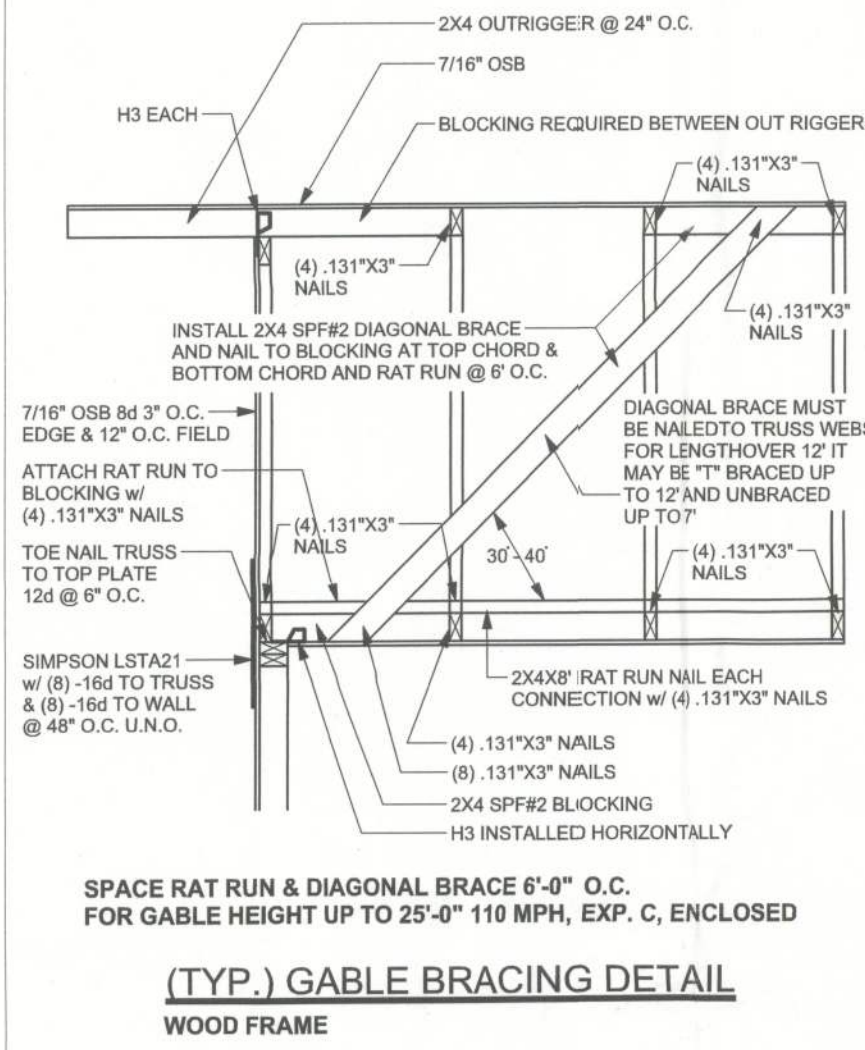
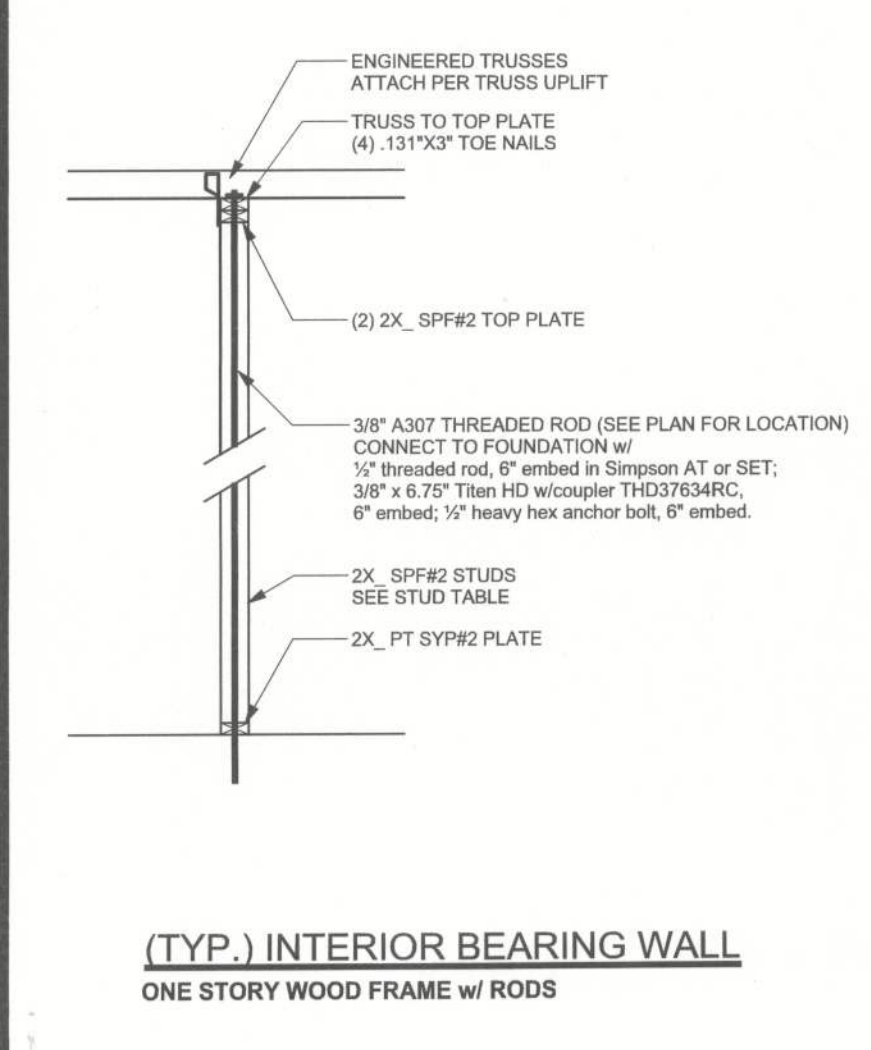
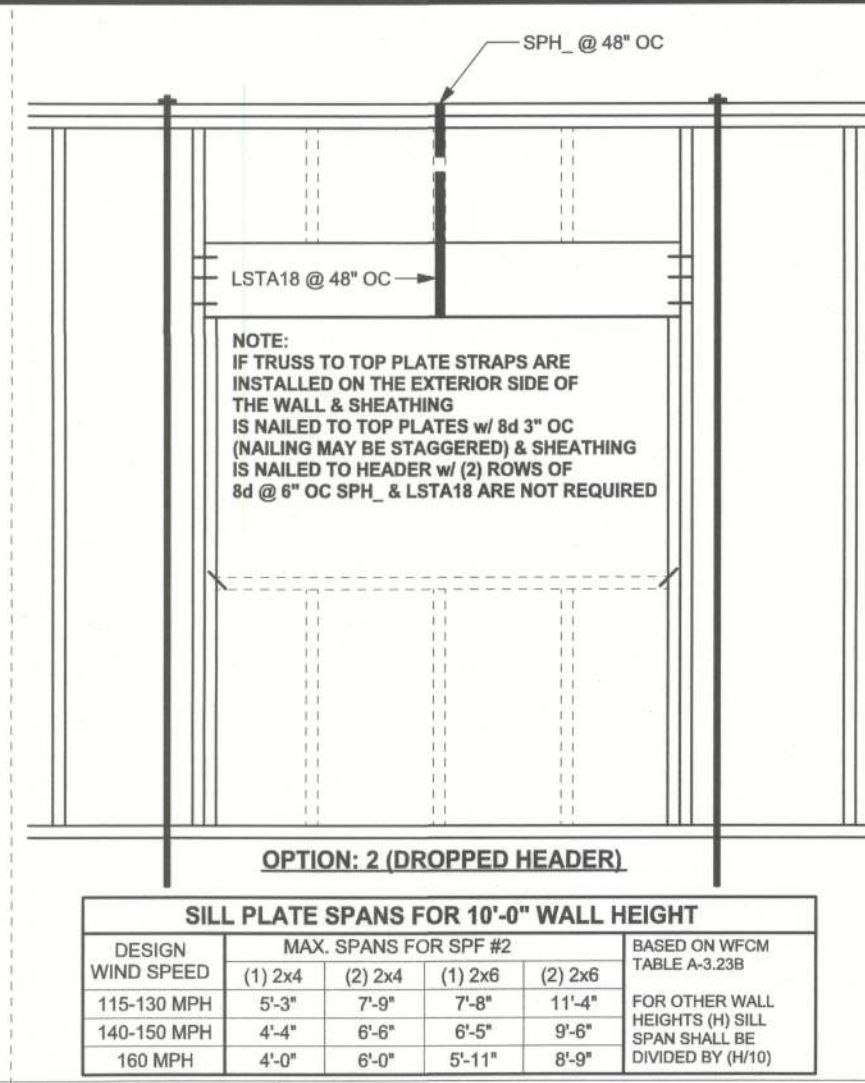
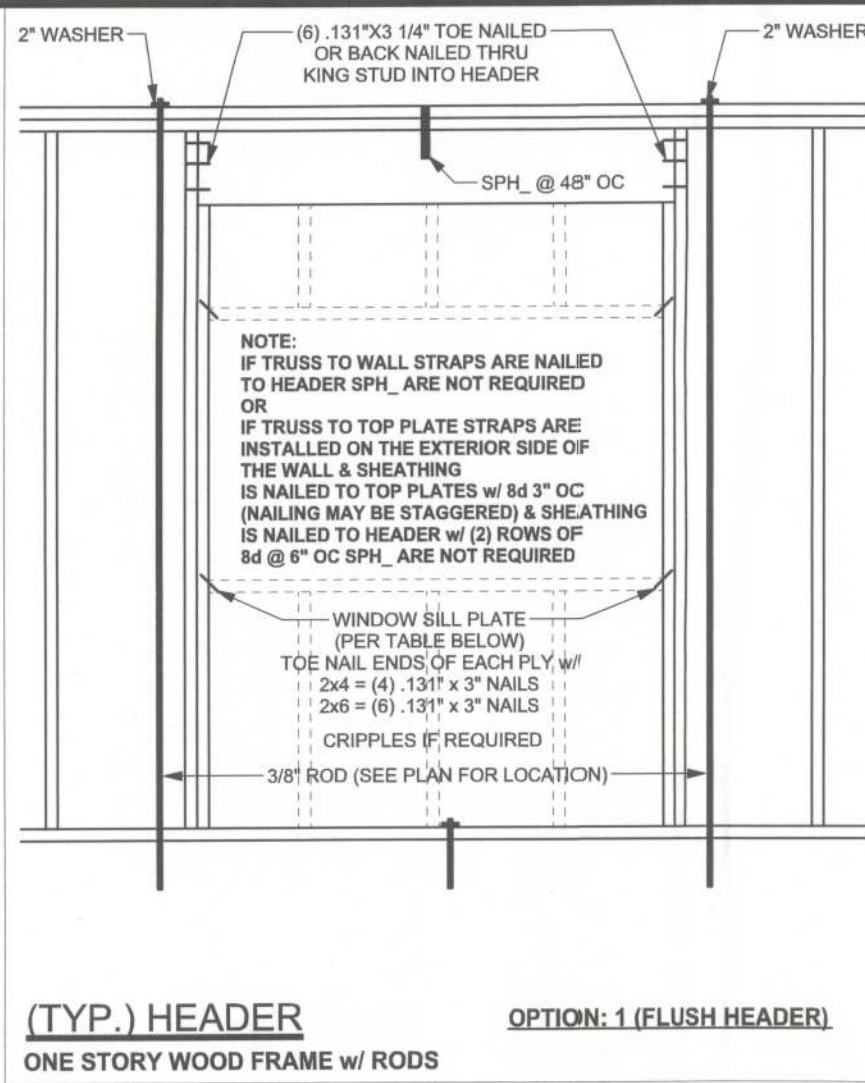
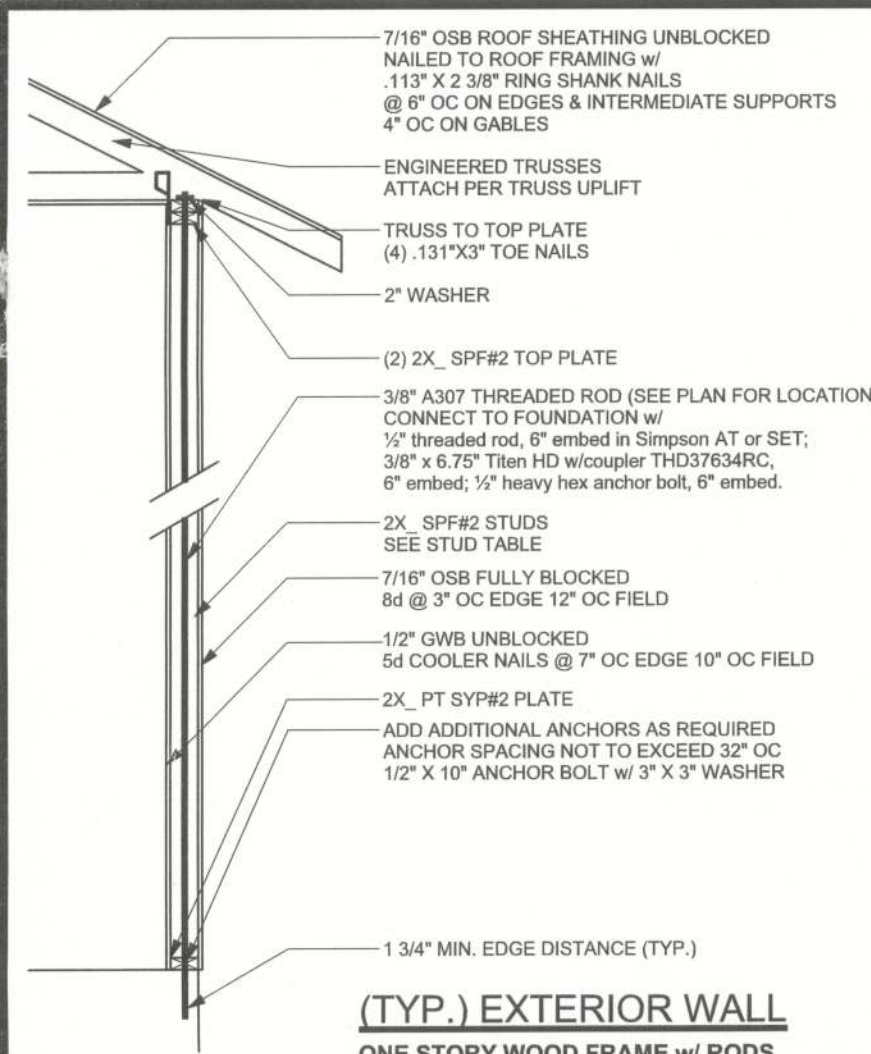
APPROVED BY: JGC

PROJECT NO:

SHEET:

A-3





## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H8	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	525	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
<b>HEAVY GIRDER TIEDOWNS</b>								
LG2	2050	1785	700	170	700	170	14-16d	14-16d
LG3-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LG4-SDS3	4060	3660	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MG1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
<b>STUD STRAP CONNECTOR</b>								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	800					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	10-10d x 1 1/2"
SP4H	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
SP6	885	760					6-10d x 1 1/2"	10-10d x 1 1/2"
SP6H	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
LSTA18	1235	1110					14-10d	14-10d
LSTA21	1235	1235					16-10d	16-10d
CS20	1030	1030					14-10d	14-10d
CS16	1705	1705					22-10d	22-10d
<b>STUD ANCHORS</b>								
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT31	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					16-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					16-16d	2-5/8" ANCHOR

- (1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES  
(2) FOR SYP GIRDER & SPF STUDS

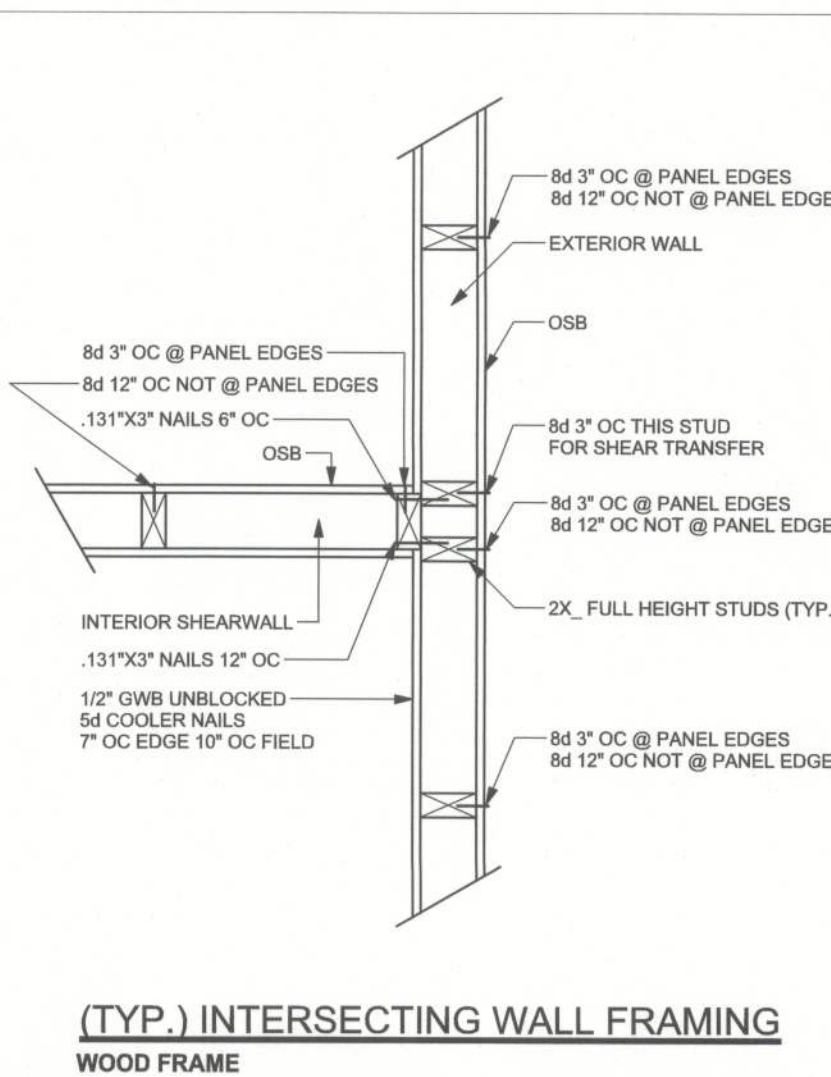
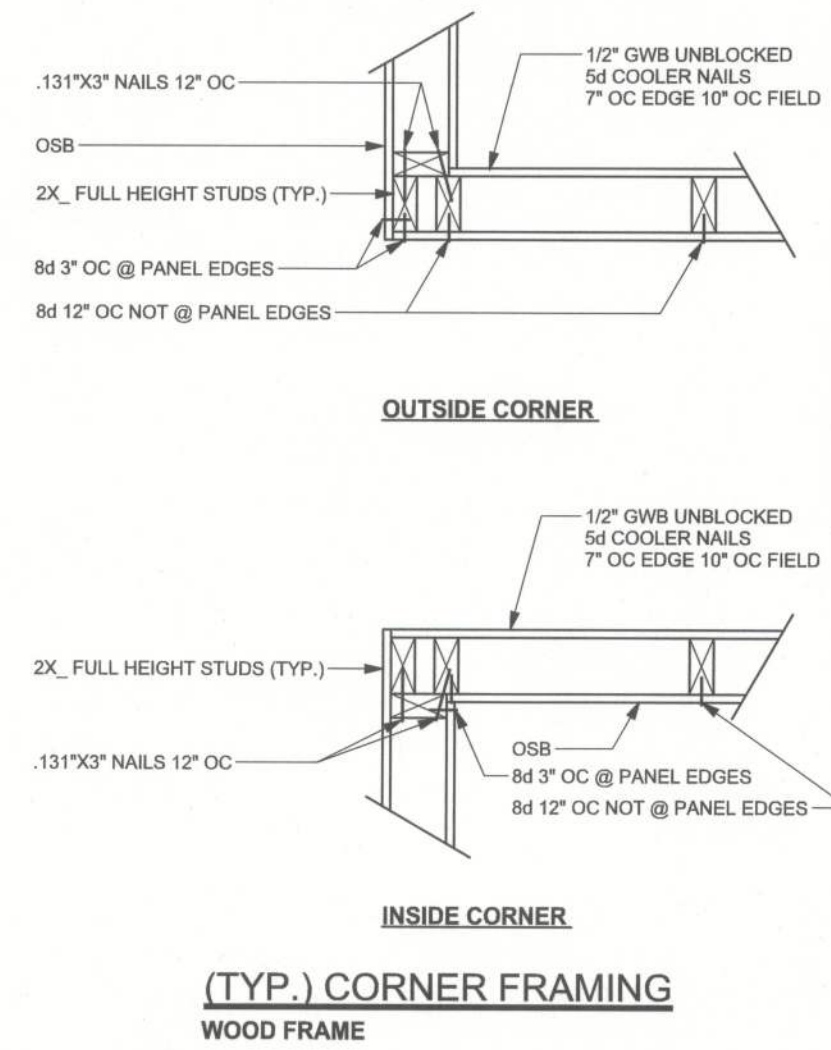
## ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL

ONE STORY WOOD FRAME w/ RODS

ALLOWABLE UPLIFT: 1800 LB

OPTION: 1) LTT20B, 10-16d (1750lb)  
1/2" ANCHOR 6" EMBEDMENT (MAY BE RECESSED BELOW FINISHED FLOOR)

OPTION: 2) MSTA21, 9-10d TO STUDS & 5-1/4\"/>



## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\"/>	TO 10'-1\"/>
(1) 2x4 @ 12\"/>	TO 11'-2\"/>
(1) 2x6 @ 16\"/>	TO 15'-7\"/>
(1) 2x6 @ 12\"/>	TO 17'-3\"/>

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2\"/>

## GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION AT EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER REINFORCED SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI. ALL LAP SPLICES 40\"/>

GLULAM BEAMS: GLB, 24F-V3SP, F<sub>b</sub> = 2,4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.  
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES TRUSS SHEETS.

## DESIGN DATA

<b>WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1</b>	
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)	
2.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY	
3.) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY	
4.) RISK CATEGORY = II, (MRI = 700 YR)	
5.) ROOF ANGLE = 7-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)	
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	

Zone	Effective Wind Area (ft <sup>2</sup> )	
1	39	-43
2	39	-68
3	39	-100
4	43	-46
5	43	-57

<b>DESIGN LOADS</b>	
FLOOR: 40 PSF (ALL OTHER DWELLING ROOMS)	
30 PSF (SLEEPING ROOMS)	
30 PSF (ATTICS WITH STORAGE)	
10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF: 20 PSF (FLAT OR <4:12)	
16 PSF (4:12 TO <12:12)	
12 PSF (12:12 AND GREATER)	
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

## REVISIONS

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 886, Lake City, FL 32066, 386-754-6419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at address 32026.

Mark Disoway, P.E. No. 53915

June 12, 2013

O'Neal Companies

Rick Parker  
Garage

ADDRESS:  
3546 SR 247  
Lake City, Florida 32024

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
June 12, 2013

DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
10Jun13

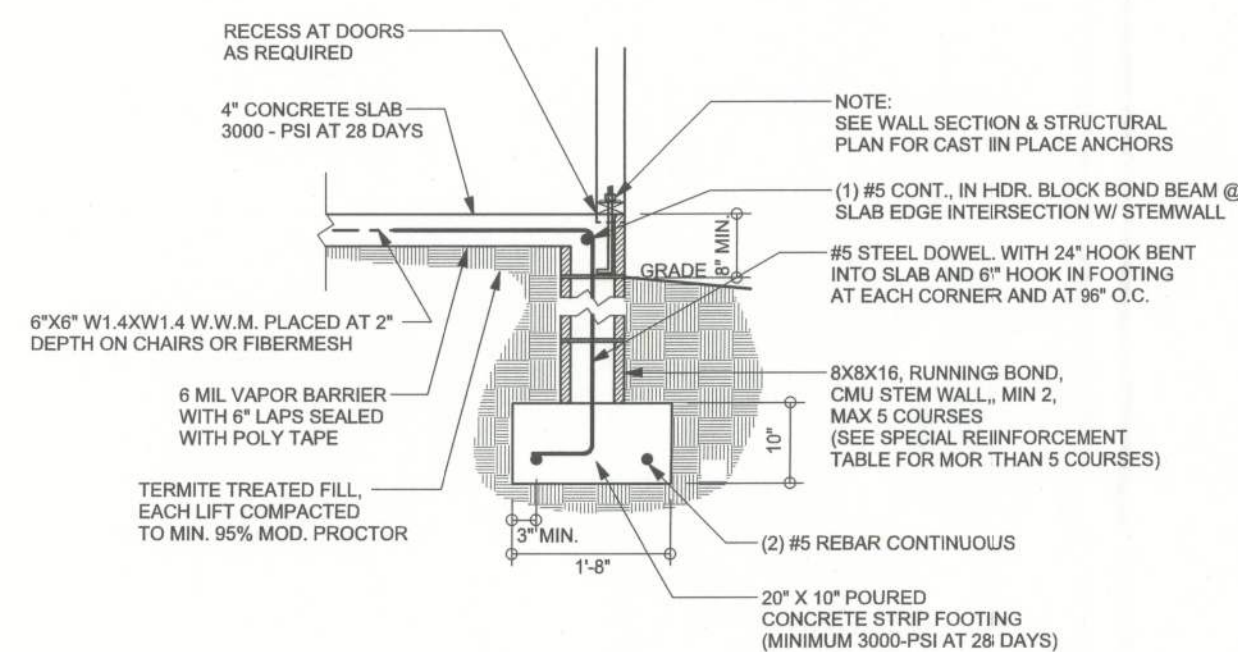
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1305108

DRAWING NUMBER  
**S-1**  
OF 3 SHEETS

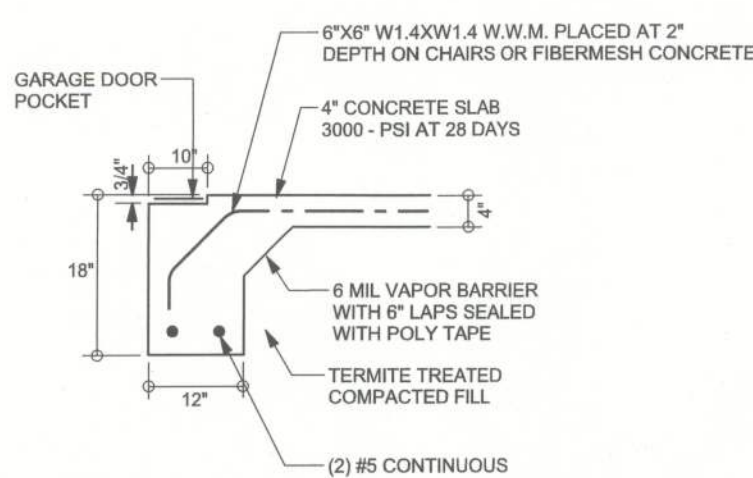


## REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



**F1  
S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"

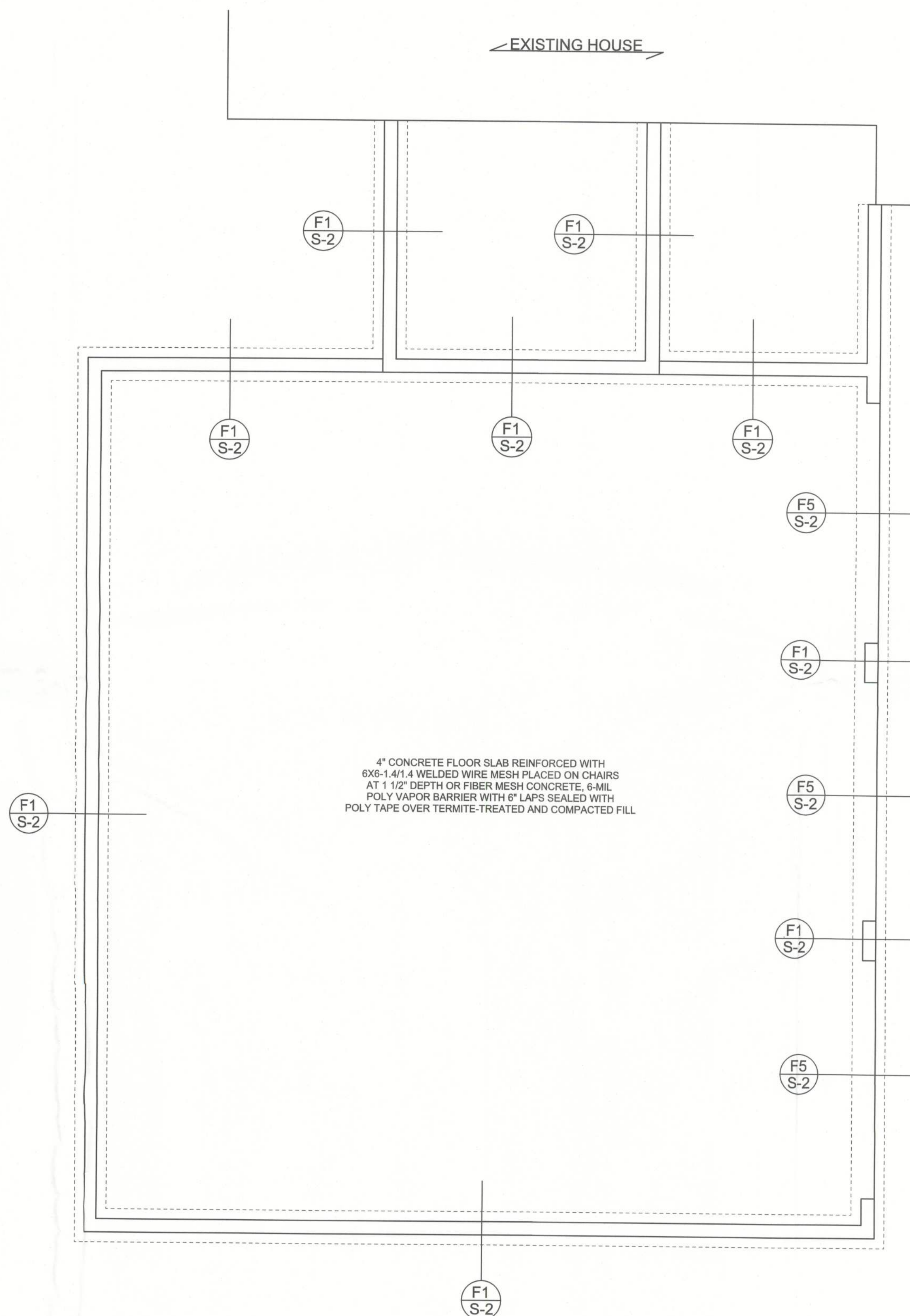


**F5  
S-2** GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"

## TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



## FOUNDATION PLAN

SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,  
PE No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-754-5419

DIMENSIONS:  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disosway, P.E. for resolution.  
Do not proceed without clarification.

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these instruments of service. This document is  
not to be reproduced, altered or copied in any  
form or manner without first the express written  
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering  
comply with section F301.2.1, 2010 Florida  
Building Code Residential  
to the best of my knowledge.

LIMITATION: This design is valid for one  
building, at specified location.



## O'Neal Companies

Rick Parker  
Garage

ADDRESS:  
3546 SR 247  
Lake City, Florida 32024

Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4671

PRINTED DATE:  
June 12, 2013

DRAWN BY: STRUCTURAL BY:  
David Disosway

FINALS DATE:  
10Jun13

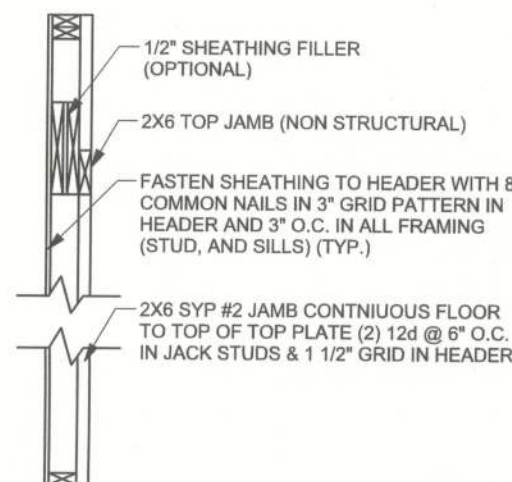
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1305108

DRAWING NUMBER  
**S-2**

OF 3 SHEETS

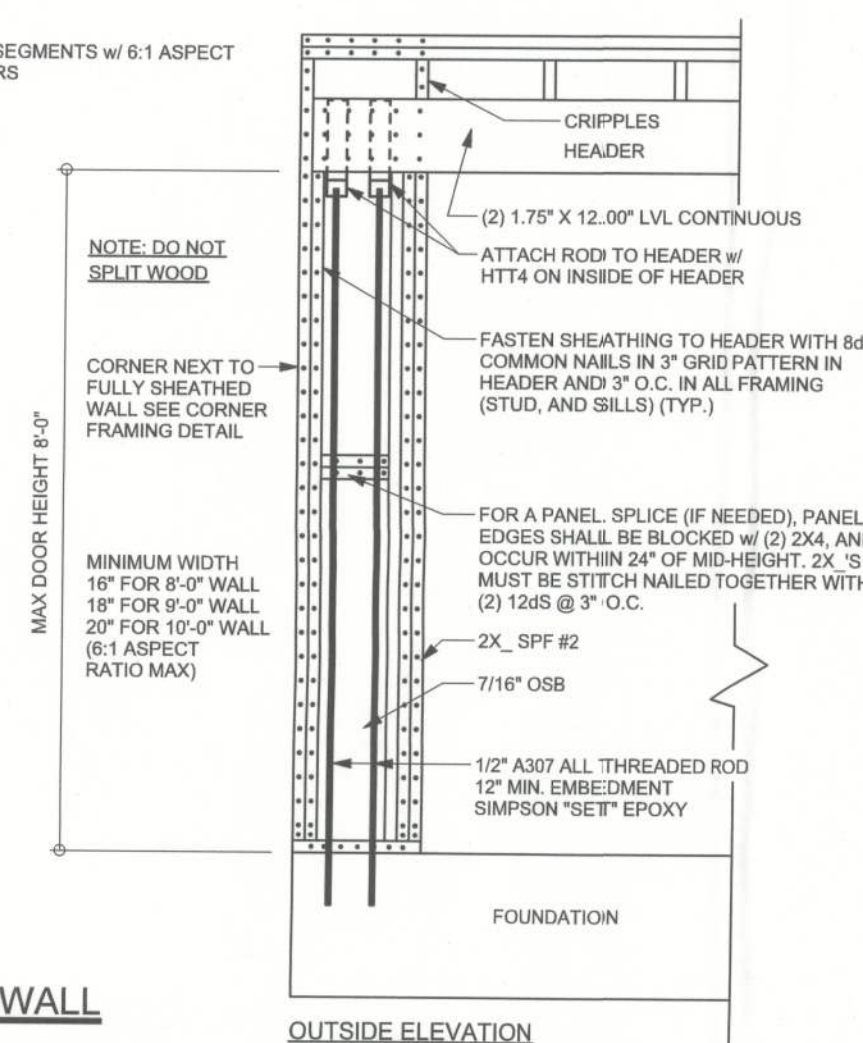


NOTE:  
THIS PORTAL FRAME IS DESIGNED FOR NARROW WALL SEGMENTS w/ 6:1 ASPECT  
RATIO MAX SUCH AS BESIDE FRONT LOAD GARAGE DOORS

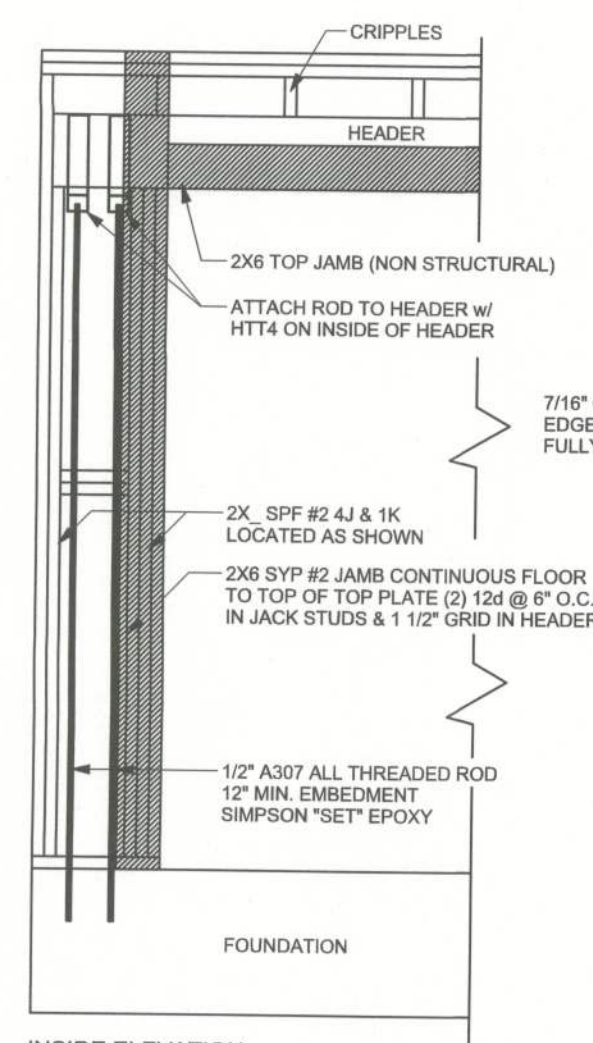


SECTION DETAIL

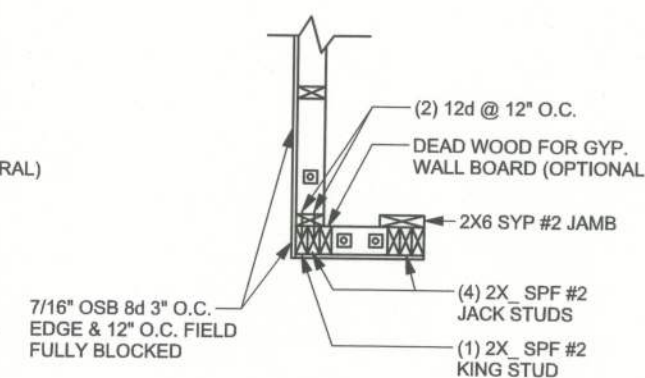
(TYP.) PORTAL FRAME SHEARWALL  
ONE STORY WOOD FRAME



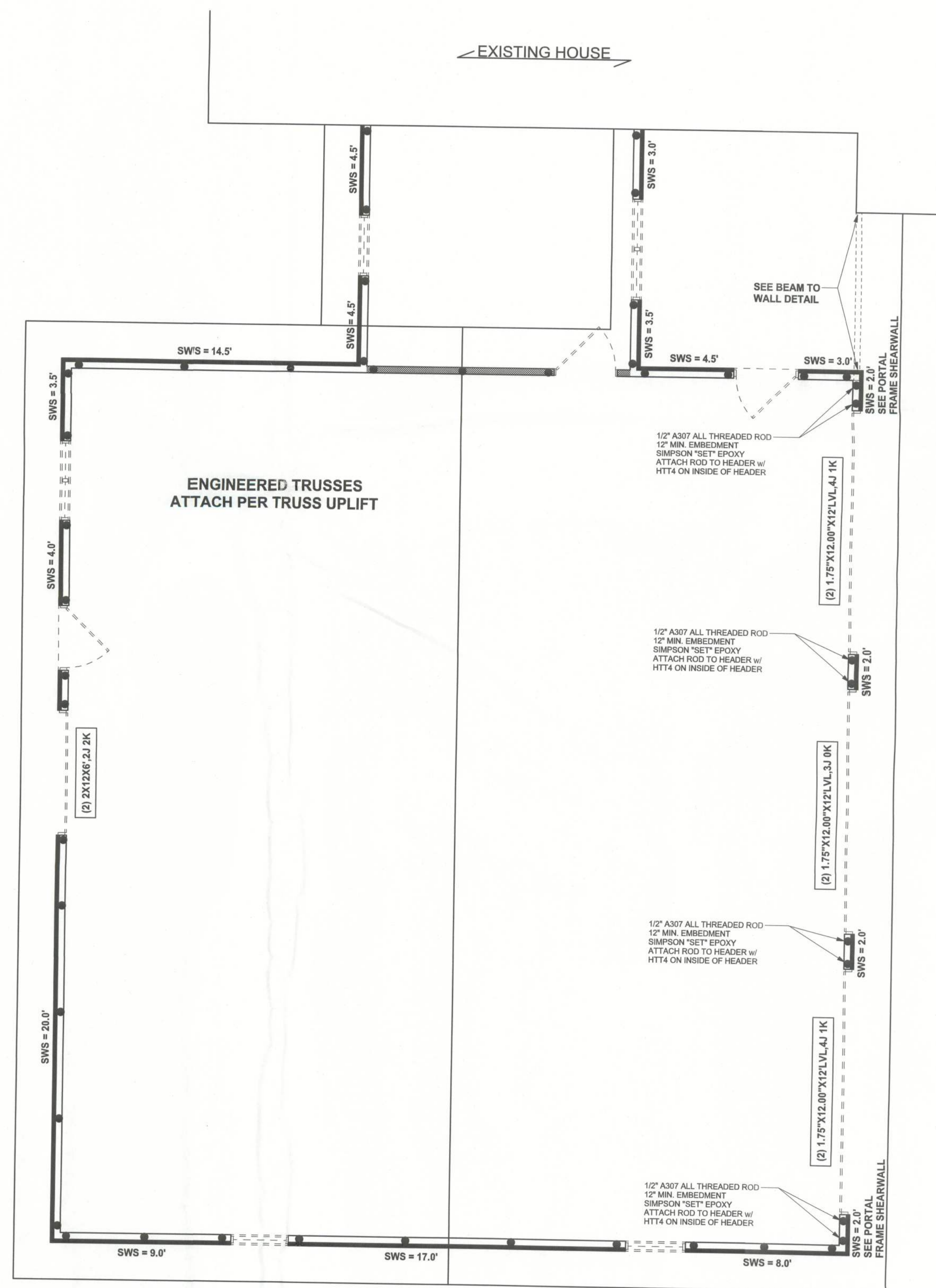
OUTSIDE ELEVATION



INSIDE ELEVATION



CORNER FRAMING DETAIL



STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

# STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03, BCSH-B1, BCSH-B2, & BCSH-B3. BCSH-B1, BCSH-B2, & BCSH-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

# WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

# HEADER LEGEND

- (2) 2X12X0', 1J 1K → HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

# THREADED ROD LEGEND

- 
- INDICATES LOCATION OF:  
1ST FLOOR 3/8" A307 ALL THREADED ROD (U.N.O.)

# TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	38.5'	56.0'
LONGITUDINAL	32.0'	51.0'

REVISION

SOFT

WINDLOAD ENGINEER  
P.E. No. 53915, P.O.B. 88  
30565, 386-754-6418

DIMENSIONS:  
Stated dimensions sup-  
ply dimensions. Refer all  
Mark Discovery, P.E. to  
Do not proceed without

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CERTIFICATION: I her  
examined this plan, an  
portions of the plan, re  
comply with section R3  
Building Code Resident  
to the best of my knowl

LIMITATION: This desig  
building, at specified loc



O'Neal Co

Rick P  
Garra

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PRINTED  
June 12, 2013

DRAWN BY:

FINALS DATE:  
10Jun13

JOB NUM  
13051

DRAWING N  
S-3

OF 3 SHE