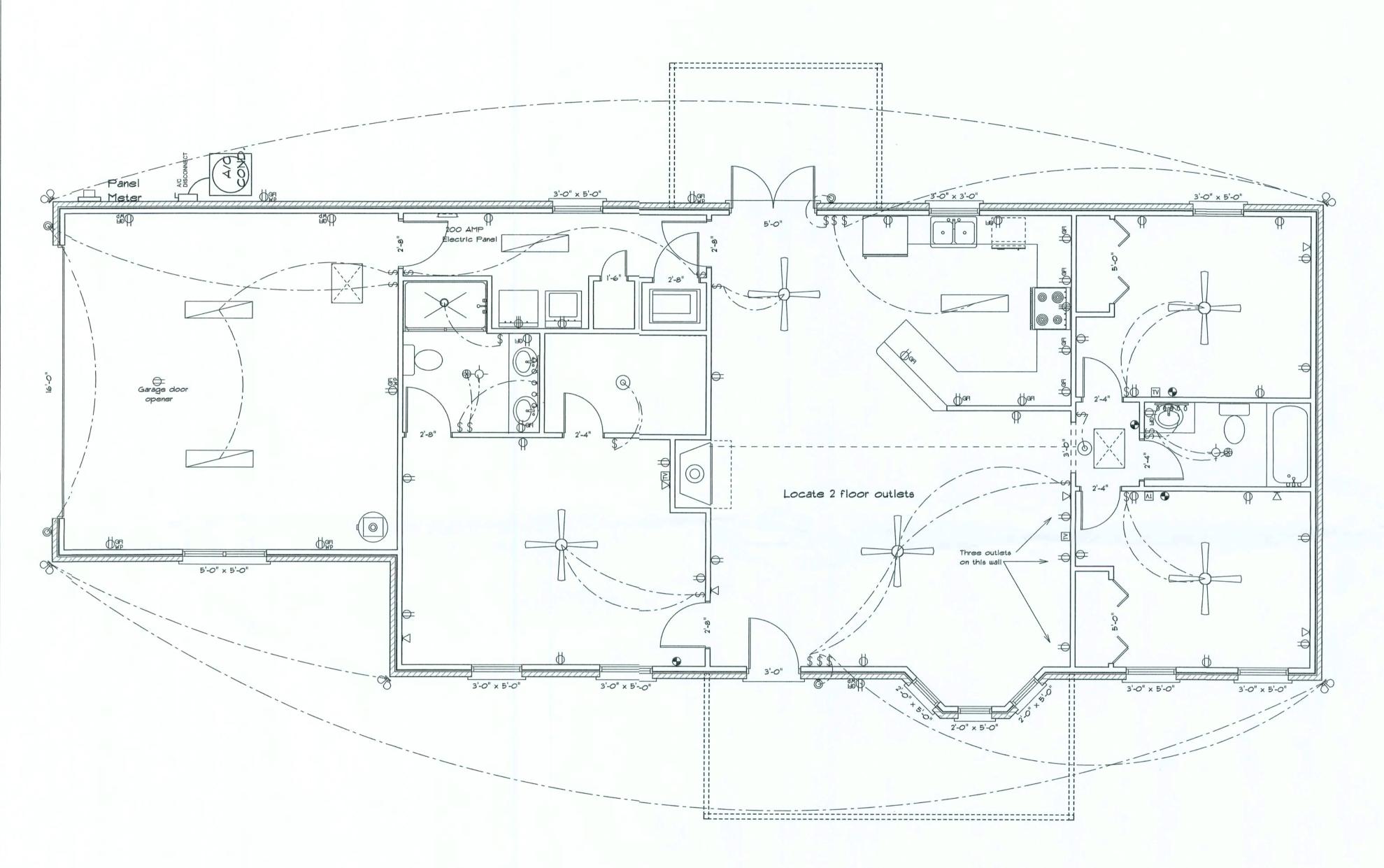




ELECTRICAL	SYMBOL
ceiling fan globe 1	
ceiling globe light	0
double spotlight	QP
fluorescent fixture	
pot light	0
vanity bar light	<u> </u>
wall mount 1	Q
electrical panel	īJ
AC Disconnect	A/Q COND
Outlet WP GFI	⊕ <sup>wp</sup>
cable tv outlet	TV
fan	₩
light	
outlet	Ф
outlet 220v	<b>(b)</b>
outlet gfi	⊕on-
smoke detector	•
switch	\$
telephone	$\nabla$



Electrical Plan

Scale 1/4" = 1'

#### Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equiptment per manufactures specifications.
- E-2 Consult the owner for the number or seperate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responssible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.

Overcurrent protection device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

**RESIDENCE** 

ADDRESS: Columbia County, Florida

Woodman Park Builders, Inc. Lake City, Florida Phone: (386) 755 - 8699 Fax: (386) 755-8684 Email:

DRAWN BY: CHECKED BY:

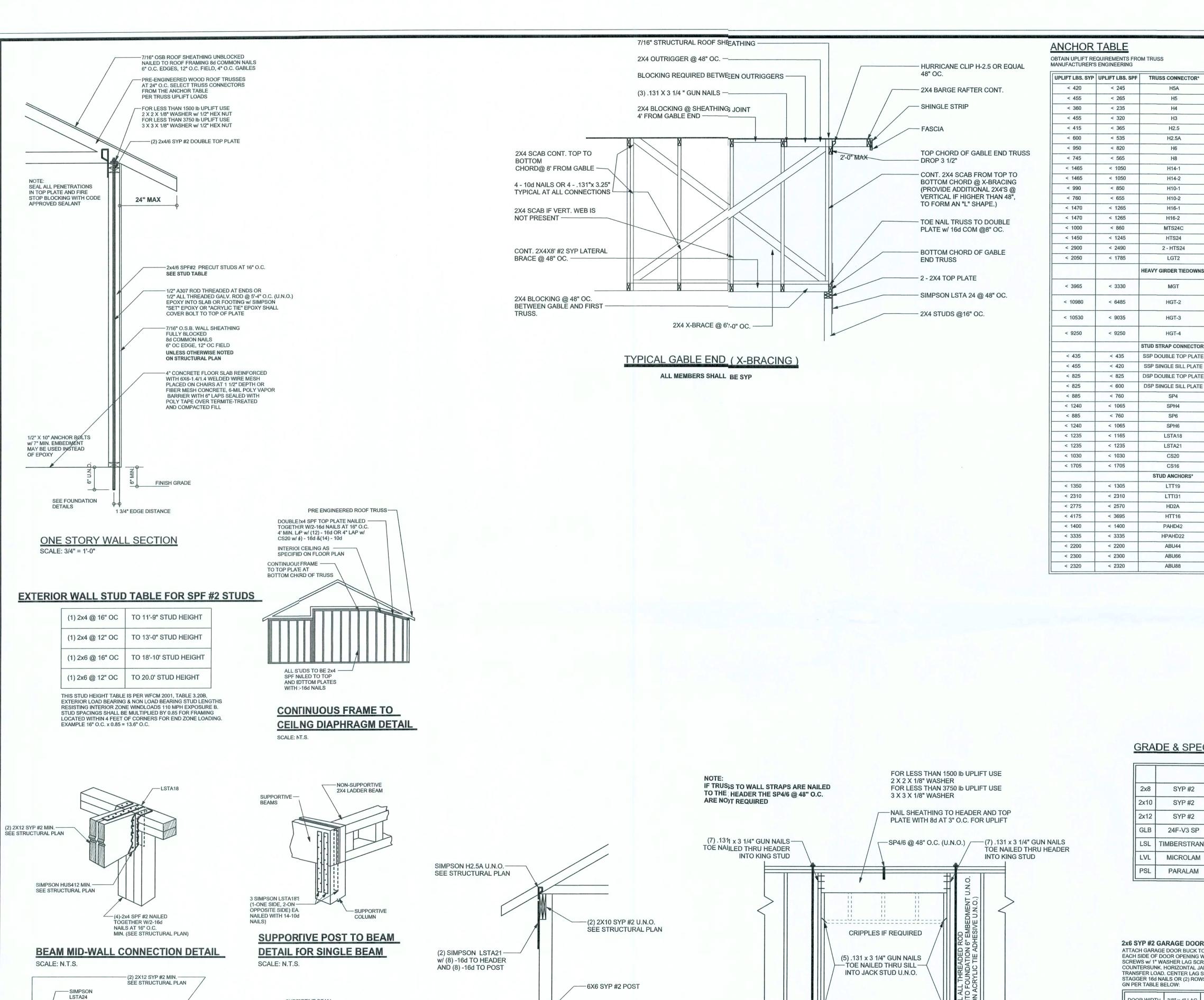
PRINTED DATE:

DESIGNED

FINALS DATE:

JOB NUMBER:

DRAWING NUMBER



-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SUPPORTIVE BEAM ---

SUPPORTIVE CENTER POST TO BEAM DETAIL

IF BEAM JOINT IS AT ----

4-SIMPSON LSTA1! -

(2-ONE SIDE,2-ON

OTHER SIDE)

POST CONNECTION,

LSTA18 ON ONE SIDE

SEE STRUCTURAL PLAN

SCALE: N.T.S.

**BEAM CORNER CONNECTION. DETAIL** 

INSTALL ONE SIMPSON

4-8d 4-8d < 360 < 235 4-8d 4-8d < 455 < 320 4-8d H3 4-8d < 415 < 365 H2.5 5-8d 5-8d < 600 < 535 H2.5A 5-8d < 950 < 820 8-8d < 745 < 565 5-10d, 1 1/2" 5-10d, 1 1/2 < 1465 < 1050 H14-1 13-8d 12-8d. 1 1/2" < 1465 < 1050 H14-2 15-8d 12-8d, 1 1/2 < 990 < 850 H10-1 8-8d, 1 1/2" 8-8d, 1 1/2" < 760 < 655 H10-2 6-10d 6-10d < 1470 < 1265 10-10d, 1 1/2" 2-10d, 1 1/2 < 1470 < 1265 H16-2 10-10d, 1 1/2" 2-10d, 1 1/2" < 1000 < 860 MTS24C 7-10d 1 1/2" 7-10d 1 1/2" < 1450 HTS24 12-10d 1 1/2" 12-10d 1 1/2' < 2900 < 2490 2 - HTS24 < 2050 < 1785 LGT2 14 -16d 14 -16d HEAVY GIRDER TIEDOWNS TO FOUNDATION 5/8" THREADED ROD < 3965 22 -10d 12" EMBEDMENT 2-5/8" THREADED ROD < 10980 < 6485 HGT-2 16 -10d 12" EMBEDMENT < 10530 < 9035 HGT-3 12" EMBEDMENT < 9250 < 9250 HGT-4 16 -10d 12" EMBEDMENT STUD STRAP CONNECTOR TO STUDS < 435 SSP DOUBLE TOP PLATE 4 -10d < 455 SSP SINGLE SILL PLATE 1 -10d 4 -10d < 825 < 825 DSP DOUBLE TOP PLATE 6 -10d 8 -10d < 825 DSP SINGLE SILL PLATE 2-10d 8 -10d < 885 < 760 6-10d, 1 1/2" < 1240 < 1065 SPH4 10-10d, 1 1/2" < 885 < 760 SP6 6-10d, 1 1/2" < 1240 < 1065 10-10d, 1 1/2" < 1235 < 1165 LSTA18 14-10d < 1235 < 1235 LSTA21 16-10d < 1030 < 1030 18-8d CS20 < 1705 < 1705 28-8d STUD ANCHORS' TO STUDS TO FOUNDATION < 1350 < 1305 LTT19 8-16d 1/2" AB < 2310 < 2310 LTTI31 18-10d, 1 1/2 1/2" AB < 2775 < 2570 2-5/8" BOLTS 5/8" AB < 4175 < 3695 HTT16 18 - 16d 5/8" AB < 1400 < 1400 16-16d PAHD42 < 3335 < 3335 HPAHD22 16-16d < 2200 < 2200 ABU44 12-16d 1/2" AB

< 245

< 2300

< 2320

ABU66

ABU88

**GRADE & SPECIES TABLE** 

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH | 3/8" x 4" LAG | 16d | (2) ROWS OF STAGGER | .131 x 3 1/4" GN

16" O.C. 3" O.C.

**GARAGE DOOR BUCK INSTALLATION DETAIL** 

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GN PER TABLE BELOW:

16' - 18'

2x6SYP #2 DOOR BUCK-

SCALE: N.T.S.

BRACKET. ---

TYPICAL STRAPPING (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

(SEE STRUCTURAL PLAN)

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

8' - 10' 24" O.C. 5" O.C.

11' - 15' 18" O.C. 4" O.C.

LSL TIMBERSTRAND

975

2900

2900

4" O.C.

3" O.C.

12-16d

18 - 16d

TO PLATES TO RAFTER/TRUSS

3-8d

3-8d

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDEFOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 300( PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS FER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACE/JENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CU'S TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTSIS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIROWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WTH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED INTHE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED III DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

IERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### **BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOJ BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR AL.

#### ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF F LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

### **DESIGN DATA**

1/2" AB

2-5/8" AB

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R3012.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE FOOFS: MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

2 O'hg		10		100		
2 O'hg	1	19.9	-21.8	18.1	18.1	
3 19.9 -25.5 18.1 21.8 3 O'hg -68.3 42.4 4 21.8 -23.6 18.5 20.4 5 21.8 -29.1 18.5 22.6 Doors & Windows Worst Case (Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9	2	19.9	-25.5	18.1	21.8	
3 O'hg	2 O'hg		-40.6		40.6	
4 21.8 -23.6 18.5 20.4 5 21.8 -29.1 18.5 22.6 Doors & Windows 21.8 29.1 Worst Case (Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9	3	19.9	-25.5	18.1	21.8	
5 21.8 -29.1 18.5 22.6  Doors & Windows 21.8 29.1  Worst Case (Zone 5, 10 ft2)  3x7 Garage Door 19.5 22.9	3 O'hg		-68.3		42.4	
Doors & Windows 21.8 29.1 Worst Case (Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9	4	21.8	-23.6	18.5	20.4	
Worst Case (Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9	5	21.8	-29.1	18.5	22.6	
Worst Case (Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9	-	0.145		04.0	Tan.	
(Zone 5, 10 ft2) 3x7 Garage Door 19.5 22.9				21.8	29.1	
3x7 Garage Door 19.5 22.9			T-1-1-1			
	(Zone	5, 10	ft2)			
6x7 Garage Door 18.5 21.0	8x7 Garage Door			19.5	22.9	
	16x7 Garage Door		18.5	21.0		
					-	
					-	

JOB NUMBER: 602071 DRAWING NUMBER

> **S-1** OF 3 SHEETS

10 100

**DESIGN LOADS** 

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

Woodman Park Builder Zone Effective Wind Area (ft2)

Coulombe Residence

VINDLOAD ENGINEER: Mark Disosway

E No.53915, POB 868, Lake City, FL

ated dimensions supercede scaled

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ts common law copyrights and property right hese instruments of service. This document i

not to be reproduced, altered or copied in any

form or manner without first the express writt ermission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

code residential 2004, to the best of my

LIMITATION: This design is valid for one

uilding, at specified location.

amined this plan, and that the applicable

rtions of the plan, relating to wind enginee

mply with section R301.2.1, florida building

MARK DISOSWAY

SEAL

P.E. 53915

imensions. Refer all questions to

Mark Disosway, P.E. for resolution.

o not proceed without clarification.

32056, 386-754-5419

REVISIONS

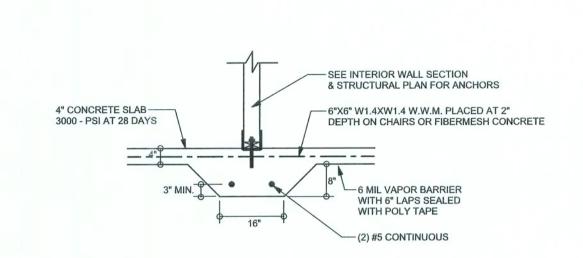
SOFTPLAN

ADDRESS: 3827 SW Birley Rd. Columbia County, Florida

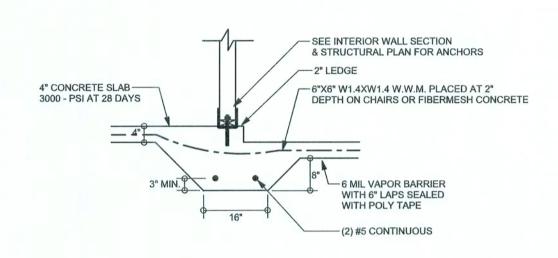
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: February 15, 2006 STRUCTURAL B DRAWN BY: David Disosway

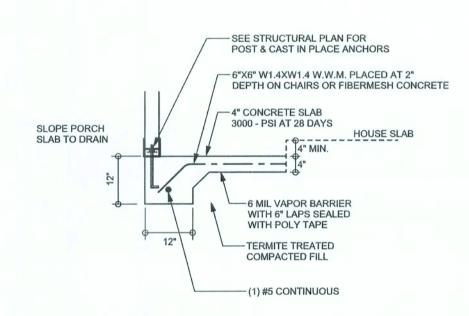
09 / Feb / 06



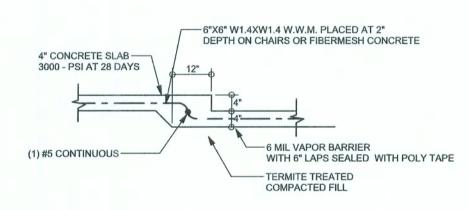
## F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



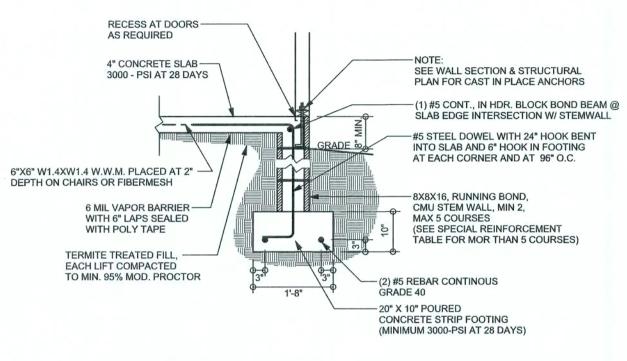
# F3 INTERIOR BEARING STEP FOOTING WITH 2" LEDGE SCALE: 1/2" = 1'-0"



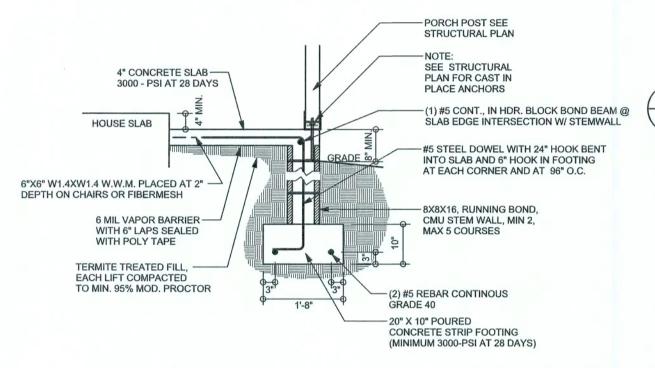
# F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



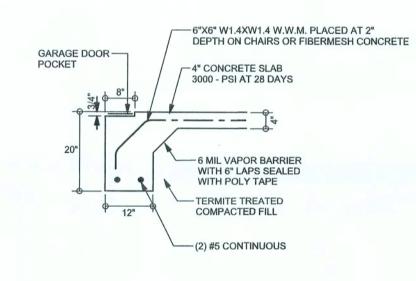
F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



### F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



# F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

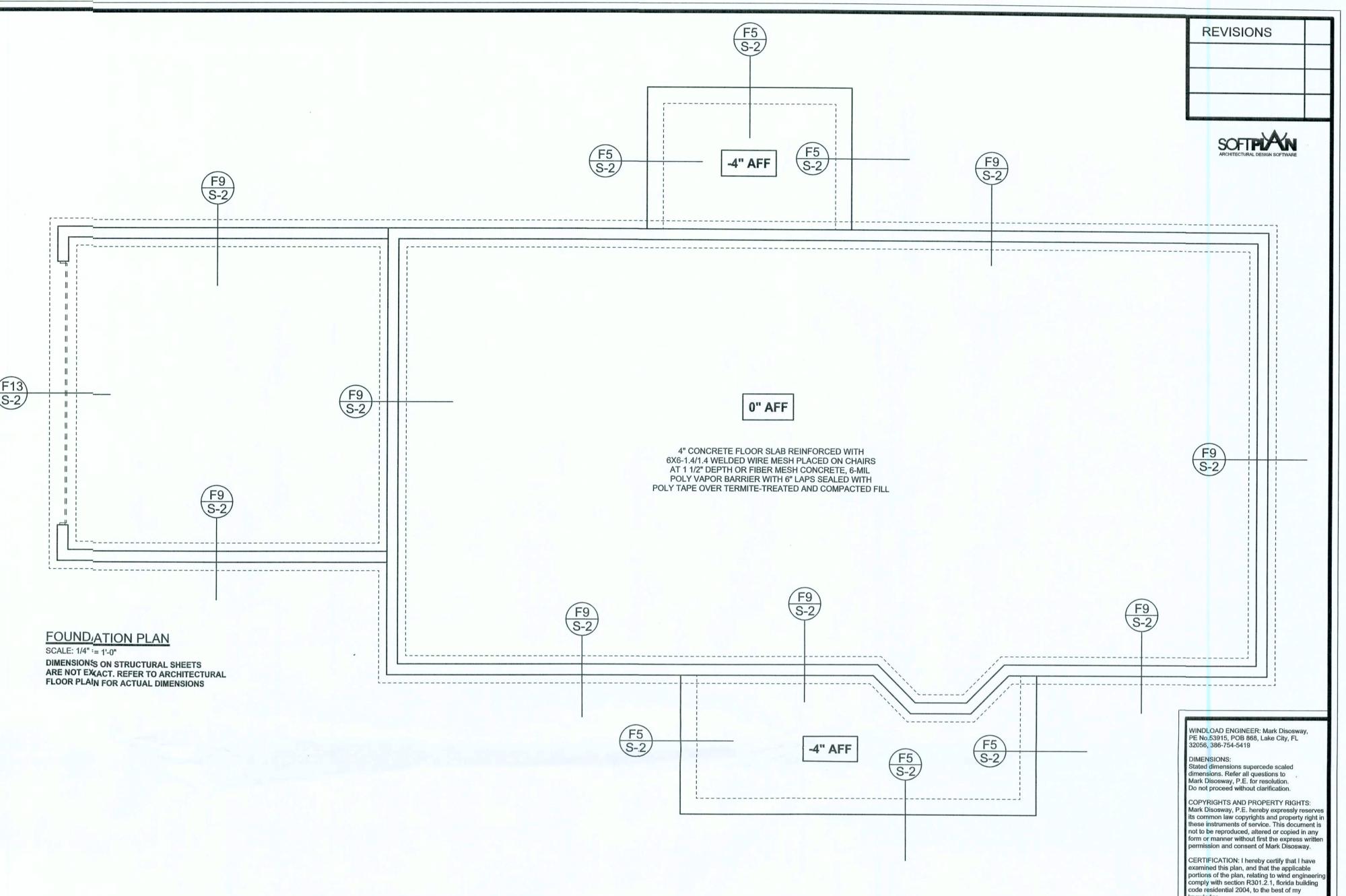


F13 ALT. STEM WALL GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL UNBALANCED HEIGHT BACKFILL (FEET) HEIGHT	BACKFILL	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



## VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)

Woodman Park Builders

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Coulombe Residence

ADDRESS: 3827 SW Birley Rd. Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
February 15, 2006

DRAWN BY: STRUCTURAL BY
David Disosway

FINALS DATE: 09 / Feb / 06

JOB NUMBER: 602071 DRAWING NUMBER

> S-2 OF 3 SHEETS

REVISIONS SOFTPINAN ARCHITECTURAL DESIGN SOFTWARE MSTA30, 10-10d (1700lb)———— (5) NAILS EACH SIDE OF STUD (OR STRAP STUD TO HEADER 20-10d —ABU (TYP.) STRUCTURAL PLAN NOTES SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.) SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) LTT20B, 10-16d (1750lb ----1/2" ANCHOR w/ 6" EMBEDMENT U.N.O., SIMPSOI -AT (MAY BE RECESSED BELOW FINISHED FLOOR) DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS SWS = 29.5° PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, ALTERNATE WALL TIE CONNECTION WHERE BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 UPLIFT ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED THREADED ROD CANNOT BE PLACED IN WALL 12" EMBEDMENT-SCALE: 1/2" = 1'-0" WALL LEGEND SWS = 0.0' 1ST FLOOR EXTERIOR WALL SWS = 0.0' 2ND FLOOR EXTERIOR USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE PE No.53915, POB 868, Lake City, FL 1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. −T03 TRUSS — 2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1 12" EMBEDMENT-COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. THREADED ROD LEGEND SVS = 7.5' SWS = 7.5CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my - INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD LIMITATION: This design is valid for one building, at specified location. UPLIFT UPLIFT UPLIFT VUPLIFT - INDICATES LOCATION OF: −851 lb MARK DISOSWAY P.E. 53915 2ND FLOOR 1/2" A307 ALL THREADED ROD UPLIFT SWS = 4.5 SWS = 5.5' SWS = 3.5' **HEADER LEGEND** (2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) 851 lb ──85β lb UPLIFT UPLIFT NUMBER OF KING STUDS (FULL LENGTH) NUMBER OF JACK STUDS (UNDER HEADER) UPLIFT -----SPAN OF HEADER Woodman Park Builders SIZE OF HEADER MATERIAL NUMBER OF PLIES IN HEADER STRUCTURAL PLAN
SCALE: 1/4" = 1'-0" Coulombe Residence 516 llb--573 lb TOTAL SHEAR WALL SEGMENTS UPLIET UPLIFT UPLIFT ADDRESS: SWS = 0.0' INDICATES SHEAR WALL SEGMENTS 3827 SW Birley Rd. REQUIRED ACTUAL TRANSVERSE 31.8' 39.5' Columbia County, Florida Mark Disosway P.E. LONGITUDINAL 28.5' 104.0' P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: February 15, 2006 DRAWN BY: STRUCTURAL B David Disosway FINALS DATE: 09 / Feb / 06 JOB NUMBER: 602071 CONNECTIONS, WALL, & HEADER DESIGN IS BASED DRAWING NUMBER ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE **S-3** (JOB #L144709) OF 3 SHEETS