GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CML OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.
- B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN"
- 2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"
- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572. FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD WEB MATERIAL CONFORMS TO ASTM-A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.
- 2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD
- 3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD
- 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR
- A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH A755 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.
- 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:
- BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
- b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".
- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.
- ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

 D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

- THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP
- C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE
- D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING.
- EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

 DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER
 SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE,

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, 13TH ED.)
- D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 13TH ED.)

 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.
- F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

 G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR
- IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.

 IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT
- THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE
- INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

 1) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 13TH ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION, ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT. BEARING VALUES. TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION. AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) MFTAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

Richard T. Smith PE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874



REVIEWED By Richard T Smith at 9:11 am, Aug 03, 2023

FRAMING COLORS Flange brace: RO RO

EAVE: ORNER WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.

COLORS: 00F: GALVALUME VALLS: RUSTIC RED GABLE: BURNISHED SLATE BURNISHED SLATE BURNISHED SLATE FRAMED OPENINGS: BURNISHED SLATE

BURNISHED SLATE



 יטר	RAWING INDEX			
PAGE	DESCRIPTION			
0-0.1	COVER PAGE			
1	ANCHOR BOLT LAYOUT			
1.1	ANCHOR BOLT DETAILS			
1.2-1.3	ANCHOR BOLT REACTIONS			
2	ROOF FRAMING LAYOUT			
2.1-2.6	RIGID FRAME CROSS SECTION			
3-3.2	ENDWALL FRAMING LAYOUT			
4-4.2	SIDEWALL FRAMING LAYOUT			
5-5.5	FRAMING DETAILS			
6	ROOF PANELS & TRIM			
6.1	ROOF PANEL DETAILS			
7	SIDEWALL PANEL DETAILS			
8	ENDWALL PANEL DETAILS			
9	SPECIAL DETAILS			

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

FLORIDA PRODUCT APPROVAL NUMBER PBR ROOF PANEL 36875 1 PBR WALL PANEL

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.



BY AN IAS ACCREDITED FACILITY.

PR NORRIS MORE 32025 BASCOM AND 딦 BUILDINGS 능 SW 792

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JOBSITE:

DRAWING STATUS 8058R1 FOR APPROVAL:

THESE DRAWINGS, BEING FOR APPROVAL, ARE BY
DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL
REPRESENTATION ONLY. THEIR PURPOSE IS TO
CONFIRM PROPER INTERRETATION OF THE PROJECT
DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION"
CAN BE CONSIDERED AS COMPLETE. 7/17/23 DAR IIILE

CAN BE CONSIDERED AS COMPLETE.

FOR PERMIT:

THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFEED. ONLY DRAWINGS ISSUED FOR CONSTRUCTION "CAN BE CONSIDERED AS COMPLETE. COVER PAGE NUMBER FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

PAGE

NONE



BUILDING PROFILE

Width (ft) $= 40$	Eave	Height (ft)	=	14
Length $(ft) = 60$	Roof	Slope (Rise/12)	=	4.0:12

BUILDING LOADS

- A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 20 / 7TH EDITION
- B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

LOCAL BUILDING DEPT.	, , , , , , , , , , , , , , , , , , ,
OCCUPANCY/RISK CATEGORY II	<u>Normal le 1.00</u>
WIND LOAD ULTIMATE 1	19 MPH NOMINAL 92.18 MPH WIND EXPOSURE B
CLOSURE TYPE Er	nclosed
INTERNAL WIND COEFFICIENT	0.18 / 0.18
COLLATERAL DEAD LOAD 3	PSF
ROOF LIVE LOAD 20	0.00 PSF (REDUCIBLE Yes)
DEAD LOAD 2.	.500 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	
SPECTRAL RESPONSE Ss _0.0913	S1_0.0529
SITE CLASS <u>d</u>	DESIGN RISK CATEGORY B Cs 0.0324
RESPONSE MODIFICATION FACTOR, R 3	.00* FRAMES 3.00* BRACING
BASIC SEISMIC FORCE RESISTING SYSTEM	
BASIC SEISMIC FORCE RESISTING SYSTEM	(LEW) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

ORDINARY STEEL CONC. BRACED FRAMES

= EQUIVALENT LATERAL FORCE PROCEDURE



BASIC SEISMIC FORCE RESISTING SYSTEM (REW)

ANALYSIS PROCEDURE

SERVICEABILITY CRITERIA

	MINIMUM DESIG	N DEFLECTIONS	
Endwall Column	= 240	Wall Panel	= 60
Endwall Rafter (Live)	= 360	Roof Panel (Live)	= 60
Endwall Rafter (Wind)	= 360	Roof Panel (Wind)	= 60
Wall Girt	= 240	Rigid Frame (Horz)	= 100
Roof Purlin (Live)	= 360	Rîgid Frame (Vert)	= 360
Roof Purlin (Wind)	= 360	Rigid Frame (Seismic)	= 50

COMPONEN ⁻	TS	& CLAD	DING	(un	factored)
Wall Field Values	=	21.483	psf	/	-23.303	psf
Wall Edge Values	=	21.483	psf	/	-28.765	psf

BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES BOILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SHOW SURCHARDES.
FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20ft. OF A HIGHER
STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES
NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20ft. ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS.

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF

FOR OCCUPANCY (RISK), CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE—STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS.



BUILDING PROFILE

Width (ft) = 15	Eave Height (ft)	=	14 H/S
Length (ft) = 60	Roof Slope (Rise/12)	=	4.0:12

BUILDING LOADS

- THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 20 / 7TH EDITION
- THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL. THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

LOCAL BUILDING DEPT. OCCUPANCY/RISK CATEGORY <u>II - Normal le 1.00</u> ULTIMATE 119 MPH NOMINAL 92.18 MPH WIND EXPOSURE B WIND LOAD Enclosed CLOSURE TYPE

-0.18 / 0.18 INTERNAL WIND COEFFICIENT COLLATERAL DEAD LOAD _1 ___ PSF

ROOF LIVE LOAD 20.00 PSF (REDUCIBLE Yes)

2.000 PSF (FOR ROOF PANELS AND PURLINS) DEAD_LOAD

SFISMIC

 SPECTRAL RESPONSE
 Sg 0.0913
 S1 0.0529
 Sds 0.0971
 Sd1 0.0832
 Cs 0.0324 SITE CLASS d DESIGN RISK CATEGORY B

RESPONSE MODIFICATION FACTOR, R 3.00* FRAMES 3.00* BRACING

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

SERVICEABILITY CRITERIA

	MINI	NUM DESK	ON DEFLECTIONS		
Endwall Column	=	120	Wall Panel	=	60
Endwall Rafter (Live)	==	180	Roof Panel (Live)	=	60
Endwall Rafter (Wind)	#	180	Roof Panel (Wind)	=	60
Wall Girt	=	90	Rigid Frame (Horz)	40.	60
Roof Purlin (Live)	=	150	Rigid Frame (Vert)	=	180
Roof Purlin (Wind)	=	150	Rigid Frame (Seismic)	-	50

COMPONENTS & CLADDING (unfactored) Wall Field Values = 20.638 psf / -22.387 psf Wall Edge Values = 20.638 psf / -27.634 psf

BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES BOILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SHOW SURCHARGES
FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20ft OF A HIGHER
STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES
NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20ft. ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS.

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF

FOR OCCUPANCY (RISK), CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS.

1.0 PSF COLL ONLY ALLOW LIGHTING AND HVAC DUCT TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE.

Richard T. Smith

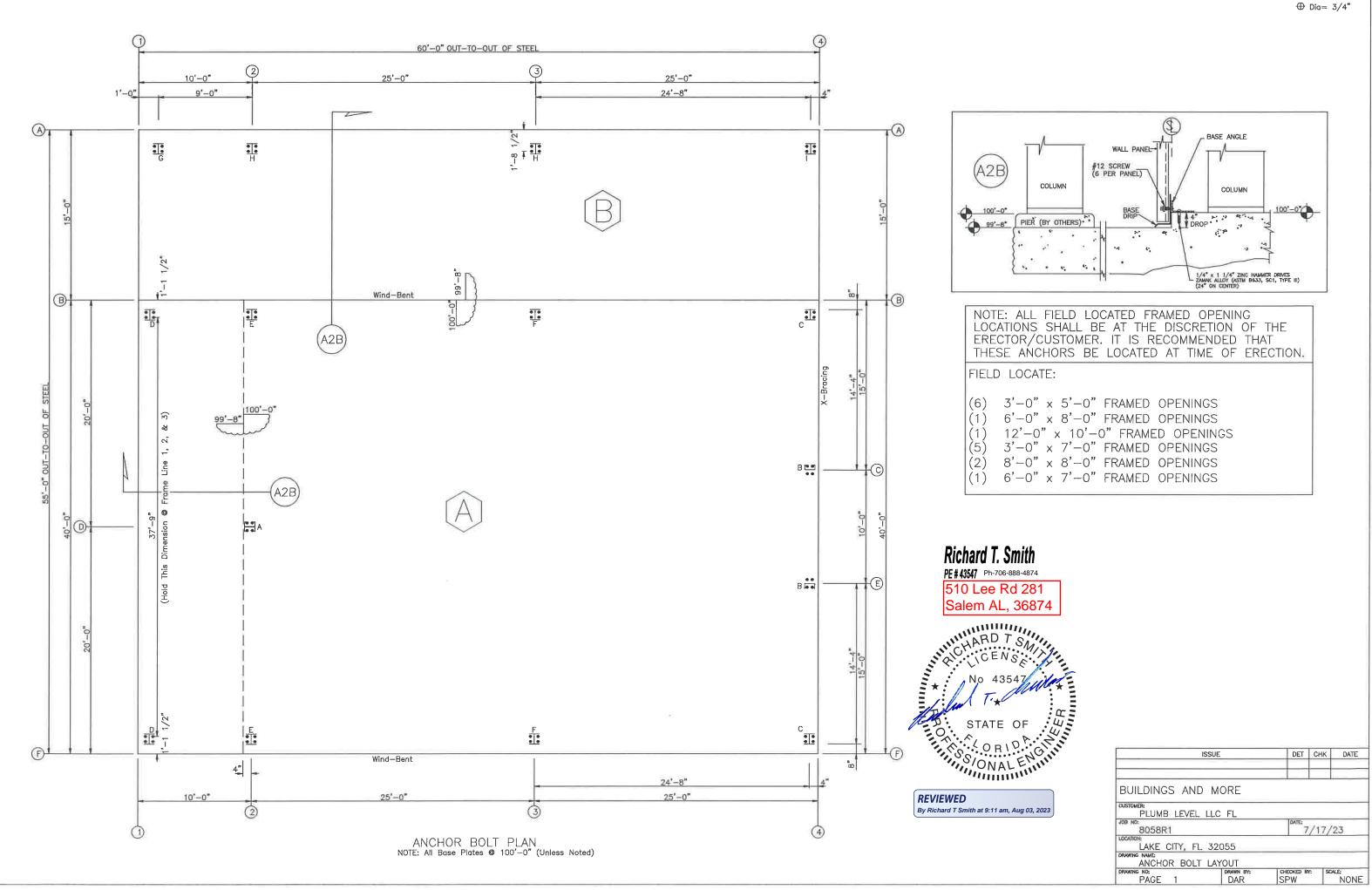
PE # 43547 Ph-706-888-4874 510 Lee Rd 281

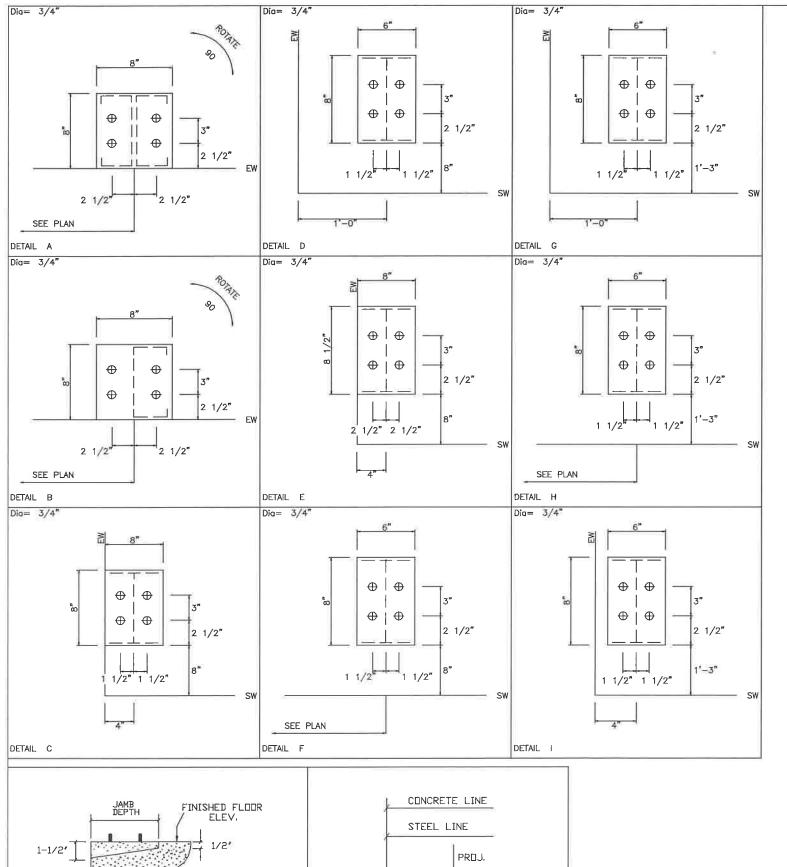
Salem AL, 36874

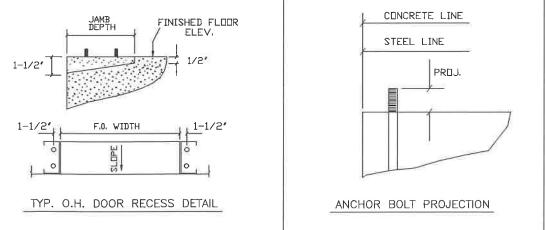


REVIEWED

ISSUE	DET	CHK DATE
BUILDINGS AND MORE		
PLUMB LEVEL LLC FL		
8058R1	DATE:	/17/23
LAKE CITY, FL 32055		
RAWING NAME: COVER PAGE		
PAGE 0.1 DRAWN BY:	SPW SPW	NONE







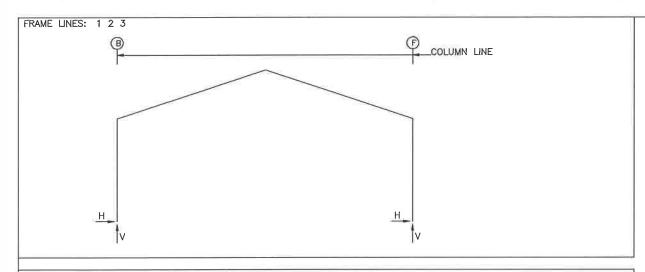
Richard T. Smith PE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874



REVIEWED

ISS	DET	СНК	DATE	
			-	-
BUILDINGS AND	MORE			
CUSTOMER: PLUMB LEVEL LI	LC FL			
JOB NO: 8058R1	DATE:	17/	23	
LAKE CITY, FL 3	32055			
DRAWING NAME: ANCHOR BOLT D			7115	
DRAWING NO: PAGE 1.1	DRAWN BY: DAR	SPW	SCA	NONE



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col	_Bolt	Base.	_Plate (i	n)	Grout
Line	Line	Dia	Width	Length	Thick	(in)
1	B F		6.000 6.000		0.375 0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

	Col Line	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
2	B F		8.000 8.000	8.500 8.500	0.375 0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col	_Bolt	Base.	_Plate (i	n)	Grout
Line	Line	Dia	Width	Length	Thick	(in)
3	B F		6.000 6.000		0.375 0.375	0.0

END	WALL	COI	_UMN:	ANCI	HOR BOL	TS & B	ASE PLATES
Frm Line	Col Line		_Bolt Dia	Base. Width	_Piate (Length	in) Thick	Grout (in)
2	D	4	0.750	8.000	8.000	0.250	0.0
4	F	4	0.750	8.000	8.000	0.375	0.0
4	Ε	4	0.750	8.000	8.000	0.250	0.0
4	С	4	0.750	8.000	8.000	0.250	0.0
4	В	4	0.750	8.000	8.000	0.375	0.0
ENDWALL COLUMN:				ANCI	HOR BOL	TS & B	ASE PLATES
Frm Line	Col Line		_Bolt Dia	Base Width	_Plate (i Length	in) Thick	Grout (in)

ANCHOR BOLT SUMMARY								
Qty	Locate	Dia (in)	Туре	Projection (in)				
⊕ 20 ⊕ 24	Endwall Frame	3/4" 3/4"	GR36 GR36	1.50 2.50				

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

RIGIE	FRAN	ΛE: E	BASIC COL	LUMN REA	CTIONS ((k)								
Frame Line 1 1 2 2 3 3	Column Line B F B F B F	Horiz 0.2 -0.2 0.4 -0.4 0.6 -0.6	-Dead Vert 0.9 0.8 1.7 1.4 2.2	Collo Horiz 0.1 -0.1 0.5 -0.5 0.7 -0.7	vert Vert 0.5 0.4 1.3 1.2 2.0 1.8	Horiz 0.8 -0.9 1.4 -1.4 2.1 -2.2	-Live Vert 3.5 2.7 6.3 4.4 8.9 6.5	Wind Horiz -0.5 1.1 -3.3 -0.4 -4.8 -0.8	_Left1 Vert -3.7 -3.0 -8.9 -5.4 -12.4 -7.1	-Wind_ Horiz -0.3 1.2 2.1 4.8 3.2 6.9	Right1- Vert -2.9 -3.2 -5.1 -8.2 -6.8 -11.6	Wind Horiz -1.2 0.5 -4.5 -0.5 -6.2 -1.3	Left2- Vert -3.4 -3.2 -6.7 -3.5 -8.4 -2.5	
Frame Line 1 1 2 2 3 3	Column Line B F B B F	-Wind_ Horiz -0.2 1.5 0.9 4.7 1.7 6.4	Right2- Vert -3.2 -2.9 -3.6 -5.6 -2.8 -7.0	Wind Horiz -0.3 1.3 1.2 1.9 2.7 2.1	_Long1— Vert -3.1 -3.2 -6.6 -6.4 -9.1 -8.5	Wind Horiz 1.0 0.0 0.6 1.4 1.5 0.9	_Long2— Vert 0.9 0.9 -6.2 -6.7 -8.3 -9.3	-Seismi Horiz 0.0 0.0 -0.1 -0.1 -0.2 -0.2	ic_Left Vert 0.0 0.0 -0.1 0.1 -0.1 0.1	Seismic Horiz 0.0 0.0 0.1 0.1 0.2 0.2	_Right Vert 0.0 0.0 0.1 -0.1 0.1 -0.1	-Seismi Horiz 0.0 0.0 0.0 0.0 0.0	c_Long Vert 0.0 0.0 0.0 0.0 0.0	

ENDWALL COLUMN	· ·
Frm Col Dead Collc Line Line Vert Vert 2 D 0.1 0.2 4 F 0.4 0.3 4 E 0.6 0.6 4 C 0.6 0.6 4 B 0.7 0.4	Vert Vert Vert Vert Horz Horz Vert Vert Vert 0.0 0.0 0.0 0.0 -3.3 3.7 0.0 0.0 0.0 2.1 -2.2 -2.2 -1.3 -1.2 -1.4 1.6 -2.4 -1.6 0.0 3.7 -3.8 -1.7 -2.8 -0.9 -1.9 2.0 -3.1 -1.5 0.0 3.6 -2.4 -3.2 -1.4 -2.6 -1.9 2.0 -1.5 -3.4 -0.1
Seis Seis Frm Col Right Long Line Line Vert Vert 2 D 0.0 0.0 4 F 0.0 0.0 4 E 0.0 0.0 4 C 0.0 0.0 4 B 0.0 0.0	

NOTES FOR REACTION	IS
Roof Slope (Rise/12) Dead Load (psf) Collateral Load (psf) Roof Live Load(psf) Frame Live Load Min(psf) Max(psf) Wind Speed (mph) Wind Code Exposure Closure Importance Wind Importance Seismic	= 40.0 = 60.0 = 14.0/ 14.0 = 4.0/ 4.0 = 2.5 = 3.0 = 20.0 = 119.0 = FBC 20/7th EDITION = B = Enclosed = 1.00 = 1.00 = B

GENERAL NOTES

- 1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUILDING BRACING REACTIONS								
# Reactions(k) Panel_Shear Wall — Col — Wind — Seismic — (lb/ft) Loc Line Line Horz Vert Horz Vert Wind Seis Note								
L_EW 1 F_SW F 2,3 1.4 1.4 0.2 0.2 (b) R_EW 4 C,B 2.9 3.6 0.2 0.2 B_SW B 2,3 2.1 2.1 0.3 0.3 (b)								
(b)Wind bent in bay, base above finish floor (h)Rigid frame at endwall								
Reactions for seismic represent shear force, Eh								

Richard T. Smith

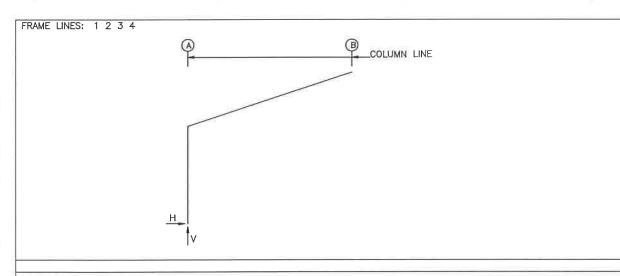
PE#43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874



REVIEWED By Richard T Smith at 9:11 am, Aug 03, 2023

BUILDING "A" ISSUE DET CHK DATE BUILDINGS AND MORE OUSTOMER:
PLUMB LEVEL LLC FL 8058R1 7/17/23 LAKE CITY, FL 32055 ANCHOR BOLT REACTIONS PAGE 1.2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

 Frm Line
 Col Line
 Anc._Bolt Qty
 Base_Plate (in) Width
 Grout Length
 Grout (in)

 1
 A
 4
 0.750
 6.000
 8.000
 0.375
 0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

 Frm Line
 Col Line
 Anc._Bolt Qty
 Base_Plate (in)
 Grout (in)

 2
 A
 4
 0.750
 6.000
 8.000
 0.375
 0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

 Frm Line
 Col Line
 Anc._Bolt Qty
 Base_Plate (in)
 Grout (in)

 3
 A
 4
 0.750
 6.000
 8.000
 0.375
 0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

 Frm Line
 Col Line
 Anc._Bolt Qty
 Base_Plate (in)
 Grout (in)

 4
 A
 4
 0.750
 6.000
 8.000
 0.375
 0.0

ANCHOR BOLT SUMMARY

 Qty
 Locate
 Dia (in)
 Type
 Projection (in)

 ⊕ 16
 Frame
 3/4" GR36
 2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

RIGIE	FRAN	/E: Е	BASIC COL	LUMN REA	CTIONS (k)								
Frame Line 1 2 3 4	Column Line A A A A	Horiz 0.0 0.0 0.0 0.0	-Dead Vert 0.3 0.5 0.6 0.4	Colla Horiz 0.0 0.0 0.0 0.0	teral— Vert 0.1 0.1 0.2 0.1	Horiz 0.0 0.0 0.0 0.0	Live——— Vert 1.1 2.6 3.3 2.2	Wind Horiz -0.3 -0.6 -0.8 -0.5	Left1- Vert -1.4 -2.7 -3.9 -2.6	-Wind_l Horiz 0.4 0.9 1.3 0.8	Right1- Vert -1.2 -2.5 -3.7 -2.3	Wind. Horiz -0.5 -1.1 -1.6 -0.9	Left2- Vert -0.9 -1.3 -1.9	
Frame Line 1 2 3 4	Column Line A A A	-Wind_ Horiz 0.2 0.4 0.5 0.4	Right2- Vert -0.7 -1.1 -1.6 -1.1	Wind Horiz 0.3 0.9 1.4 0.7	Long1- Vert -1.2 -3.2 -4.7 -2.6	Wind Horiz 0.3 1.0 1.4 0.7	Long2- Vert -0.8 -2.3 -3.3 -1.7							

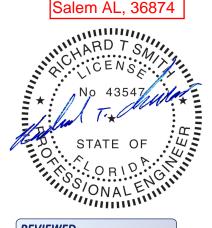
NOTES FOR REACTIONS Building reactions are based on the following building data: Width (ft) = 60.0 Eave Height (ft) = 9.0/ 14.0 Roof Slope (Rise/12) = 4.0 Dead Load (psf) = 2.0 Collateral Load (psf) = 1.0 Roof Live Load(psf) = 20.0 Frame Live Load Min(psf) = 16.5 Max(psf) = 20.0 Wind Speed (mph) = 119.0 Wind Code = FBC 20/7th EDITION Exposure = Enclosed Importance Wind = 1.00 Importance Seismic = 1.00 Seismic Zone = B Seismic Coeff (Fa*Ss) = 0.15

BUILDING BRACIN	IG REACTIONS					
Wall Col	± Reactions(k) Wind — — Seismic — (lb/ft) Z Vert Horz Vert Wind Seis Note					
L_EW 1 F_SW B R_EW 4 B_SW A Torsional Br	(h) (e) (h)					
(e)Bracing loads applied to supporting building (h)Rigid frame at endwall						
Reactions for seismic re	epresent shear force, Eh					

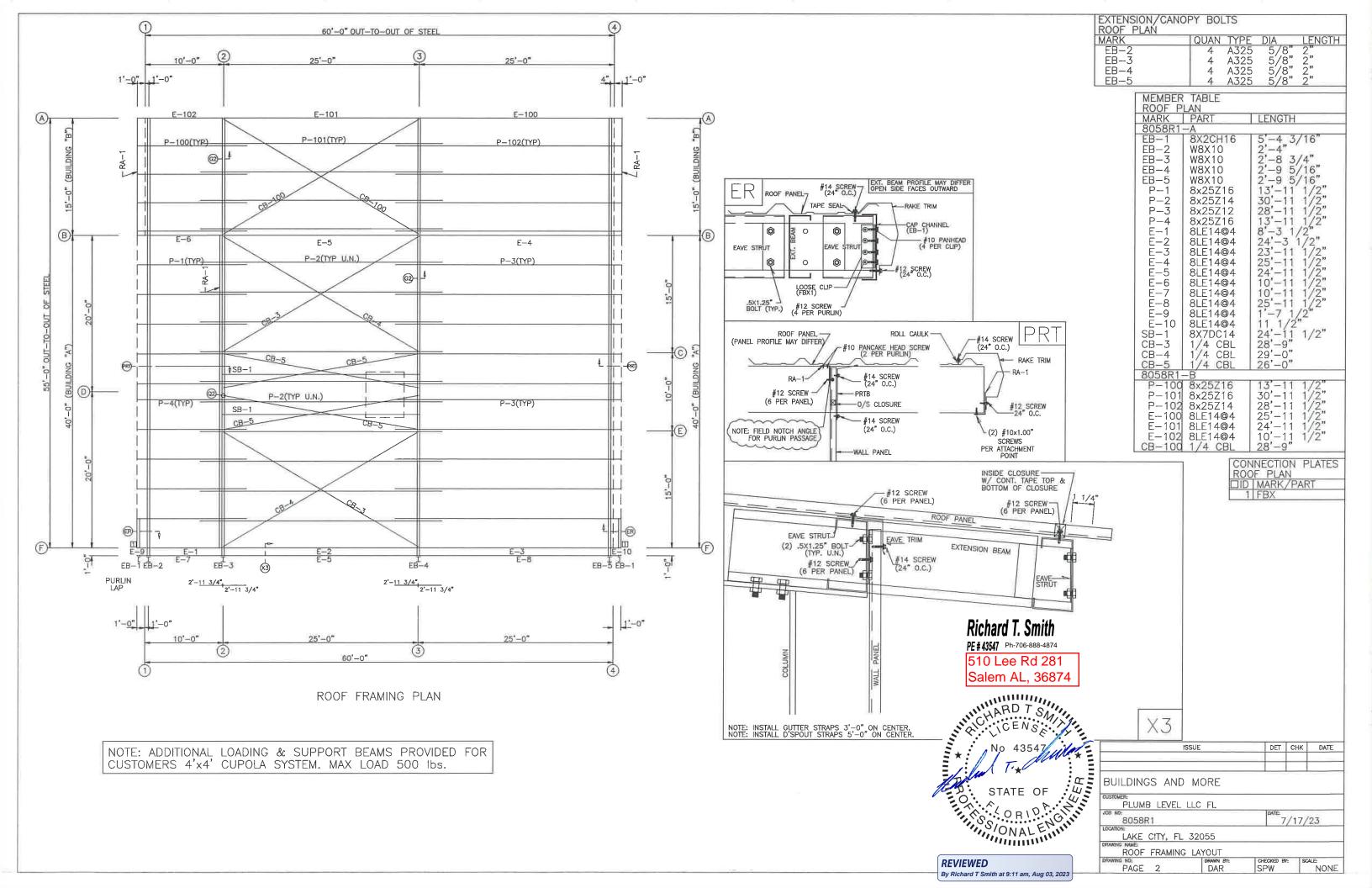


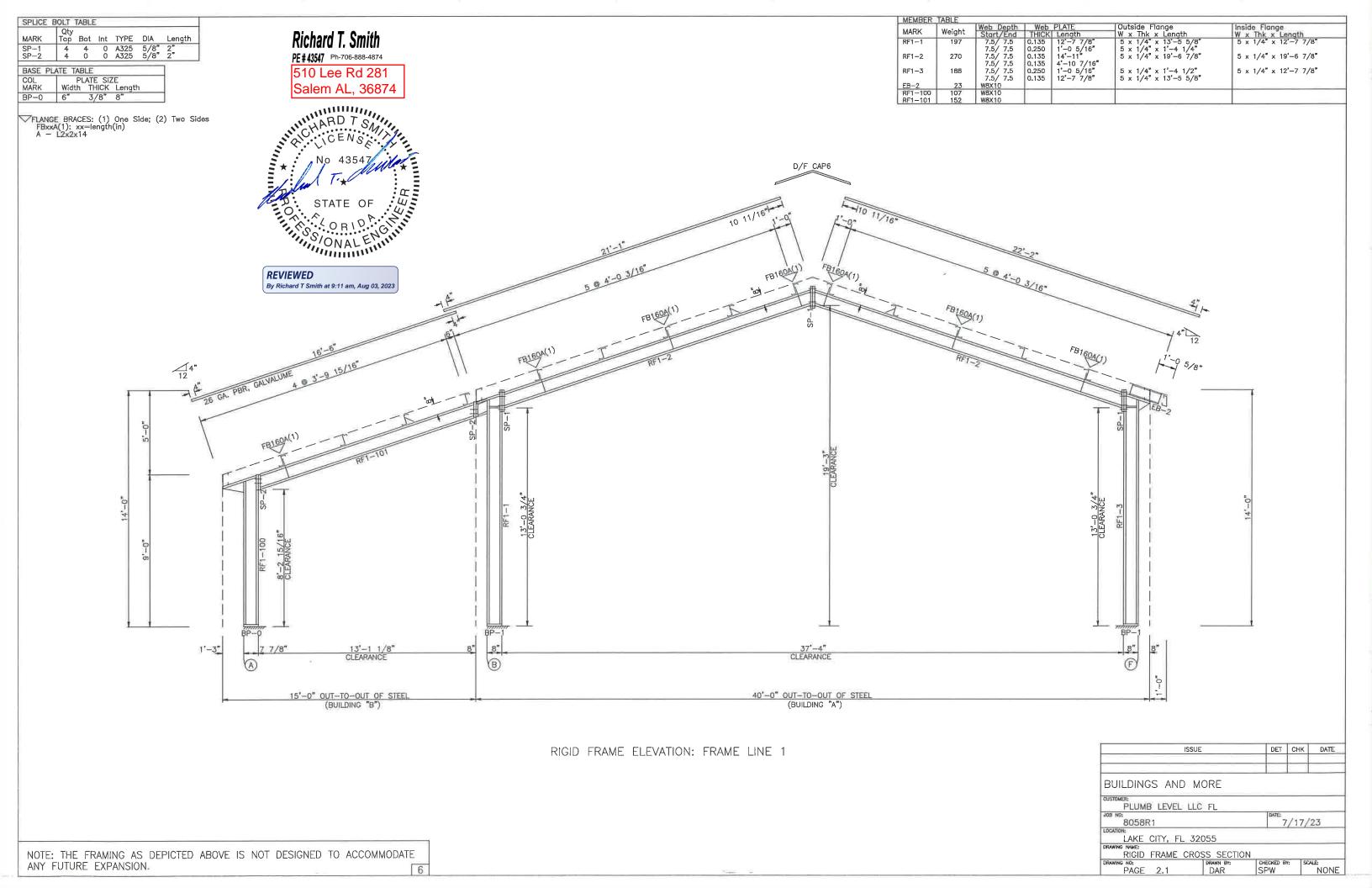
PE # 43547 Ph-706-888-4874

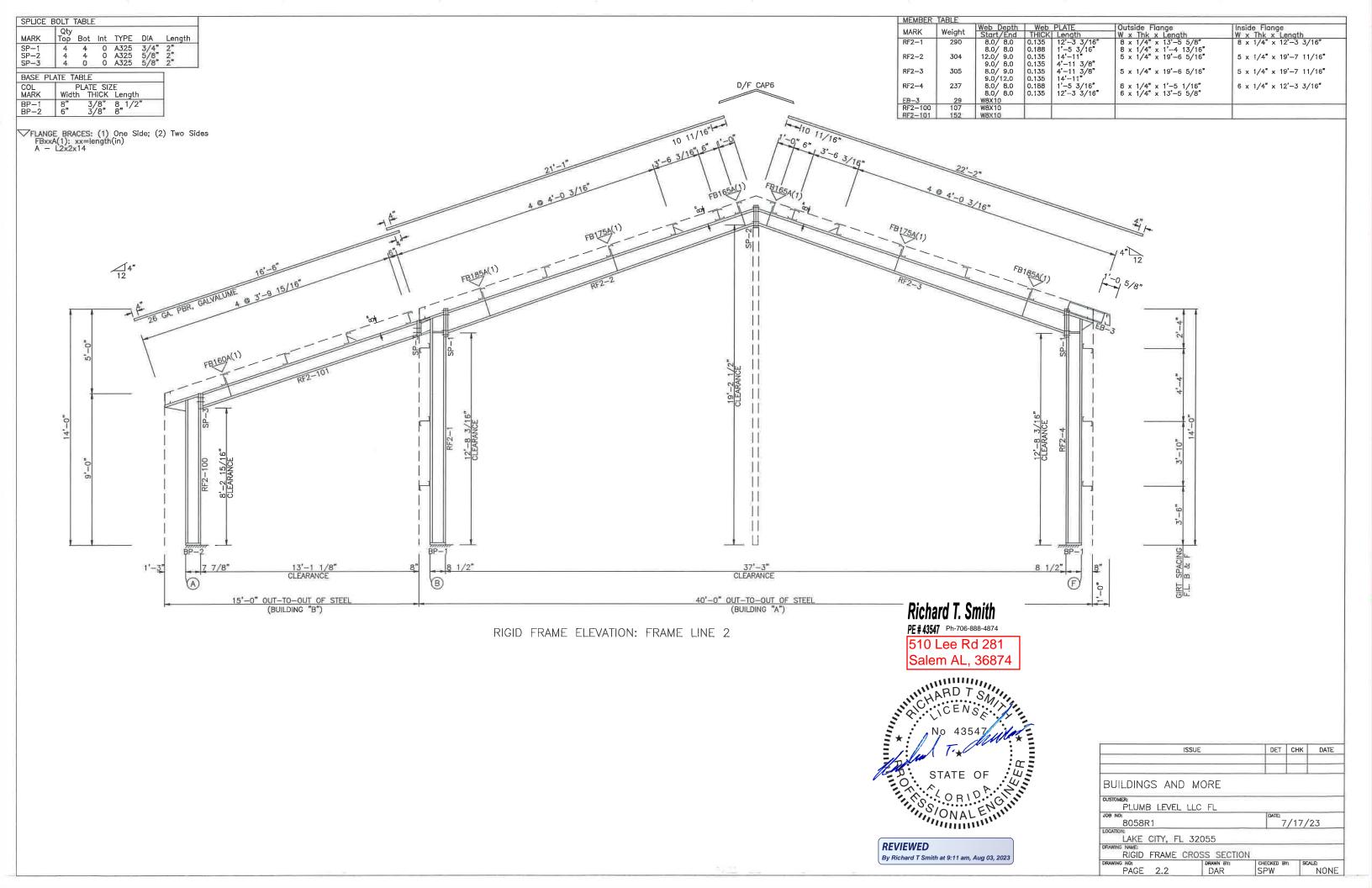
510 Lee Rd 281

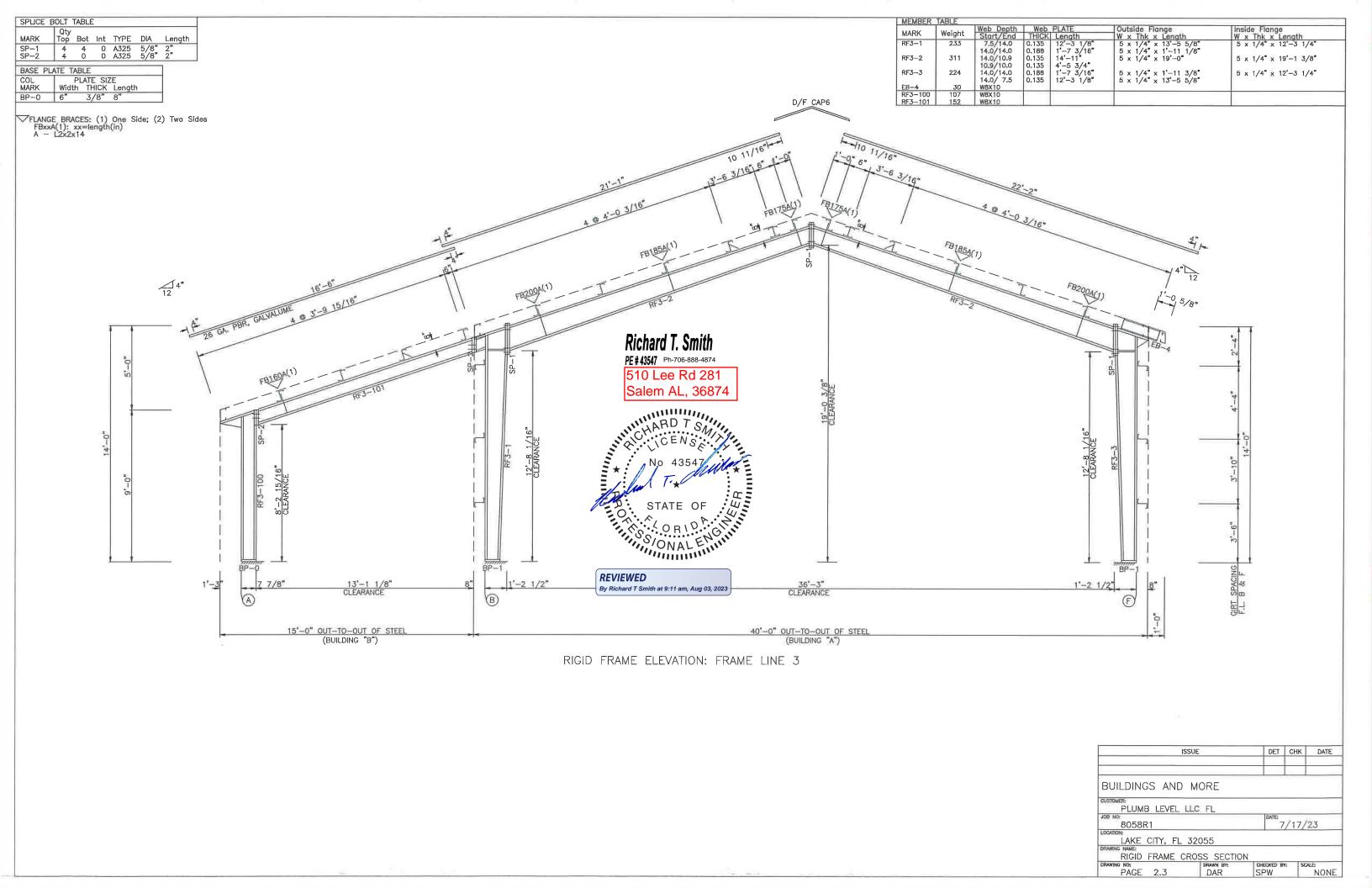


BUILDING "B"					
IS	ISSUE				
BUILDINGS AND	MORE		~7:		
CUSTOMER: PLUMB LEVEL L	LC FL	Tate~			
JOB NO: 8058R1		7 DATE:	/17/	23	
LAKE CITY, FL	32055		0 100		
DRAWING NAME: ANCHOR BOLT I	REACTIONS				
PAGE 1.3	DAR	SPW	: SC	NONE	









| SPLICE BOLT TABLE | | Qty | MARK | Top | Bot | Int | TYPE | DIA | Length | SP-1 | 4 | 0 | 0 | A325 | 5/8" | 2" |

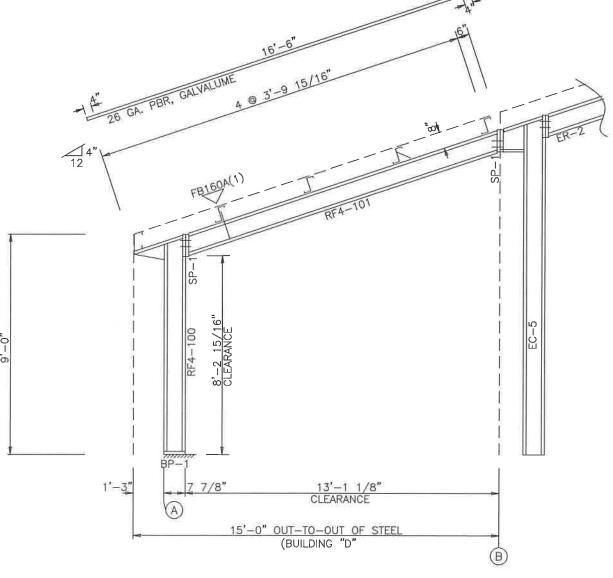
FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

 MEMBER SIZE TABLE

 MARK
 MEMBER
 LENGTH
 WEIGHT

 RF4-100
 W8X10
 8'-8 9/16"
 107

 RF4-101
 W8X10
 13'-9 9/16"
 152



RIGID FRAME ELEVATION: FRAME LINE 4

Richard T. Smith
PE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874

NO 43547

NO 43547

NO 43547

NO HIDA

ORIDA

REVIEWED

By Richard T Smith at 9:11 am, Aug 03, 2023

	DET	СНК	DATE	
BUILDINGS AND	MORE			
CUSTOMER: PLUMB LEVEL	LLC FL			
^{ЈОВ NO:} 8058R1		DATE:	/17/2	23
LAKE CITY, FL	32055			
DRAWING NAME: RIGID FRAME	CROSS SECTIO	N		
DRAWING NO: PAGE 2.4	DRAWN BY:	SPW	SCA	NONE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

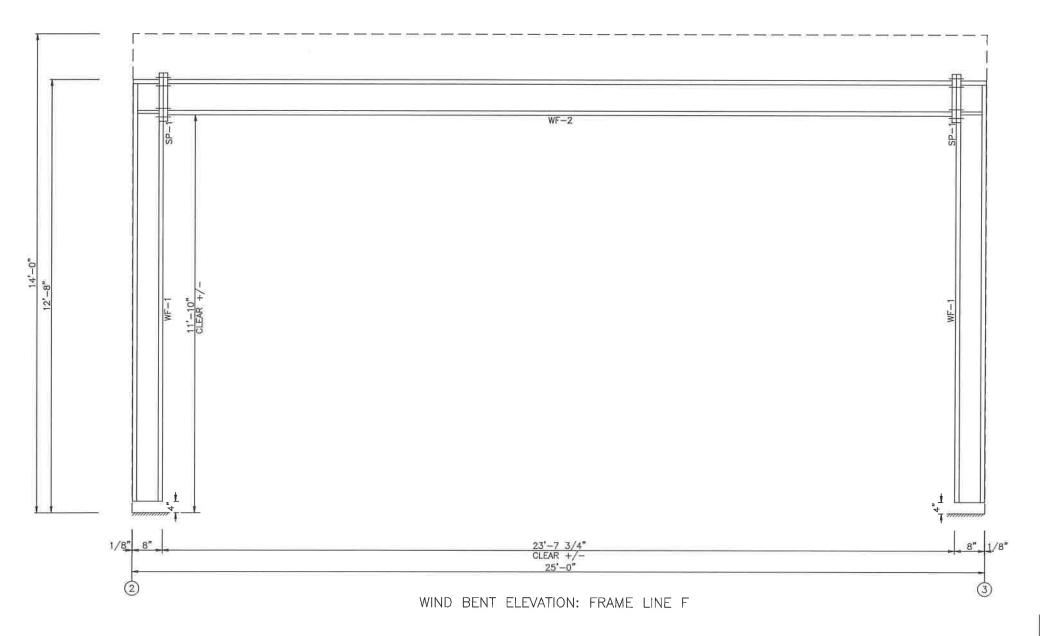
| SPLICE BOLTS | Splice | Quan | -----Bolt----- | Mark | Top/ Bot | Type | Dia | Length | SP- 1 | 4 | 4 | A325 | 5/8" | 2" |

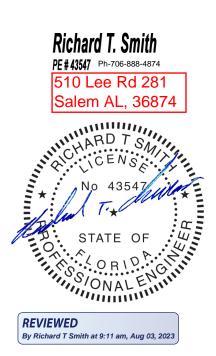
 MEMBER SIZE TABLE

 MARK
 MEMBER
 LENGTH

 WF-2
 B10651
 23'-7 1/2"

 WF-1
 B08541
 12'-4"





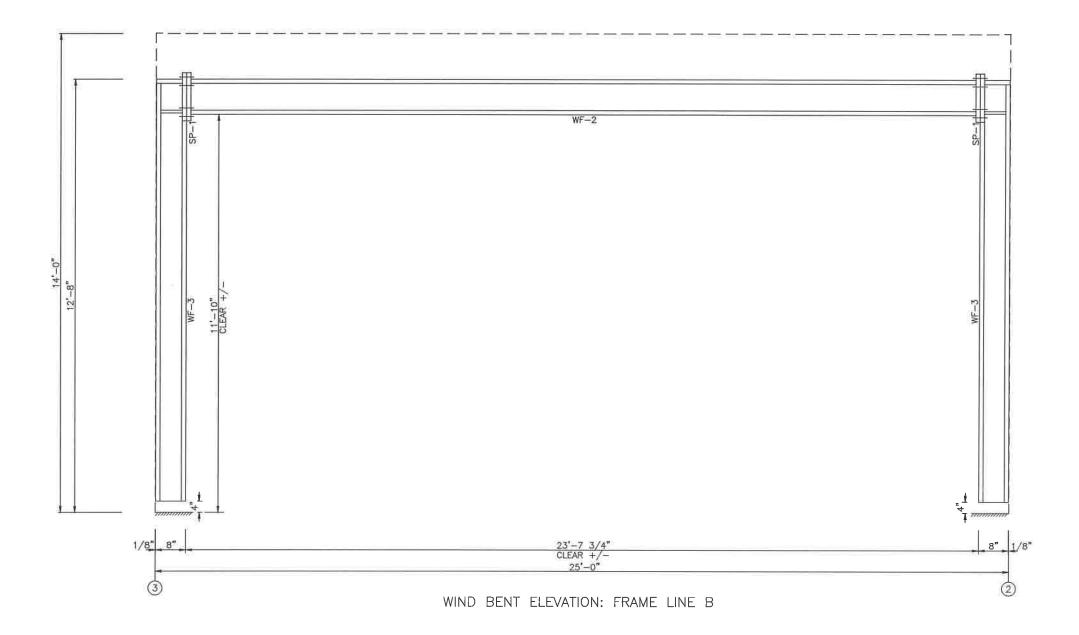
BUILDING "A"				
ISSUE		DET	СНК	DATE
BUILDINGS AND MO	ORE	•		
PLUMB LEVEL LLC	FL			
JOB NO: 8058R1		DATE:	/17/	/23
LAKE CITY, FL 320	 055	1.1	······	
DRAWING NAME: RIGID FRAME CROS	SS SECTION			
PAGE 2.5	DRAWN BY:	CHECKED BY	r: S	NONE

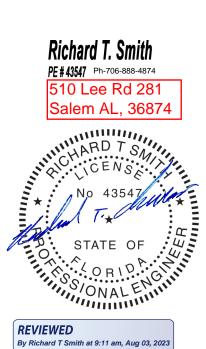
 SPLICE BOLTS

 Splice Mark
 Quan Top/ Bot Type
 Dia Length

 SP- 1
 4
 4
 A325
 5/8"
 2"

MARK MEMBER LENGTH
WF-2 B10551 23'-7 1/2"
WF-3 B08541 12'-4"





BUILDING "A"

ISSUE DET CHK DATE

BUILDINGS AND MORE

CUSTOMEPS:
PLUMB LEVEL LLC FL

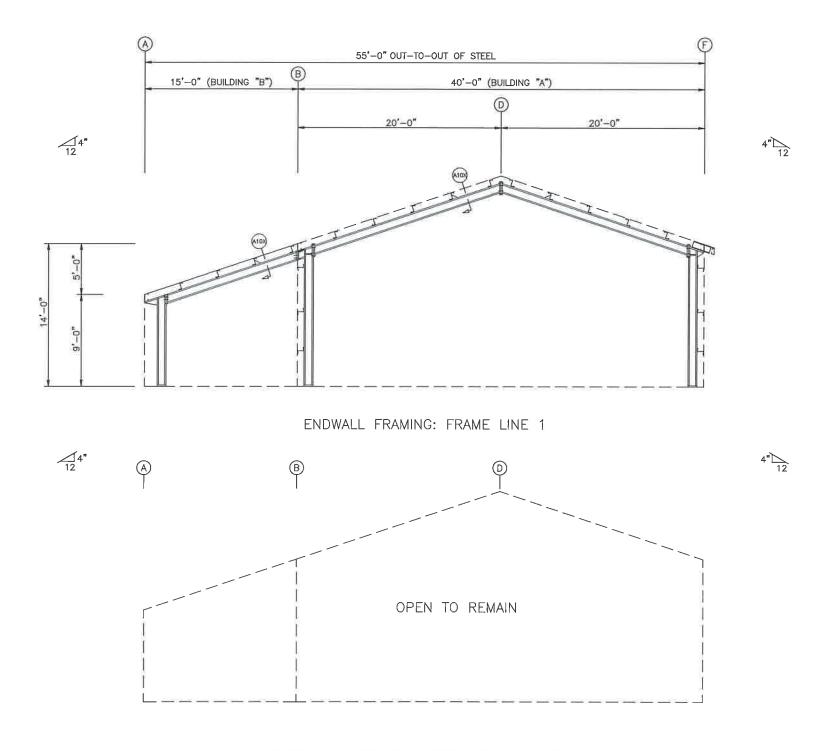
JOB NO:
BO58R1

LOCATIONS:
LAKE CITY, FL 32055

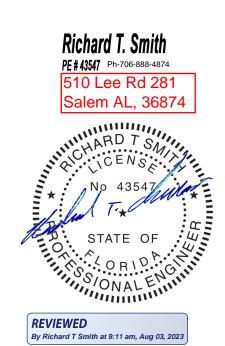
DRAWNIN PAME:
RIGID FRAME CROSS SECTION

DRAWN NO:
PAGE 2.6

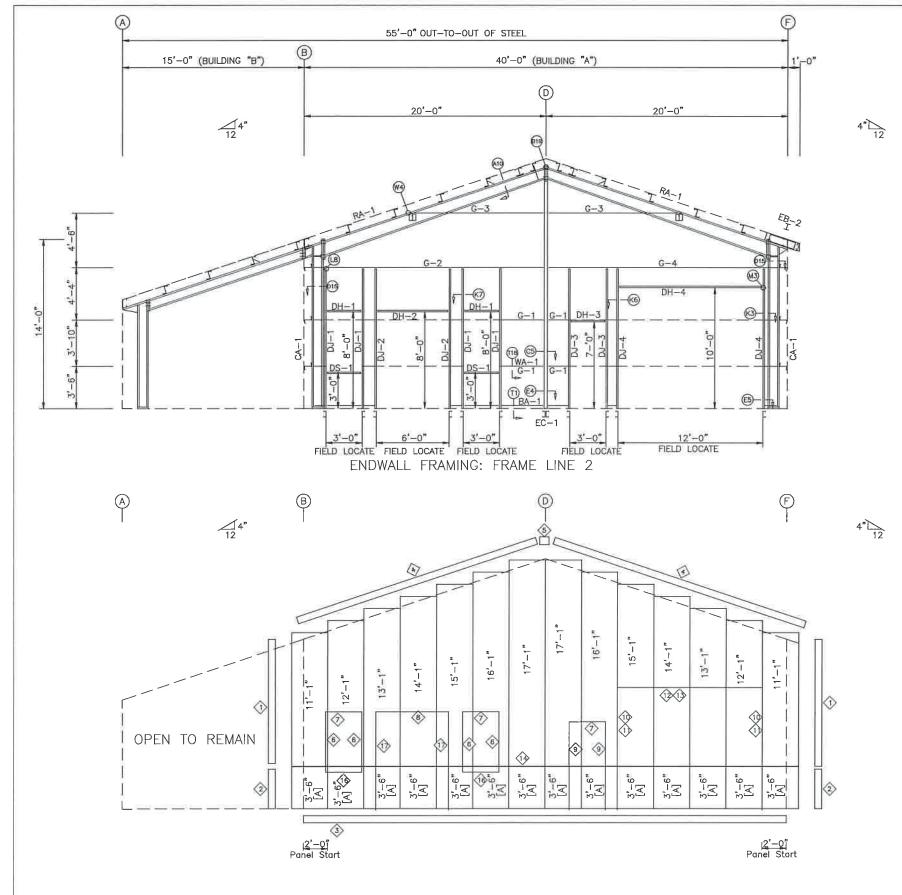
DRAWN BY:
CHECKED BY:
SCALE
NONE



ENDWALL SHEETING & TRIM: FRAME LINE 1



IS	SUE	DET	CHK	DATE		
BUILDINGS AND	MORE					
CUSTOMER: PLUMB LEVEL L	LC FL					
JOB NO: 8058R1	JOB NO:					
LAKE CITY, FL	32055		/17/:			
FRAMING & SHE	EETING LAYO	UT				
DRAWING NO:	DRAWN BY:	CHECKED IN	: SCA	NONE		



ENDWALL SHEETING & TRIM: FRAME LINE 2

PANELS: 26 Ga. R - RUSTIC RED

[A] PANELS: 26 Ga. R - BURNISHED SLATE

BOLT TABLE FRAME LINE LOCATION QUAN TYPE DIA Columns/Rat TRIM TABLE FRAME LINE ◇ID PART DID PART LENGTH
1 0/S CORN 10'-8"
2 0/S CORN 3'-8"
3 BASE TRM 20'-3"
4 PRT8 4'-6"
5 PRT8 3'-0"
6 R JAMB 5'-3"
7 PLEAD 3'
3" DETAIL TRIM_5 TRIM_5 TRIM_16 TRIM_8 7 R HEAD TRIM_61 8 R HEAD TRIM_61 9 R JAMB TRIM_8 10 CT8 TRIM_11 11 R JAMB TRIM_8 12 CT8 13 R HEAD 14 WCT-2 TRIM_10 TRIM_61 TRIM_20 16 R HEAD 17 R JAMB TRIM_7 TRIM 8

MEMBER FRAME	:: <u>:</u>	
MARK	PART	LENGTH
EB-2	W8X10	2'-4"
EC-1	8X7DC14	18'-8" 1/16"
DJ-1 DJ-2	8X25C16 8X25C16	11'-8" 11'-8"
DJ-3	8X25C16	11'-8"
DJ-4	8X35C12	11'-8"
DH-1	8.3.5CH6	3'-0"
DH-2	8X25C16	6'-0"
DH-3	8X25C16	3'-0"
DH-4 DS-1	8X35C16	12'-0" 3'-0"
G-1	8.3.5CH6 8x25Z16	18'-3"
G-1	8x25712	18'-3"
$\tilde{G}-\tilde{3}$	8x25Z16	7'-11 7/16"
G-4	8x25Z14	18'-3"

CONNECTION PLATES FRAME LINE 1

ID MARK/PART

1 SGC-1

Richard T. Smith
PE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874

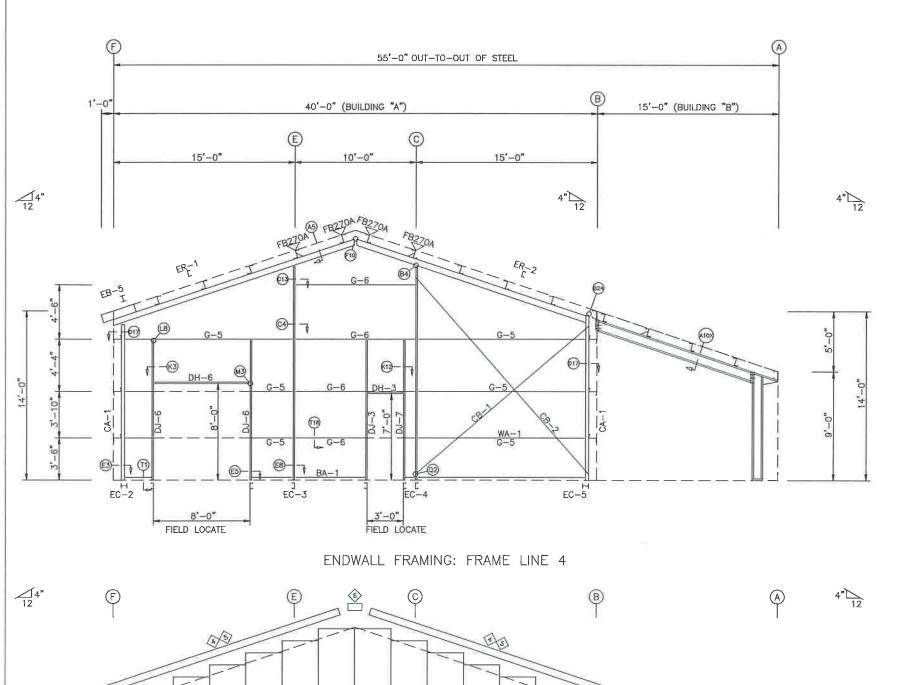
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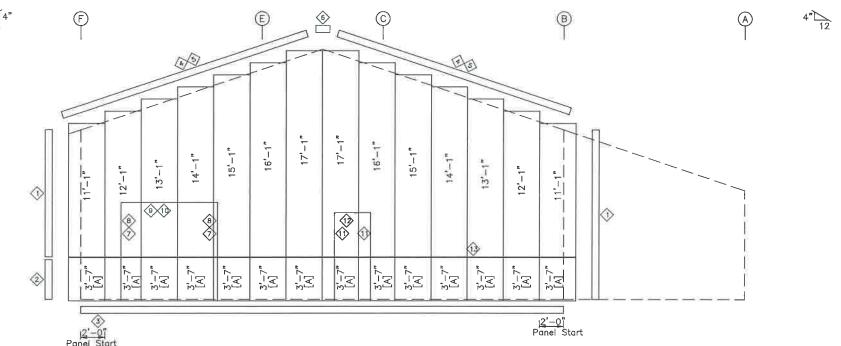
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By Richard T Smith at 9:11 am, Aug 03, 2023

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING

	IS	SUE	DET	СНК	DATE
	BUILDINGS AND	MORE			
	CUSTOMER: PLUMB LEVEL L	LC FL			
	JOB NO: 8058R1		DATE: 7/	17/:	23
	LAKE CITY, FL	32055			
)	FRAMING & SHE	ETING LAYO	JT		
5	PAGE 3.1	DAR	SPW	SCA	NONE





ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Ga. R - RUSTIC RED

[A] PANELS: 26 Ga. R - BURNISHED SLATE

NOTE: FIELD SLOT GIRTS FOR CABLE PASSAGE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

Richard T. Smith
PE#43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874

No 43547 No 43547 No 43547 Tital Crice No. 10 August 10 August

REVIEWED

By Richard T Smith at 9:11 am, Aug 03, 2023

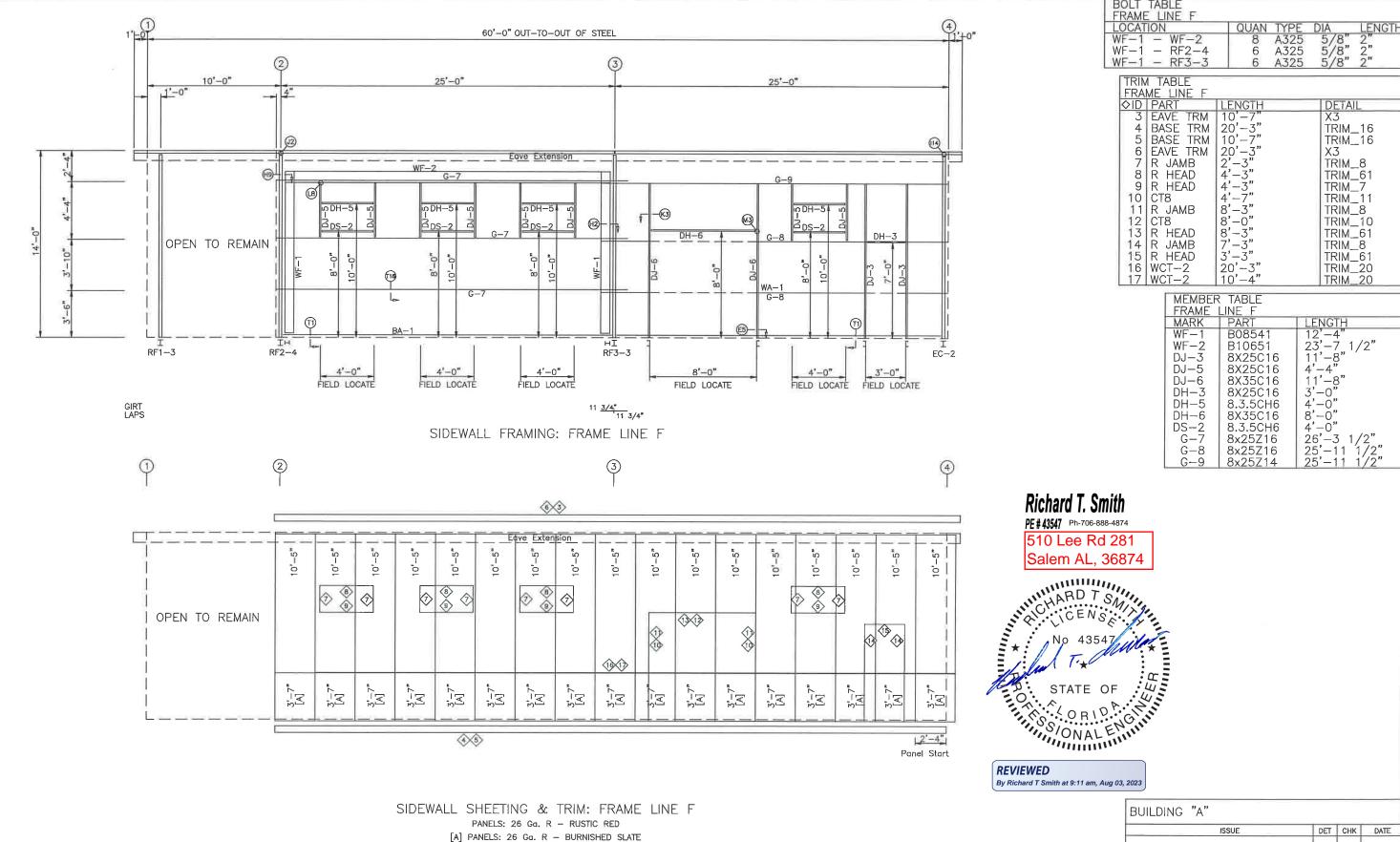
ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT
HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A
DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA.
THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR
ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR
SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD
REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING
REINFORCEMENT.

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
8058R1-A				
EC-2/ER-1	4	A325	5/8"	2"
ER-2/	4	A325	5/8"	2"
ER-1/ER-2	8	A325	5/8"	2"
Int_Column/Raf	2	A325	5′/8"	2"
ER-1/EB-5	4	A325	5/8"	2"

TRIM	1 TABLE		
FRA	ME LINE 4		
	PART	LENGTH	DETAIL
1	O/S CORN	10'-8"	TRIM_5
2	0/S CORN	3'-8"	TRIM 5
2	BASE TRM	20'-3" 5'-0"	TRIM_16
4	PRT8	5'-0"	
5	PRT8	2'-1"	
	PEAK BOX	1'-4"	TRIM_4
7	CT8	4'-7"	TRIM_11
8	R JAMB	8'-3"	TRIM_8
9	CT8	8'-0"	TRIM_10
10	R HEAD	8'-3"	TRIM_61
11	R JAMB	7'-3"	TRIM 8
	R HEAD	3'-3"	TRIM_61
	WCT-2	20'-3"	TRIM_20

	R TABLE LINE 4	
MARK	PART	LENGTH
8058R1	-A	
EB-5 FC-2	W8X10	2'-9 5/16"
EC-2 EC-3	W8X10 8X40C14	13'-8 7/8" 17'-5 1/8"
EC-4	8X40C14	17'-5 1/8"
EC-5	W8X10	13'-0 7/16"
ER-1	8X35C12	19'-10 7/8"
ER-2	8X35C12	21'-3 5/8"
DJ-3	8X25C16	11,-8"
DJ-6	8X35C16	11'-8"
DJ-7 DH-3	8X25C16 8X25C16	3'-0"
DH-6	8X35C16	8'-0"
G-5	8x25Z16	13'-3 5/8"
G-6	8x25Z16	9'-11 1/2"
CB-1	1/4 CBL	1 18'-11"
CB-2	1/4 CBL	22'-0"

IS	SUE	DET	СНК	DATE
BUILDINGS AND	MORE			
All Incomplete in				
CUSTOMER:	10 5			
PLUMB LEVEL I	LC FL			
JOB NO:		DATE:	1471	0.7
8058R1		/	/17/	23
LOCATIONS				
LAKE CITY, FL	32055			
DRAWING NAME:				
FRAMING & SH	EETING LAYOU	JT		
DRAWING NO:	DRAWN BY:	CHECKED IN	SC	LE:
PACE 3.2	DAR	SPW		NICANIE



ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

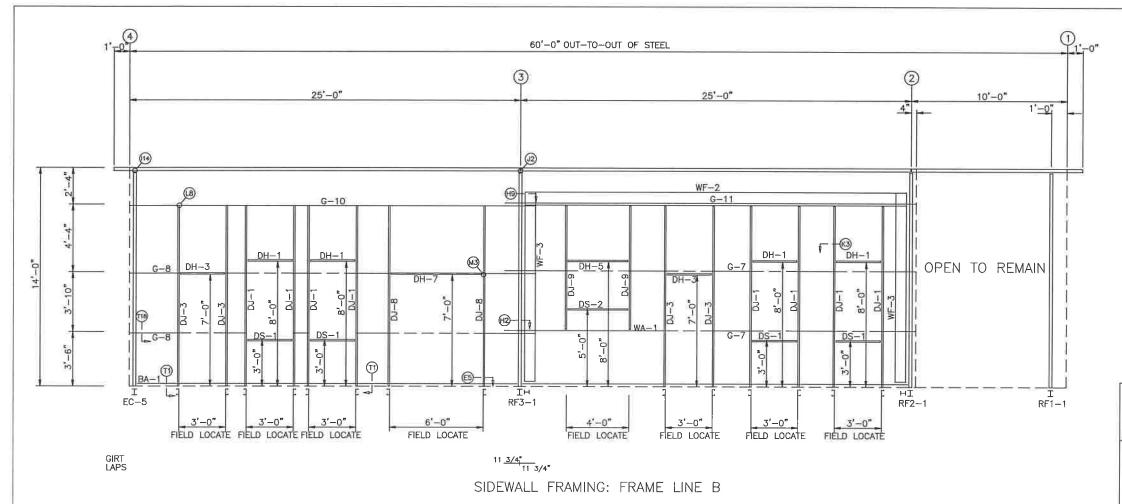
BUILDINGS AND MORE

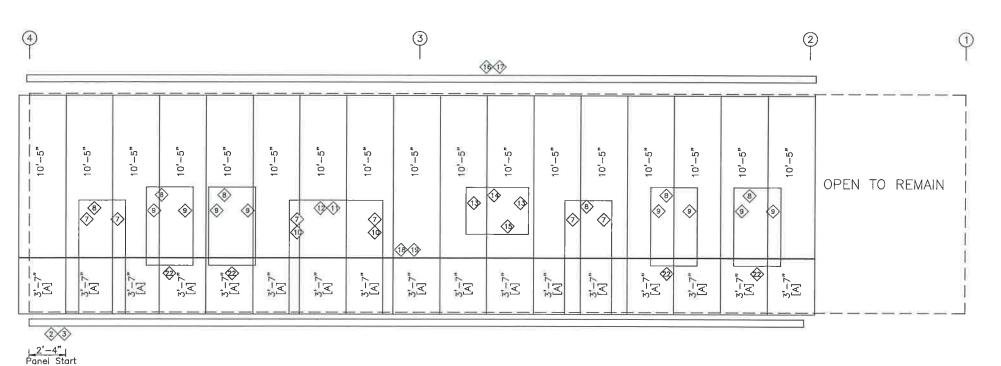
PLUMB LEVEL LLC FL . 8058R1 7/17/23 LAKE CITY, FL 32055

FRAMING & SHEETING LAYOUT PAGE 4

CHECKED BY DAR

NONE





[A] PANELS: 26 Ga. R - BURNISHED SLATE

SIDEWALL SHEETING & TRIM: FRAME LINE B PANELS: 26 Ga. R - RUSTIC RED

BOLT TABLE		
FRAME LINE B	TOLIAN TYPE	DIA LENOTH
LOCATION	QUAN TYPE	DIA LENGTH
WF-3 - WF-2	8 A325	5/8 2
WF-3 - RF3-1	6 A325	
WF-3 - RF2-1	6 A325	5/8" 2"
TRIM TABLE		
FRAME LINE B		
♦ID PART	LENGTH	DETAIL
2 BASE TRM 3 BASE TRM	20'-3"	TRIM16
3 BASE TRM	10'-7" 7'-3"	TRIM_16
l 7 R JAMB I	7'-3 "	TRIM_8
I 8 R HFAD I	3'-3"	TRIM 61
9 R JAMB	5'-3"	TRIM_8
10 CT8	5'-3" 3'-9"	TRIM_11
11 CT8	6'-0"	TRIM_10
12 R HEAD	6'-3"	TRIM_61
13 R JAMB	3'–3"	TRIM 8
14 R HEAD	4'-3"	TRIM_61
15 D LIEAD	4'-3"	
15 R HEAD	4 -5	TRIM_7
	20'-3"	ETD
17 EAVE TRM	10'-7"	ETD
	20'-3"	TRIM_20
19 WCT-2	10'-4"	TRIM_20
22 R HEAD	3'-3"	TRIM_7

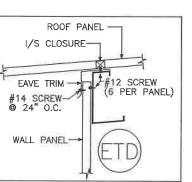
MEMBER TABLE FRAME LINE B MARK PART

B10651

B08541

WF-2

WF-3



LENGTH 23'-7 1/2' 12'-4" 11'-8" 11'-8" 8X25C16 8X25C16 DJ-1 DJ-3DJ-8 8X35C16 DJ-9 DH-1 8X25C16 8.3.5CH6 DH-3 DH-5 8X25C16 8.3.5CH6 DH-7 DS-1 DS-2 8X35C16 8.3.5CH6 8.3.5CH6 8x25Z16 26'-3 1/2" 25'-11 1/2" 25'-11 1/2" 26'-3 1/2" 8x25Z16 8x25Z12 8x25Z12 G-8 Ğ-10

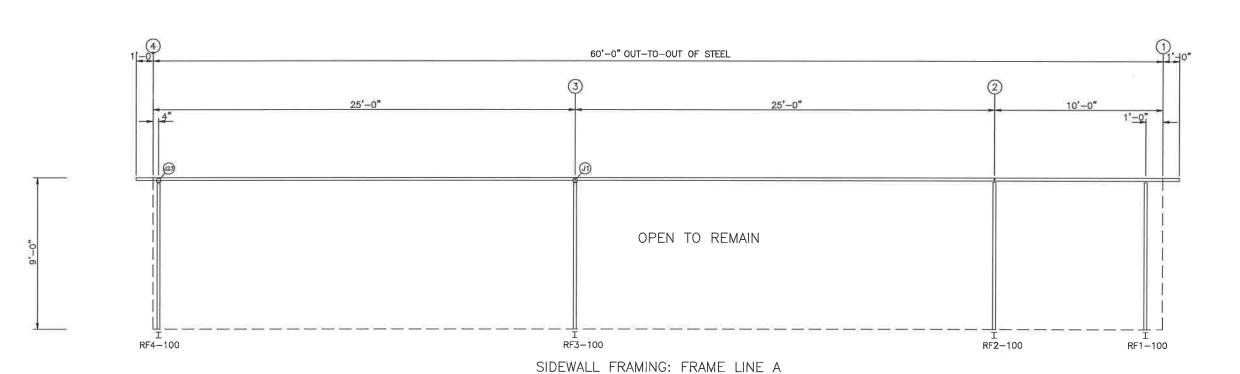
Richard T. Smith

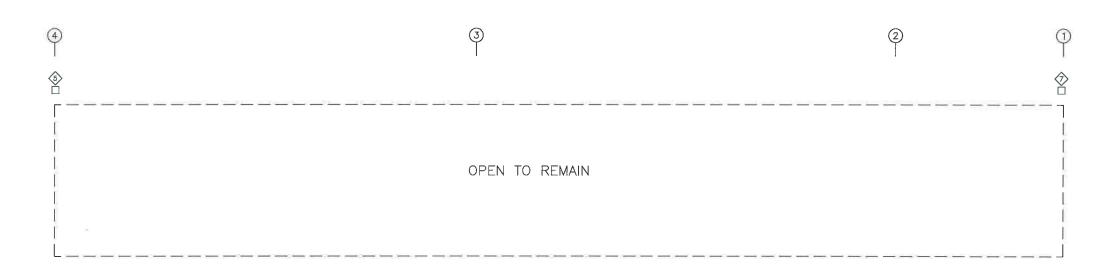
PE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874

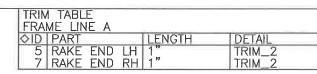
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JE	DET C	CHK DATE
JORE		
.C FL		
	DATE: 7/	17/23
2055		
TING LAYO	JT	
DRAWN BY: DAR	SPW	NONE
	MORE C FL 2055 ETING LAYOU DRAWN BY:	MORE C FL DATE: 7/ 2055 TING LAYOUT DIFFAMEN BY: CHECKED BY:



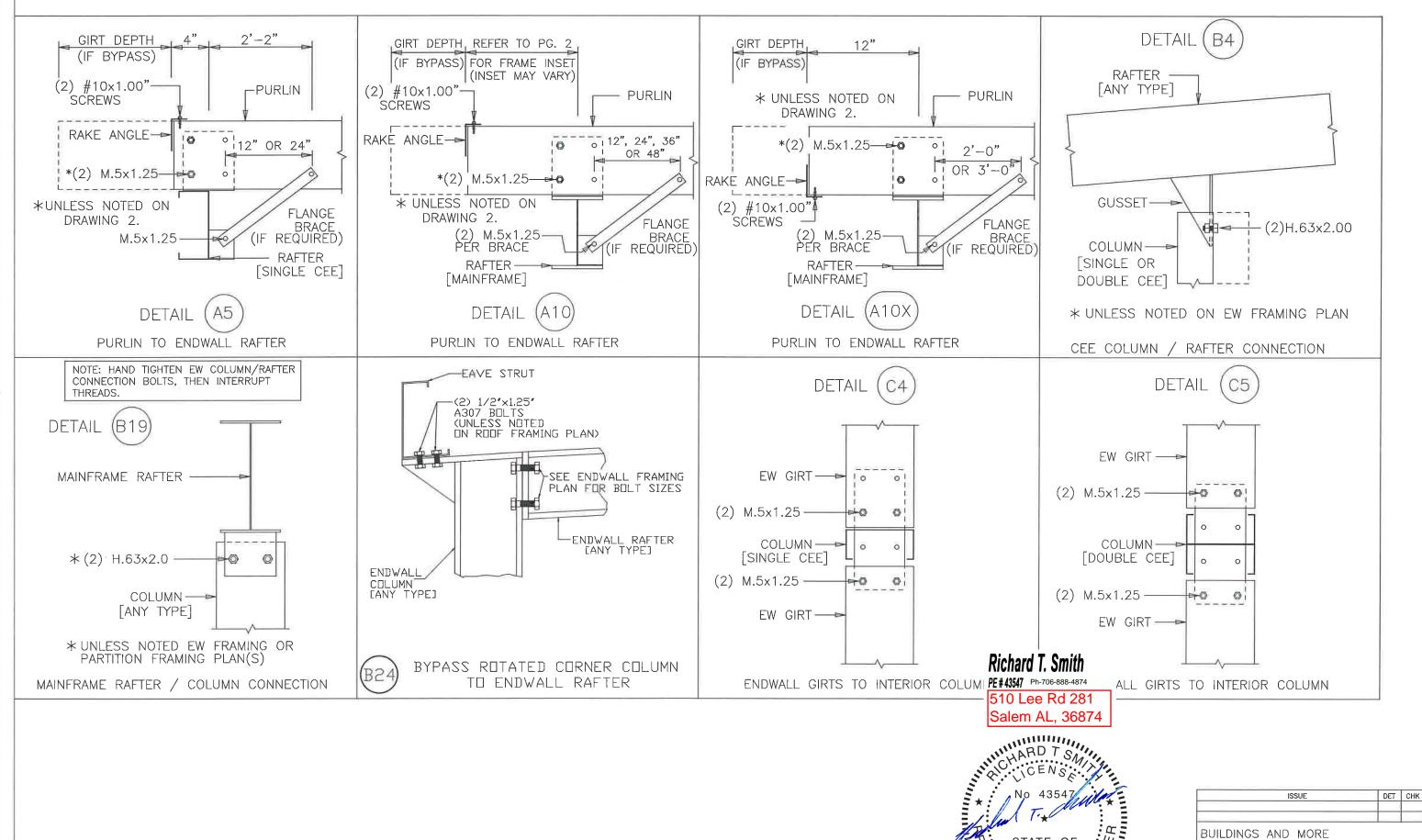


SIDEWALL SHEETING & TRIM: FRAME LINE A





UILDING "B"				
ISS	:UE	DET	СНК	DATE
			-	
UILDINGS AND	MORE			
PLUMB LEVEL L	LC FL			
NO: 8058R1		DATE:	/17/:	23
LAKE CITY, FL 3	32055	7,		
WING RAME: FRAMING & SHE	ETING LAYOU	JT		
WING NO: PAGE 4.2	DRAWN BY:	SPW	: SCA	NONE



BUILDINGS AND MORE

CUSTOMER:
PLUMB LEVEL LLC FL

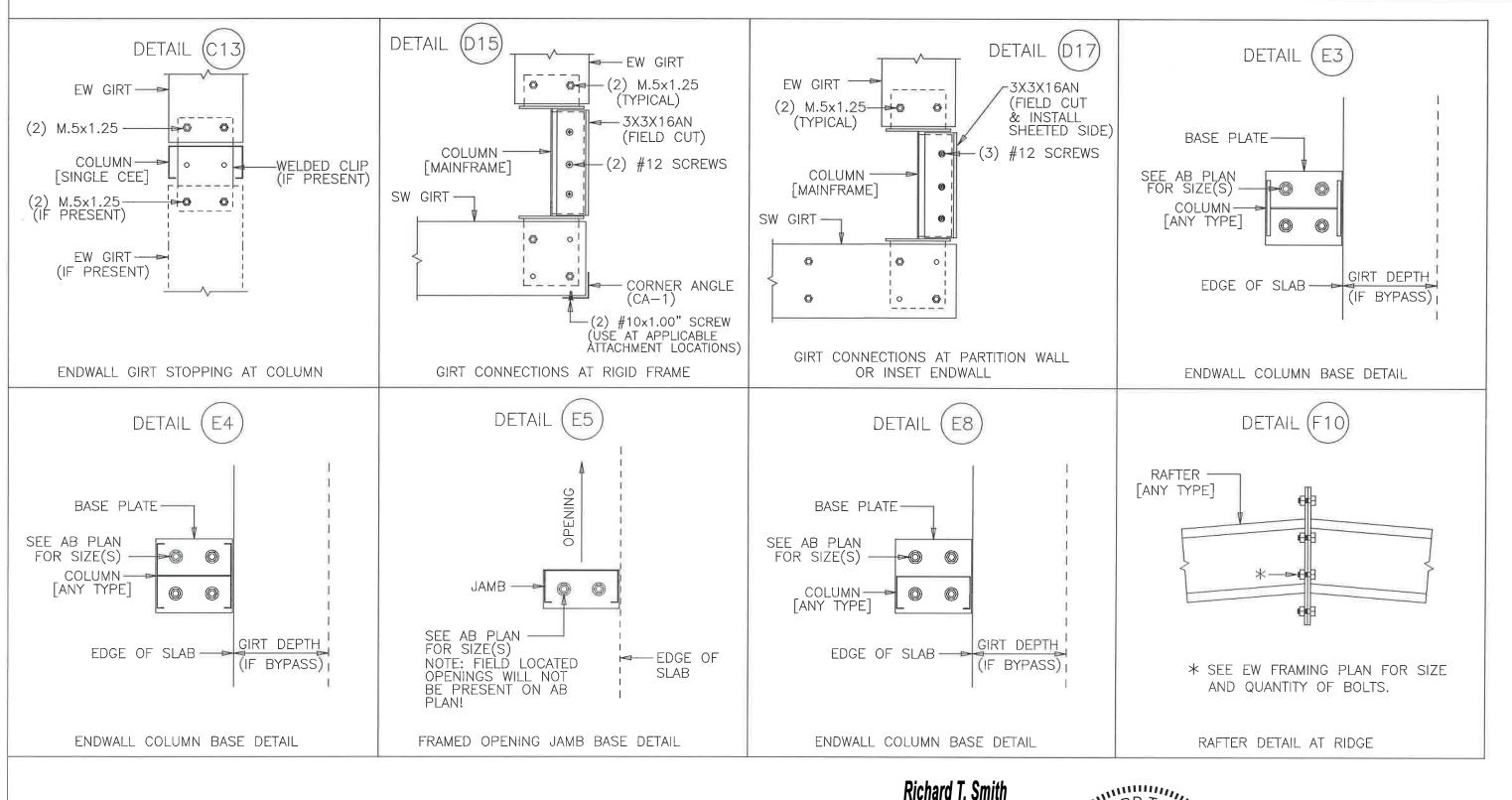
JOB NO:
8058R1

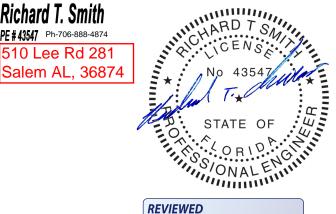
LAKE CITY, FL 32055

DRAWING NAME:
FRAMING DETAILS

DRAWING NO:
PAGE 5

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CHECKED BY:
NONE

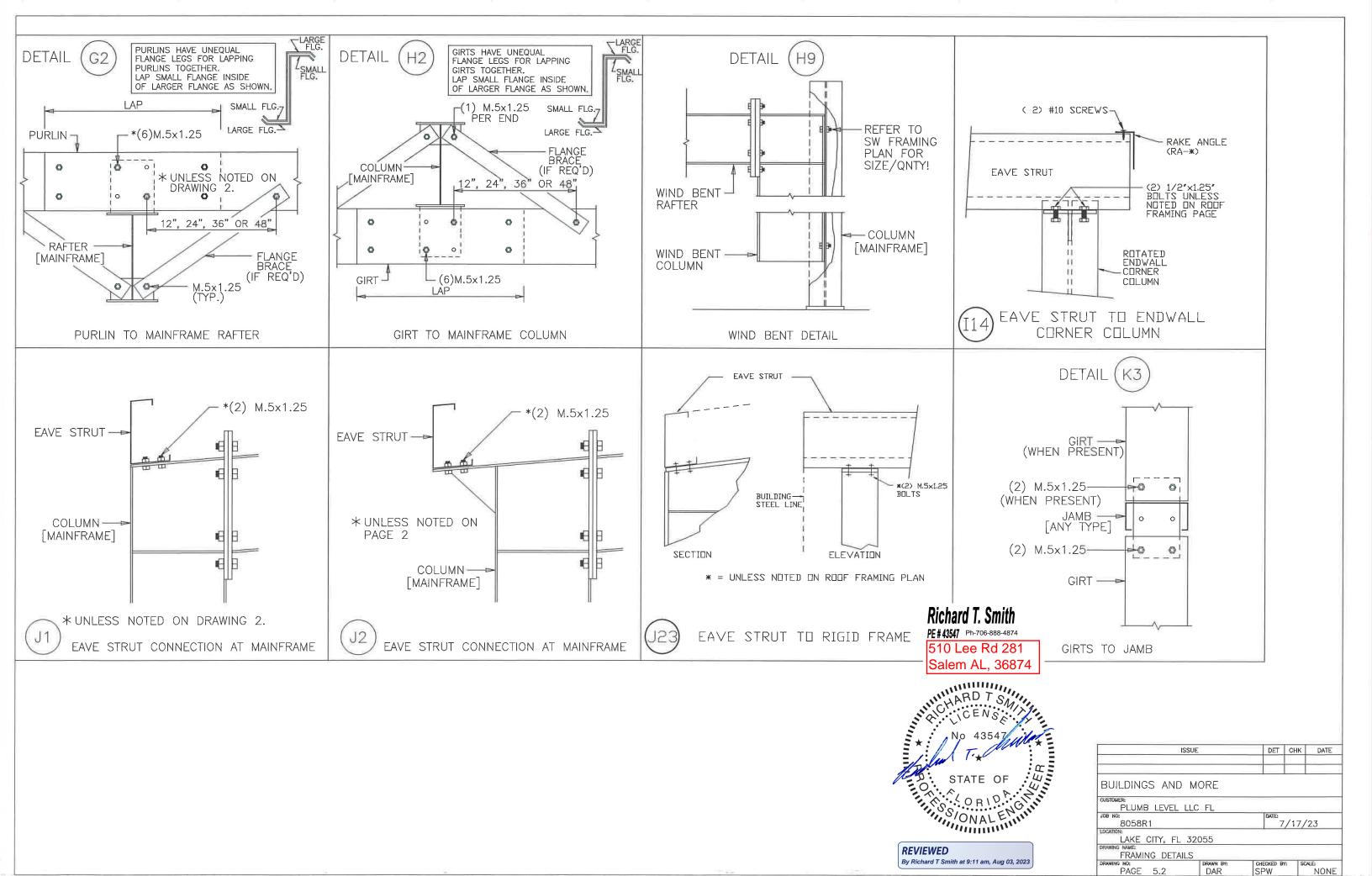




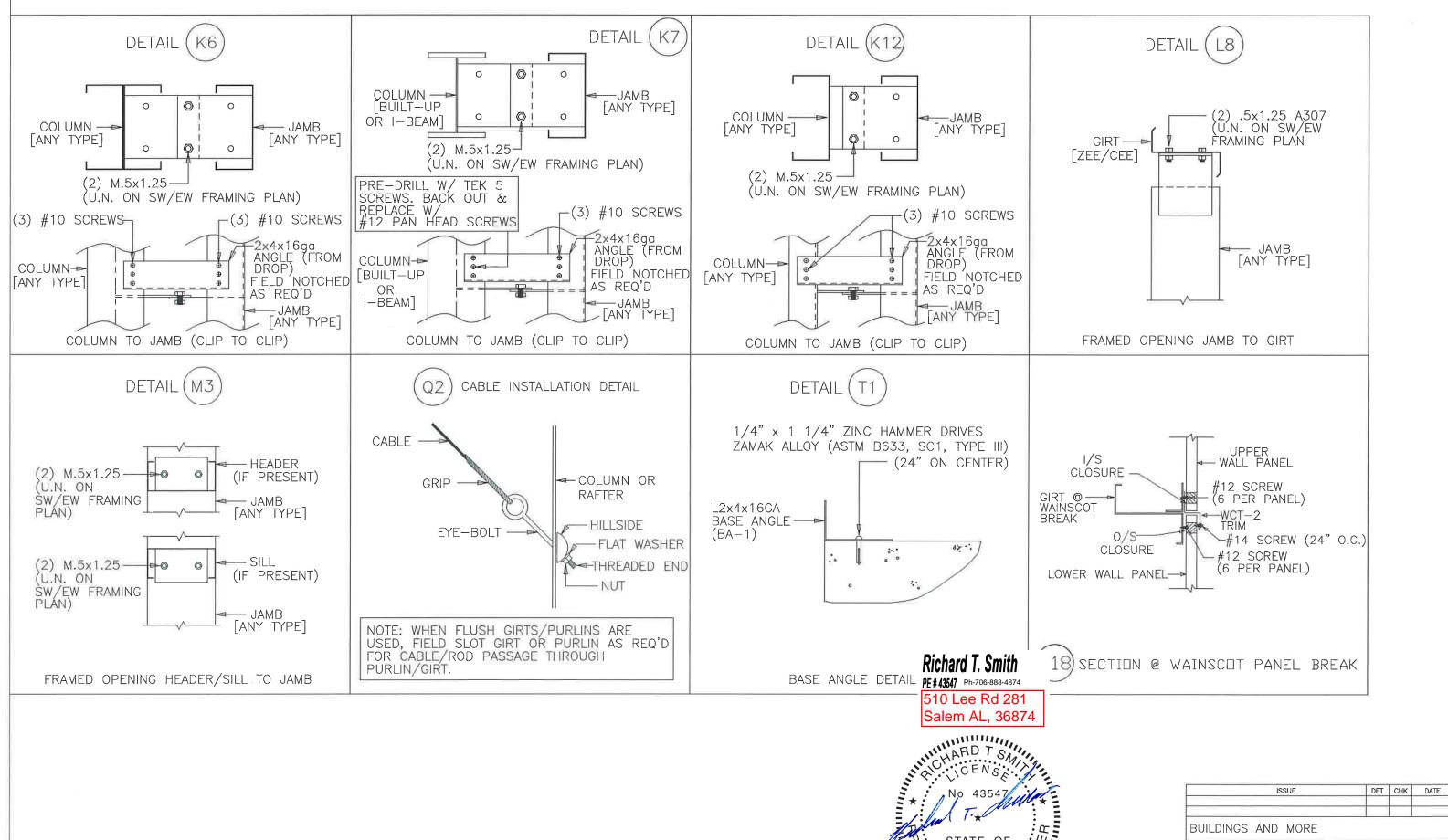
By Richard T Smith at 9:11 am, Aug 03, 2023

PE # 43547 Ph-706-888-4874 510 Lee Rd 281

> DET CHK DATE ISSUE BUILDINGS AND MORE PLUMB LEVEL LLC FL 7/17/23 8058R1 LAKE CITY, FL 32055 FRAMING DETAILS CHECKED BY: SPW PAGE 5.1



PAGE 5.2



BUILDINGS AND MORE

CUSTOMER:
PLUMB LEVEL LLC FL

8058R1

LOCATION:
LAKE CITY, FL 32055

DRAWING NOIS:
FRAMING DETAILS

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PAGE 5.3

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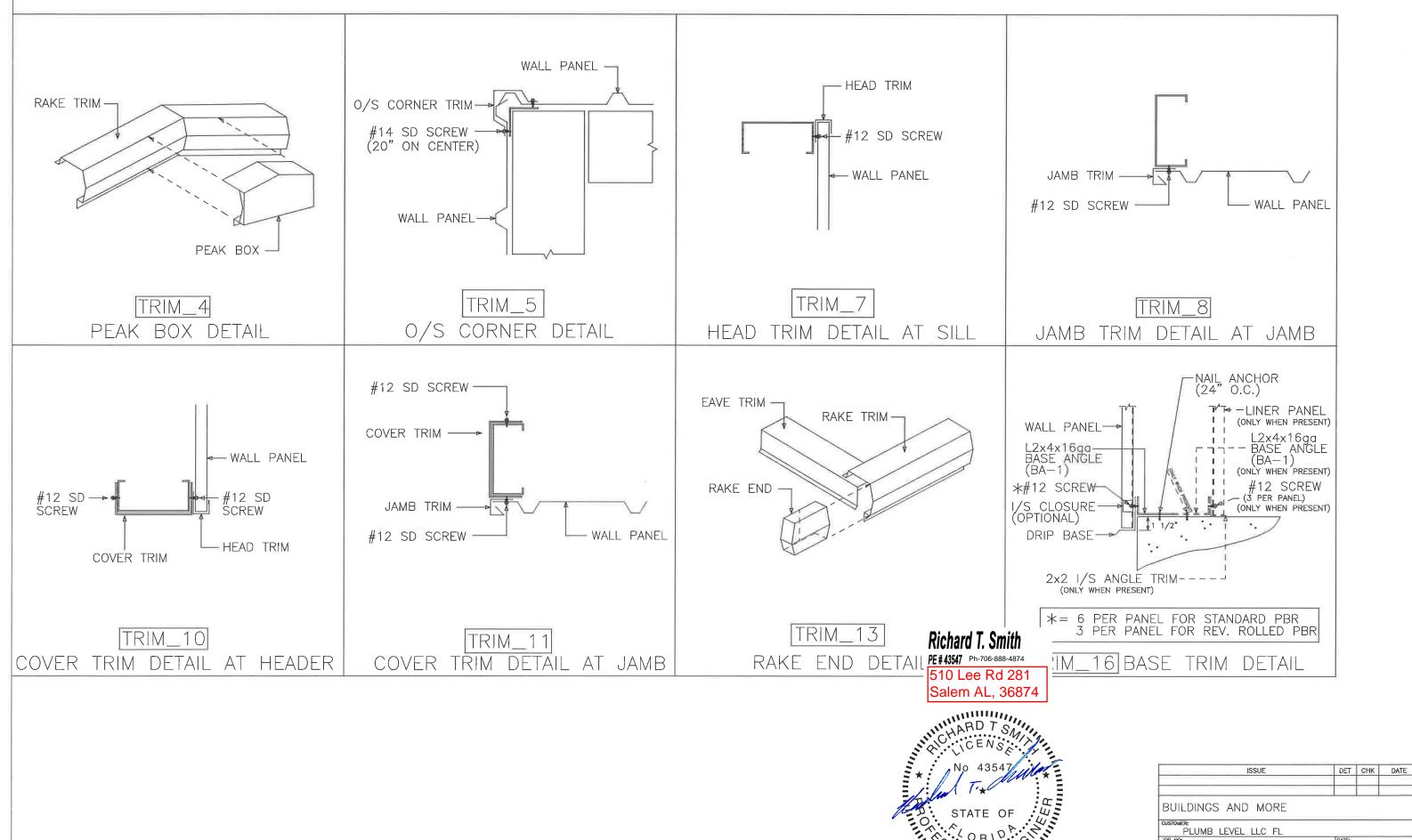
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By Richard T Smith at 9:11 am, Aug 03, 2023

REVIEWED



BUILDINGS AND MORE

CUSTOMER:
PLUMB LEVEL LLC FL

JUGG NO:
8058R1

LOCATION:
LAKE CITY, FL 32055

DEMAND: MANE:
FRAMING DETAILS

DEMAND: NO:
PAGE 5.4

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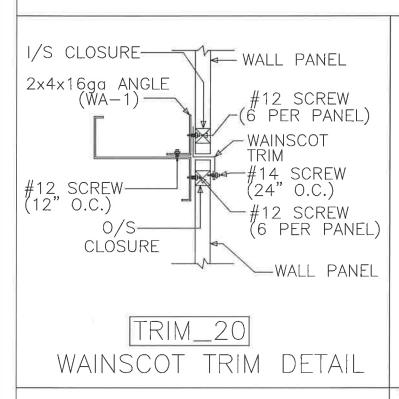
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NONE

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By Richard T Smith at 9:11 am, Aug 03, 2023

REVIEWED

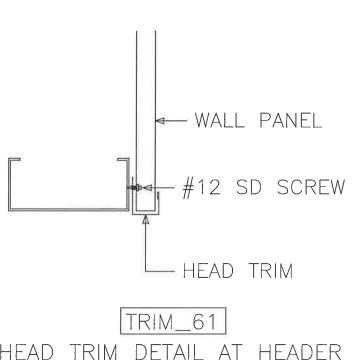


STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.



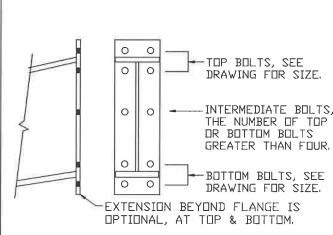
MORTISE PREPPED

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

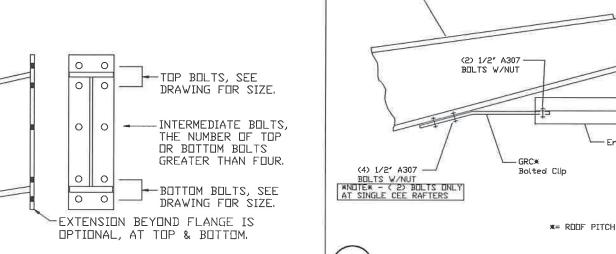
PERSONNEL DOORS

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.



BOLTED END PLATE CONNECTION

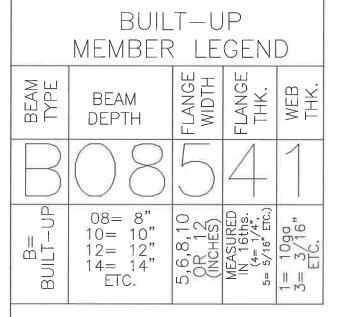


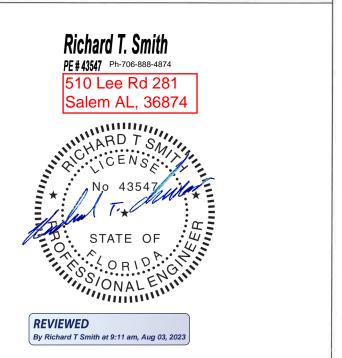


SECTION OF ENDWALL GIRT TO RAFTER

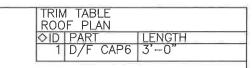
Endwall Girt

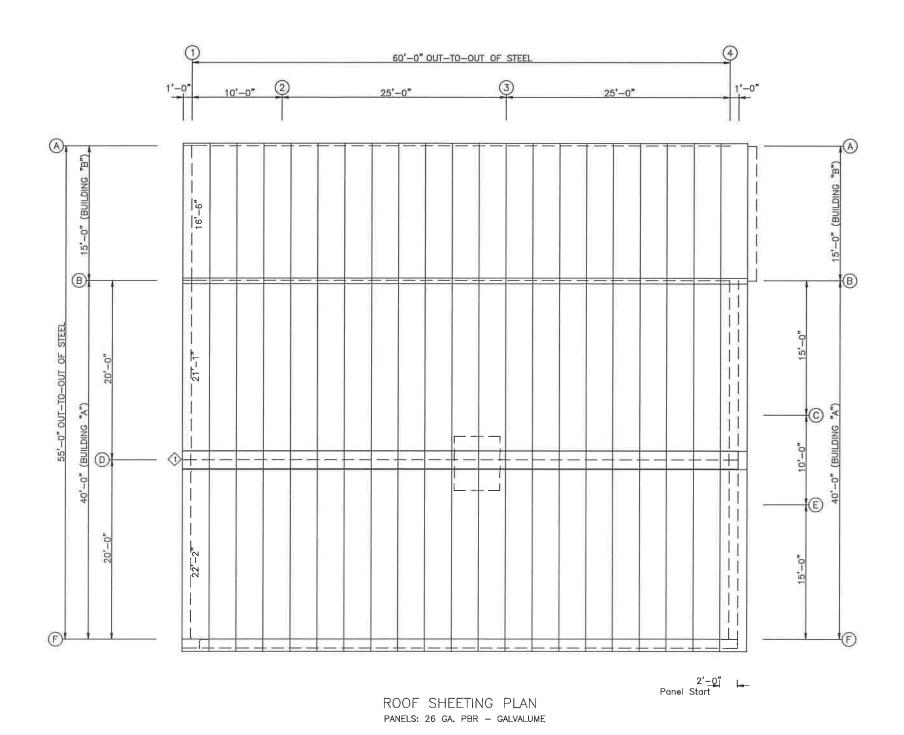
-Rigid Frame Rafter





ISSU	DET	СНК	DATE	
BUILDINGS AND M	MORE			
CUSTOMER: PLUMB LEVEL LL	C FL			
8058R1		DATE:	/17/	23
LAKE CITY, FL 3:	2055		- 1	
FRAMING DETAILS				
PAGE 5.5	DAR	SPW	SC	ALE: NONE



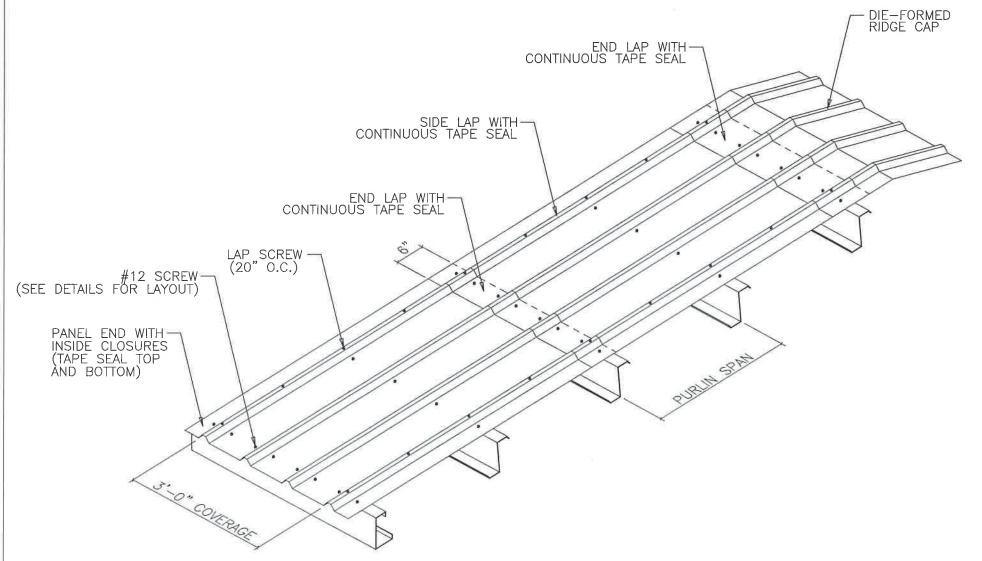


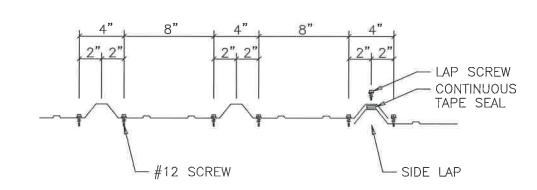
Richard T. Smith

PE#43547 Ph-706-888-4874 510 Lee Rd 281 Salem AL, 36874



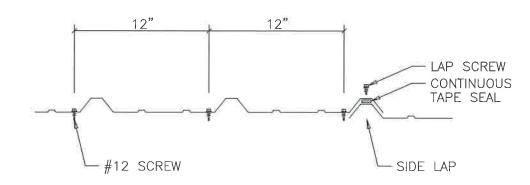
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BUILDINGS AND M	10RE		
CUSTOMER: PLUMB LEVEL LLC	C FL		
JOB NO: 8058R1		DATE: 7,	/17/23
LAKE CITY, FL 32	2055		
ROOF PANELS &	TRIM	1175 - 1111 A. S. S.	
DRAWING NO: PAGE 6	DAR	SPW	SCALE: NONE





PANEL ATTACHMENT AT PANEL END

(PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

NOTES:

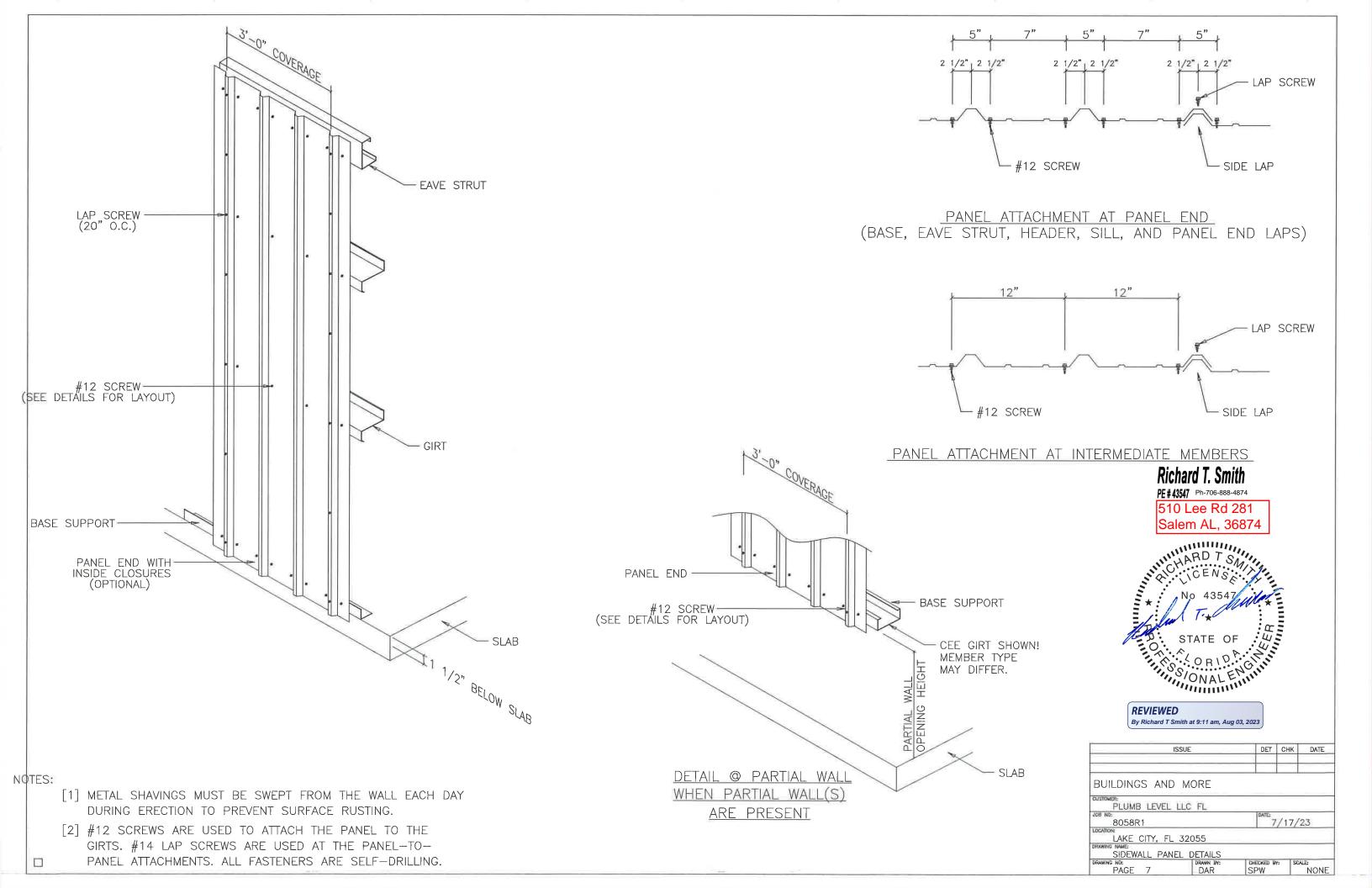
- [1] ALL END LAPS MUST BE A MINIMUM OF 6".
- [2] METAL SHAVINGS MUST BE SWEPT FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

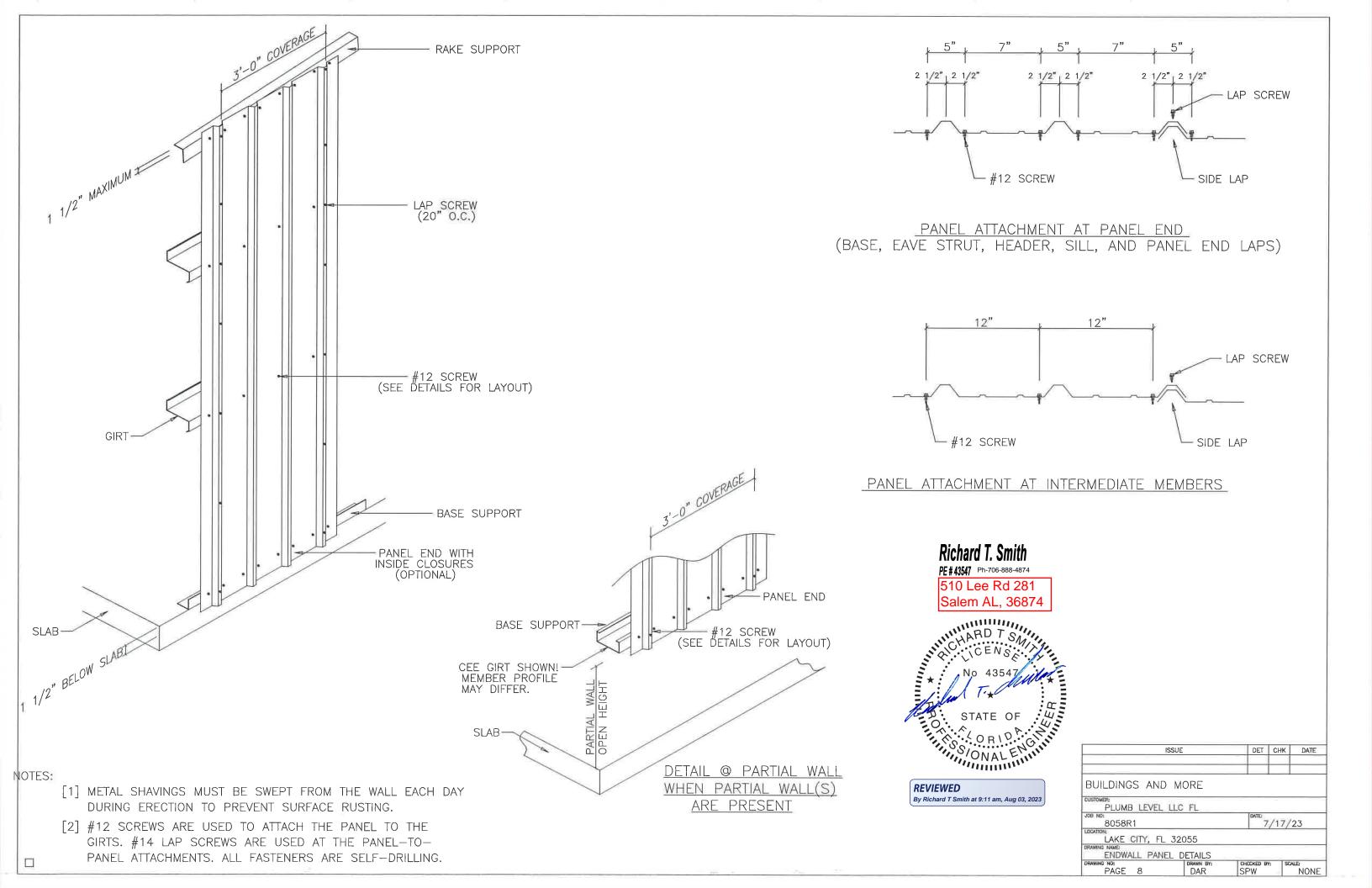
Richard T. Smith

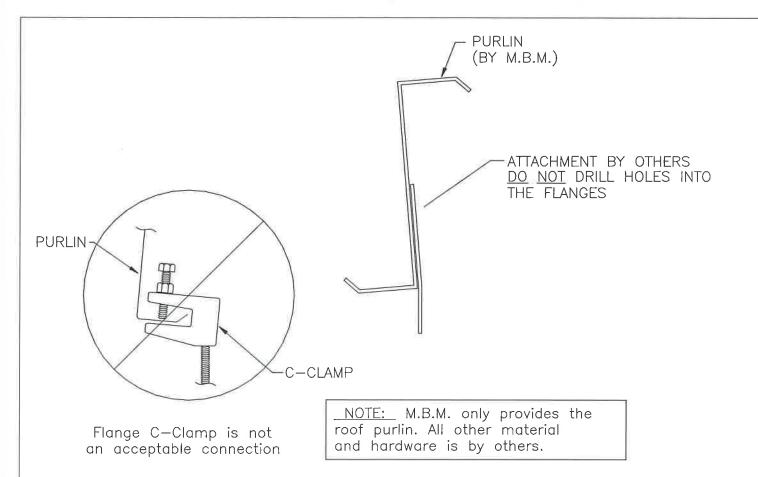
FE#43547 Ph-706-888-4874 510 Lee Rd 281 Salem AL, 36874



ISSUE		DET	CHK	DATE
BUILDINGS AND	MORE			
CUSTOMER: PLUMB LEVEL LE	_C FL			
JOB NO:		DATE:		
8058R1		7	/17/	23
LAKE CITY, FL 3	2055			
ROOF PANEL DE	TAILS	787732344 733575		
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PAGE 6.1	DAR	SPW		NONE







Recommended Connection Detail

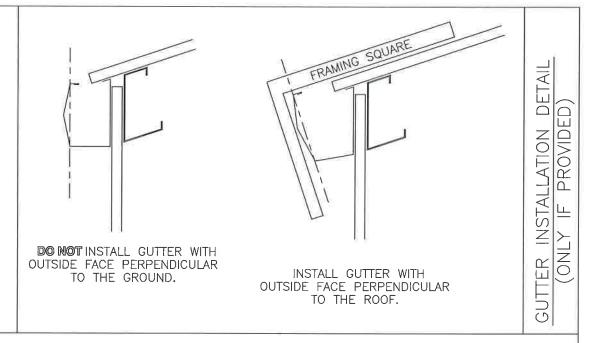
NOTE

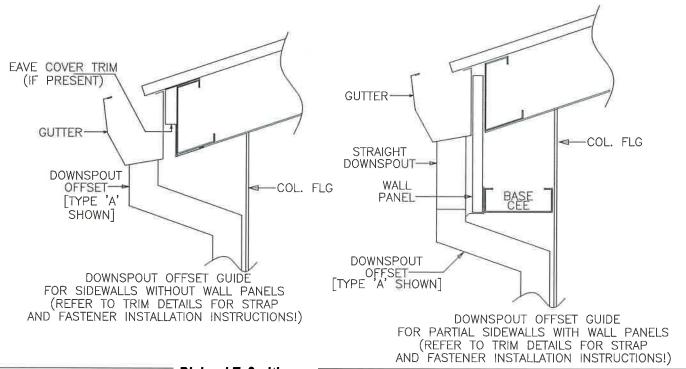
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT. OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS







510 Lee Rd 281 Salem AL, 36874



BUILDINGS AND MORE

PLUMB LEVEL LLC FL 7/17/23 8058R1

SPW

DET CHK DATE

LAKE CITY, FL 32055

SPECIAL DETAILS

ISSUE