

Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com

COA#0-278 Florida Certificate of Product Approval #FL1999

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 20-4768
Job Description: TwentyEight Fourteen LLC-Lot 6 Emerald Cove	
Address: Lot 6 Emerald Cove	

Job Engineering Criteria:	
Design Code: FBC 2017 RES	IntelliVIEW Version: 20.01.01A
	JRef #: 1XPP2150004
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 37.00
Building Type: Closed	

This package contains general notes pages, 40 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	135.23.1045.04480	A01	2	135.23.1045.03229	A02
3	135.23.1045.04478	A03	4	135.23.1045.03634	A04
5	135.23.1045.04634	A05	6	135.23.1045.03243	A06
7	135.23.1045.04056	A07	8	135.23.1045.02979	A08
9	135.23.1045.02868	A09	10	135.23.1045.04603	A10
11	135.23.1045.02822	A11	12	135.23.1045.03993	B01
13	135.23.1045.02885	B02	14	135.23.1045.04385	C01
15	135.23.1045.04290	C02	16	135.23.1045.03259	C03
17	135.23.1045.04134	C04	18	135.23.1045.03525	C05
19	135.23.1045.04556	C06	20	135.23.1045.04337	C07
21	135.23.1045.04401	C08	22	135.23.1045.03932	C09
23	135.23.1045.02837	C10	24	135.23.1045.04150	C11
25	135.23.1045.03368	C12	26	135.23.1045.04259	D01
27	135.23.1045.03853	E01	28	135.23.1045.03916	E02
29	135.23.1045.03212	E03	30	135.23.1045.03448	G01
31	135.23.1045.04462	HJ7	32	135.23.1045.04181	HJ7A
33	135.23.1050.22457	HJ1	34	135.23.1045.03228	EJ7
35	135.23.1045.03556	EJ7A	36	135.23.1045.04588	CJ5
37	135.23.1045.04104	CJ5A	38	135.23.1045.03478	CJ3
39	135.23.1045.03696	СЈЗА	40	135.23.1045.04213	CJ1
41	A14015ENC160118		42	BRCLBSUB0119	
43	GBLLETIN0118				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc). -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

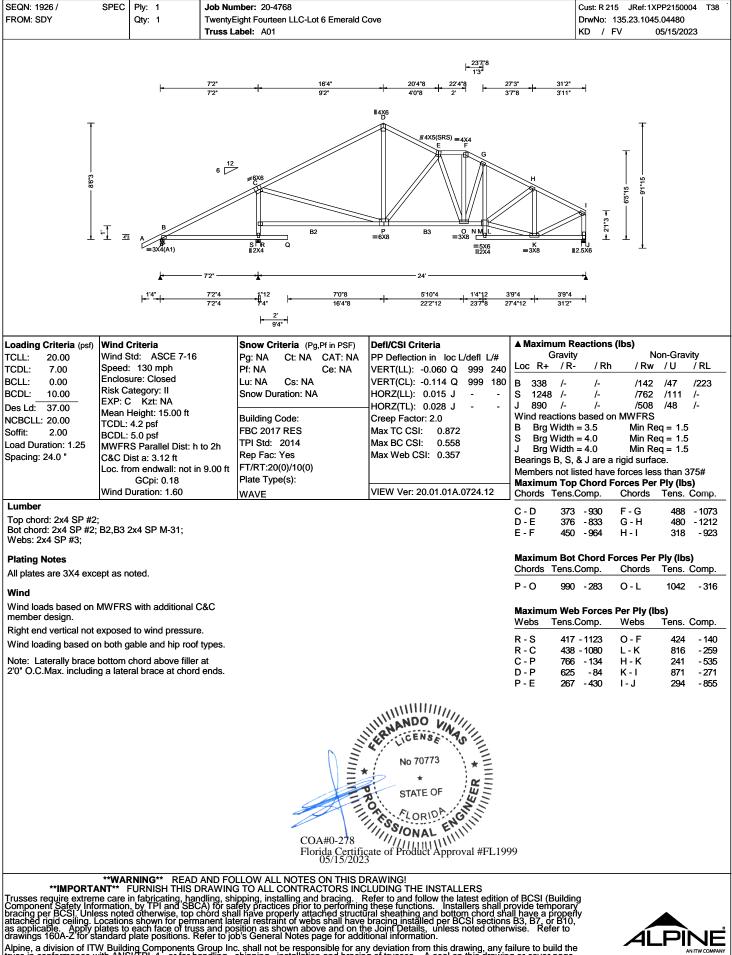
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

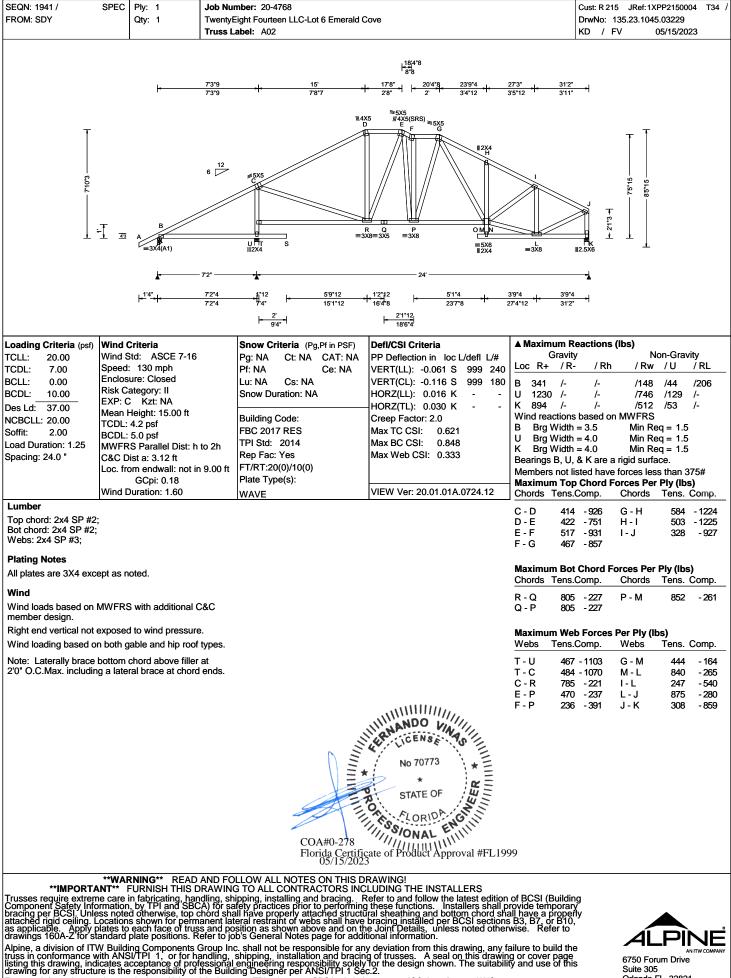
References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; <u>www.iccsafe.org</u>.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; <u>www.alpineitw.com</u>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

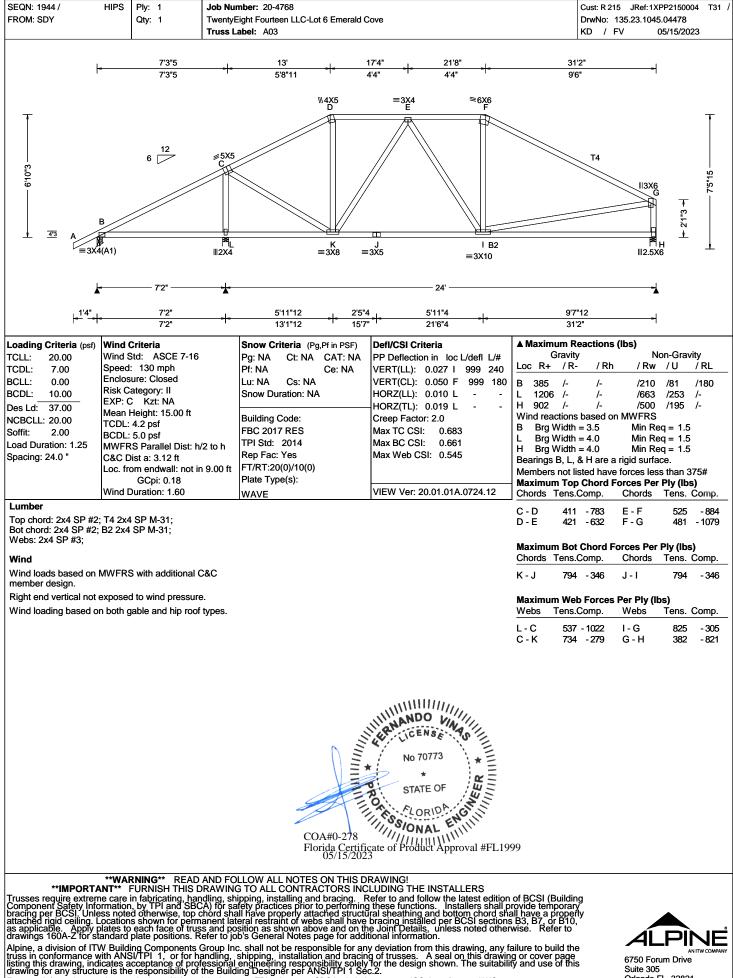


Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

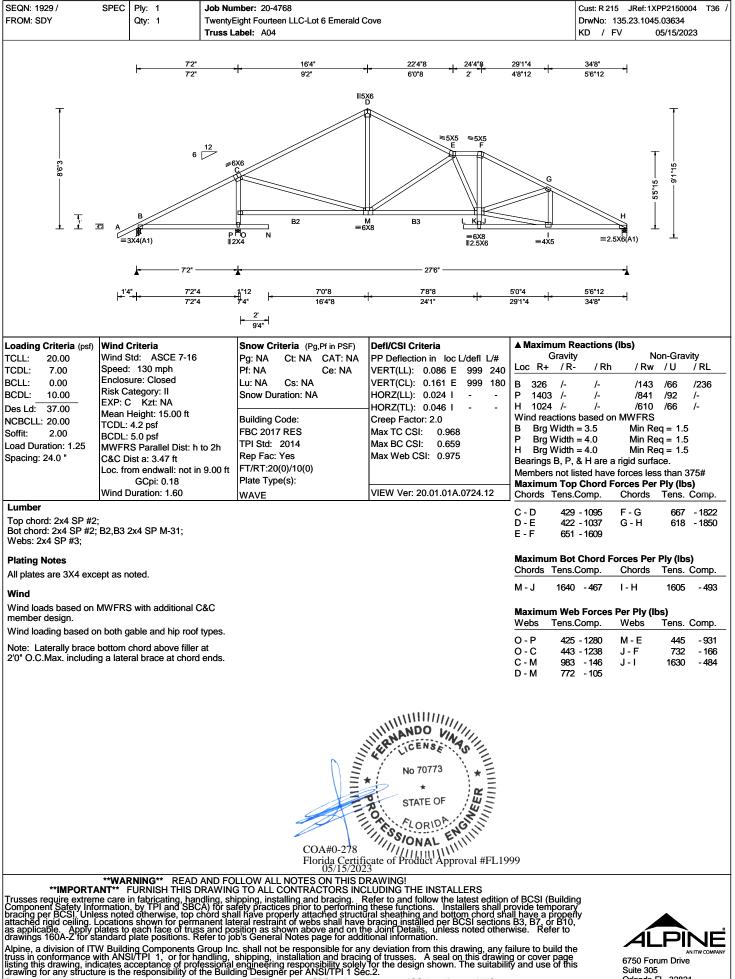
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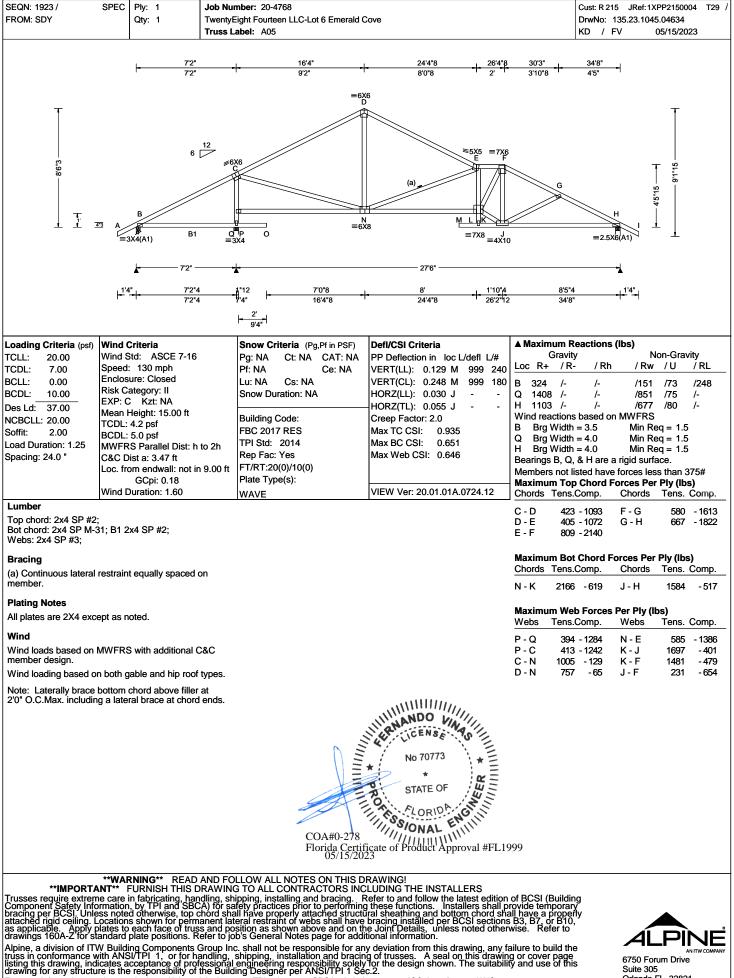




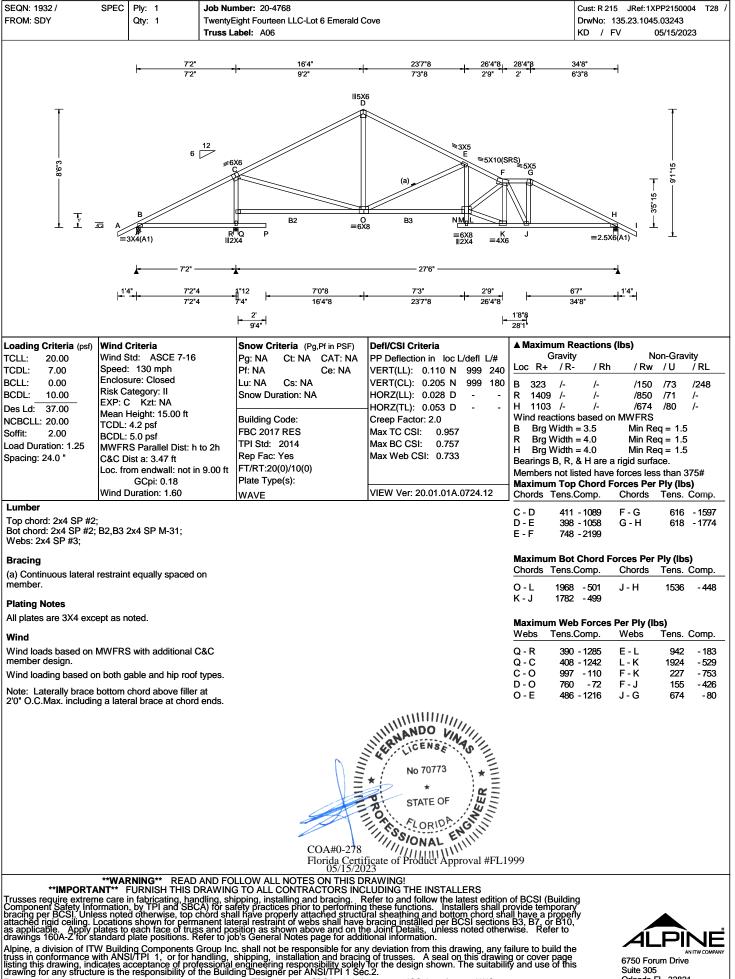




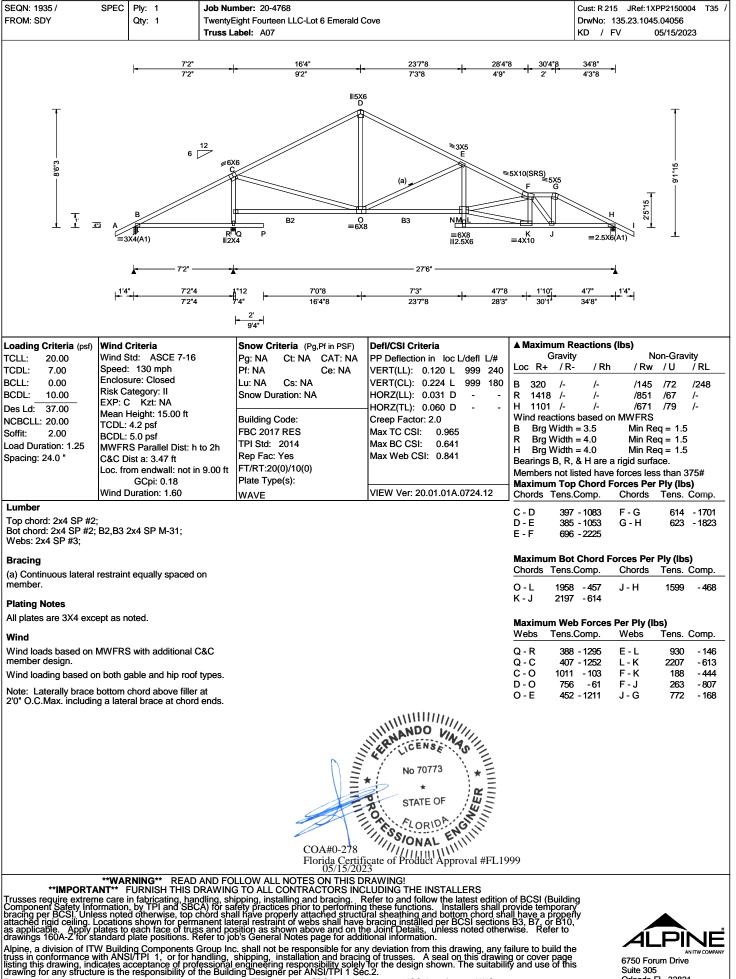




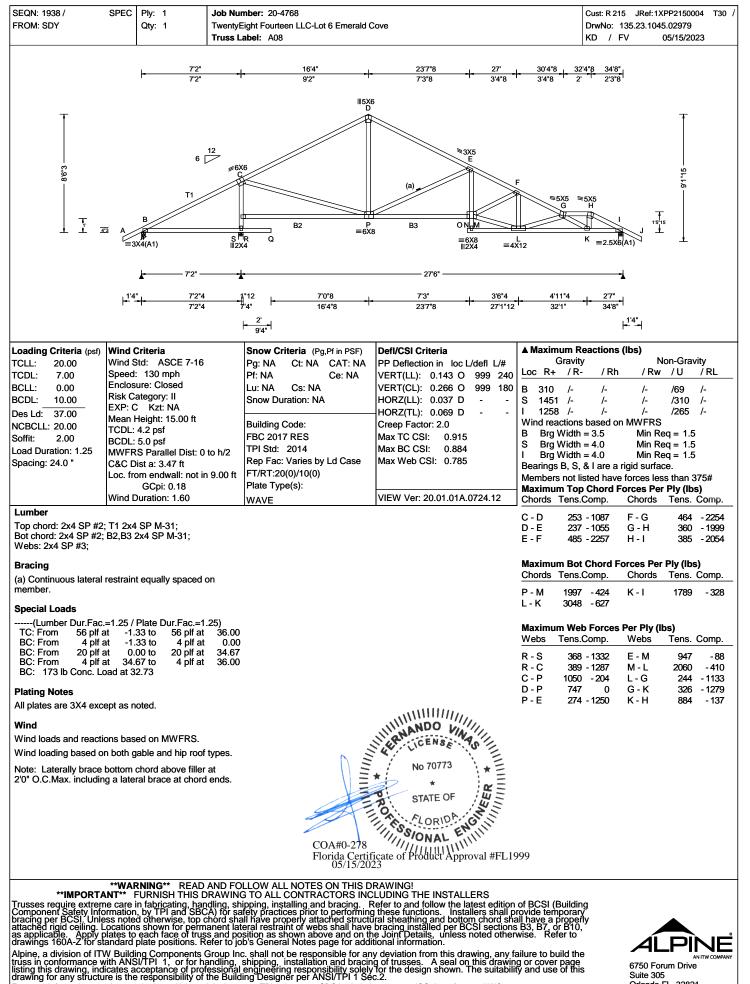




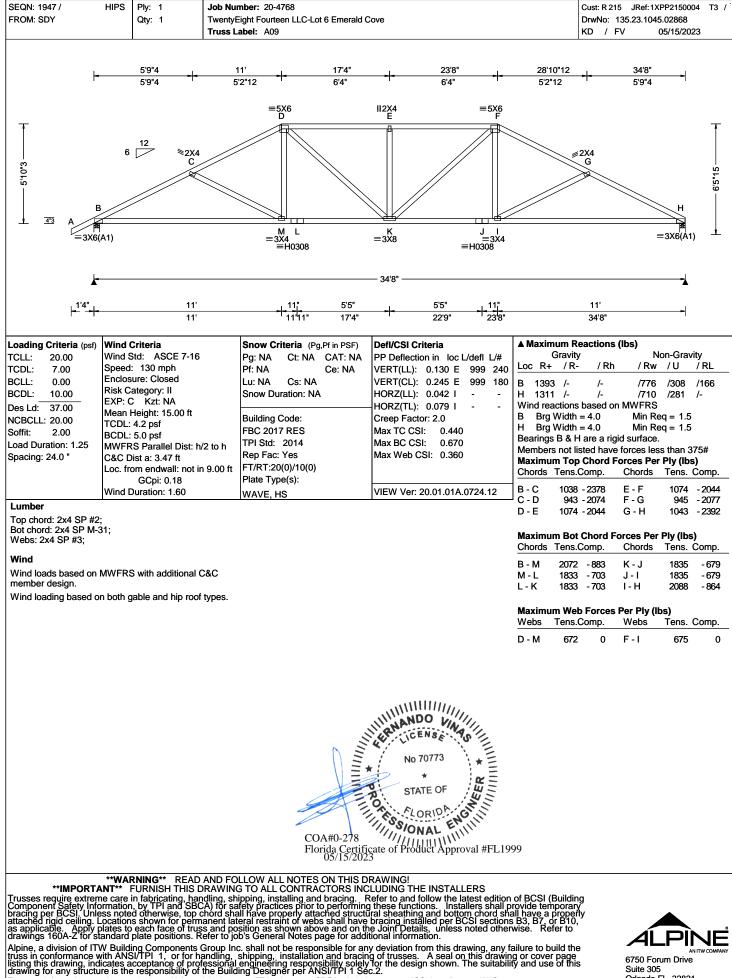




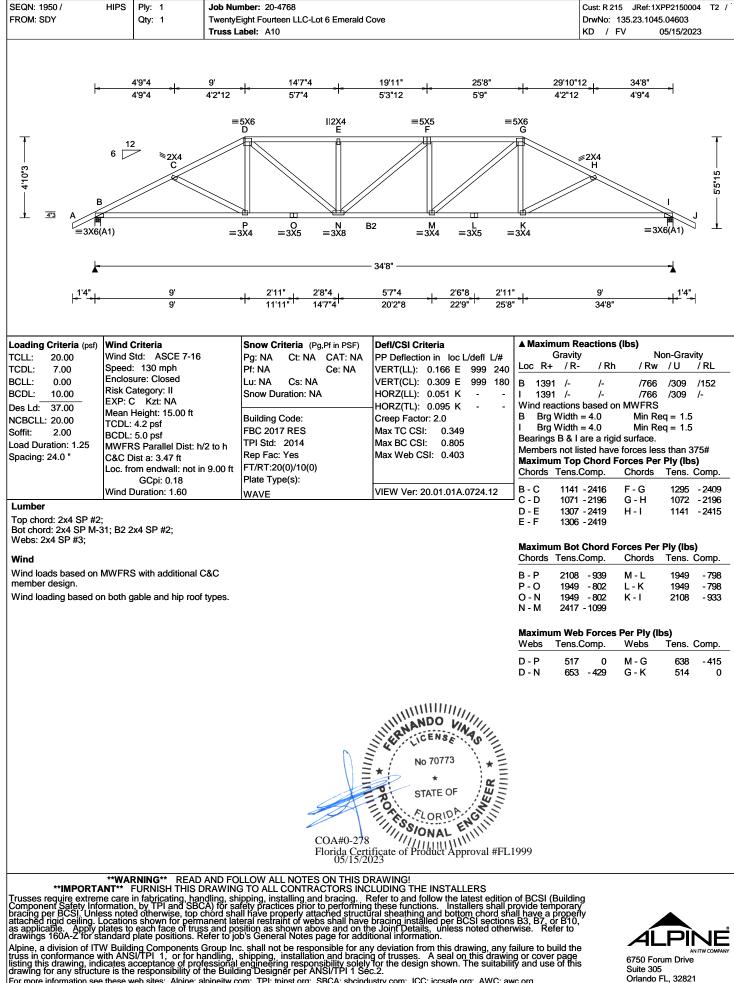


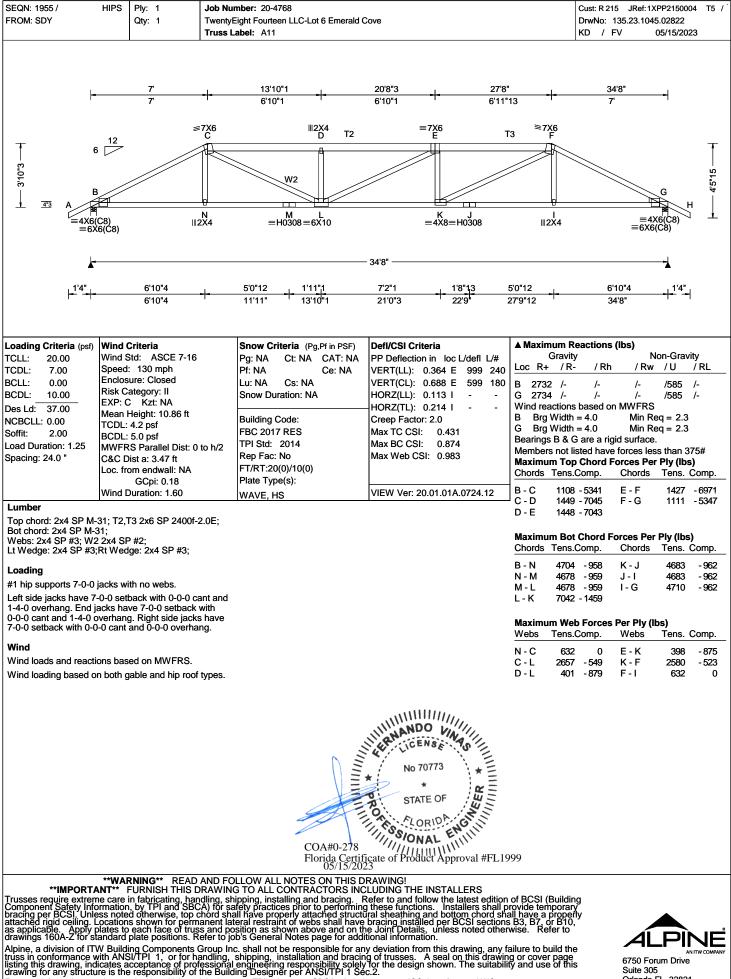


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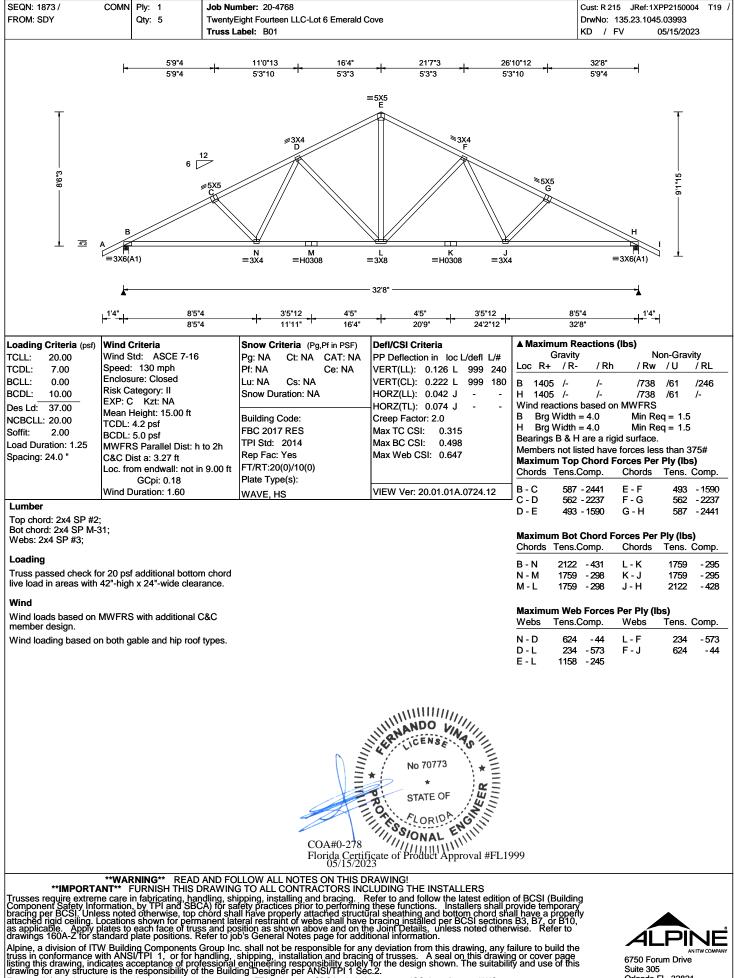


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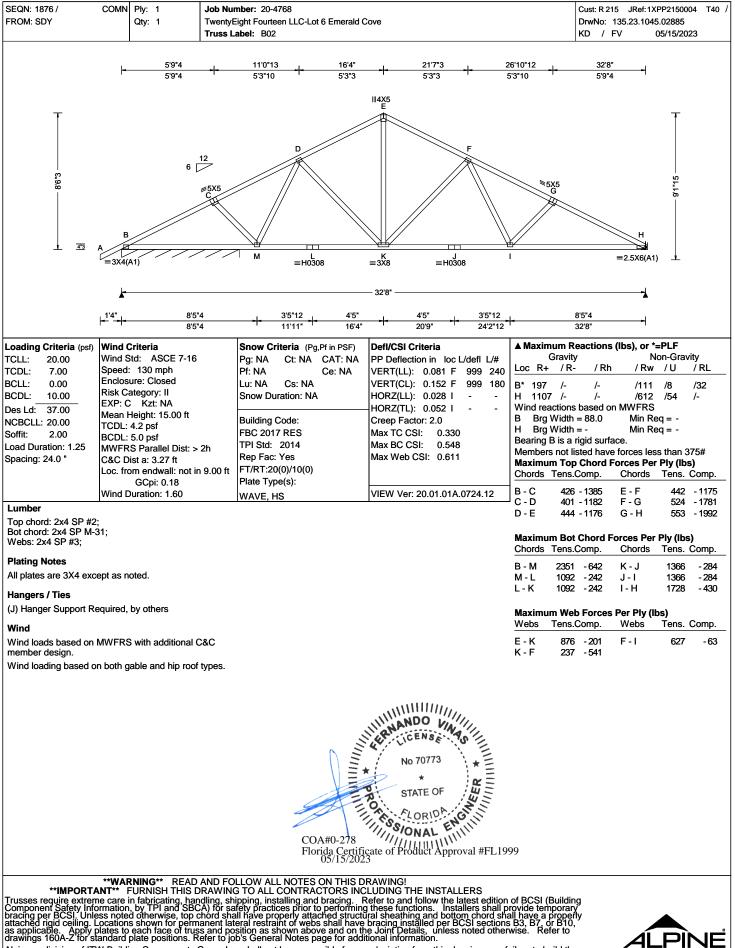




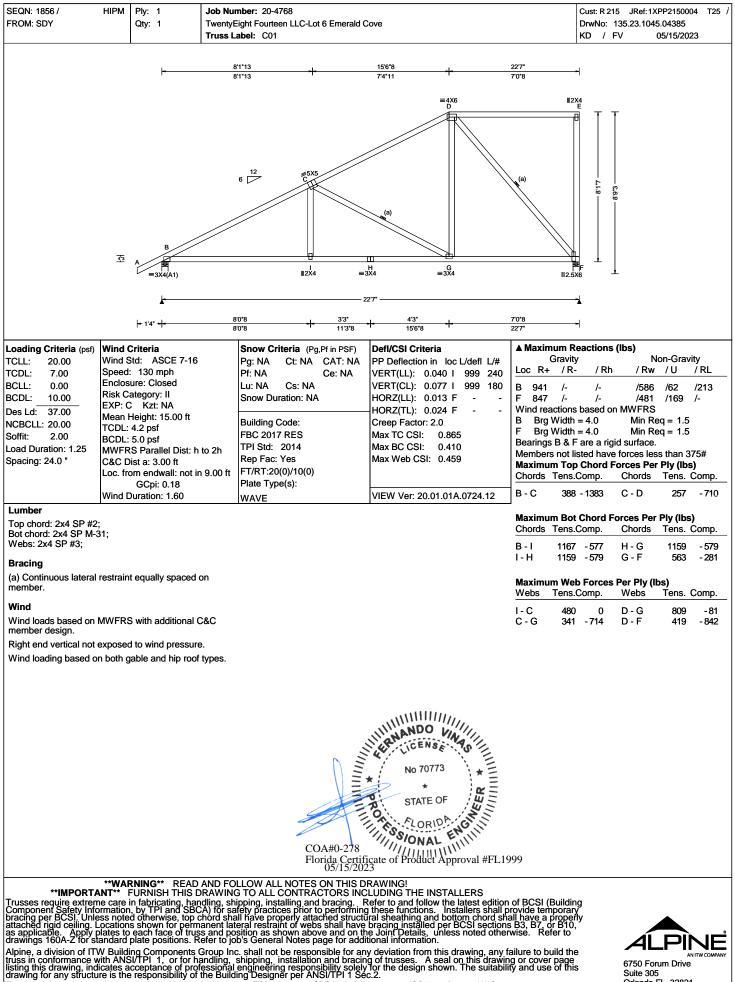




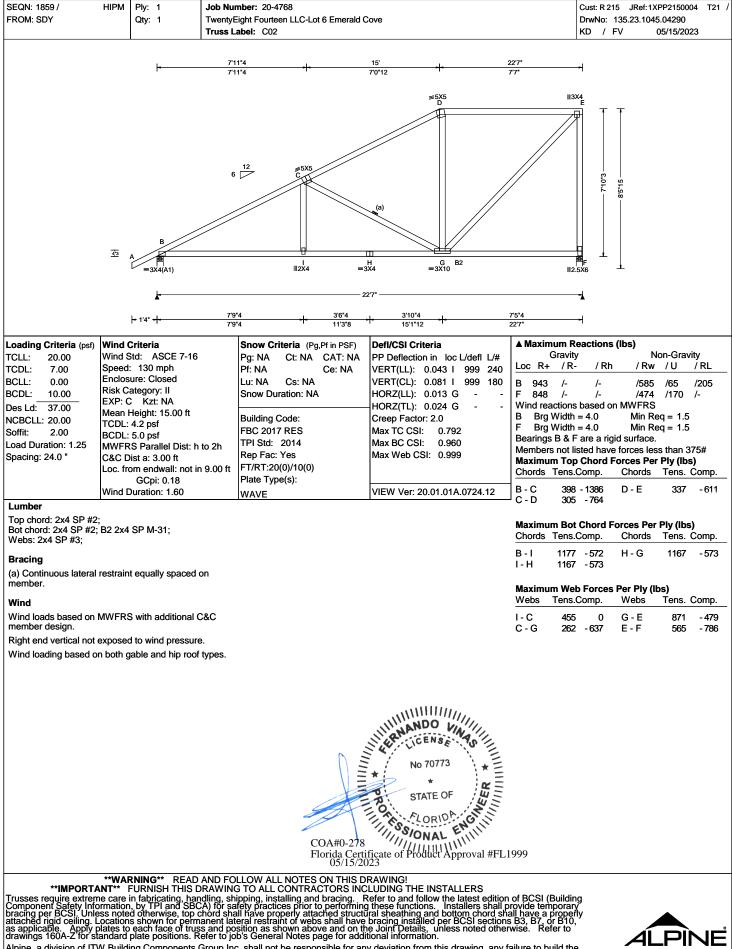




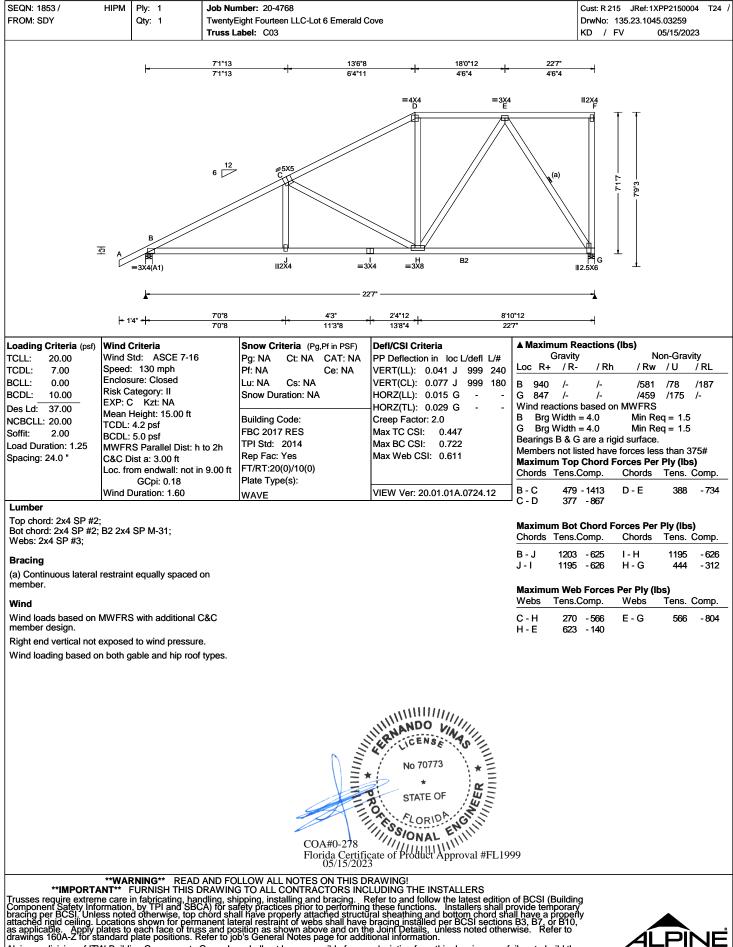




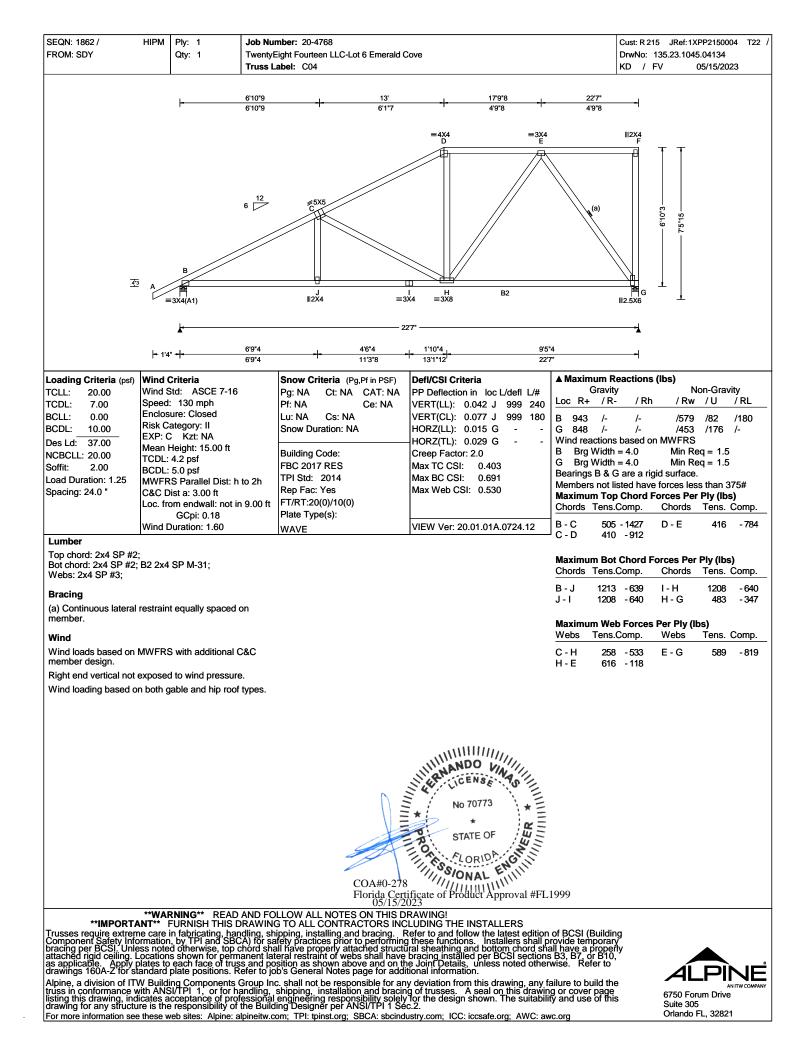


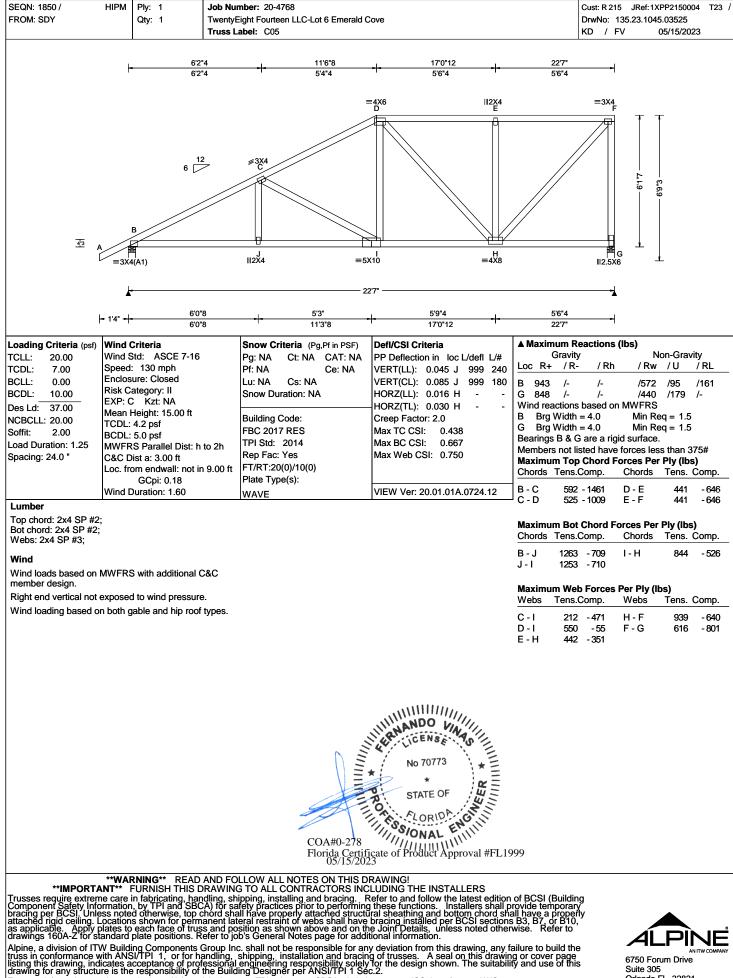




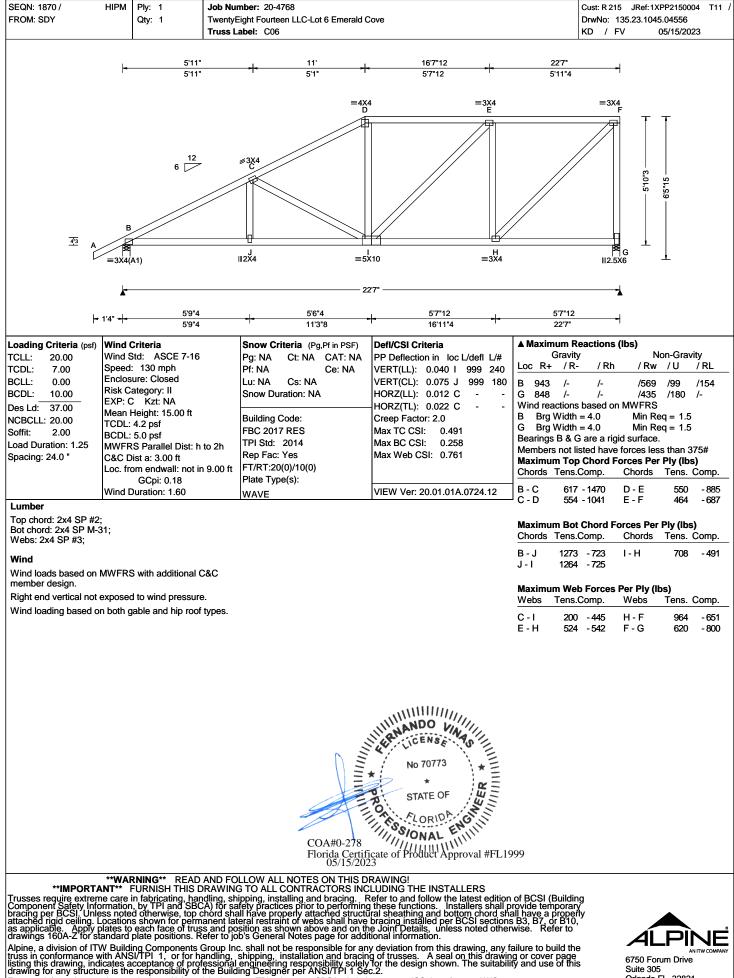




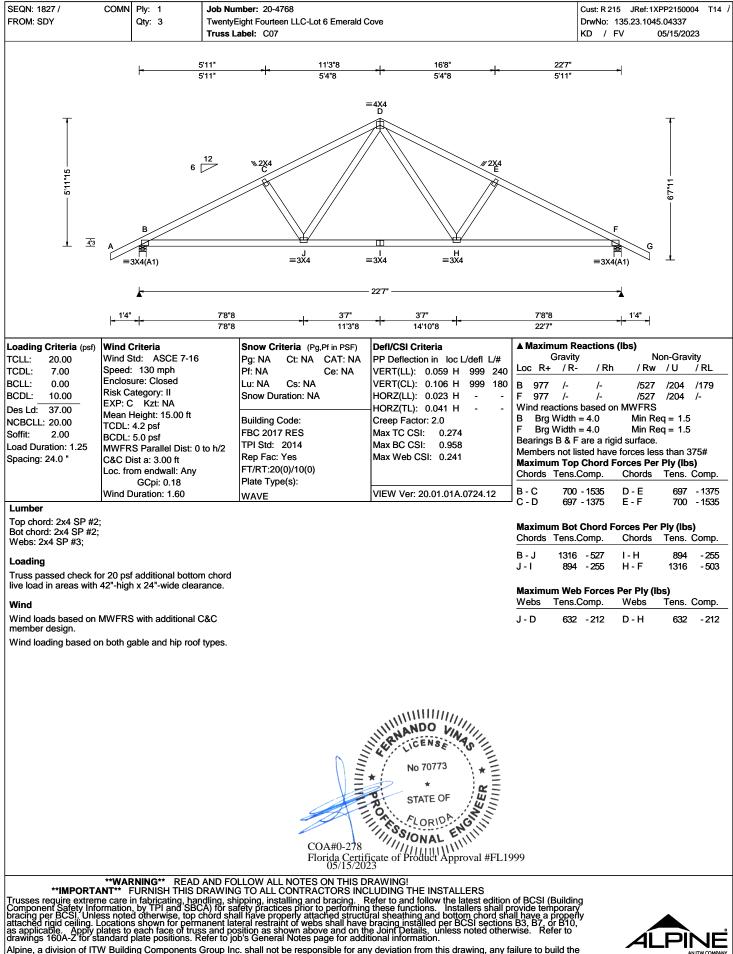




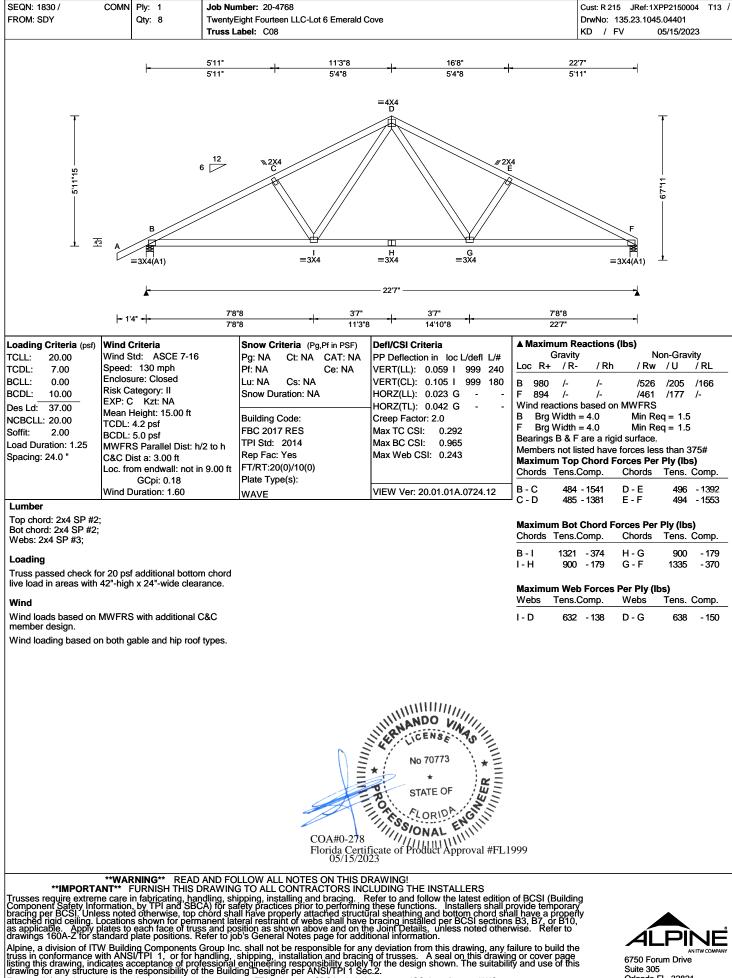
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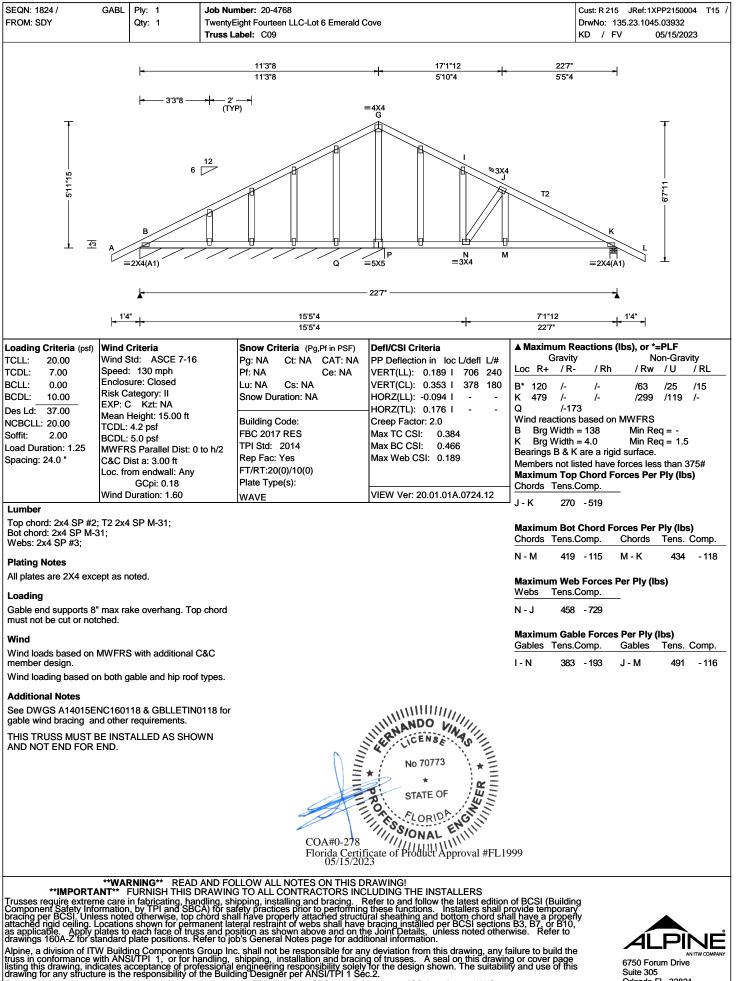
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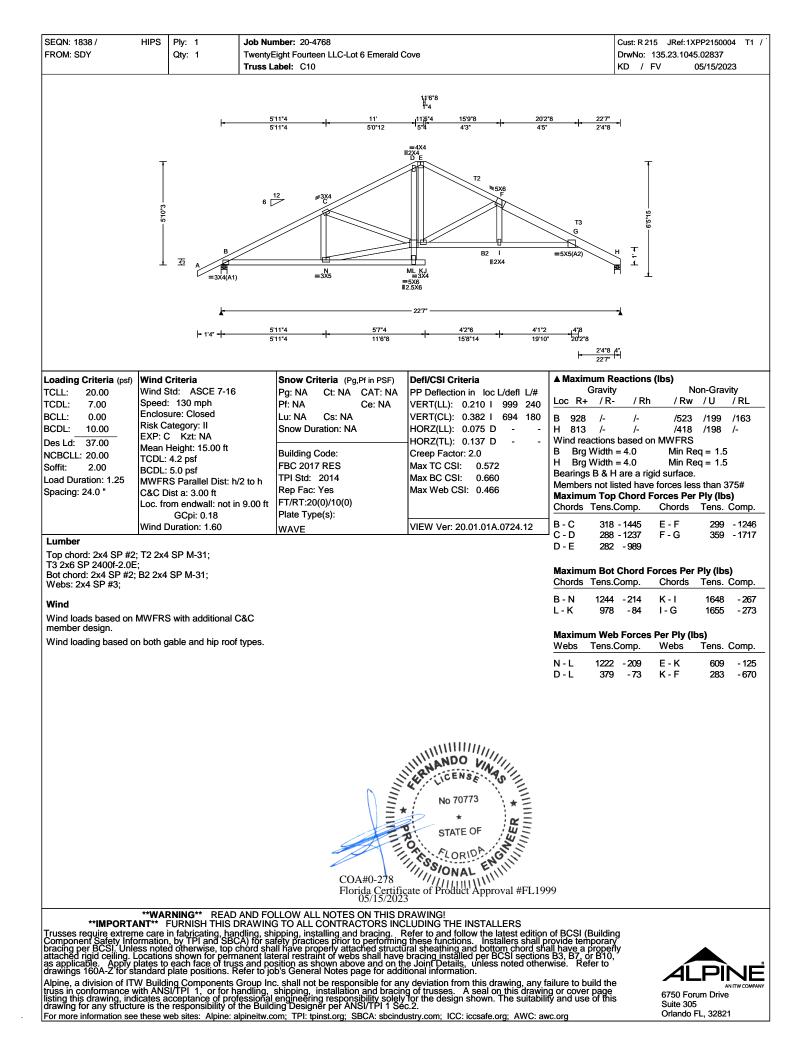


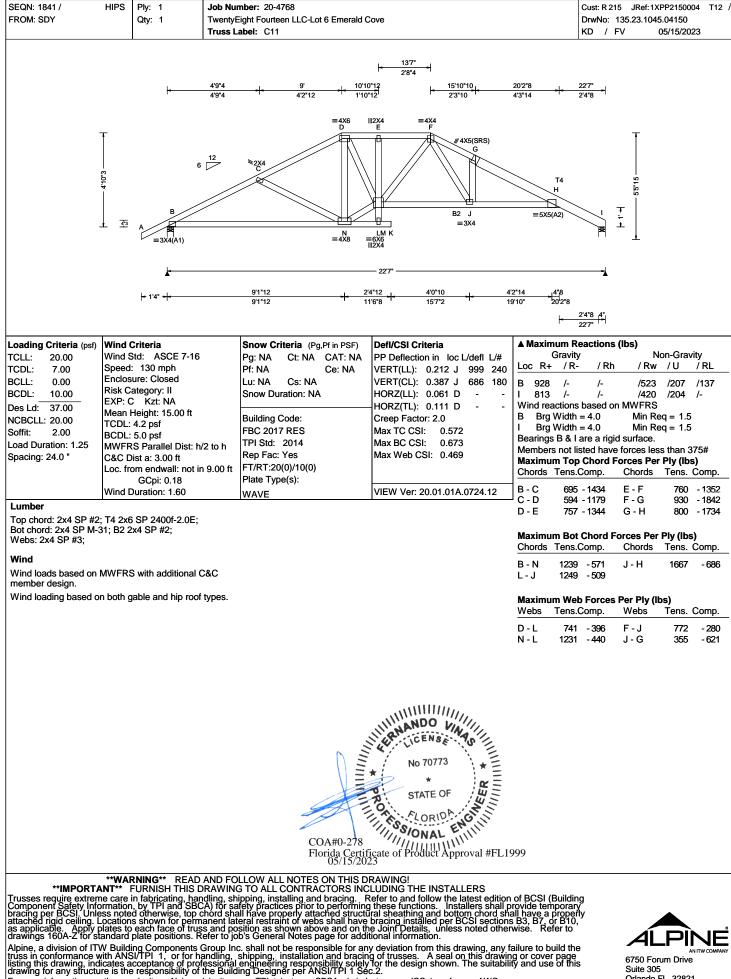






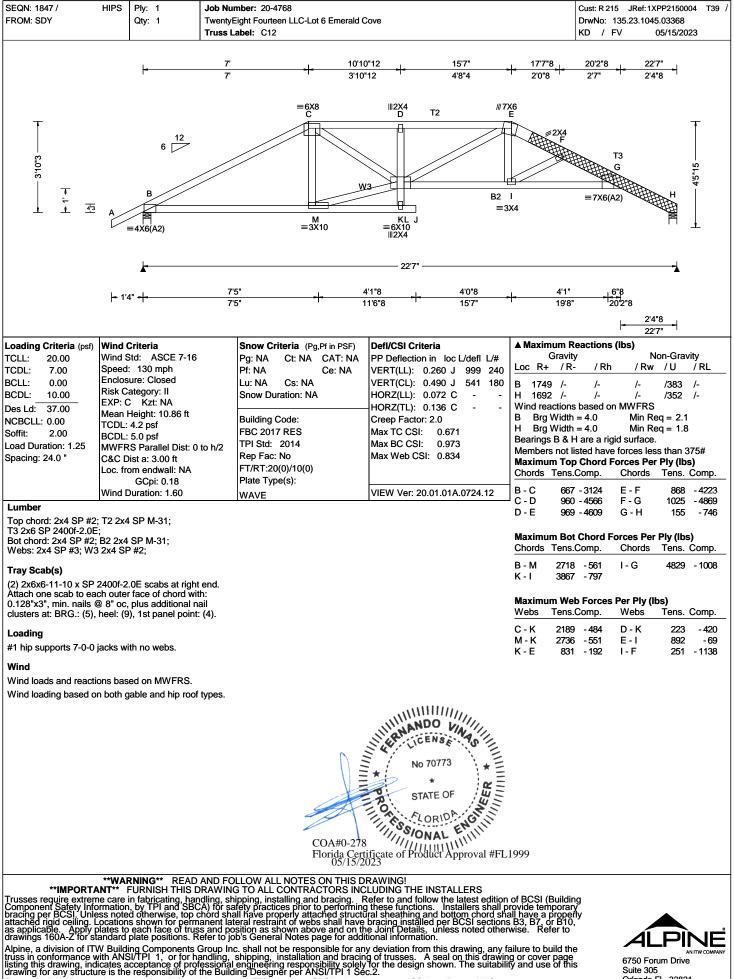
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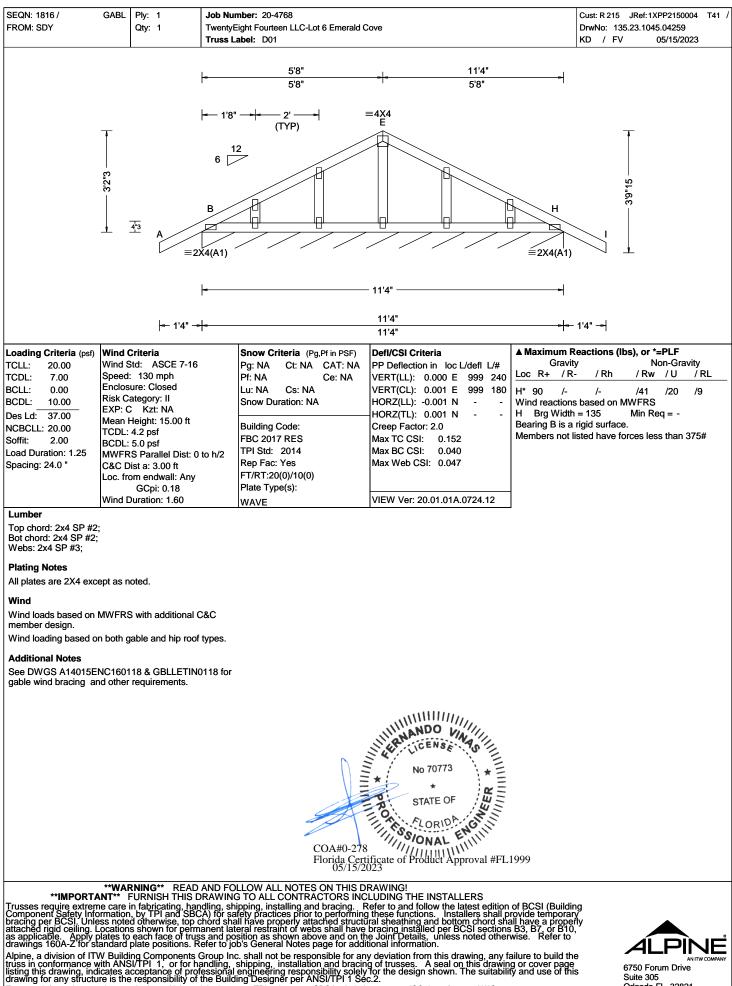


For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

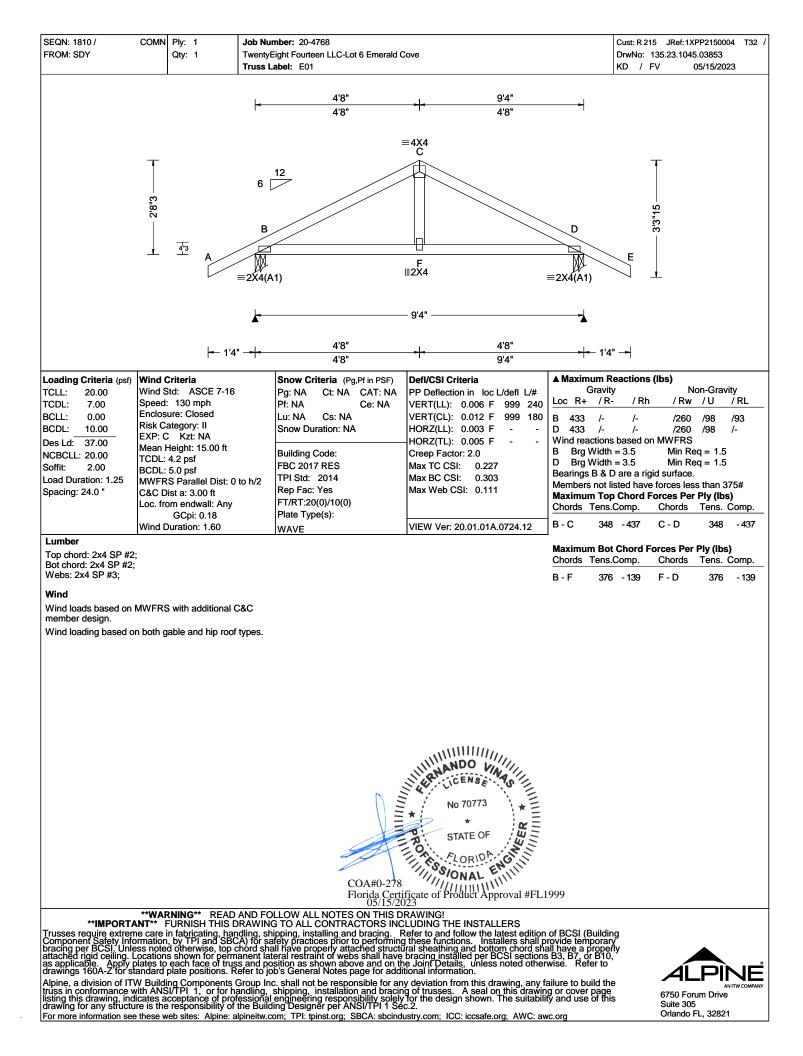
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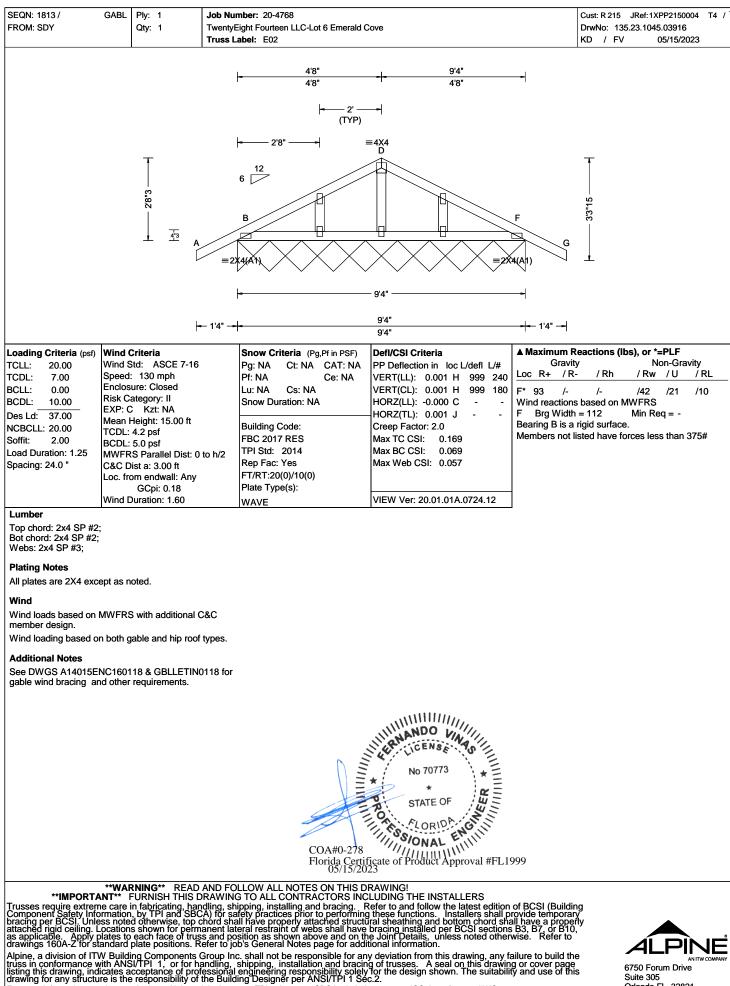


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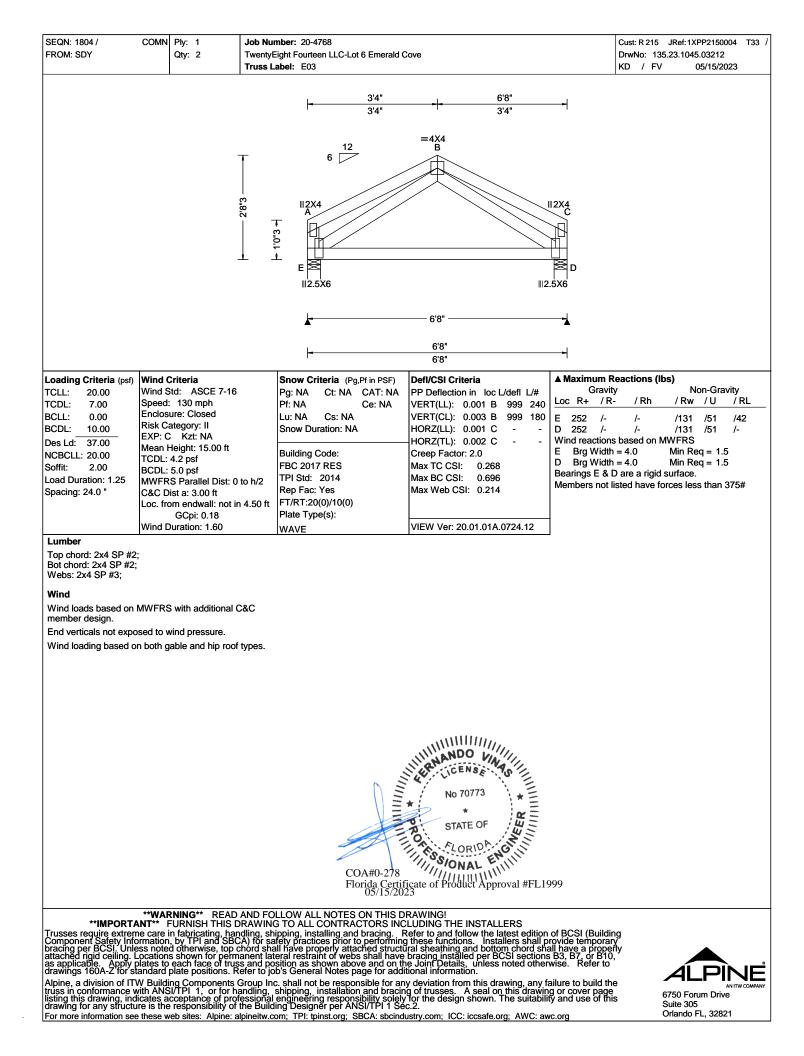


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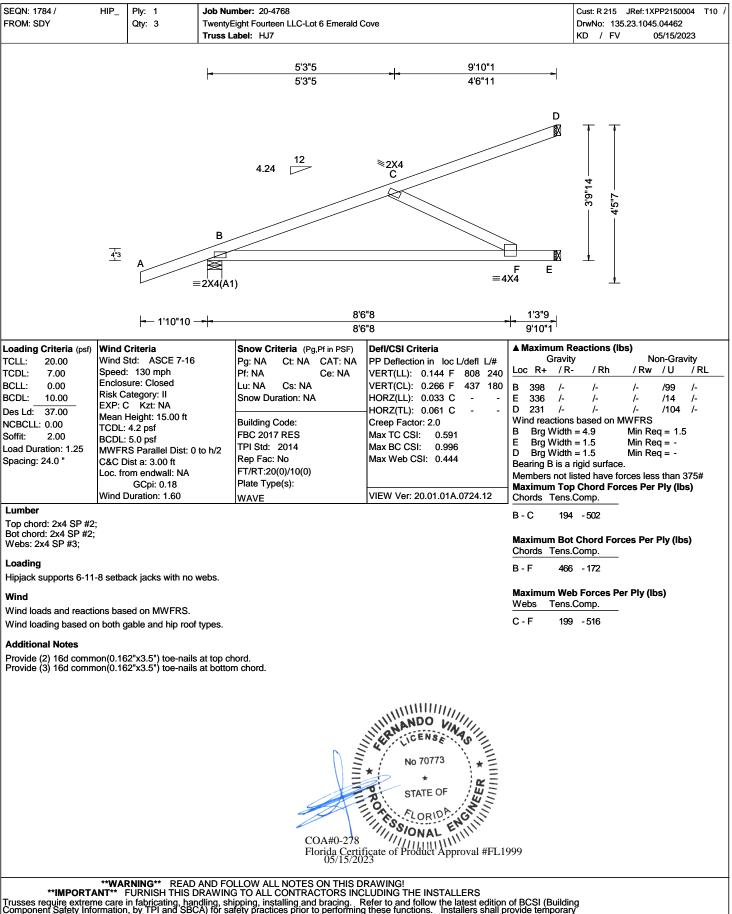




SEQN: 1879 / FROM: SDY	HIPM	Ply: 1 Qty: 1	TwentyE	nber: 20-4768 ight Fourteen LLC-Lot 6 Emerald C abel: G01	love		Cust: R 215 JRe DrwNo: 135.23.4 KD / FV		
			1	$ \begin{array}{c} 12\\ 6\\ \hline \\ 4^{*3}\\ \hline \\ = 3X4(A1) \end{array} $	St.s.t				
				▲ 	2'3"8				
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00	Wind S Speed Enclos Risk C EXP: C Mean I TCDL: BCDL: BCDL: MWFR C&C D Loc. fro	Criteria Std: ASCE 7-16 : 130 mph ure: Closed ategory: II C Kzt: NA Height: 15.00 ft 4.2 psf 5.0 psf (S Parallel Dist: 0 vist a: 3.00 ft om endwall: Any GCpi: 0.18 Duration: 1.60		Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C HORZ(TL): 0.002 C Creep Factor: 2.0 Max TC CSI: 0.088 Max BC CSI: 0.091 Max Web CSI: 0.010 VIEW Ver: 20.01.01A.0724.12	Gravit Loc R+ / R A 1079 /- C 173 /- Wind reaction A Brg Widtt C Brg Widtt Bearing A is a	- / Rh / I /- /- s based on MWF n = 4.0 Mir n = - Mir	/73 /- /14 /- RS n Req = 1.5 n Req = -	<u>RL</u>
Top chord: 2x4 SP #2; Bot chord: 2x6 SP 240 Webs: 2x4 SP #3; Special Loads (Lumber Dur.Fac. TC: From 56 plf a TC: From 10 plf a TC: 5 lb Conc. Lo BC: 1107 lb Conc. Lo BC: -3 lb Conc. Lo Hangers / Ties	=1.25 / tt 0.1 tt 1.1 tt 0. vad at 1 vad at 1 vad at 1	Plate Dur.Fac.=1. 00 to 56 plf at 87 to 40 plf at 00 to 10 plf at .87 .87 .87							
(J) Hanger Support Re Wind Wind loads and reaction Right end vertical not e Wind loading based or	ons bas exposed	ed on MWFRS. I to wind pressure			No 70773 STATE OF				
Trusses require extrem Component Safety Info bracing per BCSI. Unle attached rigid ceiling. L as applicable. Apply p drawings 160A-Z for sta	NT** I rmation ss note ocation lates to andard	FURNISH THIS D in fabricating, han i, by TPI and SBC d otherwise, top c s shown for perma each face of trus plate positions. Re	RAWING dling, shi A) for saf hord sha anent late s and pos efer to jol	LOW ALL NOTES ON THIS D TO ALL CONTRACTORS INC pping, installing and bracing. R ety practices prior to performing II have properly attached structu eral restraint of webs shall have sition as shown above and on th o's General Notes page for addit	RAWING! LUDING THE INSTALLERS tefer to and follow the latest edition these functions. Installers shall r ral sheathing and bottom chord sh bracing installed per BCSI section e Joint Details, unless noted other ional information. y deviation from this drawing, any 1 g of trusses. A seal on this drawin for the design shown. The suitabil 2.2.	n of BCSI (Buildi provide temporal all have a prope s B3, B7, or B10 rwise. Refer to	ng Yiy , , e 6750 Suite	Forum Drive	

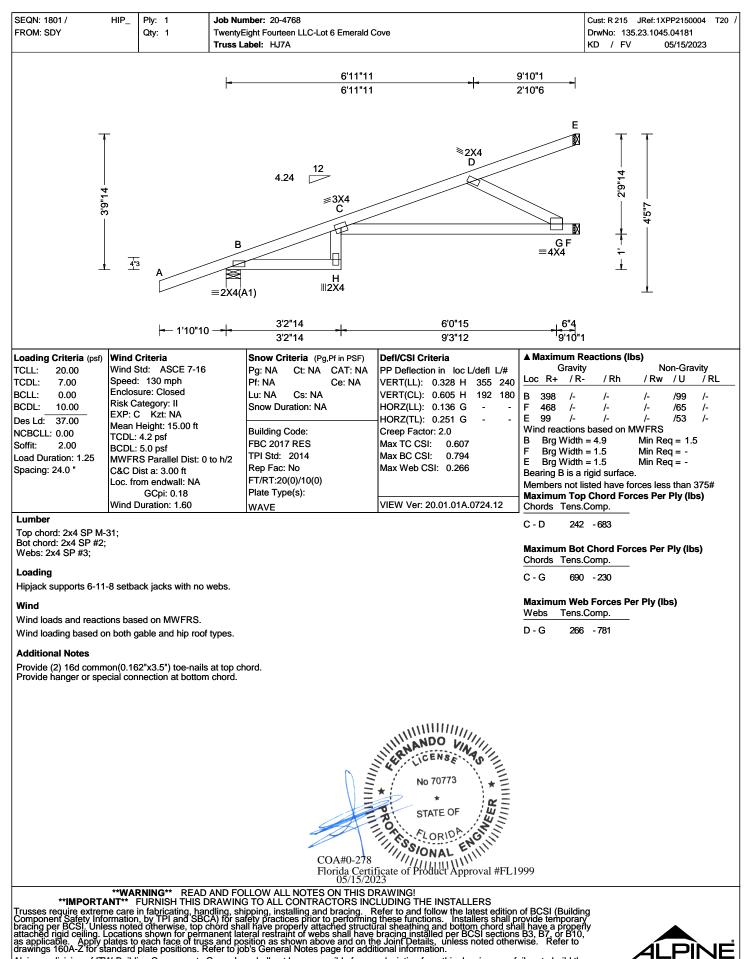
drawing for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



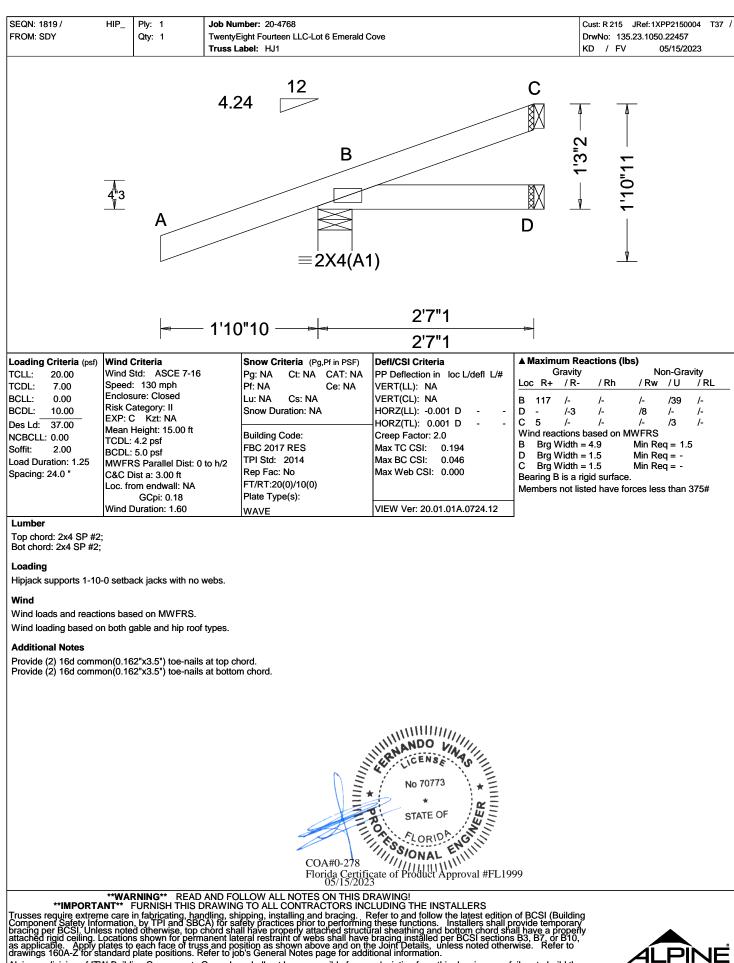


WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.



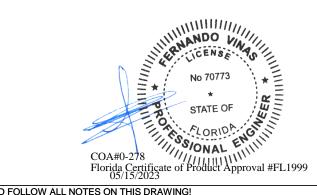








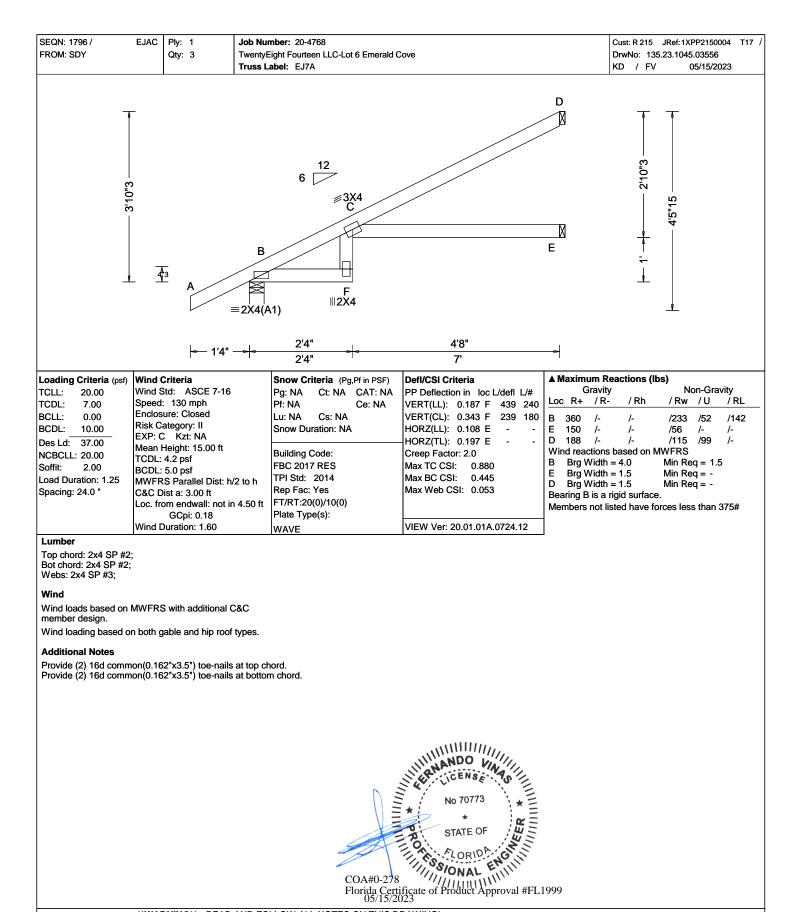
SEQN: 1781 / E FROM: SDY	EJAC Pły: 1 Qty: 15	Job Number: 20-4768 TwentyEight Fourteen LLC-Lot 6 Emerald 0 Truss Label: EJ7	Cove	Cust: R 215 JRef: 1XPP2150004 T9 DrwNo: 135.23.1045.03228 KD / FV 05/15/2023
		Truss Label: EJ7		
		a <u>12</u>		
	<u>₹</u> 3 A	B ≡2X4(A1)		□
	 - −− 1'4	4" — = =	7' 7'	- -
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R- /Rh /Rw /U /RL
JOLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 360 /- /- /233 /52 /142
	Risk Category: II EXP: C Kzt: NA	Snow Duration: NA	HORZ(LL): 0.015 D	D 187 /- /- /71 /- /-
Des Ld: 37.00	Vean Height: 15.00 ft		HORZ(TL): 0.029 D	C 169 /- /- /100 /99 /-
NCBCLL: 20.00	TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5
	BCDL: 5.0 psf	FBC 2017 RES TPI Std: 2014	Max TC CSI: 0.652 Max BC CSI: 0.723	D Brg Width = 1.5 Min Req = -
	WWFRS Parallel Dist: h	Rep Fac: Yes	Max Web CSI: 0.000	C Brg Width = 1.5 Min Req = -
	C&C Dist a: 3.00 ft _oc. from endwall: not ir			Bearing B is a rigid surface.
	GCpi: 0.18	Plate Type(s):		Members not listed have forces less than 375#
v	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12	
Lumber			•	-
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;				
Wind				
Wind loads based on M' member design.	WFRS with additional (C&C		
Wind loading based on	both gable and hip roof	types.		
Additional Notes				
Provide (2) 16d commoi Provide (2) 16d commoi				



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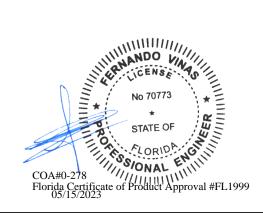


SEQN: 1778 /	JACK Ply: 1	Job Num	1ber: 20-4768			Cust: R 215 Ji	Ref:1XPP215000	04 T6
FROM: SDY	Qty: 6	TwentyEi	ight Fourteen LLC-Lot 6 Emerald C	Cove		DrwNo: 135.2	3.1045.04588	
		Truss La	ibel: CJ5			KD / FV	05/15/202	23
	<u></u> → A		6 12 B 2X4(A1)		D	3'5"15		
	-	·— 1'4" —		5' 5'				
_oading Criteria (psf)	Wind Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum R	eactions (lbs)	1	
CLL: 20.00	Wind Std: ASCE 7-		Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravit		Non-Gravi	
CDL: 7.00	Speed: 130 mph		Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R	- / Rh	/Rw /U	/ RL
BCLL: 0.00	Enclosure: Closed		Lu: NA Cs: NA	VERT(CL): NA	B 288 /-	/-	/191 /47	/106
BCDL: 10.00	Risk Category: II EXP: C Kzt: NA		Snow Duration: NA	HORZ(LL): 0.005 D	D 132 /-			/-
Des Ld: 37.00	Mean Height: 15.00	ft		-HORZ(TL): 0.010 D	C 116 /-			/-
NCBCLL: 20.00	TCDL: 4.2 psf		Building Code:	Creep Factor: 2.0	Wind reactions B Brg Width		lin Reg = 1.5	
Soffit: 2.00	BCDL: 5.0 psf		FBC 2017 RES TPI Std: 2014	Max TC CSI: 0.329 Max BC CSI: 0.357	D Brg Width		/lin Reg = -	
Load Duration: 1.25	MWFRS Parallel Dis	it: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.357	C Brg Width	= 1.5 N	/in Req = -	
Spacing: 24.0 "	C&C Dist a: 3.00 ft Loc. from endwall: no		FT/RT:20(0)/10(0)	Max Web COI. 0.000	Bearing B is a	•		
	GCpi: 0.18		Plate Type(s):		Members not I	isted have force	es less than 3.	/5#
	Wind Duration: 1.60		WAVE	VIEW Ver: 20.01.01A.0724.12				
Lumber				-	-			
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2;								
Wind								
Wind loads based on member design.	MWFRS with addition	al C&C						

member design. Wind loading based on both gable and hip roof types.

Additional Notes

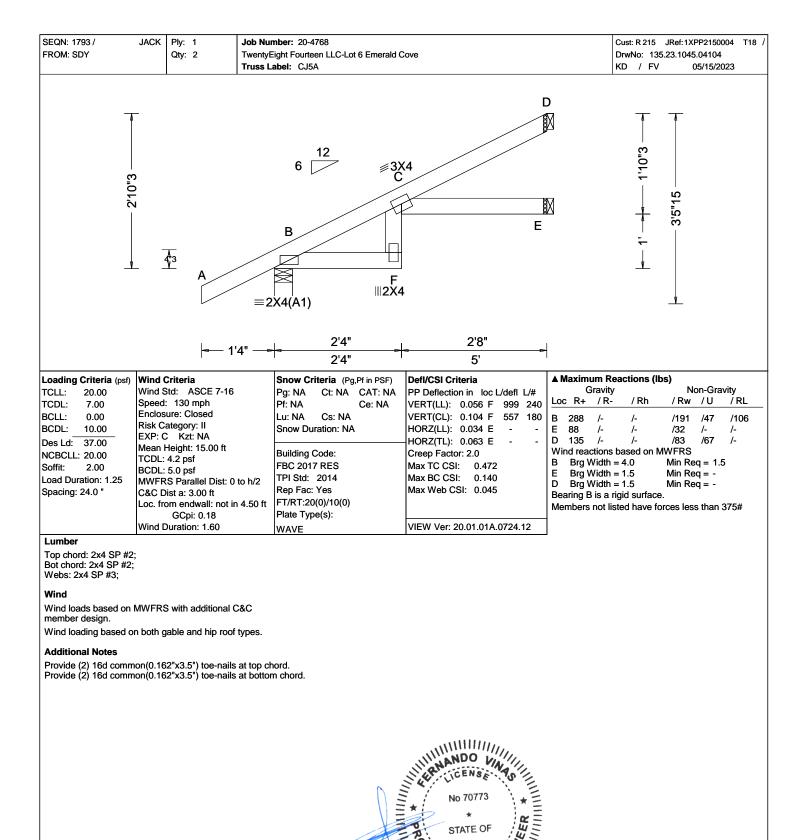
Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



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ROTESSIONAL COA#0-278 Florida Certificate of Product Approval #FL1999 05/15/2023

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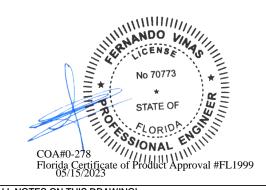
SEQN: 1775 / JA FROM: SDY	Qty: 6 Twee	Number: 20-4768 tyEight Fourteen LLC-Lot 6 Emerald (s Label: CJ3	Cove	Cust: R 215 JRef: 1XPP2150004 T7 / DrwNo: 135.23.1045.03478 KD / FV 05/15/2023
	4 [™] 3 <u>4[™]3</u> A	6 12 6 B = 2X4(A1)	C C D D	
TCLL: 20.00 Wi TCDL: 7.00 Sp BCLL: 0.00 En BCDL: 10.00 Rix Des Ld: 37.00 MC NCBCLL: 20.00 TC Soffit: 2.00 BC Load Duration: 1.25 MM	ind Criteria ind Std: ASCE 7-16 beed: 130 mph hclosure: Closed sk Category: II (P: C Kzt: NA ean Height: 15.00 ft DL: 4.2 psf CDL: 5.0 psf WFRS Parallel Dist: 0 to h/2 &C Dist a: 3.00 ft bc. from endwall: Any GCpi: 0.18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Snow Duration: NA Snow Duration: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014 Snow FT/RT:20(0)/10(0) Snow Prace (S):	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 D HORZ(TL): 0.002 D Creep Factor: 2.0 Max TC CSI: 0.246 Max BC CSI: 0.111 Max Web CSI: 0.000	$\label{eq:constraint} \begin{array}{ c c c c c } \hline \textbf{Maximum Reactions (lbs)} \\ \hline Gravity & Non-Gravity \\ \hline Loc R+ /R- /Rh / Rw /U / RL \\ \hline B 222 /- /- /153 /43 /71 \\ \hline D 75 /- /- /27 /- /- \\ \hline C 59 /- /- /32 /37 /- \\ \hline Wind reactions based on MWFRS \\ \hline B Brg Width = 4.0 & Min Req = 1.5 \\ \hline D Brg Width = 1.5 & Min Req = - \\ \hline C Brg Width = 1.5 & Min Req = - \\ \hline C Brg Width = 1.5 & Min Req = - \\ \hline Bearing B is a rigid surface. \\ \hline \end{array}$
Lumber	ind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12]
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;				
Wind	/FRS with additional C&C			

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

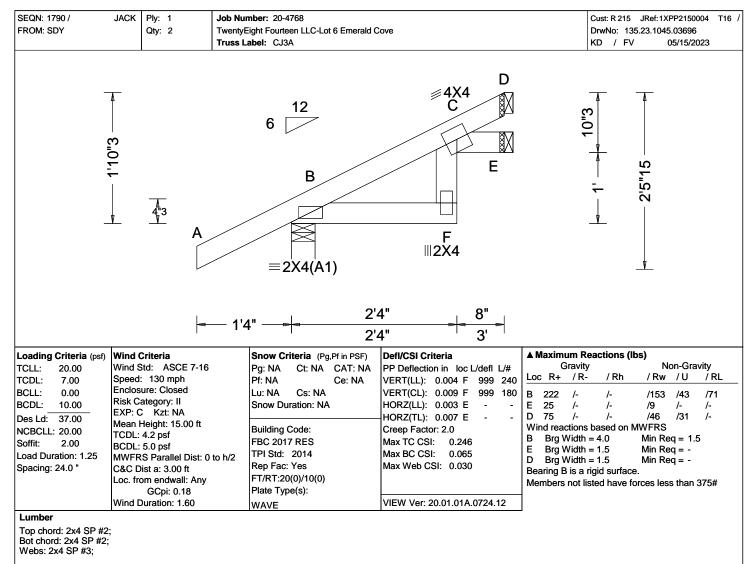
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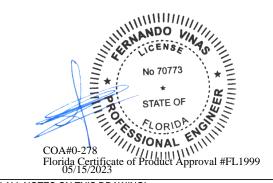
Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types.

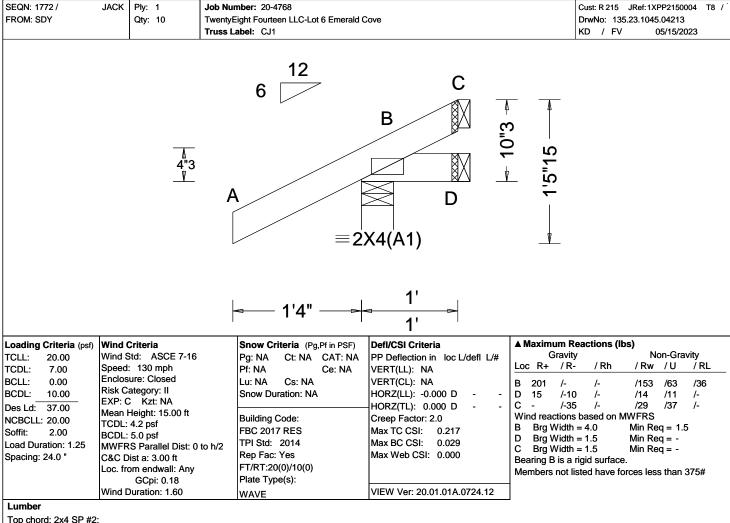
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Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types.

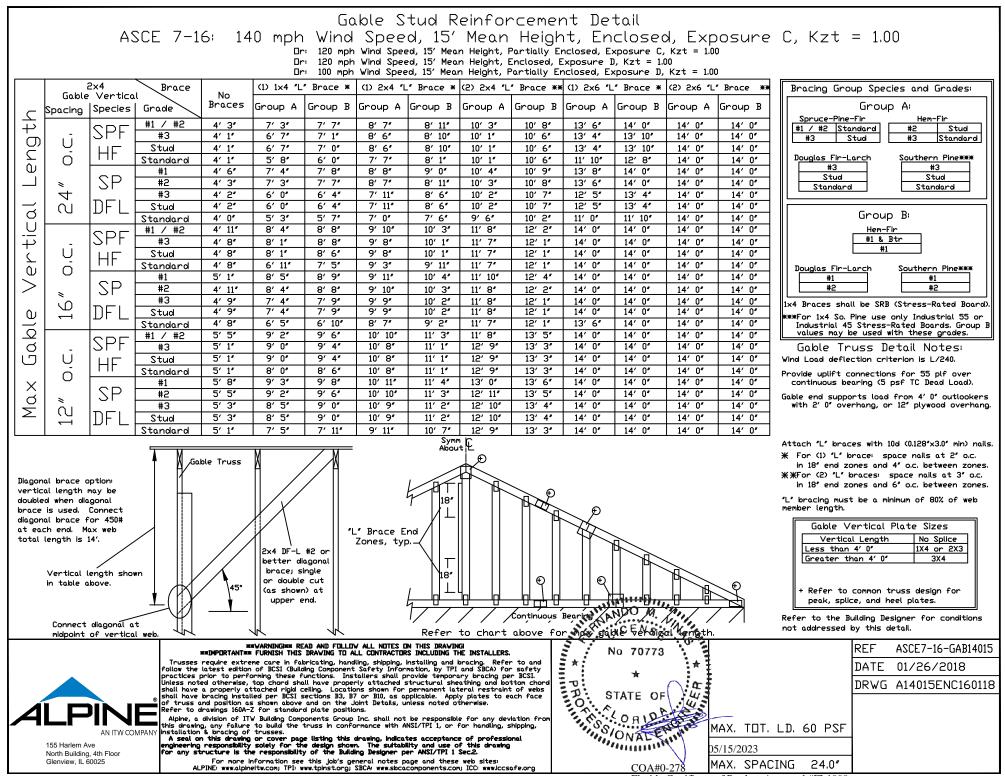
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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	
Size	Restraint	T- or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1−2×6
2×6	2 rows	2×6	2−2×4(X)
2×8	1 row	2×6	1-2×8
2×8	2 rows	2×6	2-2×6(%)

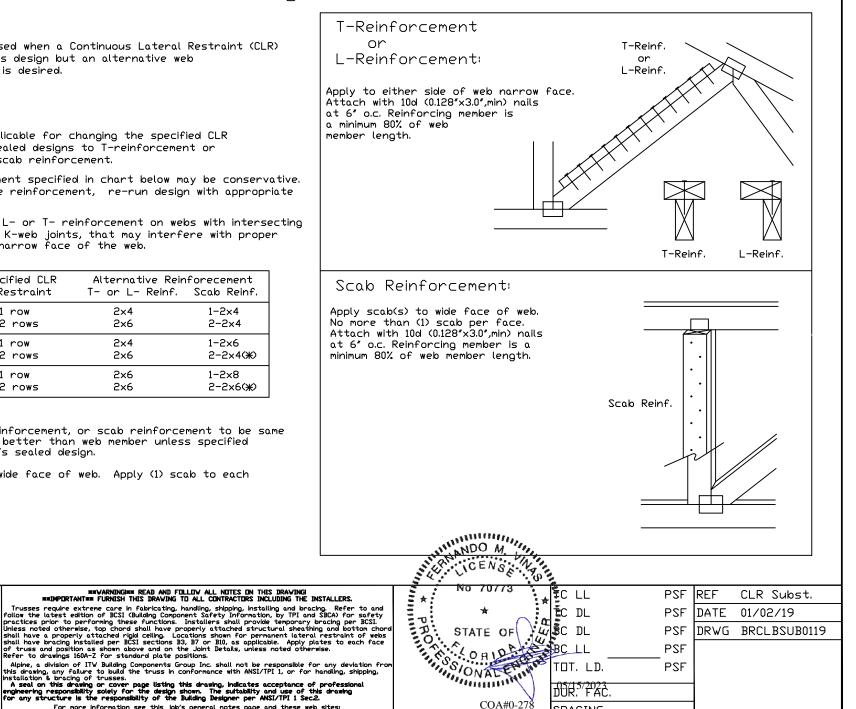
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(Ж) Center scab on wide face of web. Apply (1) scab to each face of web.

AN ITW COMPAN

155 Harlem Ave North Building, 4th Floor

Glenview, IL 60025

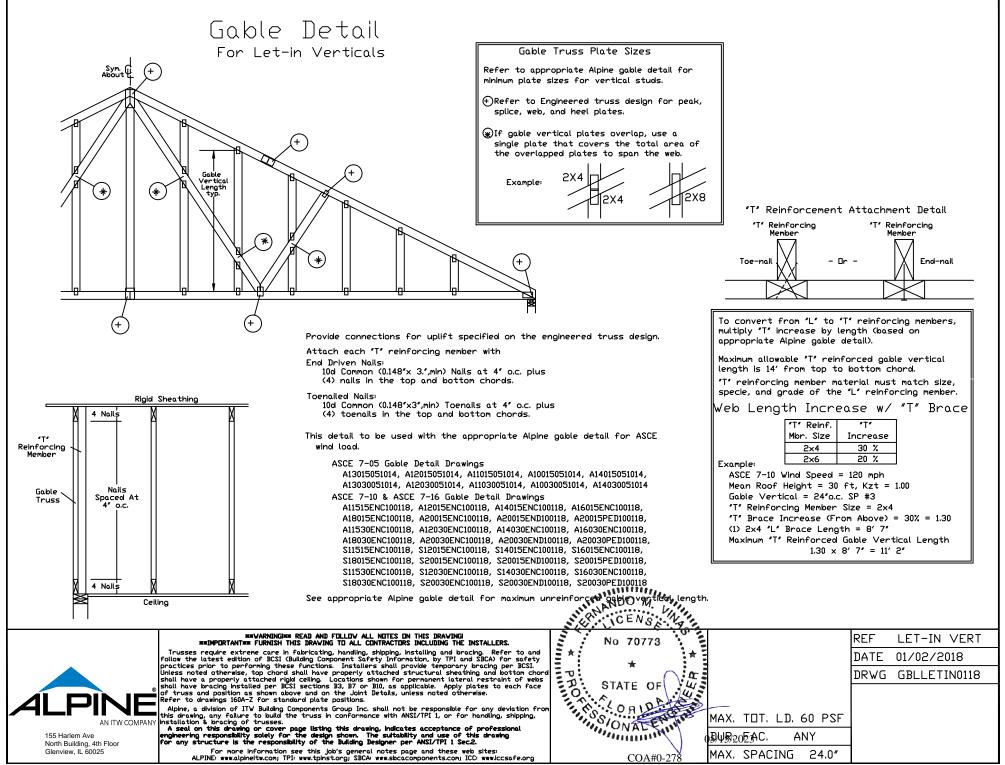


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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcaccomponents.com; ICC: www.iccsafe.org



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