For Office Use Only Application # 070/- 93 Date Rec	ejved 1/25/07 By 4 Permit # 25489
Application Approved by - Zoning Official Date	Plans Examiner OK 7th Date 1-30-06
Flood Zone NA Development Permit NA Zoning	RSF1 Land Use Plan Map Category RSF1
Comments	
Tornado Damage - No charge for P.	
Applicants Name Richardson Aluminum.	Phone 384-755-5779
Address 692 S.W. Arlington Blvd. L	AKE CITY, FI BADAS
Owners Name Bruce + Patti Eichman	Phone 386 - 758 - 1917
911 Address 225 N.W. Scienic Lake [Drive
Contractors Name RIChardSON ALUMINUM LL	C Phone 384-755-5779
Address 692 S.W. ArlINGTON Blud LAKE	
Fee Simple Owner Name & Address	
Bonding Co. Name & Address N/A	
Architect/Engineer Name & Address Bennett	
Mortgage Lenders Name & Address NA	
Circle the correct power company – FL Power & Light – Clay	Elec. – Suwannee Valley Elec. – Progressive Energy
	Estimated Cost of Construction 14,000 800
Subdivision Name Woodburrough	Lot Block Unit Phase 8
Driving Directions Lake Jeffery @ into We	and huscoush then 1st
Home on Right	to a series of the series of t
The state of the s	
Type of Construction Screen Enclosure No	umber of Existing Dwellings on Property
Total Acreage 1.20 Lot Size 175×300 Do you need a - Culve	
Actual Distance of Structure from Property Lines - Front 143	
otal Building Height 1000 Number of Stories He	
Application is hereby made to obtain a permit to do work and installation has commoned prior to the leguence of a result and	tallations as indicated. I certify that no work or
installation has commenced prior to the issuance of a permit and all laws regulating construction in this jurisdiction.	that all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing inform	nation is accurate and all work will be done in
compliance with all applicable laws and regulating construction a	•
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE O TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTE	F COMMENCMENT MAY RESULT IN YOU PAYING
LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF	F COMMENCEMENT.
	1/ Mice Michardson
Owner Builder or Agent (Including Contractor)	0-4-4-0
STATE OF FLORIDA	Contractors License Number 5618
COUNTY OF COLUMBIA AMY MARTS MY COMMISSION # DD458730	Competency Card Number 5129 NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed hefore Expires: Aug. 7, 2009	
this day of	Je 11 aves
Personally known or Produced Identification	Notary Signature

January 01, 2007

LAWRENCE E. BENNETT, P.E. P.O. BOX 214368 SOUTH DAYTONA, FL 32121 386-767-4774

TO ALL BUILDING DEPARTMENTS

. . .

Re: Master File Engineering
"ALUMINUM STRUCTURES DESIGN MANUAL"
2004 edition & 2006 edition

Dear Building Official/Plans Examiner,

This is to certify that the following contractor/company is hereby authorized to use my "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2007. When we publish and distribute the 2006 ed of the "ALUMINUM STRUCTURES DESIGN MANUAL", they will be authorized to use that manual for the remainder of 2007.

Our authorization is based on a January to January basis requardless of the edition of the manual. This authorization also applies to contractor master file drawings, "ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

Vince Richardson

Richardson Aluminum LLC 692 SW Arlington Blvd Lake City, FL 32025

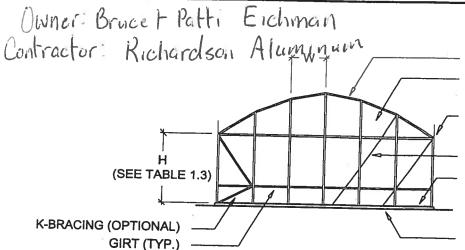
They are hereby added to my 2007 MASTERFILE LIST

Should you have any questions please contact me at your convenience.

Sincerely,

Lawrence A. Bengett, P.E. #16644

SECTION 1



PURLINS (TYP.)

SCREEN (TYP.)

CABLE CONNECTION (SEE DETAILS SECTION 1)

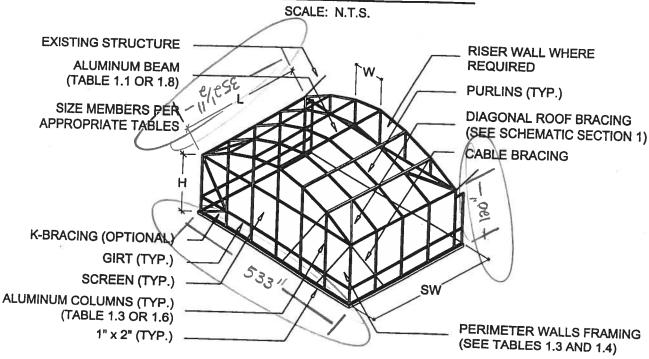
ALTERNATE CABLE

1" x 2" (TYP.)

GRADE

CABLE CONNECTION (SEE DETAILS SECTION 1)

TYPICAL DOME ROOF - ELEVATION



TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

FILE COPY

Lawrence E. Bennett, P.E.

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PAGE

SECTION 1 SCREENED ENCLOSURES

General Notes and Specifications:

- The following structures are designed to be married to block and wood frame structures of adequate structural capacity. The contractor / home owner shall verify that the host structure is in good condition and of sufficient strength to hold the proposed addition.
- 2. If there is a question about the host structure, the owner (at his own expense) shall hire an architect, engineer, or a certified home inspection company to verify host structure capacity.
- The structures designed using this section shall be limited to a maximum combined span and upright height of 55' and a maximum upright height of 20'. Structures larger than these limits shall have site specific engineering.
- 4. Spans are for enclosures with mean roof heights less than 30'. For greater heights, consult engineer.
- 5. Connections to fascia shall be limited to overhangs of 24" or less unless site specific engineering is provided.
- 6. The proper structural name for a chair rail or top rail of an enclosure is a girt. Thus the terminology shall be interchangeable.
- Screws that penetrate the water channel of the super gutter shall have ends clipped off for safety of cleaning gutter and the heads of screws through the gutter into the fascia shall be caulked.
- 8. Section 7 contains span tables and attachment details for pans and composite panels.
- 9. When using TEK screws in lieu of S.M.S., longer screws must be used to compensate for drill head.
- 10. An additional super gutter strap or ferrule is required to be located near the midpoint of the beam spacing. Straps shall be attached to each truss / rafter tall when a 2" sub-fascia does not exist. Straps at the beam are not required when straps are placed @ each truss / rafter tail and spacing of straps does not exceed 2'-0".
- 11. Super or extruded gutter details are applicable to all widths of super or extruded gutters, and gutters may be substituted. Gutter straps and/or ferrules shall be the width of the inside and outside of the super or extruded gutter respectively.
- 12. If the sub-fascia is 3/4", and the sub-fascia is in good repair, a 3/4" P.T.P. strip the width of the fascia may be added to the existing sub-fascia by attaching the plywood with (2) 3" 16d common nails or (2) 3" #8 screws. This gives the equivalent of a 2" fascia.
- 13. Spans may be interpolated between values but not extrapolated outside values.
- 14. For Design Check List and Inspection Guides for Screened Enclosures, see Appendix (Section 10).

Section 1 Design Statement:

The structures designed for Section 1 are framing systems with screen roofs & walls and are considered open structures. The design loads used are from Chapter 20 of the 2001 Florida Building Code. The loads assume a mean roof height of less than 30'; roof slope of 0° to 20°; I = 0.77. All loads are based on 20 / 20 screen or larger. Wall heights may be increased when using 18 / 14 screen. All pressures

General Notes and Specifications for Section 1 Tables:

Section 1 Design Loads

for Structures with Screen Roof & Walls

Wind Velocity	Expos	rie B.	Exposure 'C'					
	Roofs Note 1	Walls Note 1	Roofs Note 1	Walis				
100 M.P.H	10	10		Note 1				
110 M.P.H	10	11	10	13				
120 M.P.H	10		10	16				
123 M.P.H	10	14	10	19				
130 M.P.H		14.6	10	19.9				
140 M.P.H	10	16	10	22				
150 M.P.H	10	18	10	26				
130 M.P.H	10	21	10	30				

Note 1: per Table 2002.4 or paragraph 1606.1.2

Note 2: Multipliers for 'C' exposure catagory are for wall loads only.

Conversion Table 1A

Wind Zone Conversions for Screen Wall Frame Members Only

From 120 MPH Wind Zone to Others

	Expos	ure 'B'
Applied Load #/ Sq. Ft.	Deflection	Bending
10		'b'
11		1.18
14		1.13
		1.00
		0.98
	0.96	0.94
	0.92	0.88
21	0.87	0.82
	Applied Load #/ Sq. Ft. 10 11 14 15 16 18 21	Load #/ Sq. Ft. 'd' 10 1.12 11 1.08 14 1.00 15 0.99 16 0.96 18 0.92

Multipliers are for wall loads only, no conversions are necessary for roof frame members.

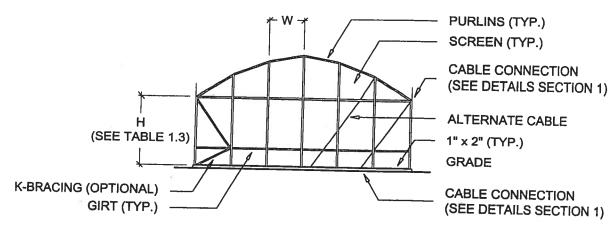
Conversion Table 1B

Conversion Based on Mean Height of Host

Structure for Screen Wall Frame Members From Exposure 'B' to 'C'

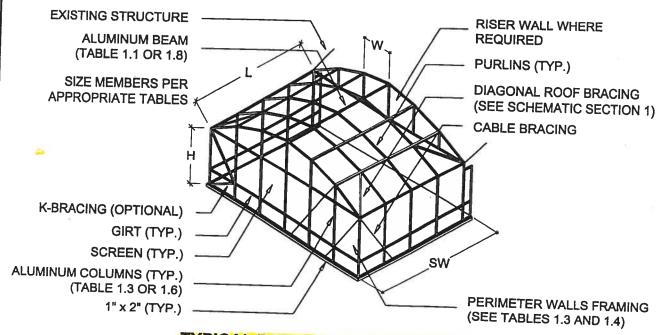
Bending
1 'b'
0.94
0.92
0.91
0.89

SECTION 1



TYPICAL DOME ROOF - ELEVATION

SCALE: N.T.S.



TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

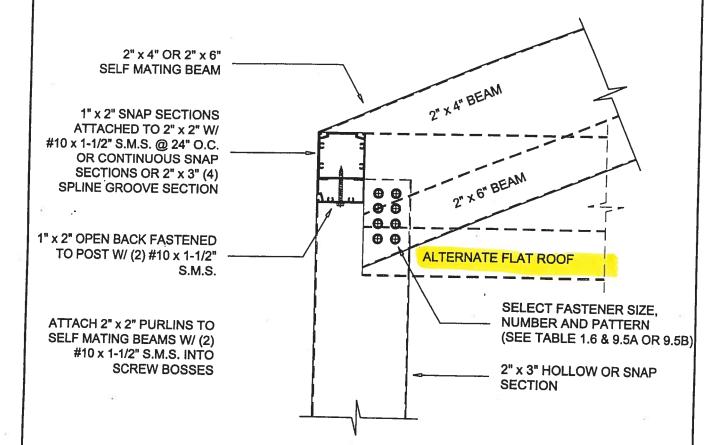
CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

Lawrence E. Bennett, P.E.

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SCREENED ENCLOSURES

MINIMUM POST SIZES REQUIRED FOR EACH BEAM SIZE (SEE TABLE 1.6)



SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

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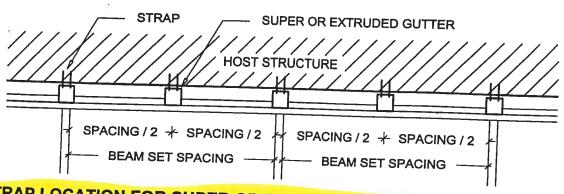
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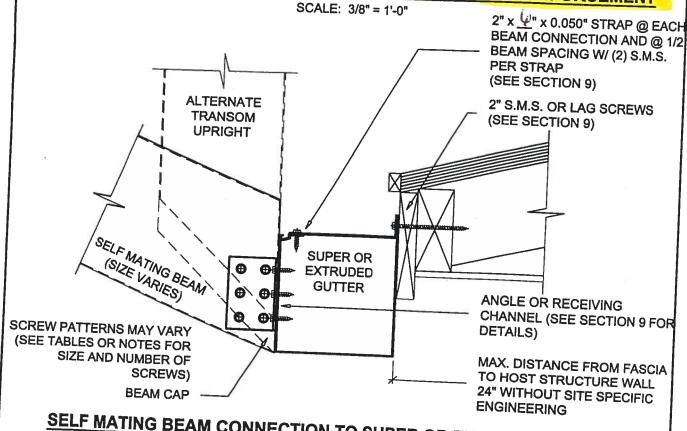
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STRAP LOCATION FOR SUPER OR EXTRUDED GUTTER REINFORCEMENT



SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

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PAGE

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SCREENED ENCLOSURES SECTION 1 COMPOSITE EAVE RAIL W/ 2" x 2" FASTENED TO BEAM / UPRIGHT W/ (3) #10 x 1-1/2" **BEAM - SCREEN ROOF** S.M.S. MIN. INTO SCREW BOSSES. 1" x 2" ATTACHED POST TO BEAM FASTENING ⊕ ⊕ TO 2" x 2" W/ #10 x 1-1/2" S.M.S. (SEE TABLE 1.6) **# #** @ 24" O.C. CONTINUOUS 2" x (TRANSOM PANEL) HEIGHT VARIES **8 8** 3" SNAP SECTION FASTENED THRU SCREW BOSSES W/ (3) **# #** MIN. #10 x 1-1/2" OR 2" x 3" **HOLLOW SECTION FASTENED** THRU SCREW BOSSES W/ 1.3) #10 x 1-1/2" S.M.S. TABLE 1 SCREEN (MAY FACE ENGTH OF IN OR OUT) KNEE BRACE SEE. 45° FASCIA AND SUB-FASCIA (2) 2" SCREWS (SEE SECTION 9 FOR SCREW SIZES) REQUIRED KNEE BRACE MININUM SIZE AND **CONNECTION (PER TABLE 1.7)** USE ANGLE EACH SIDE FOR 2" x 2" TO POST CONNECTION SUPER OR **EXTRUDED** W/ HOLLOW POST **GUTTER** 1/4"Ø BOLT @ 24" O.C. MAX. MAX. DISTANCE TO WITHIN 6" OF EACH POST HOST STRUCTURE FASTEN 2" x 2" POST W/ (3) WALL 24" WITHOUT -EACH #10 S.M.S. INTO SCREW SITE SPECIFIC **SPLINES ENGINEERING** 2" STRAP - LOCATE AND IF KNEE BRACE LENGTH **FASTEN (DETAILS PAGE 1-20) EXCEEDS TABLE 1.7 USE** 2" x 2" x 0.093" ANGLE W/ (4) **CANTILEVERED BEAM** S.M.S. (SEE SECTION 9 FOR CONNECTION DETAILS SCREW SIZES) EACH SIDE TO **KNEE BRACE ATTACHMENT 6"** BEAM TO SUPER OR ABOVE TOP OF GUTTER MAX. EXTRUDED GUTTER SUPER OR EXTRUDED GUTTER RISER (OR TRANSOM) WALL @ FASCIA - DETAIL 1 SCALE: 3" = 1'-0"

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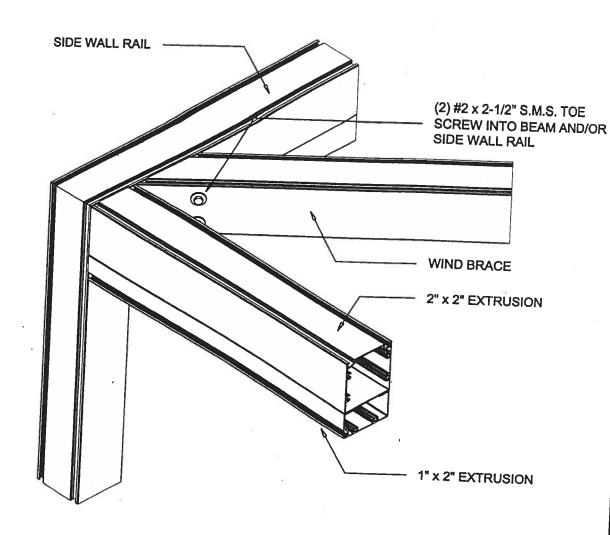
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SCREENED ENCLOSURES



WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTE:

WIND BRACING SHALL BE PROVIDED @ EACH SIDE WALL PANEL WHEN ENCLOSURE PROJECTS MORE THAN (4) PANELS FROM HOST STRUCTURE

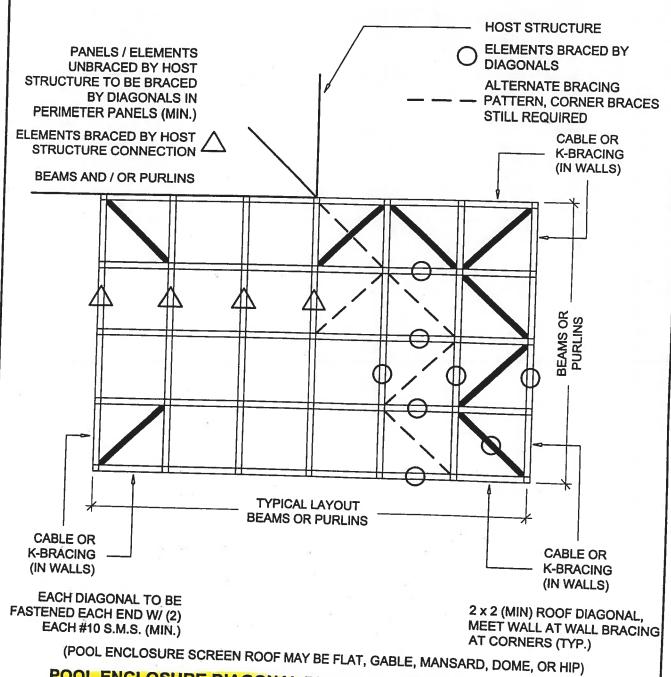
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SECTION 1



POOL ENCLOSURE DIAGONAL BRACING - SCHEMATIC PLAN VIEW

SCALE: 3/8" = 1'-0"

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CABLE BRACING

General Notes and Specifications:

- 1) The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:
 - a) FRONT WALL CABLES 7 x 7 OR 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
1/8"	445 Sq. Ft. / PAIR OF CABLES

* TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE:

FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft. SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft. TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 7 OR 7 x 19 STAINLESS STEEL

CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

^{**} SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

c) To calculate the required pair of cables for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

NOTE: For wall heights greater than 9'-8", a minimum of (2) wall cables are required.

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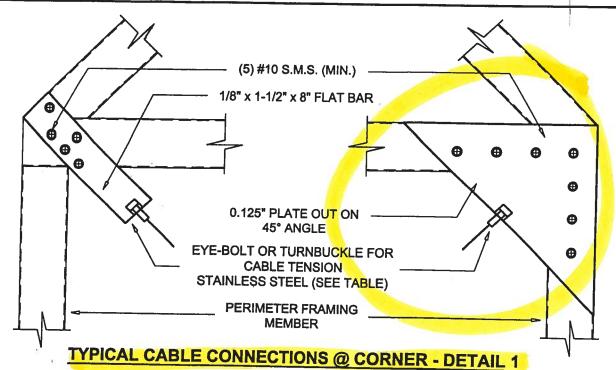
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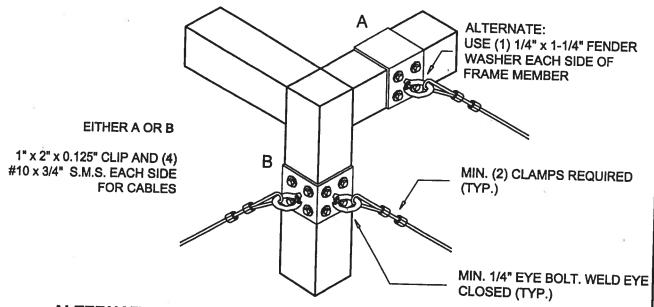
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SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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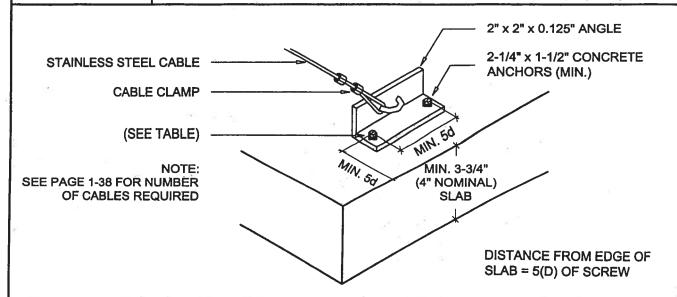
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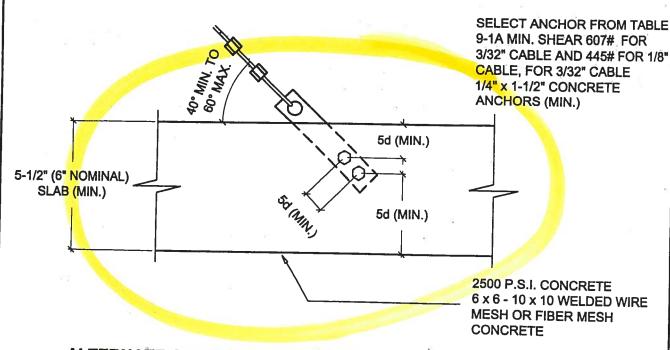
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SCREENED ENCLOSURES



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



ALTERNATE CABLE CONNECTIONS @ FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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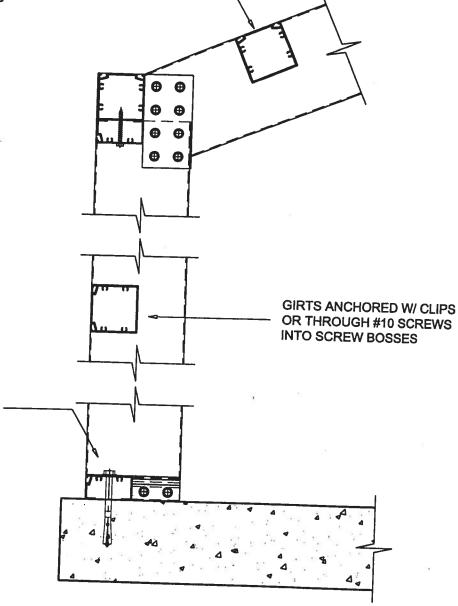
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SCREENED ENCLOSURES

PURLINS ANCHORED W/ CLIPS OR THROUGH #10 SCREWS INTO SCREW BOSSES



FRONT AND SIDE BOTTOM **RAILS ATTACHED TO** CONCRETE W/ 1/4" x 2-1/4" CONCRETE / MASONRY ANCHORS @ 6" FROM EACH POST AND 24" O.C. MAX. AND WALLS MIN. 1" FROM EDGE OF CONCRETE

PURLIN & CHAIR RAIL DETAIL

SCALE: 3" = 1'-0"

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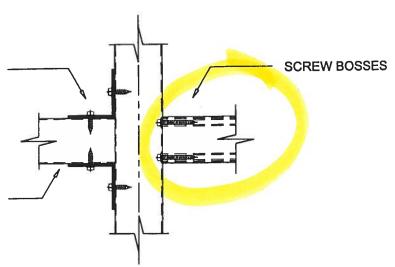
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SECTION 1

INTERNAL OR EXTERNAL 'L' CLIP OR 'U' CHANNEL CHAIR RAIL ATTACHED TO POST W/ MIN. (4) #10 S.M.S.

GIRT OR CHAIR RAIL 2" x 2" x 0.044" HOLLOW MIN.



GIRT TO POST DETAIL

SCALE: 3" = 1'-0"

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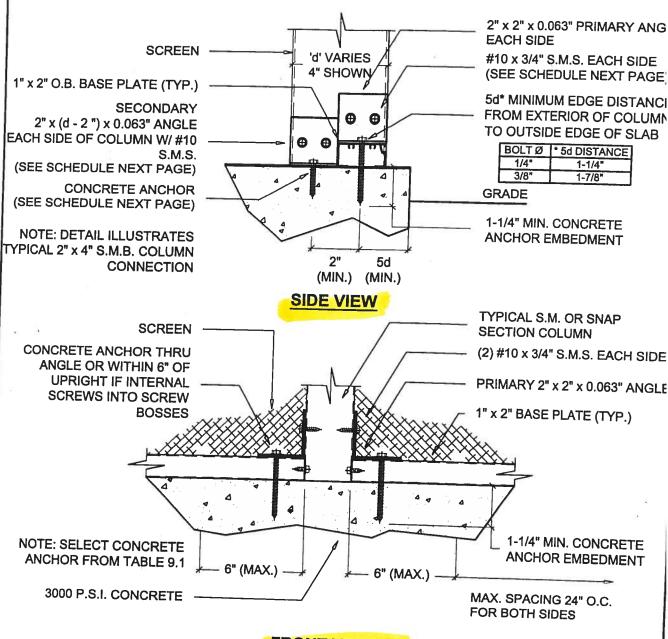
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SECTION 1



FRONT VIEW

2" x 4" OR LARGER SELF MATING OR SNAP SECTION POST TO DECK DETAILS

SCALE: 3" = 1'-0"

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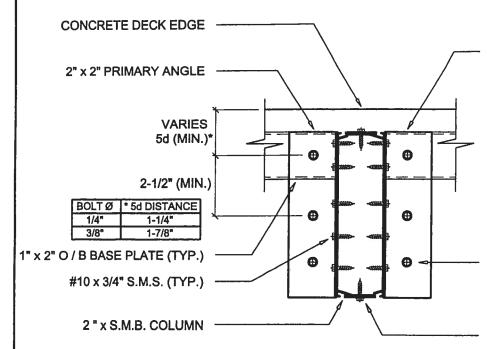
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SCREENED ENCLOSURES



DETAIL ILLUSTRATES TYPICAL 2" x 4" S.M.B. THRU 2" x 9" SUB CONNECTIONS

SCREEN

SECONDARY 2" x 2" x 0.063" ANGLE (SEE SECONDARY ANGLE ANCHOR SCHEDULE AND SECTION 9)

CONCRETE ANCHORS INTO PRIMARY AND SECONDARY **ANGLES**

S.M.S. STICHING SCREWS @ 24" O/C FOR S.M.B. (SEE TABLE 1.6 FOR SIZE)

TOP VIEW POST TO DECK DETAIL

SCALE: 3" = 1'-0"

SECONDARY	ANGLE ANCHO	R SCHEDULE
COLUMN SIZE S.M.B.OR SB	TOTAL CONCRETE ANCHORS	TOTAL #10 x 3/4" S.M.S.
2 x 4	(4) 1/4"	6
2 x 5	(4) 1/4"	10
2 x 6	(4) 1/4"	8
2 x 7	(4) 1/4"	12
2 x 8	(6) 1/4"	14
2 x 9	(6) 1/4"	16
2 x 10	(10) 1/4"	18

SEE SECTION #9 FOR ADDITIONAL ANCHOR INFORMATION

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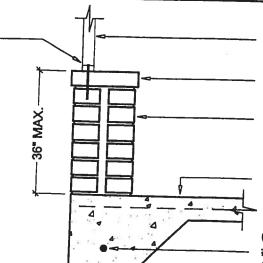
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SCREENED ENCLOSURES

1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND ROWLOCK INTO FIRST COURSE OF BRICKS

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL 1" WIDE x 0.063" THICK STRAP @ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4" S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR FOOTING



ALUMINUM FRAME SCREEN
WALL

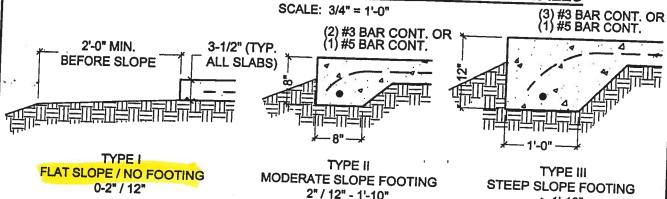
CAP BRICK

BRICK KNEEWALL TYPE 'S' MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO CONCRETE SLAB W/ 6 x 6 -10 x 10 WELDED WIRE MESH OR FIBER MESH CONCRETE

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS



- Notes: 1. No footing required except when addressing erosion until the slab width in the direction of the primary exceeds 32 ft., then a type II footing is required under the load bearing wall only unless the side wall exceeds 16 ft. in height or the enclosure is in a "C" exposure catagory in which case a type II footing is required for all walls.
 - 2. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified, prior to placing the slab, by field soil test or a soil testing lab.
 - The slab / foundation shall be cleared of debris and roots and compacted prior to placement of concrete.
 - 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or fiber mesh may be used in lieu of mesh.
 - If local building codes require a minimum footing, use type II footing or footing sections required by local code. Local code governs.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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Table 1.1 Allowable Spans For Primary Screen Roof Frame Members Aluminum Alloy 6063 T-6

For Areas with Wind Loads up to 150 M.P.H. and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

	Tributary Load Width 'W' = Beam Spacing														
Hollow Sections	3'-0"		4'-0"		5'-0'		6'-0"		7'-0"		8'-0"		9'-0'	10	
	Allowable Span 'L' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	9'-10"	b	8'-7"	Ь	7'-8"	Ь	6'-11"	БĬ	6'-6"	h	6'-1"	Ь	5'-8"	-	
2" x 2" x 0.055"	10'-9"	Ь	9'-4"	b	8'-4"	b	7'-7"	Б	7'-1"	<u> </u>	6'-7"	-	6'-3"		
2" x 3" x 0.045"	13'-4"	b	11'-7"	Ь	10'-4"	ь	9'-5"	ь	8'-9"	<u> </u>	8'-2"	뉴	7'-8"		
2" × 4" × 0.050"	14'-8"	ь	12'-8"	ь	11'-4"	ь	10'-4"	<u> </u>	9'-7"	<u> </u>	8'-11"	핅	8'-5"		

				T	ributary i	Loa	d Width	w	= Beam	Sp	acing	cing							
Self Mating Sections	3'-0'	3'-0" 4'-0"			5'-0"		6'-0"		7'-0"		8'-0"		9'-0'						
	Allowable Span 'L' / bending 'b' or deflection 'd'																		
2" x 4" x 0.044 x 0.100"	19'-11"	Ь	4	ь	15'-6"	ь	14'-2"	b	13'-1"	b	12'-3"	b	11'-6"	_					
2" x 5" x 0.050" x 0.100"	24'-9"	Ь	21'-5"	ь	19'-2"	ь	17'-6"	ь	16'-2"	b	15'-2"	<u> </u>	14'-3"	౼					
2" x 6" x 0.050" x 0.120"	28'-7"	b	24'-9"	b	22'-2"	b	20'-3"	b	18'-9"	ь	17'-6"	-	16'-6"	ᆢ					
2" x 7" x 0.055" x 0.120"	32'-3"	ь	27'-11"	ь	24'-11"	h	22'-9"	Б	21'-1"	ь	19'-9"	-		<u>D</u>					
2" x 7" x 0.055" w/ Insert	42'-10"	b	37'-1"	b	33'-2"	ь	30'-4"	ь	28'-1"	b	26'-3"	-	18'-7"	ᆢ					
2" x 8" x 0.072" x 0.224"	41'-7"	b	36'-1"	ь	32'-3"	ь	29'-5"	b	27'-3"	b	25'-6"	-	24'-9"	<u>_</u>					
2" × 9" × 0.072" × 0.224"	45'-1"	b	39'-1"	ь	34'-11"	Ь	31-11	h	29'-6"	b	27'-8"	믠	24'-0"	<u>b</u>					
2" x 9" x 0.082" x 0.310"	49'-6"	ь	42'-11"	b	38'-4"	Ь	35'-0"	뷙	32'-5"	p,	30'-4"	은	26'-1"	<u> b</u>					
2" x 10" x 0.092" x 0.369"	59'-6"	ь	51'-7"	h	46'-1"	Ь	42'-1"	Ь	38'-11"	吊	36'-5"	위	28'-7" 34'-4"	ь					

	Tributary Load Width 'W' = Beam Spacing													
Snap Sections 3'	3'-0"		4'-0"	4'-0" 5'-0			6'-0"		7'-0"		8'-0"		9'-0'	m
	Allowable Span 'L' / bending 'b' or deflection 'd'													
2" x 2" x 0.044"	11'-9"	ь	10'-2"	b	9'-1"	h	8'-4"	h	7'-8"	Ь	7'-2"	-	01.07	-
2" x 3" x 0.045"	15'-1"	ь	13'-1"	h	11'-8"	b	10'-8"	-	9'-10"	- L		_D	6'-9"	_ <u>b</u>
2" x 4" x 0.045"	18'-5"	Ь	15'-11"	Б	14'-3"	븬		D .		D	9'-3"	b	8'-8"	b
2" x 6" x 0.062"	31'-3"			끈		-0	13'-0"	D	12'-1"	ь	11'-3"	_b	10'-8"	b
2" x 7" x 0.062"		ь	27'-1"	D	24'-2"	₽	22'-1"	ь	20'-5"	ь	19'-2"	Ь	18'-0"	ь
otes:	34'-9"	D	30'-1"	b	26'-11"	b	24'-7"	b	22'-9"	Ь	21'-3"	ь	20'-1"	h

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 55' and a maximum upright height of 20'. Structures larger than these limits shall have site specific engineering.
- 3. Spans are based on a minimum of 10# / Sq. Ft. for up to a 150 M.P.H. wind load.
- 4. Span is measured from center of beam and upright connection to fascia or wall connection.
- 5. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 6. Purlin spacing shall not exceed 6'- 8". For beam spans greater than 40'-0" the beam at the center purlin and one purlin for each 14'-0" on each side of the center purlin shall include lateral bracing as shown in detail (48'-0") span with purlins at 6'- 8" o.c. center purlin and (2) purlins each side of center purlin need lateral bracing. Example: Max. 'L' for $2" \times 4" \times 0.050"$ hollow section with 'W' = 5'-0" = 11'-4"

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Table 1.3 Allowable Post / Upright Heights For Primary Screen Wall Frame Members Aluminum Alloy 6063 T-6 For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.*

e	Tributary Load Width 'W' = Upright Spacing														
Hollow Sections	3'-0'	•	4'-0'	•	5'-0	H	6'-0'		7'-0"		8'-0"		9'-0'	110	
	Allowable Height 'H' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	8'-4"	Ь	7'-3"	ь	6'-6"	Ь	5'-11"	ь	5'-6"	b	5'-1"	h	4'-10"	<u>_</u>	
2" x 2" x 0.055"	9'-1"	b	7'-11"	ь	7'-1"	ь	6'-5"	h	5'-11"	h	5'-7"	5	5'-3"	ᅳ	
2" x 3" x 0.045"	11'-3"	Ь	9'-9"	ь	8'-9"	ь	7'-11"	h	7'-5"	h	6'-11"	늯	6'-6"	- <u>b</u>	
2" x 4" x 0.050"	12'-5"	ь	10'-9"	Ь	9'-7"	ь	8'-9"	<u> </u>	8'-1"	ᡖ	7'-7"	퓠	7'-2"	ᅳ	

				Tri	butary L	oa(d Width '	<i>N</i> ' :	= Upright	S	pacing	_			
Self Mating Sections	3'-0"		4'-0"	4'-0"		5'-0"		1	7'-0"		8'-0"		9'-0"		
	Allowable Height 'H' / bending 'b' or deflection 'd'														
2" x 4" x 0.044 x 0.100"	16'-11"	b	14'-8"	b	13'-1"	b	11'-11"	b	11'-1"	b	10'-4"	ь	9'-9"	b	
2" x 5" x 0.050" x 0.100"	20'-11"	b	18'-1"	b	16'-2"	Ь	14'-9"	b	13'-8"	<u>_</u>	12'-10"	b	12'-1"		
2" x 6" x 0.050" x 0.120"	24'-2"	b	20'-11"	Ь	18'-9"	b	17'-1"	b	15'-10"	b	14'-10"	b	13'-11"	- b	
2" x 7" x 0.055" x 0.120"	27'-3"	b	23'-7"	b	21'-1"	b	19'-3"	b	17'-10"	b	16'-8"	ь	15'-9"	౼	
2" x 7" x 0.055" w/ insert	36'-3"	ь	31'-4"	b	28'-1"	b	25'-7"	ь	23'-9"	ь	22'-2"	b	20'-11"	- <u>b</u>	
2" x 8" x 0.072" x 0.224"	35'-2"	Ь	30'-6"	b	27'-3"	Ь	24'-10"	ь	23'-0"	ь	21'-6"	ь	20'-4"	౼	
2" x 9" x 0.072" x 0.224"	38'-2"	Ь	33'-0"	Ь	29'-6"	b	26'-11"	b	24'-11"	b	23'-4"	h	22'-0"	- <u>b</u>	
2" x 9" x 0.082" x 0.310"	41'-10"	Ь	36'-3"	b	32'-5"	ь	29'-7"	ь	27'-5"	ь	25'-8"	긁	24'-2"	<u> </u>	
2" x 10" x 0.092" x 0.369"	50'-4"	Ь	43'-7"	b	38'-11"	ь	35'-7"	Ы	32'-11"	ь	30'-10"	葥	29'-1"	౼	

	Tributary Load Width 'W'= Upright Spacing													
Snap Sections	3'-0'	•	4'-0"		5'-0'		6'-0'	_	7'-0"		8'-0"		9'-0'	7
			Al	lov	able He	ight	'H' / ber	ndin	g 'b' or c	iefl	ection 'd	•		-
2" x 2" x 0.044"	9'-11"	b	8'-7"	Ъ	7'-8"	h	7'-0"	h	6'-6"	h	6'-1"	_		_
2" x 3" x 0.045"	12'-9"	Ь	11'-0"	h	9'-10"	<u> </u>	9'-0"	-	8'-4"	는		<u>.</u>	5'-9"	b
2" x 4" x 0.045"	15'-7"	÷		Ë		÷		<u>.</u>		D	7'-10"	Ð	7'-4"	_ <u>b</u>
		D	13'-6"	D	12'-1"	b	11'-0"	b	10'-2"	b	9'-7"	b	8'-11"	h
2" x 6" x 0.062"	26'-5"	b	22'-10"	ь	20'-5"	Ь	18'-8"	b	17'-3"	h	16'-2"	L	15'-3"	픈
2" x 7" x 0.062"	29'-5"	b	25'-5"	h	22'-9"		20'-9"	÷		÷		<u>D</u>		
For allowable beliebte at ad-		_			22-8	Q	20'-9"	0	19'-3"	D	17'-11"	b	16'-11"	ь

For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specification page for tables at the beginning of this section and example below. Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- 3. Above heights do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.

7. Heights may be interpolated.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

IF SPANS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

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Table 1.4 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights For Secondary Screen Wall Frame Members

Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.*

A. Sections As Horizontals Fastened To Posts With Clips

	Tributary Load Width 'W'													
Hollow Sections	3'-6'		4'-0'		4'-6'	10	5'-0"	•	5'-6'	1	6'-0'	•	6'-8'	•
			Al	lowa	ble Hei	ghts	'H' / be	ndin	g 'b' or	defi	ection '	d'		
2" x 2" x 0.044"	6'-10"	d	6'-6"	ь	6'-1"	ь	5'-9"	Ы	5'-6"	ь	5'-3"	Ь	5'-0"	h
2" x 2" x 0.055"	7'-3"	d	6'-11"	d	6'-8"	ъ	6'-4"	ь	6'-0"	ь	5'-9"	ь	5'-6"	ᇴ
3" x 2" x 0.045"	7'-9"	व	7'-5"	d	7'-1"	ď	6'-10"	a	6'-7"	ь	6'-4"	ь	5'-11"	౼
2" × 3" × 0.045"	9'-4"	Ы	8'-9"	ь	8'-3"	ь	7'-10"	Б	7'-5"	ь	7'-2"	ĥ	6'-9"	늄
2" x 4" x 0.050"	10'-3"	Ы	9'-7"	ь	9'-0"	ь	8'-7"	Б	8'-2"	<u> </u>	7'-10"	5	7'-5"	౼

Snap Sections			A	lowabie Hei	ghts	'H' / be	ndin	g 'b' or	defl	ection '	ď'		\neg
2" x 2" x 0.044"	7'-6"	ď		d 6'-11"				6'-4"	b	6'-1"	ь	5'-9"	ь

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

	2.82				Tr	ibut	ary Loa	d W	idth 'W'					
Hollow Sections	3'-6'		4'-0"		4'-6'		5'-0'	1	5'-6'	10	6'-0'	- 1	6'-8	11
			Alla	owa	ble Hei	ghts	'H' / be	ndi	g 'b' or	def	ection '	ď		
2" x 2" x 0.044"	7'-9"	b 7	''-3 "	Ы	6'-10"	b	6'-6"	ь	6'-2"	b	5'-11"	ы	5'-7"	_h
2" x 2" x 0.055"	8'-5"	b 7'	-11"	Ы	7'-5"	Ы	7'-1"	ь	6'-9"	ь	6'-5"	ᆔ	6'-1"	౼
3" x 2" x 0.045"	9'-3"	b 8	'-8"	Ы	8'-2"	ь	7'-9"	b	7'-5"	- ĥ	7'-1"	ᆔ	6'-8"	౼
2" x 3" x 0.045"	10'-5"	b 9)'-9 "	Ы	9'-2"	ь	8'-9"	Б	8'-4"	葋	7'-11"	ᆔ	7'-7"	౼
2" x 4" x 0.050"	11'-6"	b 10	0'-9"	Ы	10'-1"	Б	9'-7"	b	9'-2"	Ь	8'-9"	崩	8'-4"	<u> </u>
Snap Sections		59	Alic	wa		hts		ndir		defi	ection 'c	10 1	0-4	٥
2" x 2" x 0.044"	9'-2"	6 I 8	1-7"	ы	8'-1"	hI		ь	7'-4"	P	יוסווסוו ל	-	C1 0#	
	9'-2"	Ы 8	Alic '-7"	b	8'-1"	jnts b	7'-8"	ndir b	ig 'b' or 7'-4"	defl b	ection 'c 7'-0"	<u>в</u>	6'-	8*

* For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specifications for tables page at the beginning of this section and example below. Notes:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 4. Span is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. Into the screw bosses and do not exceed 8'-0" in span.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Spans may be interpolated.

IF HEIGHTS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

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SECTION 1

Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam	Upright	Minimum Purlin, Girt		Notes	Minimu	ım Number of	Screws*	Beam Stitching
Size	Size	& Knee Brace Size**	Anchors		#8 x 1/2"	#10 x ½"	#12 x ½"	Screw @ 24" O.C
2" x 3"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	6	4	4	1371 ==
2" x 4"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#8
2" x 4"	2" x 4"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#10
2" x 5"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	- 8	6	4	#8
2" x 6"	2" x 3"	2" x 2" x 0.044"	4	Full Lap	10	8	6	#10
2" x 6"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	10	8	6	#10
2" x 7"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	.14	12	10	#12
2" x 8"	2" x 5"	2" x 3" x 0.044"	6	Partial Lap	16	14	12	#14
2" x 9"	2" x 6"	2" x 3" x 0.045"	6	Partial Lap	18	16	14	#14
2" x 9"**	2" x 7"	2" x 4" x 0.050"	8	Partial Lap	20	18	16	#14
2" x 10"	2" x 8"	2" x 4" x 0.050"	10	Partial Lap	20	18	16	#14

Screw Size	Minimum Distance and	Spacing of Screws	Gusset Plate Thickness					
	Edge To Center	Center To Center	Beam Size	Thickness				
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	1/16" = 0.063"				
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	1/8" = 0.125"				
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	1/8" = 0.125"				
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0306"	1/8" = 0.125"				
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	1/4" = 0.25"				
3/8"	1"	2"	- 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	177 - 0.23				

Refers to each side of the connection of the beam and upright and each side of splice connection.

** 0.082" wall thickness, 0.310" flange thickness

Note:

- 1. Connection of 2" x 6" to 2" x 3" shall use a full lap cut or 1/16" gusset plate.
- 2. All gusset plates shall be a minimum of 5052 H-32 Alloy or have a minimum yield strength of 23 ksi.
- 3. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 4. The number of screws is based on the maximum allowable moment of the beam.
- 5. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 6. Hollow splice connections can be made provided the connection is approved by the engineer.
- 7. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the
- 8. All beam to upright connections for 2" x 7" beams or larger shall have an internal or external gusset plates. Gusset plates are required for mansard or gable splice connections.
- 9. For gusset plate connections 2" x 9" beams or larger use 3/4" long screws.
- 10. The side wall upright shall have a minimum beam size as shown above, le., a 2" x 4" upright shall have a 2" x 3" beam.
- 11. Connect beam to upright w/ H-bar, gusset plate, or angle clips for each splice with 1/2 the screws on each side of the cut.

Table 1.7

Minimum Size Screen Enclosure Knee Braces **And Anchoring Required** Aluminum 6063 T-6

Brace Length	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 4'-6"	2" x 4" x 0.044" x 0.12"	2" H-Channel With (4) 3/4" long screws (size to be determined by beam size, see table 9.6)

(See Table 1.6 For Number And Size Of Screws)

Note:

- For required knee braces greater than 4'-6" contact engineer for specifications and details.
- 2. Cantilever beam detail shown on page 1-32 shall be used for host structure attachment when knee brace length exceeds 4'-6".

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Warranty Deed

Made this October 18, 2005 A.D.

By Michael Guilmette and Cindy L. Guilmette, husband and wife, whose address is: 225 NW Scenic Lake Drive, Lake City, Florida 32025, hereinafter called the grantor,

to Bruce W. Eichman and Patsy A. Eichman, husband and wife, whose post office address is: 225 NW Scenic Lake Drive, Lake City. FL 32025, hereinafter called the grantee:

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, (\$10.00) and other valuable considerations. receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

Lot 1, of Woodborough, Phase 8, according to the Plat thereof, as recorded in Plat Book 6, at Page 215, of the Public Records of Columbia County, Florida

Parcel ID Number: R02268-601

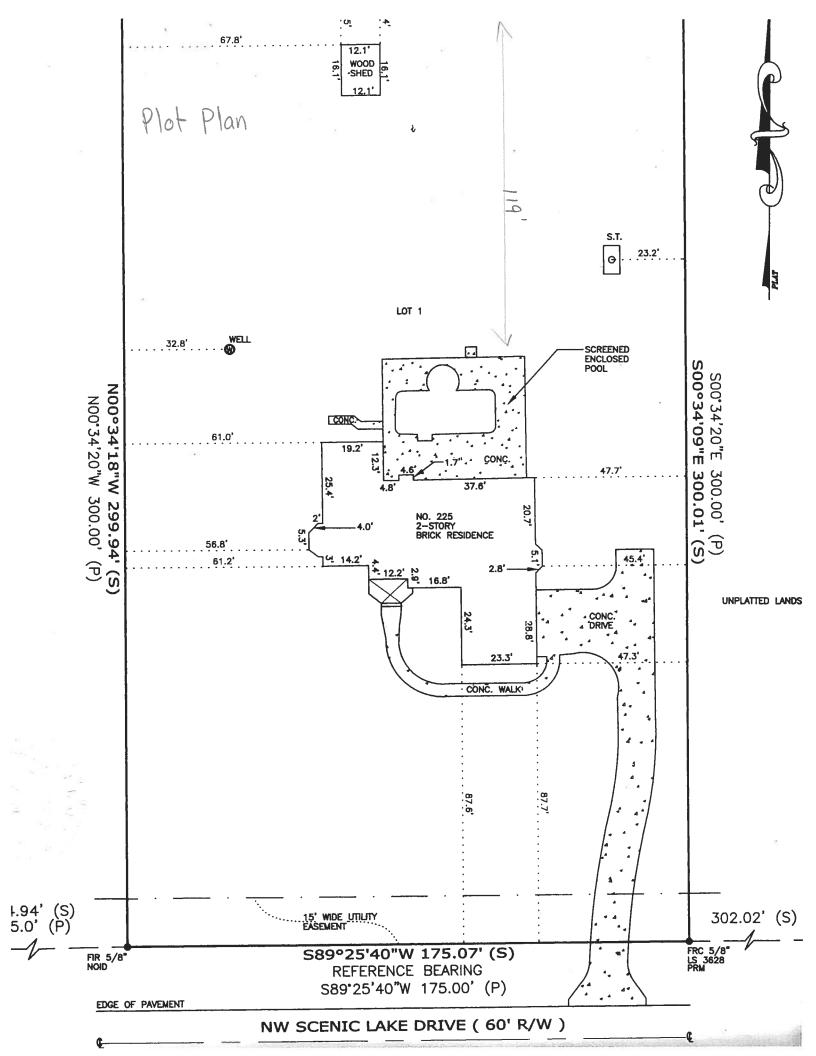
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

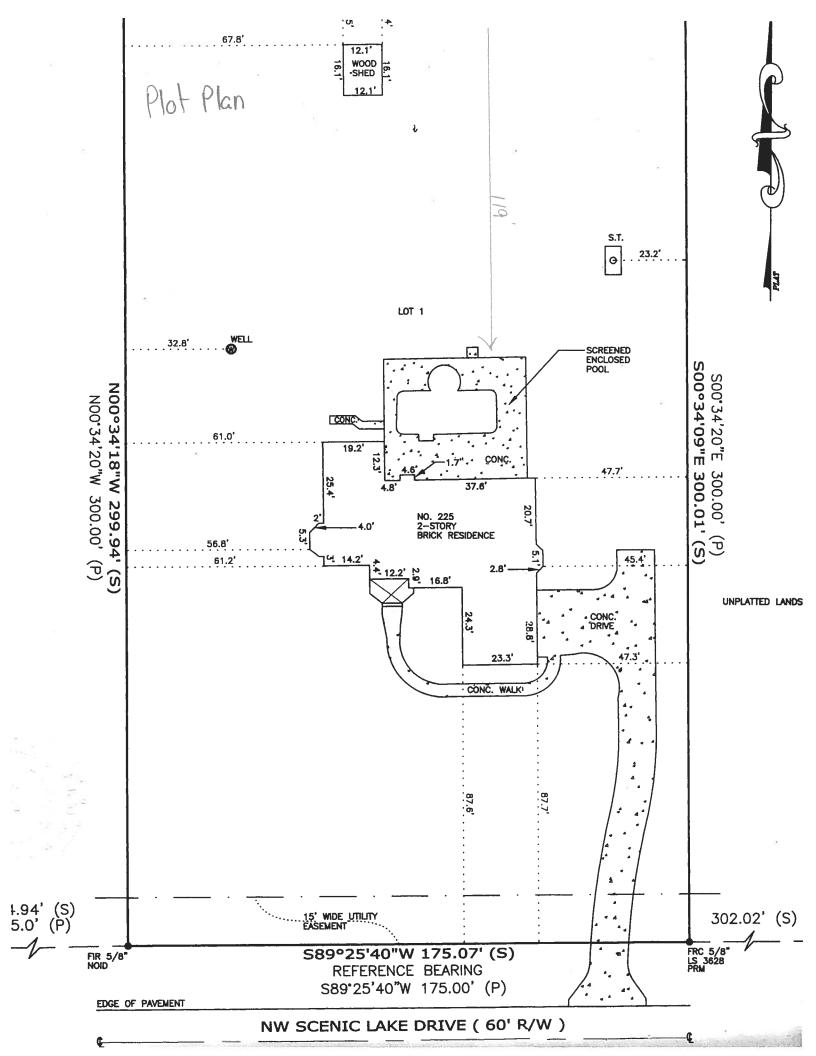
To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

In Witness Witerent, the said glantor has signed and other	
Signed, sealed and delivered in our presence:	
	(Seal)
Michael Guilmette Address 225 NW seenic Lake Drive, Lake City, Florida	32025
Witness Printed Name 1, 22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
White of the war I would be a second	(Seal)





NOTICE OF COMMENCEMENT

PERMIT NUMBER:
STATE OF: FLORIDA COUNTY OF: CITY OF:
THE UNDERSIGNED HEREBY gives notice that improvement(s) will be made to certain real property, and in accordance
with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
DESCRIPTION OF PROPERTY
LOT: BLOCK: SECTION: 23 TOWNSHIP: 35 RANGE: 16
TAX PARCEL NUMBER: 23-35-16-02268 - 601
SUBDIVISION: Woodhurrauch PLATROOK MAR BACE.
STREET ADDRESS: 225 N.W Scenic Lake Dr. Lake City, Fla. 32025
GENERAL DESCRIPTION OF IMPROVEMENTS
TO CONSTRUCT: Screen Enclosure
OWNER INFORMATION
OWNER NAME: Bruce W+ Patsy Eleman
ADDRESS: 225 N.W. Scenic Lake Dr. PHONE NUMBER 386-758-6917
CITY: Lake City STATE: Fla. ZIP CODE: 32025
INTEREST IN PROPERTY:
FEE SIMPLE TITLEHOLDER NAME:
FEE SIMPLE TITLEHOLDER ADDRESS:
(if other than owner)
CONTRACTOR NAME: Richardson Alyminum LL.C.
ADDRESS: 692 S.W. Arlington Blud. PHONE NUMBER: 386-755-5779
CITY: Lake City STATE: Flg. ZIP CODE: 32025
BONDING COMPANY:
ADDRESS:PHONE NUMBER:
CITY:STATE:ZIP CODE:
LENDER NAME: Inst:2007002755 Date:02/05/2007 Time:12:51
ADDRESS:DC,P.DeWitt Cason,Columbia County B:1109 P:2228
CITY: STATE: ZIP CODI
Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as
provided by Section 713.13(1)(a) 7., Florida Statutes:
NAME:ADDRESS:
In addition to himself, Owner designates
of to receive a copy of Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
Expiration date is one (1) year from date of recording unless a different date is specified.
SIGNATURE OF OWNER: Device Carl
SWORN to and subscribed before me this 5 day of February, A.D. 2004.
Notary Public DV 1
My commission Expires: AMY MARTS MY COMMISSION # DD458730 EXPIRES: Aug. 7, 2009 (407) 398-0153 Florids Notary Service com