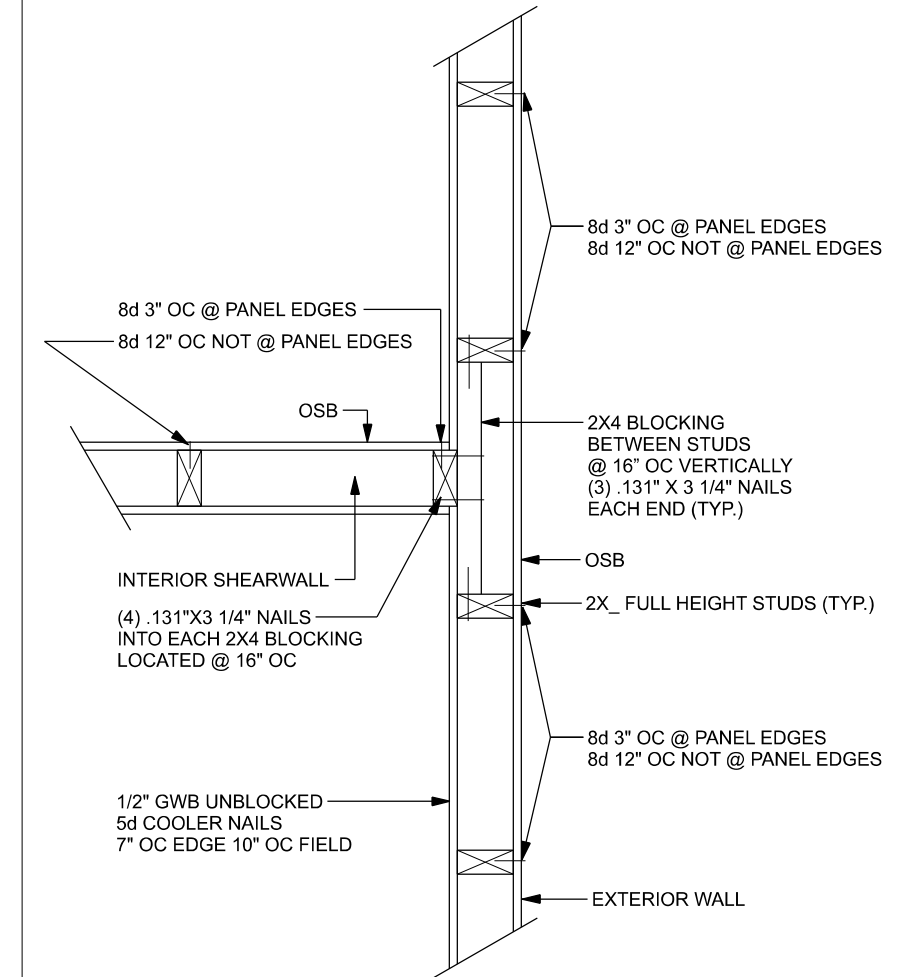
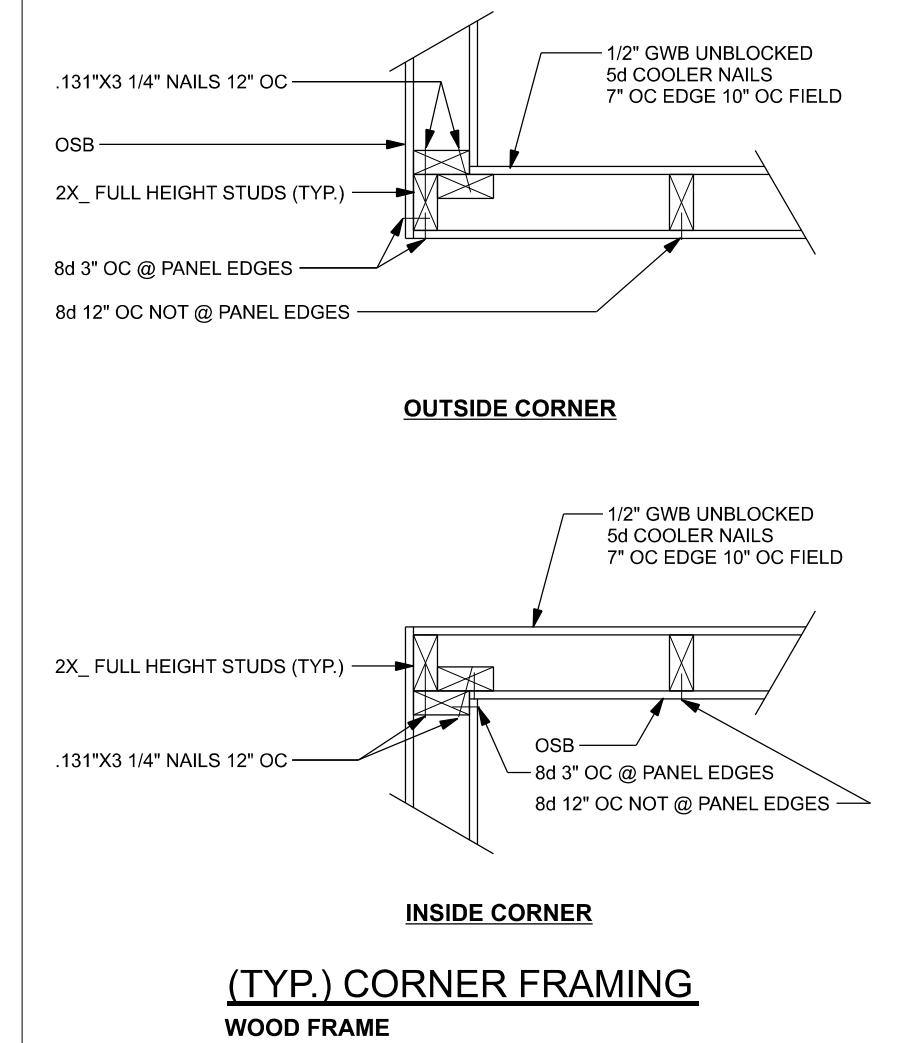


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



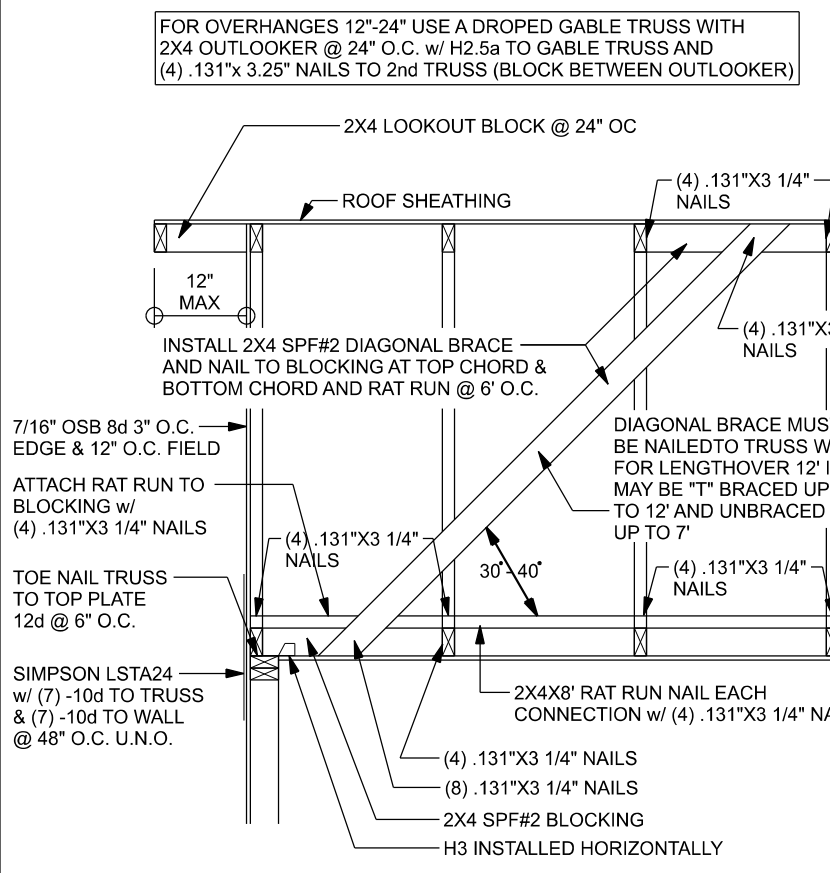
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2 3/8" x 0.113") or ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including a feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



(TYP.) GABLE BRACING DETAIL
WOOD FRAME

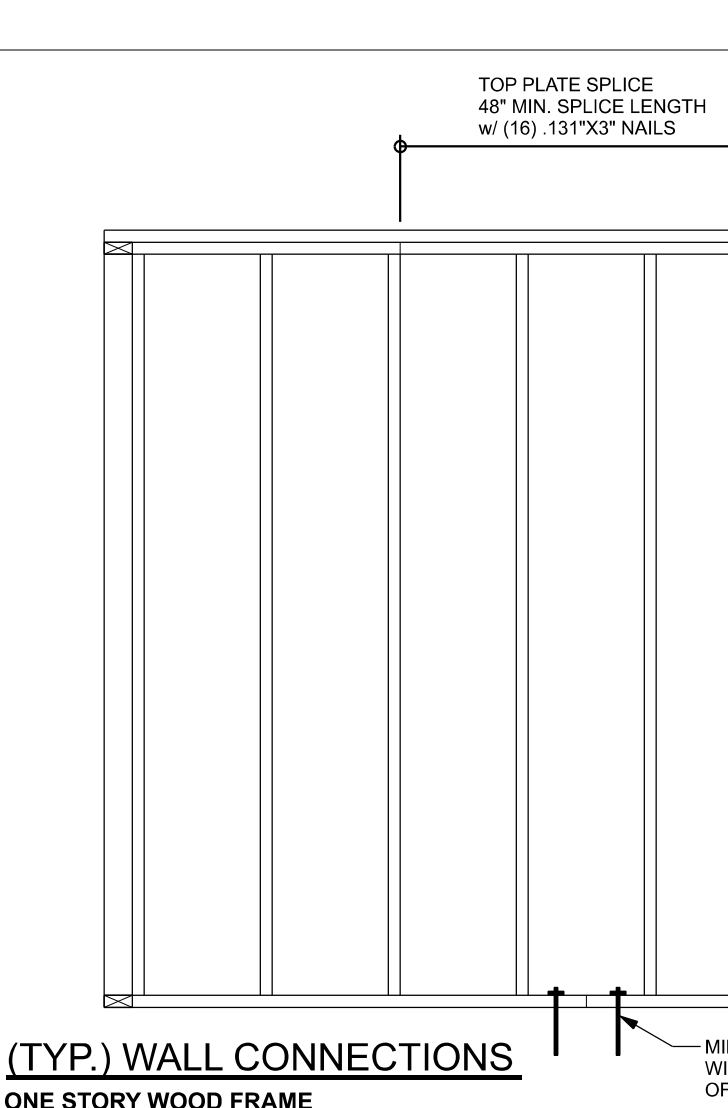
SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED



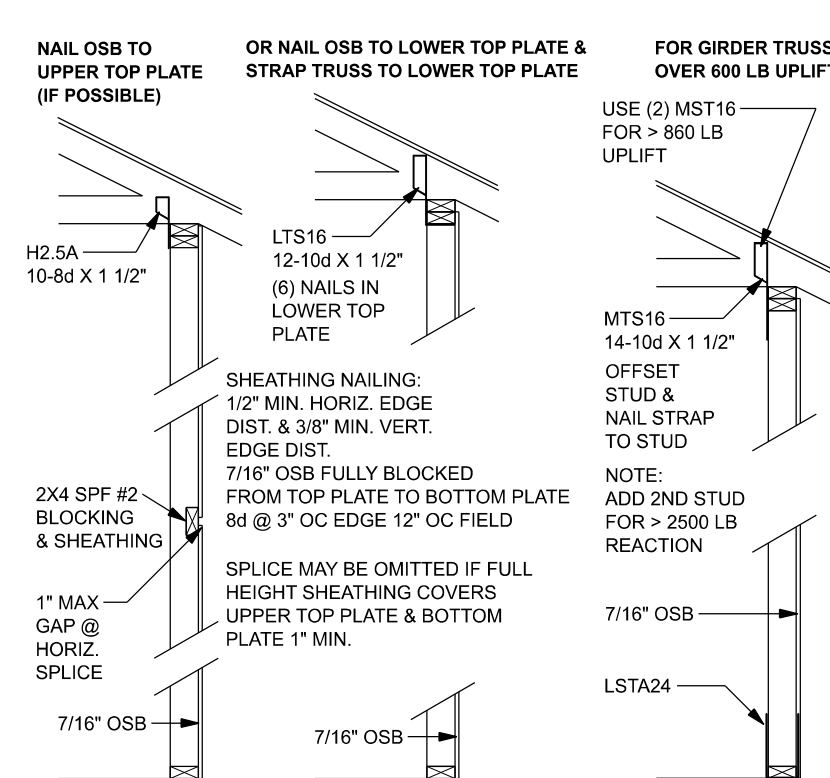
Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	DIT22
< 1800	(2) MSTA24, 18-10d header to jacks	DIT22
< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2(8)
(1) 2x4	(2) 2x4	(1) 2x6
(2) 2x4	(2) 2x4	(2) 2x6
(3) 2x6	(2) 2x4	(2) 2x6
(4) 2x6	(2) 2x4	(2) 2x6
(5) 2x6	(2) 2x4	(2) 2x6
(6) 2x6	(2) 2x4	(2) 2x6
(7) 2x6	(2) 2x4	(2) 2x6
(8) 2x6	(2) 2x4	(2) 2x6
(9) 2x6	(2) 2x4	(2) 2x6
(10) 2x6	(2) 2x4	(2) 2x6
(11) 2x6	(2) 2x4	(2) 2x6
(12) 2x6	(2) 2x4	(2) 2x6
(13) 2x6	(2) 2x4	(2) 2x6
(14) 2x6	(2) 2x4	(2) 2x6
(15) 2x6	(2) 2x4	(2) 2x6
(16) 2x6	(2) 2x4	(2) 2x6
(17) 2x6	(2) 2x4	(2) 2x6
(18) 2x6	(2) 2x4	(2) 2x6
(19) 2x6	(2) 2x4	(2) 2x6
(20) 2x6	(2) 2x4	(2) 2x6
(21) 2x6	(2) 2x4	(2) 2x6
(22) 2x6	(2) 2x4	(2) 2x6
(23) 2x6	(2) 2x4	(2) 2x6
(24) 2x6	(2) 2x4	(2) 2x6
(25) 2x6	(2) 2x4	(2) 2x6
(26) 2x6	(2) 2x4	(2) 2x6
(27) 2x6	(2) 2x4	(2) 2x6
(28) 2x6	(2) 2x4	(2) 2x6
(29) 2x6	(2) 2x4	(2) 2x6
(30) 2x6	(2) 2x4	(2) 2x6
(31) 2x6	(2) 2x4	(2) 2x6
(32) 2x6	(2) 2x4	(2) 2x6
(33) 2x6	(2) 2x4	(2) 2x6
(34) 2x6	(2) 2x4	(2) 2x6
(35) 2x6	(2) 2x4	(2) 2x6
(36) 2x6	(2) 2x4	(2) 2x6
(37) 2x6	(2) 2x4	(2) 2x6
(38) 2x6	(2) 2x4	(2) 2x6
(39) 2x6	(2) 2x4	(2) 2x6
(40) 2x6	(2) 2x4	(2) 2x6
(41) 2x6	(2) 2x4	(2) 2x6
(42) 2x6	(2) 2x4	(2) 2x6
(43) 2x6	(2) 2x4	(2) 2x6
(44) 2x6	(2) 2x4	(2) 2x6
(45) 2x6	(2) 2x4	(2) 2x6
(46) 2x6	(2) 2x4	(2) 2x6
(47) 2x6	(2) 2x4	(2) 2x6
(48) 2x6	(2) 2x4	(2) 2x6
(49) 2x6	(2) 2x4	(2) 2x6
(50) 2x6	(2) 2x4	(2) 2x6
(51) 2x6	(2) 2x4	(2) 2x6
(52) 2x6	(2) 2x4	(2) 2x6
(53) 2x6	(2) 2x4	(2) 2x6
(54) 2x6	(2) 2x4	(2) 2x6
(55) 2x6	(2) 2x4	(2) 2x6
(56) 2x6	(2) 2x4	(2) 2x6
(57) 2x6	(2) 2x4	(2) 2x6
(58) 2x6	(2) 2x4	(2) 2x6
(59) 2x6	(2) 2x4	(2) 2x6
(60) 2x6	(2) 2x4	(2) 2x6
(61) 2x6	(2) 2x4	(2) 2x6
(62) 2x6	(2) 2x4	(2) 2x6
(63) 2x6	(2) 2x4	(2) 2x6
(64) 2x6	(2) 2x4	(2) 2x6
(65) 2x6	(2) 2x4	(2) 2x6
(66) 2x6	(2) 2x4	(2) 2x6
(67) 2x6	(2) 2x4	(2) 2x6
(68) 2x6	(2) 2x4	(2) 2x6
(69) 2x6	(2) 2x4	(2) 2x6
(70) 2x6	(2) 2x4	(2) 2x6
(71) 2x6	(2) 2x4	(2) 2x6
(72) 2x6	(2) 2x4	(2) 2x6
(73) 2x6	(2) 2x4	(2) 2x6
(74) 2x6	(2) 2x4	(2) 2x6
(75) 2x6	(2) 2x4	(2) 2x6
(76) 2x6	(2) 2x4	(2) 2x6
(77) 2x6	(2) 2x4	(2) 2x6
(78) 2x6	(2) 2x4	(2) 2x6
(79) 2x6	(2) 2x4	(2) 2x6
(80) 2x6	(2) 2x4	(2) 2x6
(81) 2x6	(2) 2x4	(2) 2x6
(82) 2x6	(2) 2x4	(2) 2x6
(83) 2x6	(2) 2x4	(2) 2x6
(84) 2x6	(2) 2x4	(2) 2x6
(85) 2x6	(2) 2x4	(2) 2x6
(86) 2x6	(2) 2x4	(2) 2x6
(87) 2x6	(2) 2x4	(2) 2x6
(88) 2x6	(2) 2x4	(2) 2x6
(89) 2x6	(2) 2x4	(2) 2x6
(90) 2x6	(2) 2x4	(2) 2x6
(91) 2x6	(2) 2x4	(2) 2x6
(92) 2x6	(2) 2x4	(2) 2x6
(93) 2x6	(2) 2x4	(2) 2x6
(94) 2x6	(2) 2x4	(2) 2x6
(95) 2x6	(2) 2x4	(2) 2x6
(96) 2x6	(2) 2x4	(2) 2x6
(97) 2x6	(2) 2x4	(2) 2x6
(98) 2x6	(2) 2x4	(2) 2x6
(99) 2x6	(2) 2x4	(2) 2x6
(100) 2x6	(2) 2x4	(2) 2x6

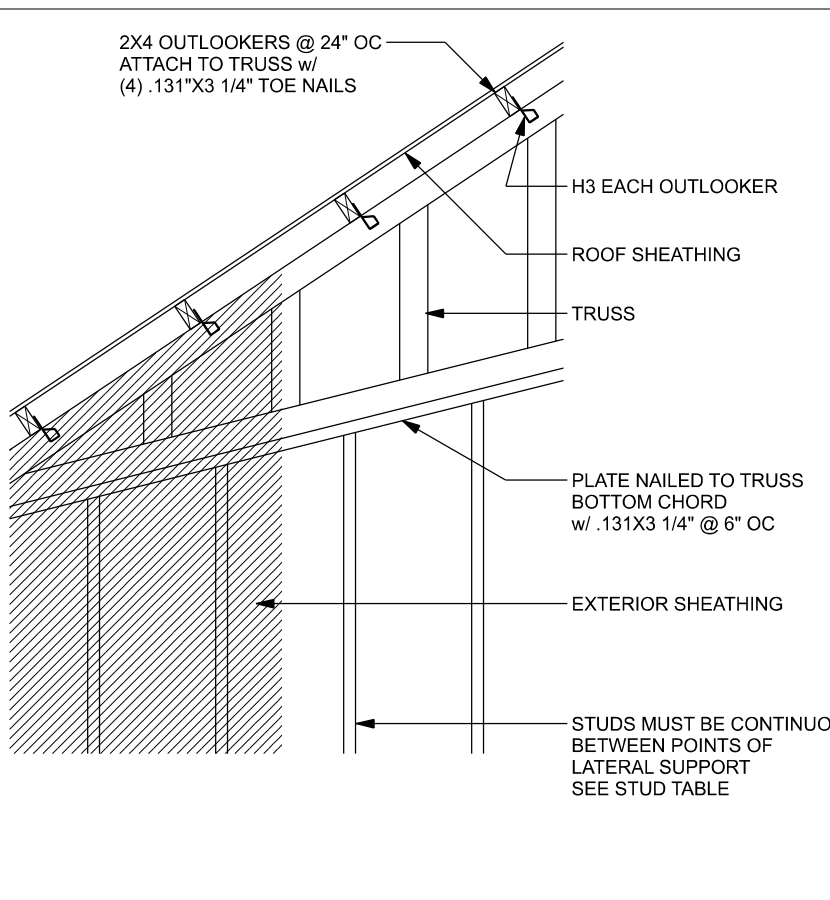
TYPICAL HEADER STRAPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



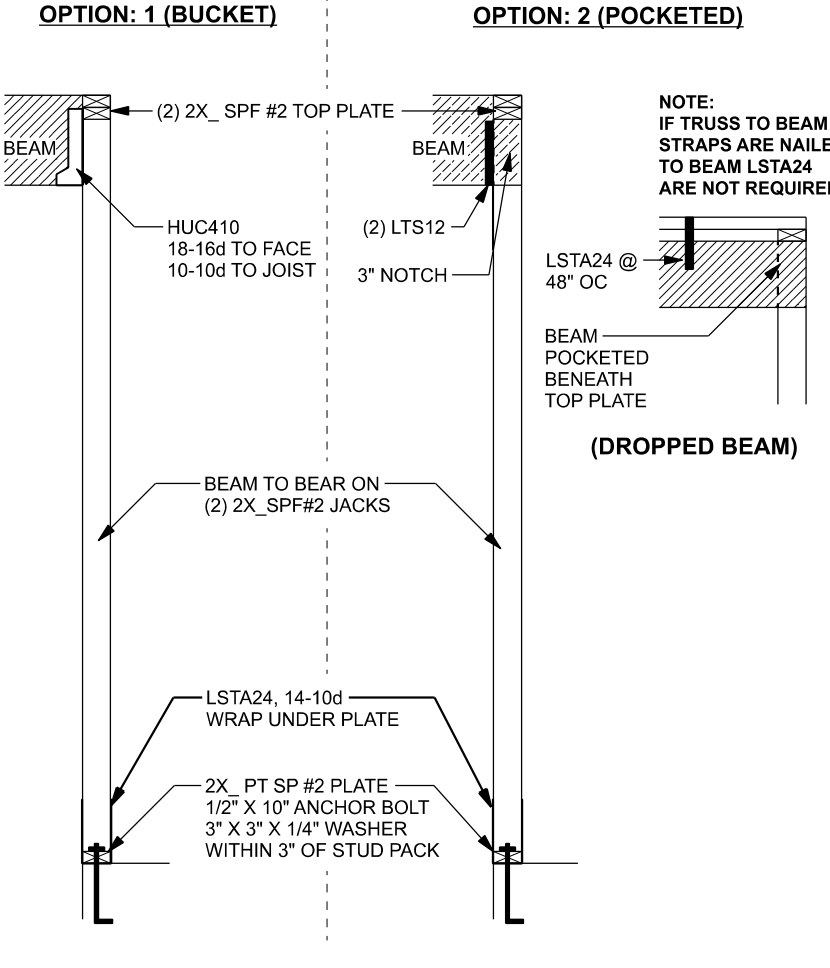
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



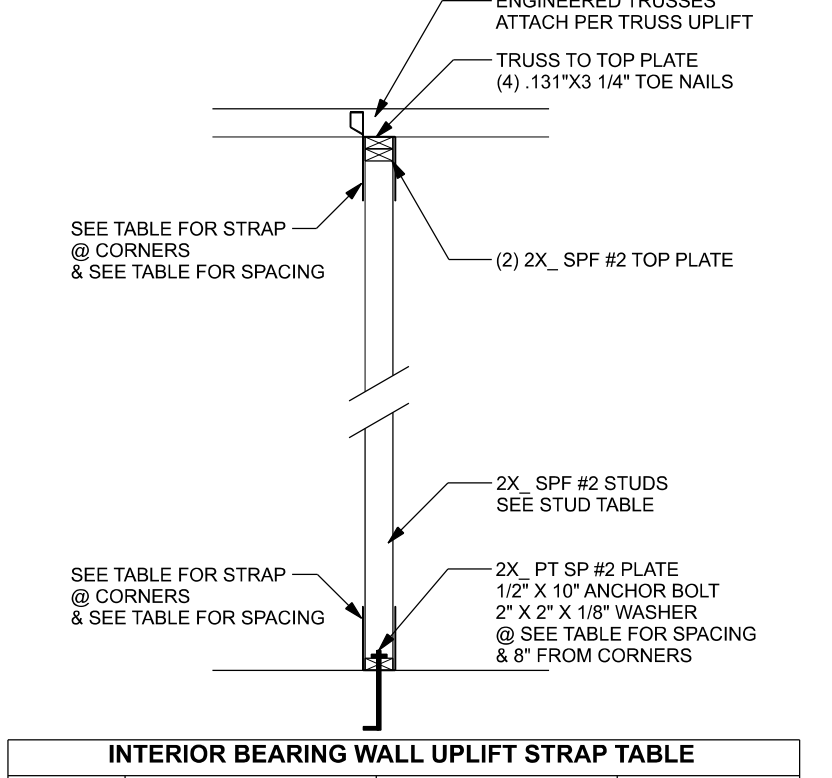
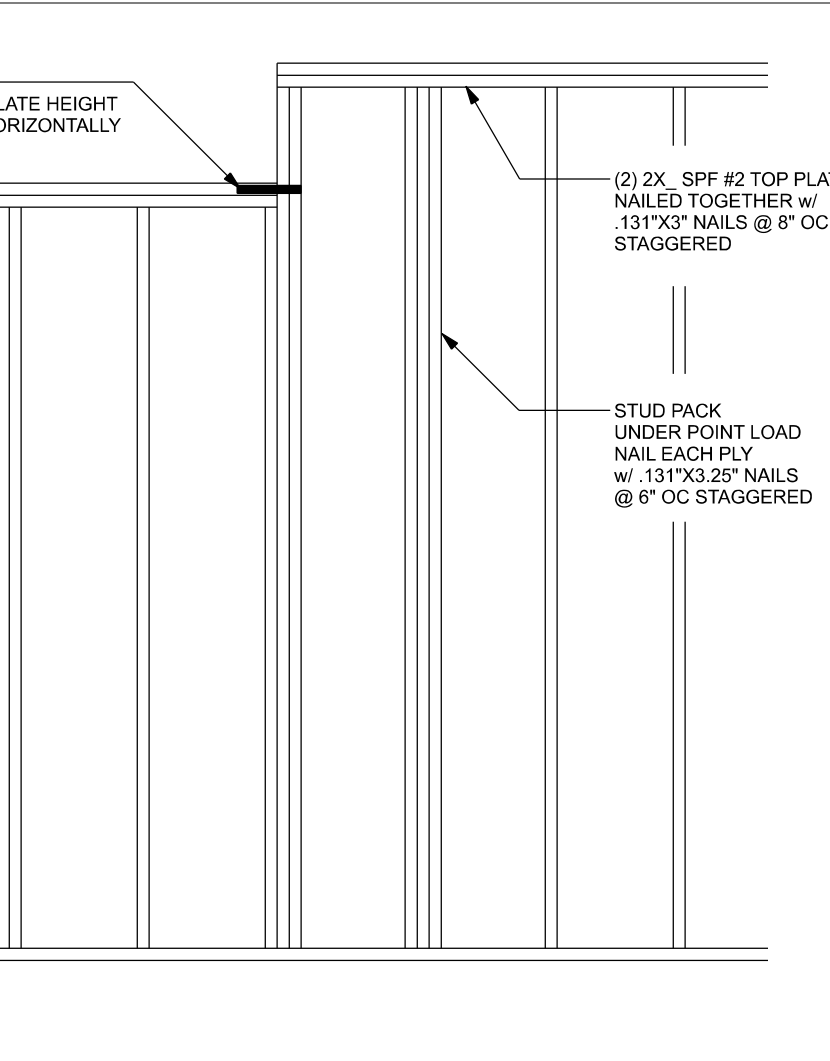
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

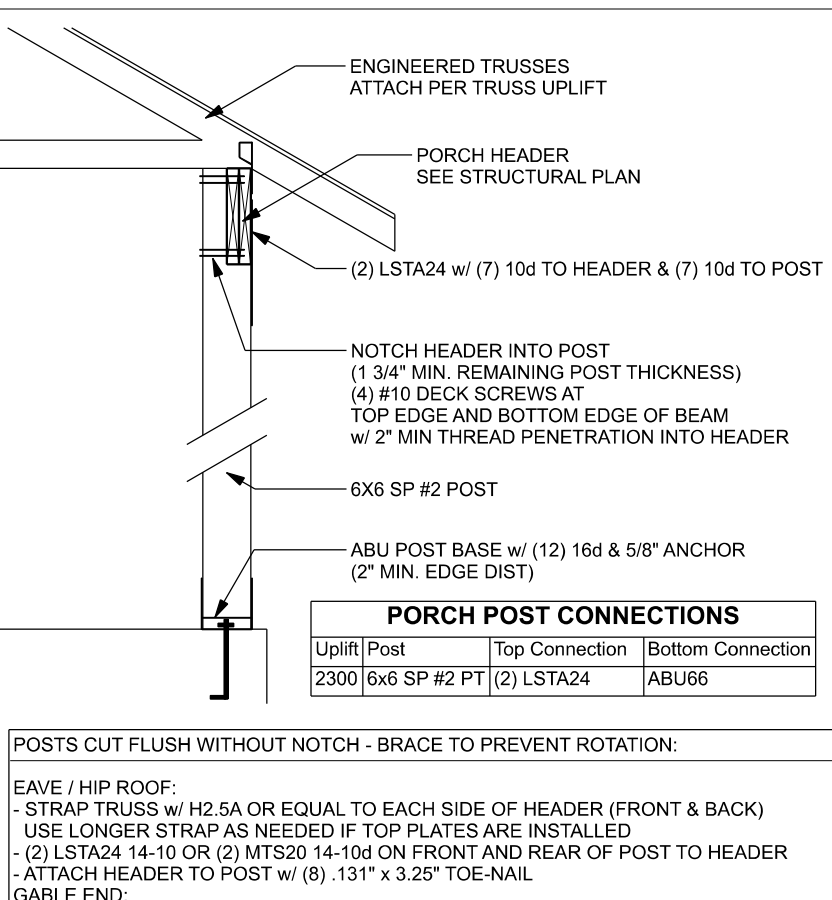


(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

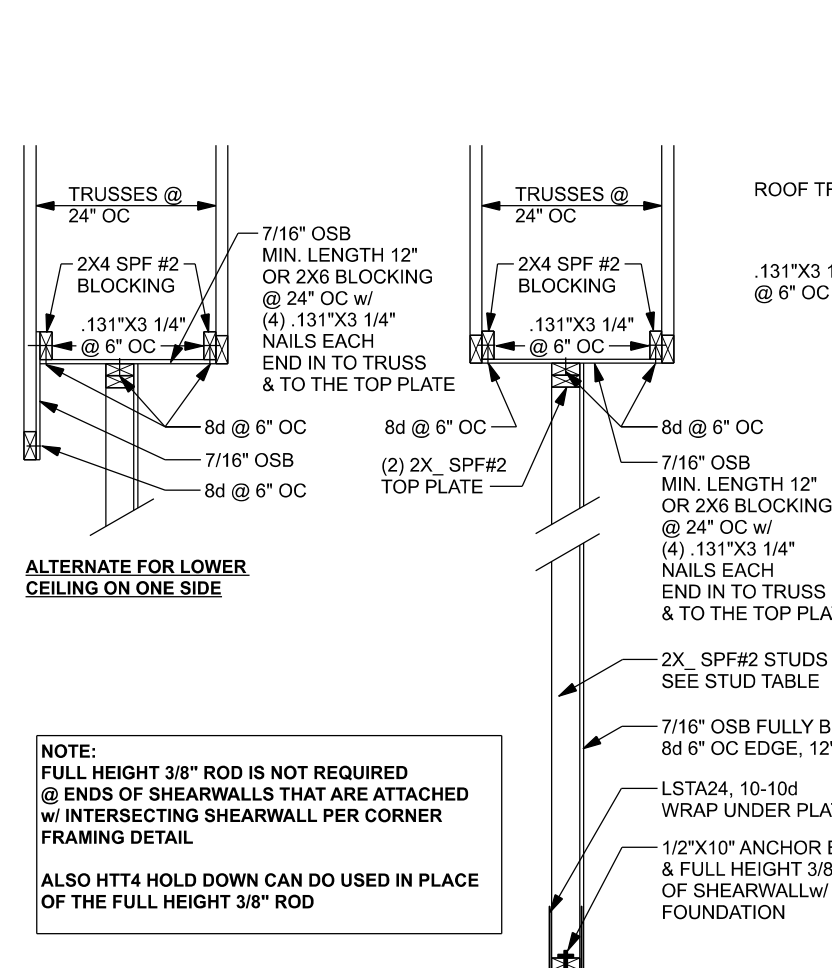


Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 pif	LSTA24, 14-10d @ 48" OC Wrap Under Plate	LSTA24, 14-10d @ 48" OC	48" OC
465 pif	LSTA24, 14-10d @ 32" OC Wrap Under Plate	LSTA24, 14-10d @ 32" OC Wrap Over Plate	32" OC

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



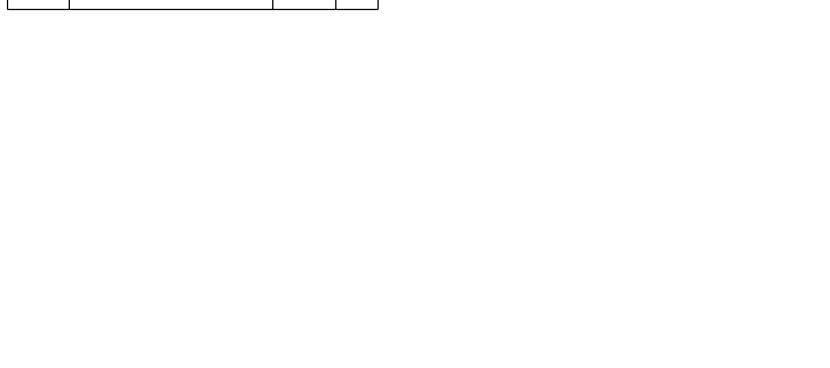
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	496	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	620	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	6-10d	6-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DIT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	16-16d x 1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DIT22	8-SDS 1/4"x1 1/2"	1/2"x8" Titen HD
4235	3640	HTT4	16-16d x 1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
2200	ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x7" Drill & Epoxy	

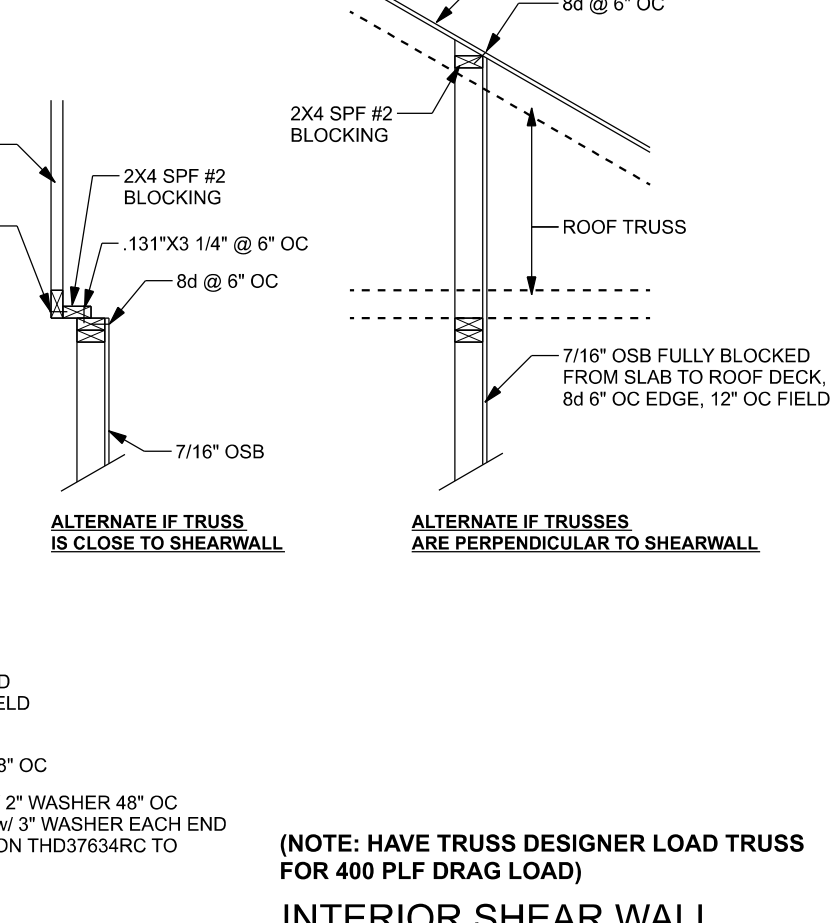
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2\"/>

(1) 2x4 @ 16\"/>

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0



(TYP.) INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB



(TYP.) INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	BASIC WIND SPEED (ASCE 7-16, 3S GUST) 130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&G DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	ROOMS OTHER THAN SLEEPING ROOM 40 PSF LIVE LOAD
ROOF LOADING	FLAT OR < 4:12 20 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

EFFECTIVE WIND AREA (FT2)	ZONE 5 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)

9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)

GENERAL NOTES:
TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO THE TRUSS MANUFACTURER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W/ 4 #1 & 4 #2 @ 8x8x8, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVALS. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE SAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

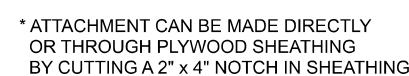
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



SCALE: N.T.S.

=====	TRUSS
= = = =	TRUSS UNDER VALLEY FRAMING
	VALLEY RAFTER OR RIDGE
●	CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX.

CONNECTION REQUIREMENT NOTES	
1	2X4 RAFTERS TO RIDGE 3-16d OR 6 - 131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE 3-16d OR 6 - 131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS 3-16d OR 6 - 131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING 3-16d OR 12 - 131 x 3" TOE NAILS
5	SLEEPER TO TRUSS 4-16d OR 8 - 131 x 3" FACE NAILS EACH TRUSS
6	RIDGE TO ROOF BLOCK 3-16d OR 6 - 131 x 3" TOE NAILS
7	ROOF BOARD TO TRUSS 3-16d OR 6 - 131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP) 3-16d OR 6 - 131 x 3" FACE NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN) 3-16d OR 6 - 131 x 3" FACE NAILS
10	TRUSS TO BLOCKING 3-16d OR 6 - 131 x 3" END NAILS
11	CRIPPLE TO TRUSS 3-16d OR 6 - 131 x 3" FACE NAILS
12	CRIPPLE TO PURLIN 3-16d OR 6 - 131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF AREA PER SUPPORT
16R2 IN ZONES 2 & 3, 24R2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN
= 16R2 OR 2'-0" X 8'-0" SPAN = 16R2)

PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM
OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
MINIMUM OF 16D NAIL.

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:

- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
- MAXIMUM WIND SPEED: 130 MPH

- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC / ASCE 7-10 WIND REQUIREMENTS
- EXPOSURE CATEGORY "C", $I = 1.0$, $K_{zt} = 1.0$
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG. NAILED W/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB'S ON BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & ROY OR COMMON NAILS.

- GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE
WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL
WRAP UNDER BOTTOM PLATE & OVER TOP PLATE
1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER
MUST BE LOCATED WITHIN 6" OF KING STUD
@ ALL DOOR LOCATIONS (U.O.N.O.)

SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD

SN-5 DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

(2) 2X6X0', 1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.)

Diagram illustrating the components of a header assembly:

- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	22716 LBF	20232 LBF
REQUIRED	18775 LBF	18955 LBF



SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #2933862

Amira Builders

Creasev Res.

PROJECT ADDRESS:
00 SW Sugar Bear Glen
Ft. White, FL

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Discosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Tuesday, November 9, 2021

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JOB NUMBER:
211511

S-3
OF 3 SHEET